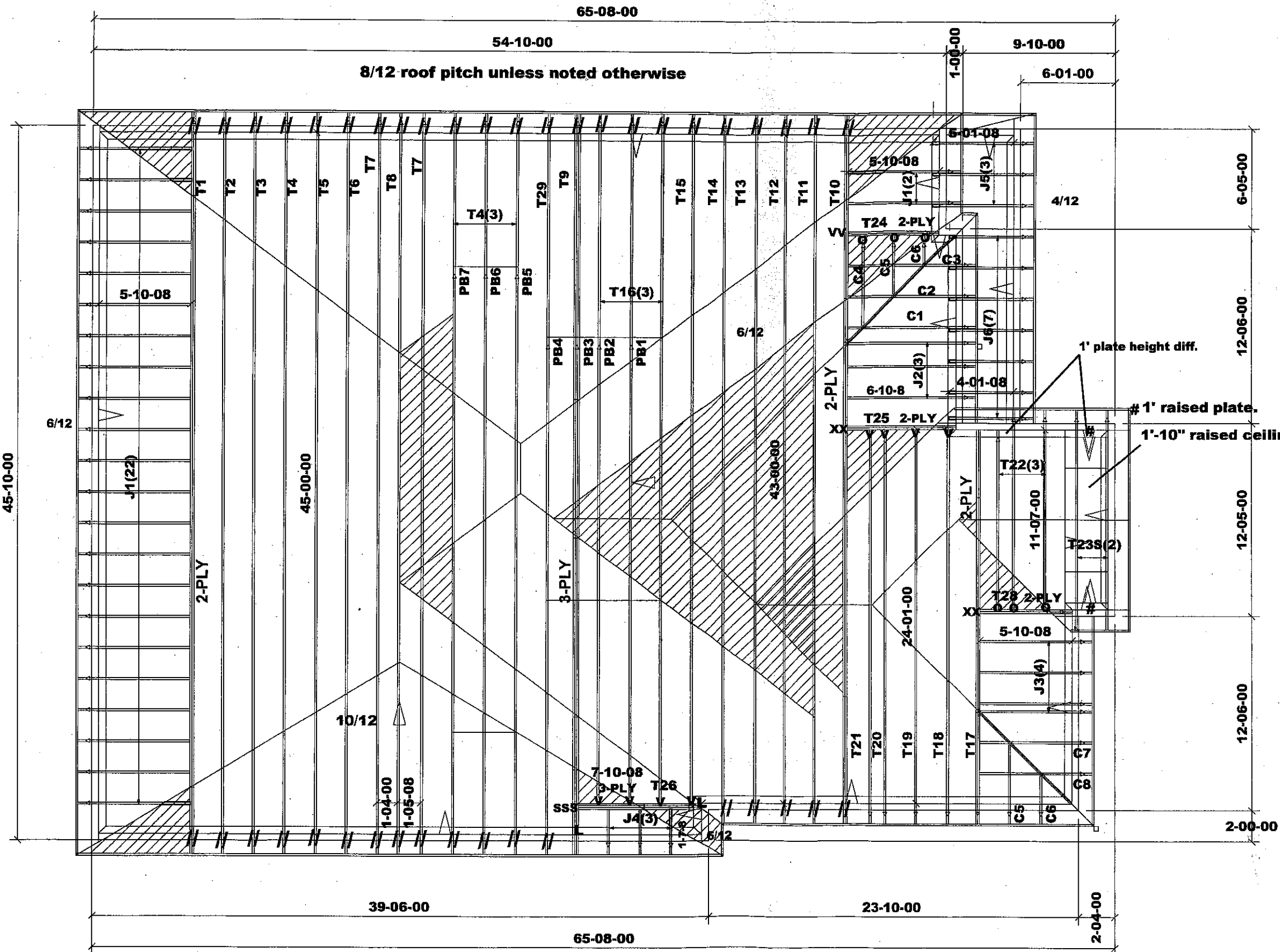




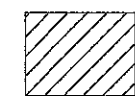
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

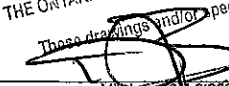
All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TC SL = 25.6 psf
TCDL = 6.0 psf
BC LL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 (VV)
HGUS28-3 - (SSS)
LGT3-SDS2.5 - (L)
H2.5A - (I)

 DENOTES: CONVENTIONAL FRAMING

CITY OF HAMILTON
Building Division
Permit No. **21-161913**
THESE STAMPED DRAWINGS SHALL BE AVAILABLE ON SITE
THE OWNER AND/OR CONTRACTOR SHALL COMPLY WITH THE ONTARIO BUILDING CODE AND ALL OTHER APPLICABLE LAW
These drawings and/or specifications have been reviewed by

FOR CHIEF BUILDING OFFICER
DATE **JAN 24/22**

M14156

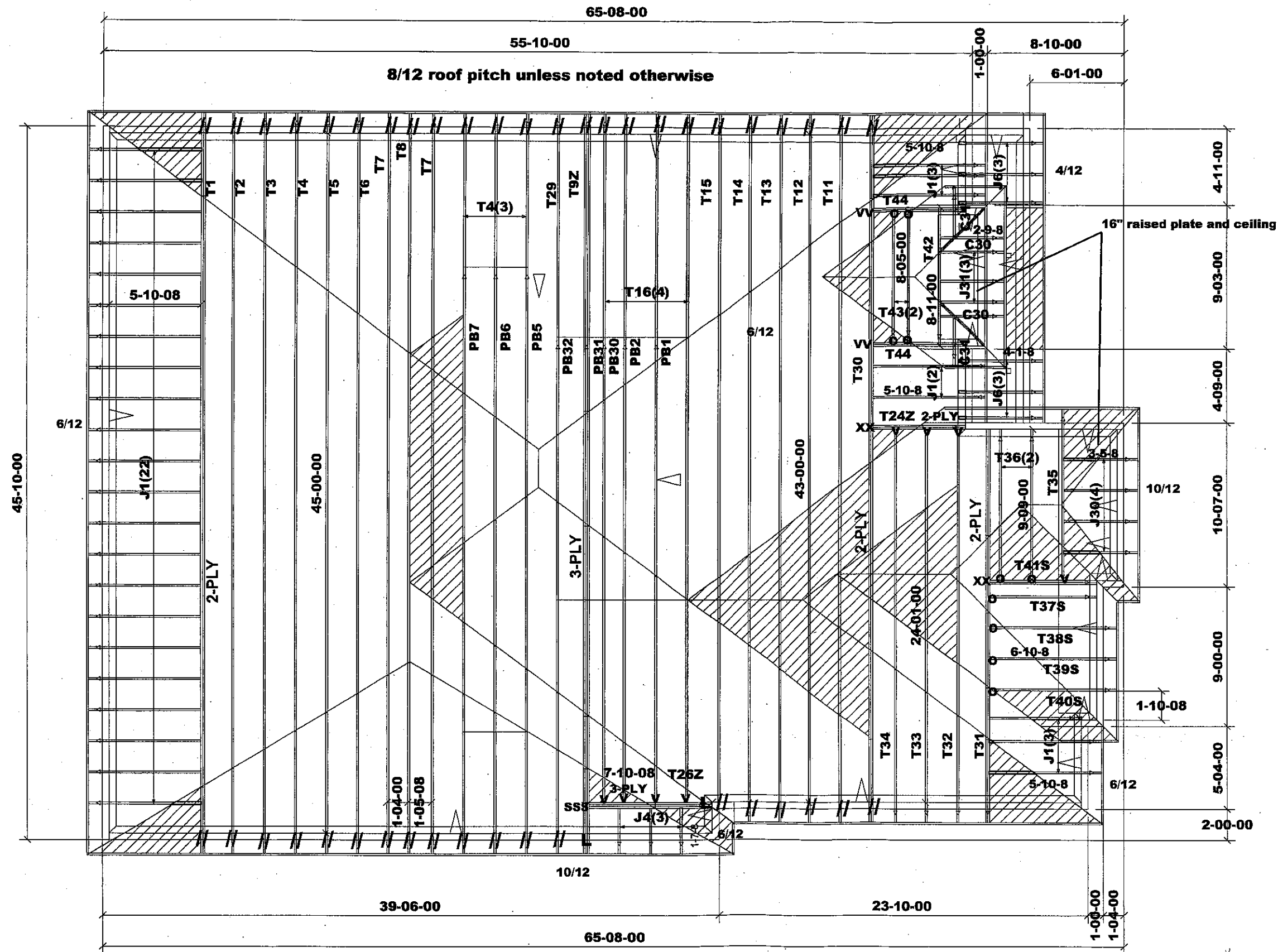


Job Track: **52166**
Plan Log: **204651**
Layout ID: **418361**

Builder / Location: **GREENPARK HOMES / HAMILTON**
Project: **RUSSELL GARDENS PH.4**
Date: 2021-07-27 Sales: **William Garcia** Designer: **JG**

Model / Elevation: **GRANDVILLE 9/1**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TC SL = 25.6 psf
TC DL = 6.0 psf
BC LL = 0.0 psf
BC DL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 (VV)
HGUS28-3 -(SSS)
H2.5A-(/)
LGT3-SDS2.5-(L)

 DENOTES: CONVENTIONAL FRAMING

M14156



Job Track: **52166**
Plan Log: **204651**
Layout ID: **418362**

Builder / Location:
GREENPARK HOMES / HAMILTON

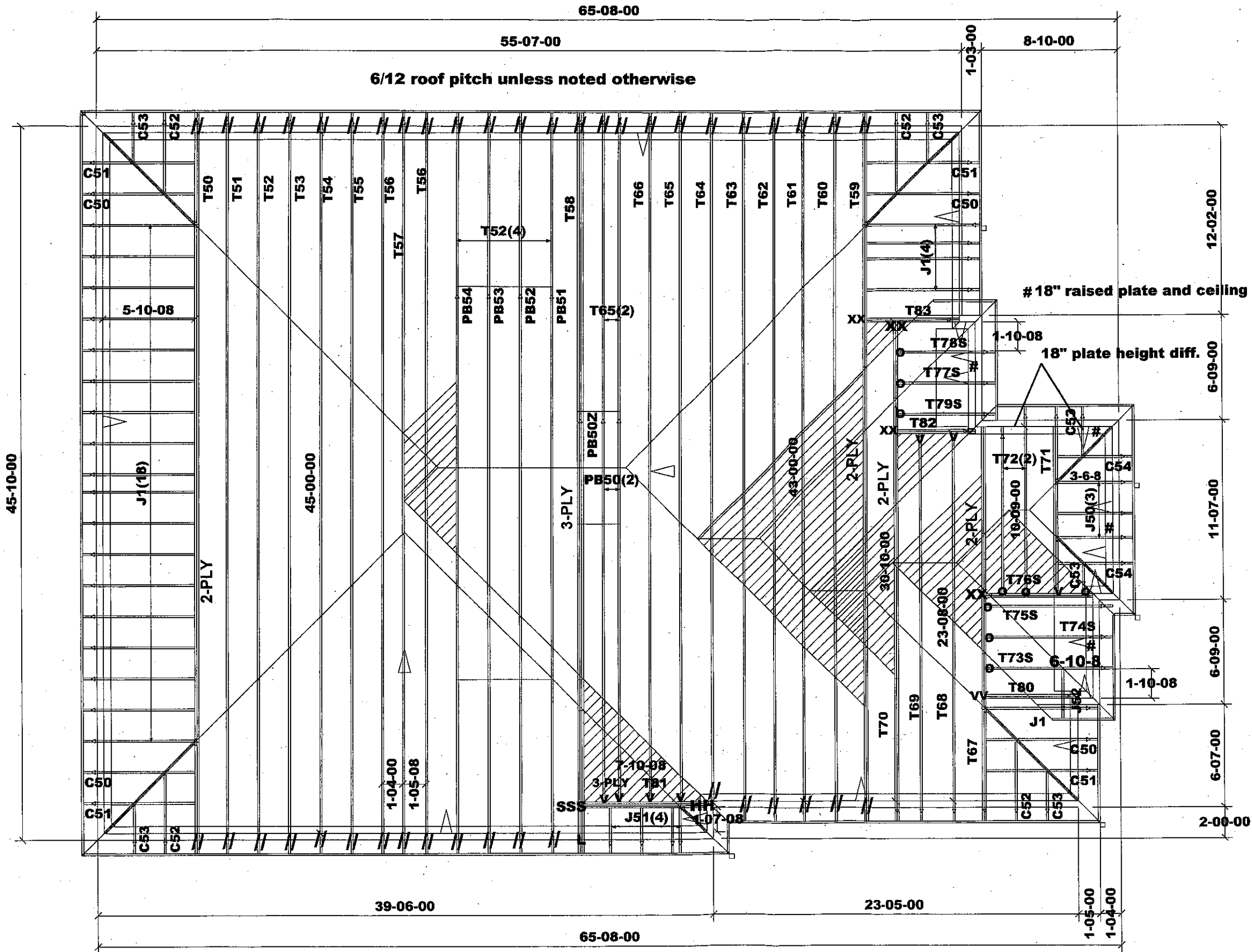
Project: **RUSSELL GARDENS PH.4**

Date: 2021-07-27 Sales: William Garcia Designer: JG

Model / Elevation:
GRANDVILLE 9/2

Mitek ver 8.4.2.286

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



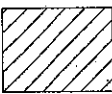
ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.35 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
LUS26-2 (VV)
HGUS28-3 -(SSS)
H2.5A-(/)
H8- (H)
LGT3-SDS2.5-(L)

 DENOTES: CONVENTIONAL FRAMING

M14156



Job Track: **52166**
Plan Log: **204651**
Layout ID: **418363**

Builder / Location:
GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

Date: 2021-07-27

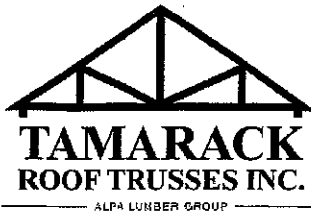
Sales: **William Garcia**

Designer: **JG**

Model / Elevation:
GRANDVILLE 9/3

Mitek ver 8.4.2.286

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



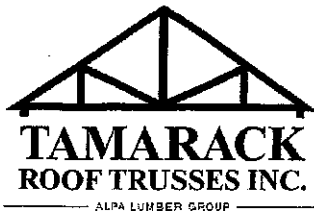
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 1

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418361
 Ref #
 Page: 1 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Roof Special Girder	8 /12	45-00-00	4-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	509.35 308.67		
	1	T2 Roof Special	8 /12	45-00-00	5-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	260.69 157.33		
	1	T3 Roof Special	8 /12	45-00-00	6-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	272.6 168.00		
	4	T4 Roof Special	8 /12	45-00-00	7-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	1114.75 680.00		
	1	T5 Roof Special	8 /12	45-00-00	8-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	292.95 176.67		
	1	T6 Roof Special	8 /12	45-00-00	9-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	293.51 177.33		
	2	T7 Roof Special	8 /12	45-00-00	10-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	608.9 362.67		
	1	T8 Roof Special	8 /12	45-00-00	10-09-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	312.04 187.00		
	1 3-ply	T9 Piggyback Base Girder	8 /12	45-00-00	10-01-11	2 x 6 2 x 8	1-03-08 1-03-08	1-04-13 1-07-11	955.77 579.00		
	1 2-ply	T10 Roof Special Girder	8 /12	43-00-00	5-11-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	531.27 326.00		
	1	T11 Hip	8 /12	43-00-00	5-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	250.47 153.33		
	1	T12 Hip	8 /12	43-00-00	6-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	253.24 154.00		
	1	T13 Hip	8 /12	43-00-00	7-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	264.71 161.33		
	1	T14 Hip	8 /12	43-00-00	8-02-06	2 x 6	1-03-08 1-05-00	1-04-13 1-04-13	272.9 165.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 1

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418361
 Ref #
 Page: 2 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T15 Hip	8 / 12	43-00-00	9-01-06	2 x 6	1-03-08	1-04-13 1-04-13	280.68 170.00		
	3	T16 Hip	8 / 12	43-00-00	10-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	828.29 487.00		
	1 2-ply	T17 Hip Girder	8 / 12	24-01-00	5-03-13	2 x 4 2 x 6	1-03-08	1-04-13 2-04-13	224.91 140.67		
	1	T18 Hip	8 / 12	24-01-00	6-07-13	2 x 4	1-03-08	1-04-13 2-04-13	106.77 58.67		
	1	T19 Hip	8 / 12	24-01-00	7-11-13	2 x 4	1-03-08	1-04-13 2-04-13	108.17 68.83		
	1	T20 Hip	8 / 12	24-01-00	9-03-13	2 x 4	1-03-08	1-04-13 2-04-13	115.55 72.00		
	1	T21 Common	8 / 12	24-01-00	9-11-02	2 x 4	1-03-08	1-04-13 2-04-13	104.07 65.00		
	3	T22 Common	8 / 12	11-07-00	6-03-02	2 x 4		2-04-13 2-04-13	143.72 91.00		
	2	T23S Roof Special	8 / 12 5 / 12	11-07-00	5-03-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	102.48 68.67		
	1 2-ply	T24 Jack-Closed Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T25 Jack-Closed Girder	8 / 12	6-10-08	5-11-13	2 x 4 2 x 6		1-04-13 5-11-13	74.36 47.33		
	1 3-ply	T26 Half Hip Girder	6 / 12	7-10-08	2-11-15	2 x 4 2 x 8		1-02-00 2-11-15	121.59 82.00		
	1 2-ply	T28 Jack-Closed Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.23 40.67		
	1	T29 Piggyback Base	8 / 12	45-00-00	10-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	295.84 177.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 1

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418361
 Ref #
 Page: 3 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	PB1 Piggyback	8 /12	16-10-11	1-00-00	2 x 4			38.5 24.83		
	1	PB2 Piggyback	8 /12	16-10-11	2-00-00	2 x 4			49.92 32.33		
	1 3-ply	PB3 Piggyback	8 /12	16-10-11	2-08-00	2 x 4			154.27 95.00		
	1	PB4 Piggyback	8 /12	16-10-11	3-08-00	2 x 4			49.19 30.50		
	1	PB5 Piggyback	8 /12	25-10-11	7-04-12	2 x 4			96.03 60.00		
	1	PB6 Piggyback	8 /12	29-10-11	6-04-12	2 x 4			102.22 65.00		
	1	PB7 Piggyback	8 /12	29-10-11	5-04-12	2 x 4			106.08 67.83		
	24	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	403.07 256.00		
	3	J2 Jack-Open	8 /12	6-10-08	5-11-13	2 x 4	1-03-08	1-04-13 5-11-13	82.78 53.00		
	4	J3 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	78.3 48.67		
	3	J4 Jack-Open	10 /12	1-07-08	2-11-15	2 x 4	1-03-08	1-07-11 2-11-15	27.37 21.00		
	3	J5 Jack-Open	4 /12	5-01-08	2-05-06	2 x 4	1-03-08	3-15 2-00-07	41 26.00		
	7	J6 Jack-Open	4 /12	4-01-08	2-01-06	2 x 4	1-03-08	3-15 1-08-07	79.28 51.33		
	1	C1 Jack-Open	8 /12	5-09-07	5-03-02	2 x 4	1-03-08 1-01-01	1-04-13 5-03-02	20.59 12.83		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 1

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418361
 Ref #
 Page: 4 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	C2 Jack-Open	8 / 12	3-09-07	3-11-02	2 x 4	1-03-08 3-01-01	1-04-13 3-11-02	17.85 11.50		
	1	C3 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 5-01-01	1-04-13 2-07-02	14.61 9.00		
	1	C4 Jack-Open	8 / 12	1-10-08	5-03-02	2 x 4	3-10-15	1-04-13 2-07-13	12.68 8.33		
	2	C5 Jack-Open	8 / 12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.42 15.33		
	2	C6 Jack-Open	8 / 12	1-10-08	2-07-02	2 x 4		1-04-13 2-07-13	14.39 10.00		
	1	C7 Jack-Open	8 / 12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	16.71 10.83		
	1	C8 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	13.47 8.33		

TOTAL # TRUSS= 110

TOTAL BFT OF ALL TRUSSES= 6288.14

BFT.

TOTAL WEIGHT OF ALL TRSSES 10263.19 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	HGUS28-3	
2		LGT3-SDS2.5	
8	Hardware	LJS26DS	
1	Hardware	LUS26-2	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 19



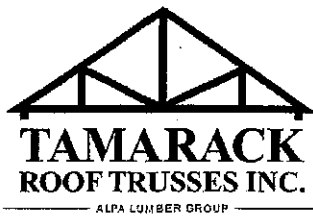
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 2

Job Track: 52166
 Plan Log: 204651
 Layout ID: 418362
 Ref #
 Page: 1 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Roof Special Girder	8/12	45-00-00	4-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	509.35 306.67		
	1	T2 Roof Special	8/12	45-00-00	5-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	260.69 157.33		
	1	T3 Roof Special	8/12	45-00-00	6-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	272.6 168.00		
	4	T4 Roof Special	8/12	45-00-00	7-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	1114.75 680.00		
	1	T5 Roof Special	8/12	45-00-00	8-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	292.95 176.67		
	1	T6 Roof Special	8/12	45-00-00	9-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	293.51 177.33		
	2	T7 Roof Special	8/12	45-00-00	10-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	608.9 362.67		
	1	T8 Roof Special	8/12	45-00-00	10-09-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	312.04 187.00		
	1 3-ply	T9Z Piggyback Base Girder	8/12	45-00-00	10-01-11	2 x 6 2 x 8	1-03-08 1-03-08	1-04-13 1-07-11	955.77 579.00		
	1	T11 Hip	8/12	43-00-00	5-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	250.47 153.33		
	1	T12 Hip	8/12	43-00-00	6-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	253.24 154.00		
	1	T13 Hip	8/12	43-00-00	7-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	257.94 157.33		
	1	T14 Hip	8/12	43-00-00	8-02-06	2 x 6	1-03-08 1-05-00	1-04-13 1-04-13	272.9 165.33		
	1	T15 Hip	8/12	43-00-00	9-01-06	2 x 6	1-03-08	1-04-13 1-04-13	280.68 170.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 2

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418362
 Ref #
 Page: 2 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	T16 Hip	8 /12	43-00-00	10-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	1104.37 649.33		
	1 2-ply	T24Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 3-ply	T26Z Half Hip Girder	6 /12	7-10-08	2-11-15	2 x 4 2 x 6		1-02-00 2-11-15	110.29 74.00		
	1	T29 Piggyback Base	8 /12	45-00-00	10-01-11	2 x 6	1-03-08 1-03-08	1-04-13 1-07-11	295.84 177.00		
	1 2-ply	T30 Hip Girder	8 /12	43-00-00	4-01-06	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	490.37 298.67		
	1 2-ply	T31 Roof Special Girder	8 /12	24-01-00	7-03-13	2 x 4 2 x 6	1-03-08	1-04-13 2-08-13	250.48 159.67		
	1	T32 Hip	8 /12	24-01-00	5-01-04	2 x 4	1-03-08	1-04-13 2-08-13	101.22 64.17		
	1	T33 Hip	8 /12	24-01-00	6-01-04	2 x 4	1-03-08	1-04-13 2-08-13	101.81 64.33		
	1	T34 Hip	8 /12	24-01-00	7-01-04	2 x 4	1-03-08	1-04-13 2-08-13	112.41 72.00		
	1	T35 Hip Girder	8 /12	9-09-00	4-06-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	48.17 29.67		
	2	T36 Common	8 /12	9-09-00	5-11-13	2 x 4		2-08-13 2-08-13	85.36 58.00		
	1	T37S Jack-Closed	8 /12	6-10-08	7-03-13	2 x 4		1-04-13 7-03-13	42.43 27.67		
	1	T38S Half Hip	8 /12	6-10-08	6-07-13	2 x 4	1-03-08	1-04-13 6-07-13	47.36 30.67		
	1	T39S Half Hip	8 /12	6-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	44.59 29.00		



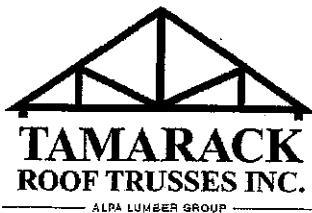
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 2

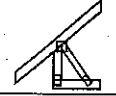
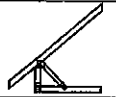
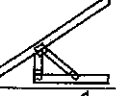
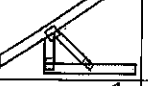
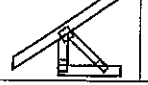
Job Track: 52166
 PlanLog: 204651
 Layout ID: 418362
 Ref #
 Page: 3 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T40S Half Hip	8 /12	6-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	38.61 26.17		
	1 2-ply	T41S Jack-Closed Girder	8 /12	6-10-08	7-03-13	2 x 4 2 x 6		1-04-13 7-03-13	97.58 64.00		
	1	T42 Hip Girder	8 /12	8-11-00	3-03-02	2 x 4 2 x 6		1-04-13 1-04-13	41.16 27.33		
	2	T43 Common	8 /12	8-05-00	5-08-08	2 x 4		2-10-13 2-10-13	77.36 52.00		
	2 2-ply	T44 Flat Girder	0 /12	5-10-08	1-04-00	2 x 6		1-04-00 1-04-00	106.11 67.33		
	1	PB1 Piggyback	8 /12	16-10-11	1-00-00	2 x 4			38.17 24.17		
	1	PB2 Piggyback	8 /12	16-10-11	2-00-00	2 x 4			49.92 32.33		
	1	PB5 Piggyback	8 /12	25-10-11	7-04-12	2 x 4			94.53 59.00		
	1	PB6 Piggyback	8 /12	29-10-11	6-04-12	2 x 4			99.85 63.33		
	1	PB7 Piggyback	8 /12	29-10-11	5-04-12	2 x 4			106.08 67.83		
	1	PB30 Piggyback	8 /12	16-10-11	2-08-00	2 x 4			50.84 31.33		
	1 3-ply	PB31 Piggyback	8 /12	16-10-11	3-02-00	2 x 4			155.33 100.50		
	1	PB32 Piggyback	8 /12	16-10-11	4-02-00	2 x 4			49.57 31.67		
	30	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	503.84 320.00		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: GRANDVILLE 9 Lot #: Elevation: 2	Job Track: 52166 PlanLog: 204651 Layout ID: 418362 Ref # Page: 4 of 4 Date: 07-26-2021 Designer: Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY	MARK	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	LOAD BY
	PLY	TYPE					LEFT RIGHT	LEFT RIGHT	BFT.	STACK #	REMARKS
	3	J4 Jack-Open	10 / 12	1-07-08	2-11-15	2 x 4	1-03-08	1-07-11 2-11-15	27.37 21.00		
	6	J6 Jack-Open	4 / 12	4-01-08	2-01-06	2 x 4	1-03-08	3-15 1-08-07	67.95 44.00		
	4	J30 Jack-Open	10 / 12	3-05-08	4-06-04	2 x 4	1-03-08	1-07-11 4-06-04	56.53 38.67		
	3	J31 Jack-Open	8 / 12	2-09-08	3-03-02	2 x 4	1-03-08	1-04-13 3-03-02	34.35 23.00		
	2	C30 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 1-00-01	1-04-13 2-07-02	19.9 12.67		
	2	C31 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	17.81 11.33		

TOTAL # TRUSS= 114

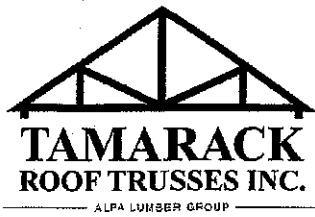
TOTAL BFT OF ALL TRUSSES= 6483.5

BFT. TOTAL WEIGHT OF ALL TRSSES 10560.99 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	HGUS28-3	
2		LGT3-SDS2.5	
8	Hardware	LJS26DS	
10	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 25



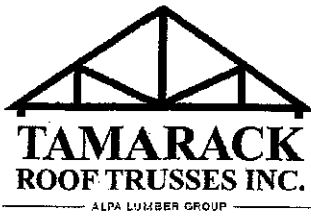
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 3

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418363
 Ref #
 Page: 1 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T50 Hip Girder	6 /12	45-00-00	4-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	477.23 296.00		
	1	T51 Hip	6 /12	45-00-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	241.14 147.33		
	5	T52 Hip	6 /12	45-00-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1265.49 780.00		
	1	T53 Hip	6 /12	45-00-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	260.78 153.33		
	1	T54 Hip	6 /12	45-00-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	265.16 161.33		
	1	T55 Hip	6 /12	45-00-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	257.67 156.67		
	2	T56 Hip	6 /12	45-00-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	547.98 333.33		
	1	T57 Hip	6 /12	45-00-00	10-09-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	289.75 163.67		
	1 3-ply	T58 Piggyback Base Girder	6 /12	45-00-00	10-01-04	2 x 6 2 x 8	1-03-08 1-03-08	1-02-00 1-02-00	952.12 574.00		
	1 2-ply	T59 Roof Special Girder	6 /12	43-00-00	6-01-04	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	440.56 276.33		
	1	T60 Hip	6 /12	43-00-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	232.03 140.67		
	1	T61 Hip	6 /12	43-00-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	234.87 144.00		
	1	T62 Hip	6 /12	43-00-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	241.96 147.33		
	1	T63 Hip	6 /12	43-00-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	242.38 146.67		



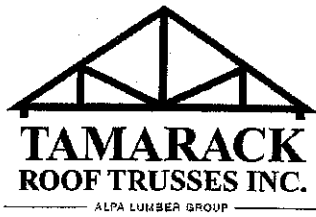
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 3

Job Track: 52166
 PlanLog: 204651
 Layout ID: 418363
 Ref #
 Page: 2 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T64 Hip	6 / 12	43-00-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	249.16 151.33		
	3	T65 Hip	6 / 12	43-00-00	10-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	734.21 448.00		
	1	T66 Hip	6 / 12	43-00-00	11-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	280.8 160.67		
	1 2-ply	T67 Roof Special Girder	6 / 12	24-01-00	6-01-04	2 x 4 2 x 6	1-03-08	1-02-00 2-08-00	240.5 155.00		
	1	T68 Hip	6 / 12	23-08-00	5-01-04	2 x 4	1-03-08	1-02-00 2-10-08	95.01 61.00		
	1	T69 Hip	6 / 12	23-08-00	6-01-04	2 x 4	1-03-08	1-02-00 2-10-08	100.34 64.00		
	1 2-ply	T70 Hip Girder	6 / 12	30-10-00	5-01-04	2 x 4 2 x 6	1-03-08	1-02-00 2-08-00	286.59 181.00		
	1	T71 Hip Girder	6 / 12	10-09-00	2-11-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	50.16 33.67		
	2	T72 Common	6 / 12	10-09-00	5-04-04	2 x 4	1-03-08 1-03-08	2-08-00 2-08-00	94.63 61.00		
	1	T73S Half Hip Girder	6 / 12	6-10-08	3-07-04	2 x 4 2 x 6	1-03-08	1-02-00 3-07-04	42.82 28.67		
	1	T74S Half Hip	6 / 12	6-10-08	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	39.91 26.83		
	1	T75S Half Hip	6 / 12	6-10-08	5-07-04	2 x 4	1-03-08	1-02-00 5-07-04	42.84 29.00		
	1 2-ply	T76S Half Hip Girder	6 / 12	6-10-08	5-11-00	2 x 4 2 x 6		1-02-00 5-11-00	90.02 60.33		
	1	T77S Half Hip	6 / 12	4-10-08	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	32.77 22.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 9
 Lot #:
 Elevation: 3

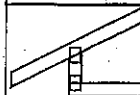
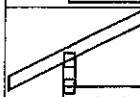
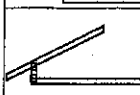
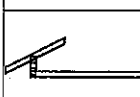
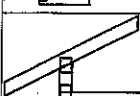
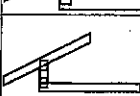
Job Track: 52166
 PlanLog: 204651
 Layout ID: 418363
 Ref #
 Page: 3 of 4
 Date: 07-26-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBs. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T78S Half Hip	6 /12	4-10-08	3-07-04	2 x 4	1-03-08	1-02-00 3-07-04	28.59 20.67		
	1	T79S Jack-Closed	6 /12	4-10-08	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	30.71 21.60		
	1 2-ply	T80 Flat	0 /12	5-10-08	1-06-00	2 x 6		1-06-00 1-06-00	53.44 32.67		
	1 3-ply	T81 Half Hip Girder	6 /12	7-10-08	1-11-12	2 x 4 2 x 6		1-02-00 1-11-12	103.99 89.00		
	1 2-ply	T82 Jack-Closed Girder	6 /12	4-10-08	5-01-04	2 x 4 2 x 6		2-08-00 5-01-04	52.44 33.67		
	1 2-ply	T83 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	59.68 38.00		
	2	PB50 Piggyback	6 /12	7-03-00	1-09-12	2 x 4			32.29 23.33		
	1	PB51 Piggyback	6 /12	23-03-00	5-09-12	2 x 4			74.78 46.33		
	1 3-ply	PB50Z Piggyback	6 /12	7-03-00	1-09-12	2 x 4			49.48 36.00		
	1	PB52 Piggyback	6 /12	25-03-00	5-09-12	2 x 4			76.42 48.33		
	1	PB53 Piggyback	6 /12	25-03-00	5-09-12	2 x 4			81.09 51.33		
	1	PB54 Piggyback	6 /12	25-03-00	5-09-12	2 x 4			80.29 50.33		
	23	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	3	J50 Jack-Open	6 /12	3-06-08	2-11-04	2 x 4	1-03-08	1-02-00 2-11-04	33.48 22.00		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP		DELIVERY SHIPLIST	
		Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: GRANDVILLE 9 Lot #: Elevation: 3	Job Track: 52166 PlanLog: 204651 Layout ID: 418363 Ref # Page: 4 of 4 Date: 07-26-2021 Designer: Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J51 Jack-Open	6 / 12	1-07-08	1-11-12	2 x 4	1-03-08	1-02-00 1-11-12	26.12 18.67		
	1	J52 Jack-Open	6 / 12	1-10-08	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	7.13 4.67		
	4	C50 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C51 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C52 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		
	6	C53 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	42.13 28.00		
	2	C54 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-09-01	1-02-00 2-00-12	17.84 12.00		

TOTAL # TRUSS= 114

TOTAL BFT OF ALL TRUSSES= 5993.32

BFT.

TOTAL WEIGHT OF ALL TRSSES 9656.19 LBS

HARDWARE

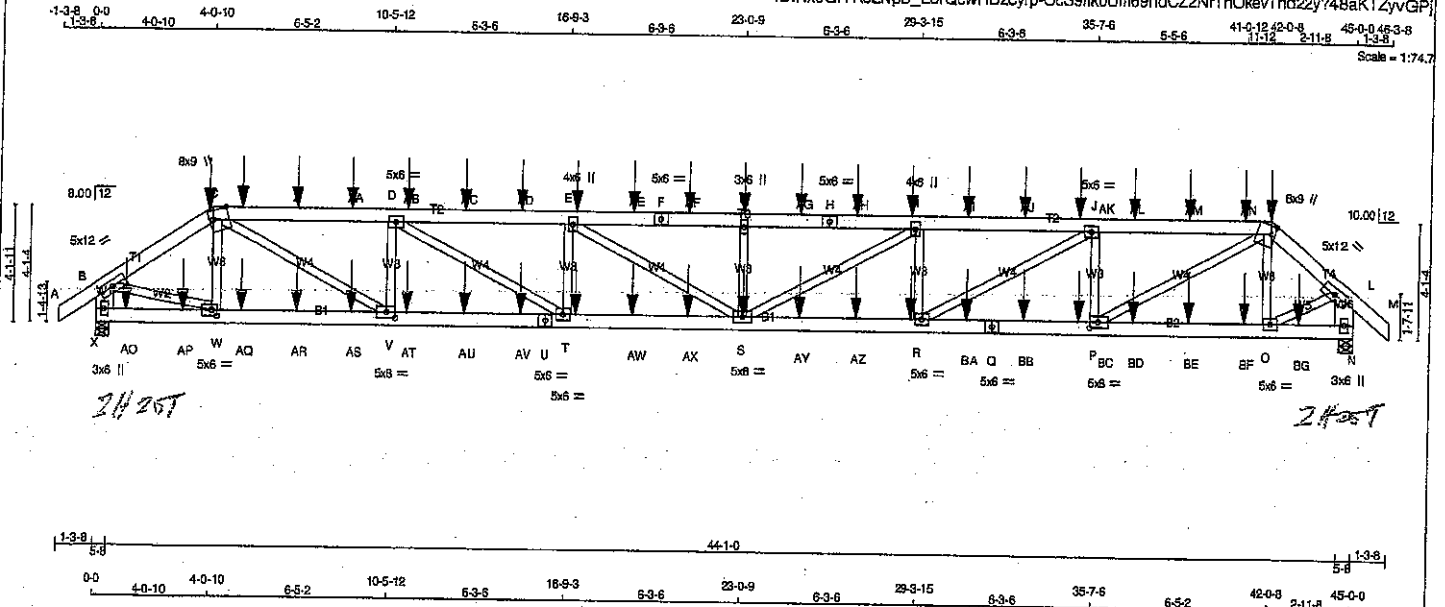
QTY	TYPE	MODEL	LENGTH
4	Hardware	HGUS26-2	
1	Hardware	HGUS28-3	
7	Hardware	LJS26DS	
9	Hardware	LUS24	
1	Hardware	LUS26-2	
86	Hardware	H2.5A	
1		LGT3-SDS2.5	
2	Hardware	H8	

TOTAL NUMBER OF ITEMS= 111

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREENPARK HOMES	DRWG NO.
418361	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Pfi Jul 23 07:27:12 2021 Page 1
ID: NxDGIYRcZlvpB_E9rGowHDZcyp-0cS9iRk0Um69hCdZ2NfThOkvTnd22y748aKTZyvGP



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
M - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	1650F 1.5E	SPF
U - Q	2x6	DRY	1650F 1.5E	SPF
Q - N	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-K	2	12
K-M	2	12
M-N	2	12
N-L	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-U	2	12
U-Q	2	12
Q-N	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	DOWN	BRG	BRG
X	5045	0	5090	205	-1161
N	5094	0	5094	0	-1158
PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1161 LBS. FACTORED UPLIFT					
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1158 LBS. FACTORED UPLIFT					
PROVIDE FOR 205 LBS. FACTORED HORIZONTAL REACTION AT JOINT X					
UNFACTORED REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
X	3693	2220 / 0	473 / 0	0 / 0	0 / -1153
N	3725	2223 / 0	473 / 0	0 / 0	0 / -1152
HORIZONTAL REACTIONS					
X	0 / 0	0 / 0	0 / 0	146 / -146	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)		CHORDS		WEBS	
MEMB.	FORCE (LBS)	MAX. FACTORED	VERT. LOAD LC1 MAX UNBRAC LENGTH (LC)	MEMB.	MAX. FACTORED
FR-TO				FR-TO	
A-B	0 / 49	-115.2	-115.2 0.05 (2)	W-C	-774 / 254
B-C	-9244 / 1399	-115.2	-115.2 0.15 (2)	C-K	-1057 / 293
C-Y	-10870 / 2633	-115.2	-115.2 0.49 (1)	B-W	-1034 / 5293
Y-Z	-10870 / 2633	-115.2	-115.2 0.49 (1)	C-L	-803 / 4207
Z-AA	-10870 / 2633	-115.2	-115.2 0.49 (1)	P-K	-1719 / 7057
AA-D	-10870 / 2633	-115.2	-115.2 0.49 (1)	C-V	-1640 / 6813
D-AB	-14041 / 3382	-115.2	-115.2 0.63 (1)	P-J	-3165 / 814
AB-AC	-14041 / 3382	-115.2	-115.2 0.63 (1)	V-D	-2345 / 874
AC-AD	-14041 / 3382	-115.2	-115.2 0.63 (1)	R-J	-998 / 4165
AD-E	-14041 / 3382	-115.2	-115.2 0.63 (1)	D-T	-410 / 3703
E-AE	-14988 / 3598	-115.2	-115.2 0.64 (1)	R-I	-1748 / 553
AE-F	-14988 / 3598	-115.2	-115.2 0.64 (1)	T-E	-1519 / 510
F-AP	-14988 / 3598	-115.2	-115.2 0.64 (1)	S-I	-381 / 1568
AP-G	-14988 / 3598	-115.2	-115.2 0.64 (1)	E-S	-295 / 1107
G-AG	-14988 / 3598	-115.2	-115.2 0.64 (1)	S-G	-899 / 379
AG-H	-14988 / 3598	-115.2	-115.2 0.64 (1)		
H-AH	-14988 / 3598	-115.2	-115.2 0.64 (1)		
AH-I	-14988 / 3598	-115.2	-115.2 0.64 (1)		
I-AI	-13632 / 3285	-115.2	-115.2 0.81 (1)		
AI-AJ	-13632 / 3285	-115.2	-115.2 0.81 (1)		
AJ-AK	-13632 / 3285	-115.2	-115.2 0.81 (1)		
AK-J	-13632 / 3285	-115.2	-115.2 0.81 (1)		
J-AL	-10049 / 2447	-115.2	-115.2 0.45 (1)		
AL-AM	-10049 / 2447	-115.2	-115.2 0.45 (1)		
AM-AN	-10049 / 2447	-115.2	-115.2 0.45 (1)		
AN-K	-5185 / 1180	-115.2	-115.2 0.10 (14)		
K-L	0 / 53	-115.2	-115.2 0.05 (1)		
X-B	-5025 / 1178	0.0	0.0 0.14 (2)		
N-L	-5101 / 1174	0.0	0.0 0.14 (1)		
X-AO	-187 / 196	-39.5	-39.5 0.07 (17)		
AO-AP	-187 / 196	-39.5	-39.5 0.07 (17)		
AP-W	-187 / 196	-39.5	-39.5 0.07 (17)		
W-AQ	-1082 / 5151	-39.5	-39.5 0.26 (2)		
AQ-AR	-1082 / 5151	-39.5	-39.5 0.26 (2)		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 33.4 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2015, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.47")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/768 (0.70")

CSI: TC=0.84/1.00 (E-G:1), BC=0.84/1.00 (S-T:1),
WB=0.62/1.00 (K-P:2), SS=0.19/1.00 (J-K:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

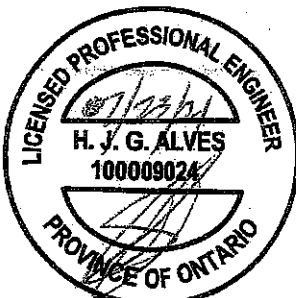
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (FL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

CONTINUED ON PAGE 2



Structural component only
DWG# T-2123954

JOB NAME 418361	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Fri Jul 23 07:27:12 2021 Page 2
ID: Nx0GIYRcZvpB EBrCcwHDzcyvP-QcS9Ifk0Um69hdCZ2NThOkevTnd22y?48aKTZvGPI

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	12.0		
C	TTWW-m	MT20	8.0	9.0	Edge	6.50
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMVW-t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0		
K	TTWW-m	MT20	8.0	9.0	3.50	1.75
L	TMVW-t	MT20	5.0	12.0		
N	BMV1-p	MT20	3.0	6.0		
O, R, T						
P	BMVW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0	2.50	3.50
S	BMVW-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMVW-t	MT20	5.0	8.0	2.50	3.75
W	BMVW-t	MT20	5.0	6.0	2.50	2.00
X	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. LENGTH FR-TO	MAX. CSI (LC)
FR-TO		FROM	TO				
AR-AS	-1082 / 5151	-39.5	-39.5 0.26 (2)			6.25	
AS-V	-1082 / 5151	-39.5	-39.5 0.26 (2)			6.25	
V-AT	-2450 / 10870	-39.5	-39.5 0.51 (1)			6.25	
AT-AU	-2450 / 10870	-39.5	-39.5 0.51 (1)			6.25	
AU-AV	-2450 / 10870	-39.5	-39.5 0.51 (1)			6.25	
AV-U	-2450 / 10870	-39.5	-39.5 0.51 (1)			6.25	
U-T	-2450 / 10870	-39.5	-39.5 0.51 (1)			6.25	
T-AW	-3198 / 14041	-39.5	-39.5 0.84 (1)			6.25	
AW-AX	-3198 / 14041	-39.5	-39.5 0.84 (1)			6.25	
AX-S	-3198 / 14041	-39.5	-39.5 0.84 (1)			6.25	
S-AY	-3084 / 13632	-39.5	-39.5 0.82 (1)			6.25	
AY-AZ	-3084 / 13632	-39.5	-39.5 0.82 (1)			6.25	
AZ-R	-3084 / 13632	-39.5	-39.5 0.82 (1)			6.25	
R-BA	-2221 / 10050	-39.5	-39.5 0.47 (1)			6.25	
BA-Q	-2221 / 10050	-39.5	-39.5 0.47 (1)			6.25	
Q-BB	-2221 / 10050	-39.5	-39.5 0.47 (1)			6.25	
BB-BC	-2221 / 10050	-39.5	-39.5 0.47 (1)			6.25	
BC-P	-2221 / 10050	-39.5	-39.5 0.47 (1)			6.25	
P-BD	-722 / 3918	-39.5	-39.5 0.22 (1)			6.25	
BD-BE	-722 / 3918	-39.5	-39.5 0.22 (1)			6.25	
BE-BF	-722 / 3918	-39.5	-39.5 0.22 (1)			6.25	
BF-O	-722 / 3918	-39.5	-39.5 0.22 (1)			6.25	
O-BG	-10 / 22	-39.5	-39.5 0.07 (17)			6.25	
BG-N	-10 / 22	-39.5	-39.5 0.07 (17)			6.25	

SPECIFIED CONCENTRATED LOADS (LBS)

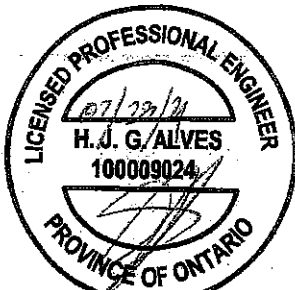
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-40	-171	---	FRONT	VERT	DEAD	---	C1
E	17-0-12	-60	-60	---	FRONT	VERT	SNOW	---	C1
G	23-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
I	29-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
K	42-0-8	-40	-40	---	FRONT	VERT	DEAD	---	C1
R	29-0-12	-21	-21	---	FRONT	VERT	SNOW	---	C1
S	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	17-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	5-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Z	7-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AA	9-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AB	11-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AC	13-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AD	15-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AE	19-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AF	21-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AG	25-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AH	27-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AI	31-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AJ	33-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AK	35-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AL	37-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AM	39-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AN	41-0-12	-61	-61	---	FRONT	VERT	TOTAL	---	C1
AO	1-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	3-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	5-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	7-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	9-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	11-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	13-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	15-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	19-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	21-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AY	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AZ	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BA	31-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BB	33-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BC	35-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BD	37-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BE	39-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BF	41-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BG	43-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NECC 4.1.2.2(B)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (6-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



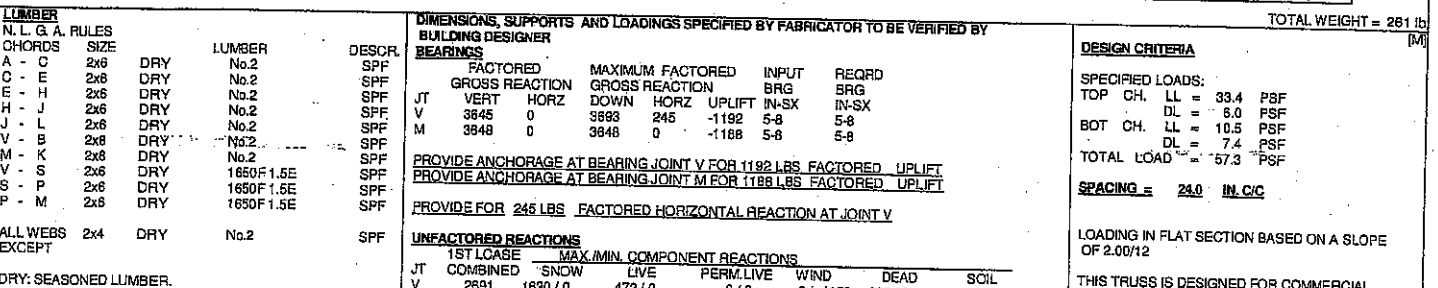
Structural component only
DWG# T-2123954

Tamarack Roof Truss, Burlington

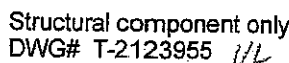
Version 8.426 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 07:27:13 2021 Page 1
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1-3-8 0-0 5-6-10 5-6-10 7-1-12 12-8-6 7-0-0 19-8-6 7-0-0 26-8-6 33-8-6 7-1-12 40-10-2 4-1-14 45-0-0 45-3-8
1-3-8 5-6-10 7-1-12 12-8-6 7-0-0 19-8-6 7-0-0 26-8-6 33-8-6 7-1-12 40-10-2 4-1-14 45-0-0 45-3-8
1-3-8 5-6-10 7-1-12 12-8-6 7-0-0 19-8-6 7-0-0 26-8-6 33-8-6 7-1-12 40-10-2 4-1-14 45-0-0 45-3-8

Scale = 1/4" = 1'-0"



NOTES: (1)
Lateral braces to be a minimum of 2X4 SPF #2.



1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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JOB NAME 418361	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

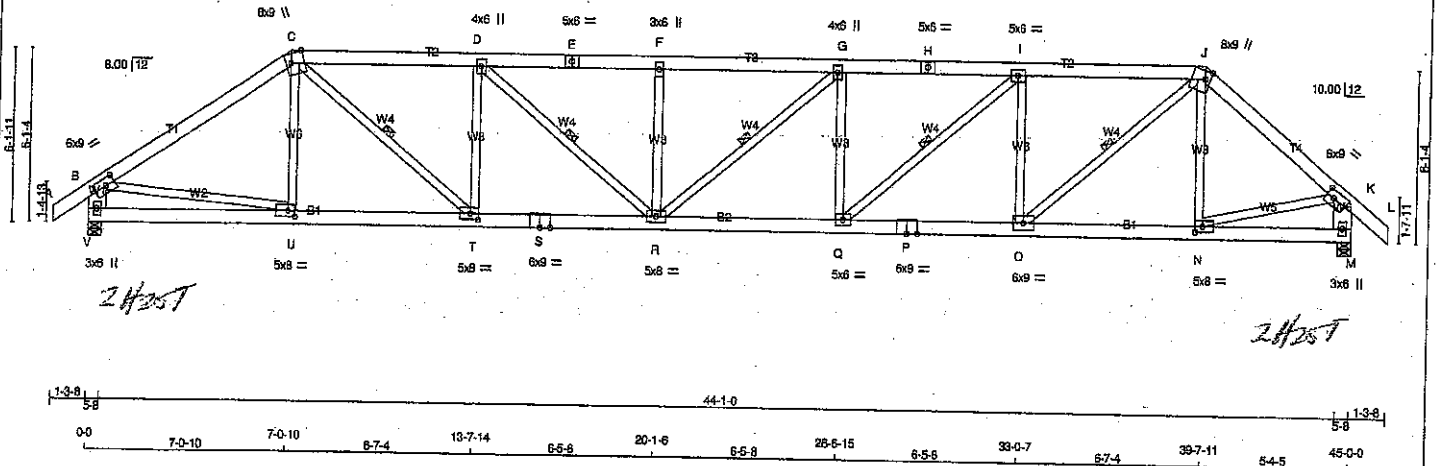
Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Fri Jul 23 07:27:13 2021 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
 DWG# T-2123955 *TH*

JOB NAME 418361	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Fri Jul 23 07:27:14 2021 Page 1					
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1-3-8 0-0 7-0-10 7-0-10 6-7-4 13-7-14 6-5-8 20-1-6 6-5-8 25-5-15 6-5-8 33-0-7 6-7-4 39-7-11 5-4-5 45-0-0 45-3-8 1-3-8					
Scale = 1:74.4					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x6	DRY	No.2
C - E	2x6	DRY	No.2
E - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
L - B	2x8	DRY	No.2
M - K	2x8	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x8	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS 2x4 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	6.0	9.0 3.00 3.75
C	TTWW-t	MT20	8.0	9.0 Edge 5.75
D	TMVW-t	MT20	4.0	5.0
E	TS-t	MT20	5.0	8.0
F	TMVW-t	MT20	3.0	8.0
G	TS-t	MT20	4.0	5.0
H	TS-t	MT20	5.0	8.0
I	TMVW-t	MT20	5.0	5.0
J	TTWW-t	MT20	8.0	9.0 3.50 2.25
K	TMVW-t	MT20	6.0	9.0 3.00 3.25
M	BMV-t	MT20	3.0	5.0
N	BMVW-t	MT20	5.0	8.0 2.50 3.25
O	BMVW-t	MT20	5.0	9.0
P	BS-t	MT20	6.0	9.0
Q	BMVW-t	MT20	5.0	6.0
R	BMVW-t	MT20	5.0	8.0
S	BS-t	MT20	6.0	9.0
T	BMVW-t	MT20	5.0	8.0 2.50 3.50
U	BMVW-t	MT20	5.0	8.0 2.50 3.00
V	BMV-t	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)



Structural component only
DWG# T-2123956

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
V	VERT	DOWN	UPLIFT	IN-SX
M	3648	0	3694	284
	3648	0	-1185	5-8
			-1179	5-8

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1185 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1179 LBS FACTORED UPLIFT

PROVIDE FOR 284 LBS FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

1ST CASE	MAX. MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
V	2691	1631 / 0 473 / 0 0 / 0 0 / -1145 620 / 0 0 / 0
M	2693	1600 / 0 473 / 0 0 / 0 75 / -1141 620 / 0 0 / 0

HORIZONTAL REACTIONS	V	—	0 / 0	0 / 0	0 / 0	203 / -202	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.63 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-O, C-T, I-Q, D-R, G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED	FORCE (LBS)
FR-TO					FR-TO		
A-B	0 / 49	-115.2	-115.2 0.09 (2)	10.00	U-C	-253 / 213	0.09 (8)
B-C	-4480 / 1464	-115.2	-115.2 0.76 (2)	3.36	N-J	-379 / 215	0.13 (8)
C-D	-5980 / 2115	-115.2	-115.2 0.59 (1)	3.18	B-U	-1004 / 3750	0.61 (14)
D-E	-6928 / 2404	-115.2	-115.2 0.66 (1)	2.88	N-K	-805 / 3027	0.48 (1)
E-F	-6928 / 2404	-115.2	-115.2 0.66 (1)	2.88	O-J	-1150 / 3410	0.55 (2)
F-G	-6928 / 2404	-115.2	-115.2 0.66 (1)	2.88	C-T	-1058 / 3031	0.49 (3)
G-H	-5782 / 2345	-115.2	-115.2 0.57 (1)	3.00	O-I	-1982 / 806	0.71 (2)
H-I	-6782 / 2345	-115.2	-115.2 0.66 (1)	2.92	T-D	-1726 / 747	0.83 (3)
I-J	-5512 / 1960	-115.2	-115.2 0.55 (1)	3.33	Q-I	-673 / 1743	0.28 (2)
J-K	-3874 / 1284	-115.2	-115.2 0.39 (1)	4.04	D-R	-478 / 1369	0.22 (3)
K-L	0 / 53	-115.2	-115.2 0.09 (1)	10.00	O-G	-591 / 434	0.32 (2)
V-B	-3576 / 1210	0.0	0.0 0.18 (2)	6.33	R-F	-721 / 373	0.28 (2)
M-K	-3585 / 1199	0.0	0.0 0.18 (1)	6.35	R-G	-88 / 251	0.04 (2)
V-U	-287 / 275	-39.5	-39.5 0.17 (17)	6.25			
U-T	-1107 / 3718	-39.5	-39.5 0.58 (1)	6.25			
T-S	-1847 / 5861	-39.5	-39.5 0.79 (1)	5.88			
S-R	-1847 / 5861	-39.5	-39.5 0.79 (1)	5.88			
R-Q	-2070 / 6782	-39.5	-39.5 0.89 (1)	5.63			
Q-P	-1638 / 5513	-39.5	-39.5 0.73 (1)	6.15			
P-O	-1838 / 5513	-39.5	-39.5 0.73 (1)	6.15			
O-N	-762 / 2958	-39.5	-39.5 0.44 (1)	6.25			
N-M	-10 / 22	-39.5	-39.5 0.13 (17)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.35")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.48")

CSI: TC=0.78/1.00 (B-C-2), BC=0.89/1.00 (Q-R-1), WB=0.71/1.00 (I-O-2), SB=0.27/1.00 (H-J-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (U) (INPUT = 0.90)
JSI METAL = 0.92 (S) (INPUT = 1.00)

JOB NAME 418361	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 07:27:14 2021 Page 2
 ID:Nx0GIYRcZlvpB E9rQcwHDzcyp-K?av7LmG0NNMbwLxAguxmpnzeHQDWw1IXS3RXRvvGPh

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.6} PSF AT
 (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
 COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL
 WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
 (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY
 FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF
 AND 5.0 PSF RESPECTIVELY.



Structural component only
 DWG# T-2123956 *TH*

JOB NAME 418361	TRUSS NAME T4	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

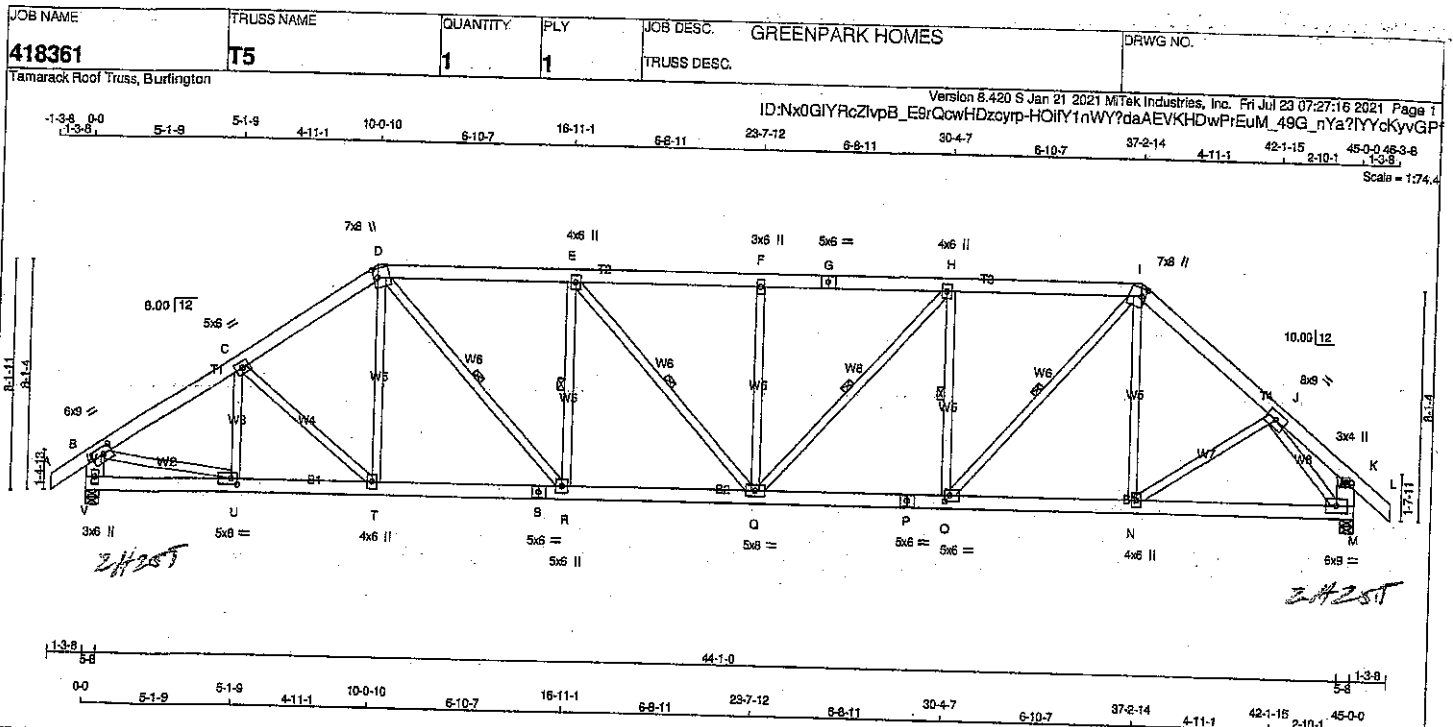
Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Fri Jul 23 07:27:15 2021 Page 2

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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2123957 *ML*



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
O - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - L	2x6	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	6.0	9.0	3.00	3.75
C	TMVW-1	MT20	5.0	6.0		
D	TTWW-m	MT20	7.0	8.0	3.75	1.75
E	TMVW-1	MT20	4.0	6.0		
F	TMVW-1	MT20	3.0	6.0		
G	TS-1	MT20	5.0	6.0		
H	TMVW-1	MT20	4.0	6.0		
I	TTWW-m	MT20	7.0	8.0	3.25	1.75
J	TMVW-1	MT20	6.0	9.0		
K	TMVW-1	MT20	3.0	4.0		
M	BMVW-1	MT20	6.0	9.0		
N	BMVW-1	MT20	4.0	8.0		
O	BMVW-1	MT20	5.0	6.0	2.50	2.00
P	BS-1	MT20	5.0	6.0		
Q	BMVW-1	MT20	5.0	8.0		
R	BMVW-1	MT20	6.0	8.0		
S	BS-1	MT20	6.0	6.0		
T	BMVW-1	MT20	4.0	8.0		
U	BMVW-1	MT20	5.0	8.0	2.50	2.50
V	BMV-1p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
V 3645	0	3683	364
M 3648	0	3648	0

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1168 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1168 LBS. FACTORED UPLIFT

PROVIDE FOR 364 LBS. FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
V 2651	1624 / 0
M 2693	1600 / 0

HORIZONTAL REACTIONS

JT	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	0 / 0	0 / 0	0 / 0	260 / -259	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-O, D-R, H-O, E-R, H-Q, E-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 49	-115.2 -115.2	0.09 (2)	10.00	U-C	-501 / 250	0.10 (2)
B-C	-4451 / 1467	-115.2 -115.2	0.32 (2)	3.87	C-T	-272 / 269	0.14 (7)
C-D	-4342 / 1558	-115.2 -115.2	0.31 (2)	3.94	T-D	-133 / 443	0.11 (13)
D-E	-4894 / 1795	-115.2 -115.2	0.56 (1)	3.49	N-I	-89 / 305	0.08 (9)
E-F	-5165 / 1849	-115.2 -115.2	0.57 (1)	3.39	N-J	-118 / 636	0.10 (2)
F-G	-5165 / 1849	-115.2 -115.2	0.57 (1)	3.38	B-U	-1074 / 3788	0.51 (2)
G-H	-5165 / 1849	-115.2 -115.2	0.57 (1)	3.38	O-I	-869 / 2508	0.44 (13)
H-I	-4579 / 1683	-115.2 -115.2	0.53 (1)	3.81	D-R	-772 / 2106	0.39 (11)
I-J	-3958 / 1373	-115.2 -115.2	0.29 (1)	4.14	O-H	-1614 / 699	0.44 (2)
J-K	-83 / 167	-115.2 -115.2	0.14 (14)	8.25	R-E	-1297 / 637	0.35 (3)
K-L	0 / 53	-115.2 -115.2	0.09 (1)	10.00	Q-H	-324 / 892	0.16 (13)
V-B	-3581 / 1180	0.0 0.0	0.18 (2)	6.35	E-Q	-236 / 561	0.12 (11)
M-K	-294 / 209	0.0 0.0	0.02 (4)	7.81	Q-F	-714 / 369	0.57 (1)
				8.25	J-M	-4185 / 1257	0.84 (1)
U	-346 / 354	-39.5 -39.5	0.10 (14)	7.81			
T	-1237 / 3731	-39.5 -39.5	0.63 (1)	6.25			
S	-1034 / 3587	-39.5 -39.5	0.62 (1)	6.25			
R	-1034 / 3587	-39.5 -39.5	0.52 (1)	6.25			
Q	-1444 / 4894	-39.5 -39.5	0.86 (1)	6.25			
P	-1282 / 4579	-39.5 -39.5	0.62 (1)	6.25			
O	-1282 / 4579	-39.5 -39.5	0.62 (1)	6.25			
N	-700 / 2889	-39.5 -39.5	0.50 (1)	6.25			
M	-771 / 2640	-39.5 -39.5	0.46 (1)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2015, ABC 2019
- PART 4 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.57/1.00 (F-H:1), BC=0.66/1.00 (Q-R:1), WB=0.84/1.00 (J-M:1), SSI=0.29/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSB) (FL) (FL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (D) (INPUT = 0.90)
JSI METAL = 0.81 (P) (INPUT = 1.00)

JOB NAME 418361	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:27:16 2021 Page 2 ID:Nx0GIYRcZlvp8 E9rQcwHDzovp-HQifY1nWY?daAEVKHDwPrEuM 49G_nYa?IYYcKvGp	

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

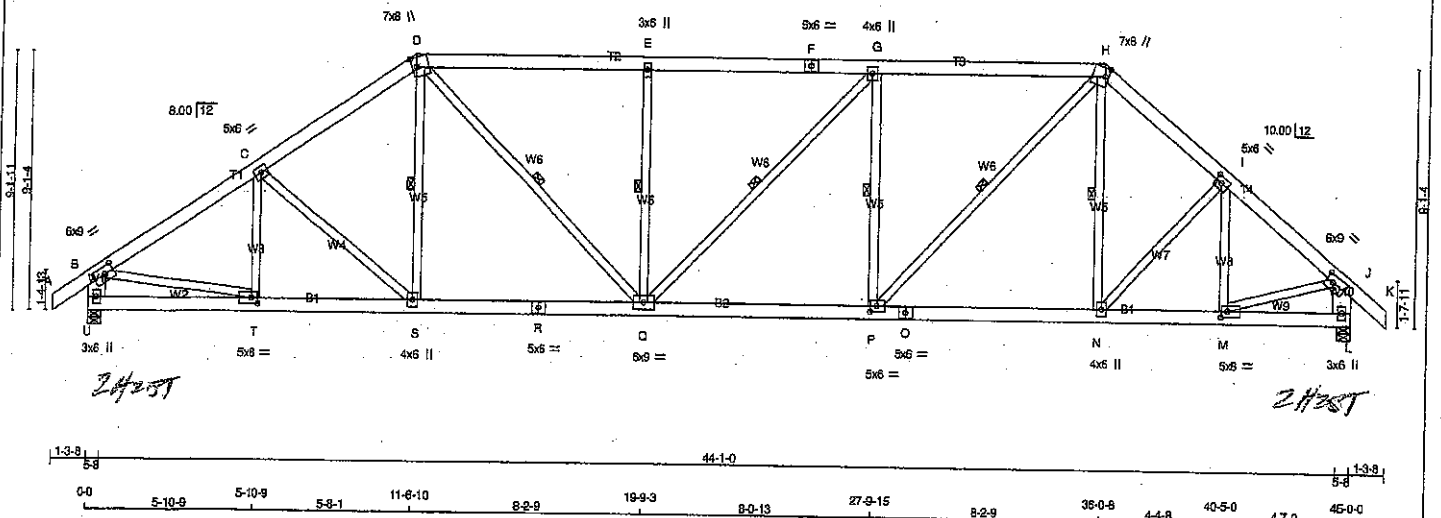


Structural component only
DWG# T-2123958 *JA*

JOB NAME: 418361 TRUSS NAME: T6 QUANTITY: 1 PLY: 1 JOB DESC: GREENPARK HOMES TRUSS DESC: DRWG NO. Tamarack Roof Truss, Burlington

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ID: N0GIYRcZlpB_E9rQcwHDzcyrp-laF2No8JiRoO4WrxReORRUUgJkDPH58myvGPg

1-3-8 0-0 5-10-9 5-10-9 5-8-1 11-6-10 8-2-9 19-9-3 8-0-13 27-4-15 8-2-9 36-0-8 4-4-8 40-5-0 4-7-0 45-0-0 45-3-8 1-3-8
Scale = 1/74.4



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT. TYPE	PLATES	W	LEN	Y	X
B-TMWW-t	MT20	6.0	9.0	3.00	3.75
C-TMWW-t	MT20	5.0	8.0		
D-TTWW+m	MT20	7.0	8.0	4.00	1.75
E-TMWW-w	MT20	3.0	6.0		
F-TS-t	MT20	5.0	6.0		
G-TMWW-t	MT20	4.0	6.0		
H-TTWW+m	MT20	7.0	8.0	3.75	1.50
I-TMWW-t	MT20	5.0	8.0	2.50	2.75
J-TMWW-t	MT20	6.0	9.0	3.00	3.25
L-BMV1+p	MT20	3.0	6.0		
M-BMWV-t	MT20	5.0	8.0	2.50	2.75
N-BMWV-t	MT20	4.0	8.0		
O-B-t	MT20	5.0	6.0		
P-BMWV-t	MT20	5.0	6.0	2.50	2.75
Q-BMWV-t	MT20	6.0	9.0		
R-B-t	MT20	5.0	6.0		
S-BMWV-t	MT20	4.0	8.0		
T-BMWV-t	MT20	5.0	8.0	2.50	2.75
U-BMV1+p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRO	BRG
JT VERT	3545	3672	403	1153
U	3545	3672	403	1153
L	3545	3672	403	1153

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1158 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1142 LBS. FACTORED UPLIFT
PROVIDE FOR 408 LBS. FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	2691	1617 / 0
U	2691	1617 / 0
L	2691	1617 / 0

HORIZONTAL REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	2691	1617 / 0
U	2691	1617 / 0
L	2691	1617 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-S, H-N, H-P, D-Q, G-P, E-Q, G-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (16)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 49	-115.2	-115.2	0.09 (2)	10.00	T-C	-417 / 229
B-C	-4508 / 1472	-115.2	-115.2	0.44 (2)	3.74	C-S	-454 / 363
C-D	-4232 / 1524	-115.2	-115.2	0.42 (2)	3.88	S-D	-180 / 601
D-E	-4567 / 1694	-115.2	-115.2	0.72 (1)	3.40	N-H	-155 / 388
E-F	-4567 / 1695	-115.2	-115.2	0.72 (1)	3.38	H-I	-128 / 278
F-G	-4567 / 1695	-115.2	-115.2	0.72 (1)	3.38	M-I	-580 / 239
G-H	-4383 / 1618	-115.2	-115.2	0.71 (1)	3.46	B-T	-1062 / 3926
H-I	-3847 / 1415	-115.2	-115.2	0.29 (14)	4.21	M-J	-791 / 3035
I-J	-3808 / 1248	-115.2	-115.2	0.27 (14)	4.24	P-H	-775 / 2260
J-K	0 / 53	-115.2	-115.2	0.09 (1)	10.00	D-Q	-681 / 1766
U-B	-3591 / 1173	0.0	0.0	0.17 (2)	6.35	P-Q	-1313 / 636
L-J	-3554 / 1154	0.0	0.0	0.18 (1)	6.35	Q-E	-1012 / 527
						Q-G	-149 / 412
U-T	-385 / 394	-39.5	-39.5	0.12 (5)	6.25		
T-S	-1254 / 3762	-39.5	-39.5	0.57 (1)	6.25		
S-R	-978 / 3492	-39.5	-39.5	0.54 (1)	6.25		
R-Q	-978 / 3492	-39.5	-39.5	0.54 (1)	6.25		
Q-P	-1198 / 4384	-39.5	-39.5	0.65 (1)	6.25		
P-O	-664 / 2924	-39.5	-39.5	0.48 (1)	6.25		
O-N	-664 / 2924	-39.5	-39.5	0.48 (1)	6.25		
N-M	-747 / 2949	-39.5	-39.5	0.47 (1)	6.25		
M-L	-10 / 22	-39.5	-39.5	0.08 (4)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2(a)

TOTAL WEIGHT = 294 lb

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12
THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.
(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.50")
CALCULATED VERT. DEFL. (LL) = L/999 (0.19")
ALLOWABLE DEFL. (TL) = L/180 (3.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CS: TC=0.72/1.00 (E-G:1), BC=0.65/1.00 (P-Q:1), WB=0.62/1.00 (B-T:1), SSI=0.34/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

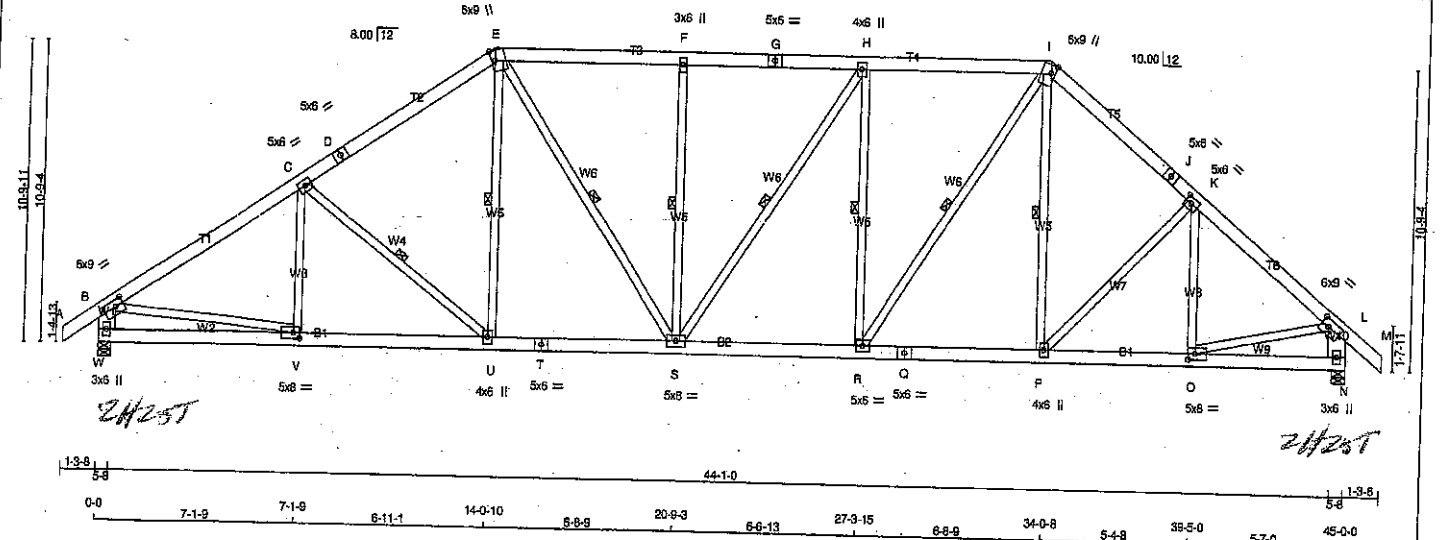
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (H) (INPUT = 0.90)
JSI METAL = 0.76 (B) (INPUT = 1.00)

JOB NAME 418361	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 07:27:17 2021 Page 2 ID:Nx0GfYRcZivpB E9rQcwHDzcvrp-laF2INp9JilRoQ4WrxReORRURUUgIJKDPH58mwvGPe	
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF 19.6 PSF AT (30-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.					
Structural component only DWG# T-2123959 <i>in</i>					

JOB NAME 418361	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 07:27:18 2021 Page 2 ID:Nx0GIYR6ZvpB_E9rQcwHDzcvp-DmpQzpn4clPYfiPeytztzh9urvsIVtS31fpCwvGPd	
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM; INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.					
 Structural component only DWG# T-2123960 <i>2/1</i>					

JOB NAME 418361	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Fri Jul 23 07:27:19 2021 Page 1					
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Scale = 1:75.4					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
D - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - J	2x6	DRY	No.2
J - M	2x6	DRY	No.2
W - B	2x8	DRY	No.2
N - L	2x8	DRY	No.2
W - T	2x8	DRY	No.2
T - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
ALL WEBS 2x4 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	6.0	9.0	2.75 3.75
C	TMVW-t	MT20	5.0	6.0	
D	TS-t	MT20	5.0	6.0	
E	TTWW-m	MT20	6.0	9.0	4.25 1.50
F	TMVW-w	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	6.0	
I	TTWW-m	MT20	6.0	9.0	3.75 1.75
K	TMVW-t	MT20	5.0	6.0	2.50 2.75
L	TMVW-t	MT20	6.0	9.0	3.00 3.25
N	BMV-t	MT20	3.0	6.0	
O	BMVW-t	MT20	5.0	8.0	2.50 3.25
P	BMVW-t	MT20	4.0	6.0	
Q	BS-t	MT20	5.0	6.0	
R	BMVW-t	MT20	5.0	6.0	
S	BMVW-t	MT20	5.0	6.0	
T	BS-t	MT20	5.0	6.0	
U	BMVW-t	MT20	4.0	6.0	
V	BMVW-t	MT20	5.0	8.0	2.50 2.75
W	BMV-t	MT20	3.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
W	3645	0	3645	469	-1137	5-8	5-8		
N	3645	0	3645	0	-1114	5-8	5-8		

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 1137 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1114 LBS. FACTORED UPLIFT

PROVIDE FOR 469 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	2691	1599 / 0	473 / 0	0 / 0	97 / -1110	620 / 0	0 / 0
N	2693	1600 / 0	473 / 0	0 / 0	192 / -1094	620 / 0	0 / 0

