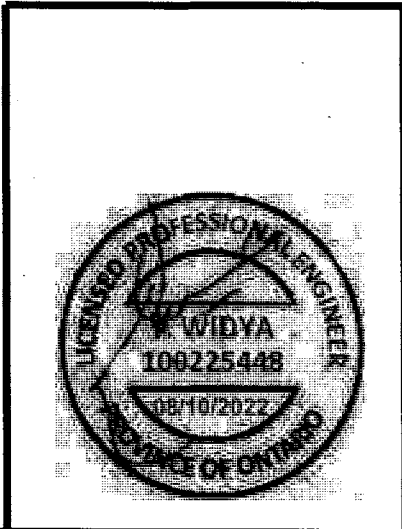
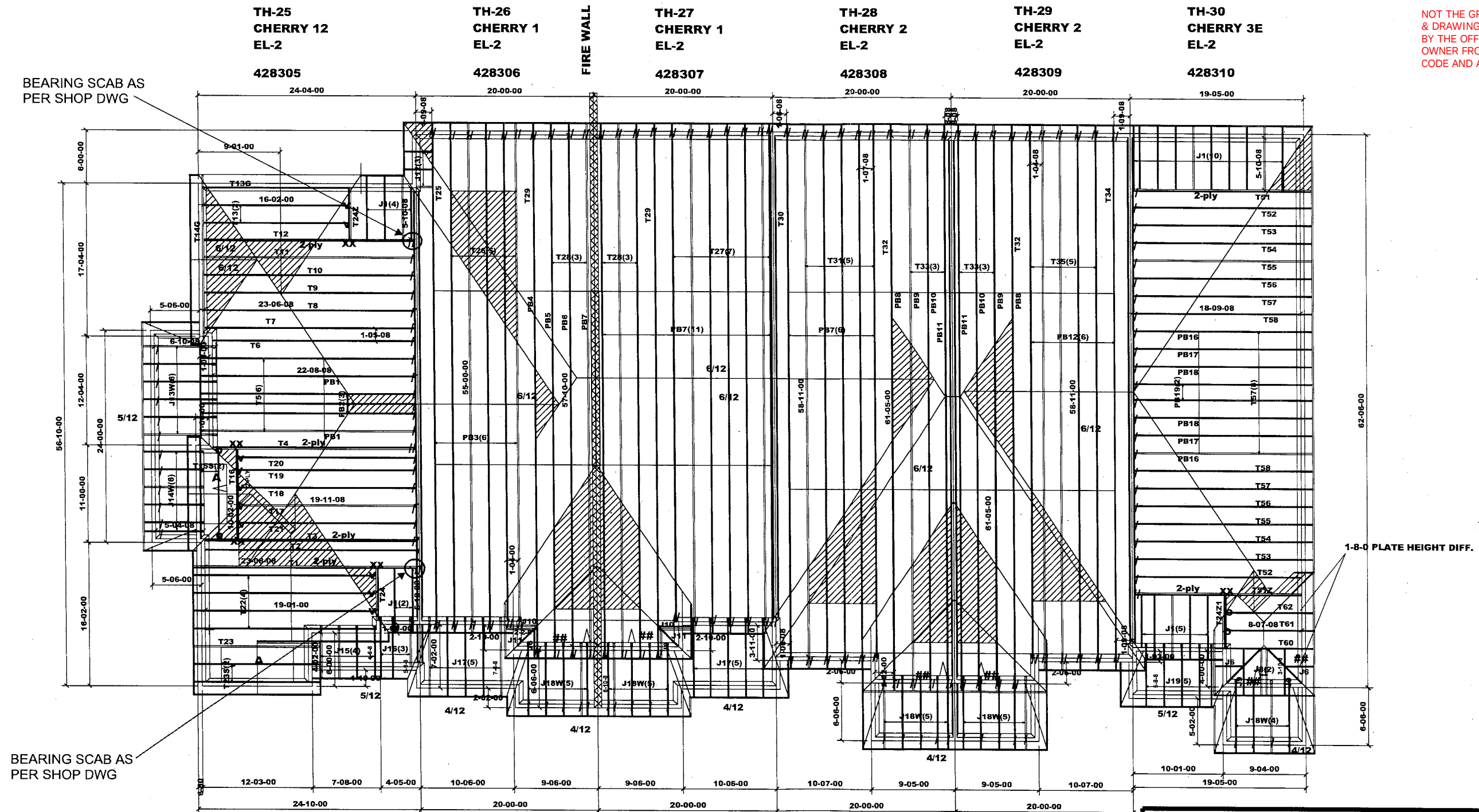


NOT THE GRANTING OF A PERMIT NOR REVIEWING OF SPECS & DRAWINGS NOR INSPECTIONS MADE DURING INSTALLATION BY THE OFFICIAL HAVING JURISDICTION SHALL RELIEVE THE OWNER FROM REQUIREMENTS OF THE ONTARIO BUILDING CODE AND ANY OTHER REFERENCED REQUIREMENTS.



TRUSS PLACEMENT PLAN.
This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

9/12 ROOF PITCH UNLESS NOTED OTHERWISE
A- 12" RAISED CEILING
1-8-0 RAISE PLATE AND CEILING

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 33.4 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 26.7 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5A- (V)

DENOTES:
CONVENTIONAL FRAMING

Job Track: **52956**

Layout ID: **428250**

Plan Log: **206506**

Builder / Location: **GREEN PARK HOMES / CAMBRIDGE**

Project: **BARLASSINA**

Date: 2022-07-29 Designer: YPG

Model / Elevation: **BLOCK 119 / TH-25 ~ TH-30 (EL.2)**

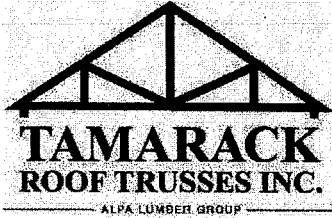
These drawings constitute the property of TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milek ver 8.5.3.233

M15079

E22085161 - E22085241

DELIVERY SHIPLIST



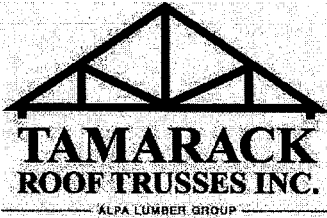
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-25,TH-13 (CHERRY 12 ,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428305
 Ref #
 Page: 1 of 3
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	9 /12	23-08-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	229.42 145.33		
	1	T2 Half Hip	9 /12	23-08-08	5-01-04	2 x 4		1-06-04 5-01-04	95.24 60.00		
	1 2-ply	T3 Half Hip Girder	9 /12	23-08-08	5-07-08	2 x 4 2 x 6		1-06-04 5-07-08	235.61 145.67		
	1 2-ply	T4 Half Hip Girder	9 /12	22-08-08	10-10-00	2 x 4 2 x 6		1-06-04 10-10-00	304.95 190.00		
	6	T5 Piggyback Base	9 /12	22-08-08	10-10-00	2 x 4	1-03-08	1-06-04 10-10-00	814.16 510.00		
	1	T6 Half Hip	9 /12	22-08-08	9-10-00	2 x 4		1-06-04 9-10-00	121.18 76.00		
	1	T7 Half Hip	9 /12	23-06-08	9-01-04	2 x 4		10-12 9-01-04	118.81 73.67		
	1	T8 Half Hip	9 /12	23-06-08	8-01-04	2 x 4		10-12 8-01-04	113.18 71.00		
	1	T9 Half Hip	9 /12	23-06-08	7-01-04	2 x 4		10-12 7-01-04	110.64 70.67		
	1	T10 Half Hip	9 /12	23-06-08	6-01-04	2 x 4		10-12 6-01-04	101.97 64.00		
	1	T11 Half Hip	9 /12	23-06-08	5-01-04	2 x 4		10-12 5-01-04	94.33 59.83		
	1 2-ply	T12 Half Hip Girder	9 /12	23-06-08	4-01-04	2 x 4 2 x 6		10-12 4-01-04	211.3 127.33		
	2	T13 Common	9 /12	16-02-00	7-03-04	2 x 4		10-12 1-06-04	139.14 89.67		
	1	T13G GABLE	9 /12	16-02-00	7-03-04	2 x 4		10-12 1-06-04	67.17 43.83		

DELIVERY SHIPLIST

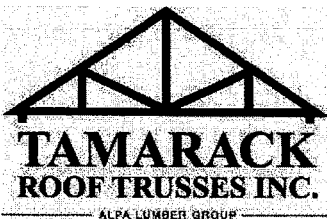


Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #: (CHERRY)
 Elevation: TH-25,TH-13 (CHERRY 12 ,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428305
 Ref #
 Page: 2 of 3
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T14G GABLE	6 /12	16-06-00	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	62.19 38.83		
	2	T15S Roof Special	9 /12	10-02-00	5-04-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	103.29 73.33		
	1 2-ply	T16 Half Hip Girder	9 /12	10-02-00	4-01-12	2 x 4 2 x 6		1-06-04 4-01-12	95.83 65.33		
	1	T17 Half Hip	9 /12	19-11-08	7-07-08	2 x 4		3-07-00 7-07-08	91.36 57.00		
	1	T18 Half Hip	9 /12	19-11-08	8-07-08	2 x 4		3-07-00 8-07-08	107.13 68.00		
	1	T19 Half Hip	9 /12	19-11-08	9-07-08	2 x 4		3-07-00 9-07-08	115.76 72.67		
	1	T20 Half Hip	9 /12	19-11-08	10-04-00	2 x 4		3-07-00 10-04-00	119.9 74.50		
	1	T21 Half Hip	9 /12	19-11-08	6-07-08	2 x 4		3-07-00 6-07-08	95.31 60.33		
	4	T22 Common	9 /12	19-01-00	8-08-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	367.61 232.00		
	1	T23 Common	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	48.96 31.67		
	2	T23S Roof Special	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	112.41 77.67		
	1 2-ply	T24 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T24Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB1 Piggyback	9 /12	10-03-08	1-00-00	2 x 4		1-00-00	47.72 31.33		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: BLOCK 119 , BLOCK 120 Lot #: Elevation: TH-25,TH-13 (CHERRY 12 ,EL-2)	Job Track: 52956 PlanLog: 206506 Layout ID: 428305 Ref # Page: 3 of 3 Date: 08-15-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	PB2 Piggyback	9 /12	10-03-08	2-00-00	2 x 4		2-00-00	99.06 66.00		
	6	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	6	J13W Jack-Open	5 /12	6-10-08	3-08-10	2 x 4	1-03-08	5-06 3-03-12	128.63 80.00		
	6	J14W Jack-Open	5 /12	5-04-08	3-01-02	2 x 4	1-03-08	5-06 2-08-04	91.81 60.00		
	4	J15 Jack-Open	5 /12	4-06-08	2-08-15	2 x 4	1-03-08	5-06 2-04-01	53.29 34.67		
	3	J16 Jack-Open	5 /12	5-06-08	3-01-15	2 x 4	1-03-08	5-06 2-09-01	47.09 30.00		

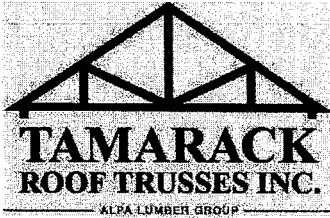
TOTAL # TRUSS= 75 TOTAL BFT OF ALL TRUSSES= 3019.67 BFT. TOTAL WEIGHT OF ALL TRSSES 4762.02 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
38	Hardware	H2.5A	
4	Hardware	HGUS26-2	
10	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 54

DELIVERY SHIPLIST

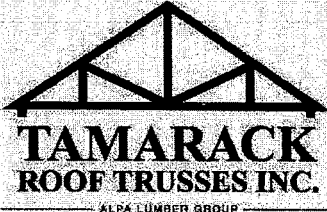


Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #: (CHERRY)
 Elevation: TH-26,TH-14 (CHERRY 1,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428306
 Ref #
 Page: 1 of 2
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T25 Piggyback Base Girder	6 /12	55-00-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	389.76 235.33		
	5	T26 Piggyback Base	6 /12	55-00-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1783.46 1076.67		
	3	T28 Piggyback Base	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1145.32 712.00		
	1	T29 Piggyback Base Girder	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	403.91 241.33		
	6	PB3 Piggyback	6 /12	13-08-00	3-05-00	2 x 4			197.69 121.00		
	1	PB4 Piggyback	6 /12	19-08-00	1-04-06	2 x 4			56.06 35.83		
	1	PB5 Piggyback	6 /12	19-08-00	2-10-06	2 x 4			56.53 36.83		
	1	PB6 Piggyback	6 /12	19-08-00	4-04-06	2 x 4			63.79 40.83		
	1	PB7 Piggyback	6 /12	19-08-00	4-11-00	2 x 4			57.58 35.67		
	1	J6 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	9.23 7.00		
	1	J10 Jack-Open	9 /12	2-02-08	3-02-02	2 x 4	1-03-08	1-06-04 3-02-02	10.47 7.67		
	1	J11 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 5-01	1-06-04 2-10-05	9.61 7.67		
	3	J12 Jack-Open	9 /12	1-09-08	2-10-06	2 x 4	1-03-08	1-06-04 2-10-06	27.43 21.00		
	5	J17 Jack-Open	4 /12	7-06-08	3-03-00	2 x 4	1-03-08	3-15 2-10-02	111.99 72.50		

		DELIVERY SHIPLIST	
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track: 52956
	Builder:	GREEN PARK HOMES	PlanLog: 206506
	Project:	BARLASSINA	Layout ID: 428306
	Location:	CAMBRIDGE	Ref #
	Model:	BLOCK 119 , BLOCK 120	Page: 2 of 2
	Lot #:	(CHERRY)	Date: 08-15-2022
Elevation:	TH-26,TH-14 (CHERRY 1,EL-2)	Designer:	
			Sales Rep: Rick DiCiano

Roof Trusses

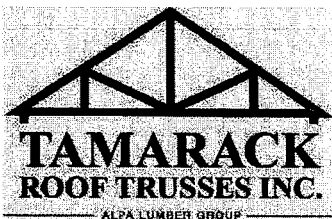
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

TOTAL # TRUSS= 36 TOTAL BFT OF ALL TRUSSES= 2712.16 BFT. TOTAL WEIGHT OF ALL TRSSES 4422.45 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
45	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 45

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: BLOCK 119 , BLOCK 120 Lot #: Elevation: TH-27 ,TH-15 (CHERRY 1,EL-2)	Job Track: 52956 PlanLog: 206506 Layout ID: 428307 Ref # Page: 1 of 1 Date: 08-15-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	T27 Piggyback Base	6 /12	55-00-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	2445.02 1467.67		
	3	T28 Piggyback Base	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1145.32 712.00		
	1	T29 Piggyback Base Girder	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	403.91 241.33		
	11	PB7 Piggyback	6 /12	19-08-00	4-11-00	2 x 4			633.33 392.33		
	1	J6 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	9.23 7.00		
	1	J10 Jack-Open	9 /12	2-02-08	3-02-02	2 x 4	1-03-08	1-06-04 3-02-02	10.47 7.67		
	1	J11 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 5-01	1-06-04 2-10-05	9.61 7.67		
	5	J17 Jack-Open	4 /12	7-06-08	3-03-00	2 x 4	1-03-08	3-15 2-10-02	111.99 72.50		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

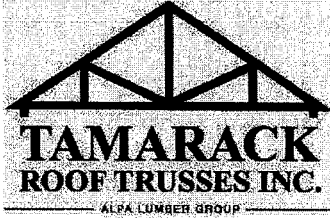
TOTAL # TRUSS= 35 TOTAL BFT OF ALL TRUSSES= 2969 BFT. TOTAL WEIGHT OF ALL TRSSES 4868.5 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
49	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 49

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-28 ,TH-16 (CHERRY 2,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428308
 Ref #
 Page: 1 of 1
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T30 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	399.03 241.33		
	5	T31 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1970.6 1183.33		
	1	T32 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	438.02 263.33		
	3	T33 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1336.72 818.00		
	6	PB7 Piggyback	6 /12	19-08-00	4-11-00	2 x 4			345.45 214.00		
	1	PB8 Piggyback	6 /12	23-07-00	1-04-12	2 x 4			68.61 43.67		
	1	PB9 Piggyback	6 /12	23-07-00	2-10-12	2 x 4			68.38 43.50		
	1	PB10 Piggyback	6 /12	23-07-00	4-04-12	2 x 4			75.01 46.50		
	1	PB11 Piggyback	6 /12	23-07-00	5-10-12	2 x 4			69.91 44.33		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

TOTAL # TRUSS= 25

TOTAL BFT OF ALL TRUSSES= 2958.82 BFT.

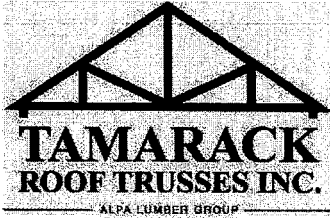
TOTAL WEIGHT OF ALL TRSSES 4871.34 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
45	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 45

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #: (CHERRY)
 Elevation: TH-29,TH-17 (CHERRY 2,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428309
 Ref #
 Page: 1 of 1
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T32 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	438.02 263.33		
	3	T33 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1336.72 818.00		
	1	T34 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	395.49 239.33		
	5	T35 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1995.15 1213.33		
	1	PB8 Piggyback	6 /12	23-07-00	1-04-12	2 x 4			68.61 43.67		
	1	PB9 Piggyback	6 /12	23-07-00	2-10-12	2 x 4			68.38 43.50		
	1	PB10 Piggyback	6 /12	23-07-00	4-04-12	2 x 4			75.01 46.50		
	1	PB11 Piggyback	6 /12	23-07-00	5-10-12	2 x 4			69.91 44.33		
	6	PB12 Piggyback	6 /12	22-04-00	5-07-00	2 x 4			395.84 242.00		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

TOTAL # TRUSS= 25

TOTAL BFT OF ALL TRUSSES= 3014.82 BFT.

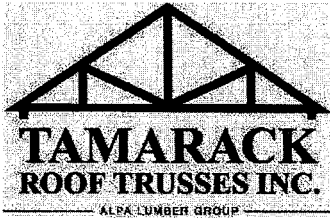
TOTAL WEIGHT OF ALL TRSSES 4942.73 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
45	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 45

DELIVERY SHIPLIST

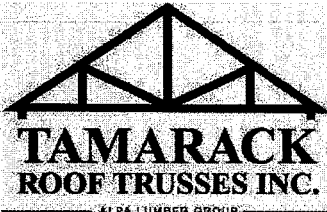


Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-30 , TH-18 (CHERRY 3E,EL-2)

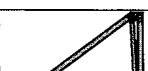
Job Track: 52956
 PlanLog: 206506
 Layout ID: 428310
 Ref #
 Page: 1 of 2
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T2421 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T51 Half Hip Girder	9 /12	18-09-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	177.93 112.00		
	1 2-ply	T51Z Half Hip Girder	9 /12	18-09-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	177.93 112.00		
	2	T52 Half Hip	9 /12	18-09-08	5-01-04	2 x 4		1-06-04 5-01-04	150.17 97.00		
	2	T53 Half Hip	9 /12	18-09-08	6-01-04	2 x 4	1-03-08	1-06-04 6-01-04	166.98 106.67		
	2	T54 Half Hip	9 /12	18-09-08	7-01-04	2 x 4	1-03-08	1-06-04 7-01-04	176.84 113.33		
	2	T55 Half Hip	9 /12	18-09-08	8-01-04	2 x 4	1-03-08	1-06-04 8-01-04	191.55 121.67		
	2	T56 Half Hip	9 /12	18-09-08	9-01-04	2 x 4	1-03-08	1-06-04 9-01-04	202.89 127.00		
	10	T57 Half Hip	9 /12	18-09-08	10-01-04	2 x 4	1-03-08	1-06-04 10-01-04	1062.03 670.00		
	2	T58 Half Hip	9 /12	18-09-08	11-01-04	2 x 4	1-03-08	1-06-04 11-01-04	208.01 128.67		
	1	T60 Hip Girder	9 /12	8-06-00	4-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	40.88 26.67		
	1	T61 Common	9 /12	8-06-00	6-04-08	2 x 4	1-03-08 1-03-08	3-02-04 3-02-04	44.63 28.67		
	1	T62 Hip	9 /12	8-06-00	5-06-12	2 x 4	1-03-08 1-03-08	3-02-04 3-02-04	50.53 32.00		
	2	PB16 Piggyback	9 /12	7-04-03	2-00-00	2 x 4		2-00-00	45.61 31.00		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: BLOCK 119 , BLOCK 120 Lot #: Elevation: TH-30 , TH-18 (CHERRY 3E,EL-2)	Job Track: 52956 PlanLog: 206506 Layout ID: 428310 Ref # Page: 2 of 2 Date: 08-15-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB17 Piggyback	9 /12	7-04-03	3-00-00	2 x 4		3-00-00	49.1 32.33		
	2	PB18 Piggyback	9 /12	7-04-03	4-00-00	2 x 4		4-00-00	53.72 36.00		
	2	PB19 Piggyback	9 /12	7-04-03	5-00-00	2 x 4		5-00-00	52.5 34.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	2	J6 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	18.46 14.00		
	2	J8 Jack-Open	9 /12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	29.83 20.33		
	2	J9 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 2-01-01	1-06-04 2-10-05	23.19 16.67		
	4	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	79.69 48.67		
	5	J19 Jack-Open	5 /12	5-08-08	3-02-13	2 x 4	1-03-08	4-01 2-08-09	76.47 46.67		

TOTAL # TRUSS= 69 TOTAL BFT OF ALL TRUSSES= 2153.35 BFT. TOTAL WEIGHT OF ALL TRSSES 3389.27 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
30	Hardware	H2.5A	
1	Hardware	HGUS26-2	
2	Hardware	LUS24	

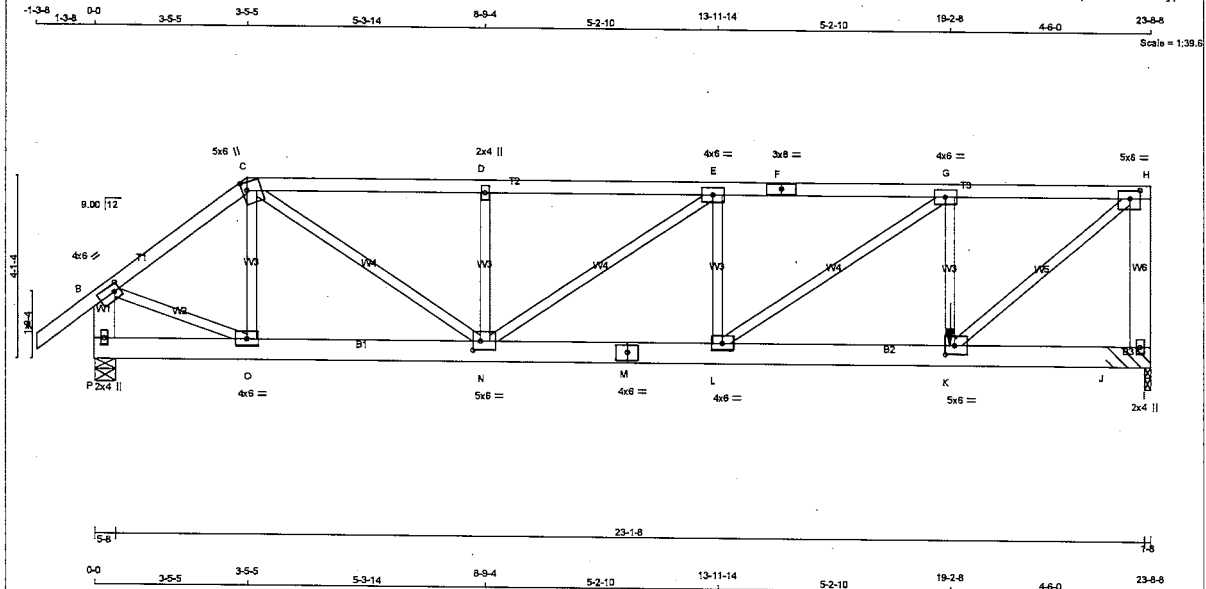
TOTAL NUMBER OF ITEMS= 33

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085191
428250	T1		2	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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ID:4IRITKRO8ssghv2KQFYp5dytbbD-XbrfKvRdz7WkISBhSDFAS24xVIONupxWSFRXmMypD6c



TOTAL WEIGHT = 2 X 115 = 229 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - F 2x4 DRY No.2 SPF D - H 2x4 DRY No.2 SPF I - H 2x6 DRY No.2 SPF P - B 2x6 DRY No.2 SPF P - M 2x6 DRY No.2 SPF M - I 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX I 3223 0 3223 0 0 1-8 1-8 P 1860 0 1860 0 0 5-8 1-8				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 40.1 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM GIRDER TYPE: C-Profile LEFT SETBACK = 3'-5" RIGHT SETBACK = 0-0 END SETBACK = 5'-10-8 END WALL WIDTH = 0-0 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE -ADDTL LOADS BASED ON 55 % OF GSL LOADS APPLIED TO FIRST 4'-6-0 OF SPAN MEASURED FROM THE RIGHT. *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015 THIS DESIGN COMPLIES WITH: - PART 9 OF NBC2015, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.79") CALCULATED VERT. DEFL.(LL) = U/999 (0.08") ALLOWABLE DEFL.(TL) = L/360 (0.79") CALCULATED VERT. DEFL.(TL) = U/999 (0.12") CSI: TC=0.31/1.00 (G-H:1), BC=0.26/1.00 (L-N:1), WB=0.54/1.00 (H-K:1), SSI=0.21/1.00 (G-H:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (K) (INPUT = 0.80) JSI METAL= 0.43 (M) (INPUT = 1.00)			
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DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)	
TOP CHORDS : (0.122"x3") SPIRAL NAILS	
A-C 1 12 TOP	
C-F 1 12 TOP	
F-H 1 12 TOP	
H-I 2 12 TOP	
I-P 2 12 TOP	
P-M 2 12 TOP	
M-I 2 12 TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS	
P-M 2 12 TOP	
M-I 2 12 TOP	
WEBS : (0.122"x3") SPIRAL NAILS	
G-K 1 6 SIDE(174.2)	
2x3 1 6 SIDE(285.2)	

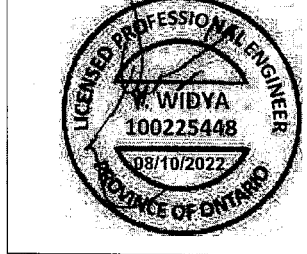
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.										DON'T STANDARD GIRDER LOADS									
A-B	0/36	-95.2	-95.2	0.07 (1)	10.00	O-C	-334/0	0.04 (1)	ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.										
B-C	-1819/0	-95.2	-95.2	0.12 (1)	6.25	B-D	0/1520	0.19 (1)											
C-D	-2951/0	-95.2	-95.2	0.26 (1)	5.10	K-G	-914/0	0.12 (1)											
D-E	-2952/0	-95.2	-95.2	0.26 (1)	5.10	K-H	0/4338	0.54 (1)											
E-F	-3546/0	-95.2	-95.2	0.31 (1)	4.88	L-G	0/253	0.03 (1)											
F-G	-3546/0	-95.2	-95.2	0.31 (1)	4.88	C-N	0/1840	0.23 (1)	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL-BUILDING REQUIREMENTS OF PART 8, NBC2015										
G-H	-3340/0	-187.4	-187.4	0.31 (1)	4.77	L-E	-23/61	0.01 (4)											
H-I	-3121/0	0.0	0.0	0.25 (1)	7.81	N-O	-562/0	0.07 (1)											
P-B	-1845/0	0.0	0.0	0.07 (1)	7.81	N-E	-731/0	0.27 (1)											
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.										THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2015, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TC14 2014									
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE ABLE TO TRANSFER.																			
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.																			
P-O	0/0	-18.5	-18.5	0.03 (4)	10.00														
O-N	0/1442	-18.5	-18.5	0.12 (1)	10.00														
N-M	0/3546	-18.5	-18.5	0.26 (1)	10.00														
M-L	0/3546	-18.5	-18.5	0.26 (1)	10.00														
L-K	0/3340	-18.5	-18.5	0.26 (1)	10.00														
K-J	0/0	-36.4	-36.4	0.05 (1)	10.00														
J-I	0/0	-36.4	-36.4	0.05 (1)	10.00														
PLATES (table is in inches)										(55 % OF 33.4 P.S.F. G.S.S. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD									
JT	TYPE	PLATES	W	LEN	Y	X													

PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMWW-I MT20 4.0 6.0 2.00 1.50 C TTTWW+m MT20 5.0 6.0 2.25 1.25 D TMWW+w MT20 2.0 4.0 E TMWW+H MT20 4.0 6.0 F TS-I MT20 3.0 8.0 G TMWW-I MT20 4.0 6.0 H TMWW-I MT20 5.0 6.0 2.25 2.75 I BMV1+p MT20 2.0 4.0 K BMWW-I MT20 5.0 6.0 2.50 2.50 L BMWW-I MT20 4.0 6.0 M BS-I MT20 4.0 6.0 N BMWW-I MT20 5.0 6.0 2.50 2.00 O BMWW-I MT20 4.0 6.0 P BMV1+p MT20 2.0 4.0				FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. K 19-2-8 -1761 -1761 - FRONT VERT TOTAL - C1			
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CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

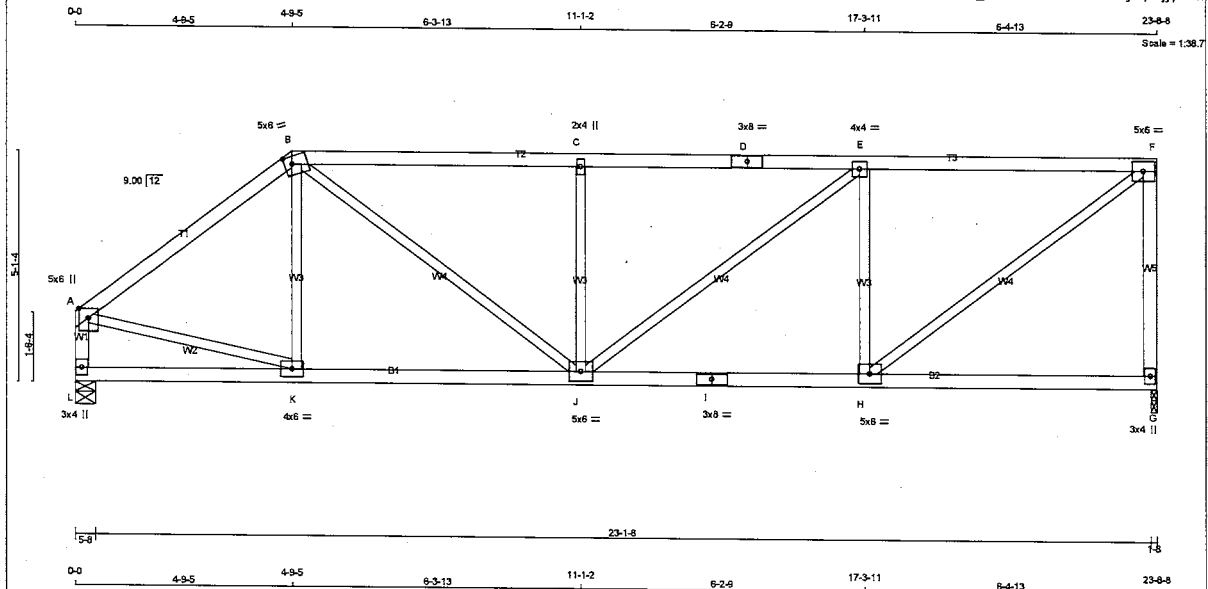
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428250	T2		1	GREEN PARK HOMES JT 53080	E22085192

Alpa Roof Truss, Maple

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LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMWW+p	MT20	5.0	6.0	Edge
B TMWW-m	MT20	5.0	6.0	2.00 2.00
C TMWW+w	MT20	2.0	4.0	
D TS+	MT20	3.0	8.0	
E TMWW-t	MT20	4.0	4.0	
F TMWW-t	MT20	5.0	6.0	
G BMV1+p	MT20	3.0	4.0	
H BMWW-t	MT20	5.0	6.0	
I BS+	MT20	3.0	8.0	
J BMWWH-t	MT20	5.0	5.0	
K BMWW-t	MT20	4.0	5.0	
L BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

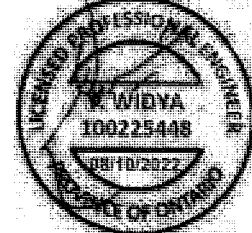
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REQRD					
JT	GROSS REACTION	GROSS REACTION	BRG	BRG					
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX			
G	1348	0	1348	0	0	1-8			
L	1348	0	1348	0	0	5-8			
UNFACTORED REACTIONS									
	1ST CASE	MAX	MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
G	951	634 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0		
L	951	634 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0		

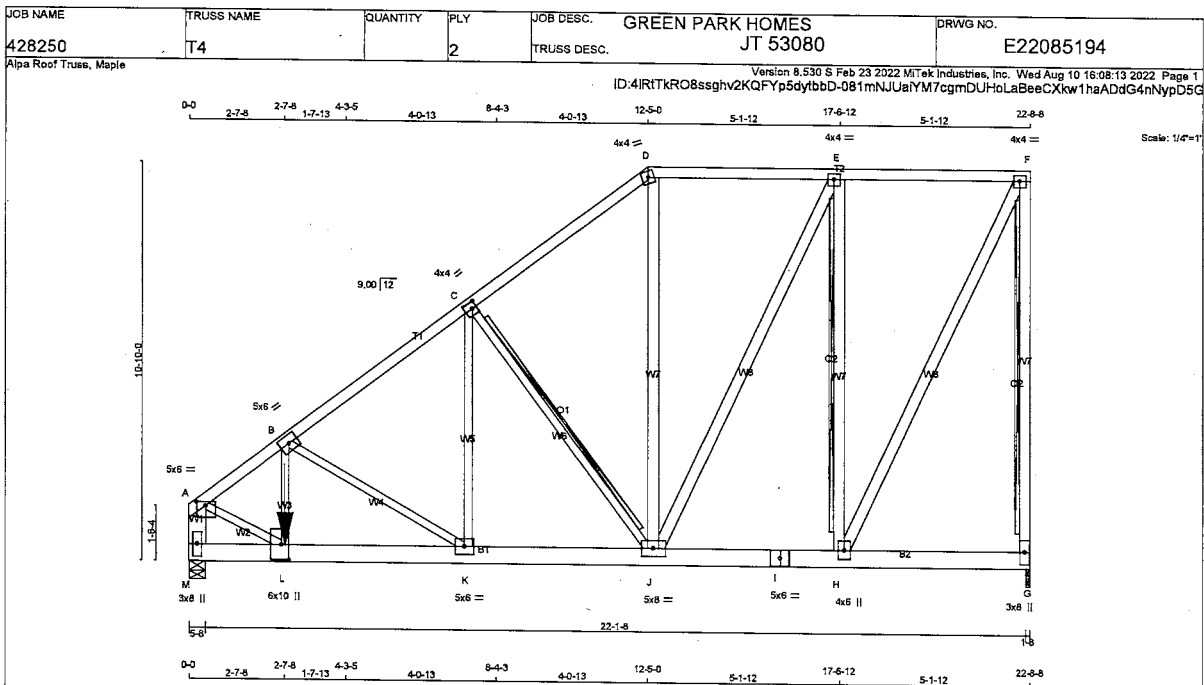
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED	MAX.	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.	MAX.
	FORCE	VERT. LOAD	LC1	LC2		FORCE	VERT. LOAD	LC1	LC2
	(LBS)	(PLF)	(LBS)	(LBS)		(LBS)	(PLF)	(LBS)	(LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		FROM	TO	LENGTH
A-B	-1364 / 0	-95.2	-95.2	0.44 (1)	5.02	K-B	-149 / 39	0.09 (1)	
B-C	-1739 / 0	-95.2	-95.2	0.62 (1)	4.30	A-K	0 / 1121	0.25 (1)	
C-D	-1738 / 0	-95.2	-95.2	0.64 (1)	4.26	H-F	0 / 1752	0.39 (1)	
D-E	-1738 / 0	-95.2	-95.2	0.64 (1)	4.26	B-J	0 / 817	0.18 (1)	
E-F	-1400 / 0	-95.2	-95.2	0.61 (1)	4.69	H-E	-821 / 0	0.35 (1)	
G-F	-1299 / 0	0.0	0.0	0.57 (1)	7.08	J-C	-848 / 0	0.25 (1)	
L-A	-1314 / 0	0.0	0.0	0.14 (1)	7.05	J-E	0 / 425	0.10 (1)	
L-K	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
K-J	0 / 1085	-18.5	-18.5	0.25 (1)	10.00				
J-I	0 / 1400	-18.5	-18.5	0.32 (1)	10.00				
I-H	0 / 1400	-18.5	-18.5	0.32 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.16 (4)	10.00				

TOTAL WEIGHT = 95 lb
DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF
SPACING = 24.0 IN./C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(5% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN/LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEF. (LL) = L/360 (0.79")
CALCULATED VERT. DEF. (LL) = L/999 (0.07")
ALLOWABLE DEF. (TL) = L/360 (0.79")
CALCULATED VERT. DEF. (TL) = L/999 (0.13")
CSI: TC=0.64/1.00 (C-E-1), BC=0.32/1.00 (H-J-1), WB=0.39/1.00 (F-H-1), SS=0.28/1.00 (E-F-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.53 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 2 X 152 = 305 lb			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	DESIGN CRITERIA	
A - D	2x4	DRY	No.2	SPF	SPF	FACTORED	GROSS REACTION	BRG	BRG	*** SPECIAL LOADS ANALYSIS ***	
D - F	2x4	DRY	No.2	SPF	SPF	GROSS REACTION	DOWN	UP	IN-SX	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.	
G - F	2x4	DRY	No.2	SPF	SPF	DOWN	HORIZ	UPLIFT	IN-SX	LOADS WERE DERIVED FROM USER INPUT	
M - A	2x6	DRY	No.2	SPF	SPF	DOWN	0	0	1-8	NO FURTHER MODIFICATIONS WERE MADE	
M - I	2x6	DRY	No.2	SPF	SPF	DOWN	0	0	1-8		
I - G	2x6	DRY	No.2	SPF	SPF	DOWN	0	0	2-8		

ALL WEBS EXCEPT				UNFACTORED REACTIONS				SPECIFIED LOADS:				
J - D	2x4	DRY	No.2	SPF	1ST CASE MAX./MIN. COMPONENT REACTIONS							TOP CH. LL = 26.7 PSF
J - E	2x4	DRY	No.2	SPF	GT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DL = 6.0 PSF
H - E	2x4	DRY	No.2	SPF	M	1195	796 / 0	0 / 0	0 / 0	399 / 0	0 / 0	BOT CH. LL = 0.0 PSF
H - F	2x4	DRY	No.2	SPF	M	3298	2197 / 0	0 / 0	0 / 0	1101 / 0	0 / 0	DL = 7.4 PSF
				SPF								TOTAL LOAD = 40.1 PSF
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M												SPACING = 24.0 IN. C/C
DRY, SEASONED LUMBER.												