HORIZONTAL REACTIONS		W	0 / 0	0 / 0	0 / 0	335 / -335	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-U, E-U, I-P, I-R, E-S, H-R, F-S, H-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0 / 49	-115.2	-115.2	0.09 (2)	V-C	-293 / 210	0.11 (8)
B-C	-4556 / 1455	-115.2	-115.2	0.70 (2)	C-U	-794 / 507	0.31 (2)
C-D	-3996 / 1453	-115.2	-115.2	0.64 (2)	U-E	-266 / 778	0.14 (13)
D-E	-3996 / 1453	-115.2	-115.2	0.64 (2)	P-I	-244 / 503	0.13 (14)
E-F	-3630 / 1479	-115.2	-115.2	0.43 (1)	P-K	-365 / 404	0.32 (8)
F-G	-3630 / 1480	-115.2	-115.2	0.44 (1)	O-K	-437 / 212	0.17 (8)
G-H	-3630 / 1480	-115.2	-115.2	0.44 (1)	B-V	-1022 / 3863	0.62 (1)
H-I	-3996 / 1413	-115.2	-115.2	0.43 (1)	O-L	-766 / 3088	0.50 (1)
I-J	-3722 / 1379	-115.2	-115.2	0.37 (14)	R-I	-609 / 1760	0.50 (13)
J-K	-3722 / 1379	-115.2	-115.2	0.37 (14)	E-S	-532 / 1258	0.44 (11)
K-L	-3996 / 1339	-115.2	-115.2	0.37 (1)	R-H	-1210 / 570	0.66 (2)
L-M	0 / 53	-115.2	-115.2	0.09 (1)	S-F	-820 / 428	0.44 (3)
W-B	-3523 / 1156	0.0	0.0	0.17 (2)	S-H	-167 / 460	0.14 (13)
N-L	-3547 / 1130	0.0	0.0	0.18 (1)			
W-V	-451 / 460	-39.5	-39.5	0.18 (17)			
V-U	-1263 / 3832	-39.5	-39.5	0.56 (1)			
U-T	-871 / 3288	-39.5	-39.5	0.49 (1)			
T-S	-871 / 3288	-39.5	-39.5	0.49 (1)			
S-R	-924 / 3683	-39.5	-39.5	0.52 (1)			
R-Q	-591 / 2823	-39.5	-39.5	0.42 (1)			
Q-P	-591 / 2823	-39.5	-39.5	0.42 (1)			
P-O	-719 / 3029	-39.5	-39.5	0.44 (1)			
O-N	-10 / 22	-39.5	-39.5	0.10 (4)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOY CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/939 (0.16")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/939 (0.22")

CSI: TC=0.70/1.00 (B-C-2), BC=0.58/1.00 (U-V-1), WB=0.66/1.00 (H-R-2), SS=0.28/1.00 (H-1-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 758 1987 1878

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (O) (INPUT = 0.90)
JSI METAL = 0.77 (B) (INPUT = 1.00)

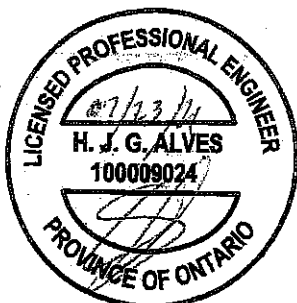


Structural component only
DWG# T-2123961

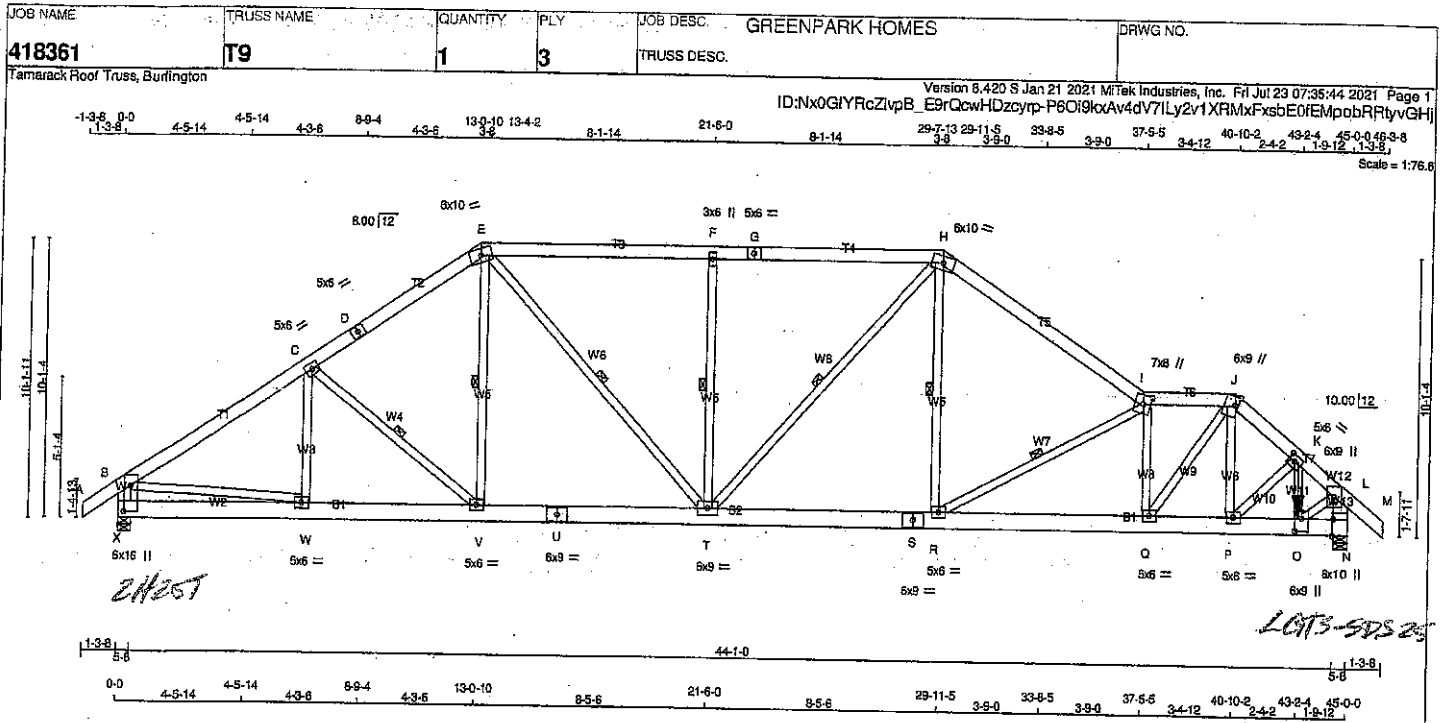
JOB NAME 418361	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:27:19 2021 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2123961 *2/11*



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
L - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	2	TOP
D-E	2	TOP
E-G	2	TOP
G-H	2	TOP
H-I	2	TOP
I-J	2	TOP
J-M	2	TOP
M-N	2	TOP
N-L	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-U	2	TOP
U-S	2	TOP
S-N	3	SIDE (1790.0)
WEBS : (0.122"x3") SPIRAL NAILS		
K-O	2	SIDE (324.0)
2x4	1	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
X	3921	0	3921	438
N	10271	0	10271	0

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1197 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 3230 LBS. FACTORED UPLIFT

PROVIDE FOR 438 LBS. FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
X	2894	1721 / 0
N	7584	4550 / 0

HORIZONTAL REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
X	0 / 0	0 / 0
N	313 / -312	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-V, E-V, H-R, H-T, E-T, F-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

FR-TO	CHORDS		WEBS	
	MEMB.	FORCE	MEMB.	FORCE
A-B	0 / 49	-115.2	W-C	-379 / 224
B-C	-4995 / 1550	-115.2	C-V	-645 / 444
C-D	-4586 / 1578	-115.2	V-E	-239 / 750
D-E	-4586 / 1578	-115.2	E-H	-528 / 1894
E-F	-4817 / 1718	-115.2	H-I	-2948 / 1131
F-G	-4818 / 1718	-115.2	I-J	-2122 / 736
G-H	-4818 / 1718	-115.2	J-K	-726 / 2583
H-I	-5428 / 1796	-115.2	K-L	-892 / 2472
I-J	-6960 / 2271	-115.2	L-M	-1108 / 4224
J-K	-6991 / 2287	-115.2	M-N	-892 / 2472
K-L	-8523 / 2727	-115.2	N-O	-1751 / 658
L-M	0 / 53	-115.2	O-P	-2122 / 7420
M-N	-3780 / 1211	0.0	P-Q	-291 / 774
N-O	-9546 / 3011	0.0	Q-R	-669 / 1845
O-P	-422 / 430	-39.5	R-S	-1178 / 618
P-Q	-1333 / 4193	-39.5	S-T	
Q-R	-995 / 3781	-39.5		
R-S	-895 / 3781	-39.5		
S-T	-1078 / 4545	-39.5		
T-U	-1078 / 4545	-39.5		
U-V	-1949 / 7028	-39.5		
V-W	-1503 / 5455	-39.5		
W-X	-1899 / 5438	-39.5		
X-Y	-10 / 21	-39.5		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
O	43-2-4	-5074	-5074	1253	BACK	VERT		C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 38.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2015, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.50")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/180 (3.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.24/1.00 (F-H:1), BC=0.40/1.00 (O-P:1), WB=0.40/1.00 (L-O:1), SS=0.13/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.81 (O) (INPUT = 0.80)
JSI METAL = 0.80 (L) (INPUT = 1.00)

JOB NAME 418361	TRUSS NAME T9	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITak Industries, Inc. Fri Jul 23 07:35:44 2021 Page 2
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GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TTWW-m	MT20	6.0	10.0		
F	TMW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	10.0		
I	TTWW+m	MT20	7.0	8.0	4.00	3.25
J	TTWW+m	MT20	6.0	9.0	4.00	1.25
K	TMWW-t	MT20	5.0	6.0	2.50	2.75
L	TMVW+p	MT20	6.0	9.0		
N	BMV1-t	MT20	6.0	10.0	Edge	0.50
O	BMWW-t	MT20	6.0	9.0	5.50	3.00
P, Q, R, V, W						
P	BMWW-t	MT20	5.0	6.0		
S	BS-t	MT20	6.0	9.0		
T	BMWWW-t	MT20	6.0	9.0		
U	BS-t	MT20	6.0	9.0		
X						
X	TMBMVW1+p	MT20	5.0	16.0	11.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(6)

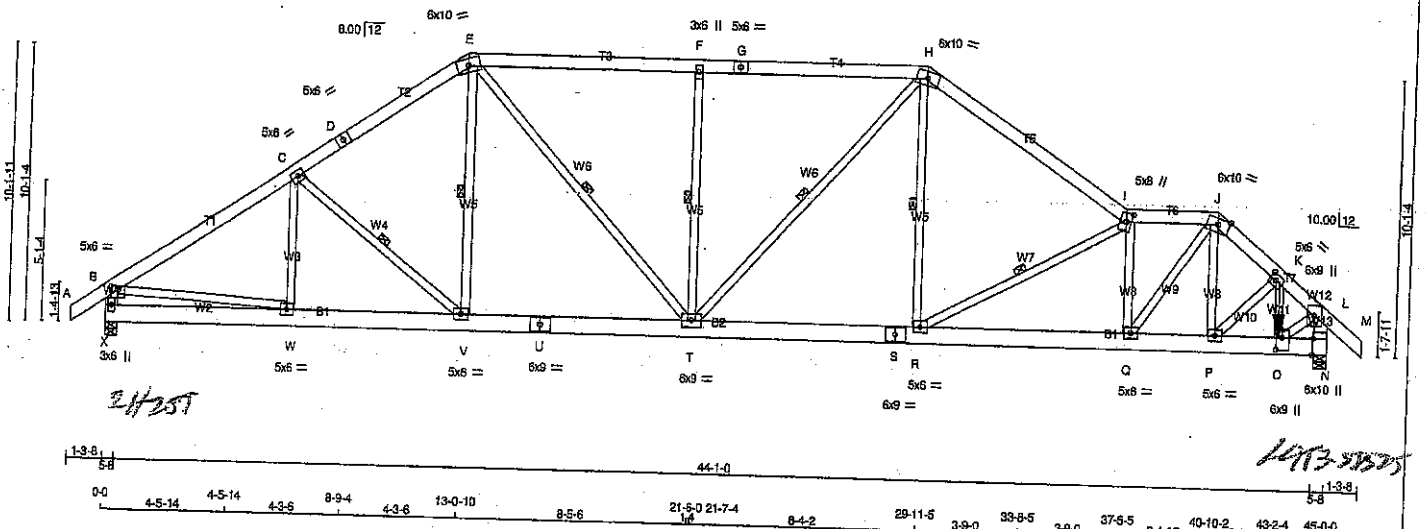
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2123962 *in*

JOB NAME 418362	TRUSS NAME T9Z	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 25 08:20:53 2021 Page 1
 ID: Nx0GIYRcZlvpB_E39QowHDzcyip-aHajj1jRfQw7ddxcEL8akddCH9xEqy5imKG0MyvFdO
 1-3-8 0-0 4-5-14 4-3-6 8-9-4 4-3-6 13-0-10 13-4-2 8-1-14 21-6-0 8-1-14 29-7-13 29-11-5 39-8-5 39-0 37-5-5 3-4-12 40-10-2 43-2-4 48-0-0 48-3-8 1-3-8 1-3-8
 Scale = 1/76.8



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY	No.2
D - E	2x8	DRY	No.2
E - G	2x8	DRY	No.2
G - H	2x8	DRY	No.2
H - I	2x8	DRY	No.2
I - J	2x8	DRY	No.2
J - M	2x8	DRY	No.2
X - B	2x8	DRY	No.2
N - L	2x8	DRY	No.2
X - U	2x8	DRY	No.2
U - S	2x8	DRY	No.2
S - N	2x8	DRY	No.2

ALL WEBS 2x4 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 2	12	TOP
D - E 2	12	TOP
E - G 2	12	TOP
G - H 2	12	TOP
H - I 2	12	TOP
I - J 2	12	TOP
J - M 2	12	TOP
X - B 2	12	TOP
N - L 2	12	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS	LOAD (PLF)
X - U 2	TOP
U - S 2	TOP
S - N 2	TOP

W-S : (0.122"x3") SPIRAL NAILS
 K - O 1
 2x4 1

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
X	3984 0	3984 438	-1217 5-8	IN-SX
N	11789 0	11789 0	-3708 5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1217 LBS. FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT N FOR 3708 LBS. FACTORED UPLIFT
 PROVIDE FOR 438 LBS. FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
X	2941 1749 / 0 515 / 0 0 / 0 123 / -1195 877 / 0 0 / 0
N	8682 5226 / 0 1477 / 0 0 / 0 814 / -3603 1979 / 0 0 / 0

HORIZONTAL REACTIONS

X	0 / 0 0 / 0 0 / 0 313 / -312 0 / 0 0 / 0
---	--

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.81 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-V, E-V, H-R, H-T, E-T, F-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 49	-115.2 -115.2 0.03 (2)	W-C	-389 / 227
B-C	-5091 / 1580	-115.2 -115.2 0.16 (2)	C-V	-635 / 441
C-D	-4691 / 1612	-115.2 -115.2 0.15 (2)	V-E	-237 / 745
D-E	-4691 / 1612	-115.2 -115.2 0.15 (2)	E-H	-599 / 2089
E-F	-4980 / 1763	-115.2 -115.2 0.25 (1)	H-I	-326 / 1249
F-G	-4981 / 1763	-115.2 -115.2 0.25 (1)	I-J	-2059 / 716
G-H	-4981 / 1763	-115.2 -115.2 0.25 (1)	J-K	-894 / 2481
H-I	-5884 / 1871	-115.2 -115.2 0.24 (3)	K-L	-872 / 2395
I-J	-7483 / 2435	-115.2 -115.2 0.10 (14)	L-M	-1133 / 4305
J-K	-7718 / 2514	-115.2 -115.2 0.11 (14)	M-N	-1135 / 3245
K-L	-9789 / 3125	-115.2 -115.2 0.12 (14)	N-O	-2311 / 834
L-M	0 / 53	-115.2 -115.2 0.03 (1)	O-P	-2559 / 8523
X-B	-3843 / 1231	0.0 0.0 0.08 (1)	P-Q	-254 / 688
N-L	-10925 / 3443	0.0 0.0 0.24 (1)	Q-R	-895 / 1829
			R-S	-1177 / 618
X-W	-422 / 430	-39.5 -39.5 0.04 (4)		
W-V	-1358 / 4273	-39.5 -39.5 0.17 (1)		
V-U	-1022 / 3868	-39.5 -39.5 0.16 (1)		
U-T	-1022 / 3868	-39.5 -39.5 0.16 (1)		
T-S	-1136 / 4745	-39.5 -39.5 0.19 (1)		
S-R	-1136 / 4745	-39.5 -39.5 0.19 (1)		
R-Q	-2113 / 7549	-39.5 -39.5 0.25 (1)		
Q-P	-1885 / 6038	-39.5 -39.5 0.25 (1)		
P-O	-2189 / 7395	-39.5 -39.5 0.46 (1)		
O-N	-10 / 21	-39.5 -39.5 0.22 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
O	43-2-4	-6238	-6238	1539	BACK	VERT	-	C1

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 33.4 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 4 OF CBC 2015, ABC 2019
 - PART 4 OF CBC 2012 (2019 AMENDMENT)
 - CSA 088-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED
 - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (1.50")
 CALCULATED VERT. DEF. (LL) = L/999 (0.07")
 ALLOWABLE DEF. (TL) = L/180 (3.00")
 CALCULATED VERT. DEF. (TL) = L/999 (0.10")

CSI: TC=0.25/1.00 (F-H:1), BO=0.46/1.00 (O-P:1), WB=0.46/1.00 (L-O:1), SSI=0.15/1.00 (N-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (J) (INPUT = 0.80)
 JSI METAL = 0.92 (L) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
 DWG# T-2123989

JOB NAME 418362	TRUSS NAME T9Z	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 06:20:53 2021 Page 2 ID:Nx0GIYRcZlvp8 E9rQowHDzcvr-aHapijRrQw7ddxcEL8okddCHl9xEqv5/mKG0MvVFdO	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TTWW-m	MT20	6.0	10.0		
F	TMW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	10.0		
I	TTWW-m	MT20	5.0	8.0	4.00	2.25
J	TTWW-m	MT20	6.0	10.0	Edge	
K	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	TMVW-p	MT20	6.0	9.0		
N	BMV1-t	MT20	6.0	10.0	Edge	0.50
O	BMVW-t	MT20	6.0	9.0	5.50	2.75
P, Q, R, V, W						
P	BMVW-t	MT20	5.0	6.0		
S	BS-t	MT20	6.0	9.0		
T	BMVW-t	MT20	6.0	9.0		
U	BS-t	MT20	6.0	9.0		
X	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

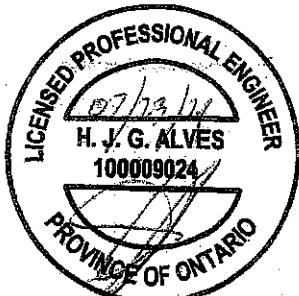
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

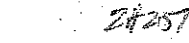
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



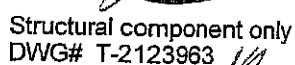
Structural component only
DWG# T-2123989

Tamarack Roof Truss, Burlington



GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

	GRIP (DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
IT20	650	371	1747	788	1987	1873
II16	438	302	2547	1258	4283	1816



JOB NAME 418361	TRUSS NAME T10	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 5 Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:27:28 2021 Page 2 ID:Nx0GIYrcZlvpB E9rQcWHDzcyro-akdJ0QtW8VbWJXgBBY2eJhXivYE7zRocLkQM0vVgPY	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW-t	MT20	5.0	9.0	2.75 3.75
C	TTWW-m	MT20	8.0	12.0	Edge 8.25
D	TTWW+m	MT20	6.0	10.0	5.25 2.25
E	TTWW+m	MT20	8.0	9.0	3.75 3.25
F	TMW+w	MT20	3.0	6.0	
G	TMW+t	MT20	4.0	6.0	
H	TS-t	MT20	5.0	6.0	
I	TMW-t	MT20	5.0	6.0	
J	TTWW+m	MT20	8.0	9.0	Edge 2.50
K	TMW-t	MT20	5.0	8.0	2.50 3.50
L	TMW+p	MT20	3.0	4.0	
N	BMW-t	MT20	6.0	9.0	
O	BMW-t	MT20	4.0	6.0	
P	BMW-t	MT20	5.0	6.0	2.50 2.00
Q	BS-t	MT20	5.0	6.0	
R	BMW-t	MT20	5.0	6.0	
S	BMW-t	MT20	7.0	8.0	
T	BMW-t	MT20	5.0	6.0	2.75 2.50
U	BS-t	MT16	5.0	12.5	
V	BMW-t	MT20	6.0	10.0	
W	BMW-t	MT20	6.0	9.0	
X	BMV-t	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-40	-40		FRONT	VERT	DEAD		C1
C	4-4-4	-88	-131		BACK	VERT	TOTAL		C1
C	4-0-10	-171	-171		FRONT	VERT	SNOW		C1
E	13-3-8	-406	-485		BACK	VERT	TOTAL		C1
S	18-9-8	-1987	-1987		BACK	VERT	TOTAL		C1
T	13-4-4	-167	-167		BACK	VERT	TOTAL		C1
V	10-4-4	-96	-96		BACK	VERT	TOTAL		C1
W	4-4-4	-21	-21		BACK	VERT	TOTAL		C1
Z	14-10-4	-8	-8		BACK	VERT	TOTAL		C1
AA	18-10-4	-8	-8		BACK	VERT	TOTAL		C1
AB	2-4-4	-21	-21		BACK	VERT	TOTAL		C1
AC	6-3-8	-482	-482		BACK	VERT	TOTAL		C1
AD	8-4-4	-36	-36		BACK	VERT	TOTAL		C1
AE	12-4-4	-36	-36		BACK	VERT	TOTAL		C1
AF	14-10-4	-167	-167		BACK	VERT	TOTAL		C1
AG	16-10-4	-167	-167		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

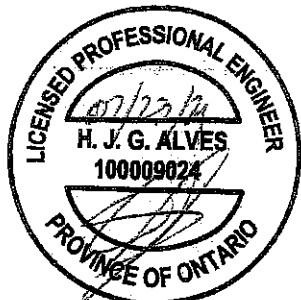
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

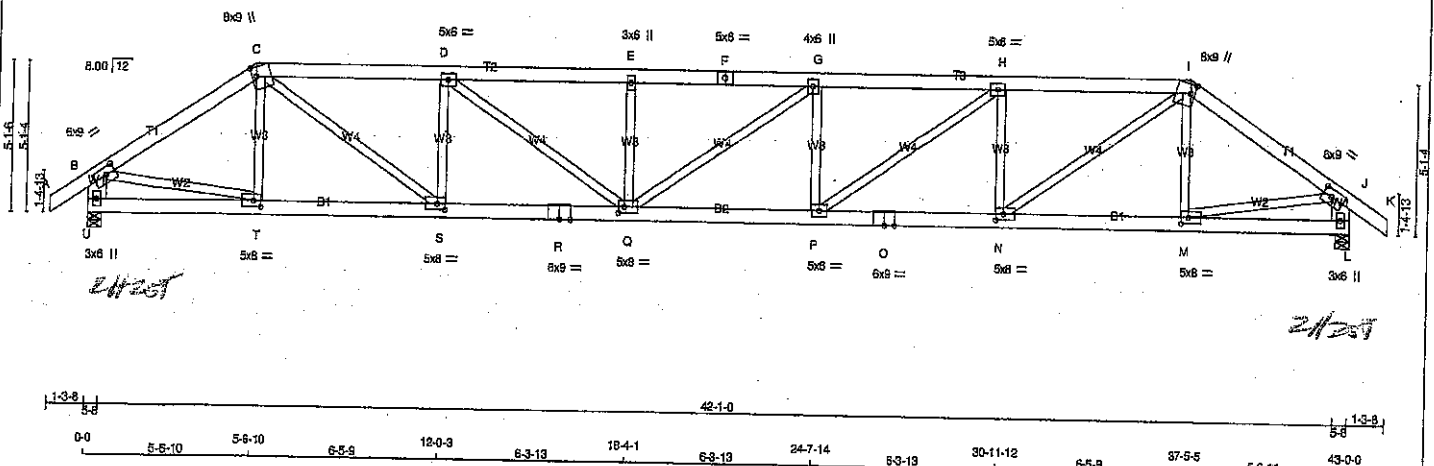
JSI GRIP = 0.90 (T) (INPUT = 0.90)

JSI METAL = 0.89 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2123963 *212*

JOB NAME 418361	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Famarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:27:24 2021 Page 1	
ID: Nx0GIYRcZvpB_E9rQcwHDzcymp-2wBhDmuYgSdST6sv3tHAWDgsJousN2mq?UzusyvGPX				Scale = 1:71.3	



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
I - K	2x6	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	6.0	9.0	3.00	3.75
C TTWW+m	MT20	8.0	9.0	3.75	1.75
D TMVW-t	MT20	5.0	6.0		
E TMVW-w	MT20	3.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW-t	MT20	4.0	6.0		
H TMVW-t	MT20	5.0	6.0		
I TTWW+m	MT20	8.0	9.0	3.75	1.75
J TMVW-t	MT20	6.0	9.0	3.00	3.75
L BMV-t+p	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	8.0	2.50	3.00
N BMVW-t	MT20	5.0	8.0	2.50	3.25
O BS-t	MT20	6.0	9.0		
P BMVW-t	MT20	5.0	6.0		
Q BMVW-t	MT20	5.0	8.0	2.50	2.50
R BS-t	MT20	6.0	9.0		
S BMVW-t	MT20	5.0	8.0	2.50	3.25
T BMVW-t	MT20	5.0	8.0	2.50	3.00
U BMV-t+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
JT VERT	0	3522	-237	-1136
U	3491	0	3522	-1136
L	3491	0	3522	-1136

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1136 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1136 LBS FACTORED UPLIFT

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
U	2576	1552 / 0	452 / 0	0 / 0	0 / -1097	593 / 0
L	2576	1552 / 0	452 / 0	0 / 0	85 / -1097	593 / 0

HORIZONTAL REACTIONS

U	0 / 0	0 / 0	0 / 0	169 / -170	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.38 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 49	-115.2 -115.2	0.09 (2)	10.00	T-C	-348 / 210	0.08 (8)
B-C	-4241 / 1393	-115.2 -115.2	0.45 (2)	3.81	M-I	-343 / 209	0.08 (8)
C-D	-8304 / 2194	-115.2 -115.2	0.59 (1)	3.11	B-T	-885 / 3565	0.57 (1)
D-E	-7803 / 2591	-115.2 -115.2	0.71 (1)	2.73	M-J	-885 / 3567	0.57 (1)
E-F	-7803 / 2591	-115.2 -115.2	0.81 (1)	2.84	N-I	-1180 / 3488	0.98 (13)
F-G	-7603 / 2591	-115.2 -115.2	0.81 (1)	2.84	C-S	-1198 / 3500	1.00 (11)
G-H	-7603 / 2591	-115.2 -115.2	0.71 (1)	2.73	N-H	-1797 / 746	0.43 (2)
H-I	-6304 / 2194	-115.2 -115.2	0.58 (1)	3.11	S-D	-1798 / 755	0.43 (3)
I-J	-4243 / 1394	-115.2 -115.2	0.45 (3)	3.81	P-H	-551 / 1684	0.46 (13)
J-K	0 / 49	-115.2 -115.2	0.09 (3)	10.00	D-Q	-561 / 1878	0.47 (1)
U-B	-3432 / 1156	0.0 0.0	0.17 (2)	6.44	P-G	-746 / 382	0.18 (2)
L-J	-3432 / 1156	0.0 0.0	0.17 (3)	6.44	Q-E	-708 / 365	0.17 (2)
					Q-G	-72 / 61	0.06 (3)
U-T	-219 / 229	-39.5 -39.5	0.12 (17)	6.25			
T-S	-1080 / 3513	-39.5 -39.5	0.50 (1)	6.25			
S-R	-1975 / 6305	-39.5 -39.5	0.84 (1)	5.73			
R-Q	-1975 / 6305	-39.5 -39.5	0.84 (1)	5.73			
Q-P	-2352 / 7608	-39.5 -39.5	1.00 (1)	5.36			
P-O	-1909 / 6304	-39.5 -39.5	0.84 (1)	5.81			
O-N	-1908 / 6304	-39.5 -39.5	0.84 (1)	5.81			
N-M	-948 / 3515	-39.5 -39.5	0.50 (1)	6.25			
M-L	-8 / 18	-39.5 -39.5	0.12 (17)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.8.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30'-0") FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_p , C_g , BASED ON THE (MAIN) WIND FORCE RESISTING SYSTEM, INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (8'-0") FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BOBC 2018, ABC 2018
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.41")
ALLOWABLE DEFL.(TL) = $L/180$ (2.87")
CALCULATED VERT. DEFL.(TL) = $L/914$ (0.56")

CS: TC=0.71/1.00 (D-E:1), BC=1.00/1.00 (P-Q:1), WB=1.00/1.00 (C-S:1), SS=0.28/1.00 (C-D:3)

DOL LUMBER-1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

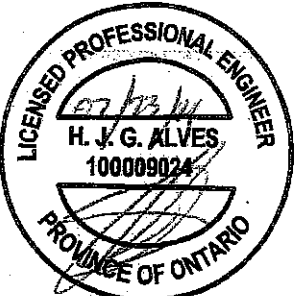
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (S) (INPUT = 0.90)

JSI METAL = 0.98 (R) (INPUT = 1.00)

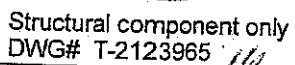


Structural component only
DWG# T-2123964

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

GRIP= 0.88 (H) (INPUT = 0.90)
METAL= 0.86 (P) (INPUT = 1.00)



JOB NAME 418361	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:27:25 2021 Page 2
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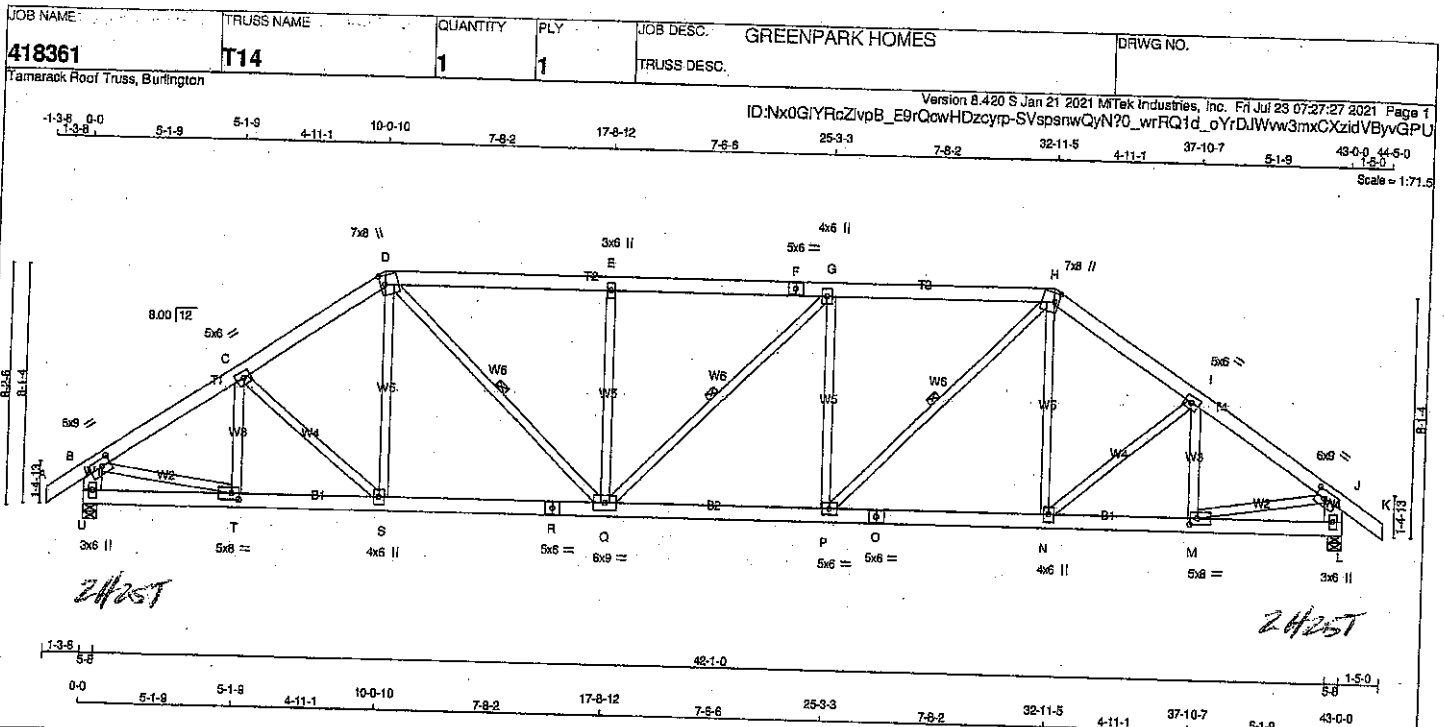
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pg} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2123965 2/11

CONTINUED ON PAGE 2

JOB NAME 418361	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:27:26 2021 Page 2 ID:Nx0GIYRcZlvpB E9rQcwHDzcytp- JIReSvoB3tANmGFK5FLJ X6YBKNB3UJz4ylywGpV			
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.					
Structural component only DWG# T-2123966 <i>2M</i>					



WOOD

N.L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY No.2	SPF
D - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - K	2x6	DRY No.2	SPF
U - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
U - R	2x6	DRY No.2	SPF
R - O	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2
EXCEPT
SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+	MT20	6.0	8.0	3.00	3.75
C	TMVW+	MT20	5.0	6.0		
D	TTWW+m	MT20	7.0	8.0	3.75	1.75
E	TMVW+	MT20	3.0	6.0		
F	TS+	MT20	5.0	6.0		
G	TMVW+	MT20	4.0	6.0		
H	TTWW+m	MT20	7.0	8.0	3.75	1.75
I	TMVW+	MT20	5.0	6.0		
J	TMVW+	MT20	6.0	9.0	3.00	3.75
L	BMVW+	MT20	3.0	6.0		
M	BMVW+	MT20	5.0	8.0	2.50	2.75
N	BMVW+	MT20	4.0	6.0		
O	BS+	MT20	5.0	6.0		
P	BMVW+	MT20	5.0	6.0		
Q	BMVW+	MT20	6.0	9.0		
R	BS+	MT20	5.0	6.0		
S	BMVW+	MT20	4.0	6.0		
T	BMVW+	MT20	5.0	8.0	2.50	2.75
U	BMVW+	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2123967

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
U	VERT	DOWN	IN-SX	IN-SX
L	3481 0	3481 -358	-1108 5-8	5-8
	3505 0	3505 0	-1108 5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1108 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1108 LBS FACTORED UPLIFT

PROVIDE FOR 358 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2576	1532 / 0	452 / 0	0 / 0	0 / -1073	593 / 0	0 / 0
L	2586	1540 / 0	452 / 0	0 / 0	188 / -1078	595 / 0	0 / 0

HORIZONTAL REACTIONS
U 0 / 0 0 / 0 0 / 0 255 / -256 0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, D-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

MAX. FACTORED				W E B S			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 49	-115.2 -115.2	0.09 (2)	10.00	T-C	-477 / 235	0.10 (1)
B-C	-4229 / 1375	-115.2 -115.2	0.31 (2)	3.99	C-S	-292 / 282	0.16 (7)
C-D	-4100 / 1454	-115.2 -115.2	0.30 (2)	4.05	S-D	-138 / 485	0.11 (13)
D-E	-4634 / 1672	-115.2 -115.2	0.82 (1)	3.48	N-H	-137 / 483	0.11 (14)
E-F	-4634 / 1673	-115.2 -115.2	0.83 (1)	3.48	H-I	-288 / 285	0.15 (3)
F-G	-4634 / 1673	-115.2 -115.2	0.83 (1)	3.48	M-I	-481 / 233	0.10 (8)
G-H	-4638 / 1675	-115.2 -115.2	0.83 (1)	3.48	B-T	-997 / 3598	0.58 (1)
H-I	-4100 / 1454	-115.2 -115.2	0.31 (2)	3.99	P-H	-667 / 1949	0.39 (13)
I-J	-4231 / 1377	-115.2 -115.2	0.31 (2)	4.05	M-J	-894 / 3599	0.58 (1)
J-K	0 / 54	-115.2 -115.2	0.11 (3)	10.00	D-Q	-701 / 1943	0.40 (11)
U-B	-3388 / 1115	0.0 0.0	0.17 (1)	6.46	P-G	-1074 / 541	0.86 (2)
L-J	-3402 / 1121	0.0 0.0	0.17 (1)	6.47	Q-E	-943 / 492	0.76 (3)
					Q-G	-188 / 177	0.11 (3)
U-T	-339 / 350	-39.5 -39.5	0.10 (4)	6.25			
T-S	-1164 / 3544	-39.5 -39.5	0.53 (1)	6.25			
S-R	-952 / 3386	-39.5 -39.5	0.51 (1)	6.25			
R-Q	-952 / 3386	-39.5 -39.5	0.51 (1)	6.25			
Q-P	-1278 / 4898	-39.5 -39.5	0.65 (1)	6.25			
P-O	-805 / 3386	-39.5 -39.5	0.51 (1)	6.25			
O-N	-805 / 3386	-39.5 -39.5	0.51 (1)	6.25			
N-M	-861 / 3545	-39.5 -39.5	0.53 (1)	6.25			
M-L	-8 / 18	-39.5 -39.5	0.10 (4)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(b)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.83/1.00 (G-H-1), BC=0.65/1.00 (P-Q-1),
WB=0.86/1.00 (G-P-2), SS=0.32/1.00 (G-H-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (T) (INPUT = 0.90)
JSI METAL= 0.71 (J) (INPUT = 1.00)

JOB NAME 418361	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

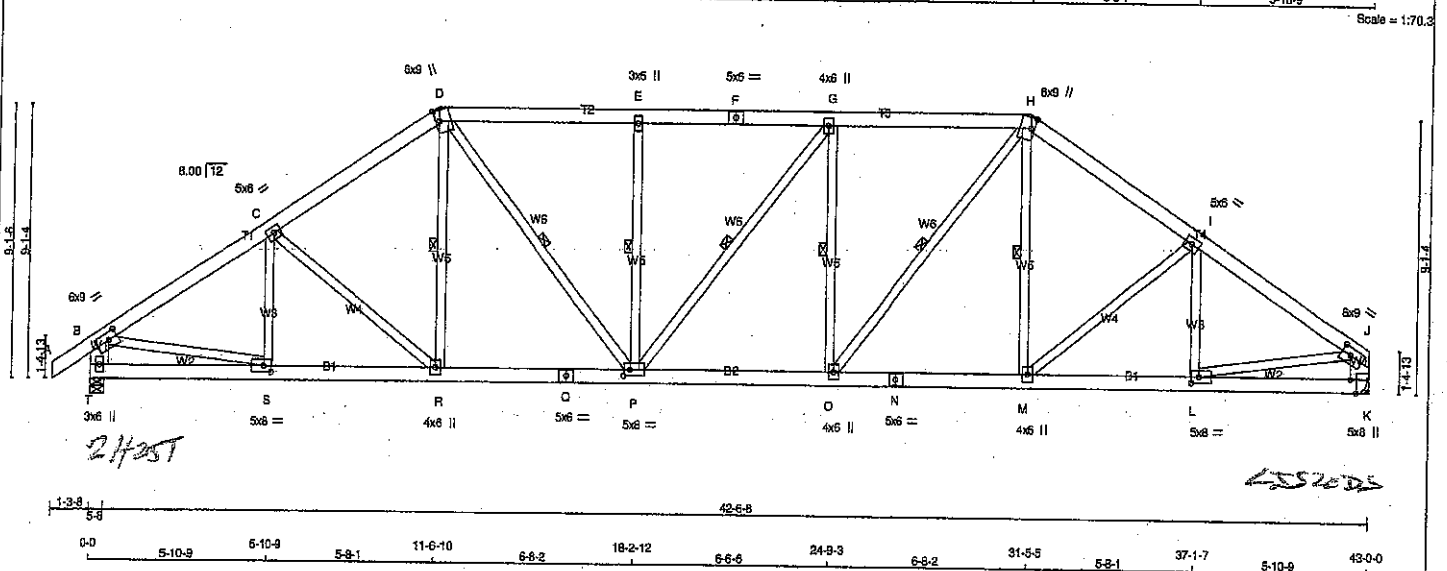
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 07:27:27 2021 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2123967 *2/2*

JOB NAME 418361	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Fri Jul 23 07:50:40 2021 Page 1	ID: N00GiYRcZlvpB_E9rQcwHDzoyrp-AUXRD6nCVNVNc7q9X56f12Jd2MCyr6ErBertyvG3j



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x8	DRY	No.2	SPF	FACTORED	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 33.4 PSF	
D - F	2x6	DRY	No.2	SPF	JT	VERT	DOWN	UP	IN-SX	BOT CH. LL = 10.5 PSF	
F - H	2x6	DRY	No.2	SPF	K	HORIZ	DOWN	UP	IN-SX	DL = 7.4 PSF	
H - J	2x6	DRY	No.2	SPF						TOTAL LOAD = 57.3 PSF	
J - L	2x8	DRY	No.2	SPF							
L - K	2x6	DRY	No.2	SPF							
K - M	2x6	DRY	No.2	SPF							
M - N	2x6	DRY	No.2	SPF							
N - K	2x6	DRY	No.2	SPF							
ALL WEBS	2x4	DRY	No.2	SPF	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.				SPACING = 24.0 IN/C		
EXCEPT					PROVIDE ANCHORAGE AT BEARING JOINT T FOR 1088 LBS FACTORED UPLIFT				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12		
DRY: SEASONED LUMBER					PROVIDE ANCHORAGE AT BEARING JOINT K FOR 1027 LBS FACTORED UPLIFT				THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015		

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	3.00	3.75
C	TMVW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	6.0	9.0	4.25	1.50

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2576	1532 / 0	452 / 0	0 / 0	43 / -1062	593 / 0	0 / 0
K	2485	1437 / 0	452 / 0	0 / 0	201 / -1010	576 / 0	0 / 0

HORIZONTAL REACTIONS

T	—	0 / 0	0 / 0	0 / 0	274 / -265	0 / 0	0 / 0
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PROVIDE ANCHORAGE AT BEARING JOINT T FOR 1088 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT K FOR 1027 LBS. FACTORED UPLIFT

PROVIDE FOR 383 LBS. FACTORED HORIZONTAL REACTION AT JOINT T

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019

- PART 4 OF OBC 2012 (2018 AMENDMENT)

- CSA 088-14

- TRC 2014

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	
A-B	0/49	-115.2	-115.2	S-C	-388/218	0.10	(8)
B-C	-4286/1369	-115.2	-115.2	C-R	-502/373	0.39	(2)
C-D	-3976/1413	-115.2	-115.2	R-D	-135/585	0.08	(2)
D-E	-4105/1510	-115.2	-115.2	M-H	-189/582	0.08	(3)
E-F	-4105/1511	-115.2	-115.2	M-I	-504/368	0.39	(3)
F-G	-4105/1511	-115.2	-115.2	L-I	-382/223	0.10	(8)
G-H	-4110/1511	-115.2	-115.2	B-S	-975/3639	0.58	(1)
H-I	-3976/1409	-115.2	-115.2	L-J	-1019/3840	0.59	(1)
I-J	-4288/1361	-115.2	-115.2	O-H	-544/1577	0.32	(13)
J-K	-3382/1103	0.0	0.0	D-P	-581/1568	0.34	(11)
K-L	-3219/1043	0.0	0.0	P-E	-863/490	0.25	(2)
L-M	-366/383	-39.5	-39.5	E-G	-814/428	0.29	(3)
M-N	-1200/3587	-39.5	-39.5	P-G	-222/208	0.13	(3)
N-O	-917/3278	-39.5	-39.5				
O-P	-917/3278	-39.5	-39.5				
P-Q	-1100/4110	-39.5	-39.5				
Q-R	-767/3278	-39.5	-39.5				
R-S	-767/3278	-39.5	-39.5				
S-T	-990/3589	-39.5	-39.5				
T-U	-8/18	-39.5	-39.5				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

Structural component only
DWG# T-2123968

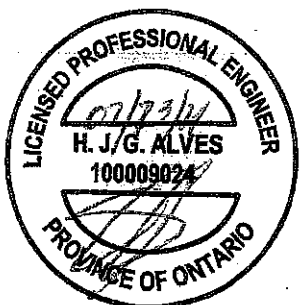
CONTINUED ON PAGE 2

JOB NAME 418361	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 07:50:40 2021 Page 2
 ID:Nx0GIYRoZvpB E9rQcwHDzcyro-AUXRD6nCvNVNc?g9X56f1l2Jd2MCvr6ErBertyyvG3j

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF 19.6 PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
 DWG# T-2123968 *2/2*

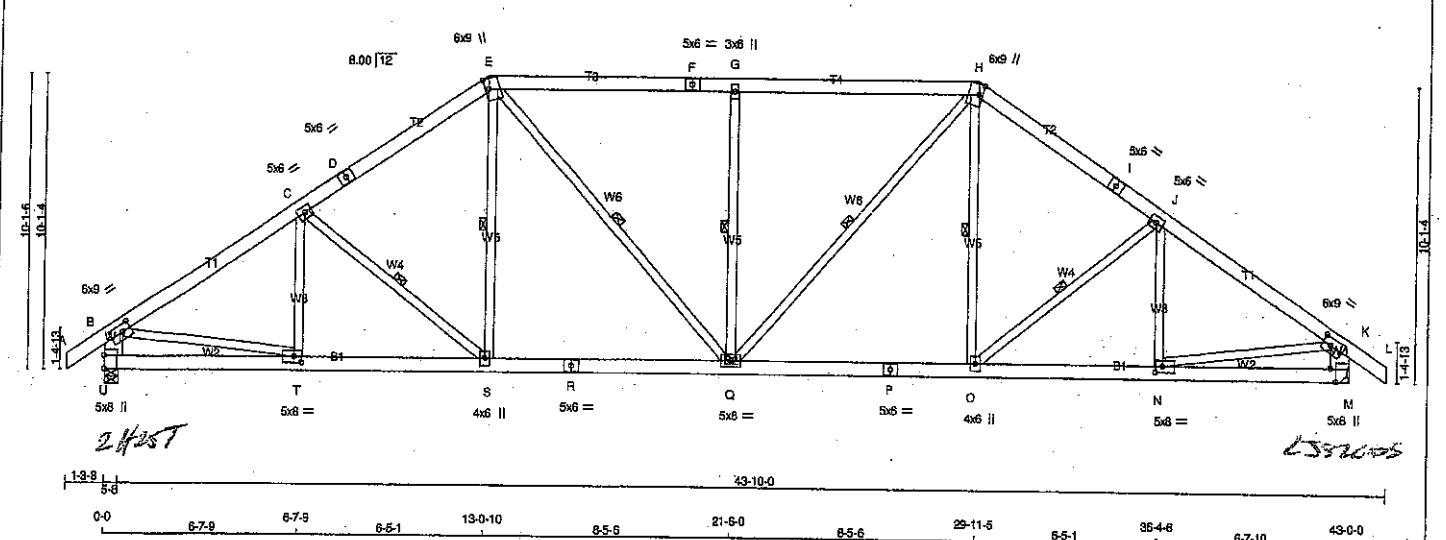
JOB NAME 418361	TRUSS NAME T16	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:50:41 2021 Page 1

ID: N00GIYRcZvpB_E9rCqwHDzcyrp-ehSpQRoqgheEE9PM5odulWaOASlahHJO4rOPQOyvG3i

Scale = 1:72.2



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x6	DRY	No.2
E - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - I	2x6	DRY	No.2
I - L	2x6	DRY	No.2
L - B	2x6	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
SPF			
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
JT	3491	0	3491	-435	-1071	5-8	5-8
U	3491	0	3491	0	-1071	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1071 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1071 LBS FACTORED UPLIFT

PROVIDE FOR 435 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS		1ST LOSE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
U	2576	1532 / 0	452 / 0	0 / 0	96 / -1051
M	2576	1532 / 0	452 / 0	0 / 0	241 / -1051

HORIZONTAL REACTIONS		U	
U	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, E-S, H-O, J-O, H-Q, E-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED COLUMN OF THE TABLE BELOW

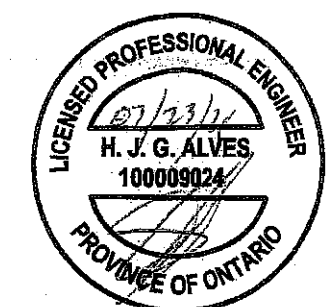
LOADING
TOTAL LOAD CASES: (18)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 49	-115.2 -115.2	0.09 (2)	T-C	-320 / 202
B-C	-4313 / 1354	-115.2 -115.2	0.57 (2)	C-S	-696 / 463
C-D	-3845 / 1361	-115.2 -115.2	0.53 (2)	S-E	-235 / 745
D-E	-3845 / 1361	-115.2 -115.2	0.53 (2)	O-H	-235 / 745
E-F	-3831 / 1413	-115.2 -115.2	0.85 (1)	C-H	-697 / 464
F-G	-3831 / 1413	-115.2 -115.2	0.85 (1)	N-J	-324 / 202
G-H	-3830 / 1413	-115.2 -115.2	0.85 (1)	B-T	-948 / 3660
H-I	-3845 / 1362	-115.2 -115.2	0.53 (3)	N-K	-948 / 3661
I-J	-3845 / 1362	-115.2 -115.2	0.53 (3)	Q-H	-462 / 1300
J-K	-4314 / 1355	-115.2 -115.2	0.57 (3)	E-Q	-518 / 1300
K-L	0 / 49	-115.2 -115.2	0.09 (3)	Q-G	-1184 / 620
U-B	-3372 / 1091	0.0	0.0		
M-K	-3372 / 1091	0.0	0.0		
U-T	-418 / 427	-39.5 -39.5	0.14 (4)		
T-S	-1177 / 3626	-39.5 -39.5	0.56 (1)		
S-R	-820 / 3165	-39.5 -39.5	0.52 (1)		
R-Q	-820 / 3165	-39.5 -39.5	0.52 (1)		
Q-P	-678 / 3166	-39.5 -39.5	0.52 (1)		
P-O	-678 / 3166	-39.5 -39.5	0.52 (1)		
O-N	-922 / 3628	-39.5 -39.5	0.56 (1)		
N-M	-8 / 18	-39.5 -39.5	0.14 (4)		

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-t	MT20	6.0	9.0	3.00 3.75
C TMVW-t	MT20	5.0	6.0	
D TS-t	MT20	5.0	6.0	
E TTWW+m	MT20	6.0	9.0	4.25 1.50
G TMW-w	MT20	3.0	6.0	
H TTWW+m	MT20	6.0	9.0	4.25 1.50
J TMVW-t	MT20	5.0	6.0	
K TMVW-t	MT20	6.0	9.0	3.00 3.75
M BMV1-t	MT20	5.0	8.0	Edge 2.25
N BMVW-t	MT20	5.0	8.0	2.50 3.00
O BMVW-t	MT20	4.0	6.0	
P BS-t	MT20	5.0	6.0	
Q BMVW-t	MT20	5.0	8.0	
R BS-t	MT20	5.0	6.0	
S BMVW-t	MT20	4.0	6.0	
T BMVW-t	MT20	5.0	8.0	2.50 3.00
U BMV1-t	MT20	5.0	8.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)



Structural component only
DWG# T-2123969

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

TOTAL WEIGHT = 3 X 276 = 828 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/998 (0.15")
ALLOWABLE DEFL.(TL) = L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.85/1.00 (E-G-1), BC=0.56/1.00 (N-O-1), WB=0.59/1.00 (K-N-1), SSI=0.36/1.00 (G-H-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN MAX MIN
MT20	650	371	1747 788 1987 1873

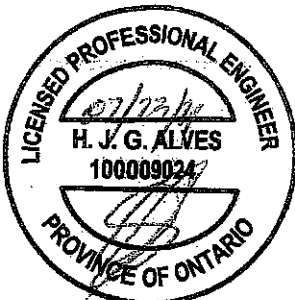
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (T) (INPUT = 0.90)
JSI METAL = 0.73 (B) (INPUT = 1.00)

JOB NAME 418361	TRUSS NAME T16	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Fri Jul 23 07:50:41 2021 Page 2 ID:Nx0GIYRcZlvB_E9rQcwhDzcyrp-eh5pQRoggheEE9PM5oduIWoOASlahHJ04rOPQOyvG3j	

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

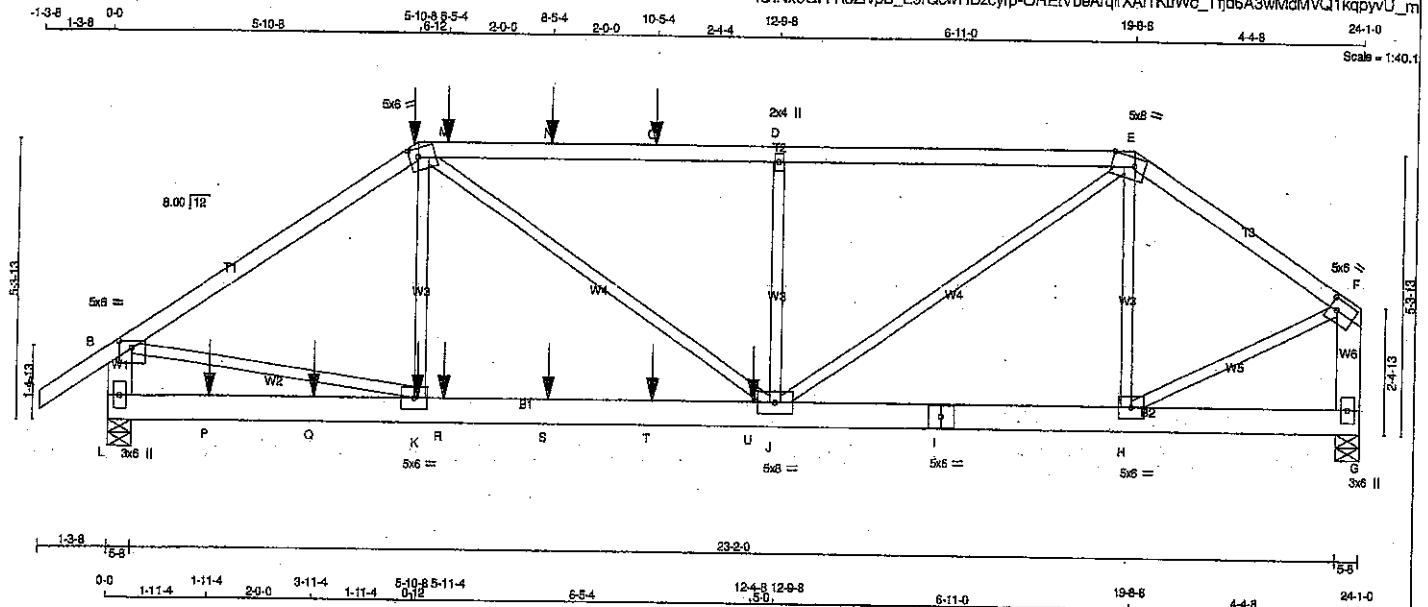


Structural component only
DWG# T-2123969

JOB NAME 418361	TRUSS NAME T17	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 16:00:13 2021 Page 1
ID:Nx0GIYRcZvpB_E9rQcwHDzcypr-OREVbeArqirXA1KuW_Tfd6A3wMdmVQ1kqpyvU_m



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x6	DRY	No.2
G - F	2x6	DRY	No.2
L - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-E	12	SIDE (61.0)
E-F	12	TOP
L-B	12	TOP
G-F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I	12	SIDE (183.1)
I-G	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
K-C	6	SIDE (5.6)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE. THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	3015	0	5-8
L	3015	0	5-8
G	2404	0	5-8

UNFACTORED REACTIONS			
1ST LCASE	MAX/MIN. COMPONENT REACTIONS		
JT COMBINED	SNOW	LIVE	PERM. LIVE
L	2129	1414 / 0	0 / 0
G	1597	1130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 35	-91.8	-91.8 0.07 (1)
B-C	-3682 / 0	-91.8	-91.8 0.45 (1)
C-M	-4280 / 0	-91.8	-91.8 0.80 (1)
M-N	-4280 / 0	-91.8	-91.8 0.80 (1)
N-O	-4280 / 0	-91.8	-91.8 0.80 (1)
O-D	-4280 / 0	-91.8	-91.8 0.80 (1)
D-E	-4280 / 0	-91.8	-91.8 0.80 (1)
E-F	-2341 / 0	-91.8	-91.8 0.20 (1)
L-B	-2960 / 0	0.0	0.0 0.11 (1)
G-F	-2370 / 0	0.0	0.0 0.10 (1)
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
K-C	-129 / 181	0.03 (4)	
C-J	0 / 1488	0.18 (1)	
J-D	-1032 / 0	0.21 (1)	
J-E	0 / 2664	0.35 (1)	
H-E	-778 / 0	0.16 (1)	
G-K	0 / 3110	0.38 (1)	
H-F	0 / 2129	0.26 (1)	

SPECIFIED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX. MAX.++
C	5-10-8	-337	-337
K	5-11-4	-29	-29
M	6-5-4	-136	-136
N	8-5-4	-122	-122
O	10-5-4	-122	-122
P	1-11-4	-29	-29
Q	3-11-4	-29	-29
R	6-5-4	-29	-29
S	8-5-4	-29	-29
T	10-5-4	-29	-29
U	12-4-8	-972	-972

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.15")
CSI: TC=0.80/1.00 (C-O:1), BC=0.34/1.00 (J-K:1), WB=0.38/1.00 (B-K:1), SS=0.46/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (E) (INPUT = 0.90)
JSI METAL = 0.35 (K) (INPUT = 1.00)



Structural component only
DWG# T-2123970

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
418361	T17	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:00:13 2021 Page 2
ID:Nx0GIYRcZivpB E9rQowHDzcymp-OREtVbeArqirXA11KuWc Tfid6A3wMgMVQ1kqpyvU m

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	5.50
F	TMVW-t	MT20	5.0	6.0	2.50	1.75
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

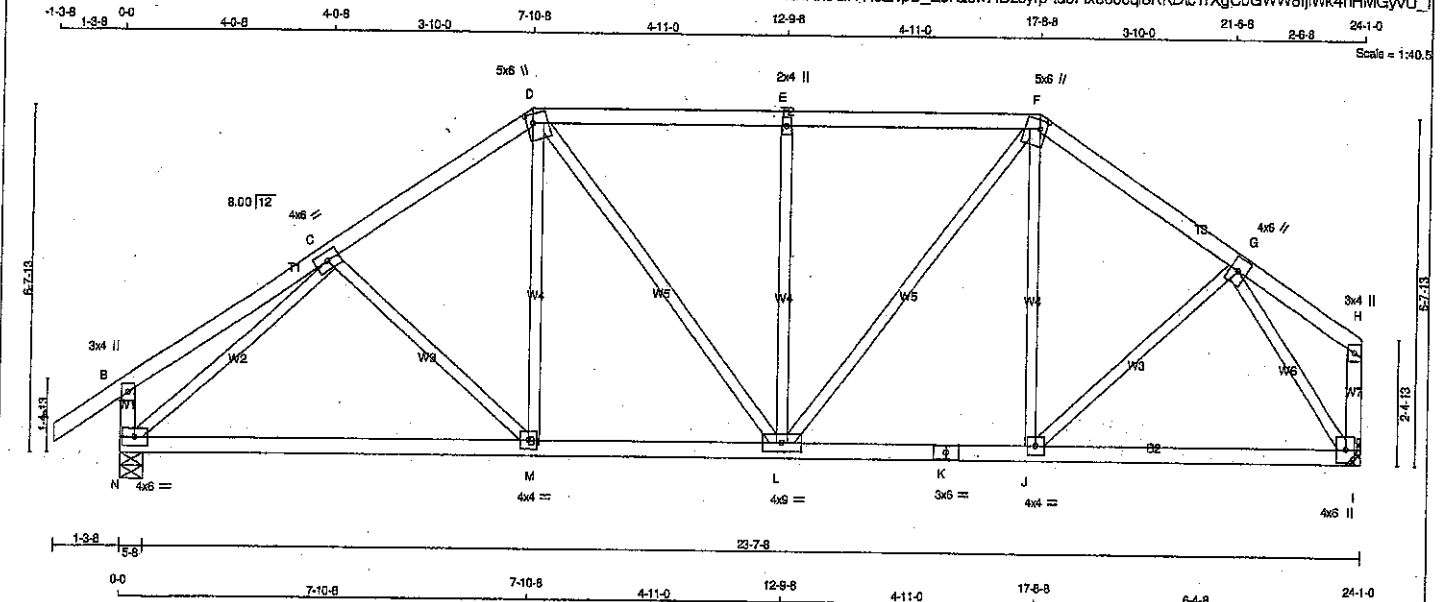


Structural component only
DWG# T-2123970 *BR*

JOB NAME 418361	TRUSS NAME T18	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:00:14 2021 Page 1
ID:Nx0GIYRcZvpB_E9rQcwHDzyp-ttoFxeoc8qj8KKDtrXgCOGWw8jWk4nHMgyvU_



TOTAL WEIGHT = 107 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
C	TMVW+T	MT20	4.0	6.0		
D	TTWW+M	MT20	5.0	6.0	2.00	1.50
E	TMVW+M	MT20	2.0	4.0		
F	TTWW+M	MT20	5.0	6.0	2.00	1.50
G	TMVW+T	MT20	4.0	6.0		
H	TMV+P	MT20	3.0	4.0		
I	BMVW+P	MT20	4.0	6.0		
J	BMVW+T	MT20	4.0	4.0		
K	BS+T	MT20	3.0	6.0		
L	BMVW+T	MT20	4.0	6.0		
M	BMVW+T	MT20	4.0	4.0		
N	BMVW+T	MT20	4.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
	VERT	HORZ	DOWN	HORZ
JT	1454	0	1454	0
N	1328	0	1328	0
I				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3'-8".

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1025	686 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
I	939	616 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8 0.12 (1)	10.00	C-M	-100 / 20	0.05 (1)	
B-C	0 / 22	-91.8 -91.8 0.22 (1)	10.00	M-D	0 / 226	0.05 (4)	
C-D	-1374 / 0	-91.8 -91.8 0.19 (1)	5.33	D-L	0 / 273	0.06 (1)	
D-E	-1294 / 0	-91.8 -91.8 0.30 (1)	5.33	L-E	-551 / 0	0.40 (1)	
E-F	-1294 / 0	-91.8 -91.8 0.30 (1)	5.33	L-F	0 / 490	0.11 (1)	
F-G	-1214 / 0	-91.8 -91.8 0.17 (1)	5.63	J-F	-47 / 60	0.03 (1)	
G-H	0 / 28	-91.8 -91.8 0.17 (1)	10.00	J-G	0 / 284	0.06 (1)	
N-B	-266 / 0	0.0 0.0 0.03 (1)	7.81	N-C	-1644 / 0	0.78 (1)	
I-H	-62 / 0	0.0 0.0 0.01 (1)	7.81	G-I	-1458 / 0	0.45 (1)	
N-M	0 / 1197	-18.5 -18.5 0.35 (4)	10.00				
M-L	0 / 1128	-18.5 -18.5 0.35 (4)	10.00				
L-K	0 / 993	-18.5 -18.5 0.28 (4)	10.00				
K-J	0 / 993	-18.5 -18.5 0.28 (4)	10.00				
J-I	0 / 798	-18.5 -18.5 0.24 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TG=0.30/1.00 (D-E:1), BC=0.35/1.00 (L-M:4),
WB=0.78/1.00 (C-N:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

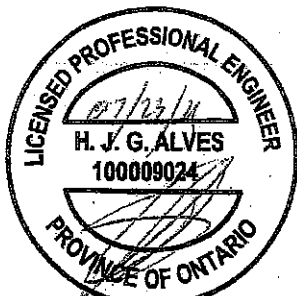
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	850	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (N) (INPUT = 0.80)
JSI METAL = 0.62 (G) (INPUT = 1.00)

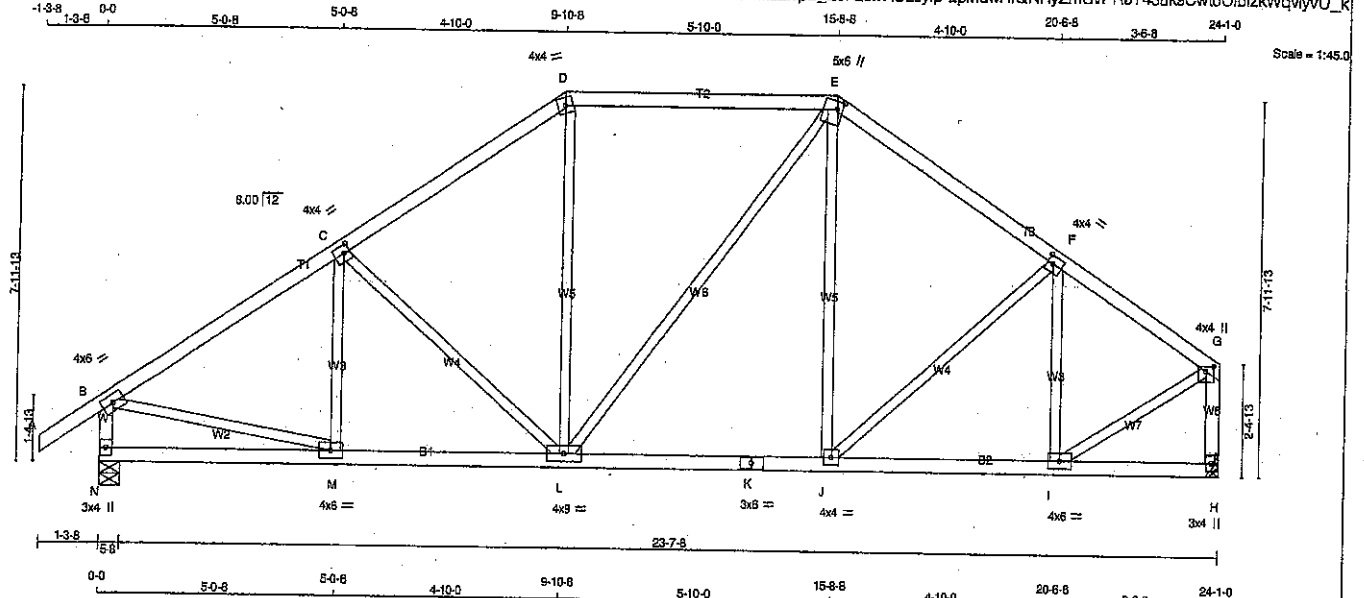


Structural component only
DWG# T-2123971

JOB NAME 418361	TRUSS NAME T19	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTK Industries, Inc. Thu Jul 22 16:00:15 2021 Page 1
ID:Nx0GIYRcZlvP8_E9rQcwHDzyp-LpMdwHQNRYZmUvPRJY43uK8CwtoOibzKWqiyvU_k



TOTAL WEIGHT = 108 lb [M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWV-m	MT20	5.0	6.0	2.00	1.50
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1-p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVWV-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMV1-p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1454	0	1454	0
H	1328	0	1328	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1026	688 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
H	939	618 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	M-C	-173 / 21	0.08 (1)					
B-C	-1484 / 0	-91.8	-91.8	0.32 (1)	5.05	C-L	-395 / 0	0.27 (1)					
C-D	-1245 / 0	-91.8	-91.8	0.31 (1)	5.41	L-D	0 / 242	0.05 (1)					
D-E	-1012 / 0	-91.8	-91.8	0.42 (1)	5.68	L-E	0 / 103	0.02 (1)					
E-F	-1187 / 0	-91.8	-91.8	0.26 (1)	5.61	J-E	0 / 110	0.04 (4)					
F-G	-1115 / 0	-91.8	-91.8	0.23 (1)	5.74	J-F	-7 / 6	0.01 (1)					
N-B	-1413 / 0	0.0	0.0	0.15 (1)	8.88	I-F	-480 / 0	0.18 (1)					
H-G	-1301 / 0	0.0	0.0	0.17 (1)	7.08	B-M	0 / 1288	0.29 (1)					
N-M	0 / 0	-18.5	-18.5	0.10 (4)	10.00	I-G	0 / 1107	0.25 (1)					
M-L	0 / 1259	-18.5	-18.5	0.28 (1)	10.00								
L-K	0 / 951	-18.5	-18.5	0.21 (1)	10.00								
K-J	0 / 951	-18.5	-18.5	0.21 (1)	10.00								
J-I	0 / 954	-18.5	-18.5	0.21 (1)	10.00								
I-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CS: TC=0.42/1.00 (D-E-1), BC=0.28/1.00 (L-M-1), WB=0.29/1.00 (B-M-1), SS=0.21/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	650	371	1747	788	1387 1873

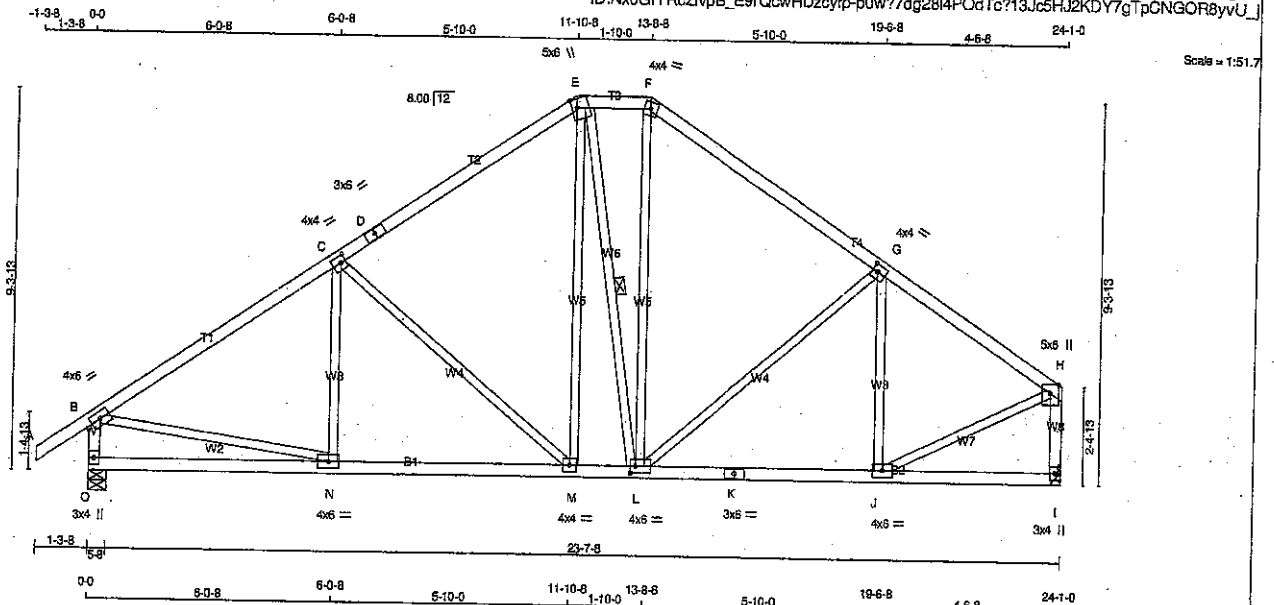
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.98 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123972



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - F	2x4	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
O - B	2x4	DRY	No.2	SPF		
I - H	2x4	DRY	No.2	SPF		
O - K	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	5.0		
C	TMVW-1	MT20	4.0	4.0	2.00	1.50
D	TS-1	MT20	3.0	8.0		
E	TTWW-m	MT20	5.0	6.0	2.50	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMVW-1	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1-p	MT20	3.0	4.0		
J	BMVW-1	MT20	4.0	6.0		
K	BS-1	MT20	3.0	8.0		
L	BMVW-1	MT20	4.0	6.0	2.00	1.50
M	BMVW-1	MT20	4.0	4.0		
N	BMVW-1	MT20	4.0	6.0		
O	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	VERT 1454	HORZ 0	DOWN 1454	UP 0
I	VERT 1328	HORZ 0	DOWN 1328	UP 0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3'-8".