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS : (0.122'X3') SPIRAL NAILS

A-D 1 12 TOP

D-F 1 12 TOP

F-G 1 12 TOP

M-A 2 12 TOP

BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS

M-I 2 11 TOP

I-G 2 12 TOP

WEBS : (0.122'X3') SPIRAL NAILS

B-L 1 2 SIDE (155.4)

2x3 1 6 SIDE (89.5)

2x4 1 6

NAILED TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC. LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC. LENGTH (LC)	CS (LC)
FR-TO				FR-TO			
A-B	-4438 / 0	-95.2	-95.2 0.14 (1)	K-C	0 / 1179	0.15 (1)	
B-C	-2355 / 0	-95.2	-95.2 0.24 (1)	C-J	-1388 / 0	0.49 (1)	
C-D	-1382 / 0	-95.2	-95.2 0.21 (1)	J-D	0 / 445	0.04 (1)	
D-E	-1084 / 0	-95.2	-95.2 0.24 (1)	E-J	0 / 843	0.07 (1)	
E-F	-721 / 0	-95.2	-95.2 0.23 (1)	H-E	-1384 / 0	0.24 (1)	
G-F	-1851 / 0	0.0	0.0 0.29 (1)	H-F	0 / 1633	0.14 (1)	
M-A	-4272 / 0	0.0	0.0 0.15 (1)	L-B	0 / 1925	0.24 (1)	
				A-L	0 / 3871	0.48 (1)	
M-L	0 / 0	-250.6	-250.6 0.12 (1)	B-K	-1951 / 0	0.58 (1)	
L-K	0 / 3565	-18.5	-18.5 0.30 (1)				
K-J	0 / 1500	-18.5	-18.5 0.14 (1)				
J-I	0 / 721	-18.5	-18.5 0.06 (1)				
I-H	0 / 721	-18.5	-18.5 0.06 (1)				
H-G	0 / 0	-18.5	-18.5 0.03 (4)				

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS			
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL
L	2-7-8	-3173	-3173	—	FRONT	VERT	CONN.

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

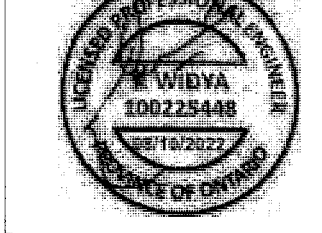
NAIL VALUES				PLATE PLACEMENT TOL = 0.250 inches			
PLATE	GRIP(DRY)	SHEAR	SECTION	PLATE	ROTATION	TOL	5.0 Deg.
(PL)	(PSI)	(PL)	(PSI)				
MAX	MIN	MAX	MIN				
MT20	850	371	1747				

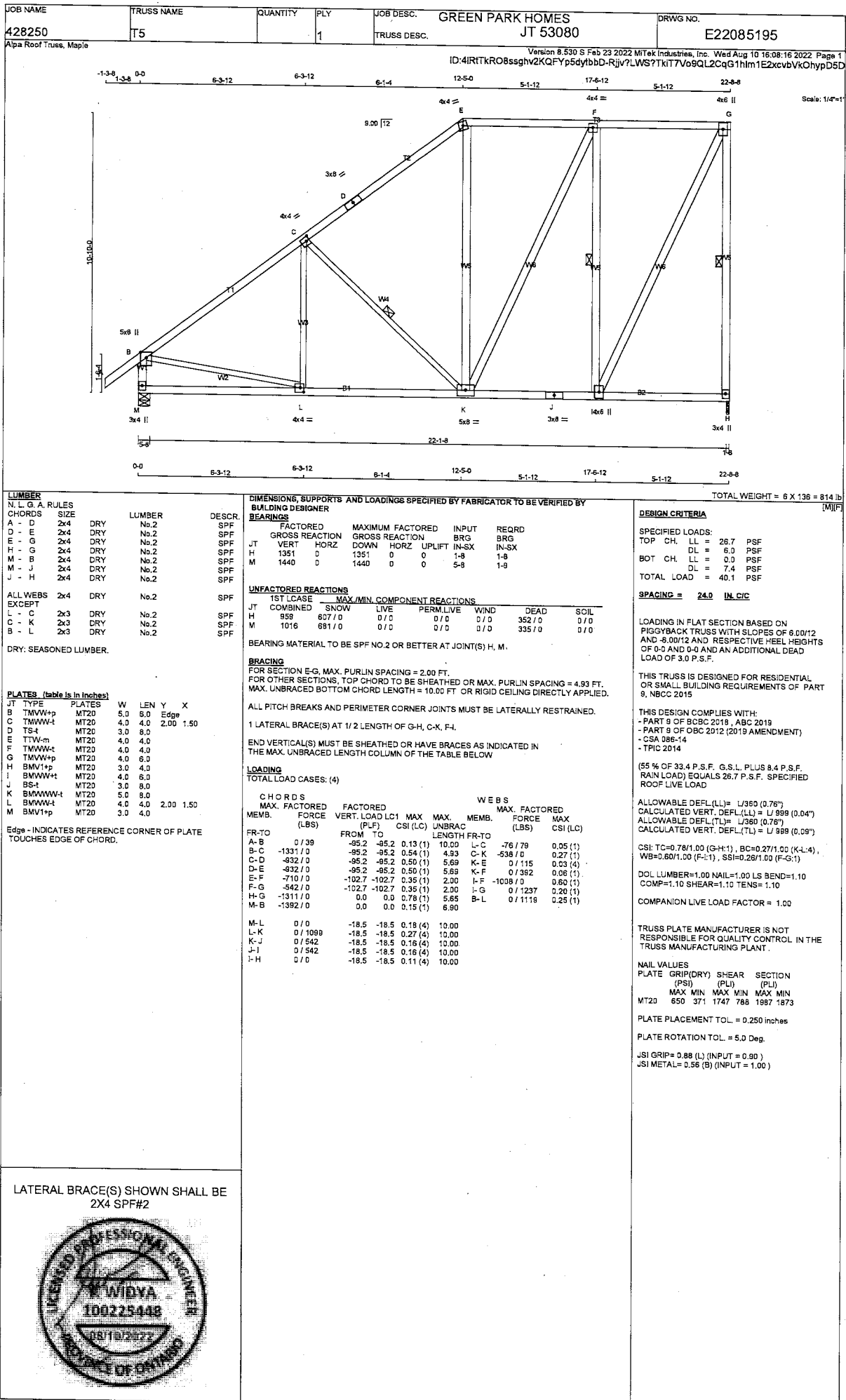
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (F) (INPUT = 0.80)

JSI METAL = 0.54 (L) (INPUT = 1.00)

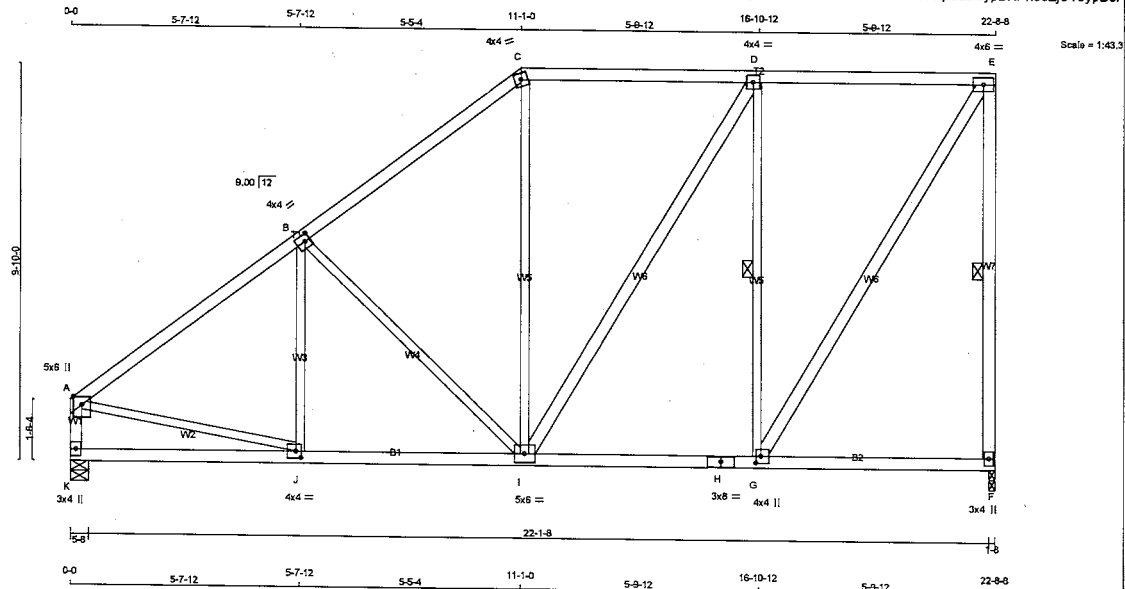




JOB NAME 428250	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085196
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Alpa Roof Truss, Maple

Version 8.530 5 Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:08:19 2022 Page 1
ID:4IRITkRO8ssghv2KQFYp5dybbD-ID1dMYKIO6HKBDNvYpSubwypBRPh3cZjO7DypD5A



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A - TMVW+p	MT20	5.0	6.0	Edge
B - TMVW+H	MT20	4.0	4.0	2.00 1.50
C - TTW-m	MT20	4.0	4.0	
D - TMVW+H	MT20	4.0	4.0	
E - TMVW+H	MT20	4.0	4.0	
F - BMV1+p	MT20	3.0	4.0	
G - BMVW+H	MT20	4.0	4.0	2.00 1.50
H - BS-1	MT20	3.0	6.0	
I - BMVW+H	MT20	5.0	6.0	
J - BMVW+H	MT20	4.0	4.0	2.00 1.50
K - BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
SPF	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
F	1291	0	0	1-8
K	1291	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	911	607 / 0	0 / 0
K	911	607 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	TOTAL LOAD CASES: (4)
CHORDS	
MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)
A-B	-1317 / 0
B-C	-1002 / 0
C-D	-771 / 0
D-E	-636 / 0
E-F	-1248 / 0
K-A	-1248 / 0
K-J	0 / 0
J-I	0 / 1084
I-H	0 / 636
H-G	0 / 636
G-F	0 / 0

WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)
FR-TO	LENGTH
J-B	-114 / 56
B-I	-441 / 0
I-C	0 / 177
I-D	0 / 265
G-E	0 / 1215
A-J	0 / 1108

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
LL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 3 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")
CSI: TC=0.59/1.00 (A-B:1), BC=0.24/1.00 (J-I:1), WS=0.60/1.00 (D-G:1), SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

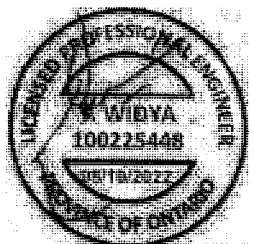
PLATE PLACEMENT TOL. = 0.250 inches

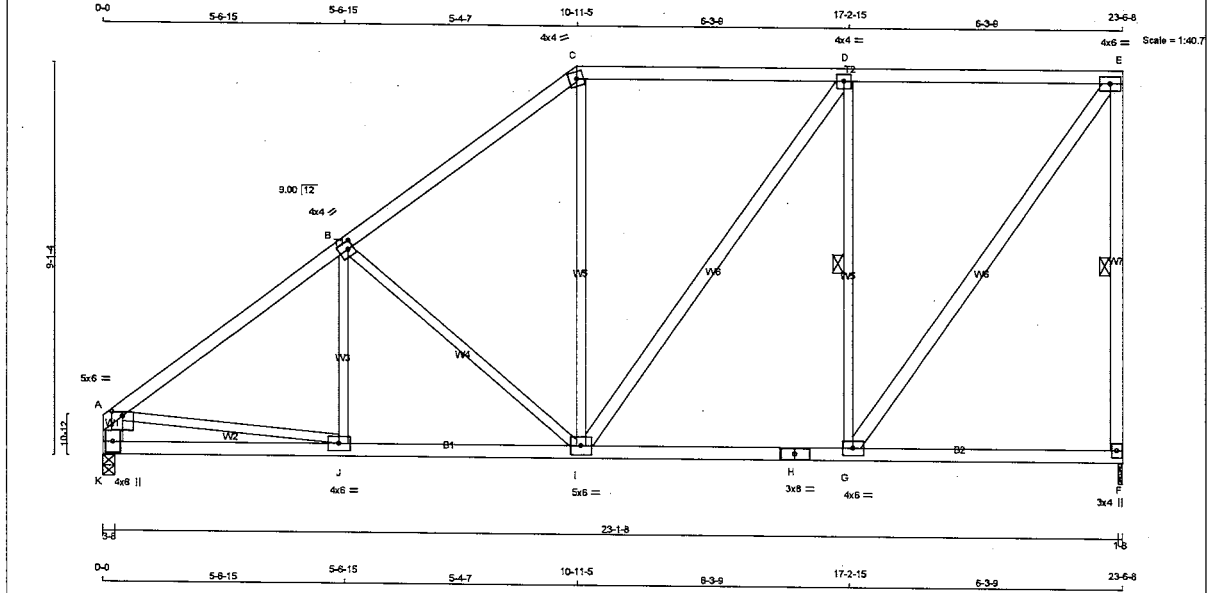
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)

JSI METAL= 0.52 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





TOTAL WEIGHT = 119 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - A	2x6	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
I - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	1.25	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	BMV1-p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1-p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	1338	0	1-8
F 1338	0	0	1-8
K 1338	0	0	3-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F 945	629 / 0 0 / 0 0 / 0 0 / 0 315 / 0 0 / 0
K 945	629 / 0 0 / 0 0 / 0 0 / 0 315 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	-1564 / 0	-95.2	-95.2 0.60 (1)	4.55	J-B	-20 / 85	0.03 (4)
B-C	-1155 / 0	-95.2	-95.2 0.55 (1)	5.16	B-I	-516 / 0	0.53 (1)
C-D	-894 / 0	-95.2	-95.2 0.69 (1)	5.27	I-C	0 / 254	0.08 (1)
D-E	-764 / 0	-95.2	-95.2 0.68 (1)	5.62	I-D	0 / 227	0.04 (1)
F-E	-1291 / 0	0.0	0.0 0.50 (1)	5.58	G-D	-830 / 0	0.50 (1)
K-A	-1295 / 0	0.0	0.0 0.08 (1)	7.81	G-E	0 / 1308	0.21 (1)
K-J	0 / 0	-18.5	-18.5 0.13 (4)	10.00	A-J	0 / 1287	0.29 (1)
J-I	0 / 1281	-18.5	-18.5 0.27 (1)	10.00			
I-H	0 / 764	-18.5	-18.5 0.24 (4)	10.00			
H-G	0 / 764	-18.5	-18.5 0.24 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

LL = 6.0 PSF

BOT CH. LL = 0.0 PSF

LL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 096-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.78")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")

ALLOWABLE DEFL.(TL) = $L/360$ (0.78")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.69/1.00 (C-D:1), BC=0.27/1.00 (I-J:1), WB=0.53/1.00 (B-I:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

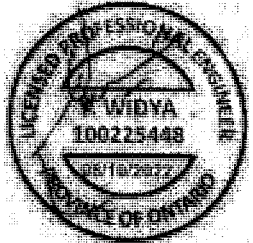
PLATE PLACEMENT TOL. = 0.250 inches

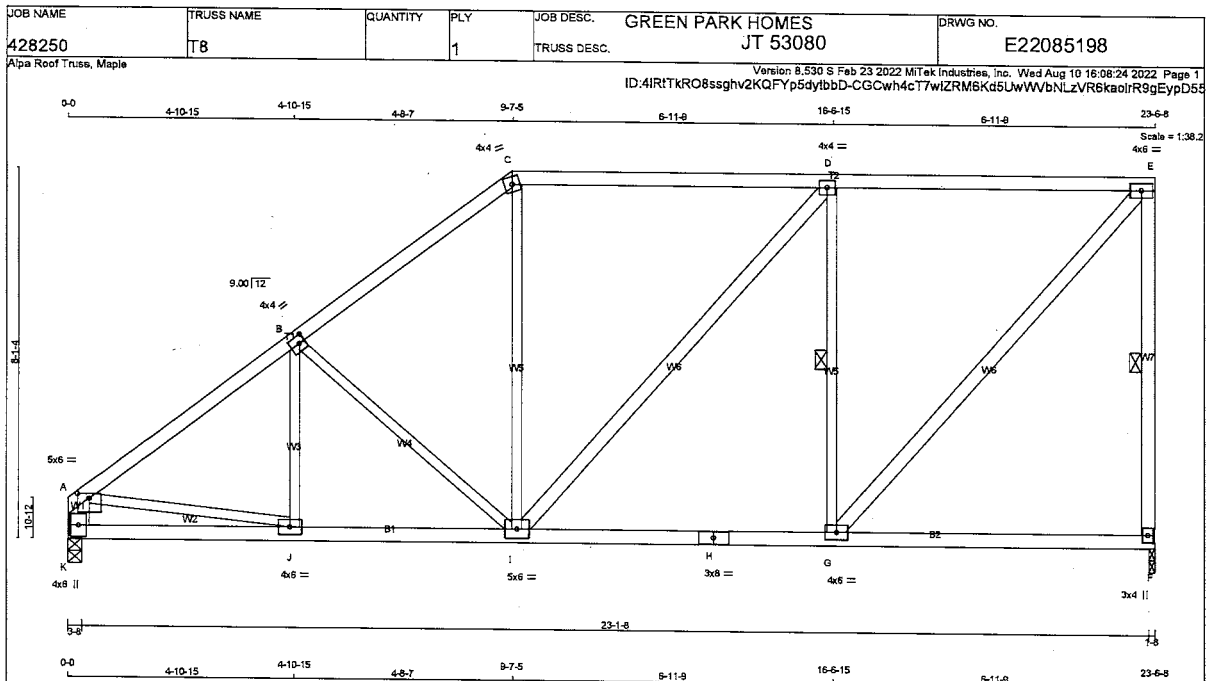
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (E) (INPUT = 0.90)