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1028	885 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
I	939	818 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD (LC1)	MAX. UNBRACED LENGTH (LC)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	N-C	-98 / 89	0.04 (1)
B-C	-1485 / 0	-91.8	-91.8	0.47 (1)	C-M	-502 / 0	0.83 (1)
C-D	-1102 / 0	-91.8	-91.8	0.44 (1)	M-E	0 / 373	0.08 (1)
D-E	-1102 / 0	-91.8	-91.8	0.44 (1)	E-L	-66 / 0	0.04 (1)
E-F	-377 / 0	-91.8	-91.8	0.05 (1)	L-F	0 / 287	0.06 (1)
F-G	-1081 / 0	-91.8	-91.8	0.39 (1)	L-G	-203 / 0	0.25 (1)
G-H	-1197 / 0	-91.8	-91.8	0.35 (1)	J-G	-366 / 0	0.16 (1)
O-B	-1408 / 0	0.0	0.0	0.15 (1)	B-N	0 / 1288	0.29 (1)
I-H	-1295 / 0	0.0	0.0	0.16 (1)	J-H	0 / 1130	0.25 (1)
O-N	0 / 0	-18.5	-18.5	0.17 (4)			
N-M	0 / 1288	-18.5	-18.5	0.29 (1)			
M-L	0 / 889	-18.5	-18.5	0.21 (1)			
L-K	0 / 1028	-18.5	-18.5	0.24 (1)			
K-J	0 / 1028	-18.5	-18.5	0.24 (1)			
J-I	0 / 0	-18.5	-18.5	0.12 (4)			

TOTAL WEIGHT = 116 lb

[M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 33.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.47/1.00 (B-C:1), BC=0.29/1.00 (M-N:1), WS=0.89/1.00 (C-M:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.48 (H) (INPUT = 1.00)

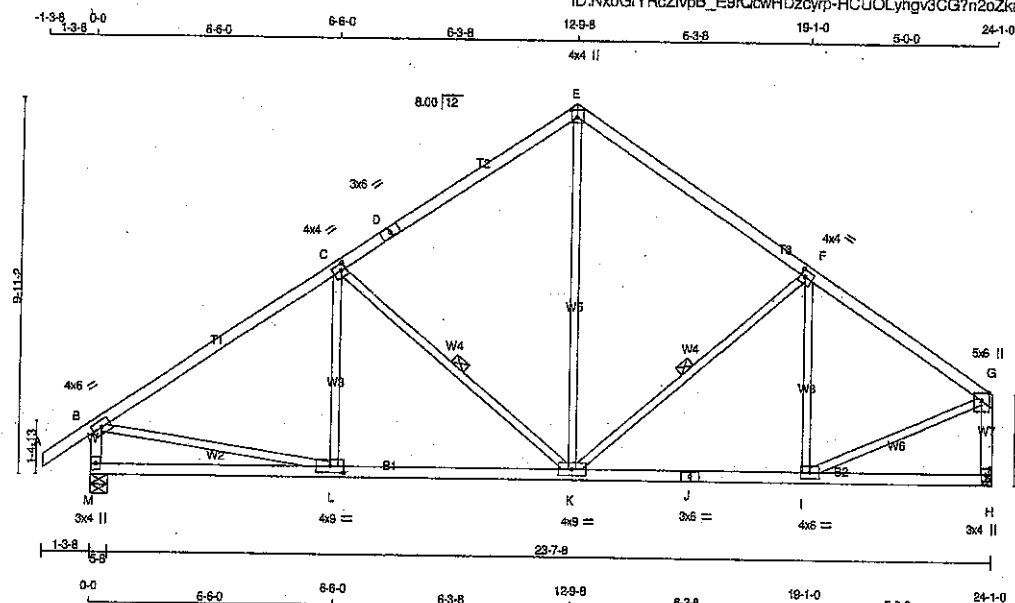


Structural component only
DWG# T-2123973

JOB NAME 418361	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:08:17 2021 Page 1
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Scale = 1:55.8

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	8.0	Edge	
H	BMVt-p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	9.0	2.00	4.26
N	BMVt-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
M	1454	0	1454	0	5-8	5-8
H	1328	0	1328	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1026	686 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0
H	939	616 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	L-C	-79 / 78	0.04 (1)
B-C	-1476 / 0	-91.8	-91.8	0.56 (1)	4.71	C-K	-562 / 0	0.28 (1)
C-D	-1044 / 0	-91.8	-91.8	0.51 (1)	5.44	K-E	0 / 667	0.15 (1)
D-E	-1044 / 0	-91.8	-91.8	0.51 (1)	5.44	K-F	-283 / 0	0.13 (1)
E-F	-1036 / 0	-91.8	-91.8	0.48 (1)	5.57	F-I	-323 / 0	0.16 (1)
F-G	-1220 / 0	-91.8	-91.8	0.41 (1)	5.29	B-L	0 / 1278	0.29 (1)
M-B	-1404 / 0	0.0	0.0	0.14 (1)	6.87	I-G	0 / 1136	0.28 (1)
H-G	-1292 / 0	0.0	0.0	0.16 (1)	7.10			
M-L	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
L-K	0 / 1261	-18.5	-18.5	0.29 (1)	10.00			
K-J	0 / 1049	-18.5	-18.5	0.25 (1)	10.00			
J-I	0 / 1049	-18.5	-18.5	0.25 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

TOTAL WEIGHT = 104 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.80")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.80")
CALCULATED VERT. DEFL. (TL) = L/998 (0.09")

CSI: TC=0.58/1.00 (B-C:1), BC=0.29/1.00 (K-L:1), WB=0.29/1.00 (B-L:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	680	371	1747
		789	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

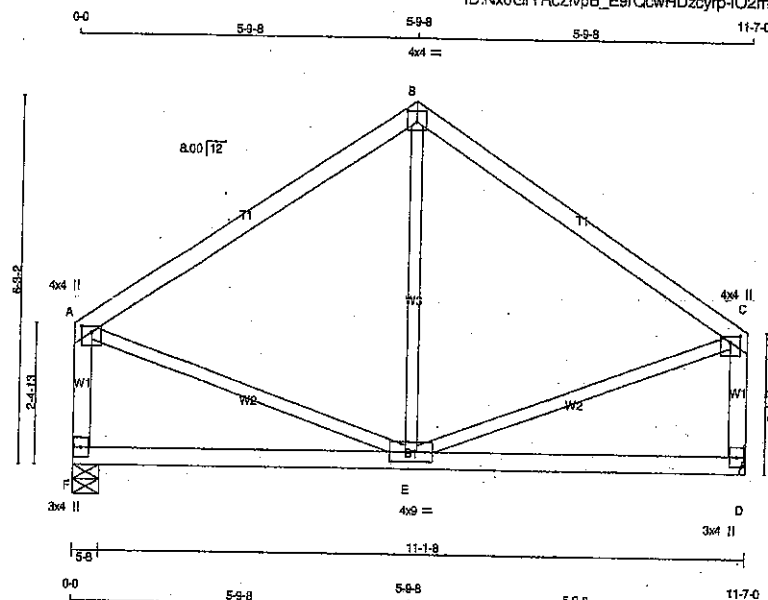
JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)



Structural component only
DWG# T-2123974

JOB NAME 418361	TRUSS NAME T22	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:36.0

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TTW+p	MT20	4.0	4.0	1.25 2.00
C	TMVW+p	MT20	4.0	4.0	1.25 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMVWW+1	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
F	639 0	639 0	5-8	5-8
D	639 0	639 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	452	298 / 0	0 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0
D	452	298 / 0	0 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	-389 / 0	-91.8	-91.8	0.40 (1)	6.25	E-B	-100 / 63	0.08 (1)	
B-C	-389 / 0	-91.8	-91.8	0.40 (1)	6.25	A-E	0 / 344	0.08 (1)	
F-A	-598 / 0	0.0	0.0	0.08 (1)	7.61	E-C	0 / 344	0.08 (1)	
D-C	-598 / 0	0.0	0.0	0.08 (1)	7.61				
F-E	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.18 (4)	10.00				

TOTAL WEIGHT = 3 X 48 = 144 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 8.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.40/1.00 (B-C:1), BC=0.18/1.00 (D-E:4), WB=0.08/1.00 (A-E:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

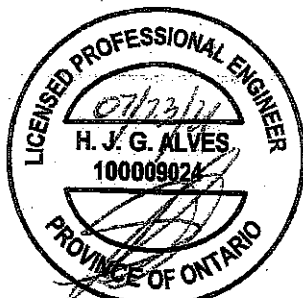
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (A) (INPUT = 0.80)

JSI METAL= 0.12 (A) (INPUT = 1.00)

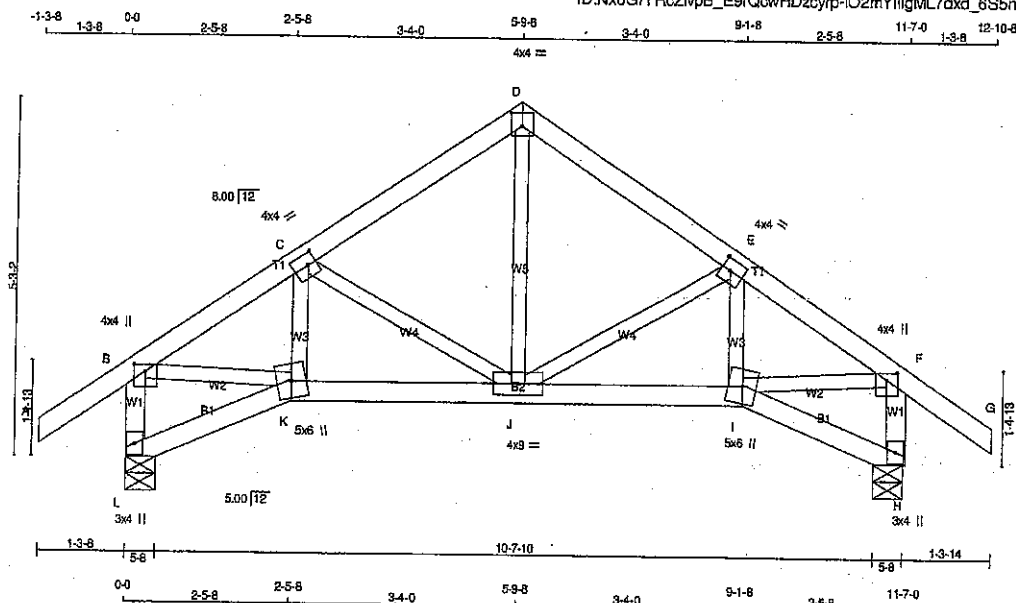


Structural component only
DWG# T-2123975

JOB NAME 418361	TRUSS NAME T23S	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1/31.1

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
L - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
L - K	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
I - H	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW+m	MT20	5.0	6.0		
J	BBWW-t	MT20	4.0	9.0		
K	BBWW+m	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
L	784	0	784	0
H	784	0	784	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): H

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
L	538	367 / 0	0 / 0	0 / 0	172 / 0	0 / 0
H	538	367 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LOAD LC1	MAX. CS1 (LC)	UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO							FR-TO			
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	J-D	0 / 363	0.08 (1)		
B-C	-817 / 0	-91.8	-91.8	0.09 (1)	6.25	J-E	-281 / 0	0.07 (1)		
C-D	-582 / 0	-91.8	-91.8	0.10 (1)	6.25	I-E	-20 / 30	0.01 (4)		
D-E	-582 / 0	-91.8	-91.8	0.10 (1)	6.25	C-J	-261 / 0	0.07 (1)		
E-F	-817 / 0	-91.8	-91.8	0.09 (1)	6.25	K-C	-20 / 30	0.01 (4)		
F-G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	B-K	0 / 698	0.16 (1)		
L-B	-742 / 0	0.0	0.0	0.08 (1)	7.81	I-F	0 / 698	0.16 (1)		
H-F	-742 / 0	0.0	0.0	0.08 (1)	7.81					
L-K	0 / 0	-18.5	-18.5	0.03 (4)	10.00					
K-J	0 / 695	-18.5	-18.5	0.14 (1)	10.00					
J-I	0 / 695	-18.5	-18.5	0.14 (1)	10.00					
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00					

TOTAL WEIGHT = 2 X 51 = 102 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.39")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/900$ (0.39")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CS1: TO=0.12/1.00 (F-G:1), BC=0.14/1.00 (I-J:1), WB=0.19/1.00 (F-I:1), GS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

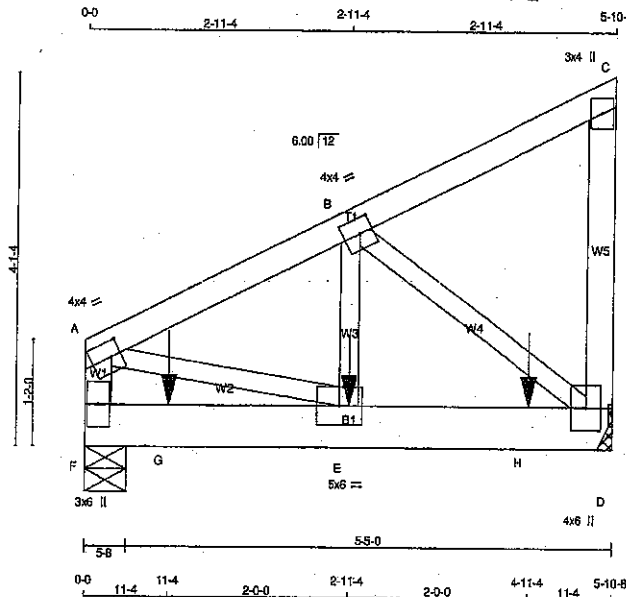
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.25 (I) (INPUT = 1.00)



Structural component only
DWG# T-2123976

JOB NAME 418361	TRUSS NAME T24	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:00:19 2021 Page 1	



LUMBER	N.L. & A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (in)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1 12	TOP	
A-C 1 12	TOP	
C-D 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 12	SIDE (0.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	4.0	4.0	2.00	1.25
B	TMWV-1	MT20	4.0	4.0	2.00	1.75
C	TMWV-1	MT20	3.0	4.0		
D	BMWV1-p	MT20	4.0	6.0		
E	BMWV1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UP/LIFT
F	588	0	5-8
D	713	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

1ST CASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW
F	411	298 / 0
D	497	388 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
F-A	-511 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 550	0.07 (1)
A-B	-581 / 0	-91.8	-91.8	0.07 (1)	6.25	E-B	0 / 343	0.04 (1)
B-C	-13 / 0	-91.8	-91.8	0.06 (1)	6.25	B-D	-569 / 0	0.08 (1)
D-C	-105 / 0	0.0	0.0	0.01 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.02 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.02 (1)	10.00			
E-H	0 / 531	-18.5	-18.5	0.10 (1)	10.00			
H-D	0 / 531	-18.5	-18.5	0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
E	2-11-4	-208	-208	---	TOP	VERT	TOTAL	---	C1
G	11-4	-58	-58	---	BACK	VERT	TOTAL	---	C1
H	4-11-4	-184	-184	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL (LL) = $L/999$ (0.00")
ALLOWABLE DEFL (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL (TL) = $L/999$ (0.01")

CSI: TC=0.07/1.00 (A-B-1), BC=0.10/1.00 (D-E-1), WS=0.08/1.00 (B-D-1), SS=0.07/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

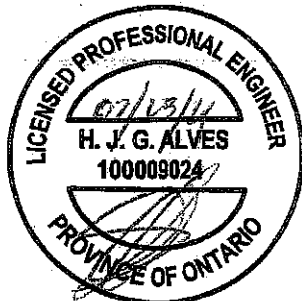
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	550	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.48 (A) (INPUT = 0.90)
JSI METAL = 0.12 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123977

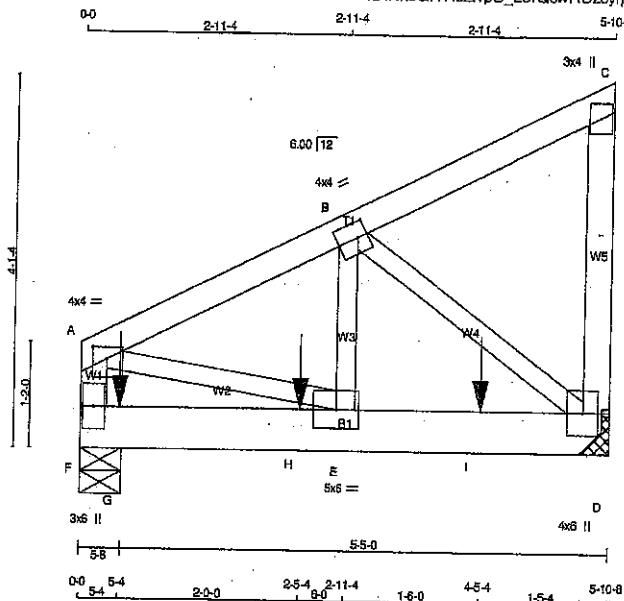
JOB NAME 418361	TRUSS NAME T24	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 16:00:19 2021 Page 2	
				ID: Nx0GYRcZlvB E8rQcwHDzcyr-Dbb8lejxRqT F5CBq9c0EjwweXHCk8iFuLU22TyvU g	

PLATES (table is in inches) <table border="1"> <thead> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>V</th> <th>X</th> </tr> </thead> <tbody> <tr> <td>F</td> <td>BMV1+p</td> <td>MT20</td> <td>3.0</td> <td>6.0</td> <td></td> <td></td> </tr> </tbody> </table> NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.	JT	TYPE	PLATES	W	LEN	V	X	F	BMV1+p	MT20	3.0	6.0				
JT	TYPE	PLATES	W	LEN	V	X										
F	BMV1+p	MT20	3.0	6.0												



Structural component only
DWG# T-2123977

JOB NAME 418362	TRUSS NAME T24Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 16:43:26 2021 Page 1			
		ID:Nx0GIYRcZivpB_E9rQowHDzcy/p-zHQ8JE0gBXOW6fTQjzXgaGY0aMA4Z7W1mKfRL0vTmF			



Scale = 1:23.3

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	5	SIDE (366.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.50 2.00
B	TMVW-l	MT20	4.0	4.0	2.00 1.25
C	TMV-p	MT20	3.0	4.0	
D	BMVW1-p	MT20	4.0	6.0	
E	BMVW-l	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
F	VERT	HORZ	DOWN	HORZ	UPLIFT
F	2627	0	2527	0	5-8
D	1953	0	1953	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	1855	1232 / 0	0 / 0	0 / 0	523 / 0	0 / 0
D	1379	917 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-A	-1433 / 0	0.0	0.0 0.08 (1)	A-E	0 / 1784
A-B	-1920 / 0	-91.8	-91.8 0.07 (1)	E-B	0 / 1871
B-C	-9 / 0	-91.8	-91.8 0.05 (1)	B-D	-2171 / 0
D-C	-115 / 0	0.0	0.0 0.01 (1)		
F-G	0 / 0	-18.5	-18.5 0.19 (1)		
G-H	0 / 0	-18.5	-18.5 0.19 (1)		
H-E	0 / 0	-18.5	-18.5 0.19 (1)		
E-I	0 / 1725	-18.5	-18.5 0.38 (1)		
I-D	0 / 1725	-18.5	-18.5 0.38 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-4	-928	-928		BACK	VERT	TOTAL		C1
H	2-5-4	-924	-924		BACK	VERT	TOTAL		C1
I	4-5-4	-924	-924		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 28 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/998 (0.01")
ALLOWABLE DEFL.(TL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TO=0.08/1.00 (A-F:1), BC=0.38/1.00 (D-E:1), WB=0.26/1.00 (B-D:1), SS=0.49/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

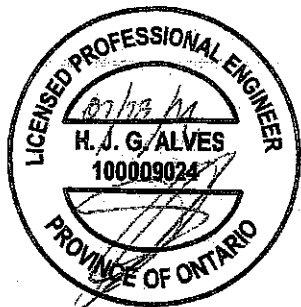
PLATE GRIP(DRY)	SHEAR	SECTOR
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)

JSI METAL= 0.38 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123990

CONTINUED ON PAGE 2

JOB NAME 418362	TRUSS NAME T24Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 15:49:26 2021 Page 2
 ID:Nx0GfYRcZlvpB E9rQcwHDzcyrp-zHQ8IE0gBXOw6fTOjzXgaGY0aMA4z7W1mKfRL0yvTMF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	8.0		

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

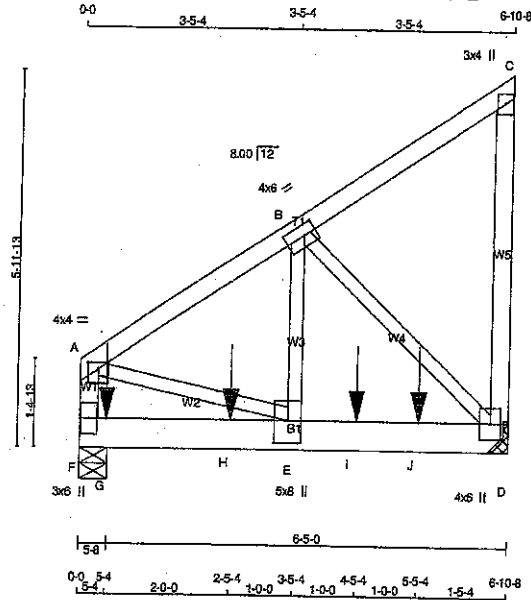


Structural component only
 DWG# T-2123990 *W*

JOB NAME 418361	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 15:00:20 2021 Page 1
ID:Nx0GIYRcZlpB_E9rQcwHdZcyp-hn9Wz_jZB_brsFnNe18FmxS5kxW53V2O6?EbavyvU_f



Scale = 1:33.6

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS		
F - A 2	2	SIDE (159.8)
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS		
F - D 2	11	SIDE (183.1)
WEBS : (0.122'X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.25 2.00
B	TMWW-t	MT20	4.0	6.0	2.00 2.25
C	TMVW-p	MT20	3.0	4.0	
D	BMVW1-p	MT20	4.0	6.0	
E	BMWW-t	MT20	5.0	8.0	4.25 2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
F	3193	0	3193	0	5-8	5-8		
D	2807	0	2807	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	2255	1488 / 0	0 / 0	0 / 0	0 / 0	757 / 0	0 / 0
D	1982	1316 / 0	0 / 0	0 / 0	0 / 0	863 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
F-A	-1949 / 0	0.0	0.11 (1)	7.81	A-E	0 / 1982	0.25 (1)		
A-B	-2272 / 0	-91.8	-91.8 0.10 (1)	5.83	E-B	0 / 2715	0.34 (1)		
B-C	-17 / 0	-91.8	-91.8 0.08 (1)	6.25	B-D	-2635 / 0	0.47 (1)		
D-C	-128 / 0	0.0	0.0 0.04 (1)	7.81					
F-G	0 / 0	-18.5	-18.5 0.40 (1)	10.00					
G-H	0 / 0	-18.5	-18.5 0.40 (1)	10.00					
H-E	0 / 0	-18.5	-18.5 0.40 (1)	10.00					
E-I	0 / 1904	-18.5	-18.5 0.57 (1)	10.00					
I-J	0 / 1904	-18.5	-18.5 0.57 (1)	10.00					
J-D	0 / 1904	-18.5	-18.5 0.57 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-4	-928	-928		BACK	VERT	TOTAL		C1
H	2-5-4	-924	-924		BACK	VERT	TOTAL		C1
I	4-5-4	-924	-924		BACK	VERT	TOTAL		C1
J	5-5-4	-924	-924		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 37 = 74 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.11/1.00 (A-F:1), BC=0.57/1.00 (D-E:1), WB=0.47/1.00 (B-D:1), SI=0.60/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.43 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123978

CONTINUED ON PAGE 2

JOB NAME 418361	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Thu Jul 22 16:00:20 2021 Page 2
ID:Nx0GIYRcZivbB E9rQcwHDzcvrp-hn9Wz JZB brsFnNEt6FmxS5kxW53V2O6?EbavvU f

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+u	MT20	3.0	5.0		

NOTES- (1)

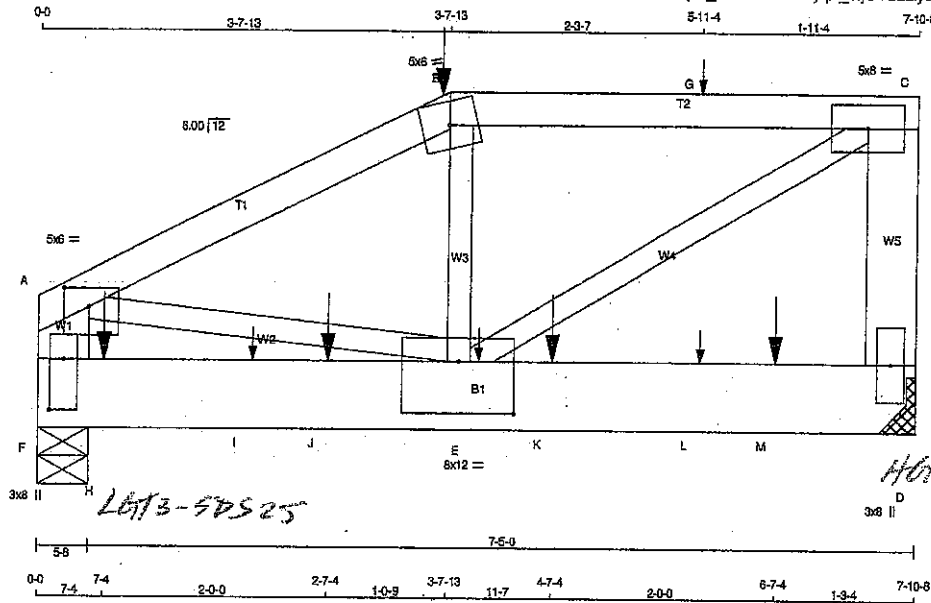
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2123978 *HR*

JOB NAME 418361	TRUSS NAME T26	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mtek Industries, Inc. Fri Jul 28 07:35:07 2021 Page 1
ID: Nx0GIYRcZlpB_E9rGowHDzcypr-?j04UzdiYAcDrNetsRYvXdyHr19vIX8zeXyvGII



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
F - A	2x6	DRY	No.2	SPF	
F - D	2x8	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	EXCEPT

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	SIDE(105.2)
B-C 1	12	SIDE(105.2)
C-D 2	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 3		SIDE(147.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	6938	0	6938	0
F	7979	0	7979	150

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 2181 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2486 LBS. FACTORED UPLIFT

PROVIDE FOR 150 LBS. FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
D	5110	3072 / 0	871 / 0
F	5885	3512 / 0	1020 / 0

HORIZONTAL REACTIONS	F	0 / 0	0 / 0	0 / 0	107 / -44	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (31)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-7115 / 2209	-115.2 -115.2	0.17 (2)	E-B	-817 / 2804	0.21 (3)	
B-G	-6576 / 2099	-115.2 -115.2	0.18 (1)	E-C	-2426 / 7786	0.58 (1)	
G-C	-6576 / 2099	-115.2 -115.2	0.18 (1)	A-E	-1940 / 6462	0.48 (1)	
D-C	-4398 / 1420	0.0	0.0				
F-A	-4560 / 1445	0.0	0.0				
				LENGTH			
F-H	-135 / 54	-39.5	-39.5	7.81			
H-I	-135 / 54	-39.5	-39.5	7.81			
I-J	-135 / 54	-39.5	-39.5				
J-E	-135 / 54	-39.5	-39.5				
E-K	-20 / 50	-39.5	-39.5				
K-L	-20 / 50	-39.5	-39.5				
L-M	-20 / 50	-39.5	-39.5				
M-D	-20 / 50	-39.5	-39.5				

SPECIFIED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	3-7-13	-8	-8	---	FRONT	VERT	DEAD	---	C1
B	3-7-13	1	1	97	BACK	VERT	TOTAL	---	C1
B	3-7-13	-26	-26	---	FRONT	VERT	SNOW	---	C1
E	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
G	5-11-4	1	1	97	BACK	VERT	TOTAL	---	C1
H	7-4	-2436	-2436	588	FRONT	VERT	TOTAL	---	C1
I	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
J	2-7-4	-2541	-2541	616	FRONT	VERT	TOTAL	---	C1
K	4-7-4	-2541	-2541	616	FRONT	VERT	TOTAL	---	C1
L	5-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
M	8-7-4	-2541	-2541	616	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE.

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2010, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/180 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.18/1.00 (B-C-1), BC=0.48/1.00 (D-E-1), WB=0.58/1.00 (C-E-1), SSI=0.81/1.00 (D-E-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

CONTINUED ON PAGE 2



Structural component only
DWG# T-2123979

JOB NAME 418361	TRUSS NAME T26	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 6.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 07:35:07 2021 Page 2
ID:Nx0GIYRcZlvpB E9:QcwhDzcyp- ?fiO4UzdwAc0rNetsRYvxDxyHr9viX8zeXyvGII

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-p	MT20	5.0	6.0	2.00	2.75
B	TTW-m	MT20	5.0	6.0		
C	TMW-l	MT20	5.0	6.0		
D	BMV1-p	MT20	3.0	8.0		
E	BMWW-l	MT20	8.0	12.0	5.50	6.00
F	BMV1-p	MT20	3.0	8.0	5.50	1.50

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBC 4.1.8.2.(B)**

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

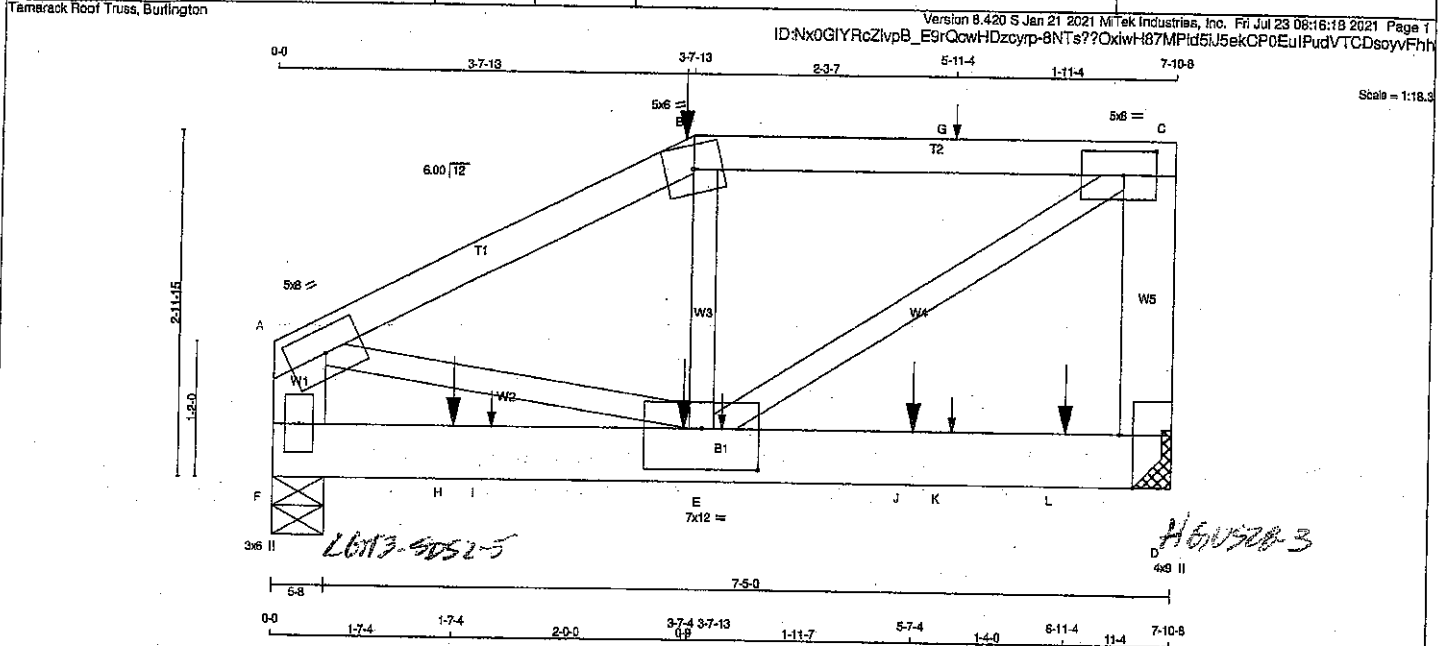
JSI GRIP= 0.89 (A) (INPUT = 0.90)

JSI METAL= 0.52 (F) (INPUT = 1.00)



Structural component only
DWG# T-2123979 *ML*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418362	T26Z	1	3	GREENPARK HOMES	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - B 2x4 DRY No.2 SPF B - C 2x4 DRY No.2 SPF D - C 2x6 DRY No.2 SPF F - A 2x6 DRY No.2 SPF F - D 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER. DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-B 1 12 SIDE (105.2) B-C 1 12 SIDE (105.2) C-D 2 12 TOP F-A 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS F-D 3 4 SIDE (2361.4) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REGRD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX D 8520 0 6520 0 -2677 MECHANICAL F 6551 0 6551 154 -2037 5-8 5-8 A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0. PROVIDE ANCHORAGE AT BEARING JOINT D FOR 2877 LBS. FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT F FOR 2037 LBS. FACTORED UPLIFT PROVIDE FOR 154 LBS. FACTORED HORIZONTAL REACTION AT JOINT F UNFACTORED REACTIONS 1ST LCASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL D 8274 3778 / 0 1087 / 0 0 / 0 518 / -2603 1431 / 0 0 / 0 F 4824 2903 / 0 821 / 0 0 / 0 417 / -1986 1100 / 0 0 / 0 HORIZONTAL REACTIONS F 0 / 0 0 / 0 0 / 0 110 / -47 0 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F BRACING MAX. UNBRACED TOP CHORD LENGTH = 4.15 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (31) CHORDS MAX. FACTORED FACTORED WEBS MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. UNBRAC. MEMB. FORCE MAX. (LBS) (PLF) CSI (LC) LENGTH FR-TO (LBS) CSI (LC) FR-TO A-B -7834 / 2438 -115.2 -115.2 0.19 (2) 4.15 E-B -916 / 3128 0.23 (3) B-G -7232 / 2307 -115.2 -115.2 0.20 (1) 4.27 E-C -2691 / 8630 0.65 (1) G-C -7232 / 2307 -115.2 -115.2 0.20 (1) 4.27 A-E -2163 / 77140 0.54 (1) D-C -4938 / 1590 0.0 0.0 0.15 (1) 7.68 F-A -5133 / 1624 0.0 0.0 0.11 (1) 7.57 F-H -137 / 58 -39.5 -39.5 0.58 (3) 6.25 H-I -137 / 58 -39.5 -39.5 0.58 (3) 6.25 I-E -137 / 58 -39.5 -39.5 0.58 (3) 6.25 E-J -20 / 52 -39.5 -39.5 0.77 (1) 6.25 J-K -20 / 52 -39.5 -39.5 0.77 (1) 6.25 K-L -20 / 52 -39.5 -39.5 0.77 (1) 6.25 L-D -20 / 52 -39.5 -39.5 0.77 (1) 6.25 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. B 3-7-13 -6 -6 FRONT VERT DEAD C1 B 3-7-13 1 1 BACK VERT TOTAL C1 B 3-7-13 -26 -26 FRONT VERT SNOW C1 E 3-7-4 -2541 -2541 616 FRONT VERT TOTAL C1 E 3-11-4 1 -1 BACK VERT TOTAL C1 G 5-11-4 1 -11 97 BACK VERT TOTAL C1 H 1-7-4 -2541 -2541 616 FRONT VERT TOTAL C1 I 1-11-4 1 1 BACK VERT TOTAL C1 J 5-7-4 -2541 -2541 616 FRONT VERT TOTAL C1 K 5-11-4 1 1 BACK VERT TOTAL C1 L 6-11-4 -2542 -2542 616 FRONT VERT TOTAL C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 33.4 PSF LL = 8.0 PSF BOT CH. LL = 10.5 PSF LL = 7.4 PSF TOTAL LOAD = 57.3 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 *** NON STANDARD GIRDER *** ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 4 OF NBCC 2015, ABC 2019 - PART 4 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED. (80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.25") CALCULATED VERT. DEFL.(LL) = L/999 (0.07") ALLOWABLE DEFL.(TL) = L/180 (0.52") CALCULATED VERT. DEFL.(TL) = L/999 (0.09") CSI: TC=0.20/1.00 (B-C:1), BC=0.77/1.00 (D-E:1), WB=0.65/1.00 (C-E:1), SSI=0.72/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL) (PL) MAX MIN MAX MIN MAX MIN MT20 850 371 1747 788 1887 1873			
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Structural component only
DWG# T-2123991

JOB NAME 418362	TRUSS NAME T26Z	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 08:16:18 2021 Page 2 ID:Nx0GIYRcZivpB E9rQcwHDzcvp-8NTs??OxiwH87MPId5U5ekCP0EuiPudVTCDsoyvfHh	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	8.0		
B	TTW-m	MT20	5.0	6.0		
C	TMVW-1	MT20	5.0	8.0	2.50	3.50
D	BMV1-t	MT20	4.0	9.0	Edge	1.50
E	BMVWW-t	MT20	7.0	12.0	4.25	6.00
F	BMV1-sp	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCq, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (C) (INPUT = 0.80)

JSI METAL = 0.47 (A) (INPUT = 1.00)

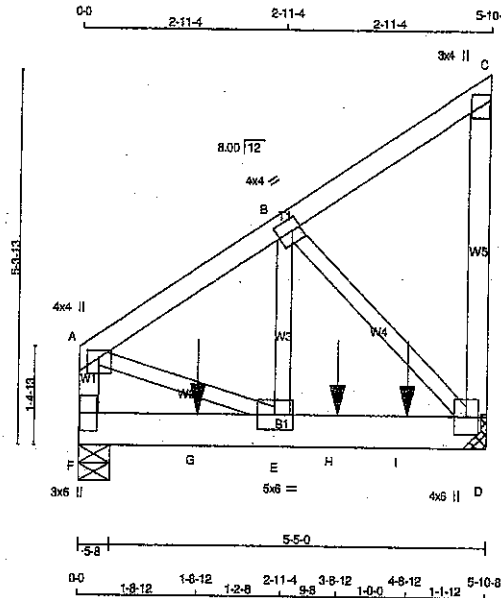


Structural component only
DWG# T-2123991

JOB NAME 418361	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 32 = 64 lb

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMVW+p MT20	4.0	4.0	1.25	2.00
B TMVW-i MT20	4.0	4.0	2.00	1.50
C TMVW+p MT20	3.0	4.0		
D BMVW1+p MT20	4.0	6.0		
E BMVW-i MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1109	1109	0	5-8
F D	1399	1399	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	782	526 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0
D	986	564 / 0	0 / 0	0 / 0	0 / 0	322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
F-A	-952 / 0	0.0	0.0	0.05 (1)	7.81	A-E	0 / 887	0.11 (1)	
A-B	-984 / 0	-91.8	-91.8	0.07 (1)	6.25	E-B	0 / 1079	0.13 (1)	
B-C	-16 / 0	-91.8	-91.8	0.08 (1)	6.25	B-D	-1192 / 0	0.17 (1)	
D-C	-107 / 0	0.0	0.0	0.02 (1)	7.81				
F-G	0 / 0	-18.5	-18.5	0.14 (1)	10.00				
G-E	0 / 0	-18.5	-18.5	0.14 (1)	10.00				
E-H	0 / 840	-18.5	-18.5	0.24 (1)	10.00				
H-I	0 / 840	-18.5	-18.5	0.24 (1)	10.00				
I-D	0 / 840	-18.5	-18.5	0.24 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-8-12	-437	-437	---	FRONT	VERT	TOTAL	---	C1
H	3-8-12	-437	-437	---	FRONT	VERT	TOTAL	---	C1
I	4-8-12	-437	-437	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.07/1.00 (A-B:1), BC=0.24/1.00 (D-E:1),
WB=0.17/1.00 (B-D:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

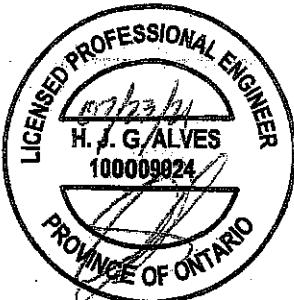
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.64 (B) (INPUT = 0.60)
JSI METAL = 0.19 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123980

CONTINUED ON PAGE 2

JOB NAME 418361	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
F BMV1+p MT20 3.0 6.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

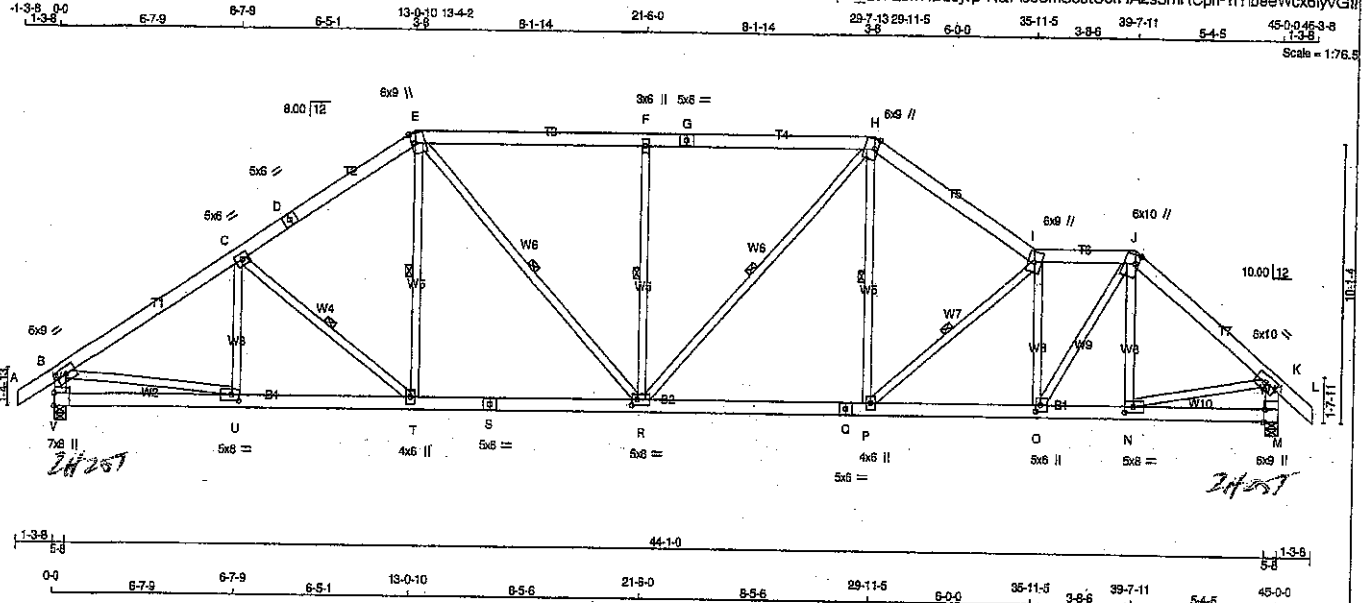


Structural component only
DWG# T-2123980 212

JOB NAME 418361	TRUSS NAME T29	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	8.0	9.0		
C TMVW-t	MT20	5.0	6.0		
D TS-t	MT20	5.0	6.0		
E TTWW+m	MT20	6.0	9.0	4.25	1.50
F TMVW-w	MT20	3.0	6.0		
G TS-t	MT20	5.0	6.0		
H TTWW+m	MT20	6.0	9.0	4.25	1.50
I TTWW+m	MT20	6.0	9.0		
J TTWW+m	MT20	6.0	10.0	3.75	1.50
K TMVW-t	MT20	5.0	10.0		
M BMV1-t	MT20	6.0	9.0	Edge 0.50	
N BMVW-t	MT20	5.0	8.0	2.50	3.50
O BMVW-t	MT20	5.0	6.0	2.75	2.00
P BMVW-t	MT20	4.0	6.0		
Q BS-t	MT20	5.0	6.0		
R BMVW-t	MT20	5.0	8.0	2.50	2.50
S BS-t	MT20	5.0	6.0		
T BMVW-t	MT20	4.0	6.0		
U BMVW-t	MT20	5.0	8.0	2.50	3.00
V BMV1-t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT GROSS REACTION	REQD BRG
JT VERT				
V	3845	0	3845	443
M	3648	0	3648	0

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1108 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1126 LBS. FACTORED UPLIFT

PROVIDE FOR 443 LBS. FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
V	2891	1589 / 0	473 / 0	0 / 0	89 / -1091	820 / 0	0 / 0
M	2693	1800 / 0	473 / 0	0 / 0	136 / -1103	820 / 0	0 / 0

HORIZONTAL REACTIONS

V	0 / 0	0 / 0	0 / 0	316 / -315	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.19 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, E-T, H-P, I-P, H-R, E-R, F-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	MEMB.	MEMB.
	FR-TO	FR-TO
A-B	0 / 49	-115.2 -115.2 0.09 (2)
B-C	-4543 / 1411	-115.2 -115.2 0.58 (2)
C-D	-4098 / 1424	-115.2 -115.2 0.54 (2)
D-E	-4098 / 1424	-115.2 -115.2 0.54 (2)
E-F	-4174 / 1519	-115.2 -115.2 0.87 (1)
F-G	-4175 / 1519	-115.2 -115.2 0.87 (1)
G-H	-4175 / 1519	-115.2 -115.2 0.87 (1)
H-I	-4364 / 1508	-115.2 -115.2 0.52 (3)
I-J	-4413 / 1516	-115.2 -115.2 0.22 (1)
J-K	-3866 / 1220	-115.2 -115.2 0.39 (1)
K-L	0 / 59	-115.2 -115.2 0.09 (1)
V-B	-3526 / 1129	0.0 0.0 0.23 (1)
M-K	-3559 / 1148	0.0 0.0 0.23 (1)
V-U	-425 / 433	-39.5 -39.5 0.14 (4)
U-T	-1218 / 3818	-39.5 -39.5 0.58 (1)
T-S	-866 / 3375	-39.5 -39.5 0.54 (1)
S-R	-866 / 3375	-39.5 -39.5 0.54 (1)
R-Q	-790 / 3648	-39.5 -39.5 0.57 (1)
Q-P	-790 / 3648	-39.5 -39.5 0.57 (1)
P-O	-1119 / 4470	-39.5 -39.5 0.86 (1)
O-N	-668 / 2951	-39.5 -39.5 0.42 (1)
N-M	-10 / 22	-39.5 -39.5 0.09 (17)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.8.2 (6)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 32.4	PSF
DL	= 6.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 51.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR, EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.87/1.00 (F-H:1), SC=0.86/1.00 (O-P:1), WB=0.80/1.00 (I-O:1), SS=0.59/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (U) (INPUT = 0.93)
JSI METAL = 0.77 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2123981 1/2

JOB NAME 418361	TRUSS NAME T29	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2123981 *1/1*

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0	3.00	3.75
C	TTWW-m	MT20	8.0	12.0	Edge	8.25
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	TTWW-m	MT20	8.0	12.0	Edge	8.25
L	TMWW-t	MT20	6.0	9.0	3.00	3.75
N	BMV-t	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	8.0	2.50	2.00
P	BMWW-t	MT20	5.0	8.0	2.50	2.25
Q	BS-t	MH16	5.0	12.5		
R	BMWW-t	MT20	6.0	9.0	4.50	2.50
S	BMWW-t	MT20	5.0	8.0		
T	BMWW-t	MT20	6.0	9.0	4.50	2.50
U	BS-t	MH16	5.0	12.5		
V	BMWW-t	MT20	5.0	8.0	2.50	2.25
W	BMWW-t	MT20	5.0	8.0	2.50	2.00
X	BMV-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-10	-40	-40		FRONT	VERT	DEAD		C1
C	4-0-10	-65	-105		BACK	VERT	TOTAL		C1
C	4-0-10	-171	-171		FRONT	VERT	SNOW		C1
T	18-9-8	-1364	-1364		BACK	VERT	TOTAL		C1
Y	14-10-4	-60	-60		BACK	VERT	TOTAL		C1
Z	16-10-4	-60	-60		BACK	VERT	TOTAL		C1
AA	1-11-4	-21	-21		BACK	VERT	TOTAL		C1
AB	2-7-4	-21	-21		BACK	VERT	TOTAL		C1
AC	3-10-4	-21	-21		BACK	VERT	TOTAL		C1
AD	4-8-8	-771	-771		BACK	VERT	TOTAL		C1
AE	13-5-8	-771	-771		BACK	VERT	TOTAL		C1
AF	14-10-4	-21	-21		BACK	VERT	TOTAL		C1
AG	16-10-4	-21	-21		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (L) (INPUT = 0.80)
JSI METAL= 0.78 (U) (INPUT = 1.00)



Structural component only
DWG# T-2123992 2/2

JOB NAME 418362	TRUSS NAME T31	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:43:31 2021 Page 2 ID:Nx0GIYRcZ/vpB E9rQcwHDzcyrp-KFD1mx4e031DCQMLWX7rHKGggNoqelH9mwcNC1EvyTMA	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TTWW+m	MT20	5.0	6.0		
E	TTW+m	MT20	4.0	6.0	Edge	
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	6.0		
J, K, N, O						
J	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	8.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

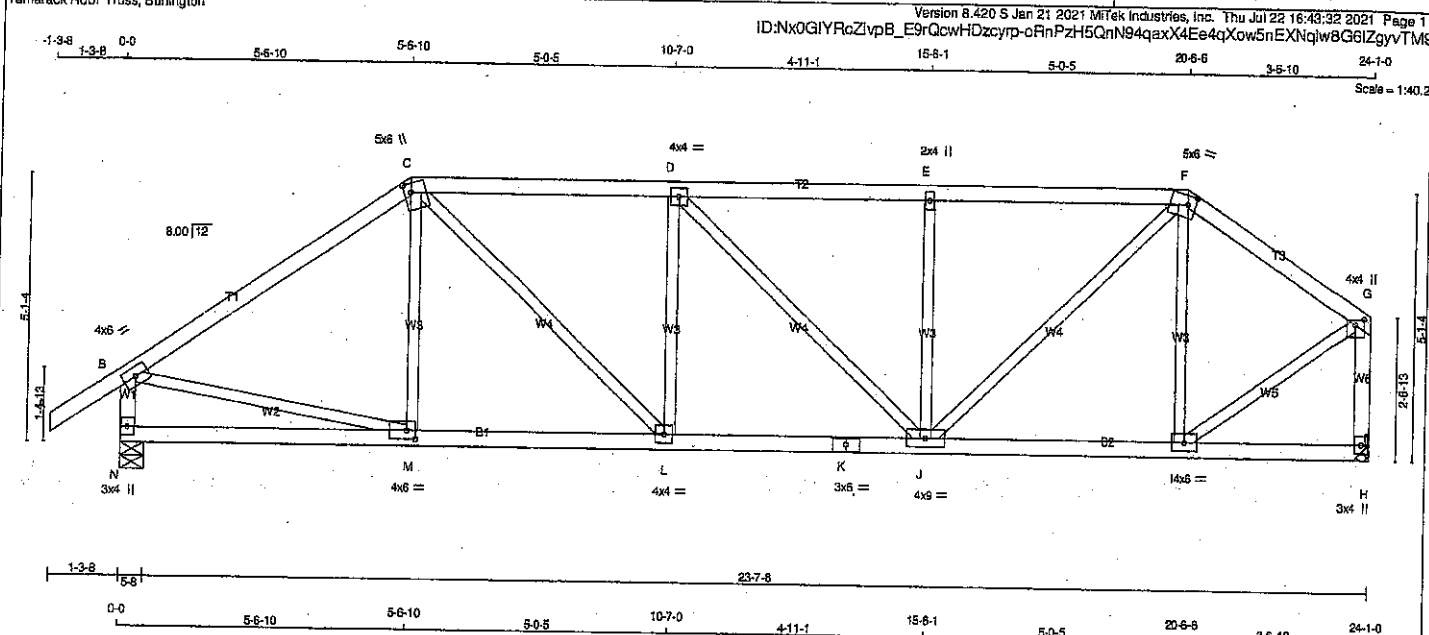
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2123993 *mm*



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
O - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.75
G	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1-p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	6.0	2.00	2.00
N	BMV1-p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral bracing to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1454	0	1454	0	5-8	5-8
H	1328	0	1328	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SO
N	1028	888 / 0	0 / 0	0 / 0	0 / 0	399 / 0	0 /
H	939	618 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 /

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	M-C	-116 / 47 0.04 (1)	
B-C	-1441 / 0	-91.8 -91.8	0.59 (1)	4.69	C-L	0 / 881 0.15 (1)	
C-D	-1887 / 0	-91.8 -91.8	0.36 (1)	4.81	L-D	-385 / 0 0.15 (1)	
D-E	-1574 / 0	-91.8 -91.8	0.35 (1)	4.85	D-J	-158 / 0 0.14 (1)	
E-F	-1574 / 0	-91.8 -91.8	0.35 (1)	4.88	J-E	-494 / 0 0.19 (1)	
F-G	-1033 / 0	-91.8 -91.8	0.22 (1)	5.90	J-F	0 / 1005 0.23 (1)	
N-B	-1410 / 0	0.0 0.0	0.15 (1)	6.87	I-F	-474 / 0 0.18 (1)	
H-G	-1304 / 0	0.0 0.0	0.19 (1)	7.07	B-M	0 / 1219 0.27 (1)	
					I-G	0 / 1027 0.28 (1)	
N-M	0 / 0	-18.5 -18.5	0.13 (4)	10.00			
M-L	0 / 1185	-18.5 -18.5	0.28 (1)	10.00			
L-K	0 / 1887	-18.5 -18.5	0.31 (1)	10.00			
K-J	0 / 1887	-18.5 -18.5	0.31 (1)	10.00			
J-I	0 / 847	-18.5 -18.5	0.19 (1)	10.00			
I-H	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.59/1.00 (B-C:1), BC=0.31/1.00 (J-L:1),
WB=0.27/1.00 (B-M:1), SSI=0.21/1.00 (C-D:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

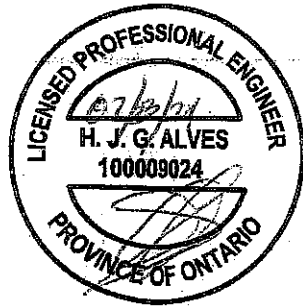
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

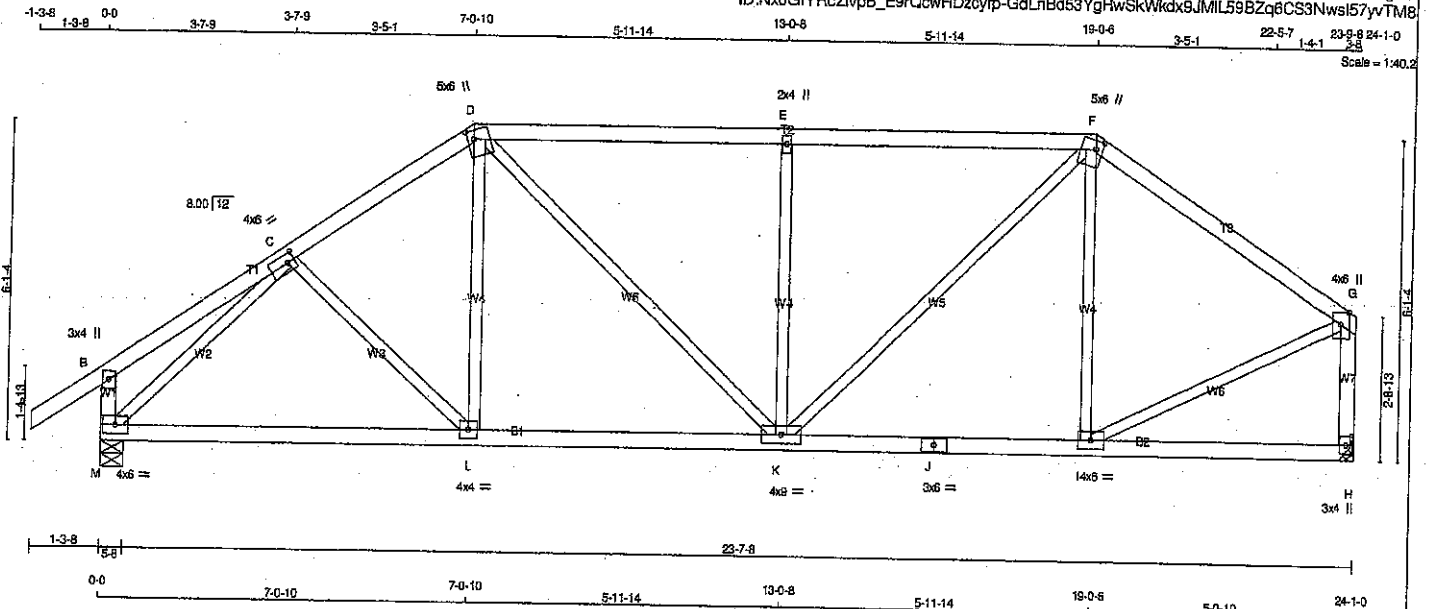
JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.53 (K) (INPUT = 1.00)



Structural component only
DWG# T-2123994

JOB NAME 418362	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 5 Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:43:33 2021 Page 1	

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	5.0	2.00	1.75
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	5.0		
K	BMVW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	HORIZ	BRG	BRG
M	1454	0	0	5-8	5-8
H	1328	0	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
M	1026	685 / 0	0 / 0	0 / 0	339 / 0	0 / 0
H	939	616 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
	FORCE (LBS)		FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 35	C-L	-31 / 38
B-C	0 / 19	L-D	0 / 163
C-D	-1410 / 0	D-K	0 / 392
D-E	-1439 / 0	K-E	-675 / 0
E-F	-1439 / 0	E-F	0 / 715
F-G	-1121 / 0	I-F	-318 / 0
G-H	-253 / 0	I-G	0 / 1028
H-G	-1291 / 0	M-C	-1644 / 0
M-L	0 / 1178		
L-K	0 / 1158		
K-J	0 / 928		
J-I	0 / 926		
I-H	0 / 0		

TOTAL WEIGHT = 102 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCG 2018, ABC 2019
- PART 9 OF CBCG 2012 (2019 AMENDMENT)
- CSA 086-14
- TPOC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.83/1.00 (E-F:1), BC=0.31/1.00 (K-L:4),
WB=0.63/1.00 (C-M:1), SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

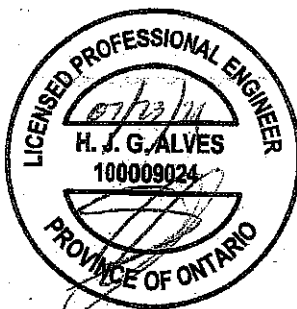
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

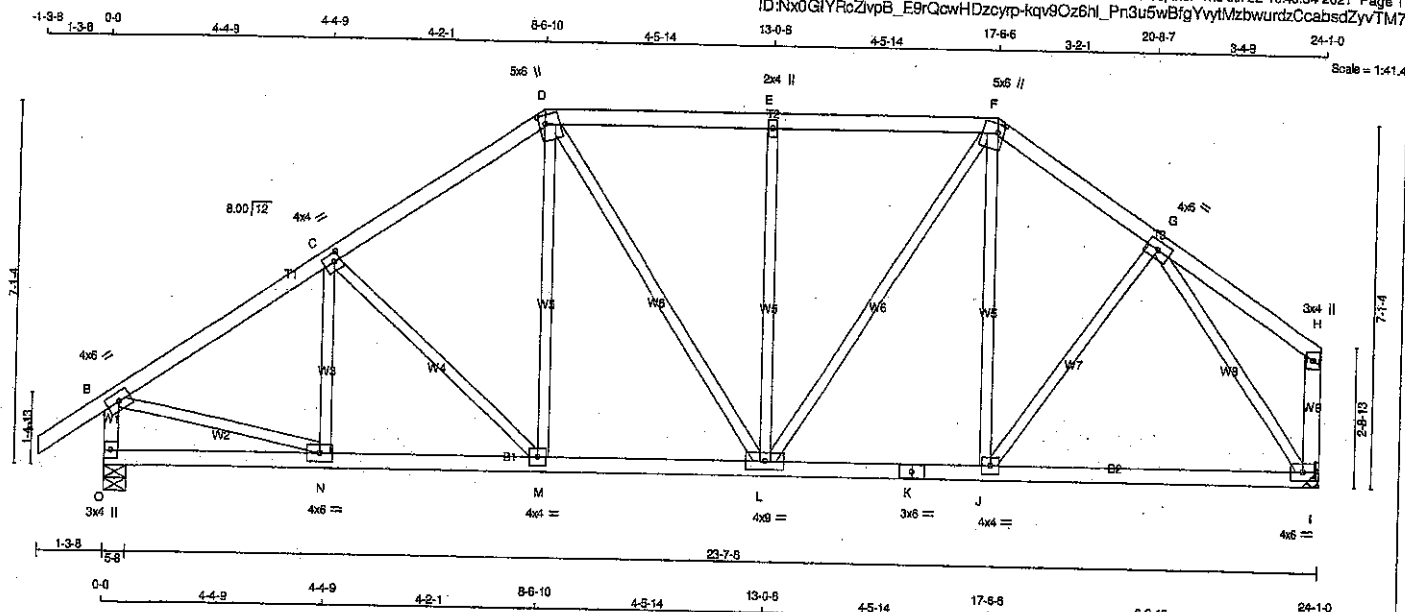
JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL = 0.57 (G) (INPUT = 1.00)



Structural component only
DWG# T-2123995

JOB NAME 418362	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Nx0GIYRcZivpB_E9rQcwHDzoyrp-kqv9Cz6hl_Pn3u5wBfgYvYlMzbwurdzCcabsdZyvTM7



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	6.0		
C	TMVW-I	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-I	MT20	4.0	6.0		
H	TMVW-p	MT20	3.0	4.0		
I	BMVW-I	MT20	4.0	6.0		
J	BMVW-I	MT20	4.0	4.0		
K	BS-I	MT20	3.0	6.0		
L	BMVW-w	MT20	4.0	9.0		
M	BMVW-I	MT20	4.0	4.0		
N	BMVW-I	MT20	4.0	6.0		
O	BMVW-p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD		
JT	GROSS REACTION	VERT	HORIZ	GROSS REACTION	DOWN	HORIZ	UPLIFT	BRG	BRG
O	1454	0	1454	0	0	5-8	5-8		
I	1328	0	1328	0	0	MECHANICAL	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-9.

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	2ND CASE	MAX/MIN. COMPONENT REACTIONS
O	1026	686 / 0	0 / 0	0 / 0
I	939	615 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 35	-91.8	-91.8 0.12 (1)	N-C	-224 / 0
B-C	-1470 / 0	-91.8	-91.8 0.24 (1)	C-M	-225 / 0
C-D	-1325 / 0	-91.8	-91.8 0.23 (1)	M-D	0 / 241
D-E	-1199 / 0	-91.8	-91.8 0.25 (1)	D-L	0 / 211
E-F	-1199 / 0	-91.8	-91.8 0.25 (1)	L-E	-501 / 0
F-G	-1155 / 0	-91.8	-91.8 0.13 (1)	L-F	0 / 484
G-H	0 / 18	-91.8	-91.8 0.15 (1)	J-F	-31 / 55
O-B	-1418 / 0	0.0	0.0 0.15 (1)	J-G	0 / 196
I-H	-118 / 0	0.0	0.0 0.02 (1)	B-N	0 / 1287
O-N	0 / 0	-18.5	-18.5 0.07 (4)	G-I	-1424 / 0
N-M	0 / 1244	-18.5	-18.5 0.24 (1)		
M-L	0 / 1083	-18.5	-18.5 0.22 (1)		
L-K	0 / 943	-18.5	-18.5 0.26 (4)		
K-J	0 / 943	-18.5	-18.5 0.26 (4)		
J-I	0 / 828	-18.5	-18.5 0.24 (4)		

TOTAL WEIGHT = 112 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPO 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TO=0.25/1.00 (D-E-I), BC=0.26/1.00 (J-L-I), WB=0.74/1.00 (G-I), SS=0.20/1.00 (E-F-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)

JSI METAL= 0.36 (B) (INPUT = 1.00)

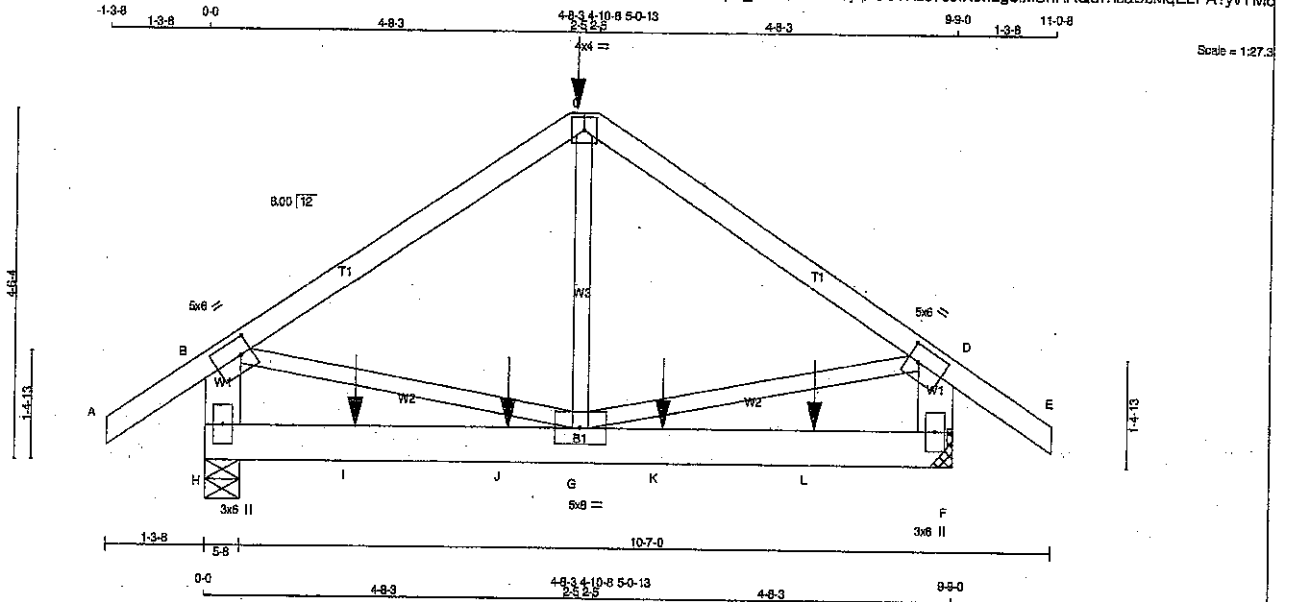


Structural component only
DWG# T-2123996

JOB NAME 418362	TRUSS NAME T35	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x8	DRY	No.2
H - F	2x8	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	6.0	2.50	1.75
C	TTW-p	MT20	4.0	4.0		
D	TMVW-1	MT20	5.0	6.0	2.50	1.75
F	BMV1+p	MT20	3.0	6.0		
G	BMVWV-1	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	877	0	877	0
F	876	0	876	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	617	424 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0	0 / 0
F	618	424 / 0	0 / 0	0 / 0	0 / 0	194 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	B-G	0 / 583
B-C	-682 / 0	-91.8	-91.8	0.43 (1)	6.25	G-D	0 / 583
C-D	-682 / 0	-91.8	-91.8	0.43 (1)	8.25	G-C	-87 / 91
D-E	0 / 35	-91.8	-91.8	0.14 (1)	10.00		
H-B	-830 / 0	0.0	0.0	0.06 (1)	7.81		
F-D	-830 / 0	0.0	0.0	0.06 (1)	7.81		
H-I	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
I-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
J-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
G-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
K-L	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
L-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-10-8	-50	-50		FRONT	VERT	DEAD		C1
C	4-10-8	-208	-208		FRONT	VERT	SNOW		C1
I	1-11-4	-11	-11		BACK	VERT	TOTAL		C1
J	3-11-4	-11	-11		BACK	VERT	TOTAL		C1
K	5-11-4	-11	-11		BACK	VERT	TOTAL		C1
L	7-11-4	-11	-11		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 48 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL =	6.0 PSF
BOT CH.	LL = 0.0 PSF
DL =	7.4 PSF
TOTAL LOAD =	39.0 PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL (LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.43/1.00 (B-C:1), BC=0.08/1.00 (G-H:4), WB=0.14/1.00 (B-G:1), SS=0.16/1.00 (B-C:1)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

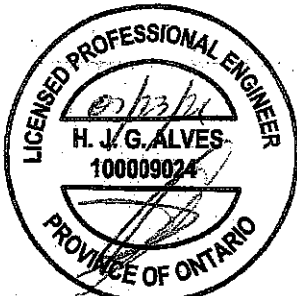
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.44 (B) (INPUT = 0.50)
JSI METAL = 0.19 (D) (INPUT = 1.00)

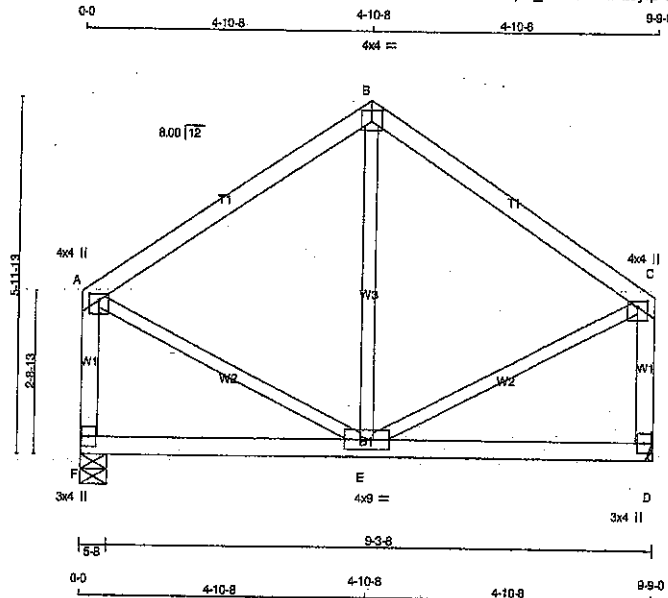


Structural component only
DWG# T-2123997

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
418362	T36	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:35.5

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TTW+p	MT20	4.0	4.0	2.25 2.00
C	TMVW+p	MT20	4.0	4.0	1.25 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMVWW-t	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	537 0	537 0	5-8	5-8
D	537 0	537 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LOADING	MAX/MIN. COMPONENT REACTIONS
F	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
D	380	249 / 0 0 / 0 0 / 0 0 / 0 131 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO						FR-TO			
A-B	-289 / 0	-91.8	-91.8	0.28 (1)	6.25	E-B	-127 / 37	0.07 (1)	0.07 (1)
B-C	-289 / 0	-91.8	-91.8	0.28 (1)	6.25	A-E	0 / 288	0.08 (1)	0.08 (1)
F-A	-503 / 0	0.0	0.0	0.07 (1)	7.81	E-C	0 / 288	0.08 (1)	0.08 (1)
D-C	-503 / 0	0.0	0.0	0.07 (1)	7.81				
F-E	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

TOTAL WEIGHT = 2 X 43 = 85 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL	LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.28/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.07/1.00 (B-E:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

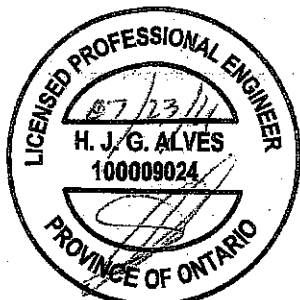
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(FS)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN			
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (A) (INPUT = 0.90)
JSI METAL= 0.10 (A) (INPUT = 1.00)

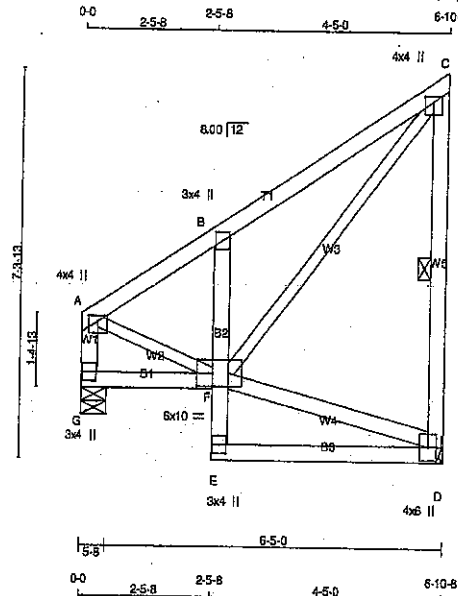


Structural component only
DWG# T-2123998

JOB NAME 418362	TRUSS NAME T37S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.420 S Jan 21 2021 MTek Industries, Inc. Thu Jul 22 16:43:36 2021 Page 1
ID:Nx0GIYRcZvpB_E9rQowHDzoyrp-hC3vpf8xqbIVJCEJJ4I0_NziFOeoJhWV3u4ziRyvTM6



Scale = 1:39.7

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
G - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
E - B	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
F - D	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMV+p	MT20	3.0	4.0	
C	TMVW+p	MT20	4.0	4.0	1.25 2.00
D	BMVW1+p	MT20	4.0	6.0	
E	BMV+p	MT20	3.0	4.0	
F	BMVW/W-I	MT20	6.0	10.0	3.00 3.50
G	BMV1+p	MT20	3.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ
G	379	0	379	0
D	379	0	379	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
G	268	176/0	0/0
D	268	176/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
G-A	-355/0	0.0	A-F	0/300
A-B	-299/0	-91.8	F-D	-16/0
B-C	-329/0	-91.8	D-C	0/428
D-C	-335/0	0.0		
G-F	0/0	-18.5		
E-F	0/44	0.0		
F-B	-398/0	0.0		
E-D	0/15	-18.5		

TOTAL WEIGHT = 42 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 6 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/939 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/939 (0.02")

CSI: TC=0.19/1.00 (B-C:1), BC=0.10/1.00 (D-E:4), WB=0.10/1.00 (C-F:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

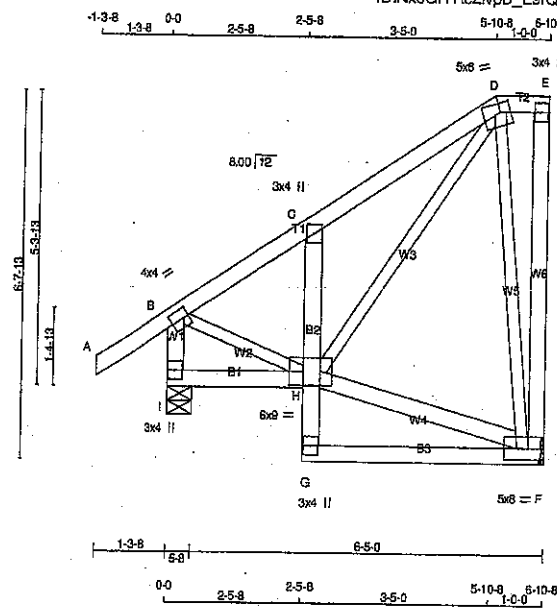
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.37 (C) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123999

JOB NAME 418362	TRUSS NAME T38S	QUANTITY 1	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
I - B	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
G - C	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT H - F	2x3 DRY	No.2	SPF		

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00	
C	TMV+p	MT20	3.0	4.0			
D	TTWV-m	MT20	5.0	6.0	2.00	2.25	
E	TMV+p	MT20	3.0	4.0			
F	BMVWW-1	MT20	5.0	8.0			
G	BMV+p	MT20	3.0	4.0			
H	BVMVWW-1	MT20	6.0	9.0	3.00	3.00	
I	BMV1+p	MT20	3.0	4.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	379	0	379	0	0	MECHANICAL	5-8		
I	505	0	505	0	0				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	268	176 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
I	365	246 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	H-F	0 / 35	0.01 (1)
B-C	-287 / 0	-91.8	-91.8 0.10 (1)	5.25	H-D	0 / 353	0.08 (1)
C-D	-305 / 0	-91.8	-91.8 0.11 (1)	6.25	B-H	0 / 277	0.05 (1)
D-E	0 / 0	-91.8	-91.8 0.01 (1)	10.00	D-F	310 / 0	0.23 (1)
F-E	-44 / 0	0.0	0.0 0.04 (1)	7.81			
I-B	-481 / 0	0.0	0.0 0.05 (1)	7.81			
I-H	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
G-H	0 / 44	0.0	0.0 0.03 (1)	10.00			
H-C	-327 / 0	0.0	0.0 0.02 (1)	7.81			
G-F	0 / 13	-18.5	-18.5 0.10 (4)	10.00			

TOTAL WEIGHT = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CS1: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (F-G:4), WB=0.23/1.00 (D-F:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.60 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

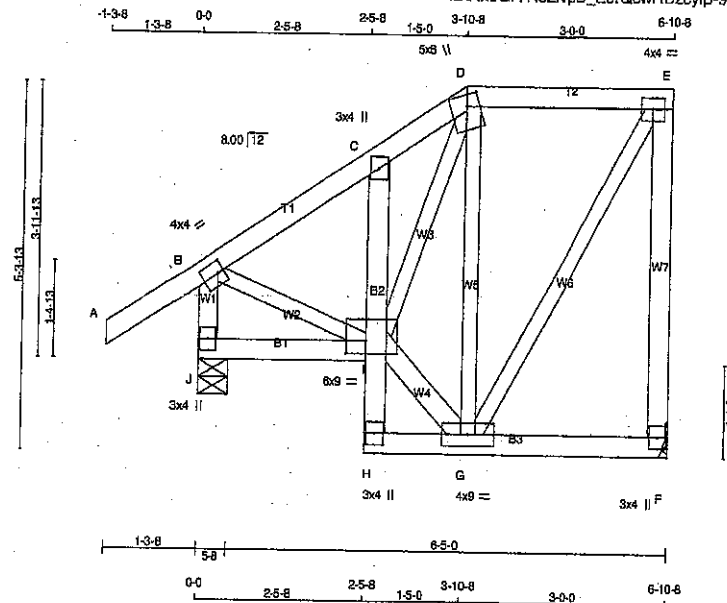


Structural component only
DWG# T-2124000

JOB NAME 418362	TRUSS NAME T39S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 16:43:37 2021 Page 1
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Scale = 1/30.5

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - I	2x4	DRY	No.2
H - C	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
I - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.00
C TMV+p	MT20	3.0	4.0		
D TTWW+m	MT20	5.0	6.0	2.00	2.25
E TMVW-t	MT20	4.0	4.0		
F BMV1+p	MT20	3.0	4.0		
G BMVWW-t	MT20	4.0	9.0		
H BMV+p	MT20	3.0	4.0		
I BVMWWH	MT20	6.0	9.0	2.25	3.50
J BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	379	0	379	0
J	505	0	505	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
F	268	176 / 0	0 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
J	355	246 / 0	0 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	VERT. LOAD	LCI	MAX. UNBRACED LENGTH	FR-TO	MEMB.	WEBS	MAX. FACTORED	FORCE	MAX. CSI (LC)
FR-TO							FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	I-G	0 / 165	0.03 (1)			
B-C	-275 / 0	-91.8	-91.8	0.07 (1)	6.25	I-D	0 / 275	0.06 (1)			
C-D	-266 / 0	-91.8	-91.8	0.04 (1)	6.25	G-D	-275 / 0	0.12 (1)			
D-E	-127 / 0	-91.8	-91.8	0.11 (1)	6.25	B-I	0 / 255	0.06 (1)			
F-E	-358 / 0	0.0	0.0	0.18 (1)	7.81	G-E	0 / 255	0.06 (1)			
J-B	-481 / 0	0.0	0.0	0.05 (1)	7.81						
J-I	0 / 0	-18.5	-18.5	0.04 (4)	10.00						
H-I	0 / 3	0.0	0.0	0.03 (1)	10.00						
I-C	-218 / 0	0.0	0.0	0.02 (1)	7.81						
H-G	0 / 14	-18.5	-18.5	0.03 (4)	10.00						
G-F	0 / 0	-18.5	-18.5	0.03 (4)	10.00						

TOTAL WEIGHT = 45 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (E-F:1), BC=0.04/1.00 (I-J:4), WB=0.12/1.00 (D-G:1), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

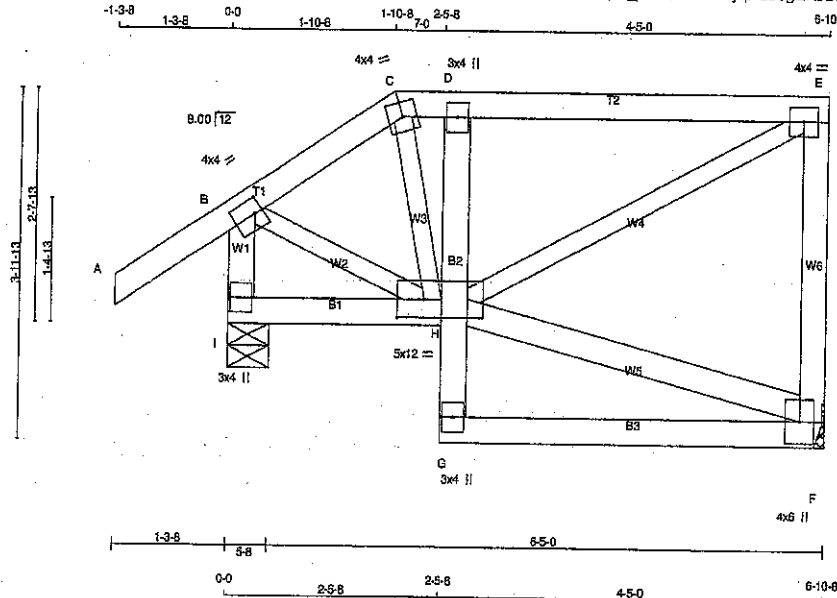


Structural component only
DWG# T-2124001

JOB NAME 418362	TRUSS NAME T40S	QUANTITY 1	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTsk Industries, Inc. Thu Jul 22 16:43:36 2021 Page 1
ID:Nx0GIYRcZlvpB_EsrQcwhDzcyrb-d8gEK9BMDvDYVohQVIU3o21NCKGnbKoXCZ3nKyvTM3



Scale: 1/2"=1'

LUMBER

N. L. G. A. RULES	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - E	2x4 DRY	No.2
E - G	2x4 DRY	No.2
G - I	2x4 DRY	No.2
I - K	2x4 DRY	No.2
K - M	2x4 DRY	No.2
M - O	2x4 DRY	No.2
O - P	2x4 DRY	No.2

ALL WEBS	2x3 DRY	No.2
EXCEPT		
H - F	2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
C	TTW-m	MT20	4.0	4.0	
D	TMVW-1	MT20	3.0	4.0	
E	TMVW-1	MT20	4.0	4.0	
F	BMVW-1-p	MT20	4.0	6.0	
G	BMVW-1-p	MT20	3.0	4.0	
H	BMVW-1-p	MT20	5.0	12.0	2.50 3.75
I	BMVW-1-p	MT20	3.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	366	0	366	0
I	518	0	518	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	COMBINED		169 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
I			253 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 35	-91.8 -91.8 0.12 (1)	H-F	-9 / 0
B-C	-285 / 0	-91.8 -91.8 0.12 (1)	H-E	0 / 342
C-D	-304 / 0	-91.8 -91.8 0.22 (1)	C-H	0 / 313
D-E	-299 / 0	-91.8 -91.8 0.22 (1)	B-H	0 / 235
F-E	-324 / 0	0.0 0.0 0.08 (1)		
I-B	-493 / 0	0.0 0.0 0.05 (1)		
I-H	0 / 0	-18.5 -18.5 0.04 (4)		
G-H	0 / 44	0.0 0.0 0.02 (1)		
H-D	-495 / 0	0.0 0.0 0.02 (1)		
G-F	0 / 8	-18.5 -18.5 0.10 (4)		

TOTAL WEIGHT = 39 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/990$ (0.23")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/980$ (0.23")
CALCULATED VERT. DEFL. (TL) = $L/989$ (0.02")

CSI: TC=0.22/1.00 (D-E), BC=0.10/1.00 (F-G), WB=0.09/1.00 (H-I), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.38 (B) (INPUT = 0.90)
JSI METAL = 0.14 (H) (INPUT = 1.00)

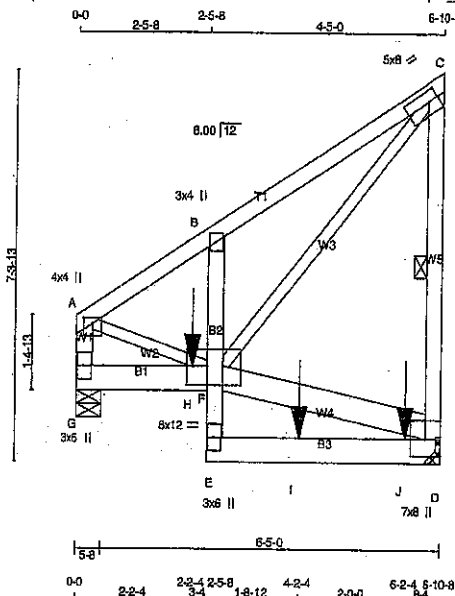


Structural component only
DWG# T-2124002

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
418362	T41S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Thu Jul 22 16:43:39 2021 Page 1
ID:Nx0GIYRcZivpB_E9rQcwHDzcyrp-5n12RgAp7W14Afzt_CGjcbEYcd2W1tyIsJdJmyvTM2



Scale = 1:39.6

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
G - F	2x6	DRY	No.2	SPF	
E - B	2x4	DRY	No.2	SPF	
E - D	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT F - D	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
G - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - F	12	SIDE(0.0)
E - D	12	SIDE(0.0)
B - E	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	1224	0	1224	0	5-8	5-8
D	1440	0	1440	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
G	861	587 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
D	1015	687 / 0	0 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
G - A	-1081 / 0	0.0	0.0	0.08 (1)
A - B	-1112 / 0	-91.8	-91.8	0.10 (1)
B - C	-1112 / 0	-91.8	-91.8	0.13 (1)
D - C	-754 / 0	0.0	0.0	0.09 (1)
G - H	0 / 0	-18.5	-18.5	0.11 (1)
H - F	0 / 0	-18.5	-18.5	0.11 (1)
E - F	0 / 450	0.0	0.0	0.09 (1)
F - B	-371 / 0	0.0	0.0	0.05 (1)
E - I	0 / 50	-18.5	-18.5	0.28 (1)
I - J	0 / 50	-18.5	-18.5	0.28 (1)
J - D	0 / 50	-18.5	-18.5	0.28 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	2-2-4	-607	-607	---	FRONT	VERT	TOTAL	---	C1
I	4-2-4	-365	-365	---	FRONT	VERT	TOTAL	---	C1
J	6-2-4	-388	-388	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS1: TC=0.19/1.00 (B-C:1), BC=0.28/1.00 (D-E:1), WB=0.19/1.00 (C-F:1), SS=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

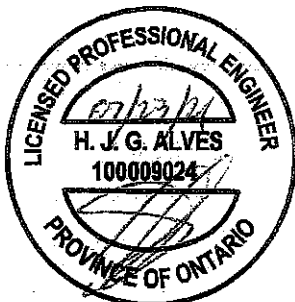
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	850	371	1747
	768	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.47 (A) (INPUT = 0.80)
JSI METAL = 0.14 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2124003

JOB NAME 418362	TRUSS NAME T41S	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:43:39 2021 Page 2
ID:Nx0GIYRcZivpB E9rQcwHDzcvp-5ni2RgAp7W14Afzt CGlc0bEYcd2W1tvisJdJmyvTM2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	6.0	2.25	2.50
D	BMVW1+p	MT20	7.0	8.0	3.75	Edge
E	BMV+p	MT20	3.0	6.0		
F	BYMVWWH	MT20	8.0	12.0	4.25	4.50
G	BMV1+p	MT20	3.0	6.0		

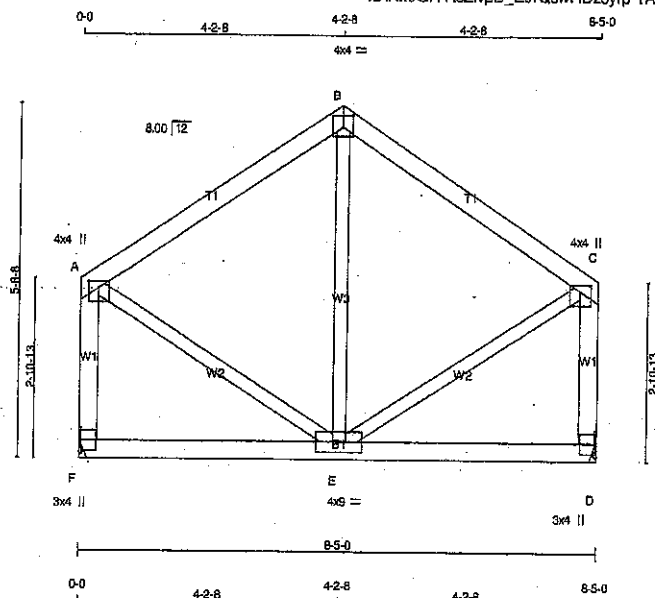
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2124003 *ML*

JOB NAME 418362	TRUSS NAME T43	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4 DRY	No.2		
B - C	2x4 DRY	No.2		
F - A	2x4 DRY	No.2		
D - C	2x4 DRY	No.2		
F - D	2x4 DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW+1	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	464	0	0	MECHANICAL
D	464	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	328	215 / 0	0 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
D	328	215 / 0	0 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-228 / 0	-91.8 -91.8	0.21 (1)	E-B	-135 / 23	0.07 (1)	
B-C	-228 / 0	-91.8 -91.8	0.21 (1)	A-E	0 / 221	0.05 (1)	
F-A	-434 / 0	0.0 0.0	0.07 (1)	E-C	0 / 221	0.05 (1)	
D-C	-434 / 0	0.0 0.0	0.07 (1)				
F-E	0 / 0	-18.5 -18.5	0.09 (4)				
E-D	0 / 0	-18.5 -18.5	0.09 (4)				

TOTAL WEIGHT = 2 X 39 = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ASC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/860$ (0.28")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.21/1.00 (A-B:1), BC=0.09/1.00 (E-F:4), WB=0.07/1.00 (B-E:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	550	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

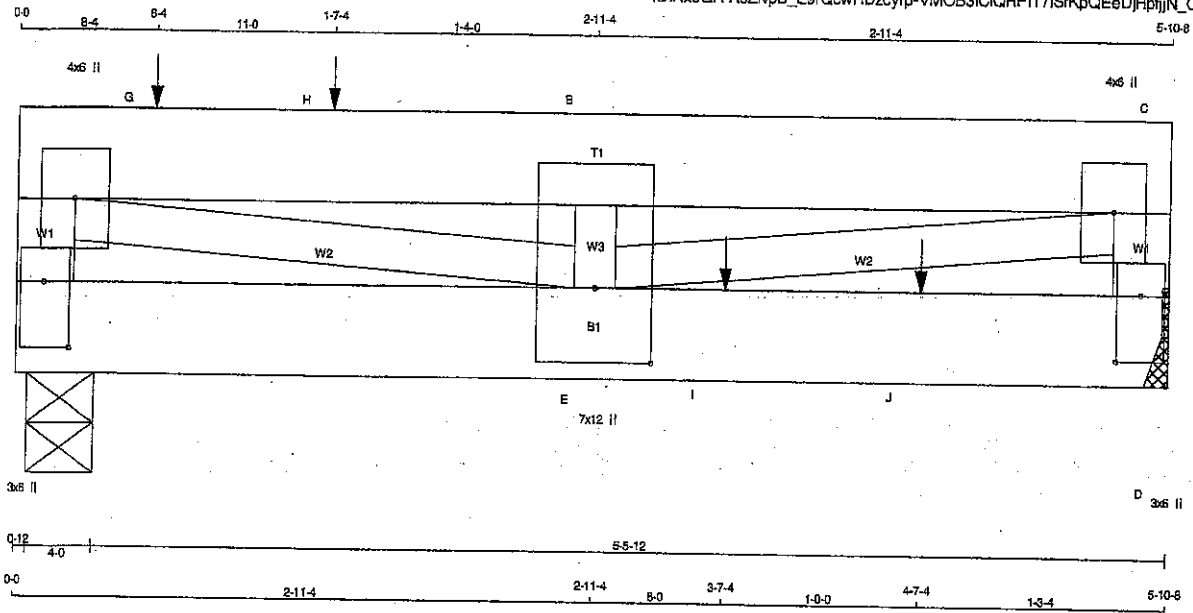
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (C) (INPUT = 0.50)
JSI METAL= 0.09 (C) (INPUT = 1.00)



Structural component only
DWG# T-2124005

JOB NAME 418362	TRUSS NAME T44	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 16:43:42 2021 Page 1	



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
C-D 1	12	TOP
A-C 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILED TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	
B					
C	TMVW+p	MT20	4.0	6.0	
D	BMV1+p	MT20	3.0	6.0	4.00 1.50
E					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	1157	0	1157	0	4-0
D	1114	0	1114	0	4-0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LOOSE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F	817 543 / 0 0 / 0 0 / 0 274 / 0 0 / 0
D	786 527 / 0 0 / 0 0 / 0 269 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
F-A	-1063 / 0	A-E	0 / 1821
A-G	-1745 / 0	E-B	-541 / 0
G-H	-1745 / 0	B-C	0 / 1821
H-B	-1745 / 0	C-D	0 / 1821
B-C	-1745 / 0	D-C	-686 / 0
D-C	-686 / 0		
F-E	0 / 0		
E-I	0 / 0		
I-J	0 / 0		
J-D	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
G	8-4	-145	-145	---	TOP	VERT	TOTAL	---	C1
H	1-7-4	-373	-373	---	TOP	VERT	TOTAL	---	C1
I	3-7-4	-313	-313	---	BACK	VERT	TOTAL	---	C1
J	4-7-4	-313	-313	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 27 = 108 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL DL = 99.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/899 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/899 (0.03")

CSI: TC=0.22/1.00 (A-B:1), BC=0.19/1.00 (D-E:1), WB=0.23/1.00 (A-E:1), SS=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMF=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.76 (A) (INPUT = 0.80)
JSI METAL = 0.39 (C) (INPUT = 1.00)



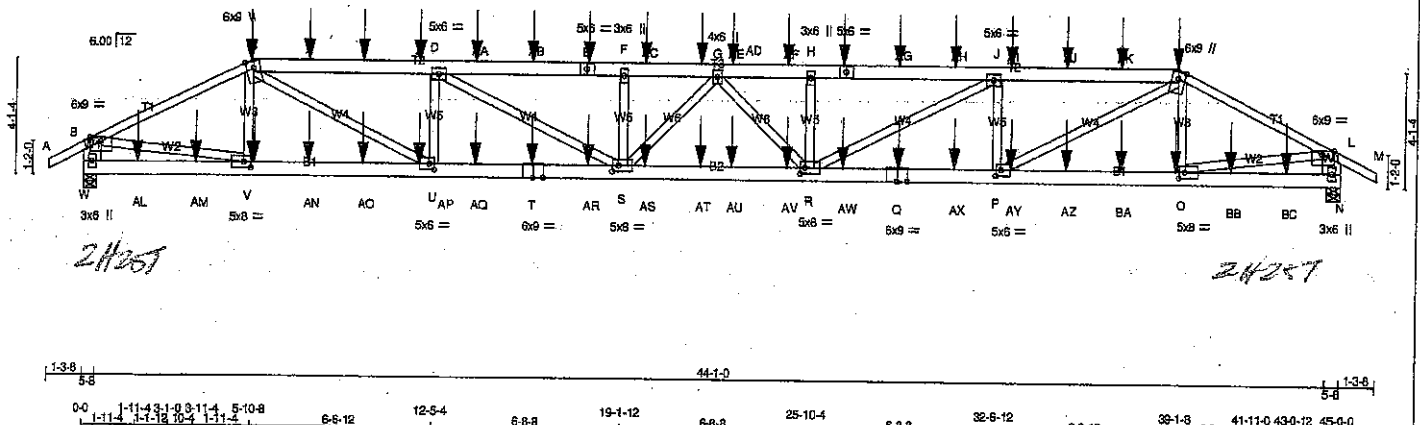
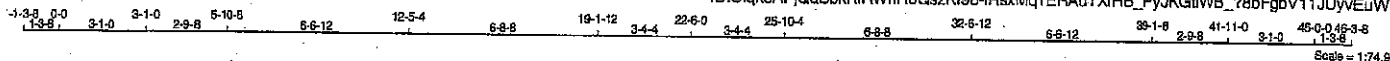
Structural component only
DWG# T-2124006

JOB NAME 418362	TRUSS NAME T44	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.																								
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 16:43:42 2021 Page 2 ID:Nx0GfYRcZlvpB E9rQowHDzcvrp-VMOB3jCiQRp17SlKpQEeDIHofjN ORgXHw5yvTM?																									
<table border="1"><thead><tr><th colspan="6">PLATES (table is in inches)</th></tr><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y X</th></tr></thead><tbody><tr><td>E</td><td>TM9MWW</td><td>MT20</td><td>7.0</td><td>12.0</td><td>4.50 3.50</td></tr><tr><td>F</td><td>BMV1+p</td><td>MT20</td><td>9.0</td><td>6.0</td><td>4.00 1.50</td></tr></tbody></table> NOTES: (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.						PLATES (table is in inches)						JT	TYPE	PLATES	W	LEN	Y X	E	TM9MWW	MT20	7.0	12.0	4.50 3.50	F	BMV1+p	MT20	9.0	6.0	4.00 1.50
PLATES (table is in inches)																													
JT	TYPE	PLATES	W	LEN	Y X																								
E	TM9MWW	MT20	7.0	12.0	4.50 3.50																								
F	BMV1+p	MT20	9.0	6.0	4.00 1.50																								
 Structural component only DWG# T-2124006 <i>7/22</i>																													

JOB NAME 418363	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY 1650F 1.5E	SPF
C - E	2x6	DRY No.2	SPF
E - I	2x6	DRY No.2	SPF
I - K	2x6	DRY No.2	SPF
K - M	2x4	DRY 1850F 1.5E	SPF
W - B	2x6	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
W - T	2x6	DRY 1650F 1.5E	SPF
T - Q	2x6	DRY 1650F 1.5E	SPF
Q - N	2x6	DRY 1650F 1.5E	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(70.1)
K-M 1	12	SIDE(70.1)
C-E 2	12	SIDE(70.1)
E-I 2	12	SIDE(0.0)
I-K 2	12	SIDE(70.1)
W-B 2	12	TOP
N-L 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
W-T 2	12	SIDE(210.5)
T-Q 2	12	SIDE(0.0)
Q-N 2	12	SIDE(210.5)
WEBS: (0.122"x3") SPIRAL NAILS		
V-C 1	6	SIDE(49.4)
O-K 1	6	SIDE(49.4)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
W	5315	0	5448	-119
N	5163	0	5278	0

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 1311 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1261 LBS. FACTORED UPLIFT

PROVIDE FOR 119 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

1ST LOOSE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	3900	2375 / 0	524 / 0	0 / 0	0 / -1266	1089 / 0	0 / 0
N	3787	2297 / 0	504 / 0	0 / 0	0 / -1218	1063 / 0	0 / 0

HORIZONTAL REACTIONS

W	0 / 0	0 / 0	0 / 0	85 / -85	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	V-C	-518 / 216
B-C	-8415 / 1971	O-K	-588 / 271
C-X	-12801 / 3131	B-V	-1630 / 7612
X-Y	-12801 / 3131	O-L	-1552 / 7342
Y-Z	-12801 / 3131	P-K	-1831 / 6229
Z-D	-12801 / 3131	C-U	-1468 / 6115
D-AA	-15263 / 3671	P-J	-2898 / 810
AA-AB	-15263 / 3671	U-D	-2840 / 794
AB-E	-15263 / 3671	D-S	-711 / 3008
E-F	-15263 / 3671	R-J	-675 / 2885
F-AC	-15263 / 3671	R-H	-823 / 312
AC-AD	-15263 / 3671	S-F	-824 / 313
AD-G	-15263 / 3671	S-G	-272 / 99
G-AE	-15215 / 3656	Q-R	-352 / 121
AE-AF	-15215 / 3656		
AF-H	-15215 / 3656		
H-I	-15215 / 3656		
I-AG	-15215 / 3656		
AG-AH	-15215 / 3656		
AH-J	-15215 / 3656		
J-AI	-12841 / 3084		
AI-AJ	-12841 / 3084		
AJ-AK	-12841 / 3084		
AK-K	-12841 / 3084		
K-L	-8117 / 1884		
L-M	0 / 41		
W-B	-6925 / 1327		
N-L	-5156 / 1278		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 33.4 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = U/999 (0.51")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = U/709 (0.76")

CSI: TC=0.74/1.00 (D-F-1), BC=0.71/1.00 (R-S-1),
WB=0.67/1.00 (B-V-2), SS=0.21/1.00 (J-K-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747

CONTINUED ON PAGE 2



Structural component only
DWG# T-2124022

JOB NAME 418363	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Oik6SAPIQaBokRIRWnRoQszK98-IRexMq1ERAuTXtHB PyJKGfWB 78bFgbV11JuyvEuW

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMVW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW-w	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-w	MT20	3.0	6.0		
I	TS-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0		
K	TTWW+m	MT20	6.0	9.0	Edge	
L	TMVW-p	MT20	6.0	9.0	Edge	
N	BMV1+p	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	8.0	2.50	2.75
P	BMVW-t	MT20	5.0	6.0	2.50	2.00
Q	BS-t	MT20	6.0	9.0		
R	BMVW-t	MT20	5.0	8.0	2.50	2.00
S	BMVW-t	MT20	5.0	8.0	2.50	2.50
T	SS-t	MT20	6.0	9.0		
U	BMVW-t	MT20	5.0	6.0	2.50	2.00
V	BMVW-t	MT20	5.0	8.0	2.50	2.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (16)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (PLF)	CS1 (LC)	UNBRAC. LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED (PLF)
FR-TO							
U-AQ	-2960 / 12800	-39.5	-39.5	0.59 (2)	6.25		
AQ-T	-2960 / 12800	-39.5	-39.5	0.59 (2)	6.25		
T-AR	-2960 / 12800	-39.5	-39.5	0.59 (2)	6.25		
AR-S	-2960 / 12800	-39.5	-39.5	0.59 (2)	6.25		
S-AS	-3542 / 15425	-39.5	-39.5	0.71 (1)	6.25		
AS-AT	-3542 / 15425	-39.5	-39.5	0.71 (1)	6.25		
AT-AU	-3542 / 15425	-39.5	-39.5	0.71 (1)	6.25		
AU-AV	-3542 / 15425	-39.5	-39.5	0.71 (1)	6.25		
AV-R	-3542 / 15425	-39.5	-39.5	0.71 (1)	6.25		
R-AW	-2835 / 12640	-39.5	-39.5	0.59 (3)	6.25		
AW-Q	-2835 / 12640	-39.5	-39.5	0.59 (3)	6.25		
Q-AX	-2835 / 12640	-39.5	-39.5	0.59 (3)	6.25		
AX-P	-2835 / 12640	-39.5	-39.5	0.59 (3)	6.25		
P-AY	-1533 / 7301	-39.5	-39.5	0.35 (3)	6.25		
AY-AZ	-1533 / 7301	-39.5	-39.5	0.35 (3)	6.25		
AZ-BA	-1533 / 7301	-39.5	-39.5	0.35 (3)	6.25		
BA-O	-1533 / 7301	-39.5	-39.5	0.35 (3)	6.25		
O-BB	-8 / 16	-39.5	-39.5	0.07 (17)	10.00		
BB-BC	-8 / 16	-39.5	-39.5	0.07 (17)	10.00		
BC-N	-8 / 16	-39.5	-39.5	0.07 (17)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	8-10-8	-367	-491	38	FRONT	VERT	TOTAL	---	C1
E	17-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
I	27-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
K	39-1-8	-367	-462	38	FRONT	VERT	TOTAL	---	C1
O	39-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	29-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	15-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	5-10-8	-151	-198	38	FRONT	VERT	TOTAL	---	C1
V	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	7-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Y	9-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
Z	11-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AA	13-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AB	15-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AC	19-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AD	21-11-4	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AE	23-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AF	25-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AG	29-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AH	31-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AI	33-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AJ	35-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AK	37-0-12	-60	-60	---	FRONT	VERT	TOTAL	---	C1
AL	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AM	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	17-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	19-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	21-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	23-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	25-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	27-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	31-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AY	33-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AZ	35-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BA	37-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BB	41-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
BC	43-0-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(b)

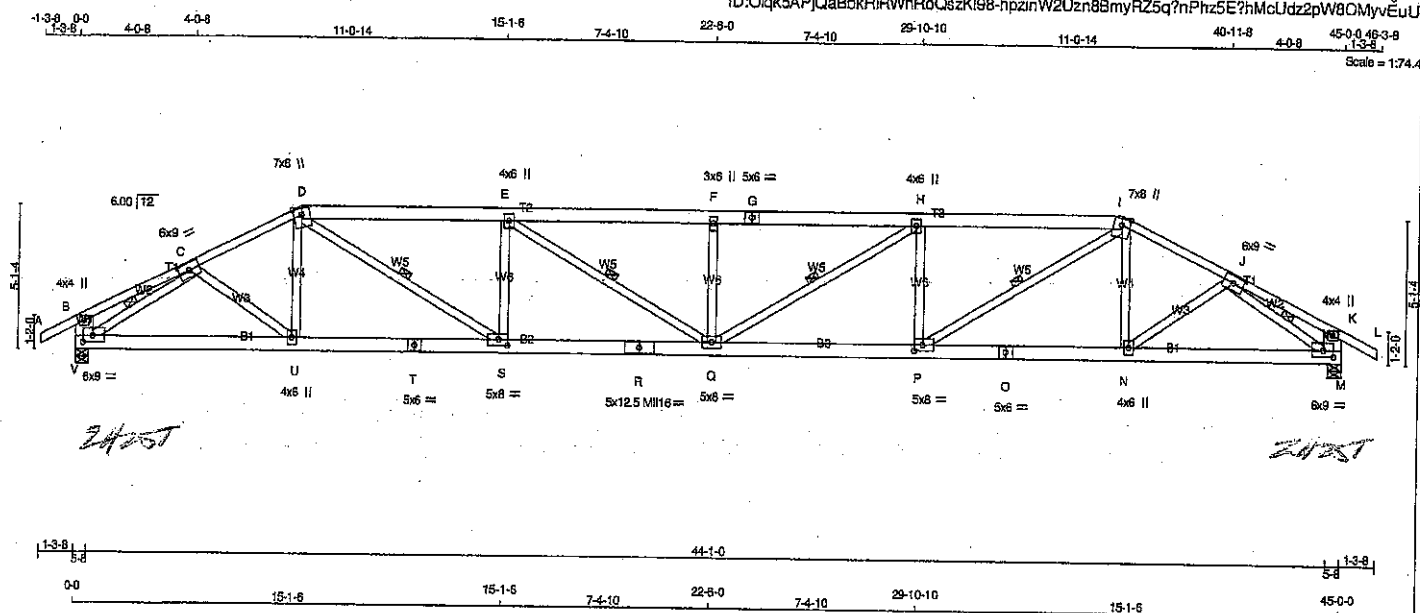
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF 19.6 PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124022

JOB NAME: 418363	TRUSS NAME: T51	QUANTITY: 1	PLY: 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x6	DRY	1650F 1.5E
G - I	2x6	DRY	1650F 1.5E
I - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - T	2x6	DRY	1650F 1.5E
T - R	2x6	DRY	1650F 1.5E
R - O	2x6	DRY	1650F 1.5E
O - M	2x6	DRY	1650F 1.5E
ALL WEBS	2x4	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0		
C	TMW-w	MT20	6.0	9.0		
D	TTWW+m	MT20	7.0	8.0	Edge	
E	TMW-w	MT20	4.0	6.0		
F	TMW-w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW-w	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	
J	TMW-w	MT20	6.0	9.0		
K	TMW+p	MT20	4.0	4.0		
M	BMWV-t	MT20	6.0	9.0	2.75	4.00
N	BMWV-t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWV-t	MT20	5.0	8.0	2.50	3.50
Q	BMWV-t	MT20	5.0	8.0		
R	BS-t	MT16	5.0	12.5		
S	BMWV-t	MT20	5.0	8.0	2.50	3.50
T	BS-t	MT20	5.0	6.0		
U	BMWV-t	MT20	4.0	6.0		
V	BMWV-t	MT20	6.0	9.0	2.75	4.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)



Structural component only
 DWG# T-2124023

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	BRG	BRG
V	3638	0	3730	-139
M	3638	0	3730	-1175

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1175 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1175 LBS FACTORED UPLIFT
 PROVIDE FOR 139 LBS FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
V	2886	1655 / 0	473 / 0	0 / 0	0 / -1138	819 / 0	0 / 0
M	2886	1655 / 0	473 / 0	0 / 0	5 / -1138	819 / 0	0 / 0

HORIZONTAL REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
V	0 / 0	0 / 0	0 / 0	99 / -99	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 2.44 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.93 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-V, J-M, I-P, D-S, H-Q, E-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 41	-115.2 -115.2	0.17 (2)	C-U	-135 / 727	0.12 (3)	
B-C	-9 / 118	-115.2 -115.2	0.28 (2)	U-D	-161 / 275	0.04 (17)	
C-D	-5448 / 1825	-115.2 -115.2	0.72 (2)	N-I	-179 / 258	0.04 (9)	
D-E	-7682 / 2602	-115.2 -115.2	0.47 (1)	N-J	-59 / 727	0.12 (2)	
E-F	-6532 / 2836	-115.2 -115.2	0.63 (1)	V-C	-5811 / 1745	0.74 (2)	
F-G	-6532 / 2835	-115.2 -115.2	0.63 (1)	J-M	-5811 / 1745	0.74 (3)	
G-H	-6532 / 2835	-115.2 -115.2	0.63 (1)	P-I	-1174 / 3469	0.56 (2)	
H-I	-7682 / 2602	-115.2 -115.2	0.47 (1)	D-S	-1174 / 3469	0.56 (3)	
I-J	-5448 / 1825	-115.2 -115.2	0.72 (3)	P-H	-1607 / 700	0.38 (2)	
J-K	-9 / 118	-115.2 -115.2	0.28 (3)	S-E	-1607 / 700	0.38 (3)	
K-L	0 / 41	-115.2 -115.2	0.17 (3)	Q-H	-369 / 1182	0.19 (2)	
V-B	-394 / 203	0.0 0.0	0.02 (2)	E-Q	-369 / 1182	0.19 (3)	
M-K	-394 / 203	0.0 0.0	0.02 (3)	Q-F	-804 / 415	0.19 (1)	
V-U	-1638 / 4604	-39.5 -39.5	0.44 (2)				
U-T	-1479 / 4869	-39.5 -39.5	0.45 (1)				
T-S	-1479 / 4869	-39.5 -39.5	0.45 (1)				
S-R	-2389 / 7682	-39.5 -39.5	0.65 (1)				
R-Q	-2389 / 7682	-39.5 -39.5	0.65 (1)				
Q-P	-2311 / 7662	-39.5 -39.5	0.65 (1)				
P-O	-1358 / 4869	-39.5 -39.5	0.45 (1)				
O-N	-1358 / 4869	-39.5 -39.5	0.45 (1)				
N-M	-1417 / 4604	-39.5 -39.5	0.44 (3)				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.8.2(8)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 33.4 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 4 OF CBC 2018, ABC 2019
 - PART 4 OF OBC 2012 (2019 AMENDMENT)
 - CSA 088-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED
 - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.48")
 ALLOWABLE DEFL.(TL) = L/180 (3.00")
 CALCULATED VERT. DEFL.(TL) = L/822 (0.68")

CS1: TC=0.72/1.00 (C-D-2), BC=0.85/1.00 (Q-S-1), WB=0.74/1.00 (J-M-3), SS1=0.30/1.00 (H-I-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747
MT16	438	302	2547

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (J) (INPUT = 0.90)
 JSI METAL = 0.94 (T) (INPUT = 1.00)

JOB NAME 418363	TRUSS NAME T51	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 09:10:55 2021 Page 2
ID: Qtk5APiQaBbkRiFWnRoQszKl98-hpzlnW2Uzn88myRZ5q?nPhz5E?hMcUdz2pW8OMyvEuU

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30'-0") FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (6'-0") FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

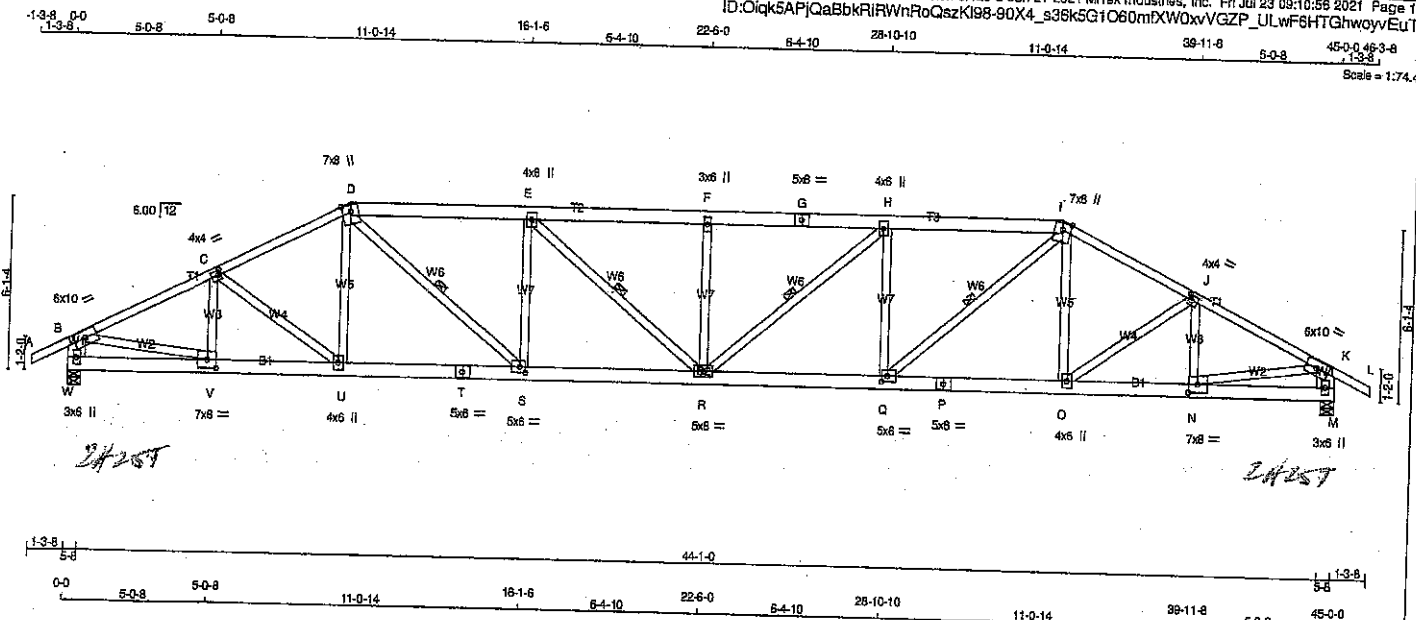


Structural component only
DWG# T-2124023 *ML*

JOB NAME 418363	TRUSS NAME T52	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 5.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 09:10:56 2021 Page 1
ID:Oiqk5APjQaBbkRIRWnRoQszKl98-90X4_s38k5G1O60mfXW0xvVGZP_ULwF6HTGhwvyeUT



LUMBER

N. L. G. A. RULES	
CHORDS	SIZE
A - D	2x4
D - G	2x6
G - I	2x6
I - L	2x4
W - B	2x8
M - K	2x8
W - T	2x6
T - P	2x6
P - M	2x6

ALL WEBS 2x4 DRY EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	10.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-t	MT20	7.0	8.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	TTWW-t	MT20	7.0	8.0	Edge	
J	TMVW-t	MT20	4.0	4.0	2.00	1.25
K	TMVW-t	MT20	6.0	10.0		
M	BMV1-p	MT20	3.0	6.0		
N	BMVW-t	MT20	7.0	8.0	3.50	3.75
O	BMVW-t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMVW-t	MT20	5.0	8.0		
S	BMVW-t	MT20	5.0	6.0	2.50	2.50
T	BS-t	MT20	5.0	6.0		
U	BMVW-t	MT20	4.0	6.0		
V	BMVW-t	MT20	7.0	8.0	3.50	3.75
W	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)



Structural component only
DWG# T-2124024

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
W	3638	0	3720	-158
M	3638	0	3720	0

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 1162 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1162 LBS. FACTORED UPLIFT

PROVIDE FOR 158 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

JT	1ST CASE	MAX. COMPONENT	MIN. COMPONENT	PERM. LIVE	WIND	DEAD	SOIL
W	2688	1849 / 0	473 / 0	0 / 0	0 / -1128	619 / 0	0 / 0
M	2688	1849 / 0	473 / 0	0 / 0	26 / -1128	619 / 0	0 / 0

HORIZONTAL REACTIONS

W	0 / 0	0 / 0	0 / 0	113 / -113	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.76 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-Q, D-S, H-R, E-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (16)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PL)	LOI	MAX. CSI (LC)	UNBRACED LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO										
A-B		0 / 41	-115.2	-115.2	0.15 (2)	10.00	V-C	-813 / 293	0.09 (2)	
B-C		-5364 / 1791	-115.2	-115.2	0.75 (2)	2.97	C-U	-169 / 390	0.05 (2)	
C-D		-5326 / 1802	-115.2	-115.2	0.71 (2)	3.00	U-D	-85 / 379	0.06 (5)	
D-E		-6468 / 2211	-115.2	-115.2	0.58 (1)	3.06	O-I	-85 / 379	0.06 (6)	
E-F		-7000 / 2336	-115.2	-115.2	0.63 (1)	2.90	O-J	-190 / 390	0.07 (10)	
F-G		-7000 / 2336	-115.2	-115.2	0.63 (1)	2.90	N-J	-813 / 293	0.09 (3)	
G-H		-7000 / 2336	-115.2	-115.2	0.63 (1)	2.90	B-V	-1444 / 4883	0.78 (2)	
H-I		-8468 / 2211	-115.2	-115.2	0.58 (1)	3.06	N-K	-1445 / 4883	0.78 (2)	
I-J		-5326 / 1802	-115.2	-115.2	0.71 (3)	3.00	Q-I	-835 / 2520	0.78 (3)	
J-K		-5384 / 1731	-115.2	-115.2	0.75 (3)	2.97	D-S	-835 / 2520	0.41 (2)	
K-L		0 / 41	-115.2	-115.2	0.15 (3)	10.00	Q-H	-1445 / 4824	0.53 (2)	
W-B		-3602 / 1169	0.0	0.0	0.18 (2)	6.32	S-E	-1445 / 4824	0.53 (3)	
M-K		-3602 / 1169	0.0	0.0	0.18 (3)	6.32	R-H	-284 / 904	0.15 (2)	
W-V		-142 / 163	-39.5	-39.5	0.12 (5)	6.25	E-R	-283 / 904	0.15 (3)	
V-U		-1560 / 4826	-39.5	-39.5	0.65 (2)	6.24	R-F	-896 / 359	0.25 (1)	
U-T		-1422 / 4744	-39.5	-39.5	0.64 (1)	6.25				
T-S		-1422 / 4744	-39.5	-39.5	0.64 (1)	6.25				
S-R		-1656 / 8468	-39.5	-39.5	0.85 (1)	5.76				
R-Q		-1670 / 8498	-39.5	-39.5	0.85 (1)	5.86				
Q-P		-1275 / 4744	-39.5	-39.5	0.64 (1)	6.25				
P-O		-1275 / 4744	-39.5	-39.5	0.64 (1)	6.25				
O-N		-1413 / 4826	-39.5	-39.5	0.65 (3)	6.25				
N-M		-8 / 16	-39.5	-39.5	0.12 (6)	10.00				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (b)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/960$ (1.50")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.38")
ALLOWABLE DEFL. (TL) = $L/180$ (3.00")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.52")

CSI: TC=0.75/1.00 (R-C-2), BC=0.85/1.00 (R-S-1), WB=0.78/1.00 (K-N-3), SS=0.29/1.00 (B-C-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION (PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

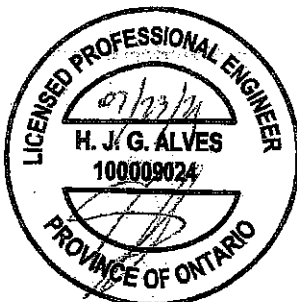
JSI GRIP = 0.89 (K) (INPUT = 0.90)
JSI METAL = 0.87 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME 418363	TRUSS NAME T52	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 09:10:56 2021 Page 2
ID:Oiqk5APfQaBbkRIRWnRoQszKI98-90X4 s36k5G1O60mfXW0xyVGZP ULwF6HTGhwoyvEuT

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

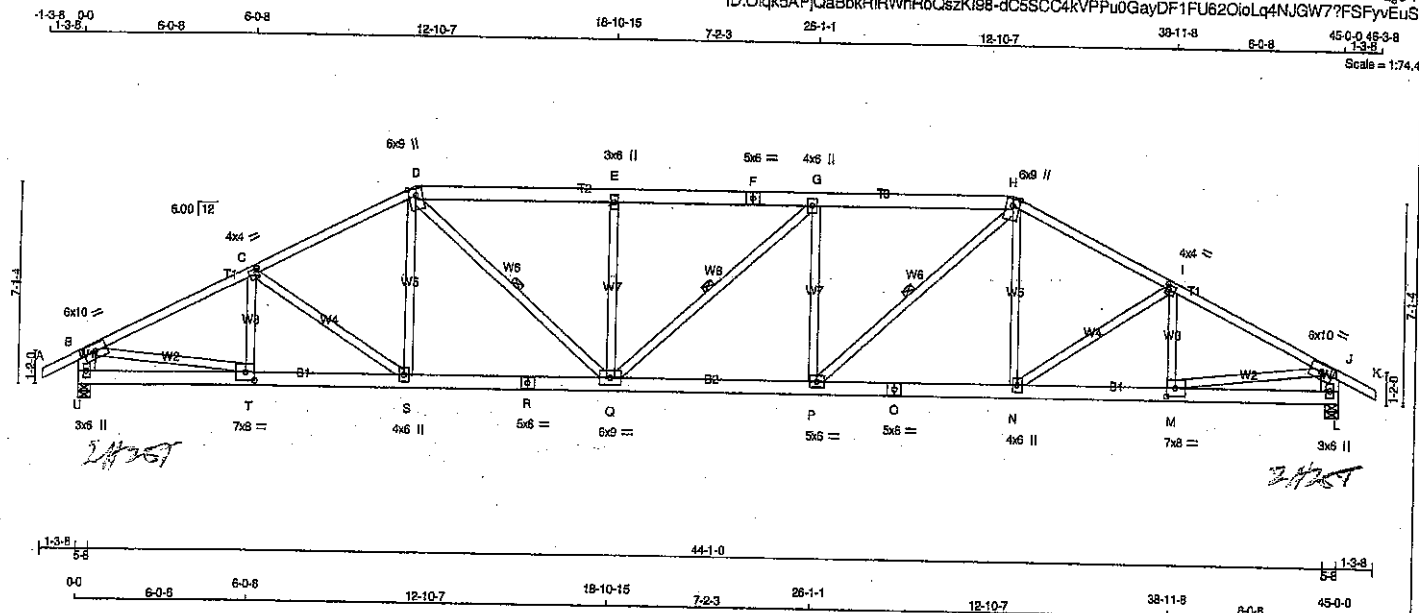


Structural component only
DWG# T-2124024 *in*

JOB NAME 418363	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 09:57 2021 Page 1
ID:Olk5APjQaBbkRIRWnRoCszKl98-dC5SCC4kVPPu0GayDF1FU62OioLq4NJGW7?FSFyveUs

Scale = 1/4" = 1'-0"



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	2100F 1.8E	SPF
D - F	2x6	DRY	No.2	2100F 1.8E	SPF
F - H	2x6	DRY	No.2	2100F 1.8E	SPF
H - K	2x6	DRY	No.2	2100F 1.8E	SPF
U - B	2x6	DRY	No.2	2100F 1.8E	SPF
L - J	2x6	DRY	No.2	2100F 1.8E	SPF
U - R	2x6	DRY	No.2	2100F 1.8E	SPF
R - O	2x6	DRY	No.2	2100F 1.8E	SPF
O - L	2x6	DRY	No.2	2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-t	MT20	6.0	10.0		
C	TMWV-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	6.0	9.0	Edge	
E	TMWV-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWV-w	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMWV-t	MT20	4.0	4.0	2.00	1.25
J	TMWV-t	MT20	6.0	10.0		
L	BMV-t+p	MT20	3.0	6.0		
M	BMWV-t	MT20	7.0	8.0	3.50	3.75
N	BMWV-t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWV-t	MT20	5.0	6.0		
Q	BSWVW-t	MT20	6.0	9.0		
R	BS-t	MT20	5.0	6.0		
S	BMWV-t	MT20	4.0	6.0		
T	BMWV-t	MT20	7.0	8.0	3.50	3.75
U	BMV-t+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT GROSS REACTION				
U VERT	3698	3700	0	0
L VERT	3698	3700	0	0

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1146 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1146 LBS FACTORED UPLIFT

PROVIDE FOR 178 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS	1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED		
U	2686	1636 / 0
L	2686	1636 / 0

HORIZONTAL REACTIONS	U	L
	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.15 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, D-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LC)
FR-TO						FR-TO		
A-B	0 / 41					T-C	-472 / 260	0.08 (8)
B-C	-5481 / 1743	-115.2	-115.2	0.11 (2)	10.00	C-D	-538 / 305	0.32 (2)
C-D	-5163 / 1730	-115.2	-115.2	0.83 (2)	3.22	S-D	-128 / 589	0.09 (2)
D-E	-5840 / 1981	-115.2	-115.2	0.82 (1)	3.16	N-H	-126 / 586	0.09 (3)
E-F	-5839 / 1980	-115.2	-115.2	0.83 (1)	3.14	N-I	-539 / 305	0.32 (3)
F-G	-5838 / 1980	-115.2	-115.2	0.83 (1)	3.14	M-I	-478 / 259	0.08 (8)
G-H	-5844 / 1984	-115.2	-115.2	0.83 (1)	3.14	B-T	-1437 / 4955	0.80 (1)
H-I	-5153 / 1730	-115.2	-115.2	0.83 (1)	3.22	M-J	-1437 / 4955	0.80 (1)
I-J	-5481 / 1743	-115.2	-115.2	0.92 (3)	3.05	P-H	-668 / 2015	0.32 (2)
J-K	0 / 41	-115.2	-115.2	0.11 (3)	10.00	D-Q	-663 / 2009	0.32 (3)
U-B	-3578 / 1159	0.0	0.0	0.17 (2)	6.33	P-G	-1104 / 527	0.81 (2)
L-J	-3578 / 1159	0.0	0.0	0.17 (3)	6.33	Q-E	-918 / 472	0.50 (1)
						Q-G	-280 / 269	0.13 (3)
U-T	-161 / 189	-39.5	-39.5	0.14 (5)	8.25			
T-S	-1584 / 4915	-39.5	-39.5	0.83 (1)	8.21			
S-R	-1327 / 4577	-39.5	-39.5	0.84 (1)	8.25			
R-Q	-1327 / 4577	-39.5	-39.5	0.84 (1)	8.25			
Q-P	-1637 / 5843	-39.5	-39.5	0.78 (1)	8.15			
P-O	-1153 / 4577	-39.5	-39.5	0.84 (1)	8.25			
O-N	-1153 / 4577	-39.5	-39.5	0.84 (1)	8.25			
N-M	-1410 / 4915	-39.5	-39.5	0.88 (1)	8.25			
M-L	-8 / 16	-39.5	-39.5	0.14 (8)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2018
- PART 4 OF CBC 2012 (2013 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.50")
CALCULATED VERT. DEFL. (LL) = L/999 (0.31")
ALLOWABLE DEFL. (TL) = L/180 (3.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.43")

CSI: TC=0.92/1.00 (I-J-3), BC=0.78/1.00 (P-Q-1), WB=0.80/1.00 (J-M-1), SS=0.35/1.00 (G-C-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

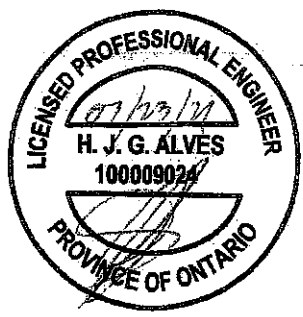
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.90)
JSI METAL = 0.88 (P) (INPUT = 1.00)

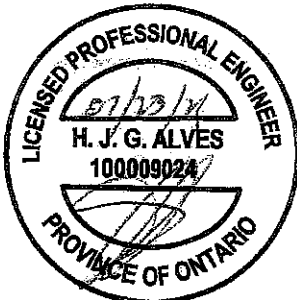


Structural component only
DWG# T-2124025

JOB NAME 418363	TRUSS NAME T53	QUANTITY 1	PLY 1	JOB DESC GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Fri Jul 23 09:10:57 2021 Page 2
ID:Oiqk5APIQaBbkRIRWnRoQszKI98-dC5SCC4kVPPu0GavDF1FU62OioLq4NJGW7?FSFvveUS

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pG} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124025 *2/2*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418363	T54	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

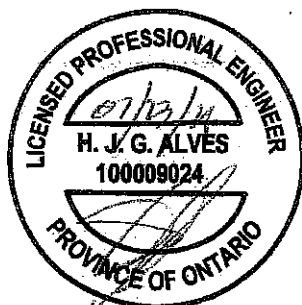
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 09:10:59 2021 Page 2

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NOTES- (1)

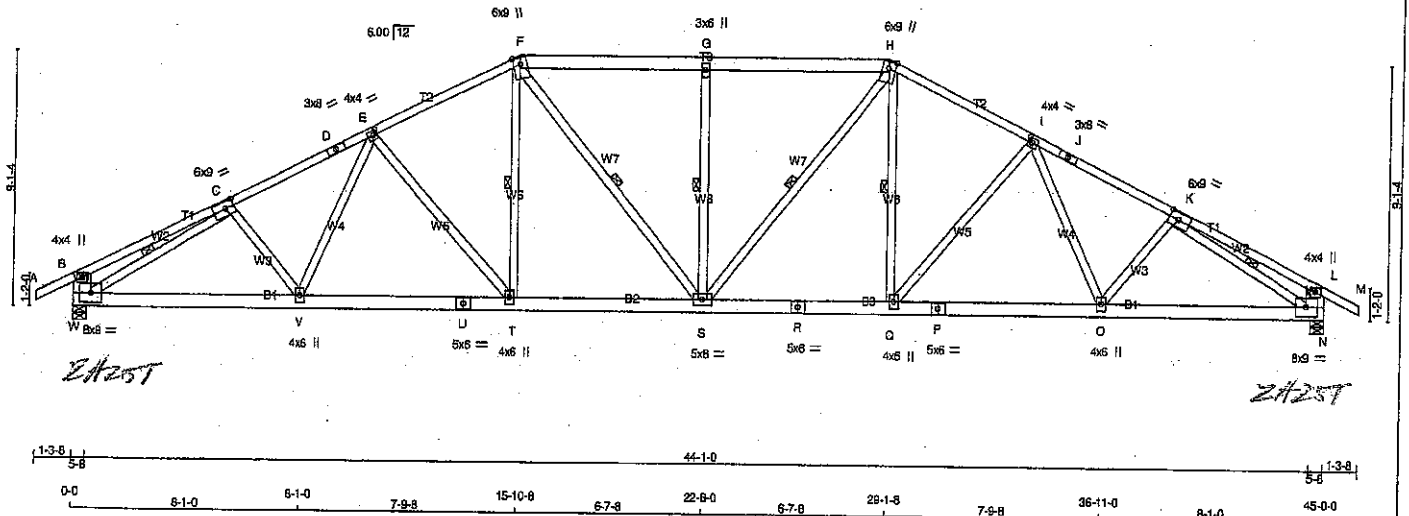
1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124026

JOB NAME 418363	TRUSS NAME T55	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Fri Jul 23 09:11:00 2021 Page 1 ID:0qk5APQaBbkRIRWnRQszKI98-1nrbqE6dnKnTikXUNay6lgyN0NCHiaC5Ev3ayvEuP	
Scale = 1:75.3					



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
A - D	2x4	DRY	1650F 1.5E
D - F	2x4	DRY	1650F 1.5E
F - H	2x6	DRY	No.2
H - J	2x4	DRY	1650F 1.5E
J - M	2x4	DRY	1650F 1.5E
N - L	2x6	DRY	No.2
W - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
W - C	2x4	DRY	1650F 1.5E
K - N	2x4	DRY	1650F 1.5E

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	4.0	4.0		
C TMWW-t	MT20	6.0	8.0	3.00	3.75
D TS-t	MT20	3.0	8.0		
E TMWW-t	MT20	4.0	4.0	2.00	1.75
F TTWW-m	MT20	6.0	9.0	Edge	
G TMW-w	MT20	3.0	6.0		
H TTWW-m	MT20	6.0	9.0	Edge	
I TMWW-t	MT20	4.0	4.0	2.00	1.75
J TS-t	MT20	3.0	8.0		
K TMWW-t	MT20	6.0	9.0	3.00	3.75
L TMV+p	MT20	4.0	4.0		
N BMWW-t	MT20	8.0	9.0		
O Q, T, V					
P BMWW-t	MT20	4.0	6.0		
R, U					
S BS-t	MT20	5.0	6.0		
T BMWW-t	MT20	5.0	8.0		
W BMWW-t	MT20	8.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT				
W	3638	0	3638	227
N	3638	0	3638	0

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 1104 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1104 LBS. FACTORED UPLIFT

PROVIDE FOR 227 LBS. FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS				
W	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
W	2686	1594 / 0	473 / 0	0 / 0	0 / -1087	519 / 0
N	2686	1594 / 0	473 / 0	0 / 0	78 / -1087	519 / 0

HORIZONTAL REACTIONS

W	0 / 0	0 / 0	0 / 0	162 / -162	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.21 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, F-S, G-S, H-S, H-Q, C-W, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 41	-115.2 -115.2	0.15 (2)	C-V	-170 / 347	0.06 (3)	
B-C	-9 / 143	-115.2 -115.2	0.39 (2)	V-E	-98 / 442	0.07 (5)	
C-D	-5387 / 1725	-115.2 -115.2	0.76 (2)	E-T	-979 / 463	0.89 (2)	
D-E	-5387 / 1725	-115.2 -115.2	0.76 (2)	T-F	-304 / 1049	0.17 (2)	
E-F	-4658 / 1566	-115.2 -115.2	0.85 (2)	F-S	-395 / 1192	0.24 (14)	
F-G	-4610 / 1564	-115.2 -115.2	0.54 (1)	S-G	-981 / 502	0.35 (1)	
G-H	-4610 / 1564	-115.2 -115.2	0.54 (1)	H-S	-395 / 1192	0.24 (13)	
H-I	-4658 / 1566	-115.2 -115.2	0.85 (3)	Q-H	-304 / 1049	0.17 (3)	
I-J	-5387 / 1725	-115.2 -115.2	0.76 (3)	Q-I	-979 / 463	0.89 (3)	
J-K	-5387 / 1725	-115.2 -115.2	0.76 (3)	I-O	-98 / 442	0.07 (6)	
K-L	-9 / 143	-115.2 -115.2	0.39 (3)	O-K	-179 / 347	0.06 (2)	
L-M	0 / 41	-115.2 -115.2	0.15 (3)	W-C	-695 / 1616	0.73 (1)	
W-B	-479 / 253	0.0	0.02 (2)	K-N	-5895 / 1816	0.73 (1)	
N-L	-479 / 253	0.0	0.02 (3)				
W-V	-1570 / 4789	-39.5	-39.5	0.74 (1)			
V-U	-1389 / 4670	-39.5	-39.5	0.66 (1)			
U-T	-1389 / 4670	-39.5	-39.5	0.66 (1)			
T-S	-1089 / 4136	-39.5	-39.5	0.60 (1)			
S-R	-942 / 4136	-39.5	-39.5	0.60 (1)			
R-Q	-942 / 4136	-39.5	-39.5	0.60 (1)			
Q-P	-1162 / 4670	-39.5	-39.5	0.66 (1)			
P-O	-1162 / 4670	-39.5	-39.5	0.66 (1)			
O-N	-1943 / 4788	-39.5	-39.5	0.74 (1)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 33.4	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.4	PSF	
TOTAL LOAD = 57.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF NBCC 2015, ABC 2019
- PART 4 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.50")
CALCULATED VERT. DEFL. (LL) = L/999 (0.26")
ALLOWABLE DEFL. (TL) = L/180 (3.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.36")

CSI: TC=0.76/1.00 (I-K-3), BC=0.74/1.00 (N-O-1), WB=0.89/1.00 (I-Q-3), SSI=0.30/1.00 (K-L-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (W) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2124027

JOB NAME 418363	TRUSS NAME T55	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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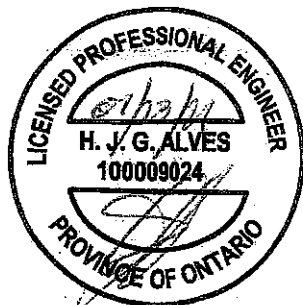
Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 09:11:00 2021 Page 2
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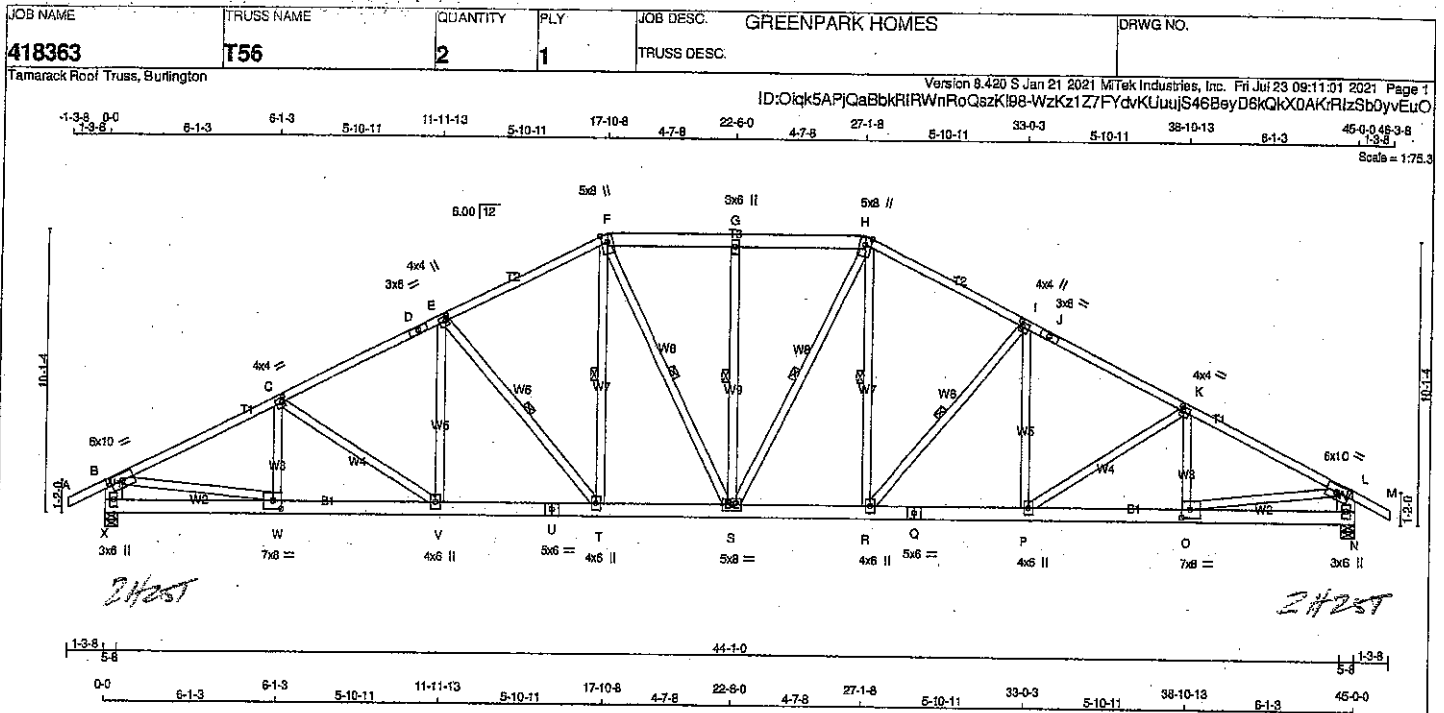
NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_p , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124027 *2/2*



LUMBER N. L. G. A. RULES CHORDS SIZE A - D 2x4 DRY 2100F 1.8E D - F 2x4 DRY 2100F 1.8E F - H 2x4 DRY No.2 H - J 2x4 DRY 2100F 1.8E J - M 2x4 DRY 2100F 1.8E X - B 2x8 DRY No.2 N - L 2x8 DRY No.2 X - U 2x6 DRY No.2 U - Q 2x6 DRY No.2 Q - N 2x6 DRY No.2 ALL WEBS 2x4 DRY No.2 EXCEPT DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 33.4 PSF BOT CH. LL = 10.5 PSF DL = 7.4 PSF TOTAL LOAD = 57.3 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 4 OF BCBC 2018, ABC 2019 - PART 4 OF CBC 2012 (2019 AMENDMENT) - CSA 089-14 - TPIC 2014 DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED - PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED. (80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/360 (1.50") CALCULATED VERT. DEFL. (LL) = L/999 (0.24") ALLOWABLE DEFL. (TL) = L/180 (3.00") CALCULATED VERT. DEFL. (TL) = L/999 (0.33") CSI: TC=0.79/1.00 (K-L:3), BC=0.67/1.00 (O-P:1), WB=0.79/1.00 (L-C:1), SS=0.34/1.00 (K-L:3) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 850 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.89 (L) (INPUT = 0.90) JSI METAL = 0.84 (C) (INPUT = 1.00)			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 6.0 10.0 C TMVW-t MT20 4.0 4.0 2.00 1.25 D TS-t MT20 3.0 8.0 E TMVW-t MT20 4.0 4.0 2.00 1.25 F TTWW+m MT20 5.0 8.0 Edge G TMVW-t MT20 3.0 8.0 H TTWW+m MT20 5.0 8.0 Edge I TMVW-t MT20 4.0 4.0 2.00 1.25 J TS-t MT20 3.0 8.0 K TMVW-t MT20 4.0 4.0 2.00 1.25 L TMVW-t MT20 6.0 10.0 N BMV1-p MT20 3.0 8.0 O BMV1-p MT20 7.0 8.0 3.50 3.75 P, R, T, V P BMVW-t MT20 4.0 6.0 Q BS-t MT20 5.0 8.0 S BMVW-t MT20 5.0 8.0 U BS-t MT20 5.0 8.0 W BMVW-t MT20 7.0 8.0 3.50 3.75 X BMV1-p MT20 3.0 6.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				NOTES (1) 1).			
CHORDS MAX. FACTORED FORCE (LBS) FR-TO FROM TO LENGTH FR-TO A-B 0 / 41 -115.2 -115.2 0.11 (2) 10.00 W-C -448 / 239 0.08 (8) B-C -5452 / 1613 -115.2 -115.2 0.79 (2) 3.31 C-V -527 / 277 0.34 (2) C-D -5134 / 1604 -115.2 -115.2 0.71 (2) 3.43 V-E -104 / 529 0.08 (5) D-E -5134 / 1604 -115.2 -115.2 0.71 (2) 3.43 E-T -1267 / 545 0.48 (2) E-F -4093 / 1399 -115.2 -115.2 0.62 (2) 3.72 T-F -368 / 1176 0.19 (2) F-G -4093 / 1399 -115.2 -115.2 0.24 (1) 4.08 F-S -306 / 945 0.18 (14) G-H -4093 / 1399 -115.2 -115.2 0.24 (1) 4.08 S-G -694 / 353 0.32 (1) H-I -4378 / 1451 -115.2 -115.2 0.62 (3) 3.72 S-H -308 / 945 0.18 (13) I-J -5134 / 1605 -115.2 -115.2 0.71 (3) 3.43 R-H -368 / 1176 0.19 (3) J-K -5134 / 1605 -115.2 -115.2 0.71 (3) 3.43 R-I -1267 / 545 0.48 (3) K-L -5452 / 1614 -115.2 -115.2 0.76 (3) 3.31 P-I -104 / 529 0.08 (6) L-M 0 / 41 -115.2 -115.2 0.11 (3) 10.00 P-K -527 / 277 0.34 (3) X-B -3519 / 1094 0.0 0.0 0.17 (1) 6.38 O-K -454 / 239 0.08 (8) N-L -3519 / 1094 0.0 0.0 0.17 (1) 6.38 B-W -1315 / 4941 0.79 (1) O-L -1315 / 4941 0.79 (1)				WEBS MAX. FACTORED FORCE (LBS) FR-TO FROM TO LENGTH FR-TO A-B 0 / 41 -115.2 -115.2 0.11 (2) 10.00 W-C -448 / 239 0.08 (8) B-C -5452 / 1613 -115.2 -115.2 0.79 (2) 3.31 C-V -527 / 277 0.34 (2) C-D -5134 / 1604 -115.2 -115.2 0.71 (2) 3.43 V-E -104 / 529 0.08 (5) D-E -5134 / 1604 -115.2 -115.2 0.71 (2) 3.43 E-T -1267 / 545 0.48 (2) E-F -4093 / 1399 -115.2 -115.2 0.62 (2) 3.72 T-F -368 / 1176 0.19 (2) F-G -4093 / 1399 -115.2 -115.2 0.24 (1) 4.08 F-S -306 / 945 0.18 (14) G-H -4093 / 1399 -115.2 -115.2 0.24 (1) 4.08 S-G -694 / 353 0.32 (1) H-I -4378 / 1451 -115.2 -115.2 0.62 (3) 3.72 S-H -308 / 945 0.18 (13) I-J -5134 / 1605 -115.2 -115.2 0.71 (3) 3.43 R-H -368 / 1176 0.19 (3) J-K -5134 / 1605 -115.2 -115.2 0.71 (3) 3.43 R-I -1267 / 545 0.48 (3) K-L -5452 / 1614 -115.2 -115.2 0.76 (3) 3.31 P-I -104 / 529 0.08 (6) L-M 0 / 41 -115.2 -115.2 0.11 (3) 10.00 P-K -527 / 277 0.34 (3) X-B -3519 / 1094 0.0 0.0 0.17 (1) 6.38 O-K -454 / 239 0.08 (8) N-L -3519 / 1094 0.0 0.0 0.17 (1) 6.38 B-W -1315 / 4941 0.79 (1) O-L -1315 / 4941 0.79 (1)			
LOADING TOTAL LOAD CASES: (18) CHORDS MAX. FACTORED FORCE (LBS) FR-TO FROM TO LENGTH FR-TO A-B 0 / 41 -115.2 -115.2 0.11 (2) 10.00 W-C -448 / 239 0.08 (8) B-C -5452 / 1613 -115.2 -115.2 0.79 (2) 3.31 C-V -527 / 277 0.34 (2) C-D -5134 / 1604 -115.2 -115.2 0.71 (2) 3.43 V-E -104 / 529 0.08 (5) D-E -5134 / 1604 -115.2 -115.2 0.71 (2) 3.43 E-T -1267 / 545 0.48 (2) E-F -4093 / 1399 -115.2 -115.2 0.62 (2) 3.72 T-F -368 / 1176 0.19 (2) F-G -4093 / 1399 -115.2 -115.2 0.24 (1) 4.08 F-S -306 / 945 0.18 (14) G-H -4093 / 1399 -115.2 -115.2 0.24 (1) 4.08 S-G -694 / 353 0.32 (1) H-I -4378 / 1451 -115.2 -115.2 0.62 (3) 3.72 S-H -308 / 945 0.18 (13) I-J -5134 / 1605 -115.2 -115.2 0.71 (3) 3.43 R-H -368 / 1176 0.19 (3) J-K -5134 / 1605 -115.2 -115.2 0.71 (3) 3.43 R-I -1267 / 545 0.48 (3) K-L -5452 / 1614 -115.2 -115.2 0.76 (3) 3.31 P-I -104 / 529 0.08 (6) L-M 0 / 41 -115.2 -115.2 0.11 (3) 10.00 P-K -527 / 277 0.34 (3) X-B -3519 / 1094 0.0 0.0 0.17 (1) 6.38 O-K -454 / 239 0.08 (8) N-L -3519 / 1094 0.0 0.0 0.17 (1) 6.38 B-W -1315 / 4941 0.79 (1) O-L -1315 / 4941 0.79 (1)				TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)			



Structural component only
DWG# T-2124028

JOB NAME 418363	TRUSS NAME T56	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 09:11:01 2021 Page 2
ID:Oibk5APIQaBbkRIRWnReQszKl98-WzKz1Z7FYdyKJuuJlS46BeyD6kQkX0AKyRizSt0yvEuO

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF [8.8] PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

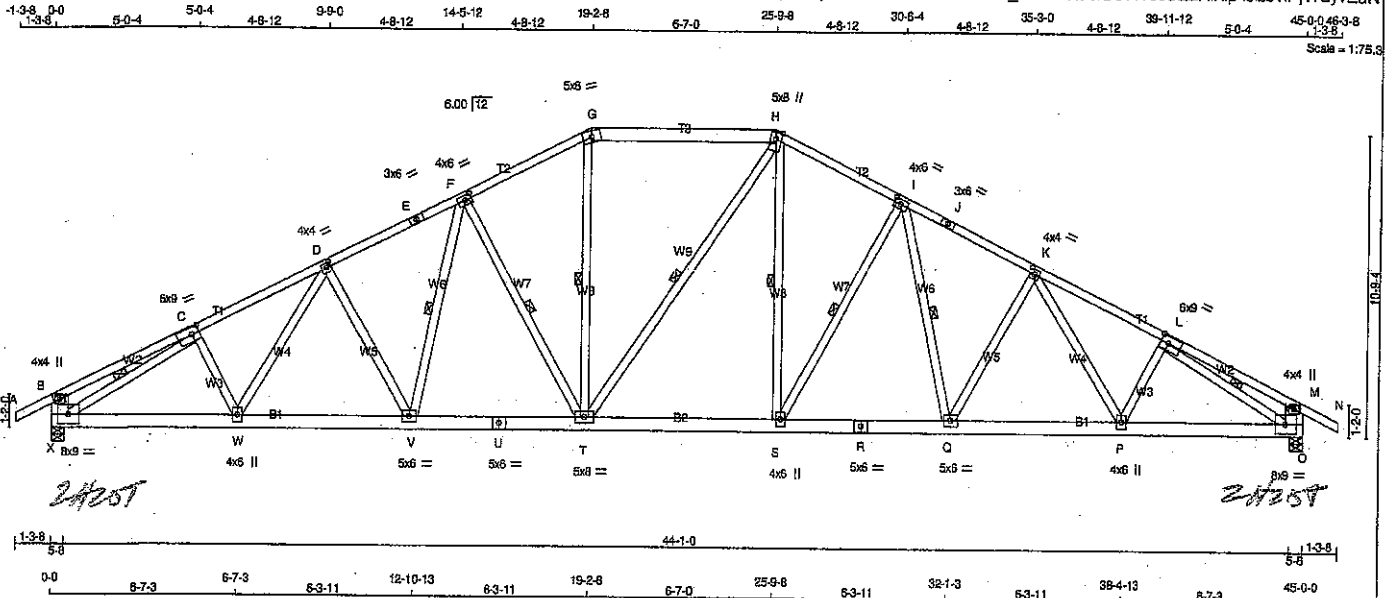


Structural component only
DWG# T-2124028 2/2

JOB NAME 418363	TRUSS NAME T57	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	1650F 1.5E
E - G	2x4	DRY	1650F 1.5E
G - H	2x6	DRY	No.2
H - J	2x4	DRY	1650F 1.5E
J - N	2x4	DRY	1650F 1.5E
X - B	2x8	DRY	No.2
O - M	2x8	DRY	No.2
X - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2

ALL WEBS

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	4.0	4.0		
C	TMVW-1	MT20	6.0	9.0	3.00	3.25
D	TMVW-1	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMVW-1	MT20	4.0	6.0	2.00	2.75
G	TTW-m	MT20	5.0	8.0		
H	TTW+m	MT20	5.0	8.0	Edge	
I	TMVW-1	MT20	4.0	6.0	2.00	2.75
J	TS-t	MT20	3.0	6.0		
K	TMVW-1	MT20	4.0	4.0	2.00	1.75
L	TMVW-1	MT20	6.0	9.0	3.00	3.25
M	TMV+P	MT20	4.0	4.0		
O	BMVW-1	MT20	8.0	9.0		
P, S, W						
P	BMVW-1	MT20	4.0	6.0		
Q	BMVW-1	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW-1	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMVW-1	MT20	5.0	6.0		
X	BMVW-1	MT20	8.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
X	3638	0	3638	271
O	3638	0	3638	271

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1060 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1060 LBS. FACTORED UPLIFT

PROVIDE FOR 271 LBS. FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
X	2686 1594 / 0 473 / 0 0 / 0 44 / -1055 819 / 0 0 / 0
O	2686 1594 / 0 473 / 0 0 / 0 100 / -1055 819 / 0 0 / 0

HORIZONTAL REACTIONS

X	0 / 0	0 / 0	0 / 0	193 / -198	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, O

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.26 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-V, F-T, G-T, H-T, H-S, I-S, I-Q, C-X, L-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-115.2 -115.2 0.15 (2)	10.00	C-W	-96 / 343	0.06 (3)	
B-C	-9 / 135	-115.2 -115.2 0.34 (2)	10.00	W-D	-110 / 304	0.06 (13)	
C-D	-5371 / 1669	-115.2 -115.2 0.60 (2)	3.18	D-V	-718 / 373	0.37 (2)	
D-E	-4959 / 1582	-115.2 -115.2 0.57 (2)	3.30	V-F	-285 / 892	0.14 (2)	
E-F	-4959 / 1582	-115.2 -115.2 0.57 (2)	3.30	F-T	-1324 / 585	0.55 (2)	
F-G	-4190 / 1394	-115.2 -115.2 0.49 (2)	3.59	T-G	-325 / 1247	0.20 (1)	
G-H	-3719 / 1310	-115.2 -115.2 0.53 (1)	3.90	T-H	-574 / 584	0.50 (3)	
H-I	-4186 / 1393	-115.2 -115.2 0.49 (3)	3.59	S-H	-448 / 1412	0.24 (14)	
I-J	-4961 / 1583	-115.2 -115.2 0.58 (3)	3.30	S-I	-1328 / 586	0.55 (3)	
J-K	-4961 / 1583	-115.2 -115.2 0.58 (3)	3.30	I-Q	-286 / 899	0.14 (3)	
K-L	-5371 / 1669	-115.2 -115.2 0.60 (3)	3.18	Q-K	-718 / 374	0.37 (3)	
L-M	-9 / 135	-115.2 -115.2 0.34 (3)	10.00	K-P	-111 / 302	0.06 (14)	
M-N	0 / 41	-115.2 -115.2 0.15 (3)	10.00	P-L	-101 / 343	0.06 (2)	
X-B	-451 / 237	0.0 0.0 0.02 (2)	7.81	X-C	-5882 / 1529	0.92 (1)	
O-M	-451 / 237	0.0 0.0 0.02 (3)	7.81	L-O	-5681 / 1529	0.92 (1)	
X-W	-1533 / 4747	-39.5 -39.5 0.69 (1)	8.25				
W-V	-1987 / 4716	-39.5 -39.5 0.65 (1)	8.25				
V-U	-1150 / 4268	-39.5 -39.5 0.61 (1)	8.25				
U-T	-1150 / 4268	-39.5 -39.5 0.61 (1)	8.25				
T-S	-768 / 3715	-39.5 -39.5 0.54 (7)	8.25				
S-R	-507 / 4268	-39.5 -39.5 0.59 (1)	8.25				
R-Q	-307 / 4268	-39.5 -39.5 0.59 (1)	8.25				
Q-P	-1117 / 4717	-39.5 -39.5 0.65 (1)	8.25				
P-O	-1262 / 4746	-39.5 -39.5 0.69 (1)	8.25				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (B)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	33.4	PSF
DL	=	8.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = U/999 (0.26")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = U/999 (0.38")

CSI: TC=0.80/1.00 (C-D-2), BC=0.69/1.00 (O-P-1), WB=0.92/1.00 (C-X-1), SS=0.27/1.00 (B-C-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

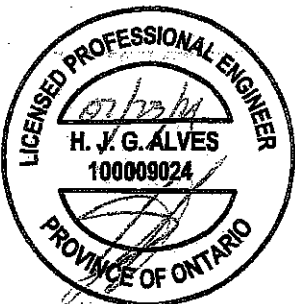
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLJ)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.80)
JSI METAL = 0.86 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2124029 1/2

JOB NAME 418363	TRUSS NAME T57	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

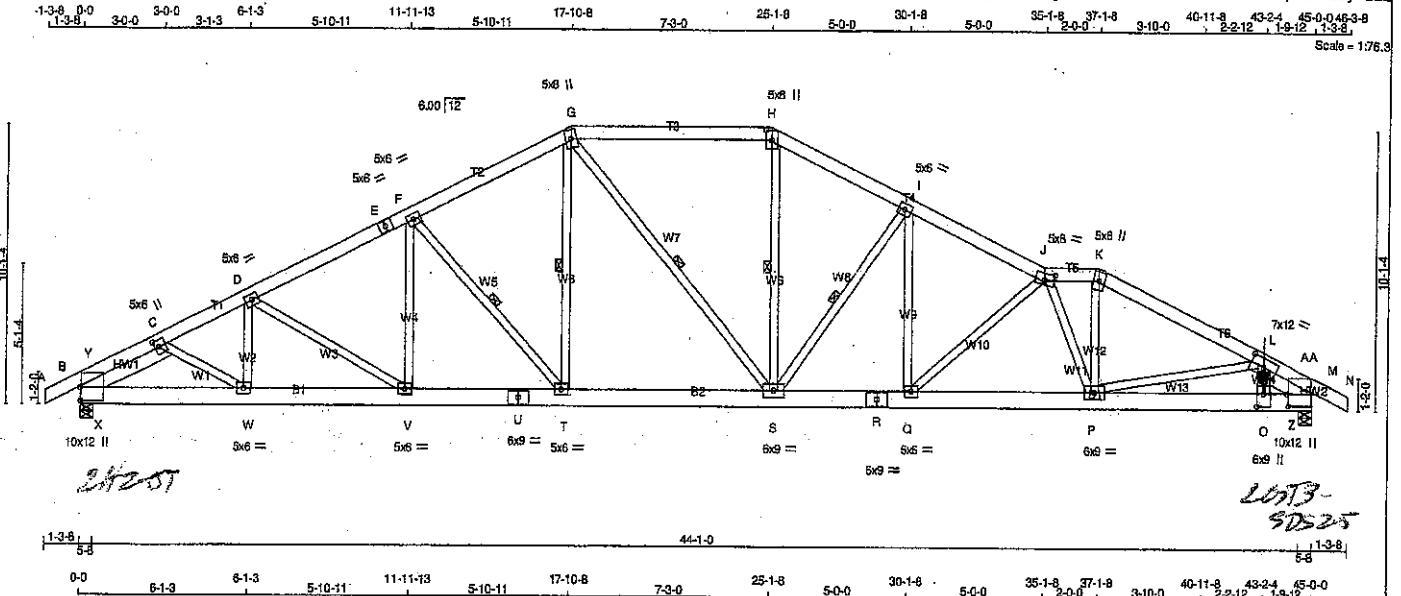
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_q$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124029 *2/2*

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x6	DRY No.2	SPF
E - G	2x6	DRY No.2	SPF
G - H	2x6	DRY No.2	SPF
H - J	2x6	DRY No.2	SPF
J - K	2x6	DRY No.2	SPF
K - N	2x6	DRY No.2	SPF
S - U	2x8	DRY No.2	SPF
U - R	2x8	DRY No.2	SPF
R - M	2x8	DRY No.2	SPF

REINFORCING MEMBERS

MEMBER	SIZE	LUMBER	DESCR.
HW1	2x6	DRY No.2	SPF
HW2	2x6	DRY No.2	SPF

ALL WEBS EXCEPT O - L

MEMBER	SIZE	LUMBER	DESCR.
O - L	2x6	DRY No.2	SPF

DRY, SEASONED LUMBER.
DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-E	2	12	TOP
E-G	2	12	TOP
G-H	2	12	TOP
H-J	2	12	TOP
J-K	2	12	TOP
K-N	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
B-U	2	12	TOP
U-R	2	12	TOP
R-M	4	12	SIDE(2421.8)
WEBS : (0.122"x3") SPIRAL NAILS			
2x4	1	6	SIDE(\$3.1)
L-O	2	6	
2x8	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	3971	0	5-8
B	3971	255	5-8
M	11539	0	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1159 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 3478 LBS. FACTORED UPLIFT

PROVIDE FOR 255 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LCASE - MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2231	1743 / 0	514 / 0	0 / 0	59 / -1153	675 / 0	0 / 0
M	8500	5109 / 0	1452 / 0	0 / 0	819 / -3421	1942 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	182 / -182	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 4.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-T, G-T, H-S, I-S, G-S.