JSI METAL= 0.32 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER															DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER															TOTAL WEIGHT = 113 lb														
N. L. G. A. RULES															BEARINGS															DESIGN CRITERIA														
CHORDS															FACTORED															SPECIFIED LOADS:														
A - C 2x4 DRY LUMBER No.2 DESCR. SPF															GROSS REACTION															TOP CH. LL = 26.7 PSF														
C - E 2x4 DRY No.2 SPF															MAXIMUM FACTORED															DL = 6.0 PSF														
F - E 2x4 DRY No.2 SPF															INPUT															LL = 0.0 PSF														
K - A 2x6 DRY No.2 SPF															BRG															LL = 7.4 PSF														
K - H 2x4 DRY No.2 SPF															REQD															TOTAL LOAD = 40.1 PSF														
H - F 2x4 DRY No.2 SPF															DOWN																													
															HORIZ																													
															UPLIFT																													
															IN-SX																													
															IN-SX																													
															1-8																													
															1-8																													
															3-8																													
ALL WEBS															UNFACTORED REACTIONS															SPACING = 24.0 IN. C/C														
EXCEPT															1ST CASE																													
I - D 2x4 DRY No.2 SPF															MAX. MIN. COMPONENT REACTIONS																													
G - E 2x4 DRY No.2 SPF																																												
															JT																													
															COMBINED																													
															SNOW																													
															LIVE																													
															PERM. LIVE																													
															WIND																													
															DEAD																													
															SOIL																													
															F																													
															945																													
															629 / 0																													
															0 / 0																													
															0 / 0																													
															0 / 0																													
															315 / 0																													
															0 / 0																													
DRY: SEASONED LUMBER.																														LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM														

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1582 / 0	-95.2 -95.2 0.44 (1)	4.74	J-B	-58 / 58	0.02 (4)	
B-C	-1258 / 0	-95.2 -95.2 0.41 (1)	5.21	B-I	-418 / 0	0.31 (1)	
C-D	-982 / 0	-95.2 -95.2 0.87 (1)	4.52	I-C	0 / 315	0.07 (1)	
D-E	-929 / 0	-95.2 -95.2 0.86 (1)	4.63	I-D	0 / 83	0.01 (1)	
F-E	-1287 / 0	0.0 0.0 0.38 (1)	5.69	G-D	-886 / 0	0.37 (1)	
K-A	-1298 / 0	0.0 0.0 0.08 (1)	7.81	G-E	0 / 1389	0.22 (1)	
K-J	0 / 0	-18.5 -18.5 0.09 (4)	10.00	A-J	0 / 1300	0.29 (1)	
J-I	0 / 1291	-18.5 -18.5 0.27 (1)	10.00				
I-H	0 / 929	-18.5 -18.5 0.30 (4)	10.00				
H-G	0 / 928	-18.5 -18.5 0.30 (4)	10.00				
G-F	0 / 0	-18.5 -18.5 0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.78")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.78")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.87/1.00 (C-D-1), BC=0.30/1.00 (G-H-4), WB=0.37/1.00 (D-G-1), SS=0.32/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

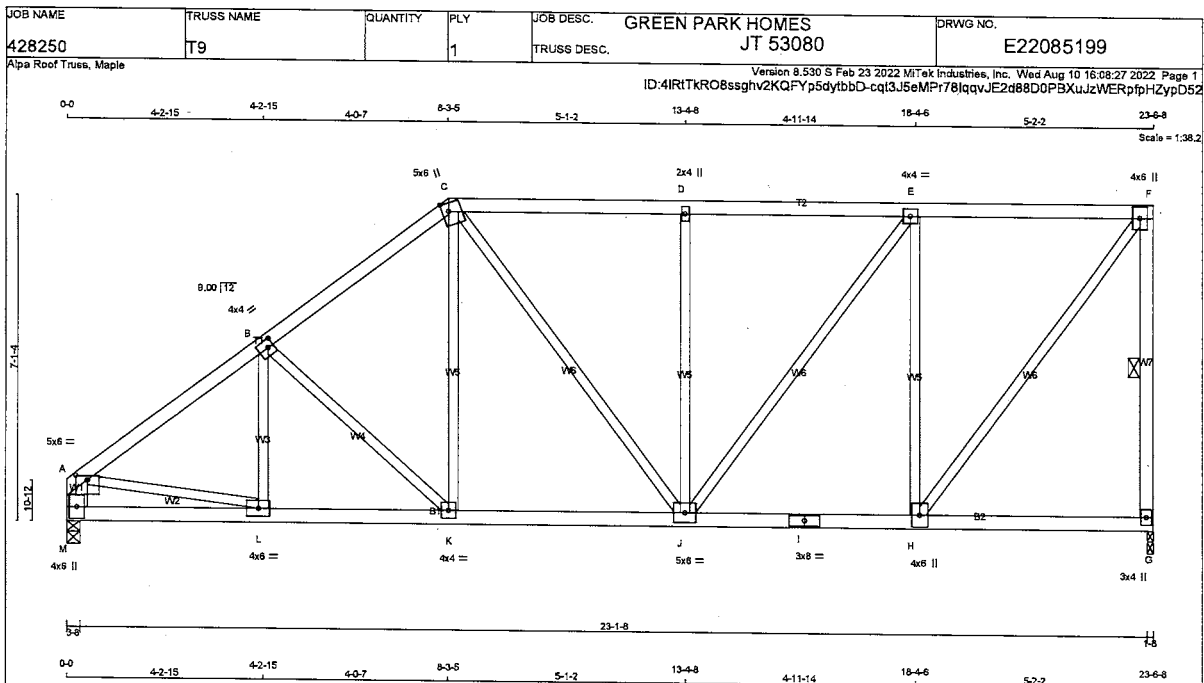
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	SPF
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - A	2x6	DRY	No.2
M - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+P	MT20	5.0	6.0	1.25	3.00
B	TMWW+L	MT20	4.0	4.0	2.00	1.50
C	TMWW+M	MT20	5.0	6.0	2.25	1.75
D	TMWW+W	MT20	2.0	4.0		
E	TMWW+L	MT20	4.0	4.0		
F	TMWW+P	MT20	4.0	6.0		
G	BMV1+P	MT20	3.0	4.0		
H	BMWW+L	MT20	4.0	6.0		
I	BS-L	MT20	3.0	8.0		
J	BMWW+L	MT20	5.0	6.0		
K	BMWW+L	MT20	4.0	4.0		
L	BMWW+L	MT20	4.0	6.0		
M	BMV1+P	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1338	0	1338	0	0	1-8	1-8
M	1338	0	1338	0	0	3-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
G	945	629 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
M	945	629 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY AP

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally Restrained

1 Lateral Brace(s) at 1/2 Length of F.G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	MAX. UNBRACED LENGTH (L)	
FR-TO					FR-TO				
A-B	-1593 / 0	-95.2	-95.2	0.32 (1)	4.90	L-B	-97 / 98	0.02 (1)	
B-C	-1354 / 0	-95.2	-95.2	0.30 (1)	5.22	B-K	-320 / 0	0.17 (1)	
C-D	-1172 / 0	-95.2	-95.2	0.35 (1)	5.43	K-C	0 / 308	0.07 (1)	
D-E	-1172 / 0	-95.2	-95.2	0.37 (1)	5.40	A-L	0 / 1307	0.29 (1)	
E-F	-836 / 0	-95.2	-95.2	0.35 (1)	6.15	H-F	0 / 1381	0.31 (1)	
F-G	-1299 / 0	0.0	0.0	0.30 (1)	5.68	C-J	0 / 182	0.04 (1)	
M-A	-1302 / 0	0.0	0.0	0.08 (1)	7.81	H-E	-593 / 0	0.87 (1)	
M-L	0 / 0	-18.5	-18.5	0.07 (4)	10.00	J-D	-619 / 0	0.45 (1)	
L-K	0 / 1295	-18.5	-18.5	0.25 (1)	10.00	J-E	0 / 570	0.13 (1)	
K-J	0 / 1063	-18.5	-18.5	0.22 (1)	10.00				
J-I	0 / 836	-18.5	-18.5	0.20 (1)	10.00				
I-H	0 / 836	-18.5	-18.5	0.20 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.37/1.00 (D-E-1), BC=0.25/1.00 (K-L-1), WB=0.87/1.00 (E-H-1), SSI=0.23/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

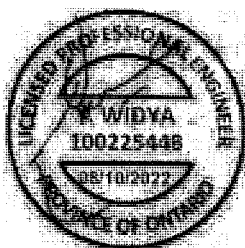
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 1.00)

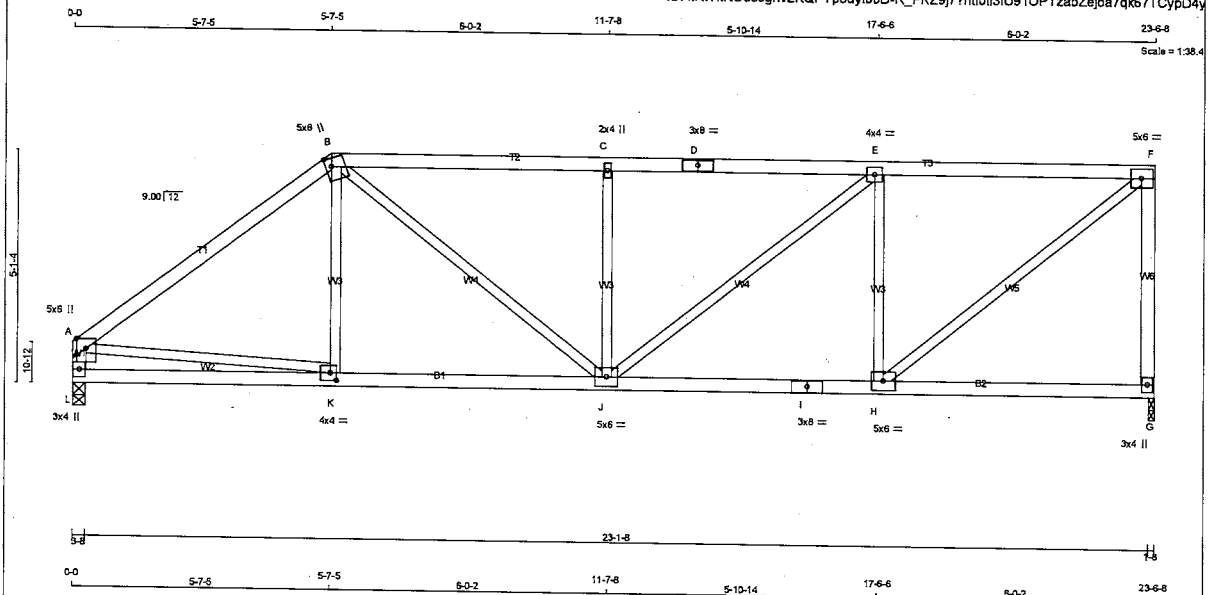
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085201
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Alpa Roof Truss, Maple

Version 8.530 8 Feb 23 2022 Mitak Industries, Inc. Wed Aug 10 16:08:33 2022 Page 1
ID:4IRITkRO8ssghv2KQFYp5dybbD-R_FKZ9j77ht0i0i3U810PTzabZeja7qk67TCypD4y



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - A	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
G	1338 0	1338 0	0 1-8	1-8
L	1338 0	1338 0	0 3-8	1-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
G	945	629 / 0 0 / 0 0 / 0 0 / 0 315 / 0 0 / 0
L	945	629 / 0 0 / 0 0 / 0 0 / 0 315 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
		FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)		FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)
	FR-TO		FROM TO			FR-TO		
	A-B	-1511 / 0	-95.2	-95.2 0.84 (1)		K-B	0 / 96	0.03 (4)
	B-C	-1711 / 0	-95.2	-95.2 0.55 (1)		B-J	0 / 545	0.15 (1)
	C-D	-1711 / 0	-95.2	-95.2 0.56 (1)		J-C	-618 / 0	0.24 (1)
	D-E	-1711 / 0	-95.2	-95.2 0.56 (1)		J-E	0 / 497	0.11 (1)
	E-F	-1326 / 0	-95.2	-95.2 0.52 (1)		H-E	-937 / 0	0.36 (1)
	G-F	-1293 / 0	0.0	0.0 0.57 (1)		H-F	0 / 1899	0.38 (1)
	L-A	-1296 / 0	0.0	0.0 0.13 (1)		A-K	0 / 1213	0.27 (1)
	L-K	0 / 0	-18.5	-18.5 0.15 (4)				
	K-J	0 / 1207	-18.5	-18.5 0.28 (1)				
	J-I	0 / 1326	-18.5	-18.5 0.30 (1)				
	I-H	0 / 1326	-18.5	-18.5 0.30 (1)				
	H-G	0 / 0	-18.5	-18.5 0.16 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2019, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")
CSI: TC=0.64/1.00 (A-B:1), BC=0.30/1.00 (H-J:1), WB=0.38/1.00 (F-H:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

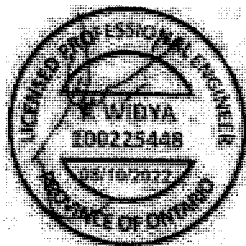
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

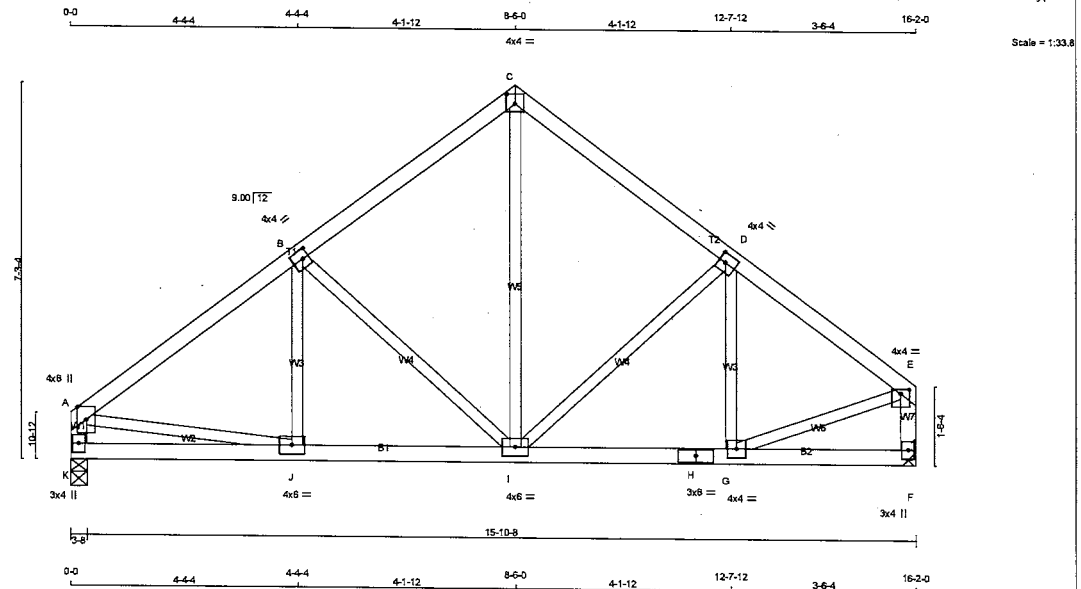
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.80)
JSI METAL= 0.59 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	8.0	Edge
B	TMWW-t	MT20	4.0	4.0	2.00 1.50
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMWW-t	MT20	4.0	4.0	2.00 1.50
E	TMWW-p	MT20	4.0	4.0	1.00 2.00
F	BMV+p	MT20	3.0	4.0	
G	BMWW-t	MT20	4.0	4.0	
H	BS-t	MT20	3.0	8.0	
I	BMWW-t	MT20	4.0	6.0	
J	BMWW-t	MT20	4.0	6.0	
K	BMV+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
K	VERT	DOWN	UPLIFT	IN-SX
F	919	0	0	3-8
F	919	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	648	432/0	0/0	0/0	0/0	217/0	0/0
F	648	432/0	0/0	0/0	0/0	217/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-999/0	-95.2 -95.2 0.23 (1)	5.99	J-I	-24/63	0.02 (4)	
B-C	-701/0	-95.2 -95.2 0.22 (1)	6.25	B-I	-387/0	0.21 (1)	
C-D	-696/0	-95.2 -95.2 0.19 (1)	6.25	I-C	0/501	0.11 (1)	
D-E	-857/0	-95.2 -95.2 0.18 (1)	6.25	I-D	-233/0	0.13 (1)	
E-F	-892/0	0.0 0.0 0.08 (1)	7.81	G-D	-166/9	0.04 (1)	
K-A	-684/0	0.0 0.0 0.08 (1)	7.81	G-E	0/748	0.17 (1)	
K-J	0/0	-18.5 -18.5 0.07 (4)	10.00	A-J	0/829	0.19 (1)	
J-I	0/822	-18.5 -18.5 0.17 (1)	10.00				
I-H	0/709	-18.5 -18.5 0.15 (1)	10.00				
H-G	0/709	-18.5 -18.5 0.15 (1)	10.00				
G-F	0/0	-18.5 -18.5 0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2019, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.23/1.00 (A-B-1), BC=0.17/1.00 (J-I-1), WB=0.21/1.00 (B-I-1), SS=0.16/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

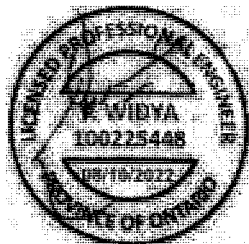
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1673

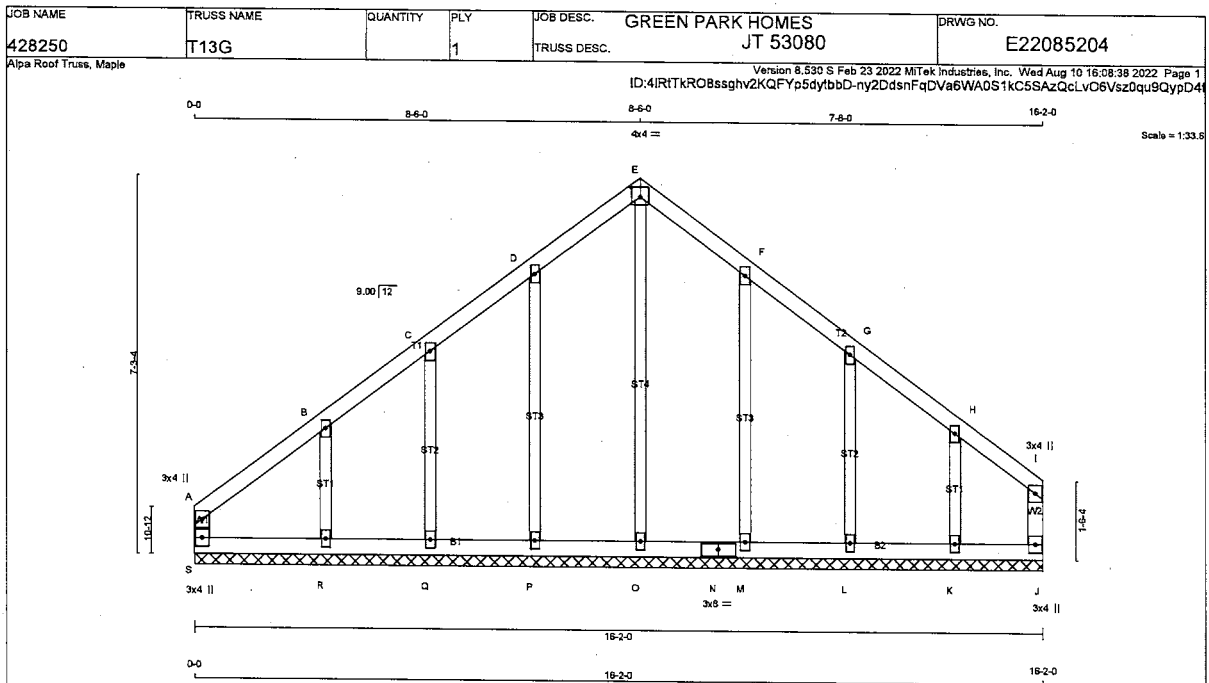
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
S - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMV+p	MT20	3.0	4.0		
B, C, D, F, G, H					
B - TMW+w	MT20	2.0	4.0		
E - TTW+p	MT20	4.0	4.0	2.25	2.00
I - TMV+p	MT20	3.0	4.0		
J - BMV1+p	MT20	3.0	4.0		
K, L, M, O, P, Q, R					
K - BMW1+w	MT20	2.0	4.0		
N - BS-1	MT20	3.0	8.0		
S - BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
S-A	-139 / 0	0.0	0.04 (1)	O-E	-162 / 0	0.15 (1)	7.81
A-B	-48 / 0	-95.2	-95.2 0.08 (1)	P-D	-193 / 0	0.10 (1)	6.25
B-C	-47 / 0	-95.2	-95.2 0.08 (1)	Q-C	-177 / 0	0.05 (1)	6.25
C-D	-35 / 0	-95.2	-95.2 0.04 (1)	R-B	-219 / 0	0.04 (1)	6.25
D-E	-32 / 0	-95.2	-95.2 0.05 (1)	M-F	-190 / 0	0.10 (1)	6.25
E-F	-32 / 0	-95.2	-95.2 0.05 (1)	L-G	-192 / 0	0.05 (1)	6.25
F-G	-37 / 0	-95.2	-95.2 0.05 (1)	K-H	-160 / 0	0.03 (1)	6.25
G-H	-39 / 0	-95.2	-95.2 0.05 (1)				
H-I	-50 / 0	-95.2	-95.2 0.04 (1)				
J-I	-106 / 0	0.0	0.04 (1)				7.81
S-R	0 / 41	-18.5	-18.5 0.03 (4)				10.00
R-Q	0 / 33	-18.5	-18.5 0.03 (4)				10.00
Q-P	0 / 28	-18.5	-18.5 0.02 (4)				10.00
P-O	0 / 26	-18.5	-18.5 0.02 (4)				10.00
O-N	0 / 26	-18.5	-18.5 0.02 (4)				10.00
N-M	0 / 26	-18.5	-18.5 0.02 (4)				10.00
M-L	0 / 29	-18.5	-18.5 0.02 (4)				10.00
L-K	0 / 34	-18.5	-18.5 0.02 (4)				10.00
K-J	0 / 40	-18.5	-18.5 0.03 (1)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2019, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1), BC=0.03/1.00 (J-K:1), WB=0.15/1.00 (E-G:1), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	550	371	1747	789	1987

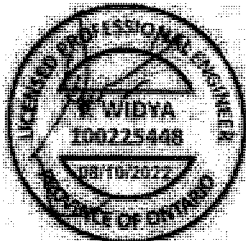
PLATE PLACEMENT TOL. = 0.250 inches

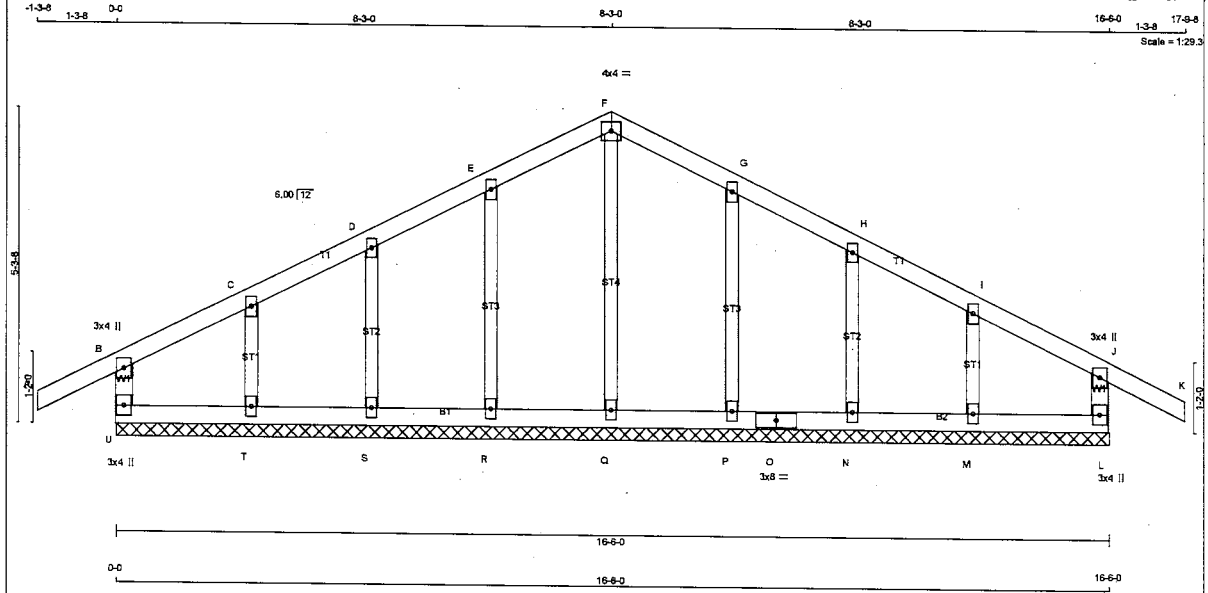
PLATE ROTATION TDL. = 5.0 Deg.

JSI GRIP= 0.46 (E) (INPUT = 0.90)

JSI METAL = 0.12 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





TOTAL WEIGHT = 62 lb

LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER DESCR.
U - B 2x4 DRY No.2 SPF
A - F 2x4 DRY No.2 SPF
P - K 2x4 DRY No.2 SPF
L - J 2x4 DRY No.2 SPF
U - O 2x4 DRY No.2 SPF
O - L 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, E, G, H, I					
C TMW+rw	MT20	2.0	4.0		
F TMW+p	MT20	4.0	4.0		
J TMV+p	MT20	3.0	4.0		
L BMV1+p	MT20	3.0	4.0		
M, N, P, Q, R, S, T					
W BWV1+rw	MT20	2.0	4.0		
O BS-1	MT20	3.0	8.0		
U BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS: (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	CS: (LC)
FR-TO				FR-TO			
U-B	-248 / 0	0.0	0.02 (1)	Q-F	-187 / 0	7.81	0.08 (1)
A-B	0 / 29	-95.2	-95.2 (1)	R-E	-193 / 0	10.00	0.05 (1)
B-C	-28 / 0	-95.2	-95.2 (1)	S-D	-188 / 0	6.25	0.04 (1)
C-D	-20 / 0	-95.2	-95.2 (1)	T-C	-195 / 0	6.25	0.03 (1)
D-E	-14 / 0	-95.2	-95.2 (1)	P-G	-193 / 0	6.25	0.05 (1)
E-F	-11 / 0	-95.2	-95.2 (1)	N-H	-188 / 0	6.25	0.04 (1)
F-G	-11 / 0	-95.2	-95.2 (1)	M-I	-195 / 0	6.25	0.03 (1)
G-H	-14 / 0	-95.2	-95.2 (1)				
H-I	-20 / 0	-95.2	-95.2 (1)				
I-J	-28 / 0	-95.2	-95.2 (1)				
J-K	0 / 29	-95.2	-95.2 (1)				
L-J	-248 / 0	0.0	0.02 (1)				
U-T	0 / 24	-18.5	-18.5 (4)				
T-S	0 / 17	-18.5	-18.5 (4)				
S-R	0 / 13	-18.5	-18.5 (4)				
R-Q	0 / 10	-18.5	-18.5 (4)				
Q-P	0 / 10	-18.5	-18.5 (4)				
P-O	0 / 13	-18.5	-18.5 (4)				
O-N	0 / 13	-18.5	-18.5 (4)				
N-M	0 / 17	-18.5	-18.5 (4)				
M-L	0 / 24	-18.5	-18.5 (4)				

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF
SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % CF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (T-U:4), WB=0.08/1.00 (F-Q:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

CDMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

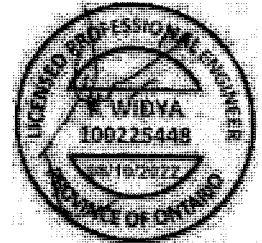
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

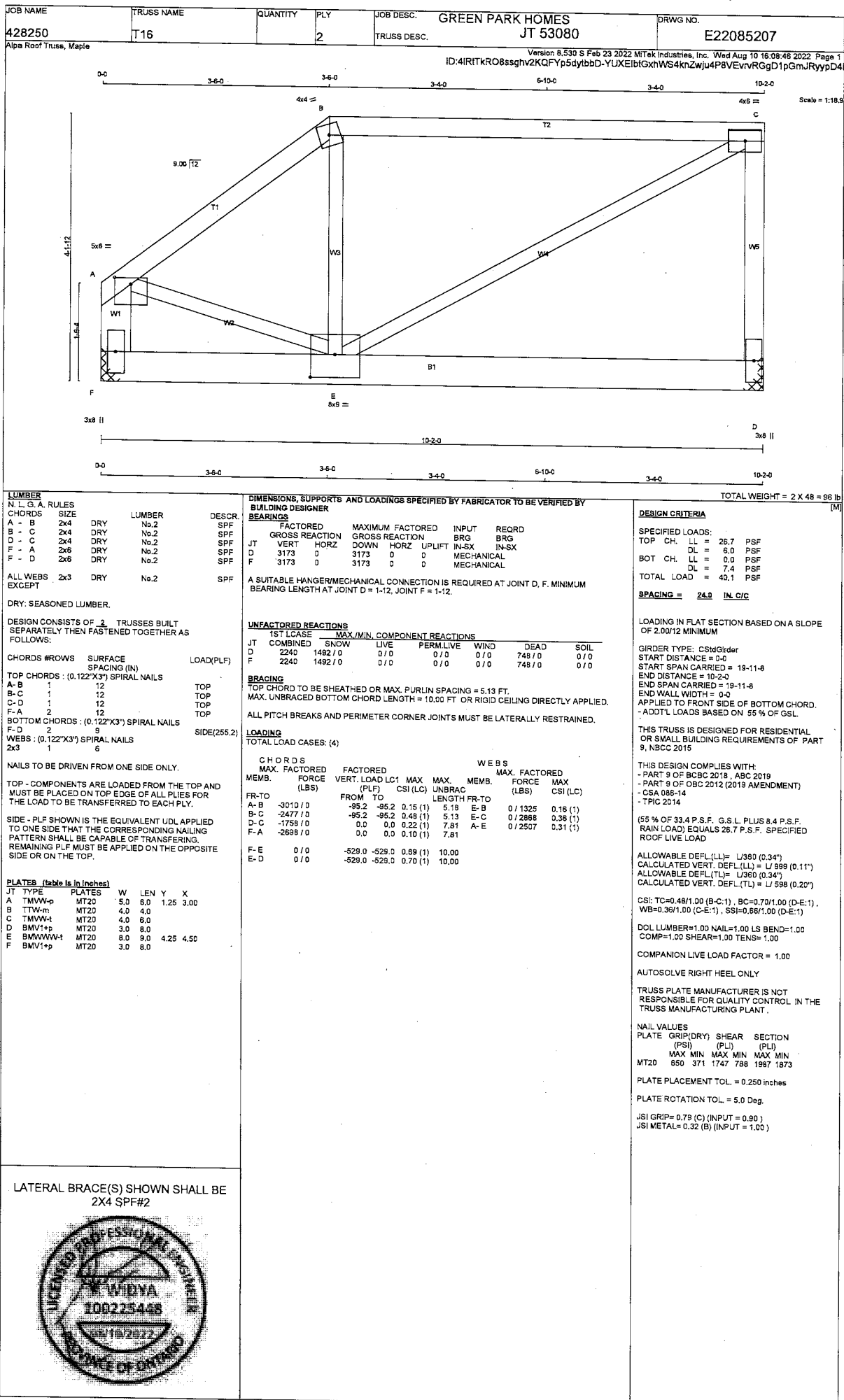
PLATE PLACEMENT TOL. = 0.250 inches

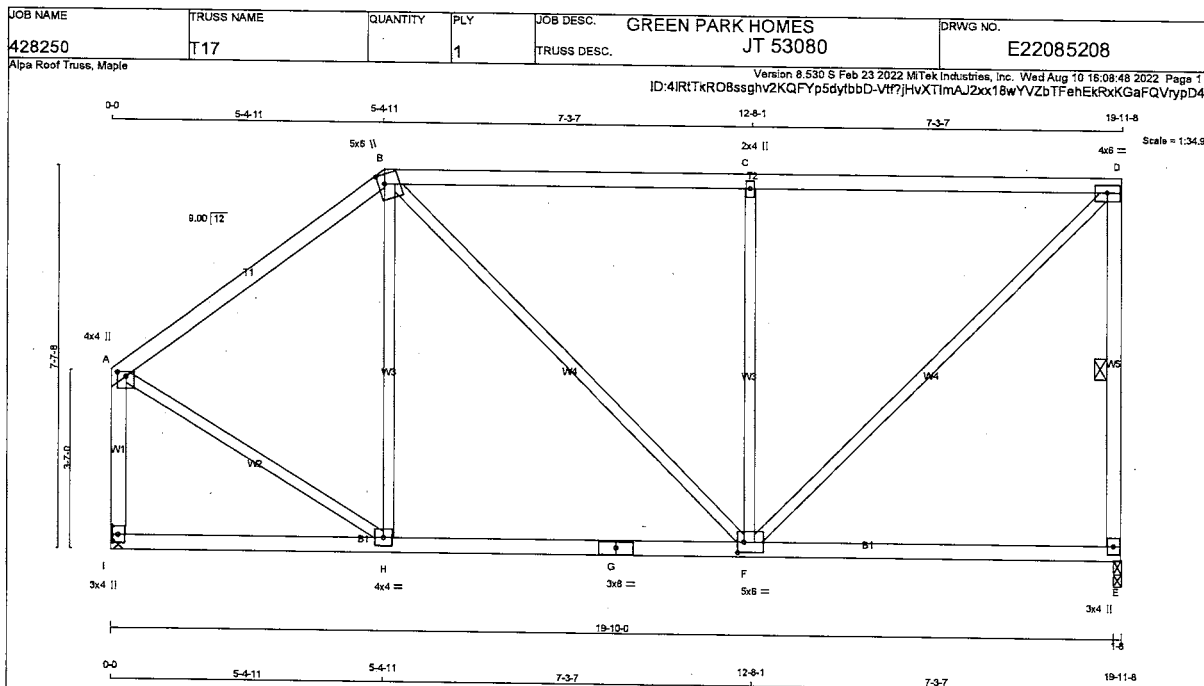
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (F) (INPUT = 0.90)
JSI METAL= 0.08 (U) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2







N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
I - A	2x4	DRY	No.2
I - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMVW+p	MT20	4.0	4.0	1.00 2.00
B TTWW+m	MT20	5.0	6.0	2.25 1.50
C TMVW+w	MT20	2.0	4.0	
D TMVW+I	MT20	4.0	6.0	
E BMV1+p	MT20	3.0	4.0	
F BMVWW+I	MT20	5.0	6.0	2.50 1.50
G BS-I	MT20	3.0	6.0	
H BMVW+I	MT20	4.0	4.0	
I BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
E	VERT 1135	DOWN 1135	0	1-8
I	HORZ 1135	UP/LIFT 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	801	534 / 0	0 / 0	0 / 0	0 / 0	267 / 0	0 / 0
I	801	534 / 0	0 / 0	0 / 0	0 / 0	267 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX. FACTORED	FORCE (LBS)	CS1 (LC)	UNBRAC	WEBS	MEMB.	MAX. FACTORED	FORCE (LBS)	CS1 (LC)
FR-TO														
A-B		-781 / 0		-95.2		-95.2		0.52 (1)	6.01	H-B		-244 / 13		0.26 (1)
B-C		-815 / 0		-95.2		-95.2		0.93 (1)	4.47	B-F		0 / 276		0.06 (1)
C-D		-815 / 0		-95.2		-95.2		0.93 (1)	4.47	F-C		-862 / 0		0.92 (1)
E-D		-1080 / 0		0.0		0.0		0.28 (1)	6.08	F-D		0 / 1153		0.26 (1)
I-A		-1098 / 0		0.0		0.0		0.24 (1)	7.55	A-H		0 / 725		0.16 (1)
I-H		0 / 0		-18.5		-18.5		0.16 (4)	10.00					
H-G		0 / 620		-18.5		-18.5		0.29 (4)	10.00					
G-F		0 / 620		-18.5		-18.5		0.29 (4)	10.00					
F-E		0 / 0		-18.5		-18.5		0.24 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	26.7	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.93/1.00 (C-D-1), BC=0.28/1.00 (F-H-4), WB=0.92/1.00 (C-F-1), SS=0.34/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650 371 1747 788 1987 1873	

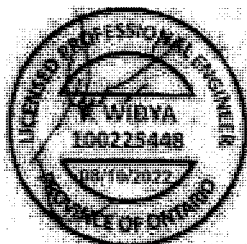
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.80)