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 6	-115.2 -115.2	0.03 (2)	W-D	-276 / 145	0.01 (8)	
B-Y	-4269 / 1165	-115.2 -115.2	0.05 (13)	D-V	-533 / 255	0.11 (2)	
Y-C	-3003 / 909	-115.2 -115.2	0.05 (1)	V-F	-82 / 497	0.03 (5)	
C-D	-6233 / 1838	-115.2 -115.2	0.08 (13)	F-T	-1805 / 565	0.15 (2)	
D-E	-5902 / 1780	-115.2 -115.2	0.13 (2)	T-G	-361 / 1250	0.07 (2)	
E-F	-5902 / 1780	-115.2 -115.2	0.13 (2)	S-H	-477 / 1849	0.10 (1)	
F-G	-5141 / 1825	-115.2 -115.2	0.12 (2)	S-I	-2584 / 943	0.29 (3)	
G-H	-4937 / 1667	-115.2 -115.2	0.19 (1)	Q-I	-562 / 1997	0.12 (14)	
H-I	-5584 / 1797	-115.2 -115.2	0.10 (14)	Q-J	-2932 / 997	0.56 (3)	
I-J	-7045 / 2160	-115.2 -115.2	0.12 (14)	P-K	-914 / 3340	0.18 (1)	
J-K	-7831 / 2422	-115.2 -115.2	0.09 (14)	P-L	-4532 / 1553	0.75 (3)	
K-L	-8555 / 2549	-115.2 -115.2	0.22 (3)	O-L	-2190 / 7661	0.30 (1)	
L-AA	-8769 / 2807	-115.2 -115.2	0.17 (1)	J-P	-1683 / 498	0.15 (1)	
AA-M	-12252 / 3665	-115.2 -115.2	0.20 (1)	G-S	-348 / 1170	0.09 (14)	
M-N	0 / 6	-115.2 -115.2	0.03 (3)	C-W	-178 / 877	0.05 (1)	
				X-Y	-388 / 1741	0.00 (1)	
B-X	-943 / 2832	-39.5 -39.5	0.13 (1)	X-G	-3058 / 878	0.10 (1)	
X-W	-1591 / 4890	-39.5 -39.5	0.21 (1)	L-Z	-7929 / 2430	0.19 (1)	
W-V	-1716 / 5814	-39.5 -39.5	0.21 (1)	Z-AA	-1603 / 5037	0.00 (1)	
V-U	-1489 / 5294	-39.5 -39.5	0.19 (1)				
U-T	-1489 / 5294	-39.5 -39.5	0.19 (1)				
T-S	-1123 / 4550	-39.5 -39.5	0.18 (1)				
S-R	-1548 / 6331	-39.5 -39.5	0.24 (1)				
R-Q	-1548 / 6331	-39.5 -39.5	0.24 (1)				
Q-P	-2285 / 8442	-39.5 -39.5	0.34 (1)				
P-O	-3545 / 11881	-39.5 -39.5	0.62 (1)				
O-Z	-3700 / 12427	-39.5 -39.5	0.64 (1)				
Z-M	-2201 / 7538	-39.5 -39.5	0.42 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	43-24	-6053	-6053	1413	BACK	VERT	TOTAL	-	CI

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF NBC 2015, ABC 2019
- PART 4 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS UNDEFINED.
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/180 (3.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TO=0.22/1.00 (K-L-3), BC=0.64/1.00 (O-Z-1), WB=0.75/1.00 (L-P-3), SS=0.30/1.00 (M-Z-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

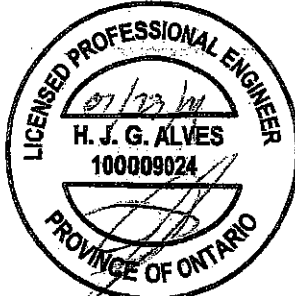
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.80)
JSI METAL= 0.99 (O) (INPUT = 1.00)



Structural component only
DWG# T-2124030

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBMW1-H	MT20	10.0	12.0	5.75	0.25
C TWWW-t	MT20	5.0	6.0	3.00	1.75
D, F, I					
D TWWW-t	MT20	5.0	6.0		
E TS-t	MT20	5.0	6.0		
G TWWW-m	MT20	5.0	8.0		
H TWW-p	MT20	5.0	8.0	4.25	2.50
J TWWW-m	MT20	5.0	8.0	3.25	4.00
K TWW-m	MT20	5.0	8.0		
L TWWWW-t	MT20	7.0	12.0	3.75	6.00
M TMBMW1m	MT20	10.0	12.0	0.50	5.25
O BWW-w	MT20	6.0	9.0	5.50	3.00
P BWWWW-t	MT20	8.0	9.0		
Q, T, V, W					
Q BWWW-t	MT20	5.0	6.0		
R BS-t	MT20	6.0	9.0		
S BWWWW-t	MT20	6.0	9.0		
U BS-t	MT20	6.0	9.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

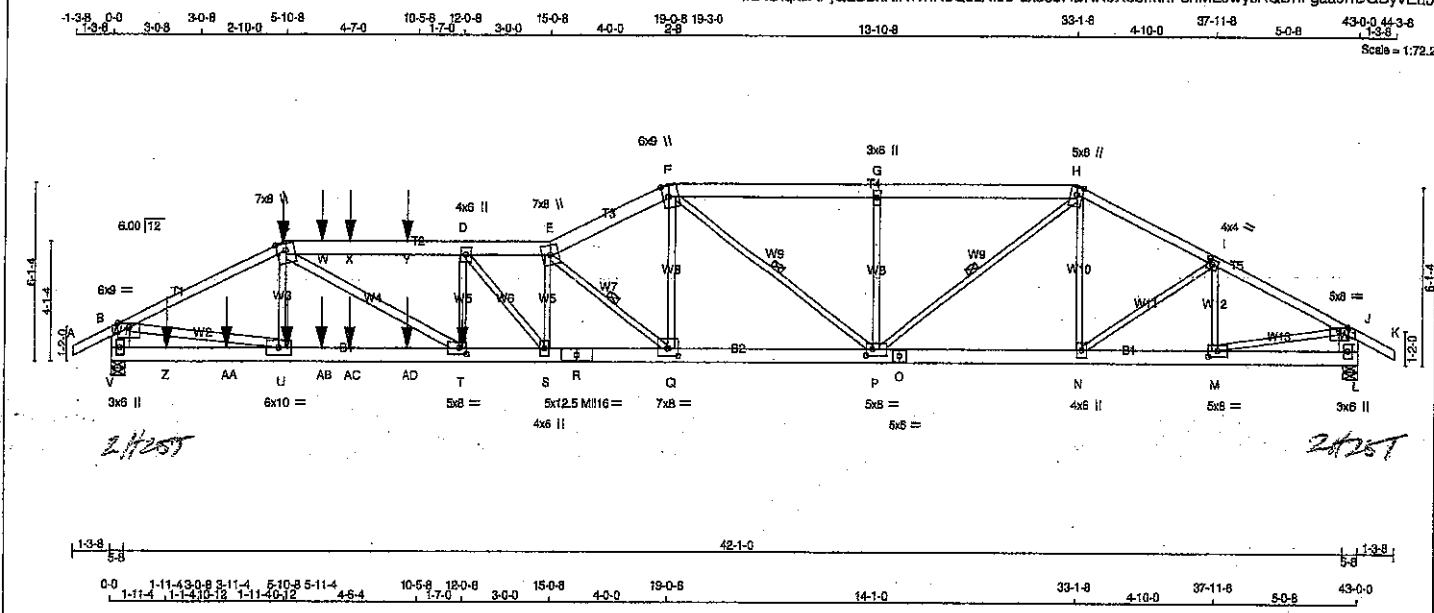
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124030 *202*

JOB NAME 418363	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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 ID:Oiqk5APjQaBbkRIRWnRQszK198-sx8sSHBNN9XdfmhFehMLDwycRQDhPgaa0hDGDyVvEuJ



TOTAL WEIGHT = 2 X 220 = 441 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY 1650F 1.5E	SPF
C - E	2x6	DRY No.2	SPF
E - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
V - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
V - R	2x6	DRY 1650F 1.5E	SPF
R - O	2x6	DRY 1650F 1.5E	SPF
O - L	2x6	DRY 1650F 1.5E	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - T	2x4	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(70.1)
H-K 1	12	TOP
C-E 2	12	SIDE(70.1)
E-F 2	12	TOP
F-H 2	12	TOP
V-B 2	12	TOP
L-J 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-R 2	12	SIDE(210.5)
R-O 2	12	TOP
O-L 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
U-C 1	6	SIDE(22.2)
D-T 1	3	SIDE(32.2)
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	6218	0	6280	-158
V	4385	0	4385	0

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1167 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1037 LBS. FACTORED UPLIFT

PROVIDE FOR 158 LBS. FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
V	4513	2824 / 0	475 / 0	0 / 0	0 / -1139	1255 / 0	0 / 0
L	3209	1951 / 0	455 / 0	0 / 0	30 / -1028	803 / 0	0 / 0

HORIZONTAL REACTIONS

V	0 / 0	0 / 0	0 / 0	113 / -113	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.58 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 41	-115.2 -115.2	0.08 (1)	U-C	-849 / 253
B-C	-9943 / 1719	-115.2 -115.2	0.78 (2)	C-T	-1228 / 7729
C-W	-15574 / 2701	-115.2 -115.2	0.87 (1)	S-E	-337 / 401
W-X	-15574 / 2701	-115.2 -115.2	0.87 (1)	E-Q	-7678 / 1260
X-Y	-15574 / 2701	-115.2 -115.2	0.87 (1)	Q-F	-813 / 5451
Y-D	-15574 / 2701	-115.2 -115.2	0.87 (1)	N-H	-160 / 344
D-E	-15758 / 2930	-115.2 -115.2	0.39 (1)	N-I	-19 / 634
E-F	-11208 / 2310	-115.2 -115.2	0.26 (2)	M-I	-832 / 267
F-G	-8836 / 2061	-115.2 -115.2	0.38 (1)	B-U	-1402 / 8994
G-H	-8837 / 2062	-115.2 -115.2	0.37 (1)	T-D	-978 / 645
H-I	-6780 / 1561	-115.2 -115.2	0.49 (3)	D-S	-427 / 383
I-J	-6574 / 1516	-115.2 -115.2	0.50 (3)	P-H	-806 / 3835
J-K	0 / 41	-115.2 -115.2	0.10 (3)	P-P	-1880 / 114
V-B	-8160 / 1183	0.0 0.0	0.17 (2)	G-P	-1004 / 521
L-J	-4247 / 1043	0.0 0.0	0.11 (1)	M-J	-1250 / 5972
V-Z	-142 / 162	-39.5 -39.5	0.07 (17)		
Z-AA	-142 / 162	-39.5 -39.5	0.07 (17)		
AA-U	-142 / 162	-39.5 -39.5	0.07 (17)		
U-AB	-1529 / 8936	-39.5 -39.5	0.44 (1)		
AB-AC	-1529 / 8936	-39.5 -39.5	0.44 (1)		
AC-AD	-1529 / 8936	-39.5 -39.5	0.44 (1)		
AD-T	-1529 / 8936	-39.5 -39.5	0.44 (1)		
T-S	-2668 / 15574	-39.5 -39.5	0.75 (1)		
S-R	-2825 / 15768	-39.5 -39.5	0.78 (1)		
R-Q	-2825 / 15768	-39.5 -39.5	0.78 (1)		
Q-P	-1897 / 10124	-39.5 -39.5	0.45 (1)		
P-O	-1184 / 8053	-39.5 -39.5	0.29 (1)		
O-N	-1184 / 8053	-39.5 -39.5	0.29 (1)		
N-M	-1220 / 5903	-39.5 -39.5	0.29 (1)		
M-L	-8 / 16	-39.5 -39.5	0.08 (1)		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 33.4 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPO 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(90 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/890 (1.43")
CALCULATED VERT. DEFL. (LL) = L/999 (0.40")
ALLOWABLE DEFL. (TL) = L/890 (2.87")
CALCULATED VERT. DEFL. (TL) = L/894 (0.58")

CSI: TC=0.78/1.00 (B-C-2), BC=0.78/1.00 (Q-S-1), WB=0.92/1.00 (E-Q-2), SSI=0.16/1.00 (C-D-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

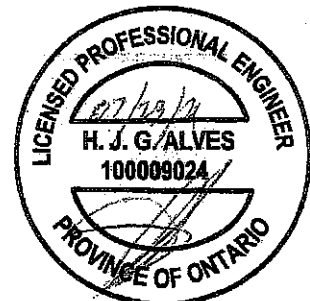
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873
MH18	438 302	2547 1256	4283 1816

Structural component only
DWG# T-2124031

CONTINUED ON PAGE 2



Structural component only
 DWG# T-2124031

CONTINUED ON PAGE 2

JOB NAME 418363	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Fri Jul 23 09:11:06 2021 Page 2	
ID:Oigk5APiQaBbkRiRWnRoQszK198-sx3s5HBNN9XdbimhFehML0wycRQDhPqaa0hDGDyveUj					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TTWW+m	MT20	7.0	8.0		
F	TTWW+m	MT20	6.0	9.0	Edge 2.25	
G	TMW-w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	8.0	Edge	
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	1.50	3.75
L	BMV1-p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.75
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.50
Q	BMWW-t	MT20	7.0	8.0	3.00	4.00
R	BS-t	MT16	5.0	12.5		
S	BMWW-t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	8.0	2.50	3.00
U	BMWW-t	MT20	6.0	10.0		
V	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-151	-196	38	FRONT	VERT	TOTAL	---	C1
C	5-10-8	-216	-378	---	BACK	VERT	TOTAL	---	C1
T	12-0-8	-1904	-1904	---	BACK	VERT	TOTAL	---	C1
U	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	7-1-4	-60	-127	---	BACK	VERT	TOTAL	---	C1
X	8-1-4	-60	-127	---	BACK	VERT	TOTAL	---	C1
Y	10-1-4	-60	-127	---	BACK	VERT	TOTAL	---	C1
Z	1-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
AA	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	7-1-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	8-1-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	10-1-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpG, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.

PLATE PLACEMENT TOL = 0.250 inches

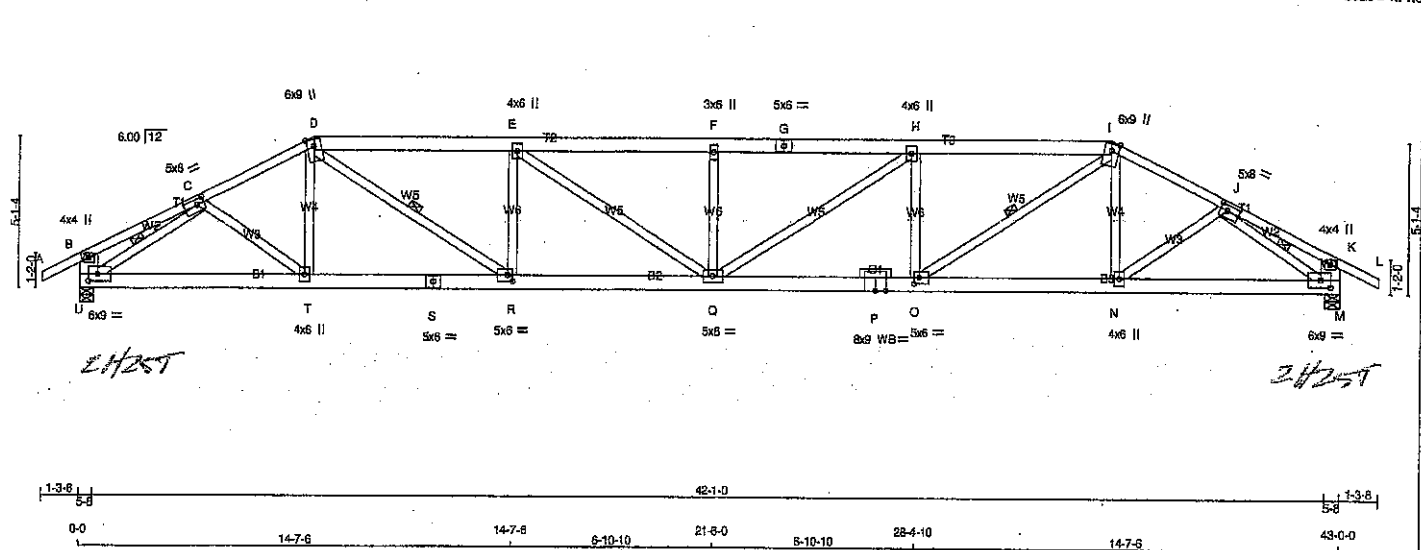
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)

JSI METAL= 0.76 (E) (INPUT = 1.00)



Structural component only
DWG# T-2124031 *212*



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x6	DRY No.2	SPF
G - I	2x6	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
L - B	2x8	DRY No.2	SPF
U - S	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF

ALL WEBS EXCEPT: 2x4 DRY No.2 SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW+t	MT20	5.0	8.0	2.25	3.00
D	TTWW+m	MT20	8.0	9.0	Edge	
E	TMWW+t	MT20	4.0	6.0		
F	TMWW+w	MT20	3.0	6.0		
G	TS+t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMWW+t	MT20	5.0	8.0	2.25	3.00
K	TMV+p	MT20	4.0	4.0		
M	BMWW+1	MT20	6.0	9.0	3.00	3.75
N	BMWW+t	MT20	4.0	6.0		
O	BMWW+t	MT20	5.0	6.0	2.50	2.00
P	BS+t	MT20	8.0	9.0		
Q	BMWWW+t	MT20	5.0	8.0		
R	BMWW+t	MT20	5.0	6.0	2.50	2.00
S	BS+t	MT20	5.0	6.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMWW+1	MT20	6.0	9.0	3.00	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

NOTES (1)



Structural component only
DWG# T-2124032

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	3483	0	3571	-139
M	3483	0	3571	0

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1124 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT M FOR 1124 LBS FACTORED UPLIFT

PROVIDE FOR 139 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

	1ST LOOSE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT								
U	2572	1588	0	452	0	0	0	0
M	2572	1588	0	452	0	0	0	0

HORIZONTAL REACTIONS

	U	M
U	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.56 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 5.52 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-U, J-M, I-O, D-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (16)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-115.2 -115.2	0.17 (2)	C-T	-120 / 661	0.11 (3)	
B-C	-9 / 118	-115.2 -115.2	0.29 (2)	T-D	-144 / 275	0.04 (17)	
C-D	-5157 / 1729	-115.2 -115.2	0.88 (2)	D-E	-161 / 259	0.04 (17)	
D-E	-7028 / 2393	-115.2 -115.2	0.70 (1)	E-F	-44 / 681	0.11 (2)	
E-F	-7785 / 2580	-115.2 -115.2	0.79 (1)	F-G	-5332 / 1655	0.71 (2)	
F-G	-7785 / 2580	-115.2 -115.2	0.79 (1)	G-H	-5332 / 1655	0.71 (3)	
G-H	-7785 / 2580	-115.2 -115.2	0.79 (1)	H-I	-1049 / 3090	0.50 (2)	
H-I	-7028 / 2393	-115.2 -115.2	0.88 (3)	I-J	-1049 / 3090	0.50 (3)	
I-J	-5157 / 1729	-115.2 -115.2	0.29 (3)	J-K	-1509 / 655	0.36 (2)	
J-K	-9 / 118	-115.2 -115.2	0.17 (3)	K-L	-1509 / 655	0.36 (3)	
K-L	0 / 41	-115.2 -115.2	0.17 (3)	L-M	-392 / 1067	0.33 (13)	
L-M	-393 / 203	0.0	0.02 (2)	M-N	-392 / 1067	0.33 (14)	
M-N	-393 / 203	0.0	0.02 (3)	N-O	-760 / 387	0.18 (1)	
N-O				O-P			
O-P	-1484 / 4375	-39.5	-39.5	0.66 (2)			
P-Q	-1392 / 4807	-39.5	-39.5	0.83 (1)			
Q-R	-1392 / 4807	-39.5	-39.5	0.66 (1)			
R-Q	-2180 / 7028	-39.5	-39.5	0.94 (1)			
Q-P	-2104 / 7028	-39.5	-39.5	0.94 (1)			
P-O	-2104 / 7028	-39.5	-39.5	0.94 (1)			
O-N	-1271 / 4607	-39.5	-39.5	0.66 (1)			
N-M	-1349 / 4375	-39.5	-39.5	0.66 (3)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 33.4 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.4 PSF

TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2018
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 888-14
- TPC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.43")

CALCULATED VERT. DEFL.(LL) = U/999 (0.43")

ALLOWABLE DEFL.(TL) = L/180 (2.87")

CALCULATED VERT. DEFL.(TL) = U/874 (0.59")

CSI: TO=0.79/1.00 (F-H:1), BC=0.94/1.00 (O-Q:1), WB=0.71/1.00 (J-M:3), SS=0.28/1.00 (H-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

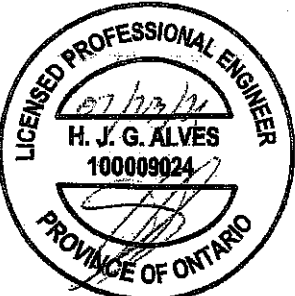
MT20 650 371 1747 786 1867 1873

PLATE PLACEMENT TOL = 0.250 inches

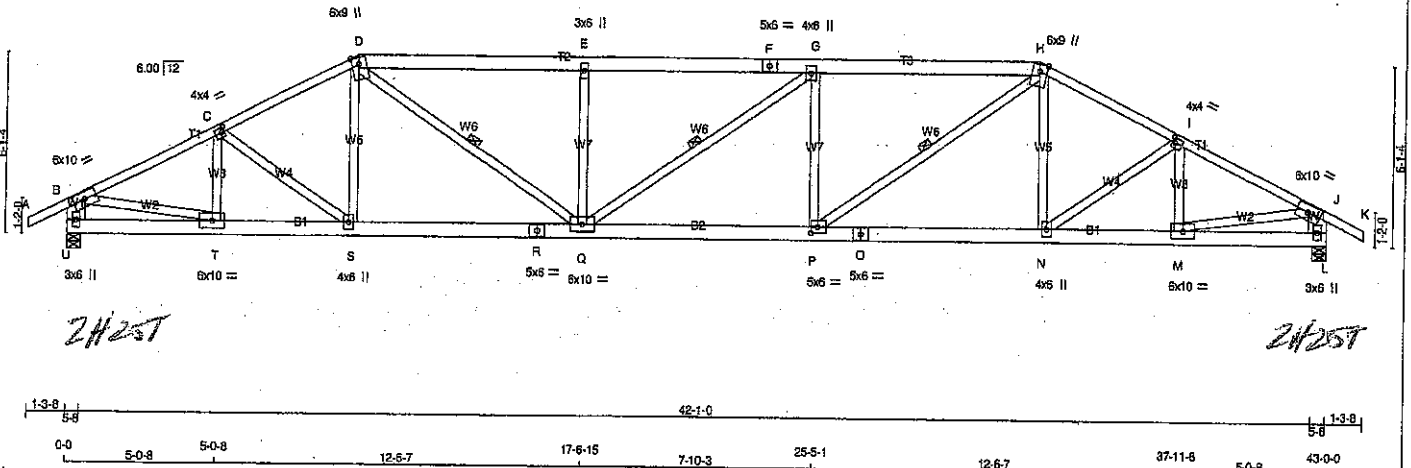
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.69 (I) (INPUT = 0.90)

JSI METAL = 0.96 (C) (INPUT = 1.00)

JOB NAME 418363	TRUSS NAME T60	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Fri Jul 23 09:11:07 2021 Page 2 ID:Olnk5APiQaBbkRIRWnRQszKj98-K7iEldB08TUDpLtoLCbuDT78qiQvAkpgQmpgyvYut			
		WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.			
 Structural component only DWG# T-2124032 <i>WJ</i>					

JOB NAME 418363	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Fri Jul 23 09:11:09 2021 Page 1	
ID:Olk5APJQaBbkRIRWnRoQszKI98-HWp?JDGg4vBSGVFwmF3zeYTbeQaup5tG_vtYvEuG				Scale = 1:71.3	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	1650F 1.5E
D - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - K	2x4	DRY	1650F 1.5E
U - B	2x8	DRY	No.2
L - J	2x8	DRY	No.2
U - R	2x8	DRY	No.2
R - O	2x8	DRY	No.2
O - L	2x8	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-I	MT20	6.0	10.0		
C TMWV-I	MT20	4.0	4.0	2.00	1.25
D TTWV+m	MT20	6.0	9.0	Edge	
E TMWV-w	MT20	3.0	6.0		
F TS-I	MT20	5.0	8.0		
G TMWV-I	MT20	4.0	8.0		
H TTWV+m	MT20	6.0	9.0	Edge	
I TMWV-I	MT20	4.0	4.0	2.00	1.25
J TMWV-I	MT20	6.0	10.0		
L BMV1-p	MT20	3.0	6.0		
M BMWV-I	MT20	6.0	10.0		
N BMWV-I	MT20	4.0	6.0		
O BS-I	MT20	5.0	8.0		
P BMWV-I	MT20	5.0	6.0	2.50	2.75
Q BMWV-I	MT20	6.0	10.0		
R BS-I	MT20	5.0	8.0		
S BMWV-I	MT20	4.0	6.0		
T BMWV-I	MT20	6.0	10.0		
U BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	3483	3560	0	0
U	HORIZ	0	-158	-1109	5-8
L	HORIZ	3483	3560	0	-1109

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1109 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1109 LBS. FACTORED UPLIFT

PROVIDE FOR 158 LBS. FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	2572	1578 / 0	452 / 0	0 / 0	0 / -1078	592 / 0	0 / 0
L	2572	1578 / 0	452 / 0	0 / 0	28 / -1078	592 / 0	0 / 0

HORIZONTAL REACTIONS

U	0 / 0	0 / 0	0 / 0	113 / -113	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.90 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, D-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	T-C	-587 / 277
B-C	-5088 / 1643	C-S	-196 / 365
C-D	-5037 / 1698	S-D	-67 / 439
D-E	-6259 / 2128	N-H	-65 / 437
E-F	-6259 / 2125	M-I	-219 / 364
F-G	-6259 / 2125	M-I	-588 / 276
G-H	-6263 / 2123	G-T	-1365 / 4634
H-I	-5038 / 1698	M-J	-1365 / 4634
I-J	-5038 / 1643	P-H	-811 / 2437
J-K	0 / 41	D-Q	-807 / 2432
U-B	-3441 / 1117	P-G	-1137 / 556
L-J	-3441 / 1117	Q-E	-1002 / 516
		Q-G	-235 / 226
U-T	-142 / 183		
T-S	-1481 / 4580		
S-R	-1329 / 4483		
R-Q	-1320 / 4483		
Q-P	-1818 / 6282		
P-O	-1182 / 4482		
O-N	-1182 / 4482		
N-M	-1334 / 4580		
M-L	-8 / 16		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 33.4 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 688-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.34")
ALLOWABLE DEFL.(TL) = L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.47")

CSI: TC=0.79/1.00 (E-G:1), BC=0.85/1.00 (P-Q:1), WB=0.74/1.00 (J-M:3), SS=0.32/1.00 (G-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

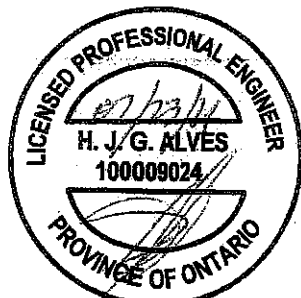
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.80 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2124033

JOB NAME 418363	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Fri Jul 23 09:11:09 2021 Page 2

ID:0lqk5APlQaBbkRlRWnRoQszKl98-HWb?JDDGp4yBS6VFwmF3zeYTbeQaup51G_vitYwEuG

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



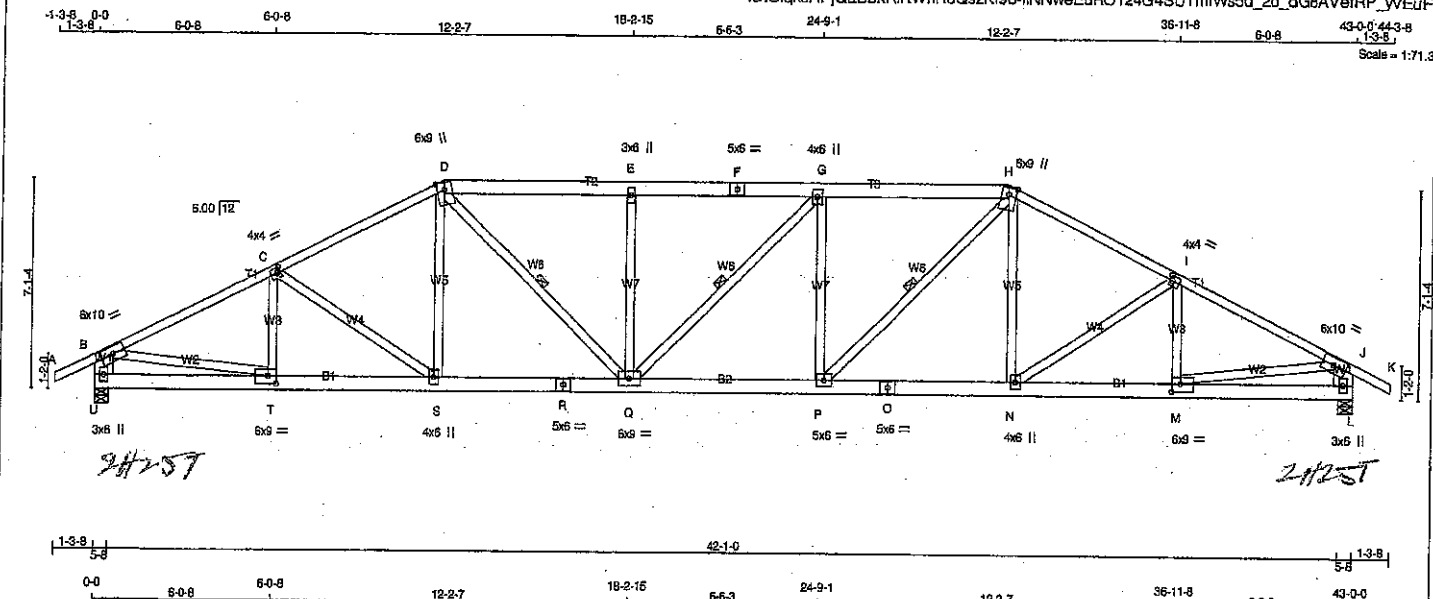
Structural component only
DWG# T-2124033 *mm*

JOB NAME 418363	TRUSS NAME T62	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:Olk5APjCaBbkRWRnRoQszK198-i1NNwEuRo124G4SUTmIW5d_2o_dGBAVeIRP_yvEuF

Scale = 1:71.3



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	2100F 1.8E	SPF
U - S	2x8	DRY	No.2	SPF
L - J	2x8	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x8	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	6.0	10.0		
C TMVW-t	MT20	4.0	4.0	2.00	1.25
D TTWW+m	MT20	6.0	9.0	Edge	
E TMVW-t	MT20	3.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW-t	MT20	4.0	6.0		
H TTWW+m	MT20	6.0	9.0	Edge	
I TMVW-t	MT20	4.0	4.0	2.00	1.25
J TMVW-t	MT20	6.0	10.0		
L BMV1+p	MT20	3.0	6.0		
M BMVW-t	MT20	6.0	9.0	3.25	3.50
N BMVW-t	MT20	4.0	6.0		
O BS-t	MT20	5.0	6.0		
P BMVW-t	MT20	5.0	6.0		
Q BMVW-t	MT20	6.0	9.0		
R BS-t	MT20	5.0	6.0		
S BMVW-t	MT20	4.0	6.0		
T BMVW-t	MT20	8.0	9.0	3.25	3.50
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT				
U	3483	0	3537	-178
L	3483	0	3537	0

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1092 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 1092 LBS FACTORED UPLIFT

PROVIDE FOR 178 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS					
JT COMBINED		LIVE	PERM LIVE	WIND	DEAD	SOIL
U	2572	1563 / 0	452 / 0	0 / 0	0 / -1065	592 / 0
L	2572	1563 / 0	452 / 0	0 / 0	47 / -1065	592 / 0

HORIZONTAL REACTIONS

U	0 / 0	0 / 0	0 / 0	127 / -127	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-P, D-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 41	-115.2	-115.2	0.11 (2)	T-C	-441 / 249	0.07 (8)
B-C	-5189 / 1648	-115.2	-115.2	0.88 (2)	C-S	-582 / 317	0.35 (2)
C-D	-4844 / 1628	-115.2	-115.2	0.79 (2)	S-D	-138 / 597	0.10 (2)
D-E	-5334 / 1824	-115.2	-115.2	0.48 (1)	N-H	-136 / 593	0.10 (3)
E-F	-5334 / 1822	-115.2	-115.2	0.49 (1)	N-I	-583 / 318	0.35 (3)
F-G	-5334 / 1822	-115.2	-115.2	0.49 (1)	M-I	-445 / 248	0.07 (8)
G-H	-5334 / 1822	-115.2	-115.2	0.49 (1)	E-T	-1352 / 4710	0.76 (1)
H-I	-4843 / 1625	-115.2	-115.2	0.79 (3)	M-J	-1352 / 4711	0.76 (1)
I-J	-5190 / 1648	-115.2	-115.2	0.88 (3)	P-H	-587 / 1774	0.29 (2)
J-K	0 / 41	-115.2	-115.2	0.11 (2)	D-Q	-582 / 1767	0.28 (3)
U-B	-3416 / 1106	0.0	0.0	0.17 (2)	P-G	-1028 / 488	0.58 (2)
L-J	-3416 / 1106	0.0	0.0	0.17 (3)	C-E	-833 / 428	0.46 (1)
					Q-G	-279 / 267	0.12 (3)
U-T	-181 / 189	-39.5	-39.5	0.14 (5)			
T-S	-1500 / 4672	-39.5	-39.5	0.64 (1)			
S-R	-1233 / 4299	-39.5	-39.5	0.60 (1)			
R-Q	-1233 / 4299	-39.5	-39.5	0.60 (1)			
Q-P	-1473 / 5338	-39.5	-39.5	0.71 (1)			
P-O	-1068 / 4299	-39.5	-39.5	0.60 (1)			
O-N	-1068 / 4299	-39.5	-39.5	0.60 (1)			
N-M	-1326 / 4672	-39.5	-39.5	0.64 (1)			
M-L	-8 / 16	-39.5	-39.5	0.14 (6)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 33.4 PSF
DL	= 6.0 PSF
SOT CH. LL	= 10.5 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 57.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.45")
CALCULATED VERT. DEFL.(LL) = $L/939$ (0.27")
ALLOWABLE DEFL.(TL) = $L/180$ (2.87")
CALCULATED VERT. DEFL.(TL) = $L/939$ (0.37")

CSI: TC=0.88/1.00 (J-J3), BC=0.71/1.00 (P-Q:1), WB=0.76/1.00 (J-M:1), SSI=0.35/1.00 (J-J3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION (PSI)	(PL)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

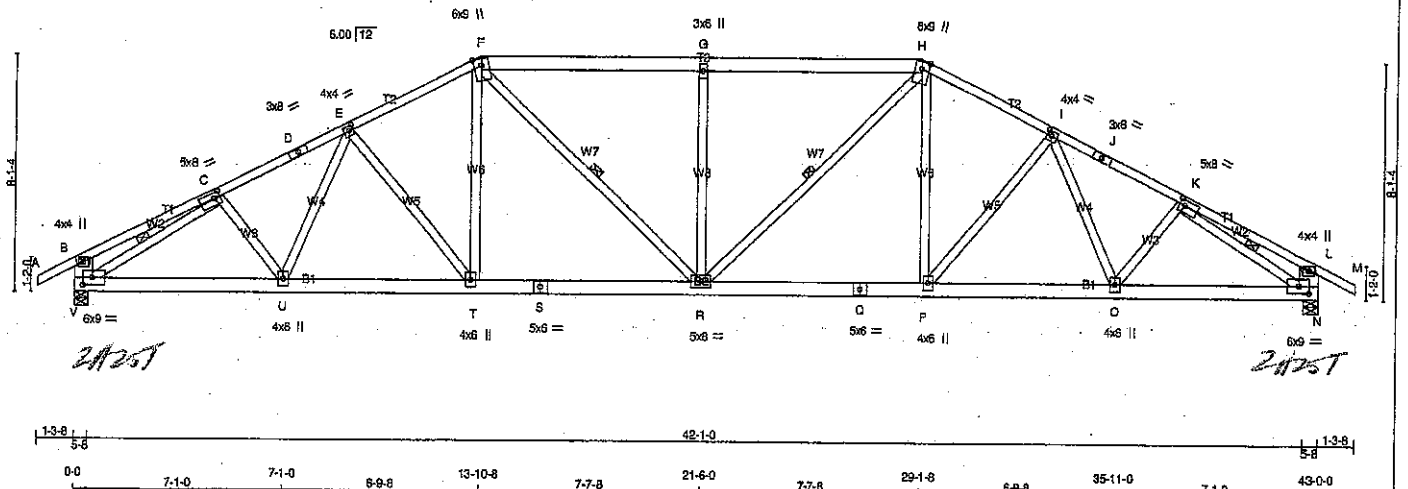
JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.80 (R) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2124034

JOB NAME 418363	TRUSS NAME T62	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 09:11:10 2021 Page 2 ID:OicK5APIQaBbkRIRWnRoQszKl98-lINNweEuRO124G4SUTmIWss5d 2o_dG8AVefRP_wEuf			
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.					
Structural component only DWG# T-2124034 <i>me</i>					



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
J - M	2x4	DRY No.2	SPF
V - B	2x8	DRY No.2	SPF
N - L	2x8	DRY No.2	SPF
V - S	2x8	DRY No.2	SPF
S - Q	2x6	DRY No.2	SPF
Q - N	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0	
C	TMVW-t	MT20	5.0	8.0	2.25 2.25
D	TS-t	MT20	3.0	8.0	
E	TMVW-t	MT20	4.0	4.0	2.00 1.75
F	TMVW+m	MT20	6.0	9.0	Edge
G	TMVW-w	MT20	3.0	8.0	
H	TMVW+m	MT20	6.0	9.0	Edge
I	TMVW-t	MT20	4.0	4.0	2.00 1.75
J	TS-t	MT20	3.0	8.0	
K	TMVW-t	MT20	5.0	8.0	2.25 2.25
L	TMV+p	MT20	4.0	4.0	
N	BMVW-t	MT20	6.0	9.0	3.00 3.75
O, P, T, U					
Q	BMVW-t	MT20	4.0	6.0	
R	BS-t	MT20	5.0	6.0	
S	BMVW-t	MT20	5.0	8.0	
V	BS-t	MT20	5.0	6.0	
V	BMVW-t	MT20	6.0	9.0	3.00 3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
V	3483	0	3503	200	-1072	5-8
N	3483	0	3503	0	-1072	5-8

PROVIDE ANCHORAGE AT BEARING JOINT V FOR 1072 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 1072 LBS. FACTORED UPLIFT

PROVIDE FOR 200 LBS. FACTORED HORIZONTAL REACTION AT JOINT V

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
V	2572	1541 / 0	452 / 0	0 / 0	0 / -1051	592 / 0	0 / 0
N	2572	1541 / 0	452 / 0	0 / 0	64 / -1051	592 / 0	0 / 0

HORIZONTAL REACTIONS	
V	0 / 0
N	143 / -143

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R, H-R, C-V, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-115.2 -115.2	0.17 (2)	10.00	C-U	-38 / 401	0.06 (3)
B-C	-9 / 133	-115.2 -115.2	0.35 (2)	10.00	U-E	-112 / 268	0.05 (10)
C-D	-5089 / 1861	-115.2 -115.2	0.85 (2)	2.49	E-T	-760 / 378	0.49 (2)
D-E	-5089 / 1861	-115.2 -115.2	0.85 (2)	2.49	T-F	-244 / 893	0.20 (13)
E-F	-4800 / 1559	-115.2 -115.2	0.73 (2)	2.75	F-R	-458 / 1380	0.28 (14)
F-G	-4801 / 1626	-115.2 -115.2	0.74 (1)	3.23	R-G	-1122 / 577	0.90 (1)
G-H	-4801 / 1626	-115.2 -115.2	0.74 (1)	3.23	H-R	-459 / 1880	0.28 (13)
H-I	-4800 / 1559	-115.2 -115.2	0.73 (3)	2.75	P-H	-244 / 893	0.20 (14)
I-J	-5089 / 1861	-115.2 -115.2	0.85 (3)	2.49	P-I	-760 / 379	0.49 (3)
J-K	-5089 / 1861	-115.2 -115.2	0.85 (3)	2.49	I-O	-123 / 288	0.05 (9)
K-L	-9 / 133	-115.2 -115.2	0.35 (3)	10.00	O-K	-50 / 401	0.05 (2)
L-M	0 / 41	-115.2 -115.2	0.17 (3)	10.00	V-C	-5364 / 1551	0.83 (1)
V-B	-444 / 233	0.0	0.02 (2)	7.81	K-N	-5364 / 1552	0.83 (1)
N-L	-444 / 233	0.0	0.02 (3)	7.81			
V-U	-1477 / 4467	-39.5 -39.5	0.67 (1)	8.25			
U-T	-1359 / 4491	-39.5 -39.5	0.84 (1)	8.25			
T-S	-1117 / 4081	-39.5 -39.5	0.80 (1)	8.25			
S-R	-1117 / 4081	-39.5 -39.5	0.80 (1)	8.25			
R-Q	-963 / 4081	-39.5 -39.5	0.80 (1)	8.25			
Q-P	-963 / 4081	-39.5 -39.5	0.80 (1)	8.25			
P-O	-1158 / 4491	-39.5 -39.5	0.64 (1)	8.25			
O-N	-1277 / 4467	-39.5 -39.5	0.67 (1)	8.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(b)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2015, ABC 2015
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.
(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.85/1.00 (H-K3), BC=0.67/1.00 (N-O-1), WB=0.80/1.00 (G-R-1), SS=0.33/1.00 (F-G-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (V) (INPUT = 0.80)
JSI METAL= 0.98 (C) (INPUT = 1.00)



JOB NAME 418363	TRUSS NAME T63	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Fri Jul 23 09:11:11 2021 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



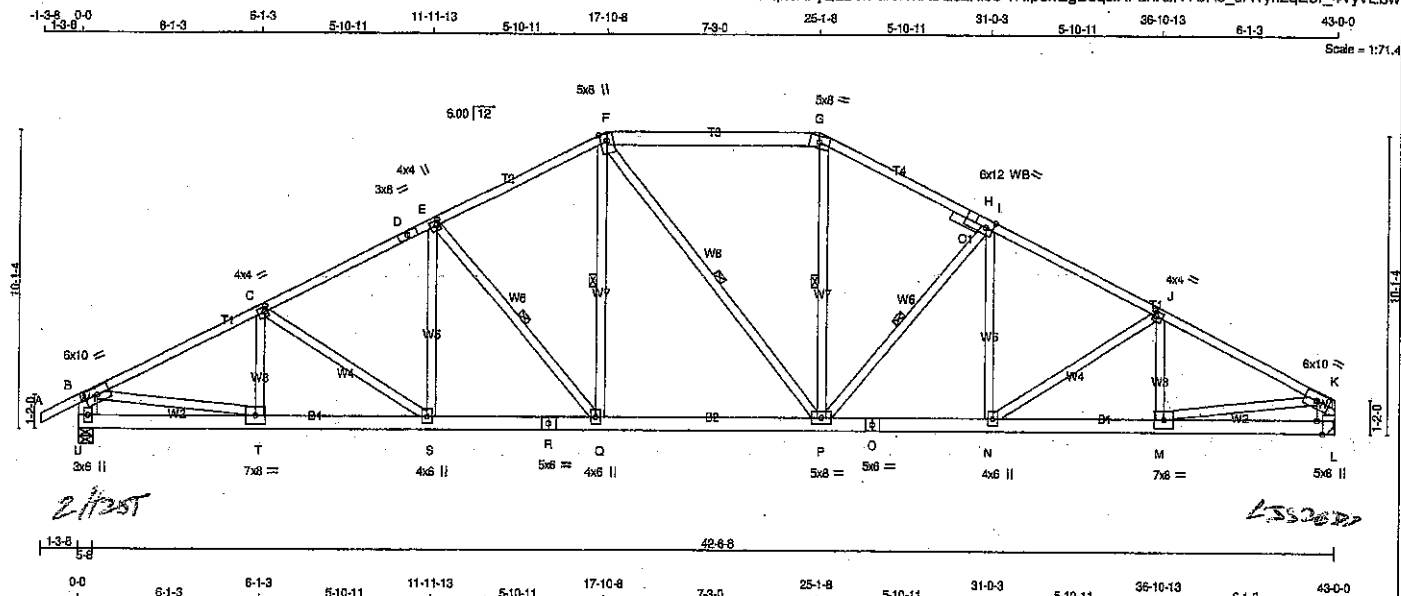
Structural component only
DWG# T-2124035 *2/2*

JOB NAME 418363	TRUSS NAME T64	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 09:11:12 2021 Page 2 ID:Ogk5APIQaBbkRIRWnRoQszKI98-h5V7LKF8z?HmJaEqbuombHA?vrTV57BTzy8XUtyvEuD					
WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.8) PSF AT (30-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.					
 Structural component only DWG# T-2124036 <i>ML</i>					

JOB NAME 418363	TRUSS NAME T65	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	2100F1.8E	SPF
D - F	2x4	DRY	2100F1.8E	SPF
F - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	2100F1.8E	SPF
H - K	2x4	DRY	2100F1.8E	SPF
U - B	2x8	DRY	No.2	SPF
L - K	2x8	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-t	MT20	6.0	10.0		
C TMWV-t	MT20	4.0	4.0	2.00	1.25
D TS-t	MT20	3.0	8.0		
E TMWV-t	MT20	4.0	4.0	2.00	1.25
F TTWV-m	MT20	5.0	8.0	Edge	
G TTW-m	MT20	5.0	8.0		
H TSWV-t	MT20	6.0	12.0	Edge	3.00
J TMWV-t	MT20	4.0	4.0	2.00	1.25
K TMWV-t	MT20	6.0	10.0		
L BMV-t	MT20	5.0	8.0	Edge	2.25
M BMWV-t	MT20	7.0	8.0		
N Q, S					
N BMWV-t	MT20	4.0	6.0		
O BS-t	MT20	5.0	6.0		
P BMWV-t	MT20	5.0	8.0		
R BS-t	MT20	5.0	6.0		
T BMWV-t	MT20	7.0	8.0		
U BMV-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
U	3483	0	3483	271
L	3327	0	3327	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1022 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 964 LBS. FACTORED UPLIFT

PROVIDE FOR 271 LBS. FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	2572	1528 / 0	452 / 0	0 / 0	30 / -1015	582 / 0	0 / 0
L	2465	1437 / 0	452 / 0	0 / 0	101 / -985	576 / 0	0 / 0

HORIZONTAL REACTIONS

U	0 / 0	0 / 0	0 / 0	184 / -155	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, F-Q, F-P, G-P, I-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (16)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 41	-115.2	-115.2 0.11 (2)	10.00	T-C	-419 / 228	0.07 (7)
B-C	-5180 / 1513	-115.2	-115.2 0.75 (2)	3.42	C-S	-570 / 288	0.37 (2)
C-D	-4825 / 1493	-115.2	-115.2 0.67 (2)	3.55	E-S	-115 / 534	0.09 (2)
D-E	-4825 / 1493	-115.2	-115.2 0.67 (2)	3.55	E-Q	-1273 / 555	0.48 (2)
E-F	-4067 / 1332	-115.2	-115.2 0.59 (2)	3.86	Q-F	-360 / 1242	0.20 (2)
F-G	-3806 / 1273	-115.2	-115.2 0.63 (1)	3.80	F-P	-551 / 558	0.46 (2)
G-H	-4070 / 1332	-115.2	-115.2 0.58 (3)	3.86	P-G	-263 / 1098	0.18 (1)
H-I	-4070 / 1332	-115.2	-115.2 0.58 (3)	3.86	P-I	-1268 / 552	0.48 (3)
I-J	-4824 / 1490	-115.2	-115.2 0.67 (3)	3.55	N-I	-112 / 529	0.09 (3)
J-K	-5181 / 1507	-115.2	-115.2 0.75 (3)	3.42	N-J	-572 / 285	0.37 (3)
U-B	-3866 / 1037	0.0	0.0 0.18 (1)	6.49	M-J	-423 / 232	0.07 (8)
L-K	-3210 / 979	0.0	0.0 0.18 (1)	6.81	B-T	-1225 / 4695	0.75 (1)
				M-K	-1256 / 4886	0.75 (1)	
U-T	-258 / 233	-39.5	-39.5 0.13 (4)	6.25			
T-S	-1471 / 4858	-39.5	-39.5 0.64 (1)	8.25			
S-R	-1229 / 4316	-39.5	-39.5 0.81 (1)	8.25			
R-Q	-1229 / 4316	-39.5	-39.5 0.81 (1)	8.25			
Q-P	-864 / 3803	-39.5	-39.5 0.54 (1)	8.25			
P-O	-991 / 4315	-39.5	-39.5 0.82 (1)	8.25			
O-N	-991 / 4315	-39.5	-39.5 0.82 (1)	8.25			
N-M	-1230 / 4659	-39.5	-39.5 0.64 (1)	8.25			
M-L	-8 / 16	-39.5	-39.5 0.13 (4)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	33.4	PSF
DL	=	8.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	57.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.75/1.00 (J-K-3), BC=0.84/1.00 (M-N-1),
WB=0.75/1.00 (K-M-1), SS=0.34/1.00 (J-K-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

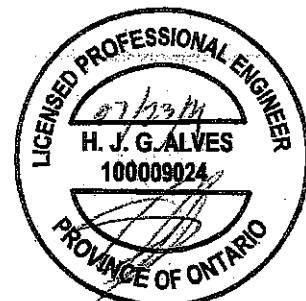
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.78 (M) (INPUT = 1.00)



Structural component only
DWG# T-2124037

CONTINUED ON PAGE 2

JOB NAME 418363	TRUSS NAME T65	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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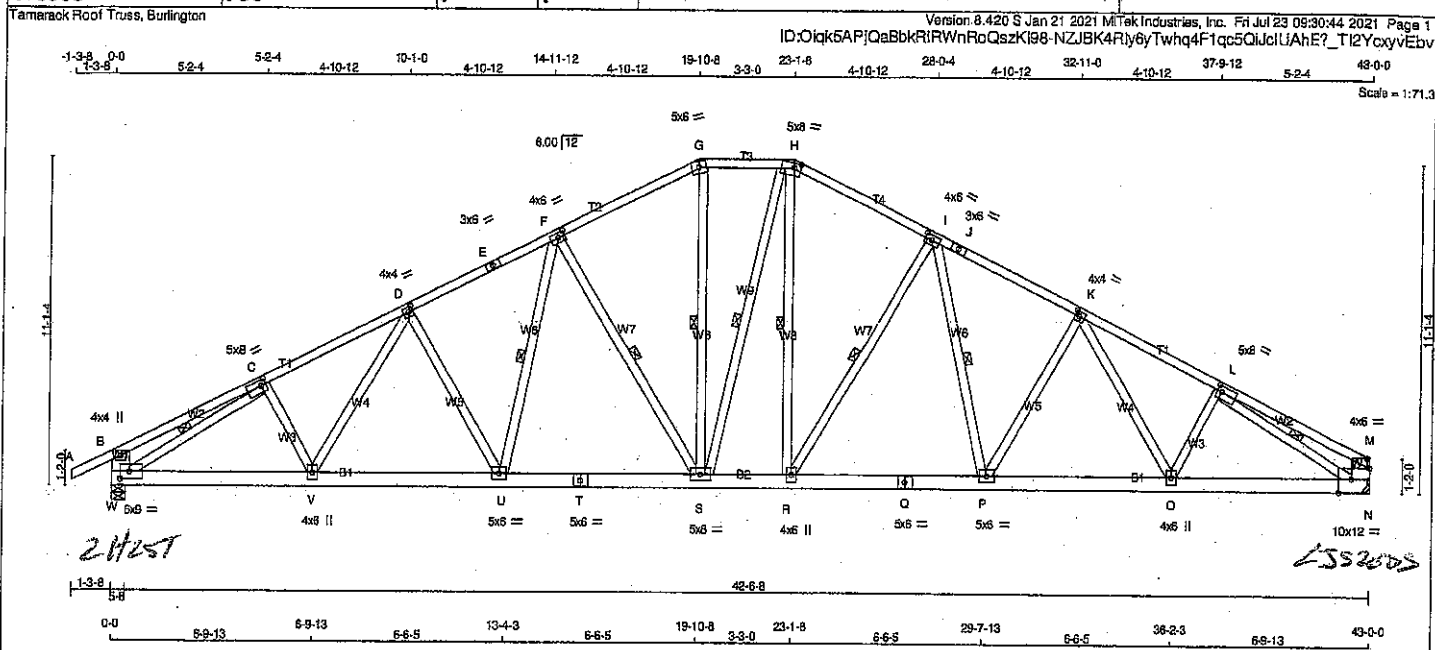
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124037 *me*



TOTAL WEIGHT = 261 lb

LUMBER

N.L.G.A. RULES	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	SPF
E - G	2x4	DRY	SPF
G - H	2x4	DRY	SPF
H - J	2x4	DRY	SPF
J - M	2x4	DRY	SPF
N - S	2x8	DRY	SPF
T - Q	2x6	DRY	SPF
Q - N	2x8	DRY	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
W - C 2x4 DRY 1650F 1.5E SPF
L - N 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW-t	MT20	5.0	8.0	2.25	2.00
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	6.0	2.00	2.75
G	TTW-m	MT20	5.0	6.0		
H	TTWW-m	MT20	5.0	8.0	2.00	2.75
I	TMVW-t	MT20	4.0	6.0	2.00	2.75
J	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	4.0	4.0	2.00	1.75
L	TMVW-t	MT20	5.0	8.0	2.25	2.00
M	TMV-p	MT20	4.0	9.0	4.00	0.75
N	BMVW-t	MT20	10.0	12.0	Edge	4.75
O, R, V						
Q	BMVW-t	MT20	4.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
S	BMVW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	6.0		
W	BMVW-t	MT20	6.0	9.0	3.00	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		GROSS REACTION		INPUT	REQD
	VERT	HORZ	DOWN	UP		
W	3483	0	3483	299	-992	5-8
N	3327	0	3327	0	-934	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT W FOR 932 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 934 LBS FACTORED UPLIFT

PROVIDE FOR 299 LBS FACTORED HORIZONTAL REACTION AT JOINT W

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
W	2572	1528 / 0	452 / 0	0 / 0	76 / -894	582 / 0	0 / 0
N	2465	1437 / 0	452 / 0	0 / 0	113 / -943	576 / 0	0 / 0

HORIZONTAL REACTIONS

JT	W	N
W	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-U, F-S, G-S, H-S, H-R, I-R, I-P, C-W, L-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)
FR-TO	0 / 41	-115.2 -115.2 0.15 (2)	10.00	C-V -173 / 294 0.05 (3)
A-B	-8 / 137	-115.2 -115.2 0.37 (2)	10.00	V-D -145 / 403 0.08 (19)
B-C	-5099 / 1551	-115.2 -115.2 0.63 (2)	3.23	D-U -790 / 403 0.44 (2)
C-D	-4601 / 1435	-115.2 -115.2 0.58 (2)	3.39	U-F -301 / 984 0.16 (2)
D-E	-4601 / 1435	-115.2 -115.2 0.58 (2)	3.39	F-S -1446 / 823 0.67 (2)
E-F	-3734 / 1218	-115.2 -115.2 0.50 (2)	3.77	S-G -377 / 1253 0.23 (14)
F-G	-3932 / 1161	-115.2 -115.2 0.34 (1)	3.54	G-H -523 / 572 0.35 (3)
G-H	-3725 / 1215	-115.2 -115.2 0.49 (3)	3.77	H-R -476 / 1379 0.29 (14)
H-I	-4605 / 1433	-115.2 -115.2 0.57 (3)	3.39	R-I -1457 / 825 0.58 (3)
I-J	-4605 / 1433	-115.2 -115.2 0.57 (3)	3.39	I-P -305 / 999 0.16 (3)
J-K	-5097 / 1546	-115.2 -115.2 0.63 (3)	3.23	P-K -788 / 400 0.44 (3)
K-L	-10 / 97	-115.2 -115.2 0.37 (3)	6.25	K-O -140 / 398 0.08 (14)
L-M	-459 / 241	0.0 0.0 0.02 (2)	7.81	O-L -176 / 294 0.05 (2)
W-B	-277 / 164	0.0 0.0 0.01 (14)	7.81	L-N -5417 / 1408 0.83 (1)
N-M				
W-V	-1462 / 4538	-39.5 -39.5 0.67 (1)	8.25	
V-U	-1284 / 4428	-39.5 -39.5 0.61 (1)	8.25	
U-T	-1028 / 3953	-39.5 -39.5 0.56 (1)	8.25	
T-S	-1028 / 3953	-39.5 -39.5 0.56 (1)	8.25	
S-R	-864 / 3323	-39.5 -39.5 0.49 (1)	8.25	
R-Q	-819 / 3852	-39.5 -39.5 0.55 (1)	8.25	
Q-P	-819 / 3852	-39.5 -39.5 0.55 (1)	8.25	
P-O	-1018 / 4430	-39.5 -39.5 0.61 (1)	8.25	
O-N	-1193 / 4535	-39.5 -39.5 0.67 (1)	8.25	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 33.4 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 57.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2015
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPOC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 39.4 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/899 (0.24")
ALLOWABLE DEFL.(TL) = L/180 (2.87")
CALCULATED VERT. DEFL.(TL) = L/899 (0.34")

CSI: TC=0.63/1.00 (C-D-2), BC=0.67/1.00 (N-O-1), WB=0.68/1.00 (I-R-3), SSI=0.28/1.00 (B-C-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

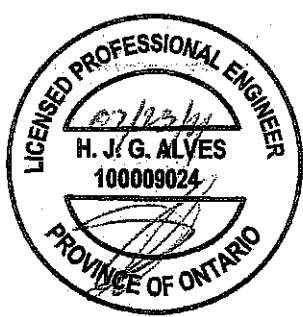
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.96 (C) (INPUT = 1.00)



Structural component only
DWG# T-2124038

JOB NAME 418363	TRUSS NAME T66	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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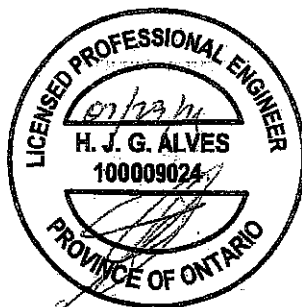
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

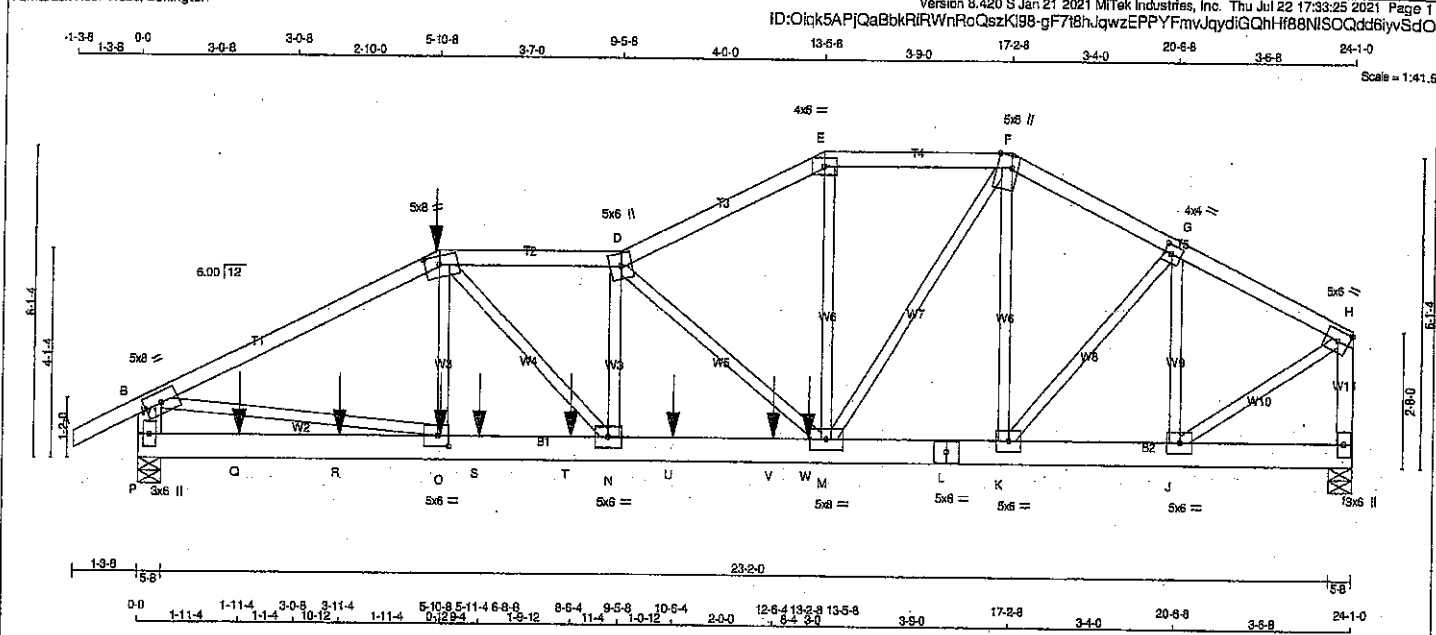
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF [9.6] PSF AT [30-0-0] FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.



Structural component only
DWG# T-2124038

JOB NAME 418363	TRUSS NAME T67	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG. NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 2 X 120 = 240 LB

LUMBER	CHORDS	SIZE	LUMBER	DESCR.	SPF
N. L. G. A. RULES					
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
P - B	2x6	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
P - L	2x8	DRY	No.2	SPF	
L - I	2x8	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12	SIDE (61.0)	
C-D 1 12	SIDE (61.0)	
D-E 1 12	TOP	
E-F 1 12	TOP	
F-H 1 12	TOP	
I-H 1 12	TOP	
P-B 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-L 2 12	SIDE (183.1)	
L-I 2 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
P	3410 0	3410 0	5-8	5-8
I	2924 0	2924 0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
P	2408 1626 / 0 0 / 0 0 / 0 777 / 0 0 / 0
I	2080 1396 / 0 0 / 0 0 / 0 664 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 28	-81.8	-81.8	0.07 (1)	10.00	B-O	0 / 4695	0.58 (1)
B-C	-5209 / 0	-81.8	-81.8	0.53 (1)	3.81	O-C	-169 / 83	0.02 (1)
C-D	-6244 / 0	-81.8	-81.8	0.25 (1)	3.72	C-N	0 / 2331	0.29 (1)
D-E	-4848 / 0	-81.8	-81.8	0.24 (1)	4.18	N-D	-768 / 0	0.10 (1)
E-F	-4369 / 0	-81.8	-81.8	0.19 (1)	4.41	D-M	-2604 / 0	0.58 (1)
F-G	-3447 / 0	-81.8	-81.8	0.13 (1)	4.95	M-E	0 / 1819	0.22 (1)
G-H	-2893 / 0	-81.8	-81.8	0.12 (1)	5.45	E-F	0 / 2452	0.30 (1)
P-B	-3832 / 0	0.0	0.0	0.12 (1)	7.61	F-F	-816 / 0	0.23 (1)
I-H	-2885 / 0	0.0	0.0	0.20 (1)	6.78	G-G	0 / 1008	0.12 (1)
P-Q	0 / 0	-18.5	-18.5	0.07 (4)	10.00	J-G	-1487 / 0	0.21 (1)
Q-R	0 / 0	-18.5	-18.5	0.07 (4)	10.00	J-H	0 / 2868	0.35 (1)
R-O	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
O-S	0 / 4651	-18.5	-18.5	0.38 (1)	10.00			
S-T	0 / 4651	-18.5	-18.5	0.38 (1)	10.00			
T-N	0 / 4651	-18.5	-18.5	0.38 (1)	10.00			
N-U	0 / 6265	-18.5	-18.5	0.81 (1)	10.00			
U-V	0 / 6265	-18.5	-18.5	0.81 (1)	10.00			
V-W	0 / 6265	-18.5	-18.5	0.81 (1)	10.00			
W-M	0 / 6265	-18.5	-18.5	0.81 (1)	10.00			
M-L	0 / 3061	-18.5	-18.5	0.33 (1)	10.00			
L-K	0 / 3061	-18.5	-18.5	0.33 (1)	10.00			
K-J	0 / 2413	-18.5	-18.5	0.18 (1)	10.00			
J-I	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	5-10-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
C	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
R	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	6-8-8	-236	-236	---	FRONT	VERT	TOTAL	---	C1
T	8-5-4	-252	-252	---	FRONT	VERT	TOTAL	---	C1
U	10-5-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
V	12-5-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
W	13-2-8	-1187	-1187	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

i) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 8 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.80")
CALCULATED VERT. DEFL. (LL) = L/959 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (0.80")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CSI: TC=0.53/1.00 (B-C:1), BC=0.61/1.00 (M-N:1), WB=0.68/1.00 (D-M:1), SSI=0.66/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION (PSI)	HEEL (PL)
MAX MIN MAX MIN MAX MIN		
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (D) (INPUT = 0.90)
JSI METAL=0.53 (O) (INPUT = 1.00)

Structural component only
DWG# T-2124039

CONTINUED ON PAGE 6

CONTINUED ON PAGE 2

JOB NAME 418363	TRUSS NAME T67	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:33:25 2021 Page 2 ID:Oigk5APIQaBbkRfRWnRoQszKf98-gF7i8hJgwzEPPYFmvJgydiGChH88NISOQdd6iyvSdO			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.50
D	TTWW+m	MT20	5.0	6.0		
E	TTW-l	MT20	4.0	6.0		
F	TTWW+m	MT20	5.0	8.0	Edge	3.50
G	TMWW-l	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	6.0		
J, K, N						
J	BMWW-t	MT20	5.0	6.0		
L	BS-l	MT20	5.0	6.0		
M	BMWWW-l	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

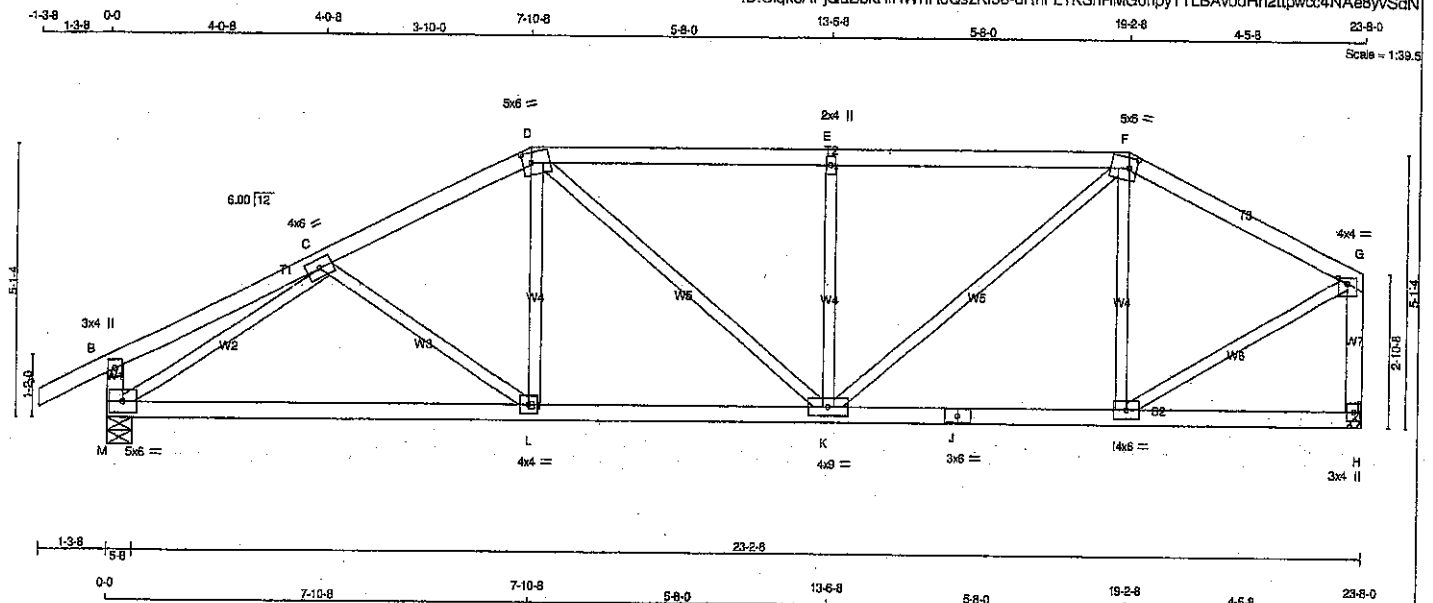


Structural component only
DWG# T-2124039 2/2

JOB NAME 418363	TRUSS NAME T68	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	5.0	6.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
M	1428 0	1428 0	5-8	5-8
H	1305 0	1305 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	1ST LOASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1008	675 / 0	0 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0
H	923	605 / 0	0 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-81.8 -81.8	0.12 (1)	C-L	-110 / 27	0.04 (1)	
B-C	0 / 17	-81.8 -81.8	0.21 (1)	L-D	0 / 215	0.05 (4)	
C-D	-1634 / 0	-81.8 -81.8	0.19 (1)	D-K	0 / 260	0.06 (1)	
D-E	-1648 / 0	-81.8 -81.8	0.41 (1)	K-E	-639 / 0	0.25 (1)	
E-F	-1648 / 0	-81.8 -81.8	0.41 (1)	K-F	0 / 858	0.19 (1)	
F-G	-1124 / 0	-81.8 -81.8	0.25 (1)	E-G	-454 / 0	0.17 (1)	
M-B	-268 / 0	0.0 0.0	0.03 (1)	M-C	-1898 / 0	0.72 (1)	
H-G	-1274 / 0	0.0 0.0	0.19 (1)	I-G	0 / 1153	0.28 (1)	
M-L	0 / 1534	-18.5 -18.5	0.38 (1)				
L-K	0 / 1449	-18.5 -18.5	0.38 (4)				
K-J	0 / 995	-18.5 -18.5	0.23 (1)				
J-I	0 / 995	-18.5 -18.5	0.23 (1)				
I-H	0 / 0	-18.5 -18.5	0.12 (4)				

TOTAL WEIGHT = 95 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 8.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TFC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.41/1.00 (D-E:1), BC=0.38/1.00 (L-M:1),
WB=0.72/1.00 (C-M:1), SS=0.25/1.00 (D-E:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	768	1987	1873

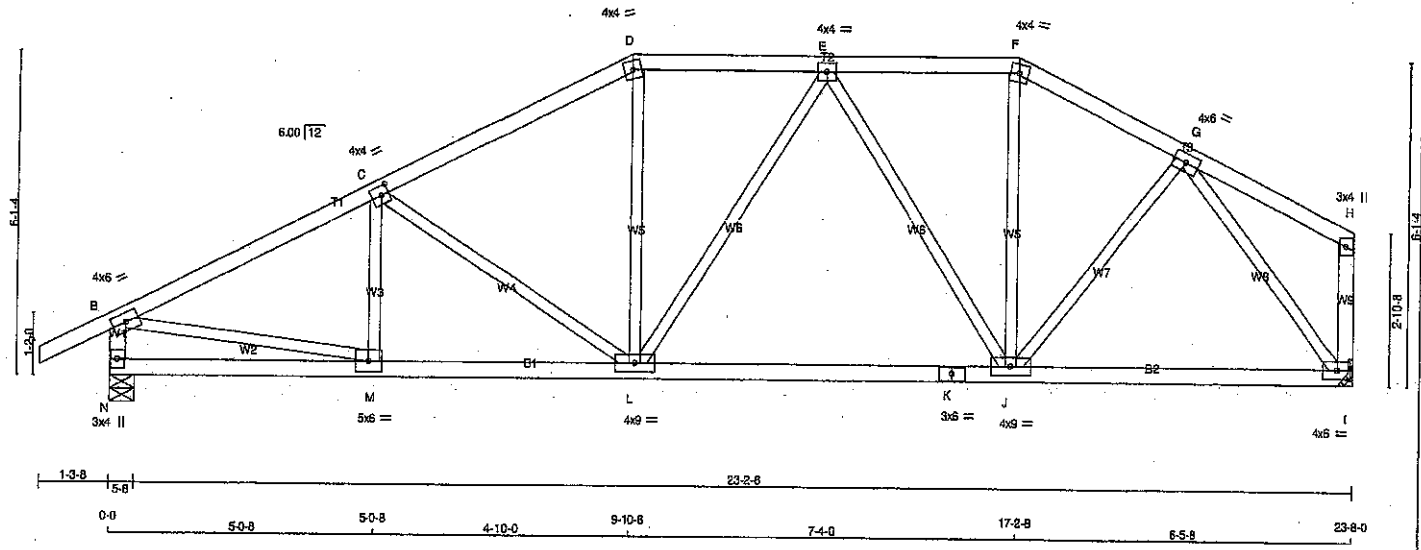
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



Structural component only
DWG# T-2124040



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECRD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	VERT	HORZ	GROSS REACTION	BRG	TOP CH. LL = 25.6 PSF	
D - F	2x4	DRY	No.2	SPF	JT	1429	0	1429	0	DL = 8.0 PSF	
F - H	2x4	DRY	No.2	SPF	N	1305	0	1305	0	BOT CH. LL = 0.0 PSF	
H - I	2x4	DRY	No.2	SPF	I					DL = 7.4 PSF	
I - K	2x4	DRY	No.2	SPF						TOTAL LOAD = 39.0 PSF	
K - L	2x4	DRY	No.2	SPF							
L - M	2x4	DRY	No.2	SPF							
M - N	2x4	DRY	No.2	SPF							
N - O	2x4	DRY	No.2	SPF							
O - P	2x4	DRY	No.2	SPF							
P - Q	2x4	DRY	No.2	SPF							
Q - R	2x4	DRY	No.2	SPF							
R - S	2x4	DRY	No.2	SPF							
S - T	2x4	DRY	No.2	SPF							
T - U	2x4	DRY	No.2	SPF							
U - V	2x4	DRY	No.2	SPF							
V - W	2x4	DRY	No.2	SPF							
W - X	2x4	DRY	No.2	SPF							
X - Y	2x4	DRY	No.2	SPF							
Y - Z	2x4	DRY	No.2	SPF							
Z - A	2x4	DRY	No.2	SPF							
A - B	2x4	DRY	No.2	SPF							
B - C	2x4	DRY	No.2	SPF							
C - D	2x4	DRY	No.2	SPF							
D - E	2x4	DRY	No.2	SPF							
E - F	2x4	DRY	No.2	SPF							
F - G	2x4	DRY	No.2	SPF							
G - H	2x4	DRY	No.2	SPF							
H - I	2x4	DRY	No.2	SPF							
I - J	2x4	DRY	No.2	SPF							
J - K	2x4	DRY	No.2	SPF							
K - L	2x4	DRY	No.2	SPF							
L - M	2x4	DRY	No.2	SPF							
M - N	2x4	DRY	No.2	SPF							
N - O	2x4	DRY	No.2	SPF							
O - P	2x4	DRY	No.2	SPF							
P - Q	2x4	DRY	No.2	SPF							
Q - R	2x4	DRY	No.2	SPF							
R - S	2x4	DRY	No.2	SPF							
S - T	2x4	DRY	No.2	SPF							
T - U	2x4	DRY	No.2	SPF							
U - V	2x4	DRY	No.2	SPF							
V - W	2x4	DRY	No.2	SPF							
W - X	2x4	DRY	No.2	SPF							
X - Y	2x4	DRY	No.2	SPF							
Y - Z	2x4	DRY	No.2	SPF							
Z - A	2x4	DRY	No.2	SPF							

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY, SEASONED LUMBER.
 TOTAL WEIGHT = 100 lb

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING			
JT TYPE	PLATES	W	LEN Y X	1ST CASE	MAX MIN COMPONENT REACTIONS						
B	TMW-t	MT20	4.0 5.0	JT COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	
C	TMW-w	MT20	4.0 4.0 2.00 1.75	N	1008	675 / 0	0 / 0	0 / 0	333 / 0	0 / 0	
D	TTW-m	MT20	4.0 4.0	I	923	605 / 0	0 / 0	0 / 0	317 / 0	0 / 0	
E	TMW-w	MT20	4.0 4.0								
F	TTW-m	MT20	4.0 4.0								
G	TMW-w	MT20	4.0 4.0								
H	TMW-p	MT20	3.0 4.0								
I	BMW-w	MT20	4.0 6.0								
J	BMW-w	MT20	4.0 9.0								
K	BS-t	MT20	3.0 6.0								
L	BMW-w	MT20	4.0 9.0								
M	BMW-w	MT20	5.0 6.0								
N	BMW-p	MT20	3.0 4.0								

NOTES: (1)
 1) Lateral braces to be a minimum of 2x4 SPF #2.
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N
 BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD
FR-TO	FROM	TO	CS (LC)	FR-TO	FROM	TO	CS (LC)
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	M-C	-185 / 14	0.04 (1)
B-C	-1783 / 0	-91.8	-91.8 0.32 (1)	4.69	C-L	-384 / 0	0.24 (1)
C-D	-1477 / 0	-91.8	-91.8 0.30 (1)	5.07	L-D	0 / 351	0.08 (1)
D-E	-1305 / 0	-91.8	-91.8 0.17 (1)	5.49	L-E	-23 / 26	0.02 (1)
E-F	-1079 / 0	-91.8	-91.8 0.16 (1)	5.90	E-J	-455 / 0	0.40 (1)
F-G	-1216 / 0	-91.8	-91.8 0.12 (1)	5.69	J-F	0 / 292	0.07 (1)
G-H	0 / 14	-91.8	-91.8 0.14 (1)	10.00	J-G	0 / 307	0.07 (1)
H-I	-1387 / 0	0.0	0.0 0.14 (1)	6.91	B-M	0 / 1838	0.37 (1)
I-H	-117 / 0	0.0	0.0 0.02 (1)	7.81	G-I	-1442 / 0	0.64 (1)
N-M	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
M-L	0 / 1615	-18.5	-18.5 0.34 (1)	10.00			
L-K	0 / 1317	-18.5	-18.5 0.33 (1)	10.00			
K-J	0 / 1317	-18.5	-18.5 0.33 (1)	10.00			
J-I	0 / 882	-18.5	-18.5 0.29 (4)	10.00			

ALLOWABLE DEFL.(LL) = L/360 (0.79")
 CALCULATED VERT. DEFL.(LL) = 1/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.79")
 CALCULATED VERT. DEFL.(TL) = 1/999 (0.11")
 CSI: TC=0.32/1.00 (B-C:1), BC=0.34/1.00 (L-M:1),
 WB=0.64/1.00 (G-I:1), SS=0.20/1.00 (B-C:1)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 850 371 1747 788 1587 1873
 PLATE PLACEMENT TOL = 0.250 inches
 PLATE ROTATION TOL = 5.0 Deg.
 JSI GRIP= 0.86 (I) (INPUT = 0.90)
 JSI METAL= 0.44 (B) (INPUT = 1.00)