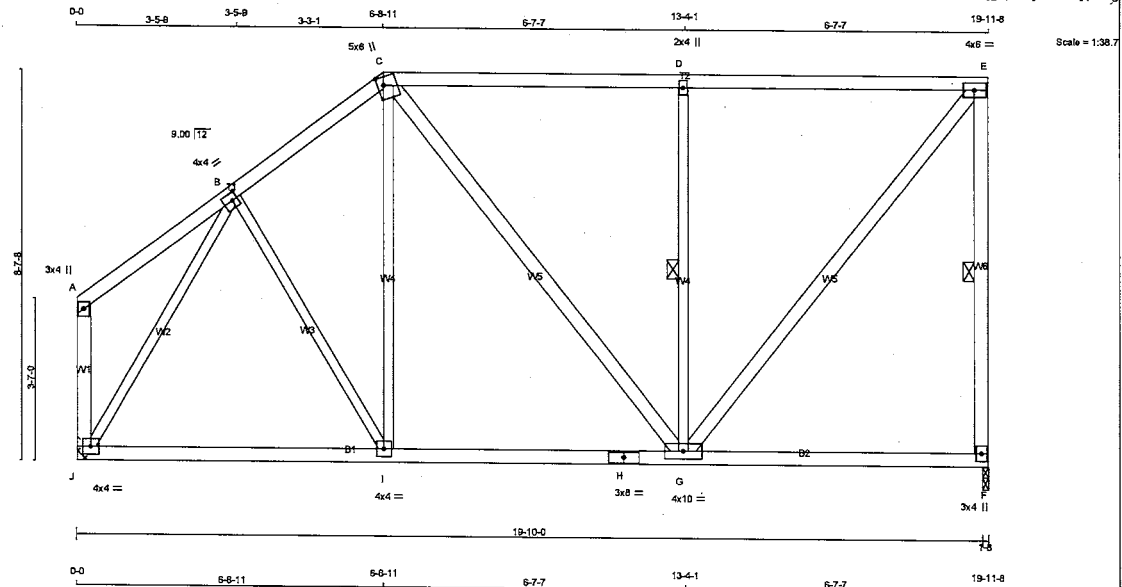
JSI METAL= 0.28 (F) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRtKr08ssghv2KQFyp5dybbD-VSK7LxPmD8kAWgVjGTF6CD1RsjgxpEmyYT46AppD4g



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0	
B	TMWW-t	MT20	4.0	4.0	2.00 1.50
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMWW-w	MT20	2.0	4.0	
E	TMWW-t	MT20	4.0	6.0	
F	BMV+p	MT20	3.0	4.0	
G	BMWW-t	MT20	4.0	10.0	
H	BS-t	MT20	3.0	8.0	
I	BMWW-t	MT20	4.0	4.0	
J	BMWW-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION		GROSS REACTION	BRG	BRG
JT VERT	DOWN	DOWN	UPLIFT	IN-SX
F	1135	0	0	1-8
J	1135	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM. LIVE
F	801	534 / 0	0 / 0
J	801	534 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	WEBS	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
FR-TO									FR-TO							
A-B		0 / 22	-95.2	-95.2	0.17 (1)	10.00	0.03 (4)		B-I		0 / 114	0.03 (4)				
B-C		-797 / 0	-95.2	-95.2	0.18 (1)	6.25	0.03 (4)		I-C		0 / 88	0.03 (4)				
C-D		-674 / 0	-95.2	-95.2	0.74 (1)	5.64	0.01 (1)		C-G		0 / 89	0.01 (1)				
D-E		-674 / 0	-95.2	-95.2	0.75 (1)	5.64	0.01 (1)		G-D		-793 / 0	0.37 (1)				
E-F		-1084 / 0	0.0	0.0	0.37 (1)	6.07	0.17 (1)		G-E		0 / 1080	0.17 (1)				
J-A		-125 / 0	0.0	0.0	0.03 (1)	7.81	0.88 (1)		J-B		-1110 / 0	0.88 (1)				
J-I		0 / 580	-18.5	-18.5	0.23 (4)	10.00										
I-H		0 / 619	-18.5	-18.5	0.24 (4)	10.00										
H-G		0 / 619	-18.5	-18.5	0.24 (4)	10.00										
G-F		0 / 0	-18.5	-18.5	0.18 (4)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(95 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.67")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.67")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.75/1.00 (D-E:1), BC=0.24/1.00 (G-I:4), WB=0.88/1.00 (B-J:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

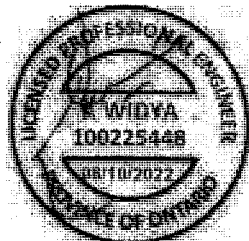
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)

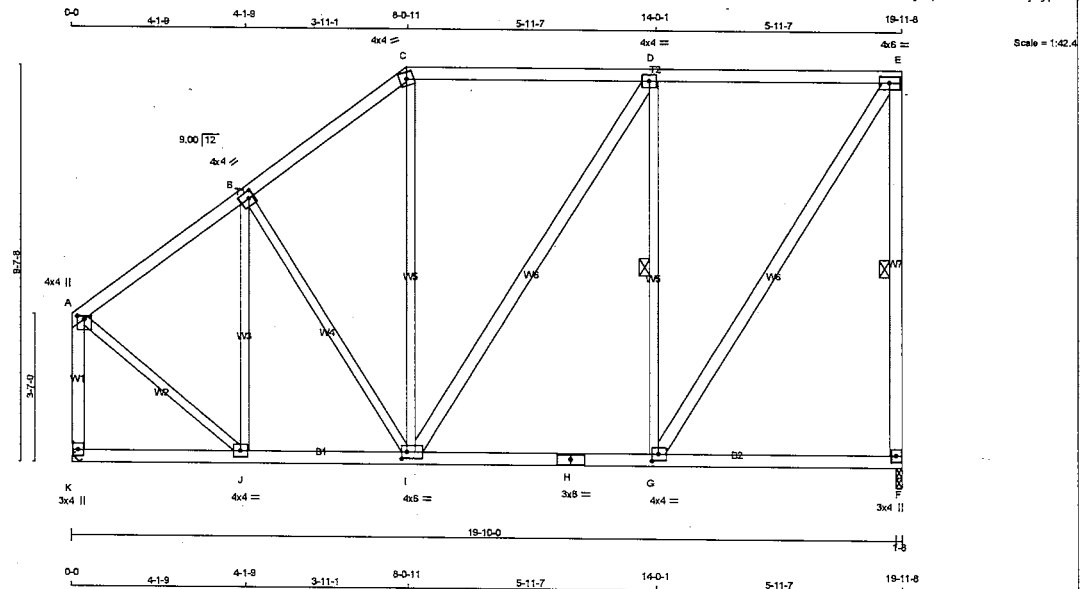
JSI METAL= 0.34 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRtkR08ssghv2KQFyp5dytbbD-K10GzKz128XJ1z0500ytkqrZ63lu8GYCvMjVypD4g



LUMBER	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMVW-i	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-i	MT20	4.0	4.0		
E	TMVW-i	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-i	MT20	4.0	4.0	2.00	1.75
H	BS-i	MT20	3.0	8.0		
I	BMVW-i	MT20	4.0	8.0	2.00	1.50
J	BMVW-i	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	1135	0	1135	0
K	1135	0	1135	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	801	534/0	0/0	0/0	0/0	287/0	0/0
K	801	534/0	0/0	0/0	0/0	287/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX.	MEMB.	MAX.	FACTORED	MAX
MEMB.	FORCE	VERT. LOAD	LC1	CS1	UNBRAC	FORCE	CS1	CS1 (LC)
(LBS)	(PLF)	(LC)	(LC)	(LC)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	-748/0	-95.2	-95.2	0.27 (1)	6.25	J-B	-406/0	0.30 (1)
B-C	-764/0	-95.2	-95.2	0.27 (1)	6.25	B-I	-59/0	0.07 (1)
C-D	-588/0	-95.2	-95.2	0.59 (1)	6.25	I-C	0/91	0.03 (4)
D-E	-561/0	-95.2	-95.2	0.59 (1)	6.25	I-D	0/51	0.01 (1)
E-F	-1091/0	0.0	0.0	0.48 (1)	5.05	G-D	-747/0	0.46 (1)
K-A	-1103/0	0.0	0.0	0.24 (1)	7.54	G-E	0/1039	0.17 (1)
K-J	0/0	-18.5	-18.5	0.07 (4)	10.00	A-J	0/787	0.18 (1)
J-I	0/519	-18.5	-18.5	0.14 (1)	10.00			
I-H	0/581	-18.5	-18.5	0.21 (4)	10.00			
H-G	0/581	-18.5	-18.5	0.21 (4)	10.00			
G-F	0/0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.67")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.67")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CS1: TC=0.59/1.00 (C-D-1), BC=0.21/1.00 (G-H-4), WB=0.48/1.00 (D-G-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

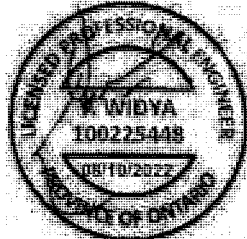
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

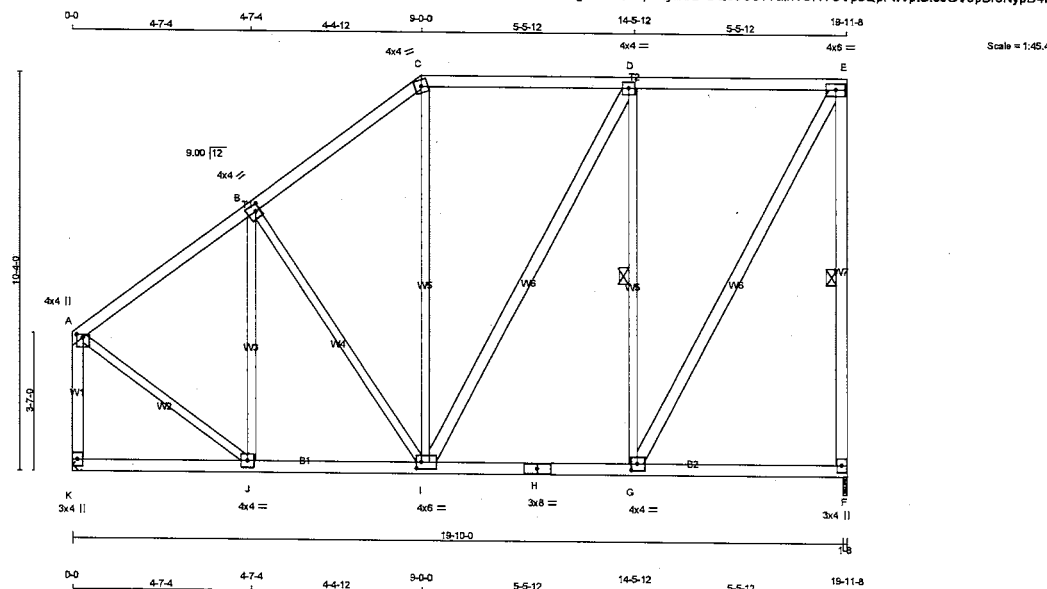
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085211
428250	T20		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:08:56 2022 Page 1
ID:4IR1TKRO8ssghv2KQFYp5dybbD-GQ800D?YainfGHYUvP3QpFwvptStc9GV6pBroNypD4b



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	4.0	4.0	1.00 2.00
B	TMWV+1	MT20	4.0	4.0	2.00 1.50
C	TTW-m	MT20	4.0	4.0	
D	TMWV+1	MT20	4.0	4.0	
E	TMWV+1	MT20	4.0	8.0	
F	BMV+1+p	MT20	3.0	4.0	
G	BMWV+1	MT20	4.0	4.0	2.00 1.75
H	BS-1	MT20	3.0	8.0	
I	BMWVW+1	MT20	4.0	6.0	2.00 1.50
J	BMWV+1	MT20	4.0	4.0	
K	BMV+1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
F	1135	0	1135	0
K	1135	0	1135	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
F	801	534 / 0	0 / 0
K	801	534 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-769 / 0	-95.2 -95.2 0.35 (1)	J-B	-358 / 0
B-C	-734 / 0	-95.2 -95.2 0.34 (1)	B-I	-142 / 0
C-D	-561 / 0	-95.2 -95.2 0.49 (1)	I-C	0 / 78
D-E	-492 / 0	-95.2 -95.2 0.49 (1)	I-D	0 / 148
E-F	-1094 / 0	0.0 0.0 0.58 (1)	G-D	-777 / 0
K-A	-1100 / 0	0.0 0.0 0.24 (1)	G-E	0 / 1020
			A-J	0 / 783
K-J	0 / 0	-18.5 -18.5 0.09 (4)		
J-I	0 / 639	-18.5 -18.5 0.15 (1)		
I-H	0 / 492	-18.5 -18.5 0.18 (4)		
H-G	0 / 492	-18.5 -18.5 0.18 (4)		
G-F	0 / 0	-18.5 -18.5 0.14 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN/CI

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.58/1.00 (E-F-1), BC=0.18/1.00 (G-I-4),
WB=0.57/1.00 (D-G-1), SSI=0.25/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

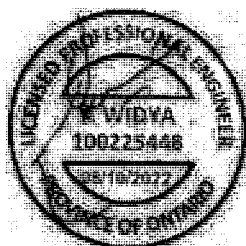
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

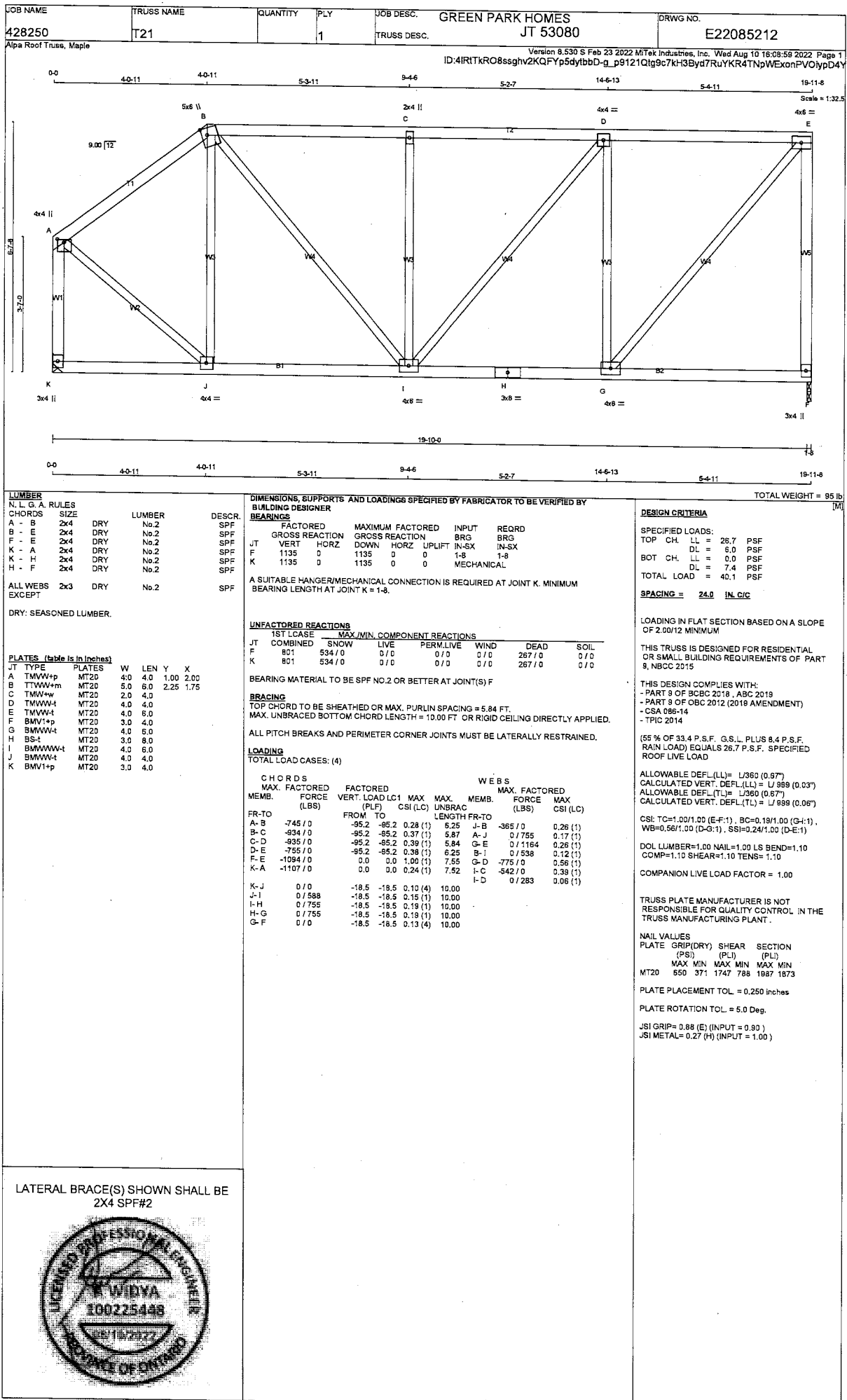
PLATE PLACEMENT TOL. = 0.250 inches

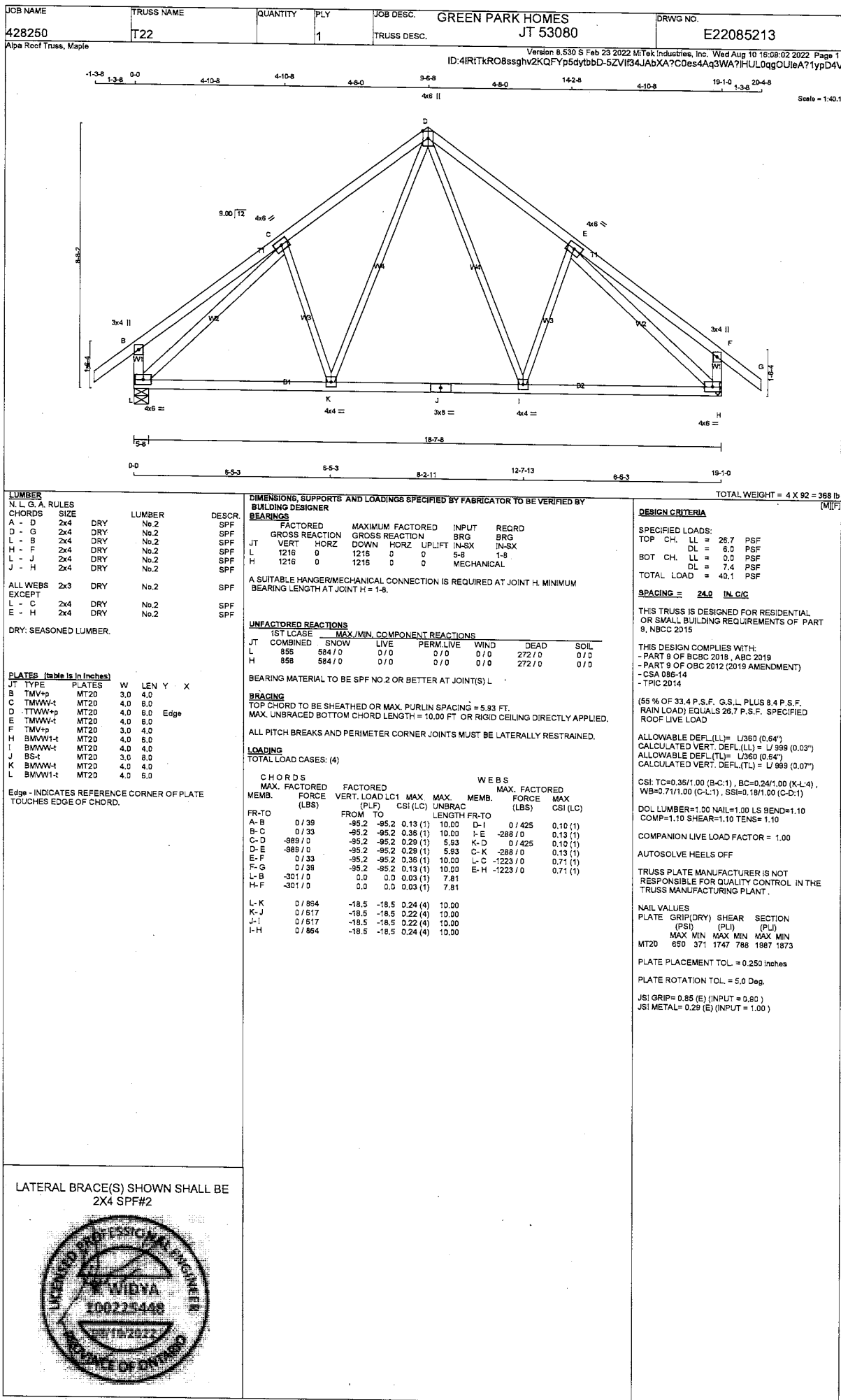
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



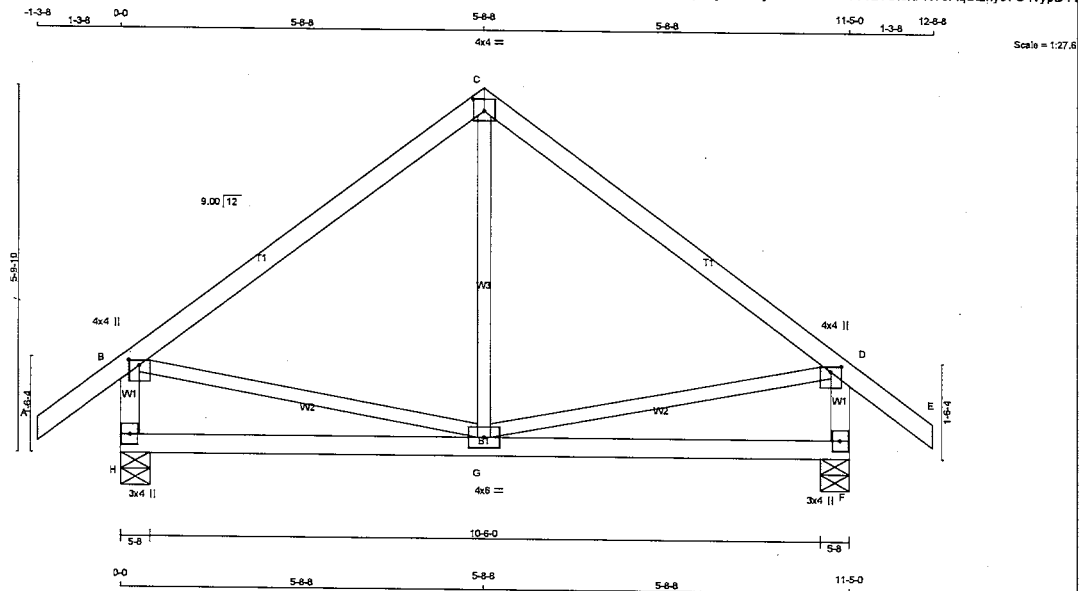




JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	T23		1	TRUSS DESC.	JT 53080	E22085214

Alpa Roof Truss, Maple

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ID:4IRtKtRO8ssghv2KQFYp5dytbbD-1yc24I5ZIDnuEV90zVCIBxKY5AqUtzhy37G4vypD4T



Scale = 1/27.6

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMWV+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWVWV-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	VERT	DOWN	IN-SX	IN-SX
F	HORZ	HORZ	UP-SX	UP-SX
			5-8	1-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN COMPONENT REACTIONS	2ND CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
H	548	379 / 0	0 / 0	0 / 0
F	548	379 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	0 / 38	-95.2	-95.2	0.13 (1)	10.00	G-C	-17 / 94	0.03 (4)	
B-C	-439 / 0	-95.2	-95.2	0.40 (1)	6.25	B-G	0 / 359	0.08 (1)	
C-D	-439 / 0	-95.2	-95.2	0.40 (1)	6.25	G-D	0 / 359	0.08 (1)	
D-E	0 / 39	-95.2	-95.2	0.13 (1)	10.00				
H-B	-740 / 0	0.0	0.0	0.08 (1)	7.81				
F-D	-740 / 0	0.0	0.0	0.08 (1)	7.81				
H-G	0 / 0	-18.5	-18.5	0.17 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.40/1.00 (C-D:1), BC=0.17/1.00 (F-G:4), WB=0.08/1.00 (D-G:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

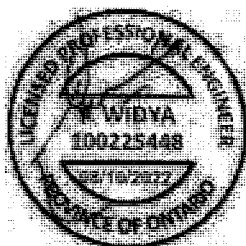
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1973

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.66 (D) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

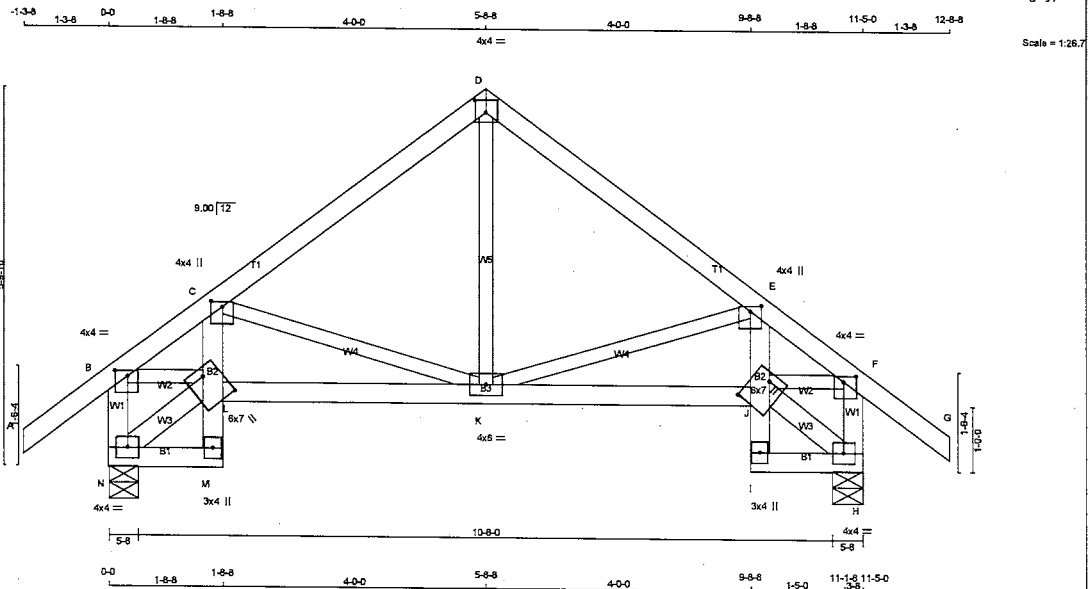
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	
428250	T23S		1	TRUSS DESC.	JT 53080	E22085215	

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 M/Tak Industries, Inc. Wed Aug 10 16:08:07 2022 Page 1
ID:4IRtKrOBssghv2KQFYp5dybbD-RXIAin7R789TSzbfm?matuclCynDR7e1LxgEypD4C



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	4.0	4.0	1.00	2.25
C	TMWV-p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWV-p	MT20	4.0	4.0	1.00	2.00
F	TMWV-p	MT20	4.0	4.0	1.00	2.25
H	BMWV-t	MT20	4.0	4.0		
I	BMV-p	MT20	3.0	4.0		
J	BMWV-w	MT20	6.0	7.0	3.00	5.50
K	BMWV-w	MT20	4.0	6.0		
L	BMWV-w	MT20	6.0	7.0	3.00	5.50
M	BMV-p	MT20	3.0	4.0		
N	BMWV-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION				
JT VERT	780	780	0	5-8
N	780	780	0	5-8
H	780	780	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED				
N	548	379 / 0	0 / 0	0 / 0
H	548	379 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	UNBRAC	MEMB.	MAX. FACTORED
FR-TO									
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	K-D	0 / 267	0.08 (1)	
B-C	-892 / 0	-95.2	-95.2	0.14 (1)	6.25	K-E	-366 / 0	0.12 (1)	
C-D	-520 / 0	-95.2	-95.2	0.20 (1)	6.25	C-K	-366 / 0	0.12 (1)	
D-E	-520 / 0	-95.2	-95.2	0.20 (1)	6.25	N-L	-35 / 0	0.00 (1)	
E-F	-892 / 0	-95.2	-95.2	0.14 (1)	6.25	B-L	0 / 723	0.16 (1)	
F-G	0 / 39	-95.2	-95.2	0.13 (1)	10.00	J-H	-35 / 0	0.00 (1)	
N-B	-745 / 0	0.0	0.0	0.08 (1)	7.81	J-F	0 / 723	0.16 (1)	
H-F	-745 / 0	0.0	0.0	0.08 (1)	7.81				
N-M	0 / 28	-18.5	-18.5	0.02 (4)	10.00				
M-L	0 / 16	0.0	0.0	0.05 (1)	10.00				
L-C	0 / 46	0.0	0.0	0.05 (1)	10.00				
L-K	0 / 750	-18.5	-18.5	0.17 (1)	10.00				
K-J	0 / 750	-18.5	-18.5	0.17 (1)	10.00				
I-J	0 / 18	0.0	0.0	0.05 (1)	10.00				
J-E	0 / 46	0.0	0.0	0.05 (1)	10.00				
I-H	0 / 28	-18.5	-18.5	0.02 (4)	10.00				

TOTAL WEIGHT = 2 X 56 = 112 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA D86-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/989$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/989$ (0.03")

CSI: TC=0.20/1.00 (D-E:1), BC=0.17/1.00 (J-K:1),
WB=0.16/1.00 (F-J:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

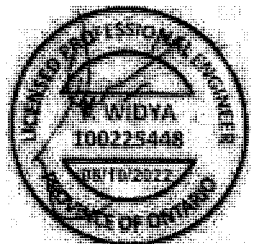
NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.25 (F) (INPUT = 1.00)

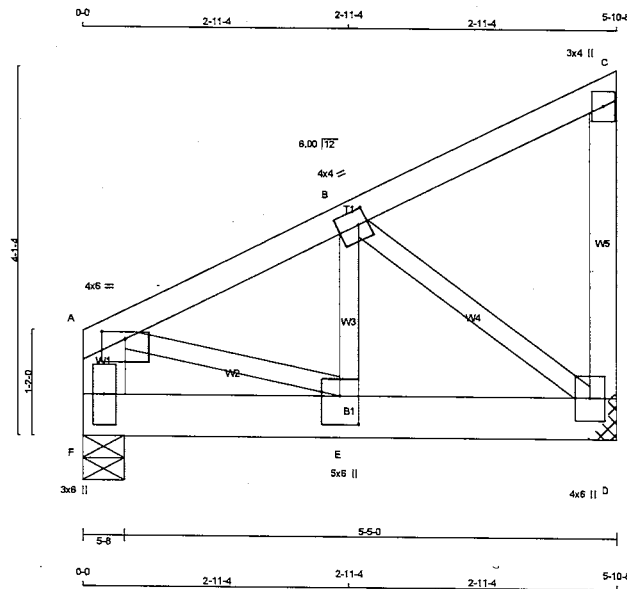
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	T24		2	TRUSS DESC.	JT 53080	E22085216

Alpa Roof Truss, Maple

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ID:4IRtKRO8ssghv2KQFYp5dytbbD-NwQx7T9WIPBKf2_m2oTr7yh-b6i896JQ5Lq1f7ypD4C



Scale = 1:19.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	9	SIDE(242.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMVV+p MT20	4.0	5.0	1.00	3.00
B TMVV+p MT20	4.0	4.0	2.00	1.25
C TMV+p MT20	3.0	4.0		
D BMVV+p MT20	4.0	6.0		
E BMVV+p MT20	5.0	6.0	3.75	2.50
F BMV+p MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT GROSS REACTION				
F VERT	1760	0	1760	0
F HORZ	0	1760	0	1760
D VERT	1760	0	1760	0
D HORZ	0	1760	0	1760

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1243	828 / 0	0 / 0	0 / 0	0 / 0	415 / 0	0 / 0
D	1243	828 / 0	0 / 0	0 / 0	0 / 0	415 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX.	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO							FR-TO		
F-A	-1139 / 0	0.0	0.0	0.04 (1)	7.81	A-E	0 / 1368	0.17 (1)	
A-B	-1488 / 0	-95.2	-95.2	0.08 (1)	6.25	E-B	0 / 1368	0.17 (1)	
B-C	-11 / 0	-95.2	-95.2	0.05 (1)	6.25	B-D	-1687 / 0	0.20 (1)	
D-C	-116 / 0	0.0	0.0	0.01 (1)	7.81				
F-E	0 / 0	-504.1	-504.1	0.14 (1)	10.00				
E-D	0 / 1340	-504.1	-504.1	0.23 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 19-1-0
END SPAN CARRIED = 5-10-8
END SPAN CARRIED = 19-1-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.23/1.00 (D-E:1), WB=0.20/1.00 (B-D:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

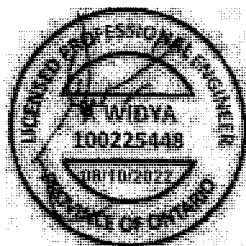
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1967 1873

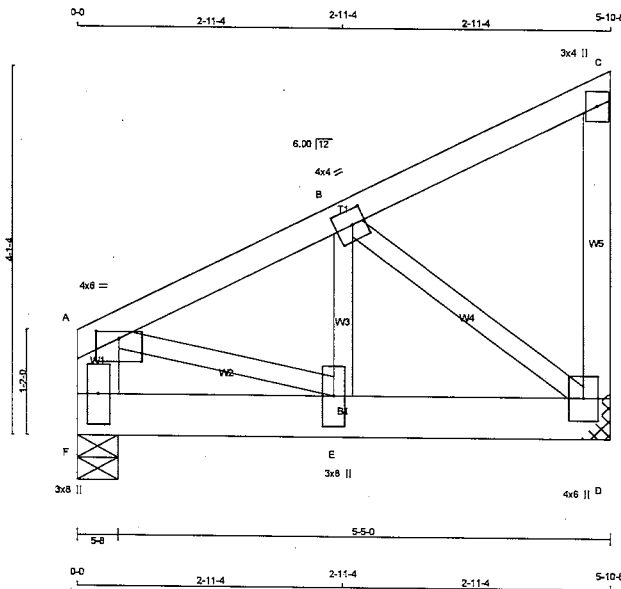
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.80)
JSI METAL= 0.30 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





Scale = 1:19.5

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
F-D 2	12	SIDE(201.3)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00 3.00
B	TMVW-i	MT20	4.0	4.0	2.00 1.75
C	TMV-p	MT20	3.0	4.0	
D	BMVW-i	MT20	4.0	6.0	
E	BMVW-i	MT20	3.0	8.0	
F	BMV-i	MT20	3.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION				
JT VERT	1517	0	1517	0
F VERT	1517	0	1517	0
D VERT	1517	0	1517	0
DOWN	0	0	0	0
UP	0	0	0	0
MECHANICAL				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	
F 1071	713 / 0
D 1071	713 / 0
SNOW	0 / 0
LIVE	0 / 0
PERM. LIVE	0 / 0
WIND	0 / 0
DEAD	358 / 0
SOIL	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	VERT.	PLF					FORCE	
(LBS)		(LBS)						(LBS)	
FR-TO		FROM	TO				LENGTH	FR-TO	
F-A	-997 / 0	0.0	0.0	0.04 (1)	7.81			A-E	0 / 1195
A-B	-1280 / 0	-95.2	-95.2	0.06 (1)	8.25			E-B	0 / 1129
B-C	-11 / 0	-95.2	-95.2	0.06 (1)	8.25			B-D	-1454 / 0
D-C	-114 / 0	0.0	0.0	0.01 (1)	7.81				
F-E	0 / 0	-421.2	-421.2	0.12 (1)	10.00				
E-D	0 / 1156	-421.2	-421.2	0.19 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 28.7 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

GIRDER TYPE: CShGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 16-2-0
 END DISTANCE = 5-10-8
 END SPAN CARRIED = 16-2-0
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 -ADD'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 085-14
 - TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/989 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.19/1.00 (D-E:1), WB=0.17/1.00 (B-C:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

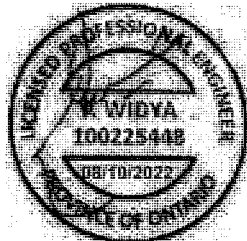
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)
 JSI METAL= 0.35 (E) (INPUT = 1.00)

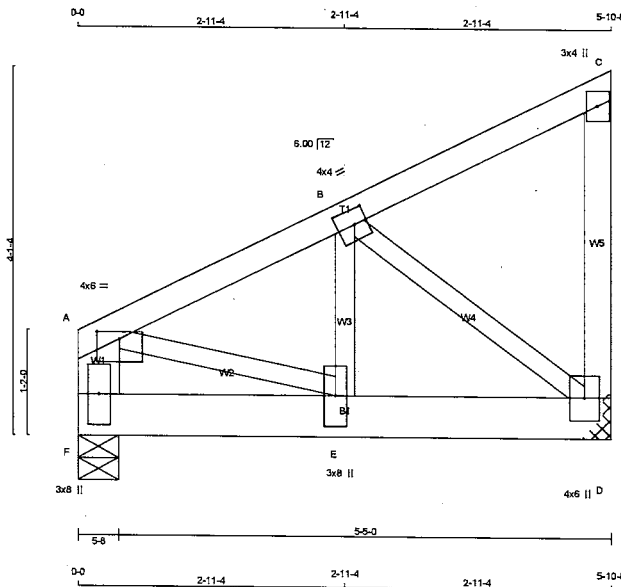
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	T24Z1		2	TRUSS DESC.	JT 53080	E22085218