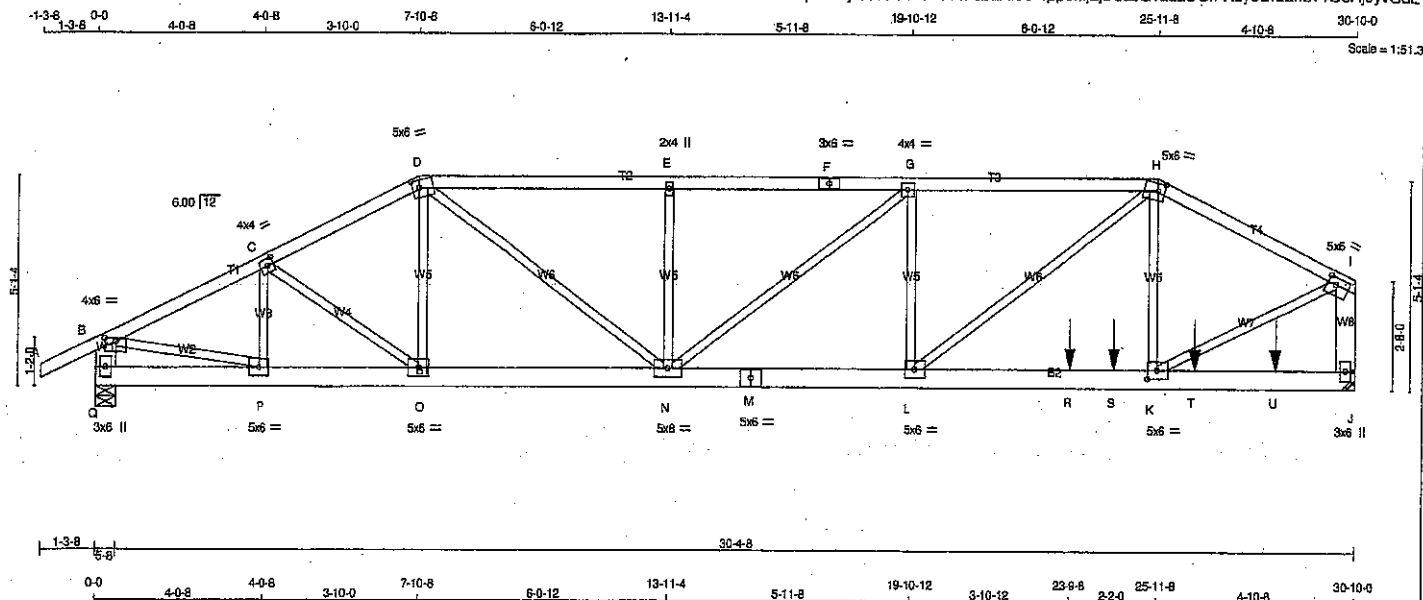


Structural component only
 DWG# T-2124041

JOB NAME 418363	TRUSS NAME T70	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
H - I	2x4	DRY	No.2 SPF
Q - B	2x6	DRY	No.2 SPF
J - I	2x6	DRY	No.2 SPF
Q - M	2x6	DRY	No.2 SPF
M - J	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D	12	TOP
D - F	12	TOP
F - H	12	TOP
H - I	12	TOP
Q - B	12	TOP
J - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - M	12	TOP
M - J	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	8

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
Q	2271	0	2271	0	0	5-8	5-8		
J	3540	0	3540	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1802	1073 / 0	0 / 0	0 / 0	0 / 0	529 / 0	0 / 0
J	2497	1679 / 0	0 / 0	0 / 0	0 / 0	818 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MAX. FACTORED (LBS)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)
FR-TO					FR-TO		
A - B	0 / 28	-91.8	-91.8 0.07 (1)	10.00	P - C	-565 / 0	0.05 (1)
B - C	-3100 / 0	-91.8	-91.8 0.15 (1)	5.12	Q - D	0 / 116	0.01 (1)
C - D	-3242 / 0	-91.8	-91.8 0.15 (1)	5.03	O - D	0 / 136	0.02 (4)
D - E	-4092 / 0	-91.8	-91.8 0.36 (1)	4.38	K - H	0 / 953	0.04 (1)
E - F	-4092 / 0	-91.8	-91.8 0.36 (1)	4.38	B - P	0 / 2938	0.35 (1)
F - G	-4092 / 0	-91.8	-91.8 0.36 (1)	4.38	K - I	0 / 3759	0.47 (1)
G - H	-4590 / 0	-91.8	-91.8 0.39 (1)	4.16	L - H	0 / 1474	0.18 (1)
H - I	-3825 / 0	-91.8	-91.8 0.29 (1)	4.56	D - N	0 / 1531	0.19 (1)
Q - B	-2202 / 0	0.0	0.0 0.08 (1)	7.81	L - G	-201 / 12	0.04 (1)
J - I	-3501 / 0	0.0	0.0 0.16 (1)	7.48	N - E	-598 / 0	0.11 (1)
					N - G	-635 / 0	0.36 (1)
Q - P	0 / 0	-18.5	-18.5 0.05 (1)	10.00			
P - O	0 / 2787	-18.5	-18.5 0.23 (1)	10.00			
O - N	0 / 2885	-18.5	-18.5 0.25 (1)	10.00			
N - M	0 / 4580	-18.5	-18.5 0.49 (1)	10.00			
M - L	0 / 4580	-18.5	-18.5 0.49 (1)	10.00			
L - R	0 / 3427	-18.5	-18.5 0.75 (1)	10.00			
R - S	0 / 3427	-18.5	-18.5 0.75 (1)	10.00			
S - K	0 / 3427	-18.5	-18.5 0.75 (1)	10.00			
K - T	0 / 0	-18.5	-18.5 0.39 (1)	10.00			
T - U	0 / 0	-18.5	-18.5 0.39 (1)	10.00			
U - J	0 / 0	-18.5	-18.5 0.39 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	23-8-8	-1083	-1083		FRONT	VERT	TOTAL		C1
S	24-10-12	-175	-175		FRONT	VERT	TOTAL		C1
T	26-10-12	-175	-175		FRONT	VERT	TOTAL		C1
U	28-10-12	-175	-175		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/899 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.39/1.00 (G-H-I), BC=0.75/1.00 (K-L-I), WB=0.47/1.00 (H-K-I), SS=0.45/1.00 (K-L-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 758
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.45 (I) (INPUT = 1.00)



Structural component only
DWG# T-2124042

CONTINUED ON PAGE 2

JOB NAME 418363	TRUSS NAME T70	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

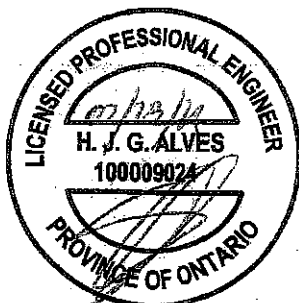
Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Thu Jul 22 17:33:28 2021 Page 2
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PLATES (table is in inches)

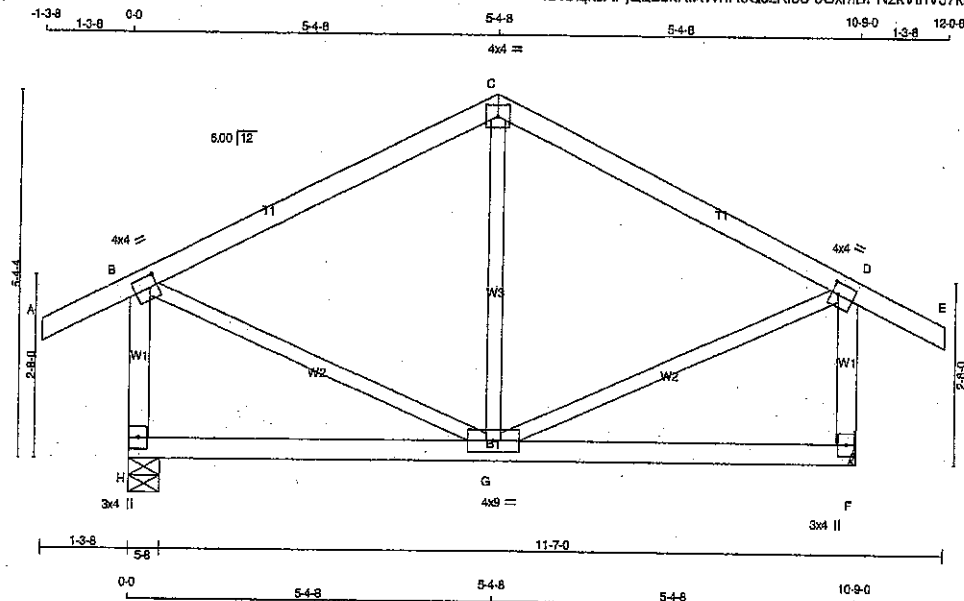
JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.00
I	TMW-t	MT20	5.0	6.0	2.00	2.25
J	BMV1-p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L, O, P						
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
Q	BMV1-p	MT20	3.0	6.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2124042



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY
C - E	2x4	DRY
H - B	2x4	DRY
F - D	2x4	DRY
H - F	2x4	DRY

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-1	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-1	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
H	717	0	0	5-8
F	717	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	505	344 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0
F	505	344 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	G-C	-165 / 32	0.07 (1)
B-C	-367 / 0	-91.8 -91.8	0.34 (1)	8.25	B-G	0 / 358	0.08 (1)
C-D	-367 / 0	-91.8 -91.8	0.34 (1)	6.25	G-D	0 / 358	0.08 (1)
D-E	0 / 28	-91.8 -91.8	0.12 (1)	10.00			
H-B	-679 / 0	0.0 0.0	0.09 (1)	7.81			
F-D	-679 / 0	0.0 0.0	0.09 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.15 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

TOTAL WEIGHT = 2 X 47 = 95 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL		= 39.0	PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.34/1.00 (B-C:1), BC=0.15/1.00 (F-G:4), WB=0.09/1.00 (B-G:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(FSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.53 (B) (INPUT = 0.90)
JSI METAL = 0.18 (B) (INPUT = 1.00)

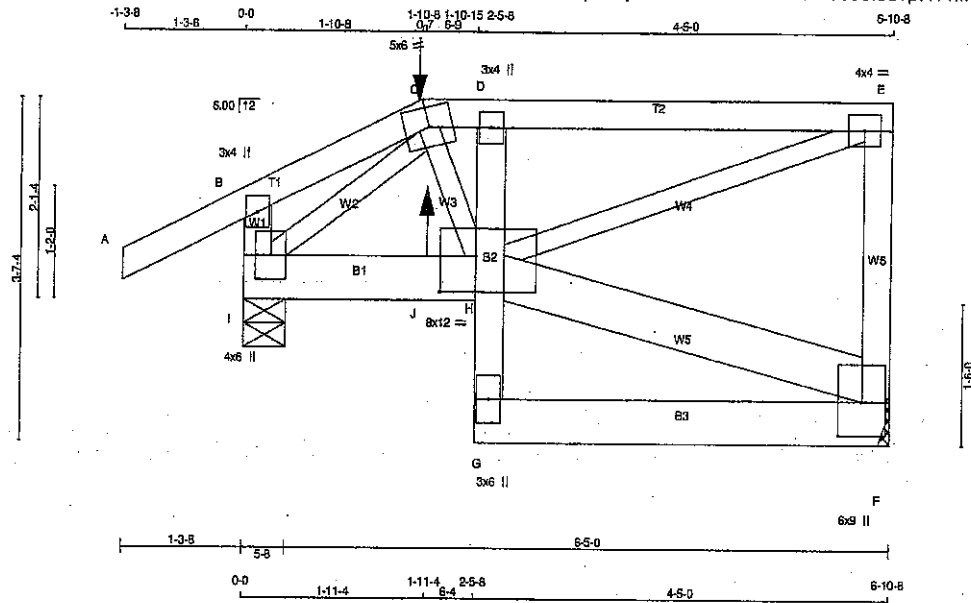


Structural component only
DWG# T-2124044

JOB NAME 418363	TRUSS NAME T73S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Jul 22 17:33:31 2021 Page 1
ID:Oiqk5APjQaBbkRIRWnRcCszkI98-U0U8OIObVp7Y7TiwGaxMtzWWkCYEhLmM4xJLlvScI



TOTAL WEIGHT = 43 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
J - H	2x6	DRY	No.2
G - D	2x4	DRY	No.2
G - F	2x8	DRY	No.2
ALL WEBS EXCEPT H - F	2x3	DRY	No.2
	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	BMVW1+p	MT20	6.0	9.0	4.25	3.00
G	BMV+p	MT20	3.0	6.0		
H	BMVWWW-1	MT20	8.0	12.0	4.50	4.50
I	BMVW1+p	MT20	4.0	6.0		

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	378	0	378	0
I	500	0	500	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	COMBINED	287	175 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
I	COMBINED	351	242 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.13 (1)	C-H	0 / 428	0.11 (5)	
B-C	0 / 0	-91.8	-91.8 0.06 (1)	H-F	-15 / 0	0.00 (1)	
C-D	-440 / 0	-91.8	-91.8 0.24 (5)	H-E	0 / 462	0.11 (1)	
D-E	-429 / 0	-91.8	-91.8 0.24 (5)	I-C	-395 / 0	0.07 (1)	
F-E	-333 / 0	0.0	0.0 0.07 (1)				
I-B	-212 / 0	0.0	0.0 0.02 (1)				
I-J	0 / 293	-18.5	-18.5 0.05 (1)				
J-H	0 / 293	-18.5	-18.5 0.05 (1)				
G-H	0 / 44	0.0	0.0 0.04 (1)				
H-D	-489 / 0	0.0	0.0 0.03 (1)				
G-F	0 / 14	-18.5	-18.5 0.05 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-15	-1	-1	67	BACK	VERT	TOTAL	---	C1
J	1-11-4	4	1	8	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL =	6.0	PSF
BOT CH.	LL = 0.0	PSF
DL =	7.4	PSF
TOTAL LOAD =	39.0	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.24/1.00 (D-E), BC=0.05/1.00 (F-G), WS=0.11/1.00 (E-H), SS=0.22/1.00 (C-D-S)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

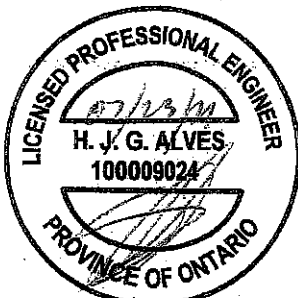
NAIL VALUES

PLATE GRIP(DRY)	SHEAR SECTION (PSI)	(PL)	(PL)
MT20	650	371	1747
	MAX MIN	MAX MIN	MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

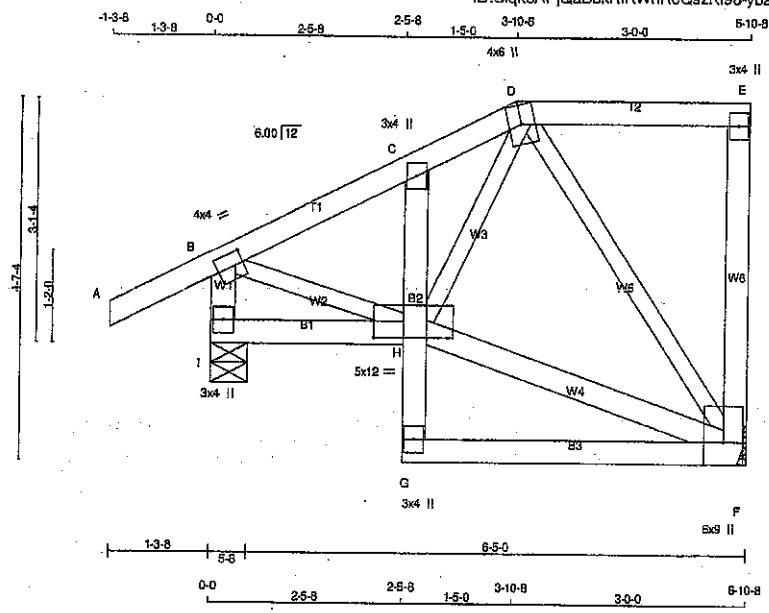
JSI GRIP= 0.54 (E) (INPUT = 0.90)
JSI METAL= 0.15 (E) (INPUT = 1.00)



Structural component only
DWG# T-2124045

JOB NAME 418363	TRUSS NAME T74S	QUANTITY 1	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Qik5APjQaBbkRIRWnRoCsziKl98-yb2Xc4ODG77PldH6pHSbPA2i36AkHhJL70qVsoyvSdH



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT H - F	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	4.0	4.0	2.00	1.25
C TMV+p	MT20	3.0	4.0		
D TMVW+m	MT20	4.0	5.0		
E TMV+p	MT20	3.0	4.0		
F BMVWW-I+p	MT20	6.0	9.0	Edge	
G BMV+p	MT20	3.0	4.0		
H BMVWW-I	MT20	6.0	12.0	2.50	4.50
I BMV-I+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT. HORZ	DOWN HORZ	UPLIFT	IN-SX
F	379 0	379 0	0	MECHANICAL
I	503 0	503 0	0	5-8 5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	268	176 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
I	354	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF#NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	H-F	0 / 181	0.03 (1)
B-C	-335 / 0	-91.8 -91.8	0.07 (1)	8.25	H-D	0 / 265	0.06 (1)
C-D	-340 / 0	-91.8 -91.8	0.04 (1)	8.25	B-H	0 / 323	0.07 (1)
D-E	0 / 0	-91.8 -91.8	0.14 (1)	10.00	D-F	-320 / 0	0.13 (1)
E-F	-136 / 0	0.0 0.0	0.05 (1)	7.81			
I-B	-479 / 0	0.0 0.0	0.05 (1)	7.81			
I-H	0 / 0	-18.5 -18.5	0.04 (4)	10.00			
G-H	0 / 44	0.0 0.0	0.03 (1)	10.00			
H-C	-218 / 0	0.0 0.0	0.02 (1)	7.81			
G-F	0 / 10	-18.5 -18.5	0.10 (4)	10.00			

TOTAL WEIGHT = 40 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.14/1.00 (D-E:1), BC=0.10/1.00 (F-G:4), WB=0.13/1.00 (D-F:1), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PL)	(PL)	(PL)
MAX MIN MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

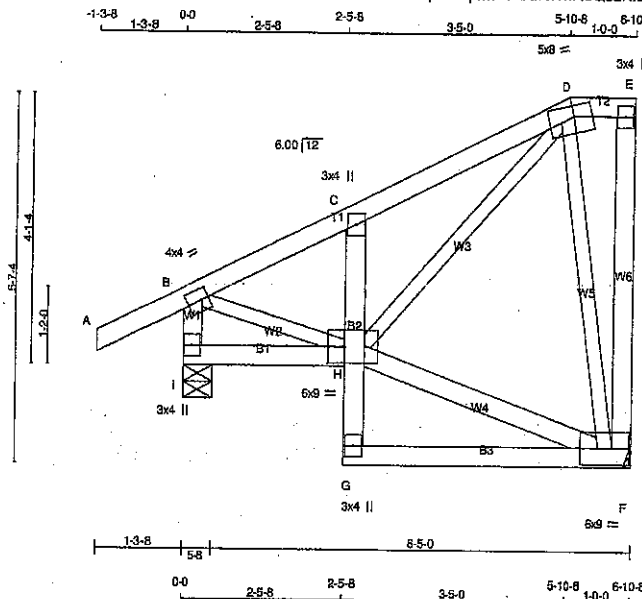


Structural component only
DWG# T-2124046

JOB NAME 418363	TRUSS NAME T75S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - E	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
I - B	2x4 DRY	No.2	SPF
I - H	2x4 DRY	No.2	SPF
G - C	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	TTWV-m	MT20	5.0	8.0	2.00 3.25
E	TMV+p	MT20	3.0	4.0	
F	BMVWW-I	MT20	6.0	9.0	
G	BMV+p	MT20	3.0	4.0	
H	BMVWW-I	MT20	6.0	9.0	3.00 3.00
I	BMV+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	HORZ	UPLIFT	IN-SX
F	379	0	379	0	MECHANICAL
I	503	0	503	0	5-8 5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LOASE	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
JT							
F	288	178 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
I	354	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LO1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LO1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	D-F	-315 / 0	0.15 (1)	
B-C	-354 / 0	-91.8	-91.8 0.10 (1)	H-F	0 / 46	0.01 (1)	
C-D	-351 / 0	-91.8	-91.8 0.11 (1)	H-D	0 / 390	0.09 (1)	
D-E	0 / 0	-91.8	-91.8 0.01 (1)	B-H	0 / 348	0.08 (1)	
F-E	-44 / 0	0.0	0.0 0.03 (1)				
I-B	-479 / 0	0.0	0.0 0.05 (1)				
I-H	0 / 0	-18.5	-18.5 0.04 (4)				
G-H	0 / 44	0.0	0.0 0.03 (1)				
H-C	-325 / 0	0.0	0.0 0.02 (1)				
G-F	0 / 13	-18.5	-18.5 0.10 (4)				

TOTAL WEIGHT = 43 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/969 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (F-G:4), WB=0.15/1.00 (D-F:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

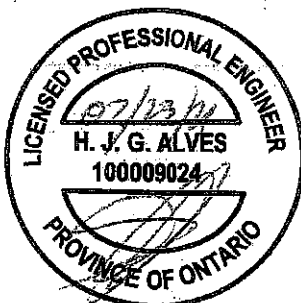
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION	(PSI)	(PLI)	(PLI)
MAX MIN MAX MIN MAX MIN			
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.54 (B) (INPUT = 0.90)
JSI METAL = 0.16 (B) (INPUT = 1.00)

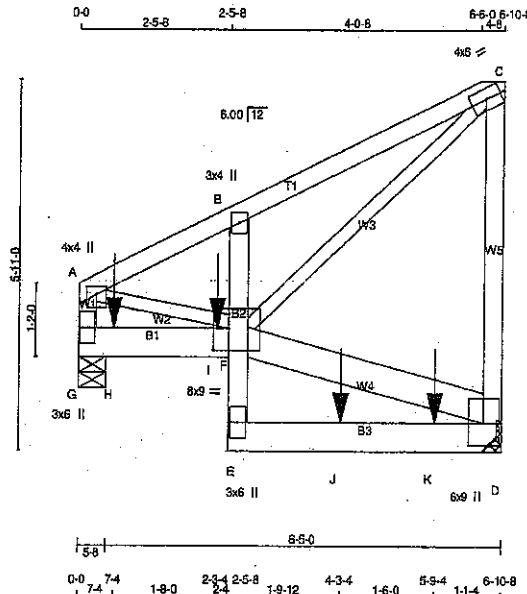


Structural component only
DWG# T-2124047

JOB NAME 418363	TRUSS NAME T76S	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:33:33 2021 Page 1
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Scale = 1:33.7

LUMBER	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS	A - C	2x4 DRY	No.2	SPF
	D - C	2x4 DRY	No.2	SPF
	G - A	2x4 DRY	No.2	SPF
	G - F	2x6 DRY	No.2	SPF
	E - B	2x4 DRY	No.2	SPF
	E - D	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF	
EXCEPT	F - D	2x8 DRY	No.2	SPF

DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-D	12	TOP
G-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G-F	12	SIDE(0.0)
E-D	12	SIDE(0.0)
B-E	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1710	0	1710	0
G	1576	0	1576	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
D	1202 830/0 0/0 0/0 0/0 372/0 0/0
G	1106 771/0 0/0 0/0 0/0 335/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD	MAX. LO1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO	(PL)			FR-TO		
A-B	-1617/0	-91.8	-91.8	0.09 (1)	8.25	F-D	-69/0	0.01 (1)
B-C	-1580/0	-91.8	-91.8	0.14 (1)	8.25	F-C	0/1971	0.24 (1)
D-C	-857/0	0.0	0.0	0.29 (1)	7.81	A-F	0/1526	0.19 (1)
G-A	-1241/0	0.0	0.0	0.07 (1)	7.81			
G-H	0/0	-18.5	-18.5	0.10 (1)	10.00			
H-I	0/0	-18.5	-18.5	0.10 (1)	10.00			
I-F	0/0	-18.5	-18.5	0.10 (1)	10.00			
E-F	0/848	0.0	0.0	0.13 (1)	10.00			
F-B	-353/0	0.0	0.0	0.07 (1)	7.81			
E-J	0/65	-18.5	-18.5	0.39 (1)	10.00			
J-K	0/65	-18.5	-18.5	0.39 (1)	10.00			
K-D	0/65	-18.5	-18.5	0.39 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	7-4	-179	-179	---	TOP	VERT	TOTAL	---	C1
I	2-3-4	-613	-613	---	FRONT	VERT	TOTAL	---	C1
J	4-3-4	-490	-490	---	FRONT	VERT	TOTAL	---	C1
K	5-9-4	-490	-490	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 45 = 90 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.23")
CALCULATED VERT. DEFL.(LL) = 1/899 (0.04")
ALLOWABLE DEFL.(TL) = 1/360 (0.23")
CALCULATED VERT. DEFL.(TL) = 1/899 (0.06")

CSI: TC=0.29/1.00 (C-D:1), BC=0.39/1.00 (D-E:1), WB=0.24/1.00 (C-F:1), SS=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

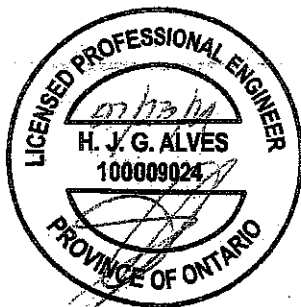
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX	MIN	MAX MIN
MT20	650 371	1747 788 1997 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)



Structural component only
DWG# T-2124048

CONTINUED ON PAGE 2

JOB NAME 418363	TRUSS NAME T76S	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

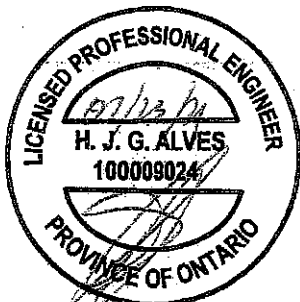
Version 8.420 © Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:33:33 2021 Page 2
ID:Oik5APIQaBbkRIRWnRoQszKIS8-RncvQPr1QFGMnsiN?zavObrVVRQ069eDqZ?OEwSdG

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	3.00
D	BMVW-t+p	MT20	5.0	9.0	4.25	3.00
E	BMV+p	MT20	3.0	6.0		
F	BVMWW-t	MT20	8.0	9.0	4.25	3.00
G	BMV-t+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

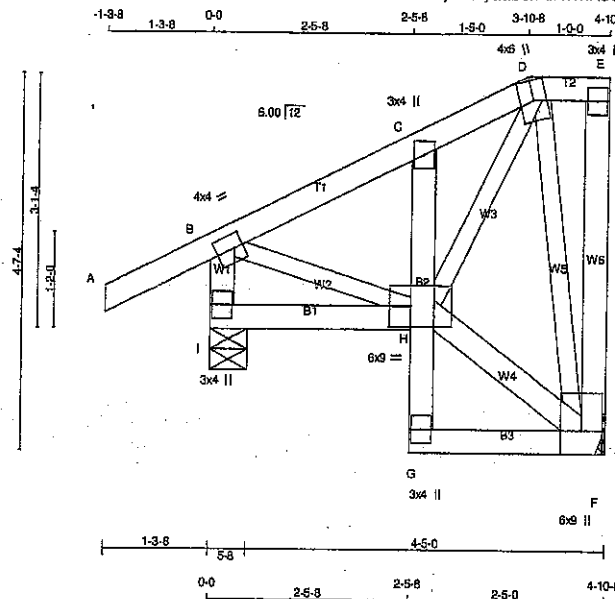


Structural component only
DWG# T-2124048 *TH*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418363	T77S	1	1	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:33:34 2021 Page 1
ID:Oiqk5APjQaBbkRiRWnRoQszKI98-vzAH1mQUokN7_wRVxiV3Vb83vtBib7nSKJbwgyvSdF



Scale = 1:25.0

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
G - C	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
H - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1+p	MT20	6.0	9.0	Edge	
G	BMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	6.0	9.0	3.00	3.00
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	289	0	289	0
I	393	0	393	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	125 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
I	276	194 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 28	-91.8 -91.8 0.12 (1)	H-F	0 / 53
B-C	-188 / 0	-91.8 -91.8 0.07 (1)	H-D	0 / 263
C-D	-188 / 0	-91.8 -91.8 0.06 (1)	B-H	0 / 184
D-E	0 / 0	-91.8 -91.8 0.01 (1)	D-F	-238 / 0
F-E	-44 / 0	0.0 0.0 0.02 (1)		
I-B	-369 / 0	0.0 0.0 0.04 (1)		
I-H	0 / 0	-18.5 -18.5 0.04 (4)		
G-H	0 / 24	0.0 0.0 0.02 (1)		
H-C	-222 / 0	0.0 0.0 0.02 (1)		
G-F	0 / 8	-18.5 -18.5 0.03 (4)		

TOTAL WEIGHT = 33 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (H-I:4),
WB=0.07/1.00 (D-F:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

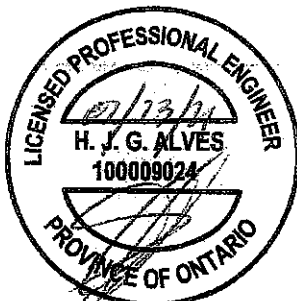
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.30)
JSI METAL= 0.10 (B) (INPUT = 1.00)

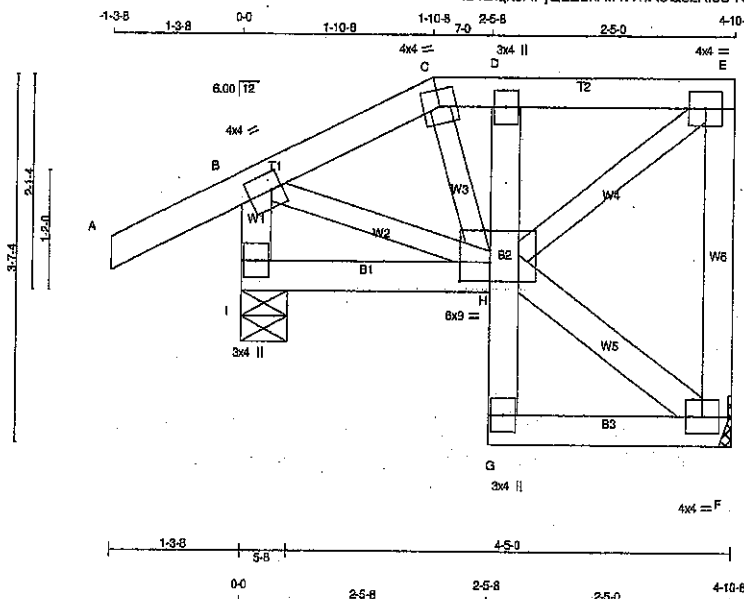


Structural component only
DWG# T-2124049

JOB NAME 418363	TRUSS NAME T78S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Thu Jul 22 17:33:35 2021 Page 1
ID:OicK5APjQaBbkRIRWnRoQszKl98-NAKIE6R6Z2V_c4?hVQ0i1pgEBJDQU3bXnz29S8yvSdE



Scale = 1:20.7

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
O - P	2x4	DRY	No.2	SPF	
Q - R	2x4	DRY	No.2	SPF	
S - T	2x4	DRY	No.2	SPF	
U - V	2x4	DRY	No.2	SPF	
W - X	2x4	DRY	No.2	SPF	
Y - Z	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
H - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-1	MT20	3.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	BMVW-1	MT20	4.0	4.0		
G	BMVW-1	MT20	3.0	4.0		
H	BMVW-1	MT20	6.0	9.0	2.25	3.50
I	BMVW-1	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	VERT	DOWN	IN-SX	IN-SX
289	0	289	0	5-8
393	0	393	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	190	125 / 0	0 / 0	0 / 0	65 / 0	0 / 0
I	276	194 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.
FR-TO		FORCE (LBS)	FROM	TO	(PLF)	CSI (LC)	UNBRAC	FR-TO		FORCE (LBS)	FROM	TO	(PLF)	CSI (LC)
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C-H	0 / 25	0.01 (1)						
B-C	-212 / 0	-91.8	-91.8	0.08 (1)	6.25	H-F	-7 / 0	0.00 (1)						
C-D	-198 / 0	-91.8	-91.8	0.05 (1)	6.25	H-E	0 / 248	0.06 (1)						
D-E	-194 / 0	-91.8	-91.8	0.03 (1)	6.25	B-H	0 / 199	0.04 (1)						
E-F	-244 / 0	0.0	0.0	0.05 (1)	7.81									
F-G	-369 / 0	0.0	0.0	0.04 (1)	7.81									
I-H	0 / 0	-18.5	-18.5	0.04 (4)	10.00									
G-H	0 / 24	0.0	0.0	0.01 (1)	10.00									
H-D	-198 / 0	0.0	0.0	0.01 (1)	7.81									
G-F	0 / 5	-18.5	-18.5	0.03 (4)	10.00									

TOTAL WEIGHT = 29 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (H-I:4), WB=0.08/1.00 (E-H:1), SS=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	850	371	1747
	788	1937	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.31 (8) (INPUT = 0.90)
JSI METAL = 0.10 (8) (INPUT = 1.00)

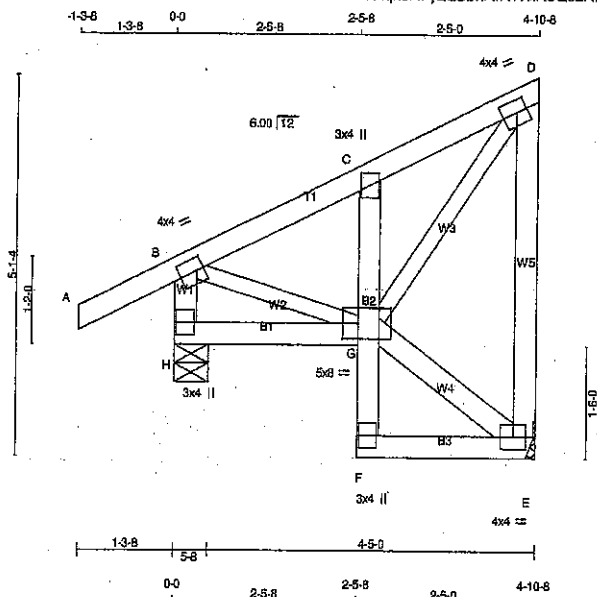


Structural component only
DWG# T-2124050

JOB NAME 418363	TRUSS NAME T79S	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.420 S Jan 21 2021 M Tek Industries, Inc. Thu Jul 22 17:33:36 2021 Page 1
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Scale = 1:28.3

CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
F - C	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
ALL WEBS EXCEPT G - E	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	BMVW-t	MT20	4.0	4.0		
F	BMVW-p	MT20	3.0	4.0		
G	BMVW-t	MT20	5.0	8.0	2.50	2.50
H	BMVW-p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
H	389	389	0	5-8
E	258	258	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	276	194 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
E	190	125 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. HORIZ. LOAD (LBS)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. HORIZ. LOAD (LBS)
FR-TO		FROM	TO			FR-TO		
H-B	-389 / 0	0.0	0.0	0.04 (1)	7.81	B-G	0 / 192	0.04 (1)
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	G-E	-10 / 0	0.00 (1)
B-C	-198 / 0	-91.8	-91.8	0.08 (1)	6.25	G-D	0 / 296	0.07 (1)
C-D	-199 / 0	-91.8	-91.8	0.08 (1)	6.25			
E-D	-242 / 0	0.0	0.0	0.10 (1)	7.81			
H-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
F-G	0 / 24	0.0	0.0	0.02 (1)	10.00			
G-C	-265 / 0	0.0	0.0	0.02 (1)	7.81			
F-E	0 / 9	-18.5	-18.5	0.03 (4)	10.00			

TOTAL WEIGHT = 31 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 8 OF OSC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), SC=0.04/1.00 (G-H:4), WB=0.07/1.00 (D-G:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

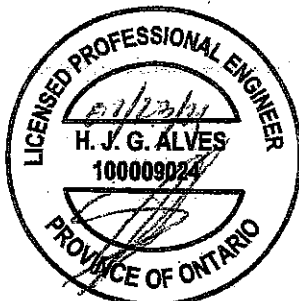
NAIL VALUES

PLATE: GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

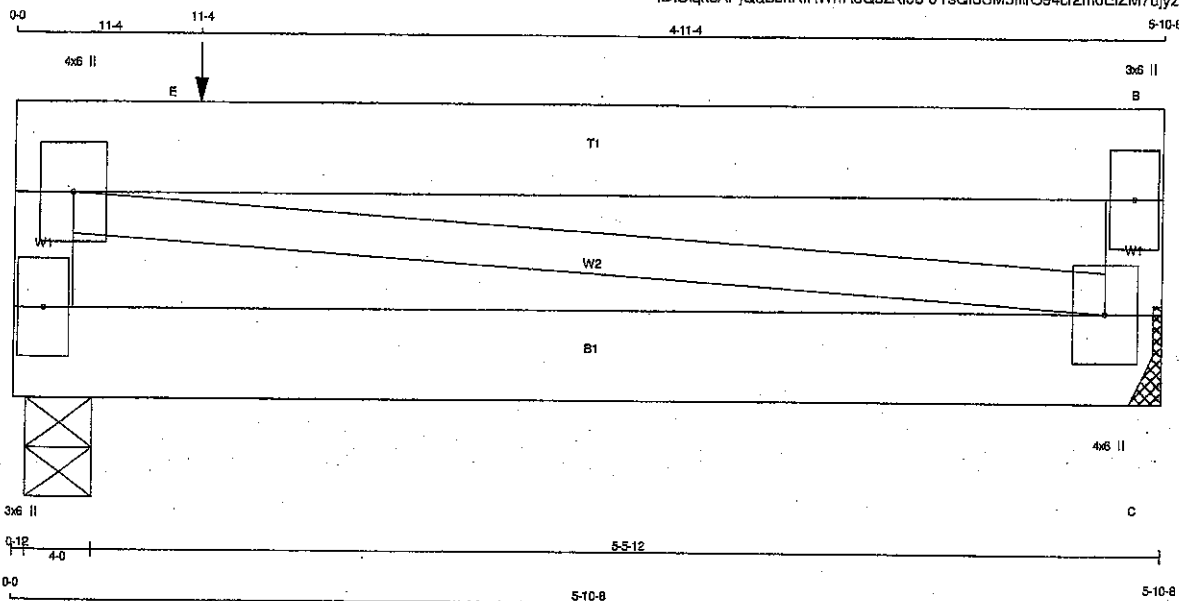
JSI GRIP = 0.34 (G) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124051

JOB NAME 418363	TRUSS NAME T80	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Oiqk5AP/QaBbkRIRWnRoQszK198-JYsCfoSM5firO94cr2m6EIZM7u/jzyD8HXFX?yvSdC



Scale = 1:10.7

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2
A - B	2x6	DRY	No.2
C - B	2x4	DRY	No.2
D - C	2x6	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D - A 1	12	TOP
B - C 1	12	TOP
A - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D - C 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	
B	TMV+p	MT20	3.0	6.0	
C	BMVW1+p	MT20	4.0	6.0	
D	BMV1+p	MT20	3.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
D	490 0	490 0	4-0
C	355 0	355 0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 2-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	345	235 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
C	251	168 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LO1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LO1 MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
D - A	-436 / 0	0.0	0.0 0.02 (1)	A - C	0 / 0	0.00 (1)	
A - E	0 / 0	-91.8	-91.8 0.17 (1)				
E - B	0 / 0	-91.8	-91.8 0.17 (1)				
C - B	-301 / 0	0.0	0.0 0.02 (1)				
D - C	0 / 0	-18.5	-18.5 0.05 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LO1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	11-4	-138	-138	---	TOP	VERT	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 "PSF"
BOT CH. LL = 0.0 PSF
DL = 7.4 "PSF"
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17/1.00 (A-B:1), BC=0.05/1.00 (C-D:4), WB=0.00/1.00 (A-C:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.05 (A) (INPUT = 0.90)
JSI METAL = 0.03 (D) (INPUT = 1.00)



Structural component only
DWG# T-2124052

JOB NAME 418363	TRUSS NAME T81	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitak Industries, Inc. Fri Jul 23 09:32:55 2021 Page 2 ID:9i87gKQZ71H6SRnQId2DVSzKFyO-1mkip0T6MSu8pLQ02gKv2eoQ5B?uRZGbAGzisyvEZs	

PLATES (table is in inches) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>TMVW-p</td> <td>MT20</td> <td>5.0</td> <td>6.0</td> <td>2.00</td> <td>2.75</td> </tr> <tr> <td>B</td> <td>TTWW-m</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td>2.00</td> <td>3.00</td> </tr> <tr> <td>C</td> <td>TMVW-w</td> <td>MT20</td> <td>2.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>D</td> <td>TMVW-t</td> <td>MT20</td> <td>5.0</td> <td>8.0</td> <td>2.00</td> <td>2.75</td> </tr> <tr> <td>E</td> <td>BMV1-t</td> <td>MT20</td> <td>4.0</td> <td>9.0</td> <td>Edge</td> <td>1.50</td> </tr> <tr> <td>F</td> <td>BMVWW-t</td> <td>MT20</td> <td>7.0</td> <td>12.0</td> <td>4.25</td> <td>3.75</td> </tr> <tr> <td>G</td> <td>BMVWW-t</td> <td>MT20</td> <td>6.0</td> <td>9.0</td> <td>4.50</td> <td>2.25</td> </tr> <tr> <td>H</td> <td>BMV1-p</td> <td>MT20</td> <td>3.0</td> <td>6.0</td> <td></td> <td></td> </tr> </tbody> </table> Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. NOTES: (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.	JT	TYPE	PLATES	W	LEN	Y	X	A	TMVW-p	MT20	5.0	6.0	2.00	2.75	B	TTWW-m	MT20	5.0	8.0	2.00	3.00	C	TMVW-w	MT20	2.0	4.0			D	TMVW-t	MT20	5.0	8.0	2.00	2.75	E	BMV1-t	MT20	4.0	9.0	Edge	1.50	F	BMVWW-t	MT20	7.0	12.0	4.25	3.75	G	BMVWW-t	MT20	6.0	9.0	4.50	2.25	H	BMV1-p	MT20	3.0	6.0			SPECIFIED CONCENTRATED LOADS (LBS) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> </thead> <tbody> <tr> <td>Q</td> <td>6-7-4</td> <td>-2429</td> <td>-2429</td> <td>545</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>C1</td> </tr> </tbody> </table> CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED. TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8) WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 5.0 PSF AND 5.0 PSF RESPECTIVELY.	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	Q	6-7-4	-2429	-2429	545	FRONT	VERT	TOTAL	---	C1
JT	TYPE	PLATES	W	LEN	Y	X																																																																														
A	TMVW-p	MT20	5.0	6.0	2.00	2.75																																																																														
B	TTWW-m	MT20	5.0	8.0	2.00	3.00																																																																														
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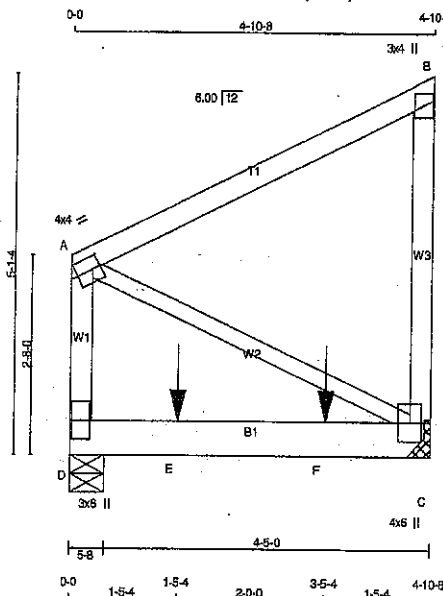


Structural component only
DWG# T-2124053 *112*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418363	T82	1	2	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1:28.3

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D - A 1	12	TOP
A - B 1	12	TOP
B - C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D - C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	5.0		
D	BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
D	1555 0	1555 0	0	5-8	5-8
C	1555 0	1555 0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1098	730 / 0	0 / 0	0 / 0	0 / 0	368 / 0	0 / 0
C	1098	730 / 0	0 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
D-A	-224 / 0	0.0	0.0 0.02 (1)	A-C	0 / 0
A-B	0 / 0	-91.8	-91.8 0.20 (1)		
C-B	-224 / 0	0.0	0.0 0.05 (1)		
D-E	0 / 0	-18.5	-18.5 0.70 (1)		
E-F	0 / 0	-18.5	-18.5 0.70 (1)		
F-C	0 / 0	-18.5	-18.5 0.70 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-5-4	-908	-908		BACK	VERT	TOTAL		C1
F	3-5-4	-908	-908		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/849 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/451 (0.13")

CSI: TC=0.20/1.00 (A-B:1), BC=0.70/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SS=0.44/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

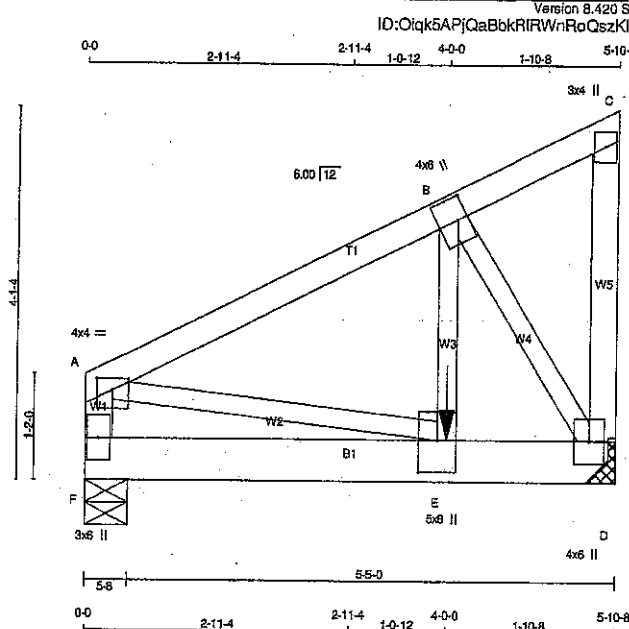
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124054

JOB NAME 418363	TRUSS NAME T83	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D	7	SIDE (368.2)
WEBS: (0.122"x3") SPIRAL NAILS		
B-E	2	SIDE (552.5)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PILES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.50 2.00
B	TMVW-t	MT20	4.0	6.0	3.00 1.25
C	TMV-p	MT20	3.0	4.0	
D	BMVW1-p	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	BRG	IN-SX	IN-SX
F	1448	0	1448	0	0	5-8	5-8		
D	2722	0	2722	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	1021	686 / 0	0 / 0	0 / 0	0 / 0	335 / 0	0 / 0
D	1919	1293 / 0	0 / 0	0 / 0	0 / 0	625 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
F-A	-1329 / 0	E-B	0 / 2993
A-B	-1872 / 0	A-E	0 / 1715
B-C	-21 / 0	B-D	-3007 / 0
D-C	-38 / 0		
F-E	0 / 0		
E-D	0 / 1684		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	4-0-0	-2482	-2482	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/998 (0.03")

CSI: TC=0.12/1.00 (A-B:1), BC=0.23/1.00 (D-E:1), WB=0.37/1.00 (B-E:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1867 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

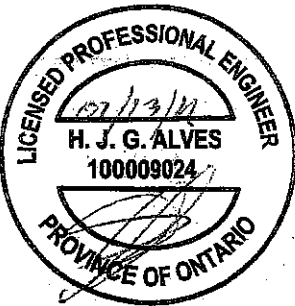
JSI GRIP= 0.85 (A) (INPUT = 0.90)

JSI METAL= 0.45 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2



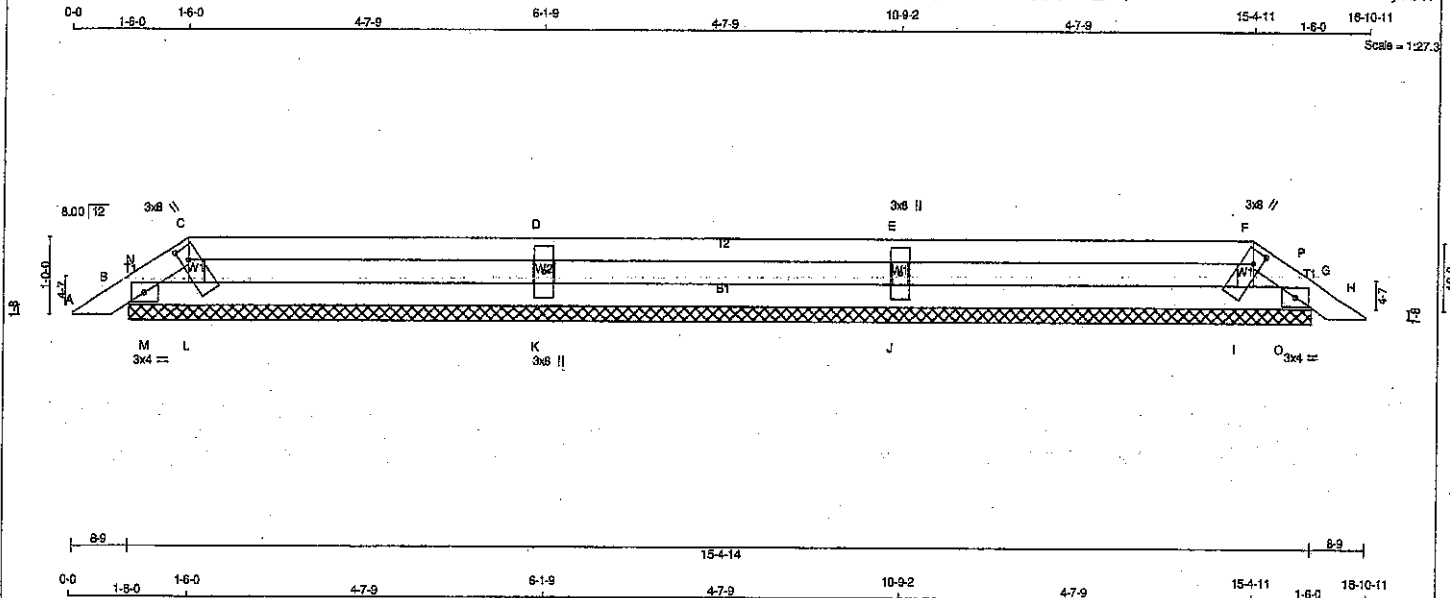
Structural component only
DWG# T-2124055

JOB NAME 418363	TRUSS NAME T83	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.																		
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Jul 22 17:33:40 2021 Page 2 ID:Olgk5APIQaBbkRIRWnRoQszKI98-k7XYHqUEOa7GirufHzcTksN5HKIZ9FvqgFmw8KvSd9																			
PLATES (table is in inches) <table border="1"><thead><tr><th>JT TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>BMWW+l</td><td>MT20</td><td>5.0</td><td>8.0</td><td>4.25</td><td>2.50</td></tr><tr><td>BMVl+p</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.						JT TYPE	PLATES	W	LEN	Y	X	BMWW+l	MT20	5.0	8.0	4.25	2.50	BMVl+p	MT20	3.0	6.0		
JT TYPE	PLATES	W	LEN	Y	X																		
BMWW+l	MT20	5.0	8.0	4.25	2.50																		
BMVl+p	MT20	3.0	6.0																				
 Structural component only DWG# T-2124055 <i>212</i>																							

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
418361	PB1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Nx0GiYRcZlvpB_E9rQcwHDZcyrp-pTs9_?Gj8loEJPt4?uSEwtYrcfv9PwoN51bF8yvU?F



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-H	MT20	3.0	4.0		
C	C, D, E, F, I, J, K, L					
C	TTBW1+H	MT20	3.0	8.0	2.00	1.25
E	TMBMW1-H	MT20	3.0	8.0		
F	TTBW1+H	MT20	3.0	8.0	2.00	1.25
G	TMB1-H	MT20	3.0	4.0		
K	TMBMW1-H	MT20	3.0	8.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
B	117	0	117	0
G	117	0	117	0
L	237	0	237	0
I	237	0	237	0
K	554	0	554	0
J	554	0	554	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	78	78 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
G	78	78 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
L	172	88 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
I	172	88 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
K	391	261 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
J	391	261 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G, L, I, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							FR-TO			
A-B	0 / 15	-91.8	-91.8	0.03 (1)	10.00		L-C	-148 / 0	0.02 (1)	
B-N	-49 / 7	-91.8	-91.8	0.03 (1)	6.25		I-F	-148 / 0	0.02 (1)	
N-C	-48 / 0	-91.8	-91.8	0.02 (4)	6.25		K-D	-467 / 0	0.05 (1)	
C-D	0 / 0	-91.8	-91.8	0.27 (1)	10.00		J-E	-467 / 0	0.07 (1)	
D-E	0 / 0	-91.8	-91.8	0.27 (1)	10.00		M-N	-41 / 0	0.00 (1)	
E-F	0 / 0	-91.8	-91.8	0.27 (1)	10.00		O-P	-41 / 0	0.00 (1)	
F-P	-48 / 0	-91.8	-91.8	0.02 (4)	6.25					
P-G	-49 / 8	-91.8	-91.8	0.03 (1)	6.25					
G-H	0 / 15	-91.8	-91.8	0.03 (1)	10.00					
B-M	0 / 27	-18.5	-18.5	0.01 (1)	10.00					
M-L	0 / 27	-18.5	-18.5	0.06 (4)	10.00					
L-K	0 / 0	-18.5	-18.5	0.08 (4)	10.00					
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00					
J-I	0 / 0	-18.5	-18.5	0.08 (4)	10.00					
I-O	0 / 27	-18.5	-18.5	0.08 (4)	10.00					
O-G	0 / 27	-18.5	-18.5	0.01 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (D-E-1), BC=0.08/1.00 (J-K-4), WB=0.07/1.00 (E-J-1), SS=0.20/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

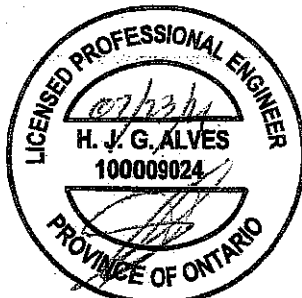
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	786	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (F) (INPUT = 0.90)
JSI METAL = 0.07 (K) (INPUT = 1.00)



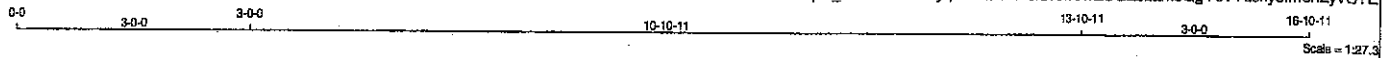
Structural component only
DWG# T-2123947

JOB NAME 418361	TRUSS NAME PB2	QUANTITY 1	PLY 1	JOB DESC GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:Nx0GfYRcZvpB_E9rQwHDzcypr-HfQXCKGLvow5wZSGZbam8GgY07YusnycIm8nZyvU?E



LUMBER

N. L. G. A. RULES					
CHORDS	SIZE		LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTWW+m	MT20	5.0	6.0	2.00	1.50
D TMW+w	MT20	2.0	4.0		
E TTWW+m	MT20	5.0	6.0	2.00	1.50
F TMB1-I	MT20	3.0	4.0		
H BMW1+w	MT20	2.0	4.0		
I BMWWW1-t	MT20	4.0	9.0		
J BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
B	199	0	199	0	15-4-14	15-4-14
F	199	0	199	0	15-4-14	15-4-14
I	738	0	738	0	15-4-14	15-4-14
J	340	0	340	0	15-4-14	15-4-14
H	340	0	340	0	15-4-14	15-4-14

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	138	107/0	0/0	0/0	0/0	31/0	0/0
F	138	107/0	0/0	0/0	0/0	31/0	0/0
I	520	353/0	0/0	0/0	0/0	167/0	0/0
J	243	144/0	0/0	0/0	0/0	100/0	0/0
H	243	144/0	0/0	0/0	0/0	100/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CS1 (LC)	MAX. UNBRAC (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CS1 (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0/15	-91.8	-91.8	0.03 (1)	10.00	I-D	-824/0	0.09 (1)
B-L	-32/0	-91.8	-91.8	0.02 (1)	8.25	J-C	-235/0	0.03 (1)
L-C	-58/0	-91.8	-91.8	0.05 (1)	6.25	H-E	-235/0	0.03 (1)
C-D	-8/0	-91.8	-91.8	0.08 (1)	10.00	C-I	-22/0	0.01 (1)
D-E	-8/0	-91.8	-91.8	0.06 (1)	10.00	I-E	-22/0	0.01 (1)
E-N	-58/0	-91.8	-91.8	0.05 (1)	8.25	K-L	-144/0	0.00 (1)
N-F	-32/0	-91.8	-91.8	0.02 (1)	6.25	M-N	-144/0	0.00 (1)
F-G	0/15	-91.8	-91.8	0.03 (1)	10.00			
B-K	0/45	-18.5	-18.5	0.08 (1)	10.00			
K-J	0/45	-18.5	-18.5	0.08 (4)	10.00			
J-I	0/29	-18.5	-18.5	0.12 (4)	10.00			
I-H	0/29	-18.5	-18.5	0.12 (4)	10.00			
H-M	0/45	-18.5	-18.5	0.08 (4)	10.00			
M-F	0/45	-18.5	-18.5	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (D-E:1), BC=0.12/1.00 (H-I:4), WB=0.09/1.00 (D-I:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

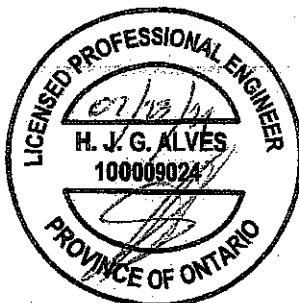
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

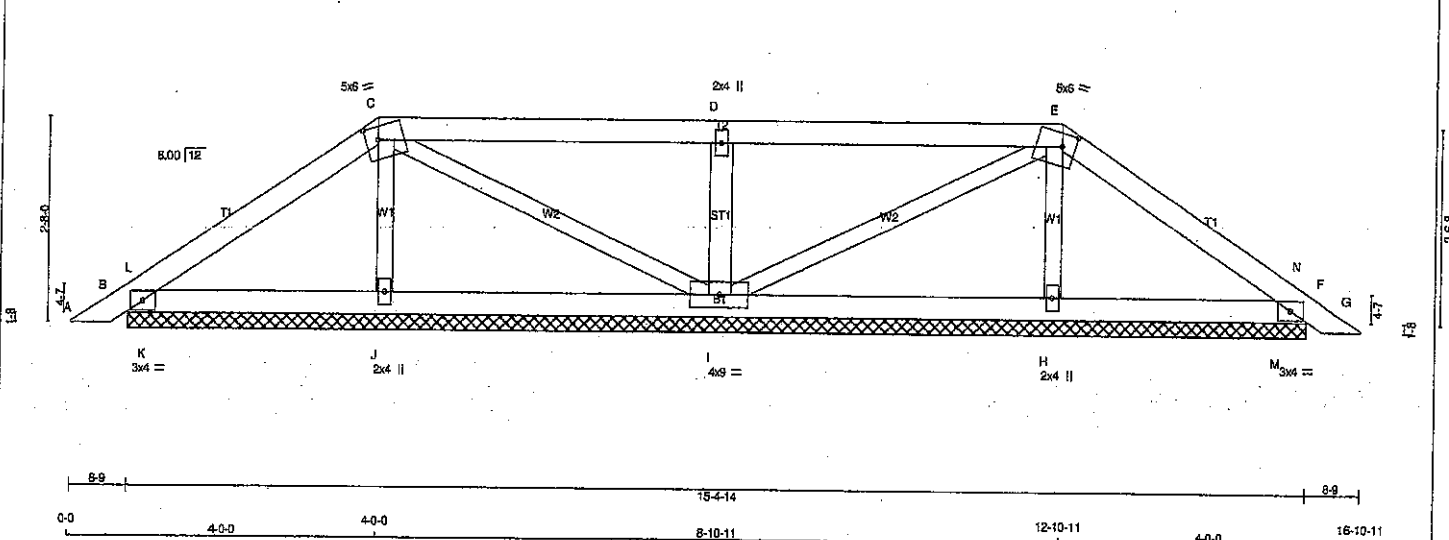
JSI GRIP = 0.31 (D) (INPUT = 0.90)
JSI METAL = 0.13 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123948

JOB NAME 418361	TRUSS NAME PB3	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Nx0GIYRcZvpB_E9rQowHDzcyrp-is_vPgHzgw2yY1S7JUpJLxvQM1dK85iPWHK?yvU?D
Scale = 1:27.3



LUMBER

N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
I - K	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			
I - D	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
E - G	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - F	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-i	MT20	3.0	4.0		
C TWW-m	MT20	5.0	6.0	2.00	2.00
D TMW-w	MT20	2.0	4.0		
E TWW-m	MT20	5.0	6.0	2.00	2.00
F TMB1-i	MT20	3.0	4.0		
H BMW1-w	MT20	2.0	4.0		
I BMWWW1-t	MT20	4.0	6.0		
J BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	GROSS REACTION	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	VERT	HORZ	DOWN	HORZ	UPLIFT
B	274	0	274	0	0
F	274	0	274	0	0
J	623	0	623	0	0
J	323	0	323	0	0
H	323	0	323	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
B	191	139 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0	0 / 0	0 / 0
F	191	139 / 0	0 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0	0 / 0	0 / 0
I	438	301 / 0	0 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0	0 / 0	0 / 0
J	231	138 / 0	0 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0	0 / 0	0 / 0
H	231	138 / 0	0 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	FORCE	MAX
FR-TO											
A-B	0 / 15	-91.8	-91.8	0.01 (1)	10.00		I-D	-510 / 0	0.02 (1)		
B-L	-91 / 0	-91.8	-91.8	0.01 (1)	6.25		J-C	-210 / 0	0.01 (1)		
L-C	-89 / 0	-91.8	-91.8	0.04 (1)	6.25		H-E	-210 / 0	0.01 (1)		
C-D	-15 / 0	-91.8	-91.8	0.10 (1)	6.25		C-I	-50 / 0	0.01 (1)		
D-E	-15 / 0	-91.8	-91.8	0.10 (1)	6.25		I-E	-50 / 0	0.01 (1)		
E-N	-89 / 0	-91.8	-91.8	0.04 (1)	6.25		K-L	-240 / 0	0.00 (1)		
N-F	-91 / 0	-91.8	-91.8	0.01 (1)	6.25		M-N	-240 / 0	0.00 (1)		
F-G	0 / 15	-91.8	-91.8	0.01 (1)	10.00						
B-K	0 / 89	-18.5	-18.5	0.04 (1)	10.00						
K-J	0 / 89	-18.5	-18.5	0.04 (1)	10.00						
J-I	0 / 59	-18.5	-18.5	0.02 (1)	10.00						
I-H	0 / 59	-18.5	-18.5	0.02 (1)	10.00						
H-M	0 / 89	-18.5	-18.5	0.04 (1)	10.00						
M-F	0 / 89	-18.5	-18.5	0.04 (1)	10.00						

TOTAL WEIGHT = 3 X 51 = 154 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 35.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-D:1), BC=0.04/1.00 (H-M:1), WB=0.02/1.00 (D-I:1), SS=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

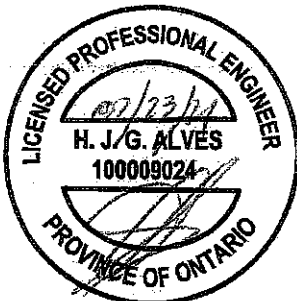
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1875

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (D) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

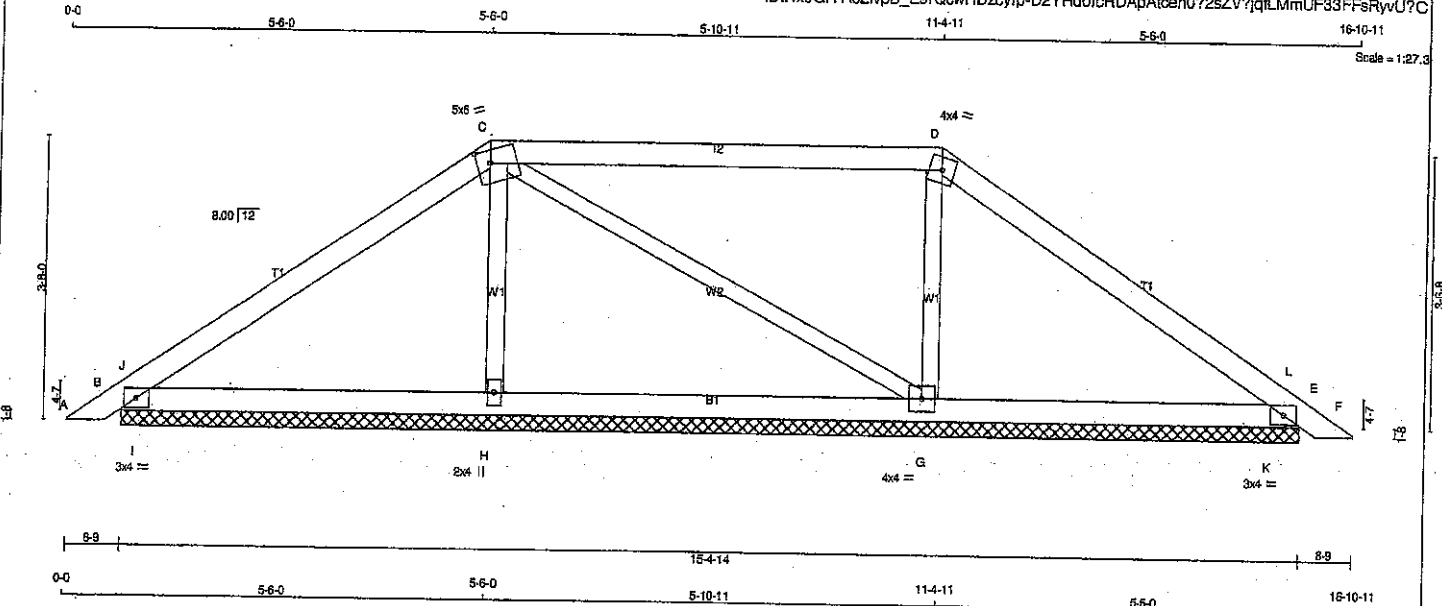


Structural component only
DWG# T-2123949

JOB NAME 418361	TRUSS NAME PB4	QUANTITY 1	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 48 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS: 2x3 DRY No.2				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B-TMB1-I	MT20	3.0	4.0		
C-TTW-m	MT20	5.0	6.0	2.00	2.00
D-TTW-m	MT20	4.0	4.0		
E-TMB1-I	MT20	3.0	4.0		
G-BMW1-I	MT20	4.0	4.0		
H-BMW1-w	MT20	2.0	4.0		

NOTES

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
B	417	0	417	0	15-4-14	15-4-14
E	394	0	394	0	15-4-14	15-4-14
H	471	0	471	0	15-4-14	15-4-14
G	535	0	535	0	15-4-14	15-4-14

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	292	210 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
E	278	197 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
H	335	208 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
G	379	242 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 15	-91.8 -91.8	0.03 (1)	10.00	H-C	-327 / 0	0.07 (1)
B-J	-46 / 0	-91.8 -91.8	0.11 (1)	6.25	G-D	-371 / 0	0.08 (1)
J-C	-206 / 0	-91.8 -91.8	0.26 (1)	6.25	C-G	-41 / 0	0.03 (1)
C-D	-116 / 0	-91.8 -91.8	0.55 (1)	6.25	I-J	-478 / 0	0.00 (1)
D-L	-164 / 0	-91.8 -91.8	0.26 (1)	6.25	K-L	-481 / 0	0.00 (1)
L-E	-38 / 3	-91.8 -91.8	0.11 (1)	6.25			
E-F	0 / 15	-91.8 -91.8	0.03 (1)	10.00			
B-I	0 / 182	-18.5 -18.5	0.22 (1)	10.00			
I-H	0 / 182	-18.5 -18.5	0.22 (1)	10.00			
H-G	0 / 152	-18.5 -18.5	0.13 (4)	10.00			
G-K	0 / 128	-18.5 -18.5	0.22 (1)	10.00			
K-E	0 / 128	-18.5 -18.5	0.22 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 8.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55/1.00 (C-D:1), BC=0.22/1.00 (B-I:1), WB=0.08/1.00 (D-G:1), SS=0.37/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	850	371	1747
		785	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

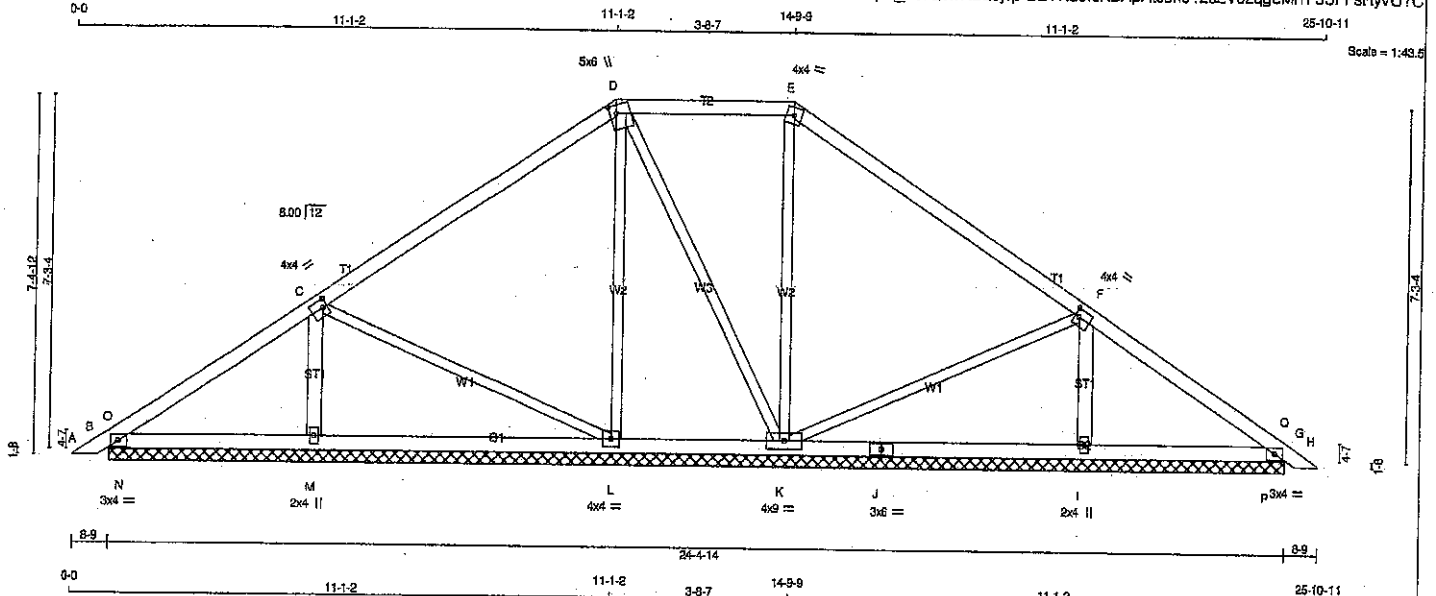


Structural component only
DWG# T-2123950

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
418361	PB5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - E 2x4 DRY No.2

E - H 2x4 DRY No.2

B - J 2x4 DRY No.2

J - G 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

M - C 2x4 DRY No.2

I - F 2x4 DRY No.2

SPF

SPF

SPF

SPF

SPF

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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
DOWN	UP	IN-SX	IN-SX
JT 288	0	24-4-14 (15-12-4-3-14)	
B 288	0	24-4-14 (15-12-4-3-14)	
M 684	0	24-4-14 (15-12-4-3-14)	
L 948	0	24-4-14 (15-12-4-3-14)	
K 333	0	24-4-14 (15-12-4-3-14)	
G 625	0	24-4-14 (15-12-4-3-14)	
G 250	0	24-4-14 (15-12-4-3-14)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	WIND	DEAD	SOIL
B 188	135 / 0	0 / 0	0 / 0	53 / 0	0 / 0
M 483	319 / 0	0 / 0	0 / 0	164 / 0	0 / 0
L 459	299 / 0	0 / 0	0 / 0	159 / 0	0 / 0
K 239	137 / 0	0 / 0	0 / 0	103 / 0	0 / 0
G 440	300 / 0	0 / 0	0 / 0	140 / 0	0 / 0
G 175	124 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, L, K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSF (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	CSF (LC)
PR-TO					FR-TO		
A-B	0 / 15	-91.8	-91.8 0.03 (1)	10.00	M-C	-568 / 0	0.06 (1)
B-O	-163 / 0	-91.8	-91.8 0.02 (4)	6.25	I-F	-530 / 0	0.07 (1)
O-C	-78 / 0	-91.8	-91.8 0.46 (1)	6.25	L-D	-231 / 0	0.21 (1)
C-D	-118 / 0	-91.8	-91.8 0.46 (1)	6.25	K-E	-389 / 0	0.38 (1)
D-E	-7 / 0	-91.8	-91.8 0.21 (1)	10.00	D-K	-134 / 0	0.17 (1)
E-F	-45 / 0	-91.8	-91.8 0.48 (1)	6.25	C-L	-38 / 0	0.03 (1)
F-Q	-45 / 0	-91.8	-91.8 0.48 (1)	6.25	K-F	-73 / 0	0.08 (1)
Q-G	-130 / 0	-91.8	-91.8 0.02 (1)	6.25	N-O	-47 / 13	0.00 (1)
G-H	0 / 15	-91.8	-91.8 0.03 (1)	10.00	P-Q	-49 / 12	0.00 (1)
B-N	0 / 108	-18.5	-18.5 0.04 (1)	10.00			
N-M	0 / 108	-18.5	-18.5 0.14 (4)	10.00			
M-L	0 / 108	-18.5	-18.5 0.14 (4)	10.00			
L-K	0 / 68	-18.5	-18.5 0.10 (4)	10.00			
K-J	0 / 79	-18.5	-18.5 0.14 (4)	10.00			
J-I	0 / 79	-18.5	-18.5 0.14 (4)	10.00			
I-P	0 / 79	-18.5	-18.5 0.14 (4)	10.00			
P-G	0 / 79	-18.5	-18.5 0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

BOT CH. LL = 8.0 PSF

DL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019

- PART 9 OF NBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.48/1.00 (C-D:1), BC=0.14/1.00 (M-N:4), WB=0.36/1.00 (E-K:1), SS=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSB) (PLD) (PLD)

MAX MIN MAX MIN MAX MIN

MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.80)

JSI METAL= 0.16 (J) (INPUT = 1.00)

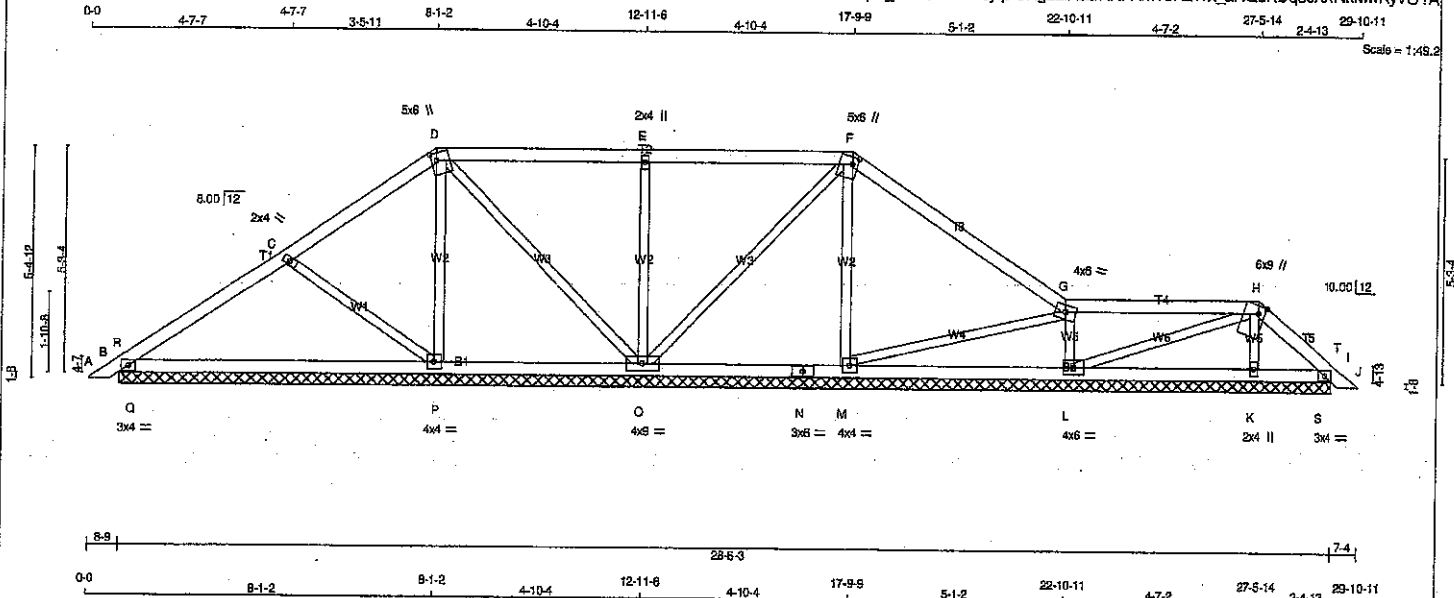


Structural component only
DWG# T-2123951

Structural component only
DWG# T-2123952

JOB NAME 418361	TRUSS NAME PB7	QUANTITY 1	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

F - G 2x4 DRY No.2

G - H 2x4 DRY No.2

H - J 2x4 DRY No.2

B - N 2x4 DRY No.2

N - I 2x4 DRY No.2

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMB1-I MT20 3.0 4.0

C TMW-w MT20 2.0 4.0

D TTWW+m MT20 5.0 6.0 2.00 1.50

E TMW-w MT20 2.0 4.0

F TTWW+m MT20 5.0 6.0 2.00 1.50

G TTWW-m MT20 4.0 6.0

H TTWW+m MT20 6.0 9.0 Edge 1.75

I TMB1-I MT20 3.0 4.0

K BMW1-w MT20 2.0 4.0

L BMWW1-t MT20 4.0 6.0

M BMWW1-t MT20 4.0 4.0

N 8S-t MT20 3.0 6.0

O BMWW1-t MT20 4.0 9.0

P BMWW1-t MT20 4.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE

TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
B	430	0	430	0
P	621	0	621	0
M	381	0	381	0
O	779	0	779	0
L	556	0	556	0
K	305	0	305	0
I	178	0	178	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	303	205 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0	0 / 0
P	442	274 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0	0 / 0
M	273	158 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0	0 / 0
O	544	395 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0	0 / 0
L	394	257 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0	0 / 0
K	217	134 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0	0 / 0
I	124	85 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P, M, O, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 15	-91.8 -91.8 0.03 (1)	10.00	P-D	-267 / 0	0.11 (1)	
B-R	-447 / 0	-91.8 -91.8 0.13 (4)	6.25	M-F	-277 / 0	0.11 (1)	
R-C	-292 / 0	-91.8 -91.8 0.21 (1)	6.25	O-E	-556 / 0	0.23 (1)	
C-D	-3 / 23	-91.8 -91.8 0.21 (1)	10.00	D-O	-84 / 0	0.06 (1)	
D-E	0 / 90	-91.8 -91.8 0.38 (1)	10.00	O-F	-152 / 0	0.13 (1)	
E-F	0 / 90	-91.8 -91.8 0.38 (1)	10.00	L-G	-456 / 0	0.07 (1)	
F-G	-27 / 0	-91.8 -91.8 0.41 (1)	6.25	K-H	-223 / 0	0.03 (1)	
G-H	-7 / 0	-91.8 -91.8 0.33 (1)	10.00	L-H	-30 / 0	0.01 (1)	
H-T	-69 / 0	-91.8 -91.8 0.03 (1)	6.25	M-G	-15 / 0	0.01 (1)	
T-I	-47 / 0	-91.8 -91.8 0.01 (1)	6.25	C-P	-378 / 0	0.13 (1)	
I-J	0 / 14	-91.8 -91.8 0.02 (1)	10.00	Q-R	0 / 219	0.00 (1)	
				S-T	-111 / 0	0.00 (1)	
B-Q	0 / 269	-18.5 -18.5 0.11 (4)	10.00				
Q-P	0 / 269	-18.5 -18.5 0.20 (4)	10.00				
P-O	-45 / 0	-18.5 -18.5 0.18 (4)	6.25				
O-N	0 / 17	-18.5 -18.5 0.10 (4)	10.00				
N-M	0 / 17	-18.5 -18.5 0.10 (4)	10.00				
M-L	0 / 37	-18.5 -18.5 0.10 (4)	10.00				
L-K	0 / 35	-18.5 -18.5 0.09 (4)	10.00				
K-S	0 / 50	-18.5 -18.5 0.05 (4)	10.00				
S-I	0 / 50	-18.5 -18.5 0.04 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A

SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR

SMALL BUILDING REQUIREMENTS OF PART 9,

NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 088-14

- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN

LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF

LIVE LOAD

CSI: TC=0.41/1.00 (F-G:1), BC=0.20/1.00 (P-Q:4),

WB=0.23/1.00 (E-O:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN THE

TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.67 (M) (INPUT = 0.90)

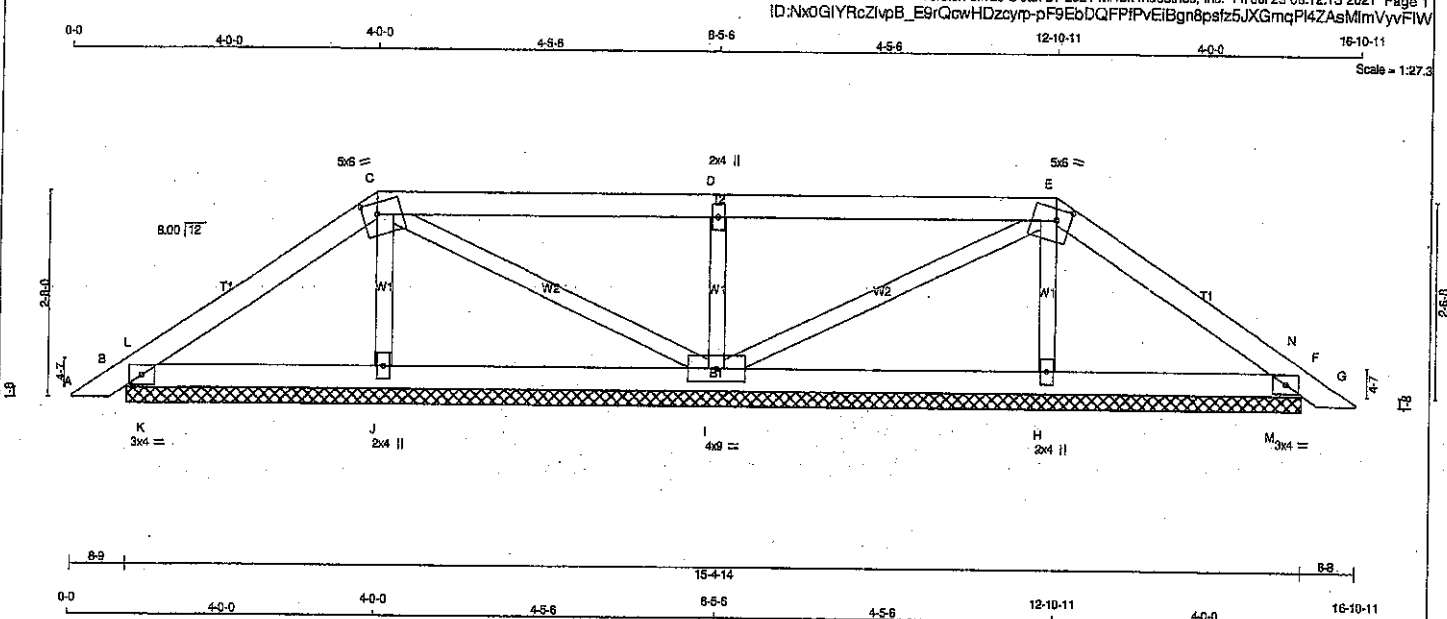
JSI METAL= 0.12 (C) (INPUT = 1.00)



Structural component only
DWG# T-2123953

JOB NAME 418362	TRUSS NAME PB30	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Fri Jul 23 08:12:13 2021 Page 1
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
E - G	2x4	DRY No.2
B - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.25
D	TMW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	1.75	2.25
F	TMB1-I	MT20	3.0	4.0		
H	BMW1-w	MT20	2.0	4.0		
I	BMWW1-I	MT20	4.0	9.0		
J	BMW1-w	MT20	2.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	273	0	273	0	15-4-14	15-4-14
F	273	0	273	0	15-4-14	15-4-14
J	324	0	324	0	15-4-14	15-4-14
I	622	0	622	0	15-4-14	15-4-14
H	324	0	324	0	15-4-14	15-4-14

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	191	138 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	191	138 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
J	231	138 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
I	438	300 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
H	231	138 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO				FR-TO		
A-B	0 / 15	-91.8	-91.8 0.03 (1)	10.00	J-C	-211 / 0 0.03 (1)
B-L	-28 / 0	-91.8	-91.8 0.04 (1)	6.25	C-I	-49 / 0 0.02 (1)
L-C	-88 / 0	-91.8	-91.8 0.12 (1)	6.25	I-D	-509 / 0 0.08 (1)
C-D	-14 / 0	-91.8	-91.8 0.31 (1)	6.25	D-E	-49 / 0 0.02 (1)
D-E	-14 / 0	-91.8	-91.8 0.31 (1)	6.25	E-N	-211 / 0 0.03 (1)
E-N	-88 / 0	-91.8	-91.8 0.12 (1)	6.25	N-F	-245 / 0 0.00 (1)
N-F	-28 / 0	-91.8	-91.8 0.04 (1)	6.25	F-G	-244 / 0 0.00 (1)
F-G	0 / 15	-91.8	-91.8 0.03 (1)	10.00		
B-K	0 / 69	-18.5	-18.5 0.11 (1)	10.00		
K-J	0 / 69	-18.5	-18.5 0.11 (1)	10.00		
J-I	0 / 59	-18.5	-18.5 0.07 (1)	10.00		
I-H	0 / 59	-18.5	-18.5 0.07 (1)	10.00		
H-M	0 / 69	-18.5	-18.5 0.11 (1)	10.00		
M-F	0 / 69	-18.5	-18.5 0.11 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-D:1), BC=0.11/1.00 (B-K:1), WB=0.08/1.00 (D-I:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 783
	1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

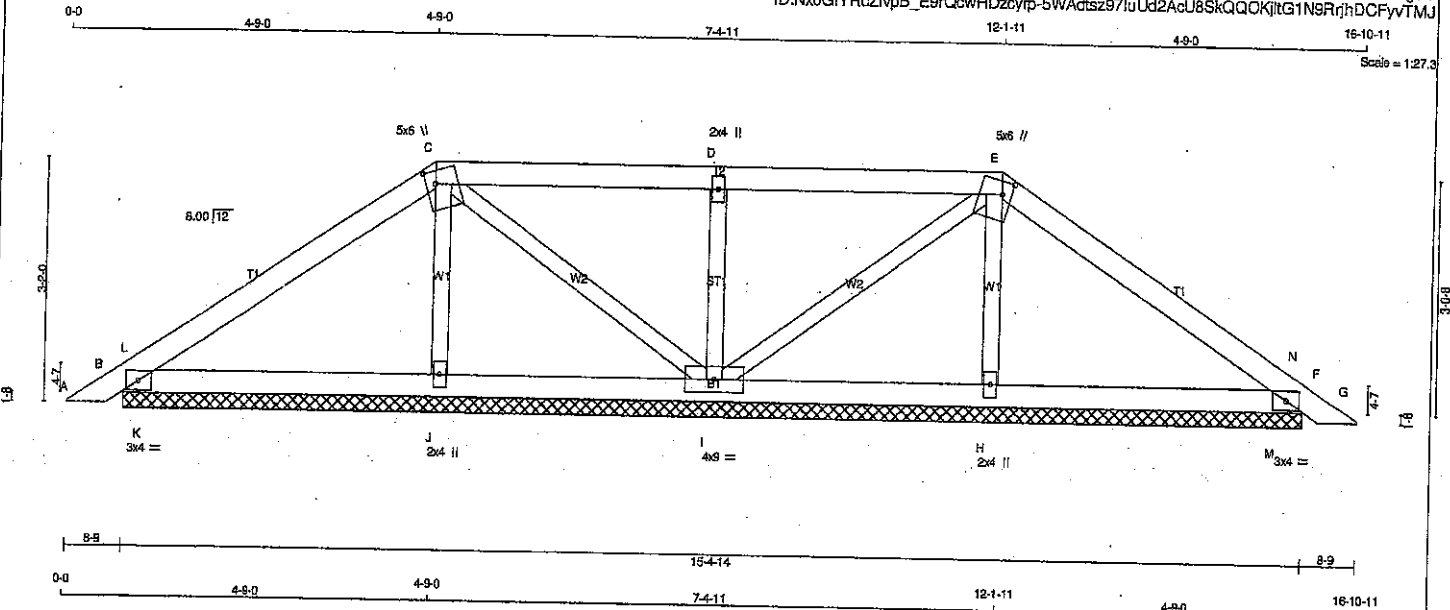
JSI GRIP = 0.25 (D) (INPUT = 0.90)
JSI METAL = 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123986

JOB NAME 418362	TRUSS NAME PB31	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:Nx0GIYRcZlvpB_E9rQcwHDZcyp-5WActs297JuUd2AcU8SkQKQKjtg1N9RrjhDCFyVTMJ



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A - C	1	12	TOP
C - E	1	12	TOP
E - G	1	12	TOP
G - I	1	12	TOP
I - K	1	12	TOP
K - M	1	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
B - F	1	12	TOP
F - J	1	12	TOP
J - L	1	12	TOP
L - M	1	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B - TM21+H	MT20	3.0	4.0	
C - TTWV+m	MT20	5.0	6.0	2.00 1.50
D - TMW+w	MT20	2.0	4.0	
E - TTWV+m	MT20	5.0	6.0	2.00 1.50
F - TM21+H	MT20	3.0	4.0	
G - BMW1+w	MT20	2.0	4.0	
H - BMWVW1+H	MT20	4.0	9.0	
I - BMW1+w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
B	329	0	329	0	0	15-4-14	15-4-14	15-4-14	15-4-14
F	329	0	329	0	0	15-4-14	15-4-14	15-4-14	15-4-14
I	540	0	540	0	0	15-4-14	15-4-14	15-4-14	15-4-14
J	309	0	309	0	0	15-4-14	15-4-14	15-4-14	15-4-14
H	309	0	309	0	0	15-4-14	15-4-14	15-4-14	15-4-14

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	230	164 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
B	230	164 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
F	379	257 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
I	221	130 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
J	221	130 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 15	I-D	-423 / 0
B-L	-34 / 0	J-C	-178 / 0
L-C	-119 / 0	H-E	-178 / 0
C-D	-26 / 0	C-I	-73 / 0
D-E	-26 / 0	I-E	-73 / 0
E-N	-119 / 0	K-L	-349 / 0
N-F	-34 / 0	M-N	-349 / 0
F-G	0 / 15		
B-K	0 / 82		
K-J	0 / 82		
J-I	0 / 85		
I-H	0 / 85		
H-M	0 / 82		
M-F	0 / 82		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-D-1), BC=0.05/1.00 (H-M-1), WB=0.03/1.00 (D-I-1), SS=0.09/1.00 (F-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

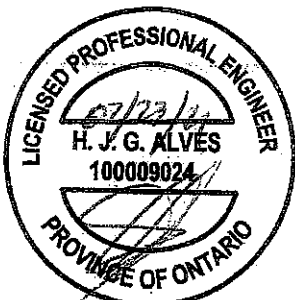
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

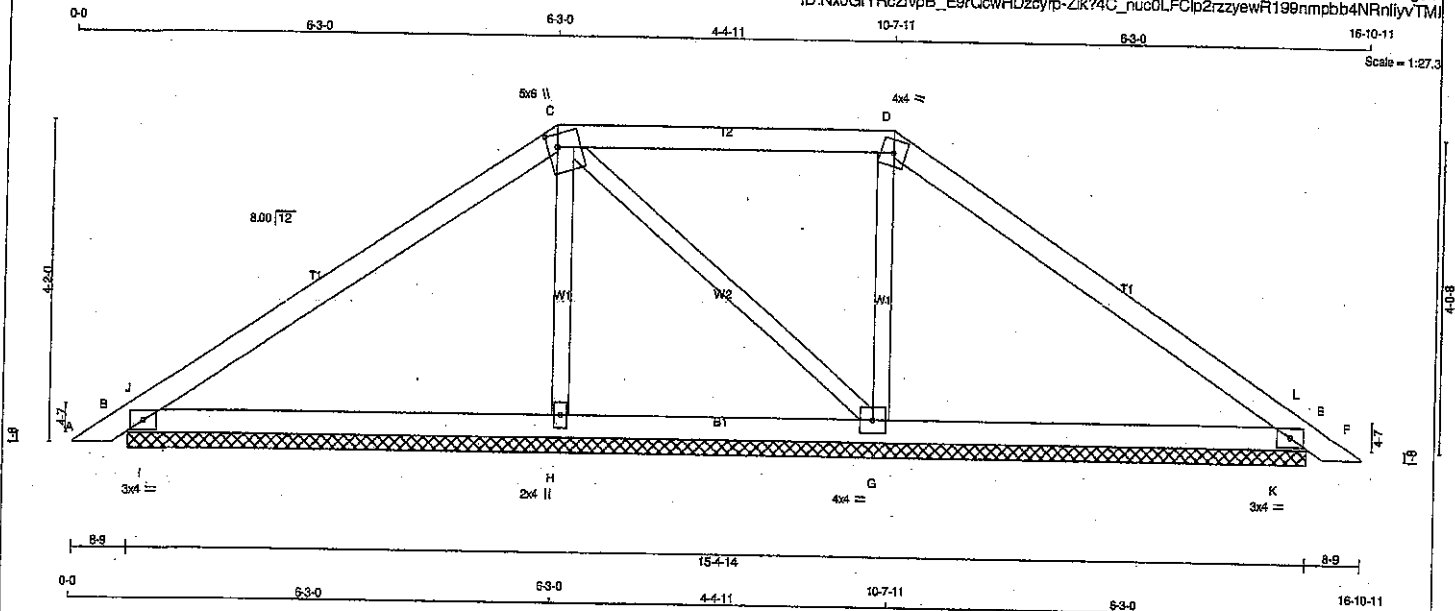
JSI GRIP = 0.09 (B) (INPUT = 0.90)
JSI METAL = 0.03 (D) (INPUT = 1.00)



Structural component only
DWG# T-2123987

JOB NAME 418362	TRUSS NAME PB32	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
B - E 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMW1-I	MT20	4.0	4.0		
H	BMW1-w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	DOWN	IN-SX	IN-SX
B	488	0	15-4-14	15-4-14
E	488	0	15-4-14	15-4-14
H	385	0	15-4-14	15-4-14
G	488	0	15-4-14	15-4-14

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	341	240 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
E	321	224 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
H	275	185 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
G	345	224 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
A-B	0 / 15	-91.8	-91.8 0.03 (1)	10.00	H-C	-244 / 0	0.08 (1)
B-J	-71 / 0	-91.8	-91.8 0.16 (1)	6.25	G-D	-309 / 0	0.08 (1)
J-C	-258 / 0	-91.8	-91.8 0.35 (1)	6.25	C-G	-57 / 0	0.03 (1)
C-D	-154 / 0	-91.8	-91.8 0.30 (1)	6.25	I-J	-622 / 0	0.00 (1)
D-L	-208 / 0	-91.8	-91.8 0.35 (1)	6.25	K-L	-627 / 0	0.00 (1)
L-E	-68 / 30	-91.8	-91.8 0.16 (1)	6.25			
E-F	0 / 15	-91.8	-91.8 0.03 (1)	10.00			
B-I	0 / 204	-18.5	-18.5 0.29 (1)	10.00			
I-H	0 / 204	-18.5	-18.5 0.29 (1)	10.00			
H-G	0 / 197	-18.5	-18.5 0.16 (1)	10.00			
G-K	0 / 183	-18.5	-18.5 0.29 (1)	10.00			
K-E	0 / 183	-18.5	-18.5 0.29 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.35/1.00 (C-J:1), BC=0.29/1.00 (B-I:1), WB=0.08/1.00 (D-G:1), SS=0.49/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

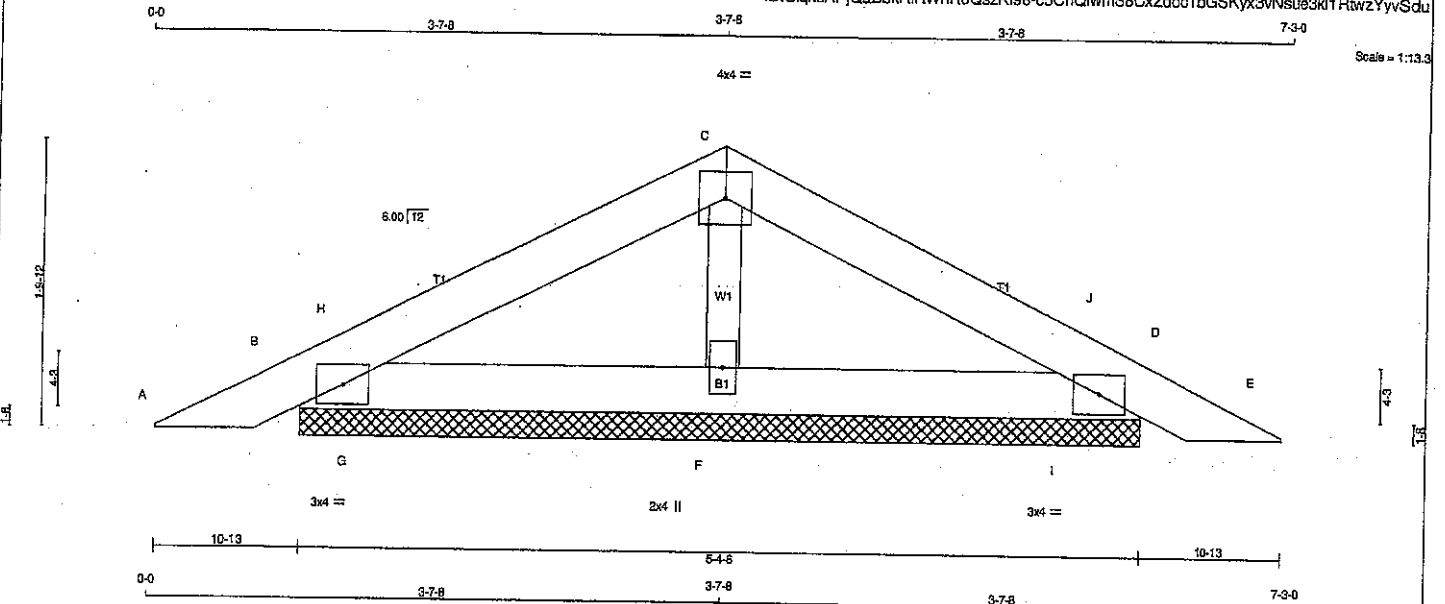
JSI GRIP = 0.40 (B) (INPUT = 0.90)
JSI METAL = 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123988

JOB NAME 418363	TRUSS NAME PB50	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-p	MT20	4.0	4.0
D	TMB1-I	MT20	3.0	4.0
F	BMW1-w	MT20	2.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	234	0	234	0	0	5-4-6	5-4-6
D	234	0	234	0	0	5-4-6	5-4-6
F	279	0	279	0	0	5-4-6	5-4-6

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	163	118/0	0/0	0/0	0/0	45/0	0/0
D	163	118/0	0/0	0/0	0/0	45/0	0/0
F	198	124/0	0/0	0/0	0/0	74/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/16	-91.8 -91.8	0.05 (1)	10.00	F-C	-148/0	0.02 (1)
B-H	-44/0	-91.8 -91.8	0.02 (1)	6.25	G-H	-151/0	0.00 (1)
H-C	-59/0	-91.8 -91.8	0.07 (1)	8.25	I-J	-151/0	0.00 (1)
C-J	-59/0	-91.8 -91.8	0.07 (1)	6.25			
J-D	-44/0	-91.8 -91.8	0.02 (1)	6.25			
D-E	0/16	-91.8 -91.8	0.05 (1)	10.00			
B-G	0/50	-18.5 -18.5	0.08 (1)	10.00			
G-F	0/50	-18.5 -18.5	0.08 (1)	10.00			
F-I	0/50	-18.5 -18.5	0.08 (1)	10.00			
I-D	0/50	-18.5 -18.5	0.08 (1)	10.00			

TOTAL WEIGHT = 2 X 16 = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 8.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCO 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-J:1), BC=0.08/1.00 (F-1:1), WB=0.02/1.00 (C-F:1), SSI=0.11/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

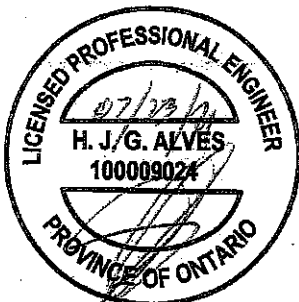
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

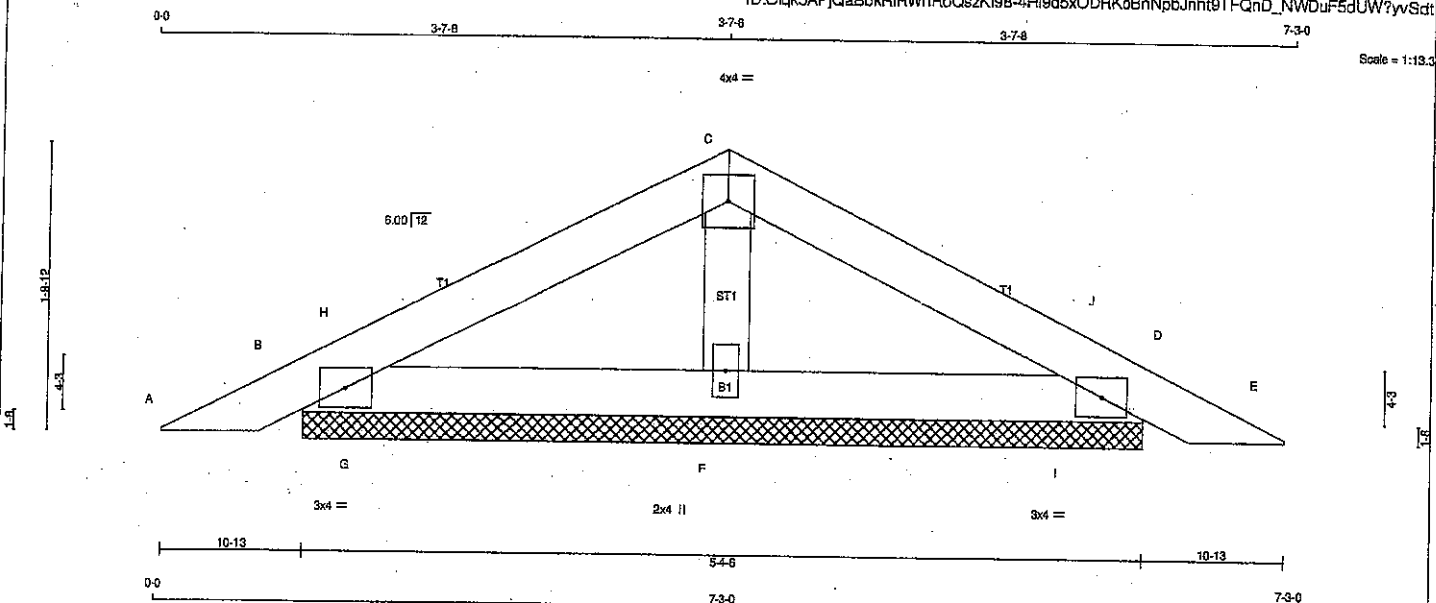
JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.04 (D) (INPUT = 1.00)



Structural component only
DWG# T-2124016

Tamarack Roof Truss, Burlington

Version 8.420 5 Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:32:54 2021 Page 1
ID:Oigk5APjQaBbkRiRWnRcQszKl9B-4Hl9d5xODRKbBnNptJnh9TFQnD_NWduF5dUW?yvsdt



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
B - D	2x4	DRY	No.2 SPF
ALL WEBS	2x4	DRY	No.2 SPF
DRY: SEASONED LUMBER			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1-w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	231	0	231	0	5-4-6	5-4-6
D	231	0	231	0	5-4-6	5-4-6
F	283	0	283	0	5-4-6	5-4-6

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
B	162	117 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
D	162	117 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
F	201	127 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED	MAX. FACTORED
	FORCE (LBS)	VERT. LOAD (PLF)	FROM	TO				FORCE (LBS)	MAX. FACTORED		
FR-TO											
A-B	0 / 16	-91.8	-91.8	0.02 (1)	10.00		F-C	-154 / 0	0.01 (1)		
B-H	-40 / 0	-91.8	-91.8	0.02 (1)	6.25		G-H	-148 / 0	0.00 (1)		
H-C	-53 / 0	-91.8	-91.8	0.02 (1)	6.25		I-J	-148 / 0	0.00 (1)		
C-J	-53 / 0	-91.8	-91.8	0.02 (1)	6.25						
J-D	-40 / 0	-91.8	-91.8	0.00 (1)	6.25						
D-E	0 / 16	-91.8	-91.8	0.02 (1)	10.00						
B-G	0 / 45	-18.5	-18.5	0.03 (1)	10.00						
G-F	0 / 45	-18.5	-18.5	0.03 (1)	10.00						
F-I	0 / 45	-18.5	-18.5	0.03 (1)	10.00						
I-D	0 / 45	-18.5	-18.5	0.03 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.02/1.00 (C-J:1), BC=0.03/1.00 (F-I:1), WB=0.01/1.00 (G-F:1), SS=0.03/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

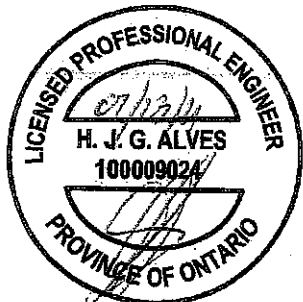
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

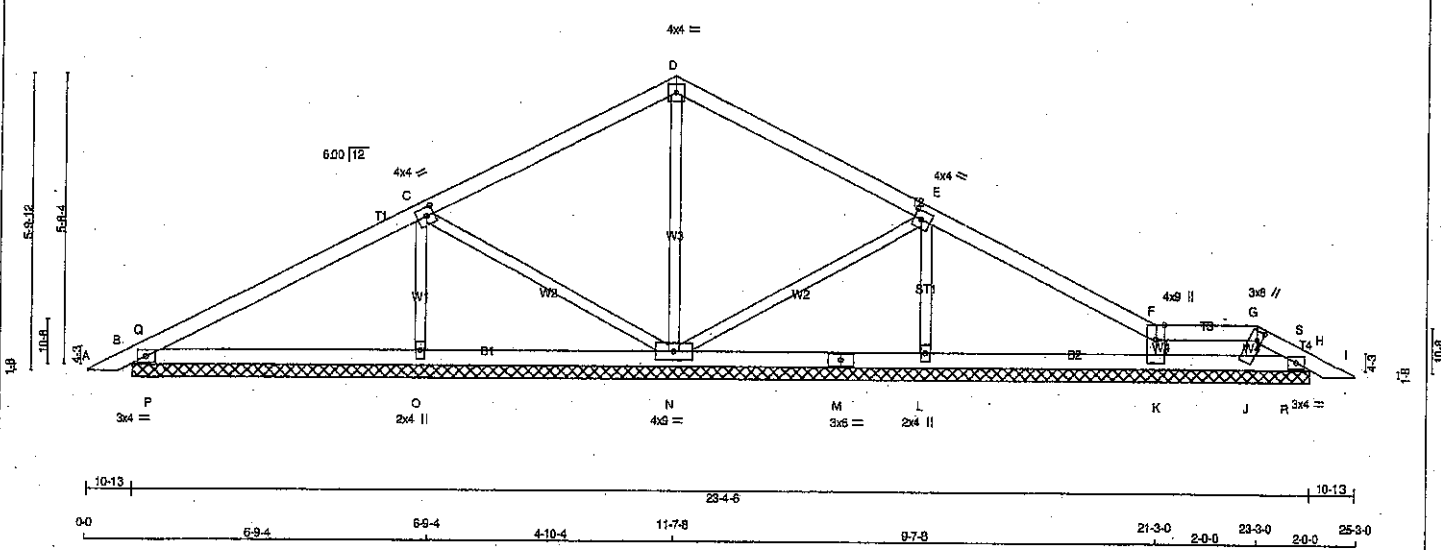
JSI GRIP = 0.06 (B) (INPUT = 0.90)
JSI METAL = 0.01 (D) (INPUT = 1.00)



Structural component only
DWG# T-2124017



Structural component only
DWG# T-2124018



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
S - M	2x4	DRY	No.2
M - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1+1	MT20	3.0	4.0		
C	TMBW1+1	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMBW1+1	MT20	4.0	4.0	2.00	1.75
F	G, J, K					
G	TTBW1+1+1	MT20	4.0	9.0	Edge	
H	TTBW1+1+1	MT20	3.0	6.0	2.25	1.00
L	TMB1+1	MT20	3.0	4.0		
N	BMW1+1	MT20	2.0	4.0		
M	SS-1	MT20	3.0	6.0		
O	BMWW1+1	MT20	4.0	9.0		
P	BMW1+1	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
B	383	0	383	0	23-4-6 (13-11-224-6	
L	577	0	577	0	23-4-6 (13-11-224-6	
J	96	0	96	0	23-4-6 (13-11-224-6	
K	356	0	356	0	23-4-6 (13-11-224-6	
N	518	0	518	0	23-4-6 (13-11-224-6	
O	608	0	608	0	23-4-6 (13-11-224-6	
H	193	0	193	0	23-4-6 (13-11-224-6	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	269	188 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
L	408	269 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0
J	67	49 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
K	253	159 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
N	366	239 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
O	431	280 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
H	134	100 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF #2 OR BETTER AT JOINT(S) B, L, J, K, N, O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 16	-91.8	-91.8	0.05 (1)	10.00	L-E	-480 / 0
B-Q	-97 / 0	-91.8	-91.8	0.05 (1)	6.25	J-G	-99 / 0
Q-C	-112 / 0	-91.8	-91.8	0.41 (1)	6.25	K-F	-288 / 0
C-D	-35 / 0	-91.8	-91.8	0.41 (1)	6.25	N-D	-338 / 0
D-E	-32 / 0	-91.8	-91.8	0.36 (1)	6.25	N-E	-78 / 0
E-F	-58 / 0	-91.8	-91.8	0.36 (1)	6.25	C-C	-460 / 0
F-G	-24 / 0	-91.8	-91.8	0.06 (1)	6.25	C-N	-131 / 0
G-S	-48 / 0	-91.8	-91.8	0.01 (1)	6.25	P-Q	-285 / 15
S-H	-87 / 0	-91.8	-91.8	0.04 (1)	6.25	R-S	0 / 28
H-I	0 / 16	-91.8	-91.8	0.05 (1)	10.00		
B-P	0 / 121	-18.5	-18.5	0.18 (1)	10.00		
P-O	0 / 121	-18.5	-18.5	0.19 (1)	10.00		
O-N	0 / 121	-18.5	-18.5	0.14 (1)	10.00		
N-M	0 / 75	-18.5	-18.5	0.10 (4)	10.00		
M-L	0 / 75	-18.5	-18.5	0.10 (4)	10.00		
L-K	0 / 75	-18.5	-18.5	0.10 (4)	10.00		
K-J	0 / 24	-18.5	-18.5	0.08 (4)	10.00		
J-R	0 / 41	-18.5	-18.5	0.01 (1)	10.00		
R-H	0 / 41	-18.5	-18.5	0.01 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (C-Q:1), BC=0.19/1.00 (O-P:1), WB=0.16/1.00 (D-N:1), SSI=0.23/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

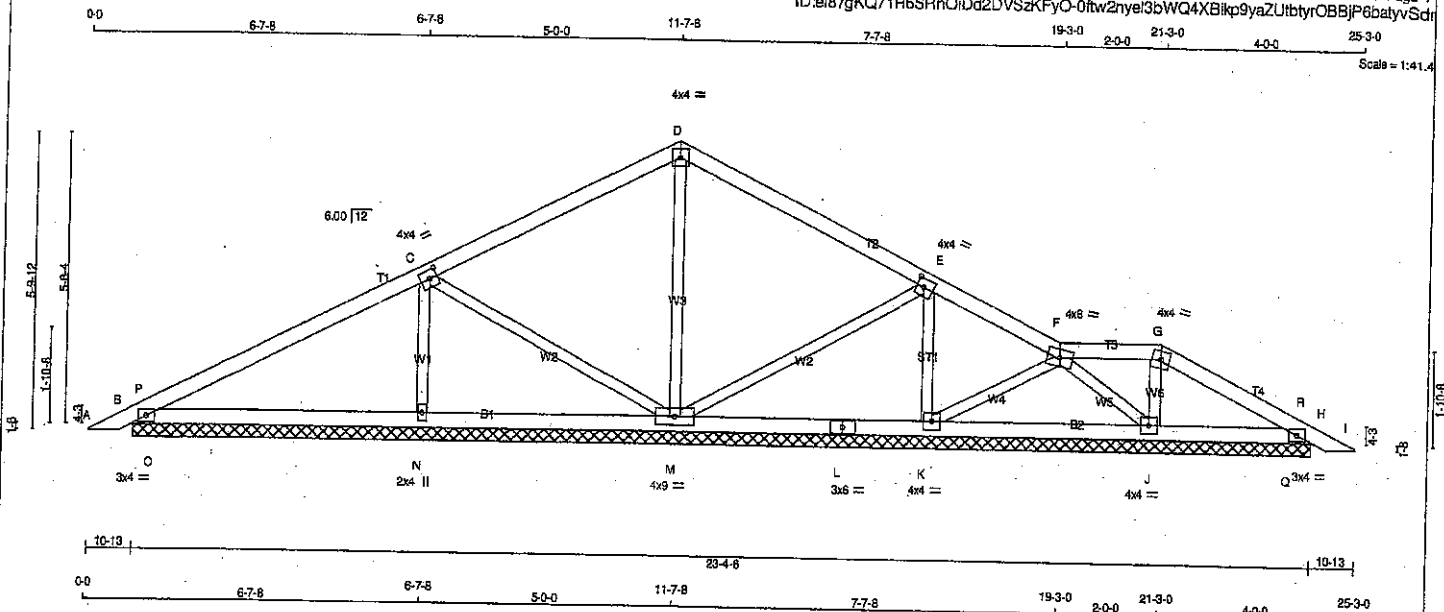
JSI GRIP = 0.75 (G) (INPUT = 0.90)
JSI METAL = 0.10 (D) (INPUT = 1.00)



JOB NAME 418363	TRUSS NAME PB53	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:32:55 2021 Page 1	

ID: a187gKQ71H65RnOIdD2DVSzKyO-0itw2nye3bWQ4XBikp9yaZUbttyrOBbJP6batyVScd

Scale = 1:41.4



TOTAL WEIGHT = 81 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
B	384	0	0	384	0	0	23-4-6 (9-4-103-4-6	0	0
K	584	0	0	584	0	0	23-4-6 (9-4-103-4-6	0	0
J	414	0	0	414	0	0	23-4-6 (9-4-103-4-6	0	0
M	513	0	0	513	0	0	23-4-6 (9-4-103-4-6	0	0
N	610	0	0	610	0	0	23-4-6 (9-4-103-4-6	0	0
H	246	0	0	246	0	0	23-4-6 (9-4-103-4-6	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
B	270	188 / 0	0 / 0
K	399	263 / 0	0 / 0
J	293	189 / 0	0 / 0
M	368	238 / 0	0 / 0
N	452	281 / 0	0 / 0
H	172	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, M, N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.	MAX. FORCE (LBS)	MAX. UNBRACED LENGTH (LBS)	MAX. UNBRACED LENGTH (LBS)
FR-TO				FR-TO			
A-B	0 / 18	-91.8	-91.8 0.05 (1)	10.00	K-E	-425 / 0	0.08 (1)
B-P	-99 / 0	-91.8	-91.8 0.05 (1)	6.25	J-G	-138 / 0	0.03 (1)
P-C	-114 / 0	-91.8	-91.8 0.41 (1)	6.25	M-D	-345 / 0	0.17 (1)
C-D	-39 / 0	-91.8	-91.8 0.41 (1)	6.25	N-C	-462 / 0	0.09 (1)
D-E	-32 / 0	-91.8	-91.8 0.29 (1)	6.25	Q-M	-128 / 0	0.07 (1)
E-F	-28 / 0	-91.8	-91.8 0.29 (1)	6.25	M-E	-53 / 0	0.03 (1)
F-G	-3 / 0	-91.8	-91.8 0.06 (1)	10.00	K-F	-97 / 0	0.02 (1)
G-R	-27 / 0	-91.8	-91.8 0.10 (1)	6.25	F-J	-171 / 0	0.03 (1)
R-H	-12 / 0	-91.8	-91.8 0.02 (1)	6.25	O-P	-294 / 15	0.00 (1)
H-I	0 / 16	-91.8	-91.8 0.05 (1)	10.00	Q-R	-171 / 0	0.00 (1)
B-O	0 / 122	-18.5	-18.5 0.18 (1)	10.00			
O-N	0 / 122	-18.5	-18.5 0.19 (1)	10.00			
N-M	0 / 122	-18.5	-18.5 0.14 (1)	10.00			
M-L	0 / 58	-18.5	-18.5 0.09 (1)	10.00			
L-K	0 / 58	-18.5	-18.5 0.09 (1)	10.00			
K-J	0 / 140	-18.5	-18.5 0.09 (1)	10.00			
J-I	0 / 22	-18.5	-18.5 0.09 (1)	10.00			
I-H	0 / 22	-18.5	-18.5 0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(58 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (C-P), BC=0.19/1.00 (N-O), WB=0.17/1.00 (D-M), SS=0.23/1.00 (B-O)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

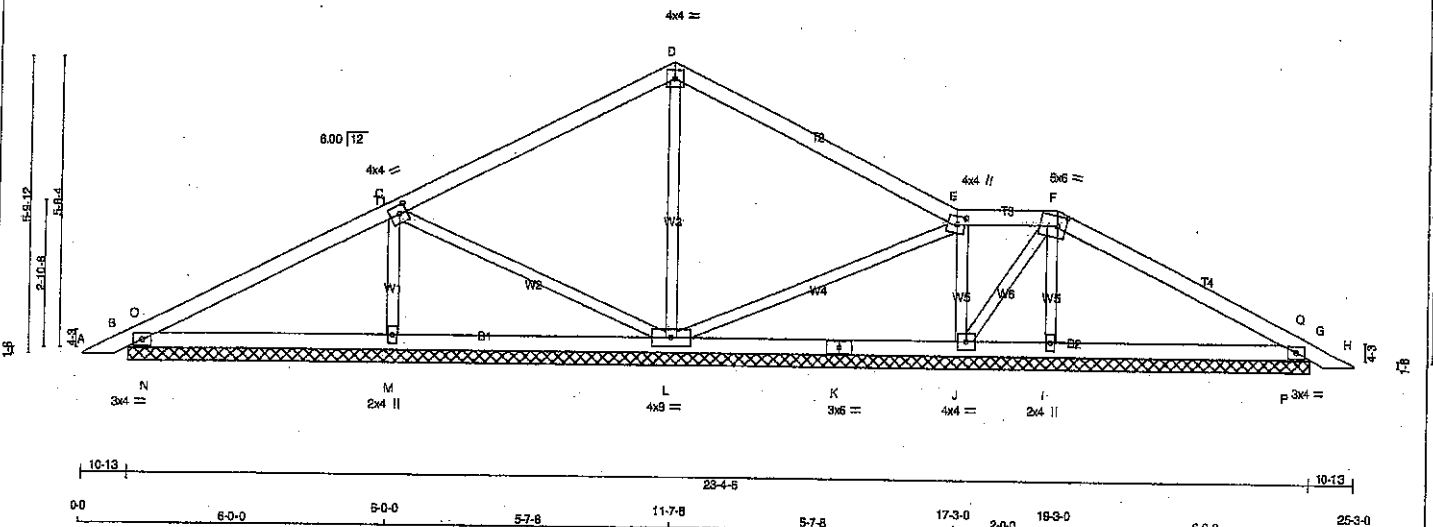
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.59 (G) (INPUT = 0.90)
JSI METAL = 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-2124020



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - K	2x4	DRY	No.2
K - S	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMB1-W	MT20	4.0	4.0	2.00 1.75
D	TTW-p	MT20	4.0	4.0	
E	TTWW+m	MT20	4.0	4.0	2.00 1.75
F	TTWW-m	MT20	5.0	6.0	2.25 2.00
G	TMB1-I	MT20	3.0	4.0	
I	BMW1-w	MT20	2.0	4.0	
J	BMW1-I	MT20	2.0	4.0	
K	BS-I	MT20	3.0	6.0	
L	BMWW1-I	MT20	4.0	6.0	
M	BMW1-w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UP/LIFT		
B	348	0	348	0	23-4 (9-4-123-4-6	
L	554	0	554	0	23-4 (9-4-123-4-6	
I	311	0	311	0	23-4 (9-4-123-4-6	
J	469	0	469	0	23-4 (9-4-123-4-6	
M	652	0	652	0	23-4 (9-4-123-4-6	
G	396	0	396	0	23-4 (9-4-123-4-6	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	244	171 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
L	394	249 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
I	221	140 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
J	331	221 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
M	481	305 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0
G	278	195 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L, I, J, M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LC)	
FR-TO				FR-TO			
A-B	0 / 16	-91.8	-91.8 0.05 (1)	L-D	-409 / 0	0.20 (1)	
B-O	-124 / 0	-91.8	-91.8 0.04 (4)	I-F	-149 / 0	0.03 (1)	
C-C	-104 / 0	-91.8	-91.8 0.42 (1)	J-E	-385 / 0	0.06 (1)	
C-D	-77 / 0	-91.8	-91.8 0.42 (1)	L-E	-20 / 0	0.01 (1)	
D-E	-52 / 0	-91.8	-91.8 0.50 (1)	M-C	-520 / 0	0.09 (1)	
E-F	-49 / 0	-91.8	-91.8 0.06 (1)	C-L	-79 / 0	0.05 (1)	
F-Q	-137 / 0	-91.8	-91.8 0.29 (1)	J-F	-102 / 0	0.02 (1)	
Q-G	-43 / 0	-91.8	-91.8 0.10 (1)	N-O	-189 / 17	0.00 (1)	
G-H	0 / 16	-91.8	-91.8 0.05 (1)	P-Q	-423 / 0	0.00 (1)	
B-N	0 / 118	-18.5	-18.5 0.13 (1)				
N-M	0 / 118	-18.5	-18.5 0.14 (1)				
M-L	0 / 118	-18.5	-18.5 0.13 (4)				
L-K	0 / 64	-18.5	-18.5 0.13 (4)				
K-J	0 / 64	-18.5	-18.5 0.13 (4)				
J-I	0 / 110	-18.5	-18.5 0.16 (1)				
I-P	0 / 116	-18.5	-18.5 0.25 (1)				
P-G	0 / 116	-18.5	-18.5 0.25 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.50/1.00 (D-E-I), BC=0.25/1.00 (I-P-I), WB=0.20/1.00 (D-L-I), SS=0.32/1.00 (G-P-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (E) (INPUT = 0.90)
JSI METAL= 0.16 (K) (INPUT = 1.00)

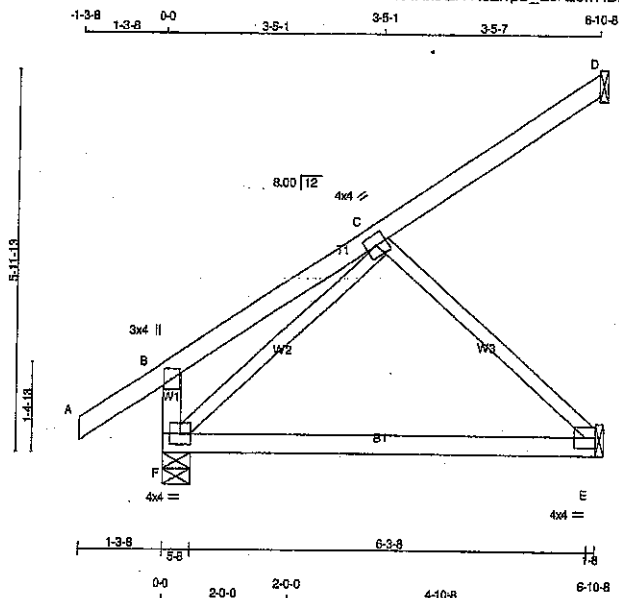


Structural component only
DWG# T-2124021

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418361	J2	3	1	GREENPARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:32.2

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVAp	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
E	BMW1-t	MT20	4.0	4.0		Edge
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT
F	502	0	5-8
D	127	0	1-8
E	254	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
F	353	245 / 0	0 / 0	0 / 0	108 / 0	0 / 0
D	87	71 / 0	0 / 0	0 / 0	17 / 0	0 / 0
E	162	108 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC1)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
F-B	-238 / 0	0.0	0.0	0.02 (1)	7.81	C-E	-278 / 0
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	F-C	-285 / 0
B-C	0 / 22	-91.8	-91.8	0.18 (1)	10.00		
C-D	-20 / 0	-91.8	-91.8	0.14 (1)	8.25		
F-E	0 / 202	-18.5	-18.5	0.26 (4)	10.00		

TOTAL WEIGHT = 3 X 28 = 83 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
BOT CH.	LL	= 6.0 PSF
	DL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = U/848 (0.10")

CSI: TC=0.18/1.00 (B-C:1), BC=0.26/1.00 (E-F:4),
WB=0.11/1.00 (C-E:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

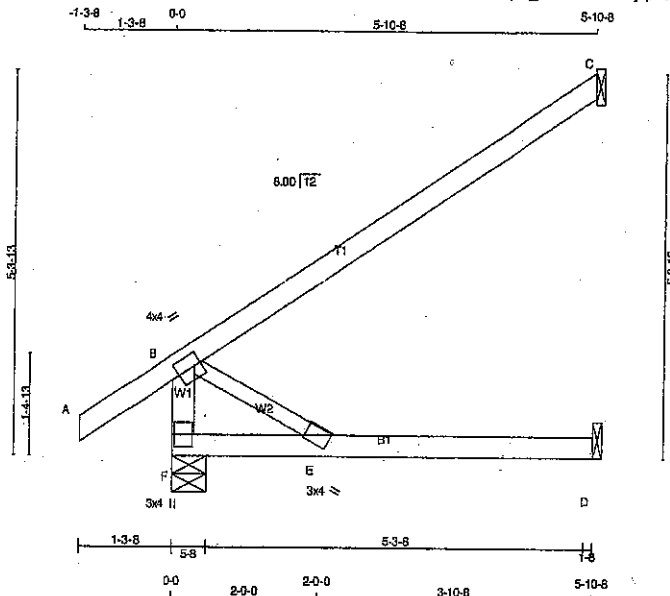
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.23 (F) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123942



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
F - B	2x4	DRY No.2
A - C	2x4	DRY No.2
F - D	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
E	BMVW-w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	450	0	450	0
C	270	0	270	0
D	54	0	61	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	316	221 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	186	150 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-395 / 0	0.0	0.0 0.04 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 35	-91.8	-91.8 0.12 (1)				
B-C	0 / 0	-91.8	-91.8 0.54 (1)				
F-E	0 / 0	-18.5	-18.5 0.17 (4)				
E-D	0 / 0	-18.5	-18.5 0.19 (4)				

TOTAL WEIGHT = 4 X 20 = 78 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/998$ (0.05")

CSI: $TC=0.54/1.00$ (B-C:1), $BC=0.19/1.00$ (D-E:4), $WB=0.00/1.00$ (B-E:1), $SSI=0.17/1.00$ (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

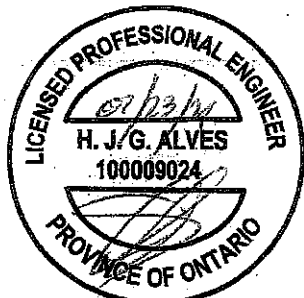
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

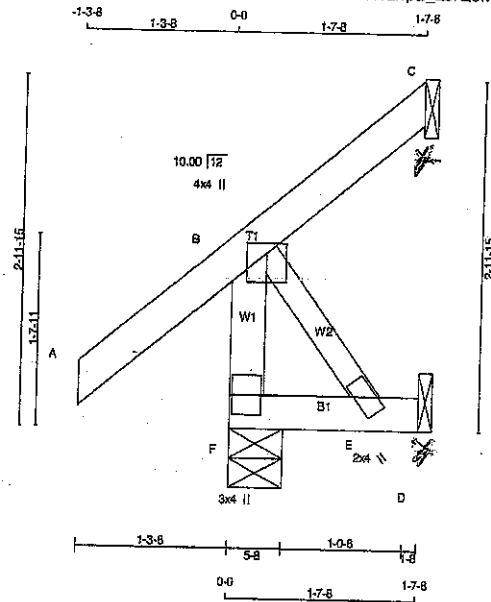
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123943

JOB NAME 418361	TRUSS NAME J4	QUANTITY 3	PLY. 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 15:59:40 2021 Page 1	



Scale = 1:18.0

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG
		VERT	DOWN	IN-SX	IN-SX
F	SPF	271	0	5-8	5-8
C	SPF	20	0	1-8	1-8
D	SPF	15	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D.

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
F	188	143 / 0	0 / 0	0 / 0	0 / 0
C	14	11 / -30	0 / 0	0 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-B	-256 / 0	0.0	0.0 0.03 (1)	8-E	0 / 0
A-B	0 / 41	-91.8	-91.8 0.13 (1)		
B-C	-35 / 0	-91.8	-91.8 0.12 (1)		
F-E	0 / 0	-18.5	-18.5 0.01 (4)		
E-D	0 / 0	-18.5	-18.5 0.01 (4)		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 9 = 27 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.
(56 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.19/1.00 (A-B:1), BC=0.01/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	(PL)
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

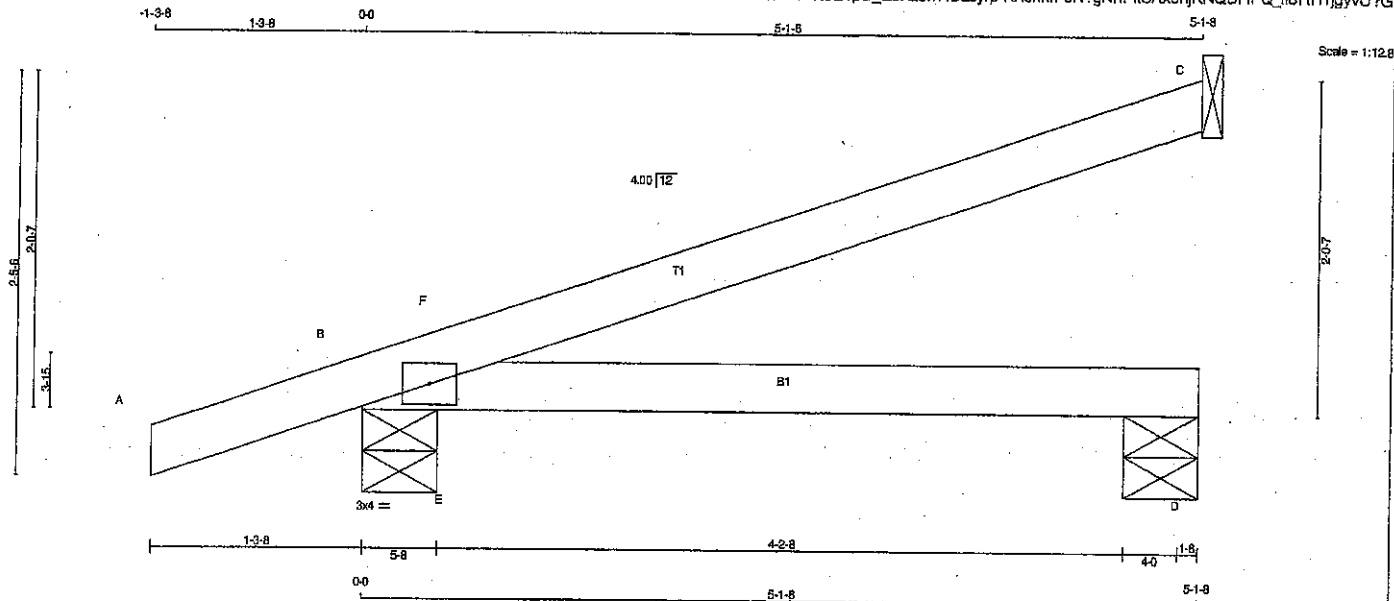
JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123944

JOB NAME 418361	TRUSS NAME J5	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 14 = 41 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

B - D 2x4 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

DESCR.

SPF

SPF

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0	

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	205	0	205	0	1-8	1-8
B	405	0	405	0	5-8	5-8
D	78	0	78	0	5-8	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE MAX/MIN COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
C	142	110/0	0/0	0/0	0/0	32/0	0/0
B	284	200/0	0/0	0/0	0/0	65/0	0/0
D	58	21/0	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/18	-91.8 -91.8	0.11 (1)	E-F	257/10	0.00 (1)	
B-F	-17/7	-91.8 -91.8	0.06 (4)				
F-C	0/2	-91.8 -91.8	0.31 (1)				
B-E	0/0	-18.5 -18.5	0.22 (1)				
E-D	0/0	-18.5 -18.5	0.22 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")

ALLOWABLE DEFL.(TL) = $L/360$ (0.19")

CALCULATED VERT. DEFL.(TL) = $L/728$ (0.08")

CSI: TC=0.31/1.00 (C-F:1), BC=0.22/1.00 (D-E:1),

WB=0.00/1.00 (E-F:1), SS=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

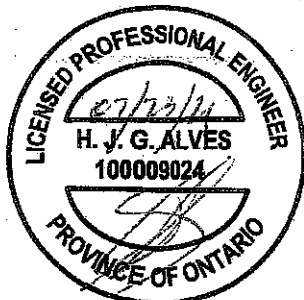
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JST GRIP= 0.26 (B) (INPUT = 0.90)

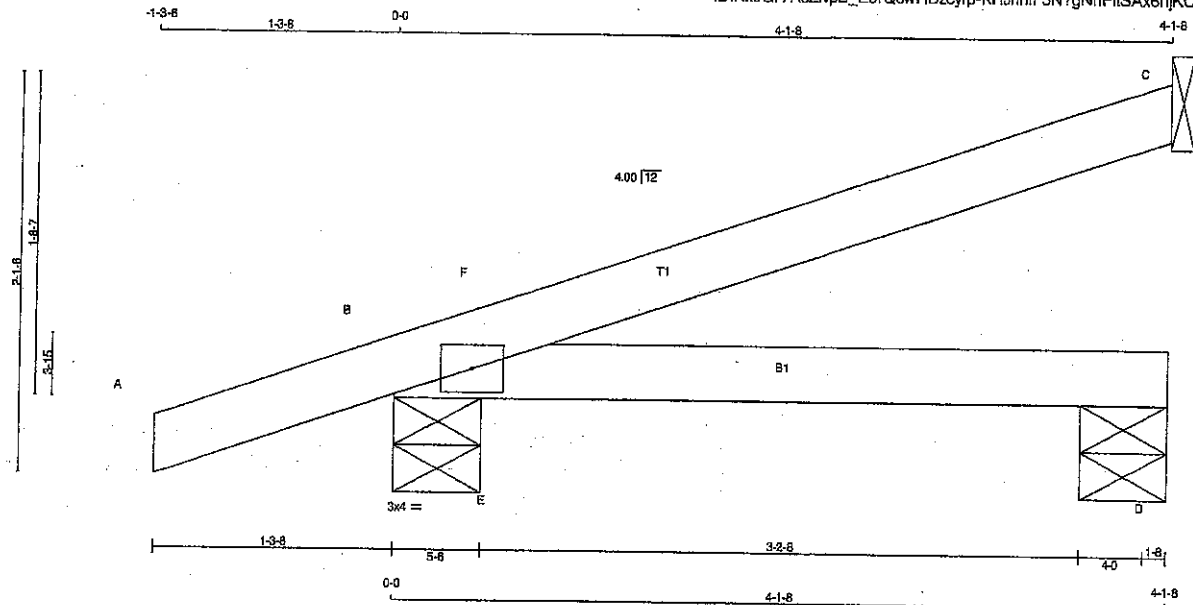
JST METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123945

JOB NAME 418361	TRUSS NAME J6	QUANTITY 7	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Nx0GIYRcZvpB_E9rQcwHDzcyrp-KHJnnfF5N?gNhFitSAx6hJKOBDIYQ_18RH1jgyvU?G



Scale = 1:11.2

TOTAL WEIGHT = 7 X 11 = 79 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-H	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
C	183	0	183	0	1-8	1-8
B	350	0	350	0	5-8	5-8
D	64	0	64	0	5-8	5-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	113	88 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	245	174 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
D	48	18 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	WEBS		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
				FR-TO	FROM-TO			
A-B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	E-F	-175 / 7	0.00 (1)
B-F	-13 / 0	-91.8	-91.8	0.04 (4)	6.25			
F-C	0 / 2	-91.8	-91.8	0.20 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.15 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.15 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.20/1.00 (C-F:1), BC=0.15/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.15/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

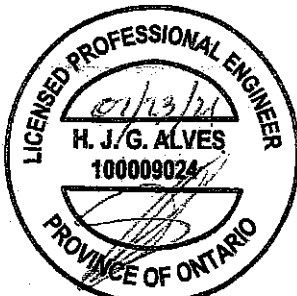
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

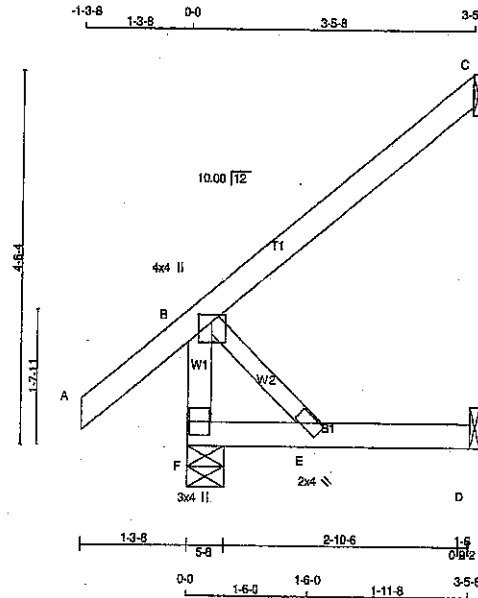
JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123946

JOB NAME 418362	TRUSS NAME J30	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 18:43:20 2021 Page 1
ID:Nx0GIYRcZlvpB_E9rQcwHDZoyrp-973eSAxvbenOk0ENJQGL?i_QyBcZT39OPC68NvYtML



Scale = 1/25.6

TOTAL WEIGHT = 4 X 14 = 57 LB

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
F	318	0	318	0
C	159	0	159	0
D	32	0	32	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	222	159 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
C	109	88 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MEMB.
F-B	-286 / 0	0.0	0.0 (0.03)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 41	-91.8	-91.8 (0.14)	10.00			
B-C	0 / 0	-91.8	-91.8 (0.19)	10.00			
F-E	0 / 0	-18.5	-18.5 (0.06)	10.00			
E-D	0 / 0	-18.5	-18.5 (0.06)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCSC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.19/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

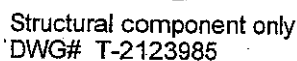
JSI GRIP = 0.21 (B) (INPUT = 0.80)
JSI METAL = 0.09 (B) (INPUT = 1.00)



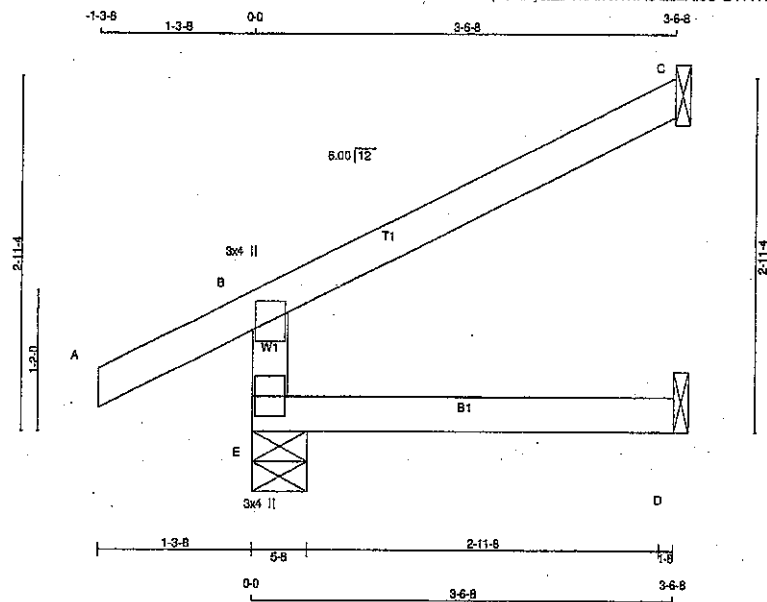
Structural component only
DWG# T-2123984



JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



JOB NAME 418363	TRUSS NAME J50	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 5.420 5 Jan 21 2021 Mitek Industries, Inc. Thu Jul 22 17:32:50 2021 Page 1 ID:Oikq5APjQaBbkRjRWnRoQszKI98-BWWequt9DqMi932MTilJjWjAshRIJLTLTGNDyvSdx			
		TRUSS DESC.			



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMVt+p	MT20	3.0	4.0

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	365	0	365	0	5-8
C	122	0	122	0	1-8
D	28	0	31	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LOASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE		
E	255	163 / 0	0 / 0	0 / 0	73 / 0	0 / 0
C	84	68 / 0	0 / 0	0 / 0	16 / 0	0 / 0
D	22	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
E-B	-327 / 0	E-B	7.81
A-B	0 / 28	A-B	10.00
B-C	-18 / 0	B-C	6.25
E-D	0 / 0	E-D	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.05/1.00 (D-E:4), WB=0.00/1.00 (W:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY) (PSI)	SHEAR SECTION (PLI)
MT20	650 371 1747 788 1987 1873

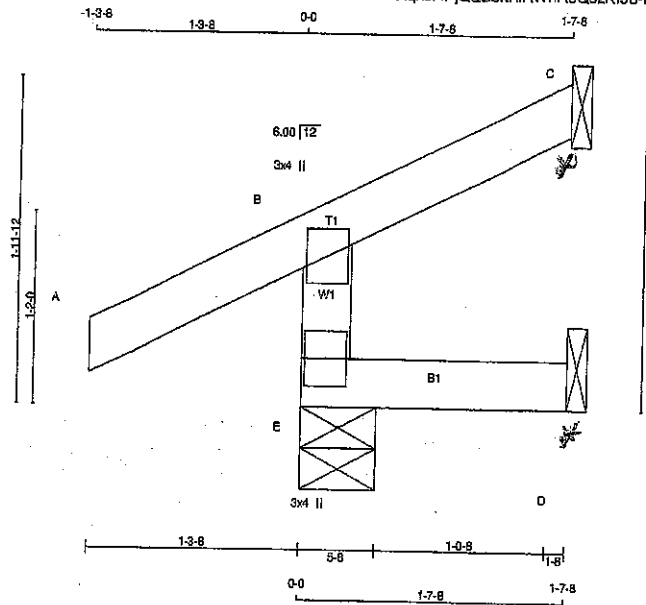
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (S) (INPUT = 1.00)



Structural component only
DWG# T-2124013



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
E	266	266	0	5-8
C	34	34	0	5-8
D	4	14	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE WIND	DEAD	SOIL
E	185	140 / 0	0 / 0	0 / 0	0 / 0
C	23	18 / -20	0 / 0	0 / 0	0 / 0
D	4	0 / -9	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. GSI (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED GSI (LC)
		FROM	TO				
E-B	-240 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-18 / 0	-91.8	-91.8	0.09 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 7 = 26 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), SC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)

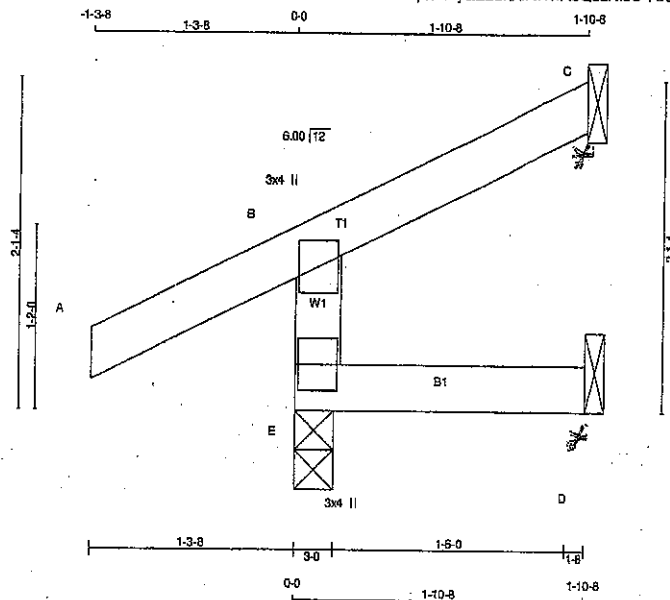
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124014

JOB NAME 418363	TRUSS NAME J52	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 3.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:32:52 2021 Page 1	

ID:Oiqk5APjQaBbkRIRWnRoQszK198-7uePCPv7hq44XTDQTukDokOuR_XLvcpon8NR6yvSdv



Scale = 1:13.4

TOTAL WEIGHT = 7 lb

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
273	0	273	0	3-0
50	0	50	0	-20
8	0	17	0	-2

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
COMBINED	SNOW	LIVE				
E	190	143 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	35	27 / -17	0 / 0	0 / 0	7 / 0	0 / 0
D	8	0 / -8	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE	FACTORED VERT. LOAD	LC1	MAX. CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE	MAX. CS1 (LC)
MEMB.	(LBS)	(PLF)		(LC)			MEMB.	(LBS)	(LC)
E-B	-247 / 0	0.0	0.0	0.04 (5)	7.81				
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00				
B-C	-16 / 0	-91.8	-91.8	0.09 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIG 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 31.3 P.S.F. G.S.I., PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CS1: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (A-B:0), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	850	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

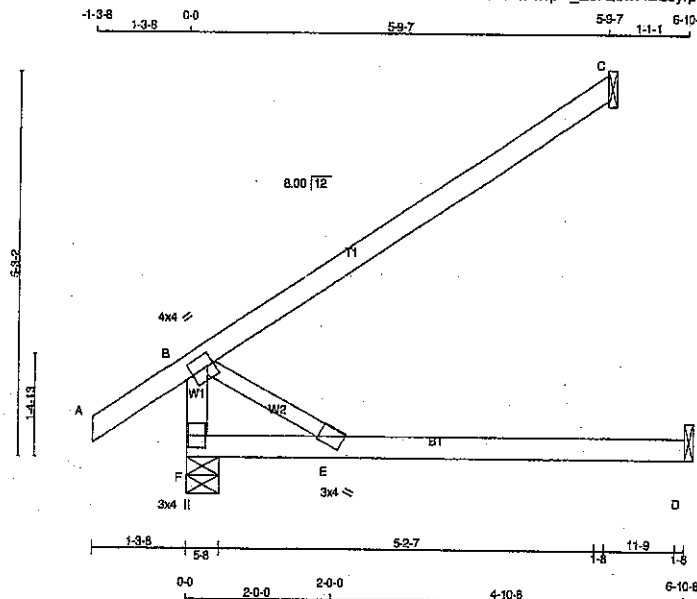
JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124015

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418361	C1	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1/2\"/>

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.00
E BMW+w	MT20	3.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	455	0	455	0	0	5-8	5-8
C	265	0	265	0	0	1-8	1-8
D	64	0	71	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	320	218 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
C	183	148 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	51	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LG1 CS1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)		
FR-TO		FROM TO			FR-TO				
F-B	-391 / 0	0.0	0.0 0.04 (1)	7.81	B-E	0 / 0	0.00 (1)		
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00					
B-C	0 / 0	-91.8	-91.8 0.52 (1)	10.00					
F-E	0 / 0	-18.5	-18.5 0.21 (4)	10.00					
E-D	0 / 0	-18.5	-18.5 0.25 (4)	10.00					

TOTAL WEIGHT = 21 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIG 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/819$ (0.10")

CS1: TO=0.52/1.00 (B-C:1), BC=0.28/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

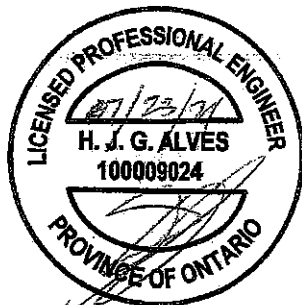
NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

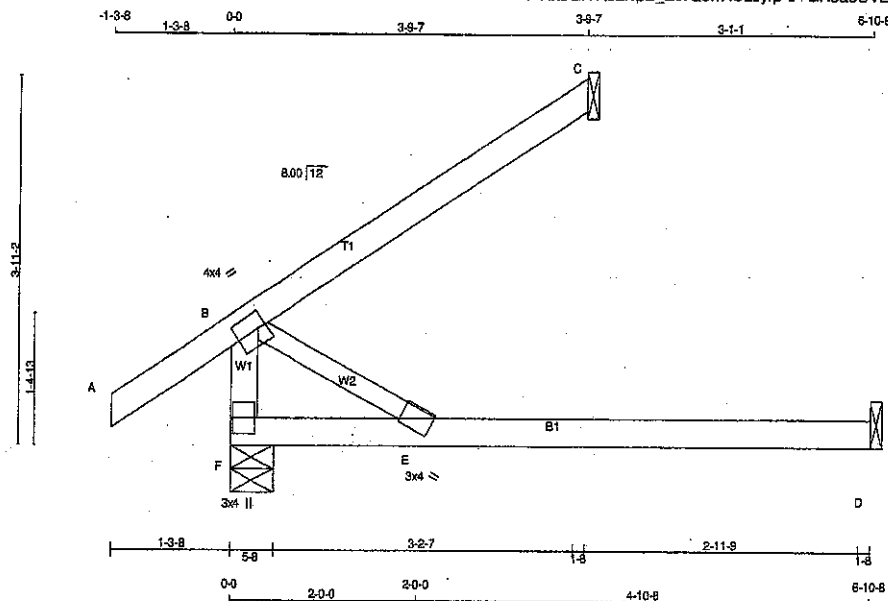


Structural component only
DWG# T-2123933

JOB NAME 418361	TRUSS NAME C2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1:22.5

TOTAL WEIGHT = 18 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	4.0	2.00	1.00
E BMW-w	MT20	3.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	383	0	383	0
C	174	0	174	0
D	54	0	71	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	257	167 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	51	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO	F-B	-300 / 0	0.0	0.0	0.03 (1)	7.81	FR-TO	B-E	0 / 0	0.00 (1)
	A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00				
	B-C	0 / 0	-91.8	-91.8	0.22 (1)	10.00				
	F-E	0 / 0	-18.5	-18.5	0.21 (4)	10.00				
	E-D	0 / 0	-18.5	-18.5	0.28 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/819$ (0.10")

CSI: TC=0.22/1.00 (B-C:1), BC=0.26/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

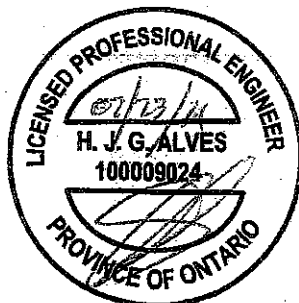
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

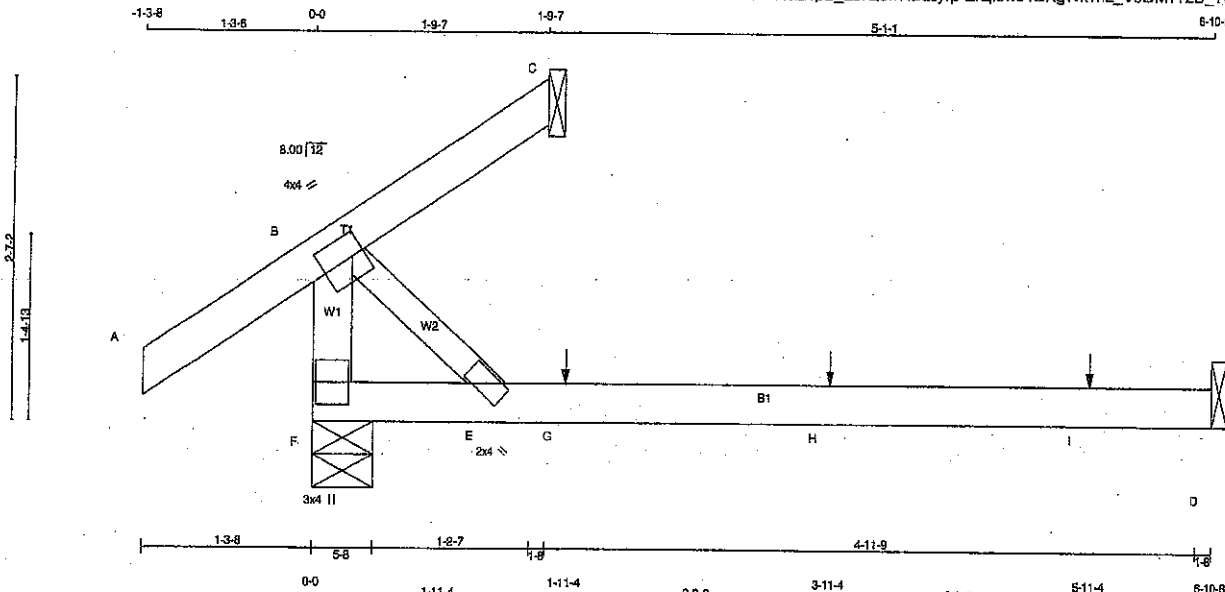
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123934



Scale: 3/4"=1'

TOTAL WEIGHT = 15 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY, SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMVW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
F	320	320	0	5-8
C	34	34	0	1-8
D	64	71	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	227	143 / 0	0 / 0	0 / 0	84 / 0	0 / 0
C	23	18 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	51	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LOAD (LC1)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
F-B	-258 / 0	0.0	0.0 (0.03 (1))	B-E	0 / 0	0.00 (1)	7.81
A-B	0 / 35	-91.8	-91.8 (0.13 (1))				10.00
B-C	-27 / 0	-91.8	-91.8 (0.10 (1))				8.25
F-E	0 / 0	-18.5	-18.5 (0.16 (4))				10.00
E-G	0 / 0	-18.5	-18.5 (0.28 (4))				10.00
G-H	0 / 0	-18.5	-18.5 (0.26 (4))				10.00
H-I	0 / 0	-18.5	-18.5 (0.26 (4))				10.00
I-D	0 / 0	-18.5	-18.5 (0.26 (4))				10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	1	FRONT	VERT	TOTAL	---	C1
I	5-11-4	1	1	1	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/819 (0.10")

CSI: TC=0.12/1.00 (A-B:1), BC=0.28/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DCL LUMBER=0.93 NAIL=0.93 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

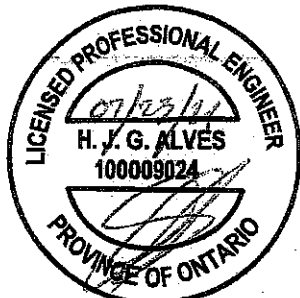
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

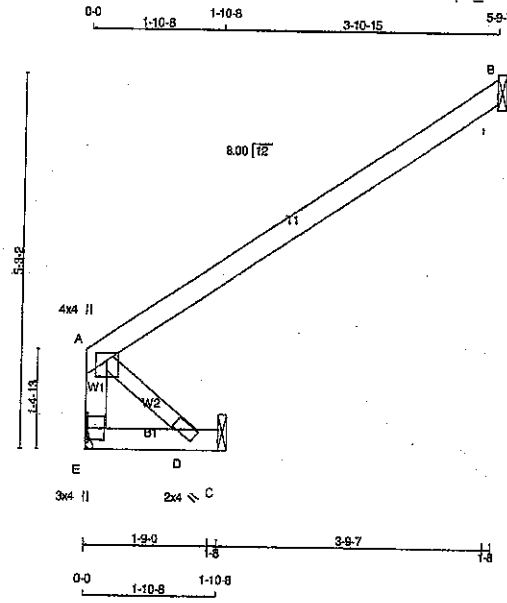
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123935

JOB NAME 418361	TRUSS NAME C4	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
D	BMW+w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	283	0	283	0	0	0
B	265	0	265	0	1-8	1-8
C	17	0	19	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM LIVE			
E	197	148 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
B	183	148 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
E-A	-285 / 0	0.0	0.0	0.03 (1)
A-B	0 / 0	-91.8	-91.8	0.52 (1)
E-D	0 / 0	-18.5	-18.5	0.02 (4)
D-C	0 / 0	-18.5	-18.5	0.01 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.52/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (A-D:1), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

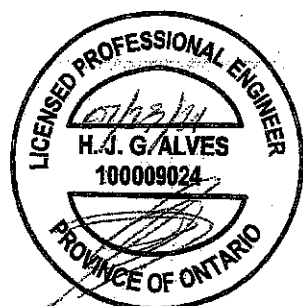
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

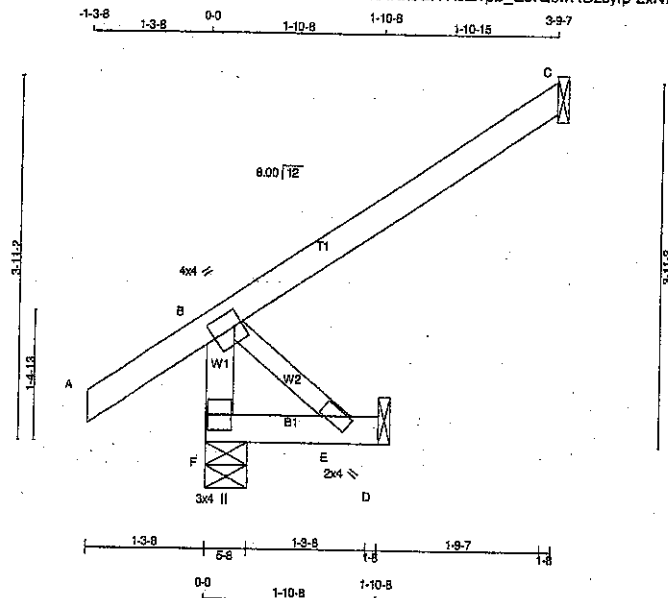
JSI GRIP= 0.18 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)



Structural component only
DWG# T-2123936

Tamarack Roof Truss, Burlington

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Scale = 1/22.8

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMV-w	MT20	2.0	4.0		
F	BMV1-p	MT20	3.0	4.0		

NOTES:

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	317	317	0	5-8
C	174	174	0	1-8
D	17	19	0	1-8

SEE MITEK STANDARD DETAIL B9779H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	MAX MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
F	220	167 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
F-B	-300 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.13 (5)	10.00			
B-C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 12 = 23 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2015
- PART 9 OF OBC 2012 (2015 AMENDMENT)
- CSA 066-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/960 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TO=0.22/1.00 (B-C:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371 1747 789 1987 1873	

PLATE PLACEMENT TOL = 0.260 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

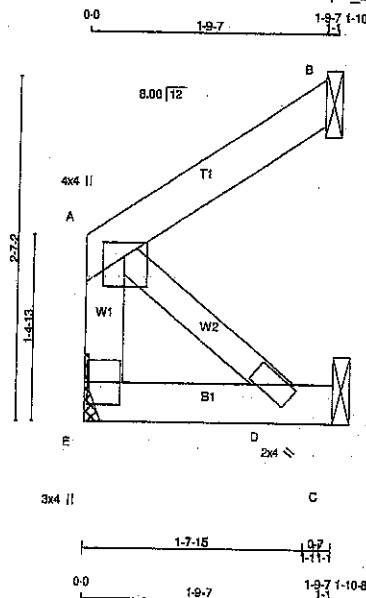


Structural component only
DWG# T-2123937

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
418361	C6	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale: 3/4"=1'



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	99	0	99	0
B	82	0	82	0
C	17	0	17	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	70	46/0	0/0	0/0	0/0	25/0	0/0
B	56	46/0	0/0	0/0	0/0	11/0	0/0
C	14	0/0	0/0	0/0	0/0	14/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO								FR-TO			
E-A	-82/0	0.0	0.0	0.01 (1)	7.81	A-D	0/0	0.00 (1)			
A-B	0/0	-91.6	-91.9	0.05 (1)	10.00						
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00						
D-C	0/0	-18.5	-18.5	0.01 (4)	10.00						

TOTAL WEIGHT = 2 X 7 = 14 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015.

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.05/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (A-D:1), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	850	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

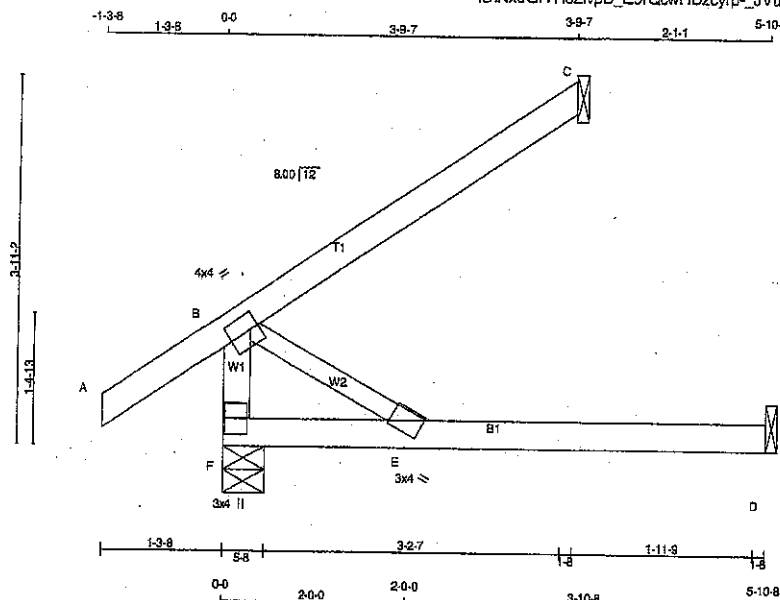
JSI GRIP= 0.05 (A) (INPUT = 0.80)
JSI METAL= 0.02 (A) (INPUT = 1.00)



Structural component only
DWG# T-2123938

JOB NAME 418361	TRUSS NAME C7	QUANTITY 1	PL 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:22.5

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	No.2	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW-w	MT20	3.0	4.0		
F	BMV1-p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
F	354	354	0	0
C	174	174	0	0
D	54	61	0	0

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	250	167 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (L/C)	
F-B	-300 / 0	0.0	0.0 (1)	F-E	0 / 0	0.00 (1)	
A-B	0 / 35	-91.8	-91.8 (1)				
B-C	0 / 0	-91.8	-91.8 (1)				
F-E	0 / 0	-18.5	-18.5 (4)				
E-D	0 / 0	-18.5	-18.5 (4)				

TOTAL WEIGHT = 17 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL	6.0	PSF
BOT CH.	LL = 0.0	PSF
DL	7.4	PSF
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

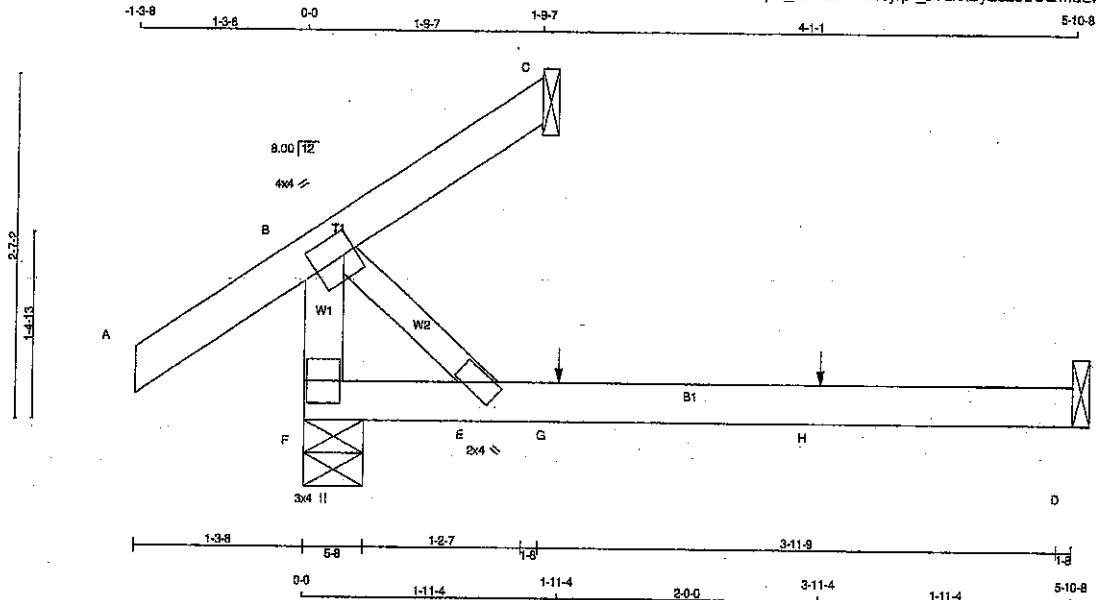
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123939

JOB NAME 418361	TRUSS NAME C8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale: 3/4"=1'

TOTAL WEIGHT = 13 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	NO.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMW-1	MT20	4.0	4.0	2.00	1.00
E	BMW+w	MT20	2.0	4.0		
F	BMW+P	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	311	0	311	0	0	5-8	5-8
C	34	0	34	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
F	220	143 / 0
C	23	19 / 0
D	43	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT - OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LO1 MAX. CSI (LC)	UNBRAC LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
F-B		-256 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B		0 / 35	-91.8	-91.8	0.13 (1)	10.00			
B-C		-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F-E		0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E-G		0 / 0	-18.5	-18.5	0.19 (4)	10.00			
G-H		0 / 0	-18.5	-18.5	0.19 (4)	10.00			
H-D		0 / 0	-18.5	-18.5	0.19 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	1	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.05")

CSI: TC=0.13/1.00 (A-B:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=0.88 NAIL=0.98 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

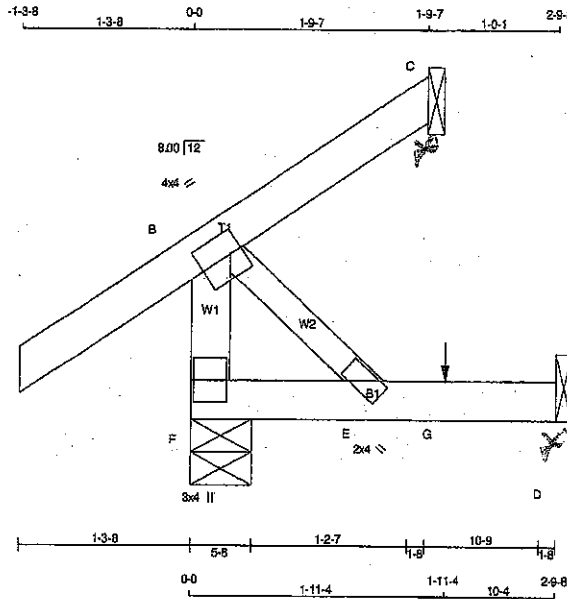
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123940



Scale: 3/4"=1'

TOTAL WEIGHT = 2 X 10 = 20 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER, No.2

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	4.0	4.0	2.00	1.00
E BMVW-w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	282	282	5-8	5-8
C	34	34	1-8	1-8
D	28	28	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	197	143 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (6)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSF (LC)	MAX. FACTORED UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSF (LC)
FR-TO		FROM TO			FR-TO		
F-B	-258 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.14 (5)			
B-C	-27 / 0	-91.8	-91.8	0.13 (5)			
F-E	0 / 0	-18.5	-18.5	0.04 (4)			
E-G	0 / 0	-18.5	-18.5	0.04 (4)			
G-D	0 / 0	-18.5	-18.5	0.04 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIFL.	TYPE	HEEL	CONN.
G	1-11-4	-0	-0	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 8.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 31.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.04/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

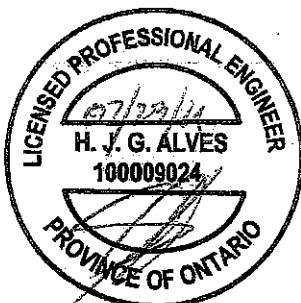
NAIL VALUES

PLATE GRIP (DRY)	SHEAR SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

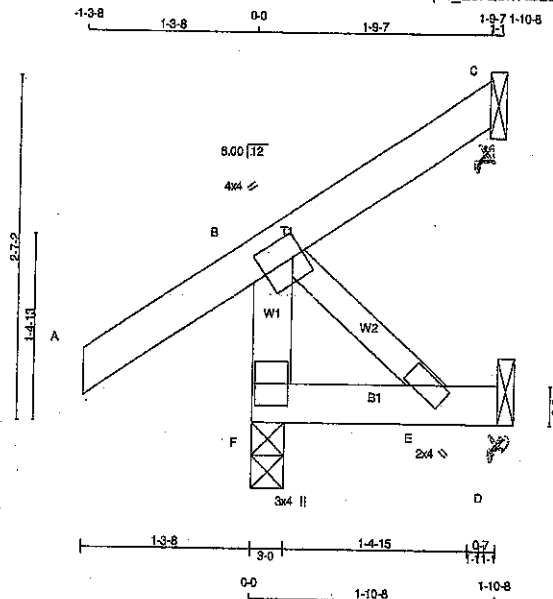
JSI GRIP = 0.16 (B) (INPUT = 0.80)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123982

JOB NAME 418362	TRUSS NAME C31	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale: 3/4"=1'

LUMBER

N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	4.0	4.0	2.00	1.00
E BMVW-w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	274	0	274	0	3-0	3-0
C	34	0	34	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	180	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-256 / 0	0.0	0.0 (1)	B-E	0 / 0	0.00 (1)	
A-B	0 / 35	-91.8	-91.8 (1)				
B-C	-27 / 0	-91.8	-91.8 (1)				
F-E	0 / 0	-18.5	-18.5 (4)				
E-D	0 / 0	-18.5	-18.5 (4)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 9 = 18 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)

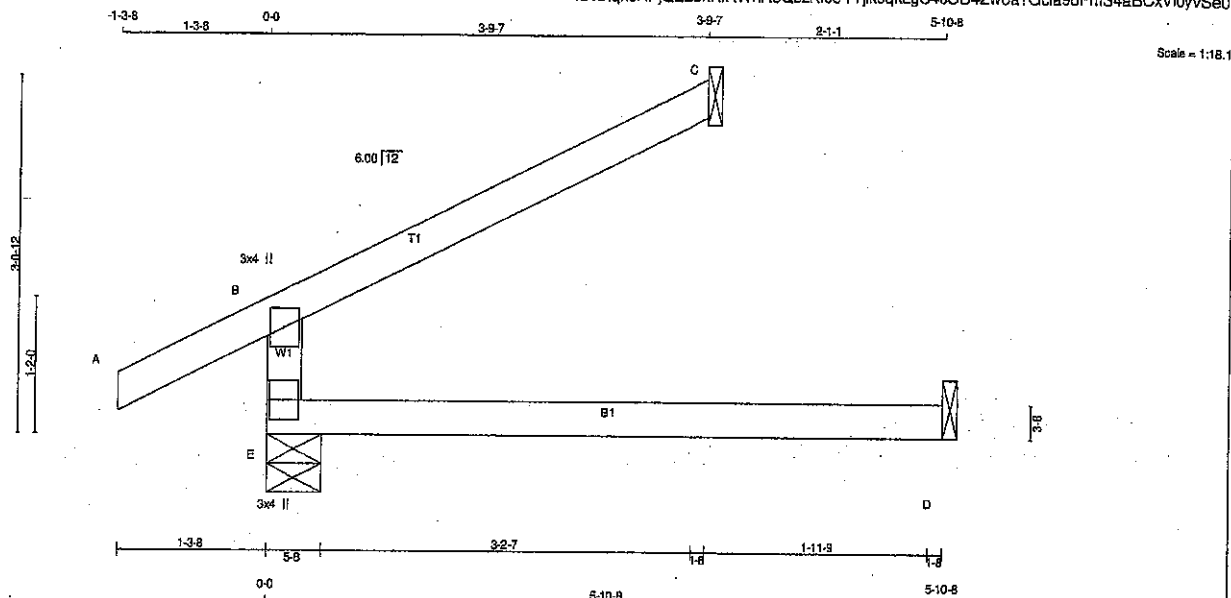
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2123983

JOB NAME 418363	TRUSS NAME C50	QUANTITY 4	PLY 1	JOB DESC GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES:

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	405	0	405	0	0
C	130	0	130	0	0
D	45	0	50	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	288	190 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
E-B	-342 / 0	0.0	0.0 0.13 (4)
A-B	0 / 28	-91.8	-91.8 0.12 (1)
B-C	-19 / 0	-91.8	-91.8 0.22 (1)
E-D	0 / 0	-18.5	-18.5 0.13 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: $TC=0.22/1.00$ (B-C:1), $BC=0.13/1.00$ (D-E:4),
 $WB=0.00/1.00$ (n/a:0), $SSI=0.15/1.00$ (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

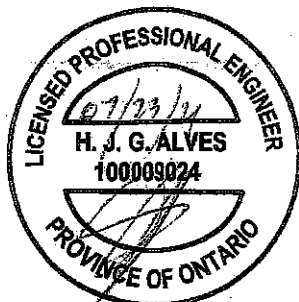
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

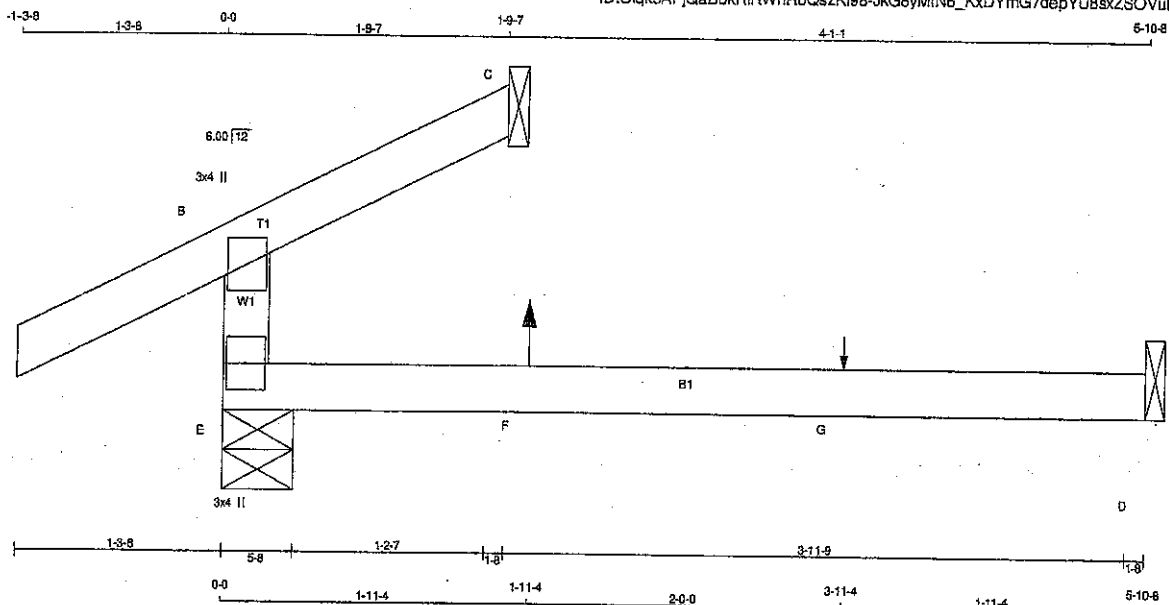
JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124007

JOB NAME 418363	TRUSS NAME C51	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER

N. L. & A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV+p	MT20	3.0	4.0	

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
E	284	0	284	0
C	63	0	63	0
D	44	0	52	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND
E	200	137/0	0/0	0/0	0/0
C	46	21/0	0/0	0/0	0/0
D	35	0/3	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
E-B	-227/0	0.0	0.0	0.11 (4)	7.81
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00
B-C	-9/9	-91.8	-91.8	0.08 (4)	10.00
E-F	0/0	-18.5	-18.5	0.14 (4)	10.00
F-G	0/0	-18.5	-18.5	0.14 (4)	10.00
G-D	0/0	-18.5	-18.5	0.14 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	BACK	VERT	TOTAL	—	C1
G	3-11-4	1	1	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 12 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
SOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (W:0), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

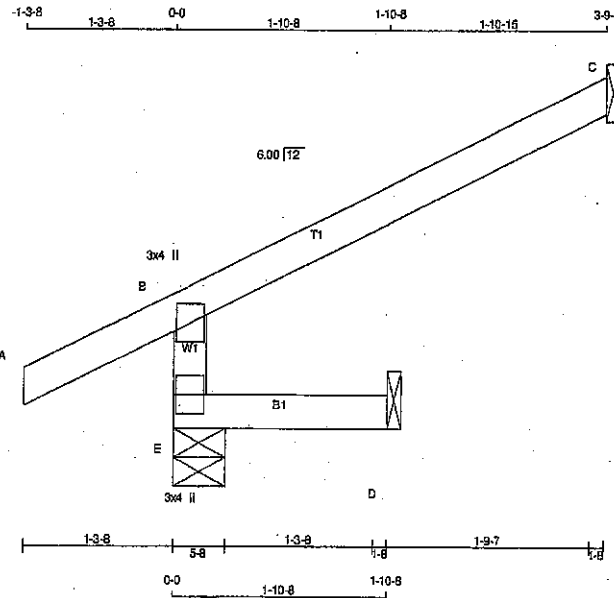
PLATE ROTATION TOL. = 5.0 Deg.

JST GRIP = 0.09 (E) (INPUT = 0.30)

JST METAL = 0.06 (E) (INPUT = 1.00)



Structural component only
DWG# T-2124008



Scale = 1:18.3

TOTAL WEIGHT = 4 X 10 = 38 lb

LUMBER

N. L. G. A. RULES	SIZE	DRY	No.2
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV+p	MT20	3.0	4.0		

NOTES - (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	361	0	381	0	0	5-8	5-8
C	130	0	130	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	180 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
E-B	-342 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = 1/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TO=0.22/1.00 (B-C-1), BC=0.02/1.00 (D-E-4),
WB=0.00/1.00 (v-a-0), SSI=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

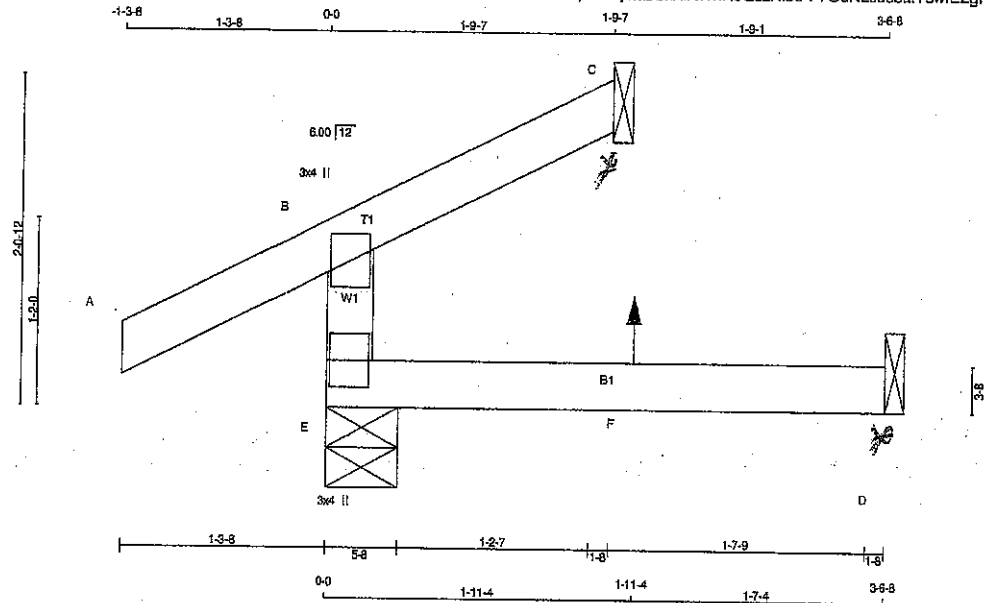
JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124009

JOB NAME 418363	TRUSS NAME C54	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Jul 22 17:32:48 2021 Page 1
ID:Oiqk5APjQaBbkRIRWnRoQszKI98-F7OuN2sdebatTswE2gHdvEBDMABzop0tAAAILySdz



Scale = 1:13.2

TOTAL WEIGHT = 2 X 9 = 18 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
E TMV+p MT20 3.0 4.0
E BMV1+p MT20 3.0 4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
BEARINGS		GROSS REACTION	GROSS REACTION	BRG	BRG
		VERT	DOWN	UPLIFT	IN-SX
JT		277	0	0	5-8
E		47	0	0	5-8
C		47	0	-21	1-8
D		23	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS	1ST LOASE	MAX/MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT	194	138 / 0 0 / 0 0 / 0 0 / 0 58 / 0
E	34	22 / -20 0 / 0 0 / 0 0 / 0 11 / 0
C	19	0 / -5 0 / 0 0 / 0 0 / 0 23 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC	MEMB.	FORCE MAX (LBS) CSI (LC)
FR-TO		FROM TO LENGTH FR-TO		
E-B	-242 / 0	0.0 0.0 0.04 (5) 7.81		
A-B	0 / 28	-91.8 -91.8 0.13 (5) 10.00		
B-C	-16 / 0	-91.8 -91.8 0.09 (6) 8.25		
E-F	0 / 0	-18.5 -18.5 0.05 (4) 10.00		
F-D	0 / 0	-18.5 -18.5 0.05 (4) 10.00		

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
	F	1-11-4	5	1	8	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.
CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.05/1.00 (D-E-4), WB=0.00/1.00 (N/A-0), SS=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

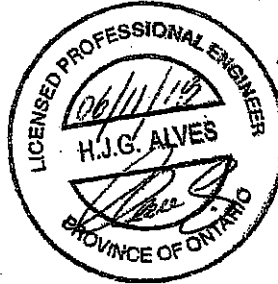
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2124011



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate Institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering and General Safety notes.

T-1900219

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

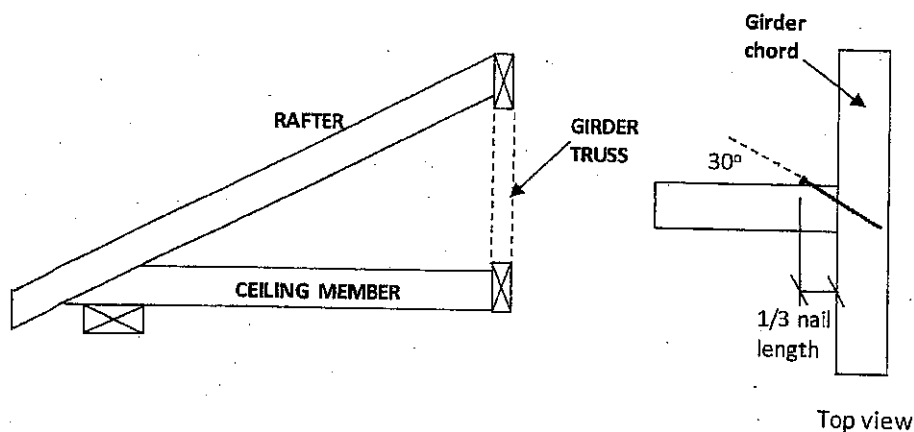


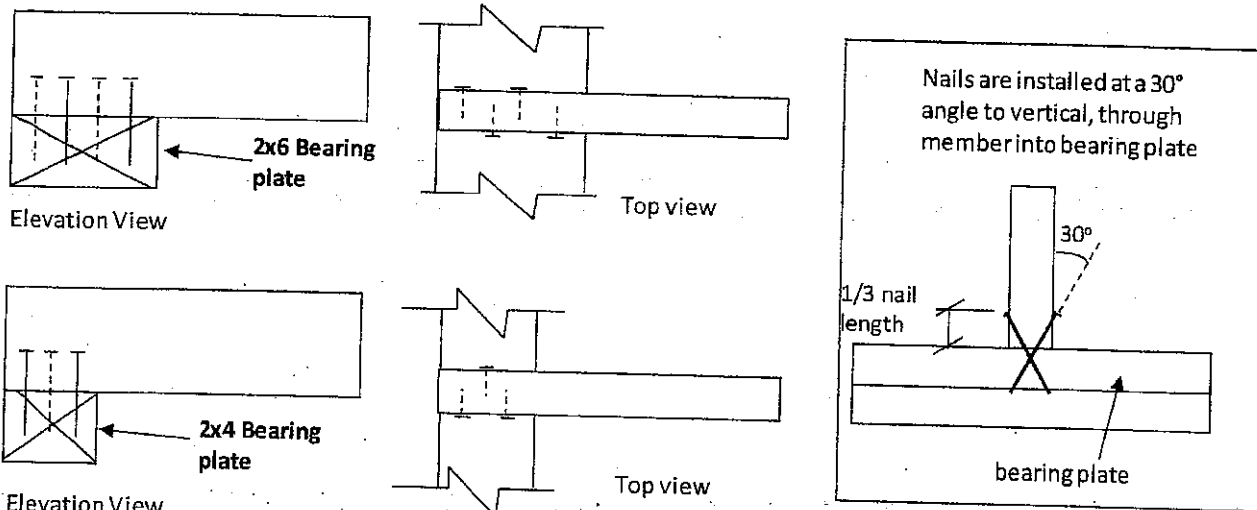
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

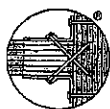
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

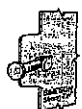
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

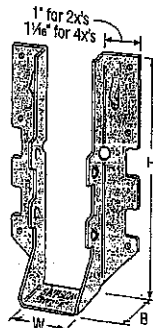
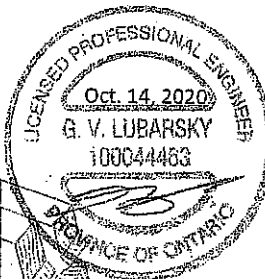
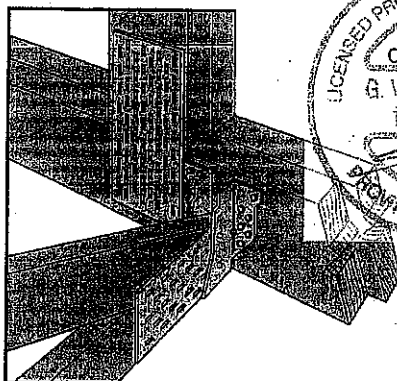


Double-Shear
Nailing
Side View;
Do not
bend tab

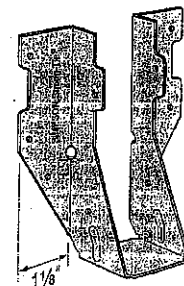


Dome Double-Shear
Nailing
Side View
(available on
some models)

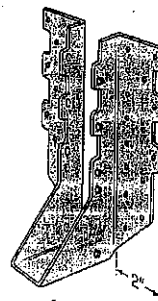
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



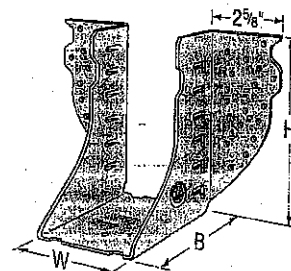
✓ LUS28



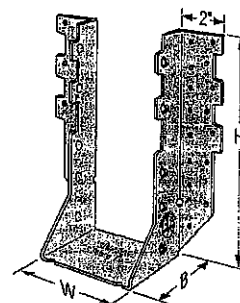
LU26L



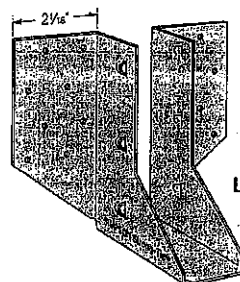
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

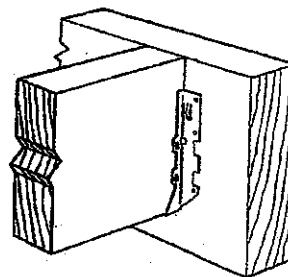
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

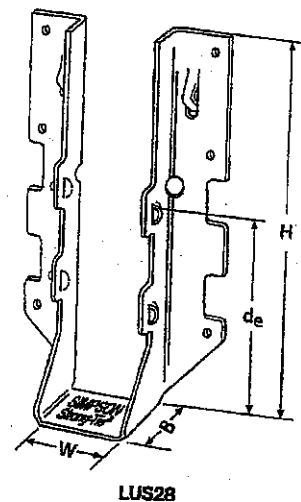
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

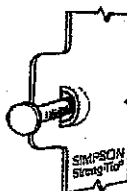


Typical LUS Installation



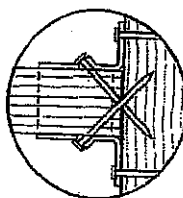
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

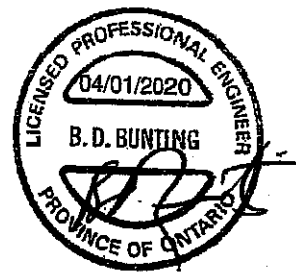


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

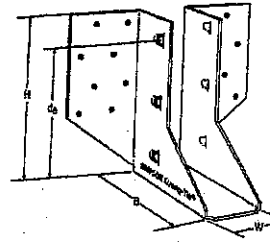
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

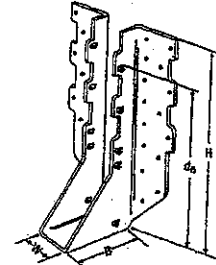
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

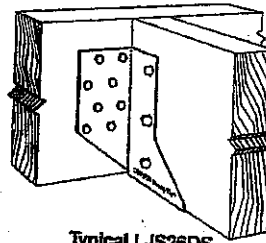
- See current catalogue for options



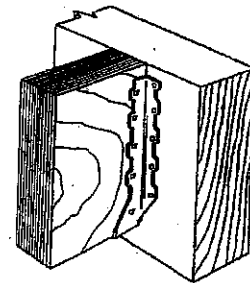
LJS26DS



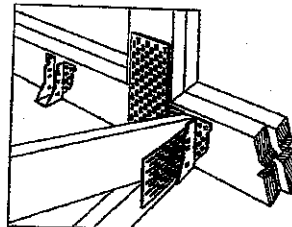
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



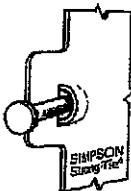
Typical HUS
Installation



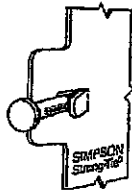
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.	Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.
LJS26DS	18	1 1/8	5	3 1/2	4%	(16) 16d	(8) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2085	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

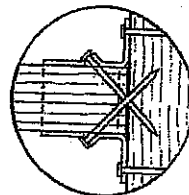
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22



(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

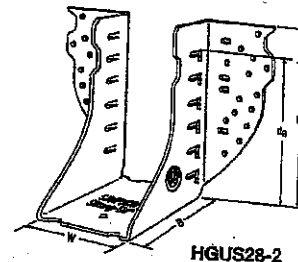
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

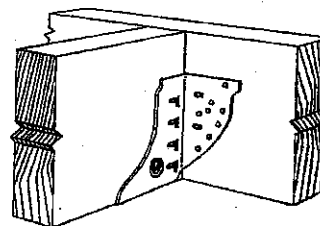
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

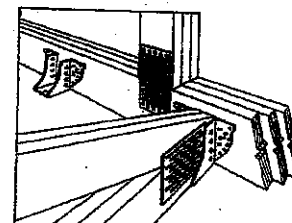
- See current catalogue for options



HGUS28-2



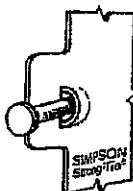
Typical HGUS Installation



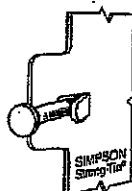
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HGUS26	12	1 1/2	5 1/2	5	4 3/8	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/2	7 1/2	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/2	9 1/2	5	7 1/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/8	9 1/8	4	8 1/4	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 1/8	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/4	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/2	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/2	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645

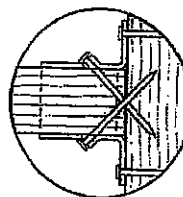
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



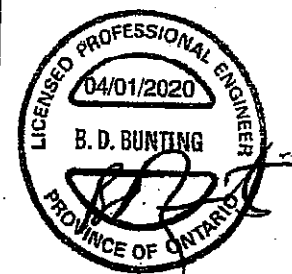
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,680



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

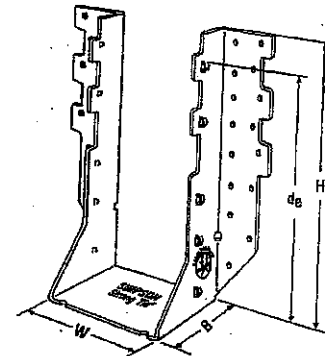
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

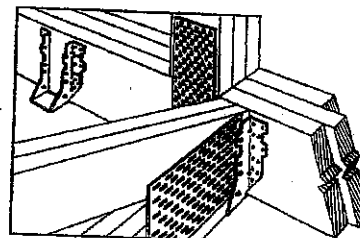
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



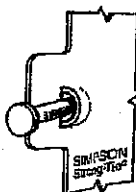
HHUS410



Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

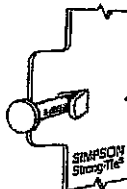
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _p =1.00)	Uplift (K _u =1.15)	Normal (K _p =1.00)
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 5/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7060
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 3/8	3	7 7/8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 3/8	5 3/8	3	3 1/8	(14) 16d	(8) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d	4670	9655	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 3/8	7 3/8	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_s is the distance from the seat of the hanger to the highest joist nail.

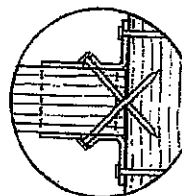


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

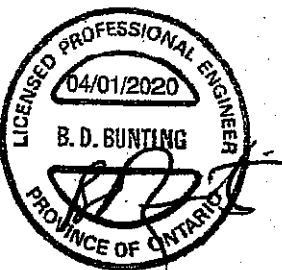
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020.
This information is updated periodically and should not be relied upon after June 30, 2022.
Contact Simpson Strong-Tie for current information and limited warranty, or see strongtie.com.

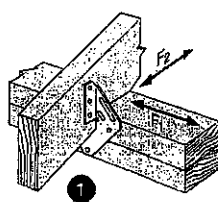
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

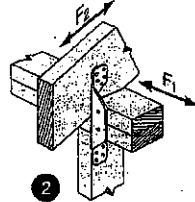
(800) 999-5099
strongtie.com

H/TSP

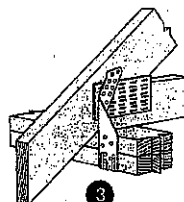
Seismic and Hurricane Ties (cont.)



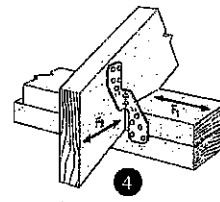
1 H1 Installation



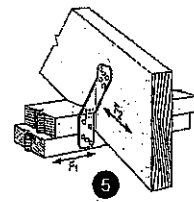
2 H2A Installation



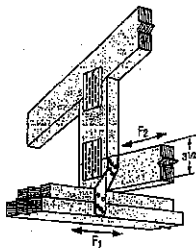
3 TSP Installation



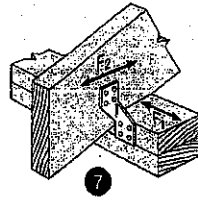
4 H2.5A Installation
(Nails into both top plates)



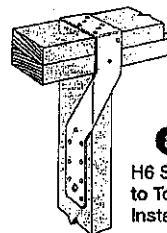
5 H2.5T Installation
(Nails into both top plates)



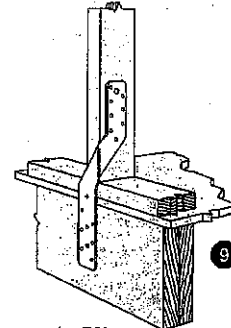
6 H2.5T Installation



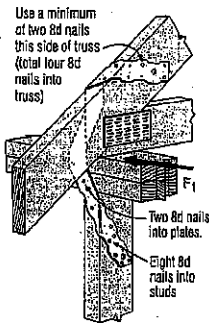
7 H3 Installation
(Nails into upper top plate)



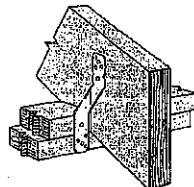
8 H6 Stud
to Top Plate
Installation



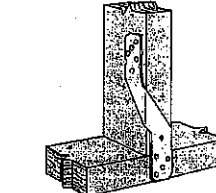
9 H8 Stud to
Band Joist
Installation



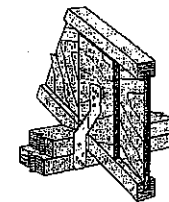
10 H7Z Installation



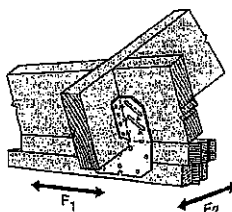
11 H8 Attaching
Rafter to Double
Top Plates



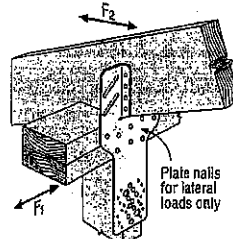
12 H8 attaching Stud to Sill
(4) 8d into plate, (5) 8d into stud)



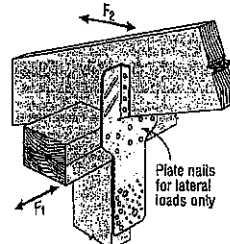
13 H8 attaching
I-Joist to Double
Top Plates



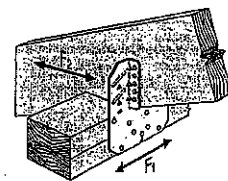
14 H10A Field-Bent
Installation



15 H10S Installation

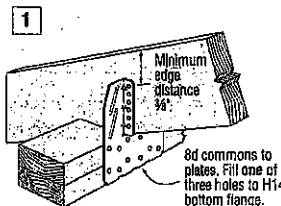


16 H10S Installation
with Stud Offset

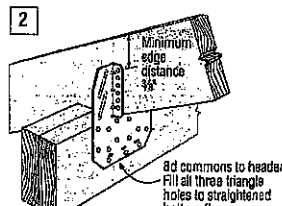


17 H10A
Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 8/12, use 75% of the table uplift value; bend one time only.

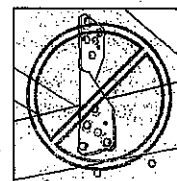


18 H14 Installation to
Double Top Plates



19 H14 Installation
to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

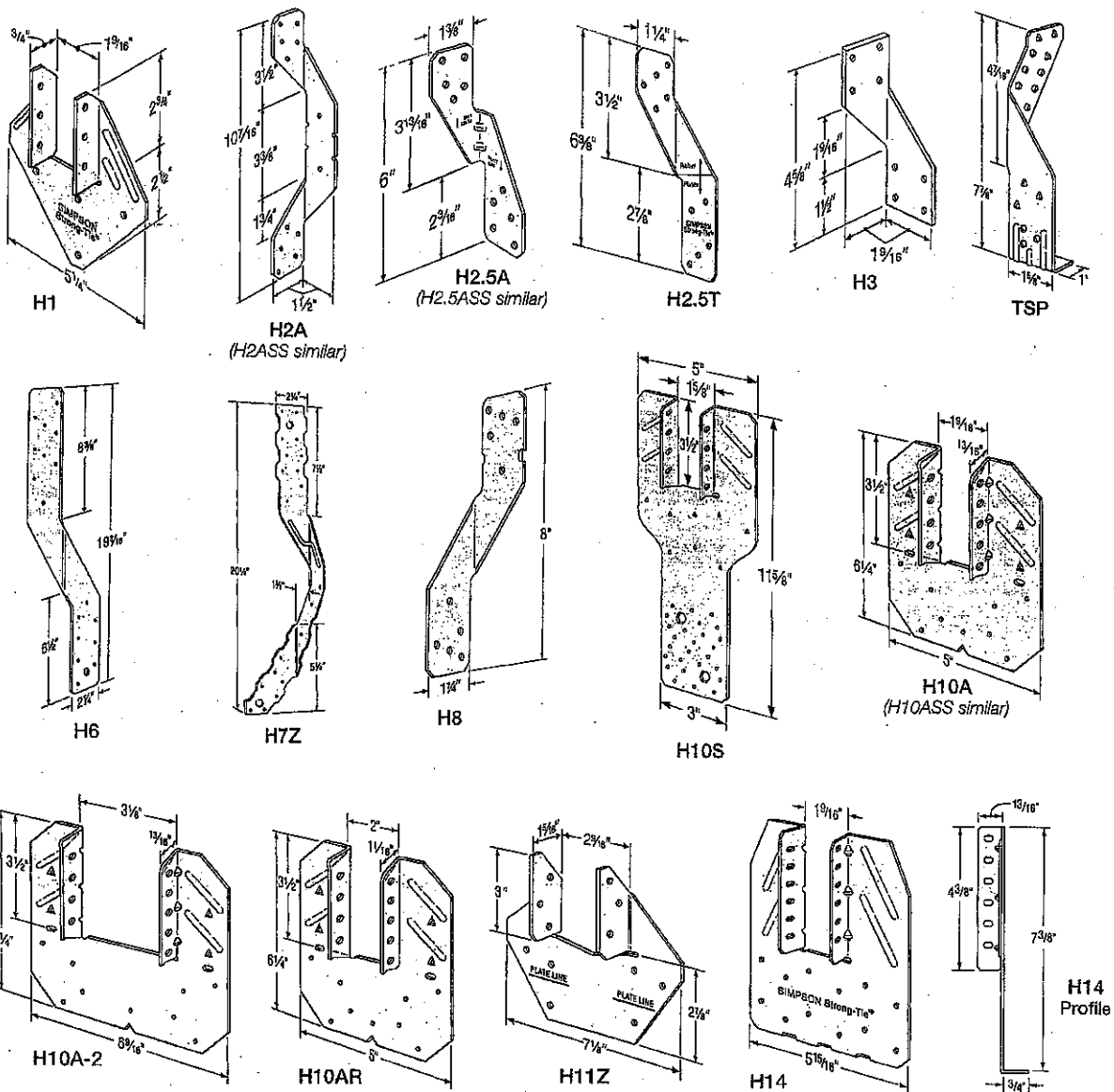
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



Straps and Ties

TECHNICAL BULLETIN

H - Seismic and Hurricane Ties

SIMPSON
Strong-Tie

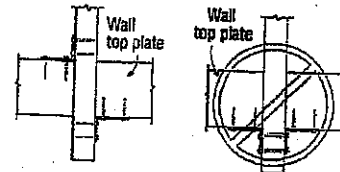
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2 1/2" long common wire, 8d x 1 1/2" = 0.131" x 1 1/2" long, 10d x 1 1/2" = 0.146" x 1 1/2" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

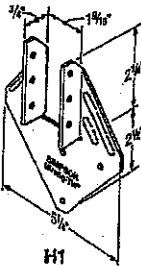
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

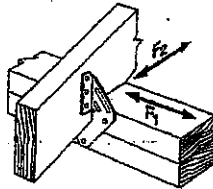


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



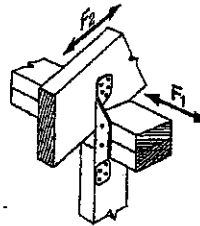
H1



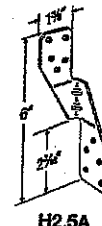
H1 Installation



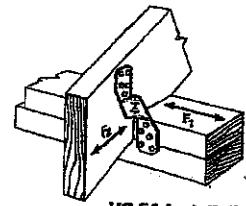
H2A



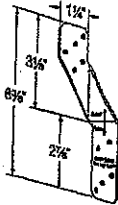
H2A Installation



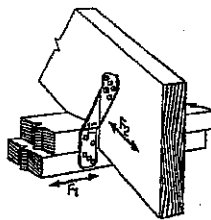
H2.5A



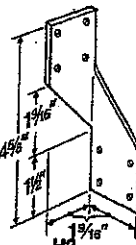
H2.5A Installation



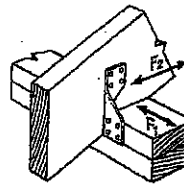
H2.5T



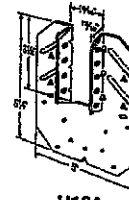
H2.5T Installation
(Nails into both top plates)



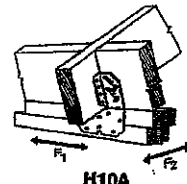
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-1			S-P-F		
					Uplift	Normal		Uplift	Normal	
		To Rafter	To Plates	To Studs		F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1 1/2"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1 1/2"	(2) 8d x 1 1/2"	(5) 8d x 1 1/2"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	285	615	125	190
H10A	18	(9) 10d x 1 1/2"	(9) 10d x 1 1/2"	—	1735	795	410	1505	565	290

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

- When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
- Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

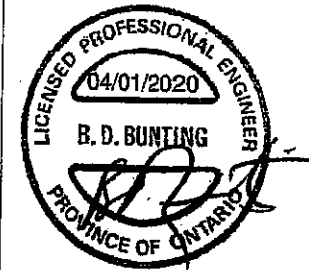


**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22



(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

TC - Truss Connectors

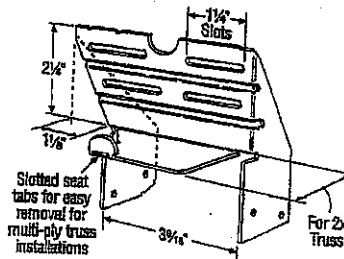
SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

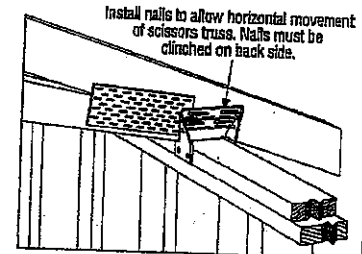
Material: 16 gauge

Finish: G90 galvanized

Design: Factored resistances are in accordance with CSA 086-14



TC24
U.S. Patent 4,932,173



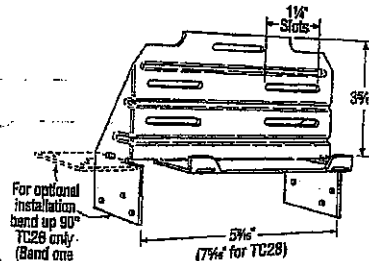
Typical TC24 Installation

Installation:

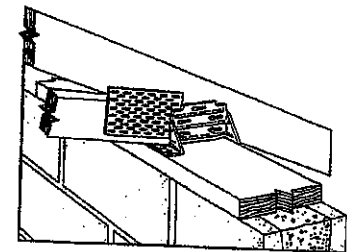
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

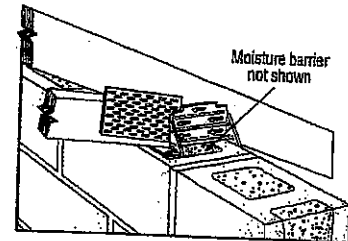
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



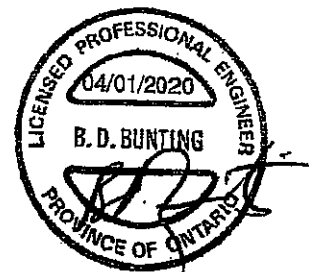
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



LIMIT
STATES
DESIGN

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T-SPECTC20 3/20 exp. 6/22

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HTU

SIMPSON
Strong-Tie

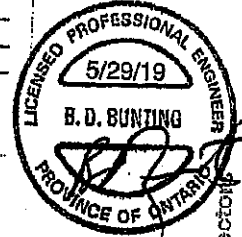
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Nail Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3%	(2) 2x4	(10) 16d	(14) 10d x 1 1/2"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	6%	(2) 2x4	(10) 16d	(20) 10d x 1 1/2"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU26 (Max.)	3%	(2) 2x6	(20) 16d	(26) 10d x 1 1/2"	4150	6395	2945	4540
					18.45	28.45	13.10	20.19
HTU210 (Max.)	7 1/4"	(2) 2x6	(20) 16d	(32) 10d x 1 1/2"	4150	6395	2945	4540
					18.45	28.45	13.10	20.19

See table footnotes on p. 280.

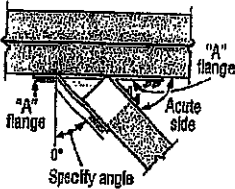


Hanger Options

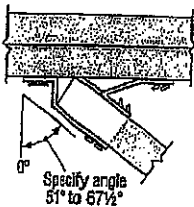
See Hanger Options Information on pp. 125-127.

Skewed Seat

- Skewable up to 67 1/2°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
				lb.	lb.	lb.	lb.
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1 1/2"	1835	4110	1300	2905
	51-67 1/2	(20) 16d	(12) 10d x 1 1/2"	8.16	18.28	6.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1 1/2"	1350	3820	955	2560
	51-67 1/2	(26) 16d	(17) 10d x 1 1/2"	8.01	16.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1 1/2"	2810	4270	1985	3030
	51-67 1/2	(32) 16d	(22) 10d x 1 1/2"	12.50	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1495	2780
	51-67 1/2	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67 1/2	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67 1/2	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67 1/2	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1510	3920	1140	2785
	51-67 1/2	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2815	3855
	51-67 1/2	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1895	3855
	51-67 1/2	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4690
	51-67 1/2	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6580	2225	4745
	51-67 1/2	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

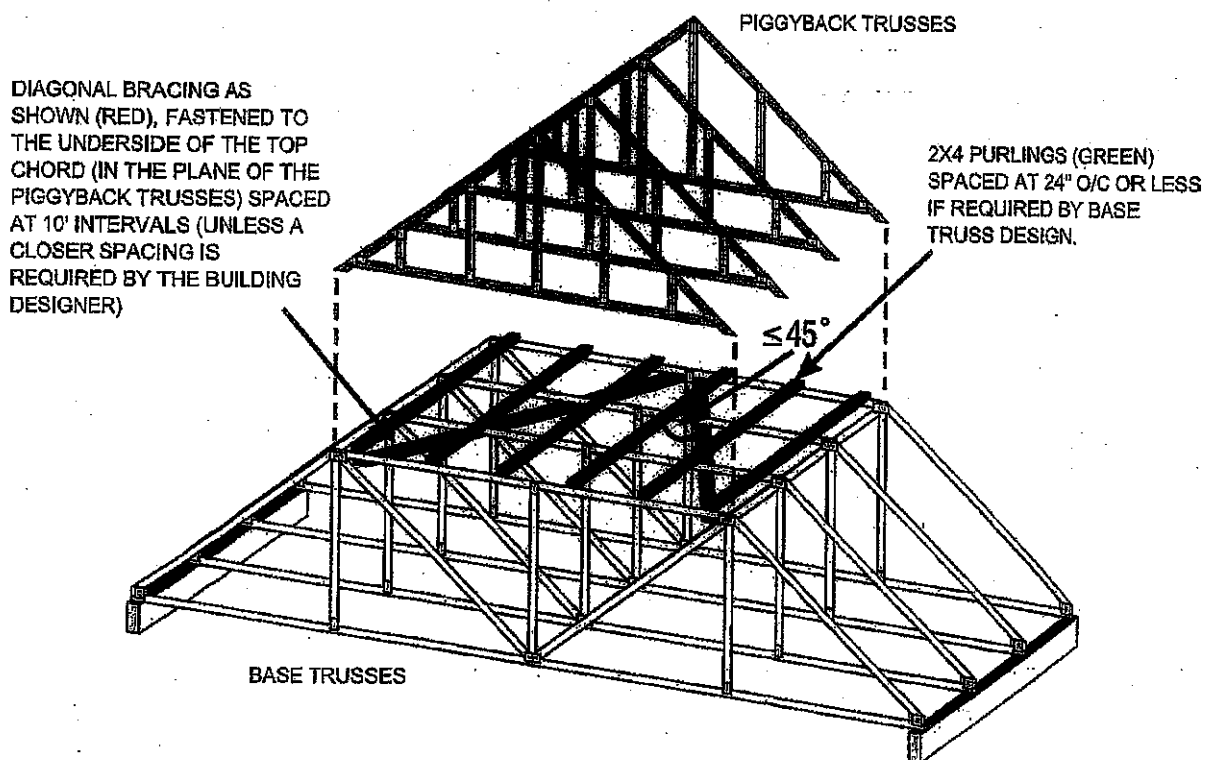
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced nail heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3 3/4" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

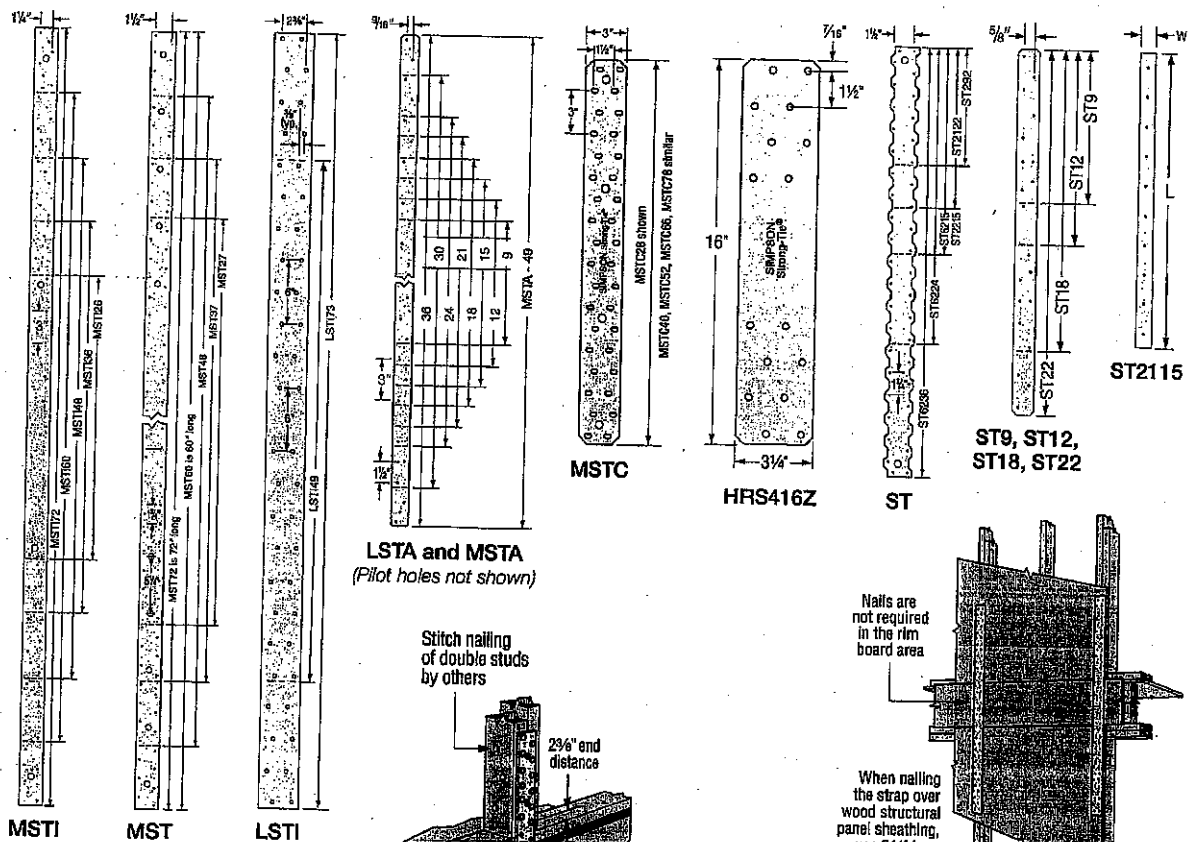
MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

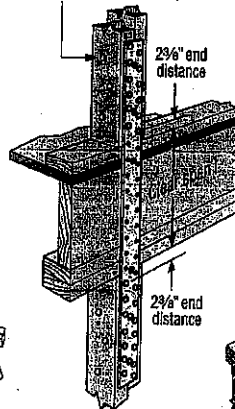
Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

Straps and Ties

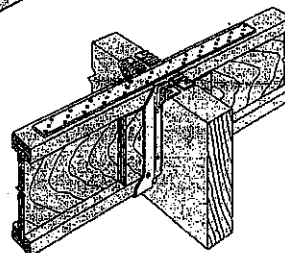


LSTA and MSTA
(Pilot holes not shown)

Stitch nailing
of double studs
by others



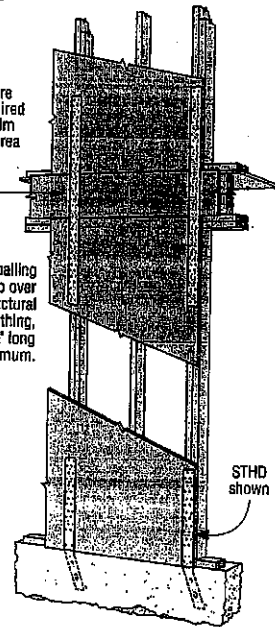
Floor-to-Floor Tie
Installation
Showing a
Clear Span



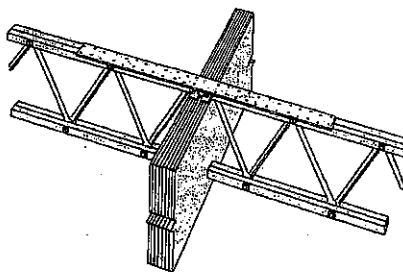
Typical MSTI Installation
(MIT hanger shown)
LSTI similar

Nails are
not required
in the rim
board area

When nailing
the strap over
wood structural
panel sheathing,
use 2 1/2\"/>



Typical Detail with
Strap Installed over
Wood Structural Panel
Sheathing



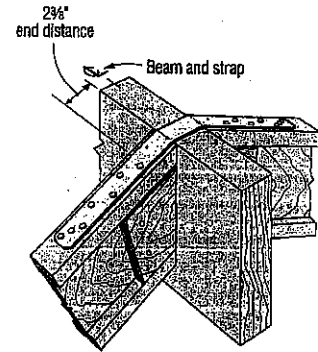
Typical LSTI Installation

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

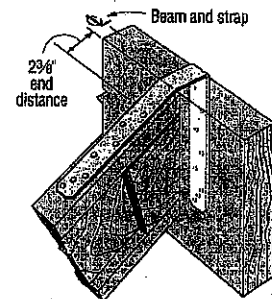
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366-370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance				
					D.Fir-L		S-P-F		
		W	L		(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	
					lb.	lb.	lb.	lb.	
					kN	kN	kN	kN	
LSTA9	20	1¼	9	(6) 10d	600	690	555	635	
LSTA12		1¼	12	(8) 10d	2.67	3.07	2.47	2.82	
LSTA15		1¼	15	(10) 10d	800	920	735	845	
LSTA18		1¼	18	(12) 10d	3.56	4.09	3.27	3.76	
LSTA21		1¼	21	(14) 10d	1000	1150	920	1060	
LSTA24		1¼	24	(16) 10d	4.45	5.12	4.09	4.72	
ST292		2⅝	9⅞	(8) 8d	1200	1380	1105	1270	
ST2122		2⅝	12⅞	(12) 8d	5.34	6.14	4.92	5.65	
ST2115		¾	16⅞	(8) 8d	1400	1610	1290	1485	
ST2215		2⅝	16⅞	(16) 8d	6.23	7.18	5.74	6.61	
LSTA30		18	1¼	30	(20) 10d	1600	1840	1475	1695
LSTA36			1¼	36	(24) 10d	7.12	8.19	6.56	7.54
LSTI49			3¾	49	(32) 10d x 1 ½"	585	675	535	615
LSTI73			3¾	73	(48) 10d x 1 ½"	2.60	3.00	2.38	2.74
MSTA9			1¼	9	(6) 10d	940	1085	865	995
MSTA12			1¼	12	(8) 10d	4.18	4.83	3.85	4.43
MSTA15	1¼		15	(10) 10d	670	770	615	710	
MSTA18	1¼		18	(12) 10d	2.98	3.43	2.74	3.16	
MSTA21	1¼		21	(14) 10d	1335	1540	1235	1420	
MSTA24	1¼		24	(16) 10d	5.94	6.85	5.49	6.32	
MSTA30	16		1¼	30	(20) 10d	2235	2465	2075	2385
MSTA36			1¼	36	(24) 10d	9.94	10.97	9.23	10.61
ST6215			2⅝	16⅞	(16) 8d	2465	2465	2465	2465
ST6224			2⅝	23⅞	(24) 8d	10.97	10.97	10.97	10.97
ST9			1¼	9	(6) 8d	3115	3580	2852	3280
ST12			1¼	11⅞	(8) 8d	13.86	15.93	12.69	14.59
ST18		1¼	17⅞	(12) 8d	4670	5370	4280	4920	
ST22		1¼	21⅞	(18) 8d	20.77	23.89	19.04	21.89	
		1¼	9	(6) 10d	670	770	625	715	
		1¼	12	(8) 10d	2.98	3.43	2.78	3.18	
		1¼	15	(10) 10d	895	1030	830	955	
		1¼	18	(12) 10d	3.98	4.58	3.69	4.25	
		1¼	21	(14) 10d	1120	1285	1040	1195	
		1¼	24	(16) 10d	4.88	5.72	4.63	5.32	
		1¼	27	(18) 10d	1340	1545	1245	1430	
		1¼	30	(20) 10d	5.96	6.87	5.54	6.36	
	1¼	33	(22) 10d	1565	1800	1455	1670		
	1¼	36	(24) 10d	8.96	8.01	6.47	7.43		
	1¼	39	(26) 10d	1790	2060	1660	1910		
	1¼	42	(28) 10d	7.96	9.16	7.38	8.50		
	1¼	45	(30) 10d	2470	2840	2260	2595		
	1¼	48	(32) 10d	10.99	12.63	10.05	11.54		
	1¼	51	(34) 10d	2965	3070	2710	3070		
	1¼	54	(36) 10d	13.19	13.66	12.06	13.66		
	1¼	57	(38) 10d	2725	2725	2545	2725		
	1¼	60	(40) 10d	12.12	12.12	11.32	12.12		
	2⅝	16⅞	(16) 8d	1405	1615	1300	1500		
	2⅝	23⅞	(24) 8d	6.25	7.18	5.78	6.67		
	2⅝	30⅞	(32) 8d	2305	2650	2155	2475		
	2⅝	37⅞	(40) 8d	10.25	11.79	9.59	11.01		
	1¼	9	(6) 8d	525	605	490	560		
	1¼	11⅞	(8) 8d	2.34	2.69	2.18	2.49		
	1¼	14⅞	(10) 8d	700	805	650	750		
	1¼	17⅞	(12) 8d	3.11	3.58	2.89	3.34		
	1¼	20⅞	(14) 8d	1050	1210	975	1125		
	1¼	23⅞	(16) 8d	4.67	5.38	4.34	5.00		
	1¼	26⅞	(18) 8d	1580	1790	1465	1685		
	1¼	29⅞	(20) 8d	7.03	7.96	6.52	7.50		



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

Straps and Ties

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22-23 for other nail sizes and information.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI**SIMPSON**
Strong-Tie

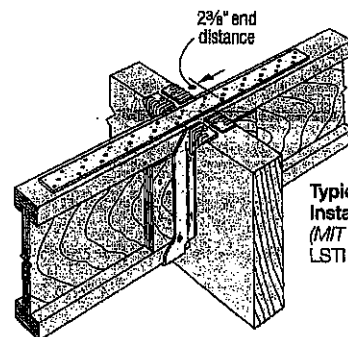
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

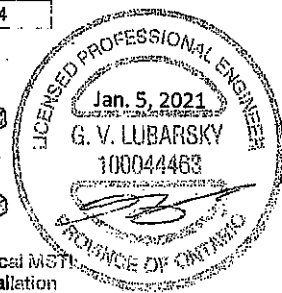
SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance				
		W	L		D-Fir-L		S-P-F		
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	
					lb.	lb.	lb.	lb.	
					kN	kN	kN	kN	
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155	
MSTC40		3	40¼	(48) 10d	17.69	20.22	16.08	18.48	
MSTC52		3	52¼	(54) 10d	5930	6820	5420	6235	
					26.38	30.34	24.11	27.74	
MSTC66	14	3	65¼	(66) 10d	6670	6940	6100	6940	
MSTC78		3	77¼	(66) 10d	29.67	30.87	27.14	30.87	
ST6236		2⅝	33⅝	(36) 8d	8515	8565	7455	8565	
					37.88	38.10	33.16	38.10	
MSTI26	12	2⅝	26	(22) 10d x 1½"	8515	8565	7455	8565	
MSTI36		2⅝	36	(32) 10d x 1½"	37.88	38.10	33.16	38.10	
MSTI48		2⅝	48	(44) 10d x 1½"	3735	4295	3270	3760	
MSTI60		2⅝	60	(56) 10d x 1½"	16.61	19.11	14.55	16.73	
MSTI72		2⅝	72	(68) 10d x 1½"	2825	3250	2475	2850	
MST27		2⅝	27	(26) 8d	12.57	14.46	11.01	12.68	
MST37		2⅝	37½	(38) 8d	4110	4725	3600	4140	
MST48		2⅝	48	(50) 8d	18.28	21.02	16.01	18.42	
HRS416Z		3¼	16	(16) ¼" x 1½" SDS	5650	6500	4955	5695	
MST60		10	2⅝	60	(64) 8d	25.13	28.91	22.04	25.33
MST72			2⅝	72	(78) 8d	7195	7360	6305	7250
						32.01	32.74	28.05	32.25
					7360	7360	7240	7360	
				32.74	32.74	32.21	32.74		
				2685	3090	2365	2710		
				11.94	13.75	10.48	12.06		
				3930	4515	3440	3960		
				17.48	20.08	15.30	17.62		
				5170	5945	4530	5210		
				23.00	26.45	20.15	23.18		
				2400	2760	2120	2440		
				10.68	12.28	9.43	10.85		
				6820	7610	5800	6670		
				29.45	33.85	25.80	29.67		
				8065	9135	7065	8125		
				35.88	40.64	31.43	36.14		

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.

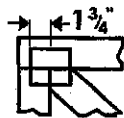


Typical MSTI
Installation
(MIT hanger shown)
LSTI similar

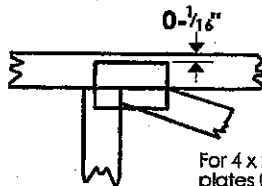


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I, or Eliminator bracing if indicated.

BEARING



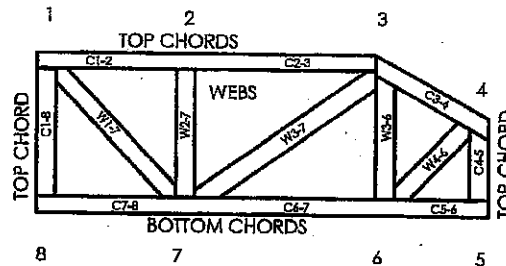
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
- Design assumes trusses will be suitably protected from the environment in accord with TPIC.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.

LGT/MGT/VGT

Girder Tiedowns

The LGT, MGT and VGT are girder tiedowns for moderate- to high-load applications. The LGT and VGT are also suitable for retrofit applications.

LGT connectors provide a low-profile connection to the studs for easy installation of drywall. Simple to install and can be installed on the inside or outside of the wall.

The variable girder tiedown (VGT) is a higher capacity alternative to the LGT and MGT for girder trusses. It attaches with Strong-Drive® SDS Heavy-Duty Connector screws to the side of truss and features a predeflected crescent washer that allows it to accommodate top chord pitches up to 8:12. The VGT is also available with one flange concealed for attachment to trusses with no overhang.

Material: VGT — 7 gauge, LGT2 — 14 gauge, MGT, LGT3, LGT4 — 12 gauge

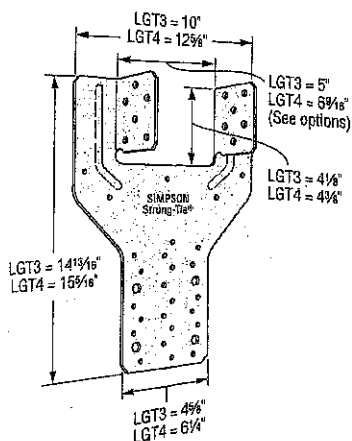
Finish: Galvanized

Installation:

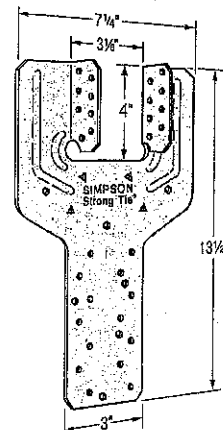
- Before installing fasteners, ensure LGT3-SDS2.5 makes complete contact with bottom of truss.
- Strong-Drive SDS Heavy-Duty Connector screws included with LGT3, LGT4 and VGT series.
- Strong-Drive SDS Heavy-Duty Connector screws driven through truss plates must be approved by the Truss Designer. Predrilling using a $\frac{1}{2}$ " bit is required.
- VGT — Can be installed on roof pitches up to 8/12 or on a bottom chord designed to transfer the load.
- VGT — Screw holes are configured to allow for double installation on a two-ply (minimum) truss.
- VGT — The product can be installed in a single application or in pairs to achieve a higher uplift capacity.
- VGT — When installed on trusses with no overhangs, specify VGTR/L.
- VGT — Install washer component (provided) so that top of washer is horizontal as well as parallel with top of wall top plate.
- LGT3-SDS2.5 and LGT4-SDS3 — The four large hexagon holes are intended for GFCMU and concrete applications.
- MGT — Install a minimum of (6) 10d nails into the face of roof member that is on same side as MGT base.
- See p. 303 for masonry applications.

Options:

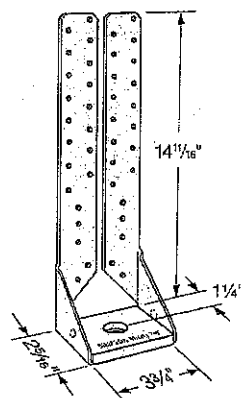
- LGT3 and LGT4 are available with reduced widths of $W = 4\frac{1}{8}$ " and $W = 6\frac{1}{8}$ " — order as LGT3N-SDS2.5 and LGT4N-SDS3.



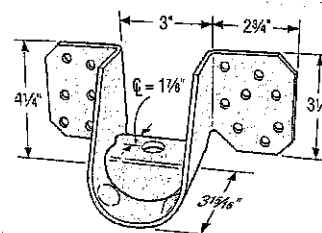
LGT3-SDS2.5
(LGT4-SDS3 similar)



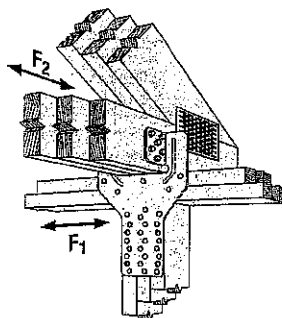
LGT2



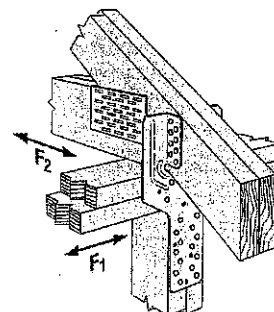
MGT



VGT
US Patent
7,707,785



Typical LGT3-SDS2.5 Installation



Typical LGT2 Installation

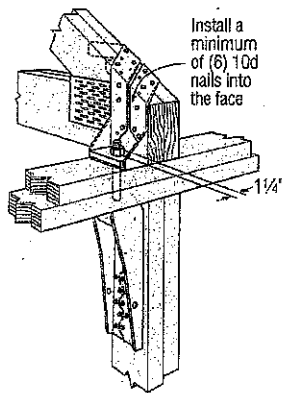
LGT/MGT/VGT

Girder Tiedowns (cont.)

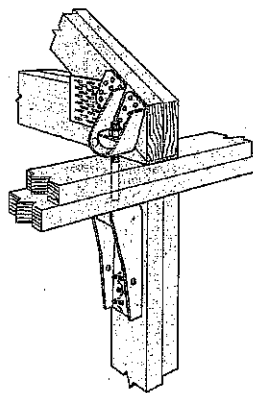
These products are available with additional corrosion protection. For more information, see p. 20.

Model No.	Qty.	No. of Plies	Fasteners		Factored Resistance ($K_D = 1.15$)					
			Studs or Anchor	Girder Truss	D.Fir-L			S-P-F		
					Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
					lb.	lb.	lb.	lb.	lb.	lb.
					kN	kN	kN	kN	kN	kN
LGT2	1	Two ply	(14) 10d ⁴	(16) 10d	3670	1170	285	2605	830	200
					16.33	5.20	1.27	11.59	3.69	0.89
LGT3-SDS2.5	1	Three ply	(26) 10d	(12) ¼" x 2½" SDS	6415	1335	670	4930	945	475
					28.54	5.94	2.98	21.93	4.20	2.11
LGT4-SDS3	1	Four ply	(30) 10d ⁵	(16) ¼" x 3" SDS	6030	2785	1125	3980	1980	800
					26.82	12.39	5.00	17.70	8.81	3.56
MGT	1	Two ply min.	(1) ¾" diameter	(22) 10d	5610	—	—	3985	—	—
					24.96	—	—	17.73	—	—
VGT	1	Two ply min.	(1) ¾" diameter	(16) ¼" x 3" SDS	8600	—	—	6195	—	—
					38.26	—	—	27.56	—	—
	2	Two ply min.	(2) ¾" diameter	(32) ¼" x 3" SDS	11690	—	—	8420	—	—
					52.00	—	—	37.46	—	—
VGTR/L	1	Two ply min.	(1) ¾" diameter	(16) ¼" x 3" SDS	3475	—	—	2505	—	—
					15.46	—	—	11.14	—	—
	2	Two ply min.	(2) ¾" diameter	(32) ¼" x 3" SDS	6950	—	—	5010	—	—
					30.92	—	—	22.29	—	—

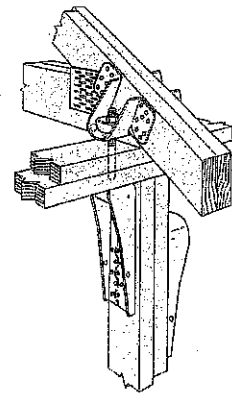
1. Attached members must be designed to resist the factored resistances.
2. Factored resistances have been increased 15% for uplift with no further increase allowed; reduce where other loads govern.
3. Additional anchorage products to be designed by others.
4. LGT2 — F₂ factored resistance requires installation of (4) 10d nails in optional nail holes.
5. LGT4 — F₂ factored resistance requires installation of (7) 10d nails in optional nail holes.
6. MGT can be installed with straps vertical for full table value provided (26) 10d nails are installed to either a solid header or minimum double 2x6 web.
7. Nails: 10d = 0.148" dia. x 3" long. See pp. 22–23 for other nail sizes and information.



Typical MGT
Installation with HDU4



Typical VGTR Single
Installation with HDU2



Typical VGT Double
Installation with HDU4s