Alpa Roof Truss, Maple

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Scale = 1:19.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
	SPACING (IN)	
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(92.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	5	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	5.0	1.00 3.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.75
C	TMV-p	MT20	3.0	4.0	
D	BMVW-t+p	MT20	4.0	8.0	
E	BMVW-t	MT20	3.0	8.0	
F	BMV-t+p	MT20	3.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
F	877	0	877	0
D	877	0	877	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX /MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	619	412 / 0	0 / 0	0 / 0	207 / 0	0 / 0
D	619	412 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO						
F-A	-821 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 692	0.09 (1)
A-B	-735 / 0	-95.2	-95.2	0.07 (1)	6.25	E-B	0 / 507	0.06 (1)
B-C	-13 / 0	-95.2	-95.2	0.07 (1)	6.25	B-D	-842 / 0	0.10 (1)
D-C	-110 / 0	0.0	0.0	0.01 (1)	7.81			
F-E	0 / 0	-203.3	-203.3	0.06 (1)	10.00			
E-D	0 / 698	-203.3	-203.3	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStnGdr
START DISTANCE = 0-0
START SPAN CARRIED = 8-6-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 8-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADDTL LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF BCBC 2018, ABC 2019
-PART 9 OF CBC 2012 (2019 AMENDMENT)
-CSA 086-14
-TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = U/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = U/999 (0.01")

CSI: TC=0.071/1.00 (A-B:1), BC=0.101/1.00 (D-E:1), WB=0.101/1.00 (B-D:1), SSI=0.111/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

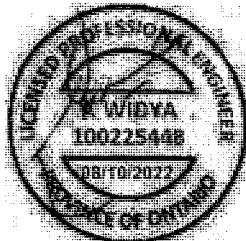
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

.JSI GRIP= 0.48 (E) (INPUT = 0.80)
.JSI METAL= 0.20 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

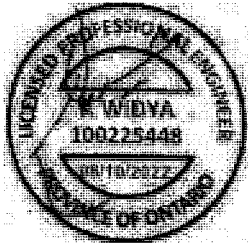


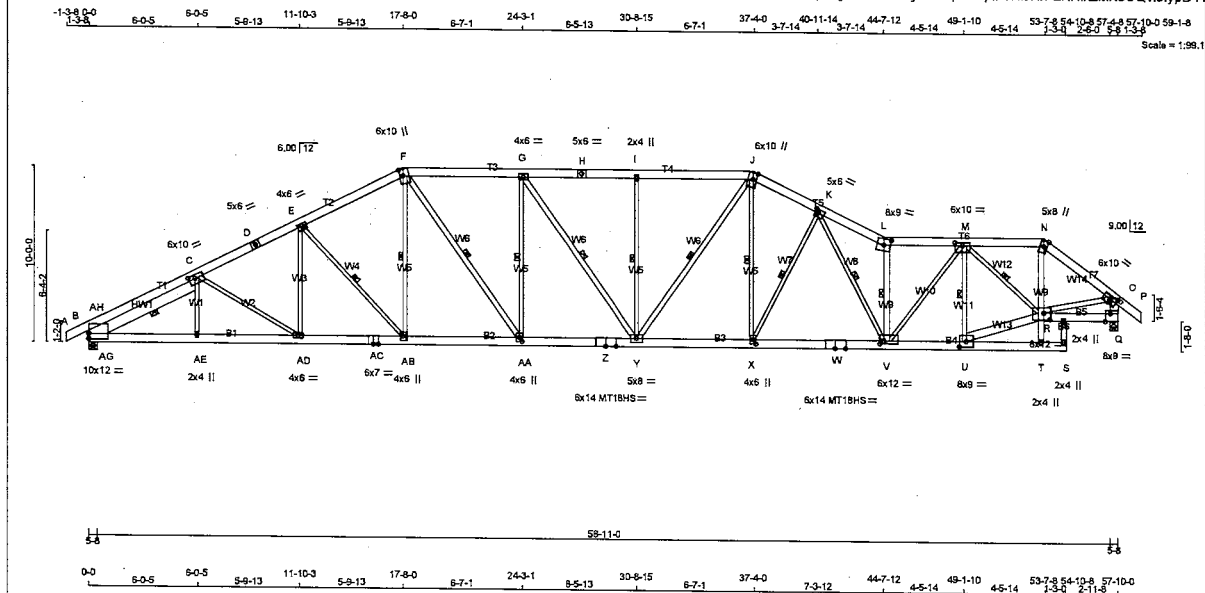
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_q, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM); INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.9 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2





Scale = 1/8\"/>

REINFORCING MEMBERS HW1 2x8 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT F-AA 2x4 DRY No.2 SPF

G-Y 2x4 DRY No.2 SPF

V-J 2x4 DRY No.2 SPF

T-N 2x4 DRY No.2 SPF

R-O 2x4 DRY No.2 SPF

U-R 2x6 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBW1-1	MT20	10.0	12.0	3.50	
C TMBW1-1	MT20	6.0	10.0	2.25	4.25
D TS-1	MT20	5.0	6.0		
E TMBW1-1	MT20	4.0	6.0		
F TMBW1-1	MT20	6.0	10.0	Edge	2.25
G TMBW1-1	MT20	4.0	6.0		
H TS-1	MT20	5.0	6.0		
I TMBW1-1	MT20	2.0	4.0		
J TMBW1-1	MT20	6.0	10.0	4.25	2.50
K TMBW1-1	MT20	6.0	10.0	2.50	4.25
L TMBW1-1	MT20	6.0	10.0	4.25	2.50
M TMBW1-1	MT20	6.0	10.0	2.50	4.25
N TMBW1-1	MT20	6.0	10.0	4.25	2.50
O TMBW1-1	MT20	6.0	10.0	2.50	4.25
P TMBW1-1	MT20	6.0	10.0	4.25	2.50
Q TMBW1-1	MT20	6.0	10.0	2.50	4.25
R TMBW1-1	MT20	6.0	10.0	4.25	2.50
S TMBW1-1	MT20	6.0	10.0	2.50	4.25
T TMBW1-1	MT20	6.0	10.0	4.25	2.50
U TMBW1-1	MT20	6.0	10.0	2.50	4.25
V TMBW1-1	MT20	6.0	10.0	4.25	2.50
W TMBW1-1	MT20	6.0	10.0	2.50	4.25
X TMBW1-1	MT20	6.0	10.0	4.25	2.50
Y TMBW1-1	MT20	6.0	10.0	2.50	4.25
Z TMBW1-1	MT20	6.0	10.0	4.25	2.50
AA TMBW1-1	MT20	6.0	10.0	2.50	4.25
AB TMBW1-1	MT20	6.0	10.0	4.25	2.50
AC TMBW1-1	MT20	6.0	10.0	2.50	4.25
AD TMBW1-1	MT20	6.0	10.0	4.25	2.50
AE TMBW1-1	MT20	6.0	10.0	2.50	4.25
AF TMBW1-1	MT20	6.0	10.0	4.25	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

TRAUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 5.1.9.2 (b)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM, INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE, TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

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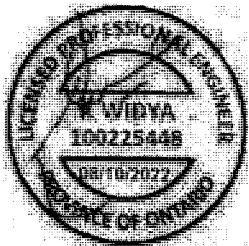
TRAUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 5.1.9.2 (b)

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30'-0") FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0'-0") FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



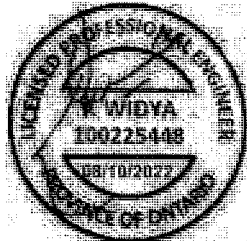
JOB NAME 428250	TRUSS NAME T30	QUANTITY	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085224(2)
Alpa Roof Truss, Maple				Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Aug 10 16:09:47 2022 Page 2 ID:4IRITkRO8ssghv2KQFYp5dytbbD-GEIj5ScXaQCou6yh7?1UN00XZBQgg36K0f-124wypD3c	

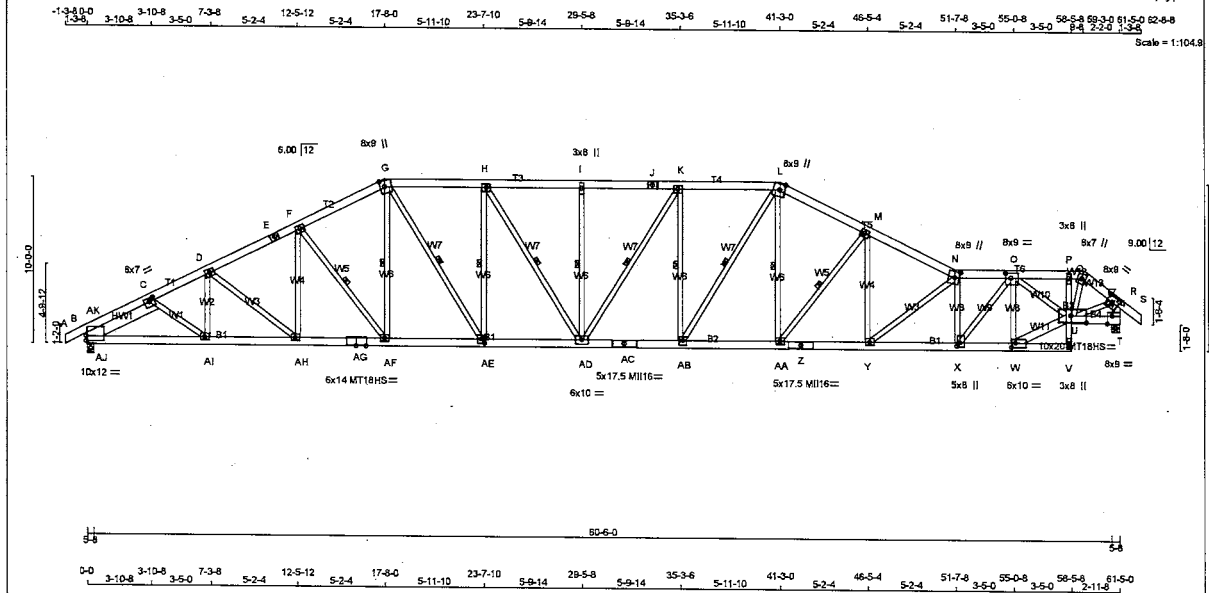
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LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2





LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x6	DRY	No.2 SPF
E - G	2x6	DRY	No.2 SPF
G - J	2x6	DRY	No.2 SPF
J - L	2x6	DRY	No.2 SPF
L - N	2x6	DRY	No.2 SPF
N - Q	2x6	DRY	No.2 SPF
Q - S	2x6	DRY	No.2 SPF
T - R	2x6	DRY	No.2 SPF
B - AG	2x8	DRY	2100F 1.8E SPF
AG - AC	2x6	DRY	2100F 1.8E SPF
AC - Z	2x6	DRY	2100F 1.8E SPF
Z - V	2x6	DRY	2100F 1.8E SPF
V - P	2x4	DRY	No.2 SPF
U - T	2x6	DRY	2100F 1.8E SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2 SPF
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ALL WEBS 2x4 DRY, EXCEPT

W - U	2x6	DRY	No.2 SPF
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DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
JT VERT	5227	0	5227	182
B	5227	0	5219	0
T	5219	0	5219	0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 218 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT T FOR 246 LBS FACTORED UPLIFT

PROVIDE FOR 182 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST CASE	MAX MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
B	3860 2253 / 0 645 / 0 0 / 0 98 / -775 962 / 0 0 / 0
T	3853 2255 / 0 645 / 0 0 / 0 139 / -778 953 / 0 0 / 0

HORIZONTAL REACTIONS

B	0 / 0 0 / 0 0 / 0 130 / -78 0 / 0 0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, T

BEARING SIZE FACTOR = 1.15 AT JNT(S) B, T (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION G-L, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.

FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.37 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-AF, G-AF, L-AA, M-AA, L-AB, G-AE, K-AB, H-AE, K-AD, H-AD, I-AD.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 35.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 59.0 PSF

SPACING = 24.0 IN C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 8.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCCC 2015

THIS DESIGN COMPLIES WITH:

- PART 4 OF CBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = U/360 (2.05")

CALCULATED VERT. DEFL.(LL) = U/999 (0.51")

ALLOWABLE DEFL.(TL) = U/180 (4.59")

CALCULATED VERT. DEFL.(TL) = U/969 (0.73")

CSI: TC=0.75/1.00 (H-K), BC=0.58/1.00 (X-Y), WB=0.95/1.00 (N-Y), SS=0.33/1.00 (K-L)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788 1897 1873	
MT18HS	586 403 2455 1382 2163 3004	
MT15	438 302 2547 1256 4283 1816	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (X) (INPUT = 0.90)

JSI METAL= 0.99 (X) (INPUT = 1.00)

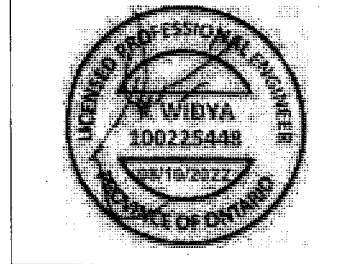
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.50
C	TMBMW1-I	MT20	6.0	7.0	2.25 3.50
D, F, H, K, M					
D	TMBMW1-I	MT20	5.0	6.0	
E	TS-I	MT20	5.0	6.0	
G	TTWW+m	MT20	6.0	9.0	4.25 3.00
I	TTWW+m	MT20	3.0	6.0	
J	TS-I	MT20	5.0	6.0	
L	TTWW+m	MT20	6.0	9.0	Edge 3.25
N	TTWW+m	MT20	6.0	9.0	4.50 3.25
O	TMBMW1-I	MT20	6.0	9.0	3.50 3.50
P	TMV+p	MT20	3.0	6.0	
Q	TTWW+m	MT20	6.0	7.0	
R	TMBMW1-I	MT20	6.0	9.0	4.00 4.25
T	BMV1-I	MT20	6.0	9.0	Edge 3.50
U	BVMMW1-I	MT20	10.0	20.0	5.00 11.00
V	BMV+p	MT20	3.0	6.0	
W	BVMMW1-I	MT20	6.0	10.0	3.00 3.00
X	BVMMW1-I	MT20	5.0	6.0	2.75 2.00
Y, AA, AF, AH, AI					
Y	BVMMW1-I	MT20	5.0	6.0	
Z	BS-I	MT16	5.0	17.5	
AB	BVMMW1-I	MT20	5.0	6.0	
AC	BS-I	MT16	5.0	17.5	
AD	BVMMW1-I	MT20	6.0	10.0	
AE	BVMMW1-I	MT20	6.0	6.0	3.00 2.25
AG	BS-I	MT16HS	6.0	14.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



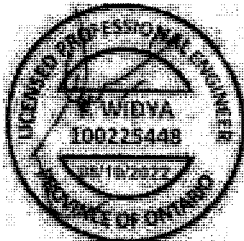
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCCC 4.1.6.2 (8)

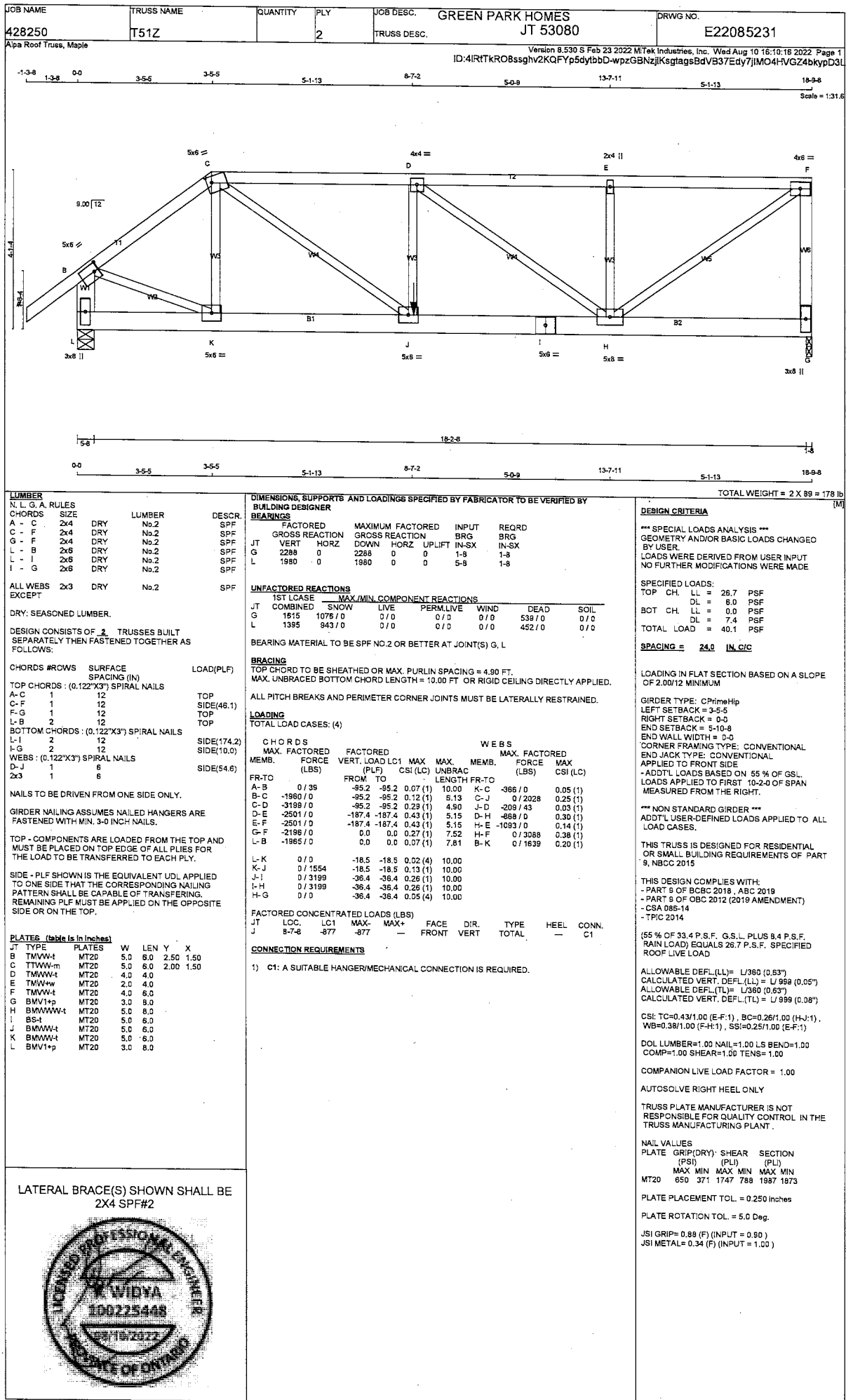
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428250	T32		1	GREEN PARK HOMES JT 53080	E22085226(2)
Alpa Roof Truss, Maple					

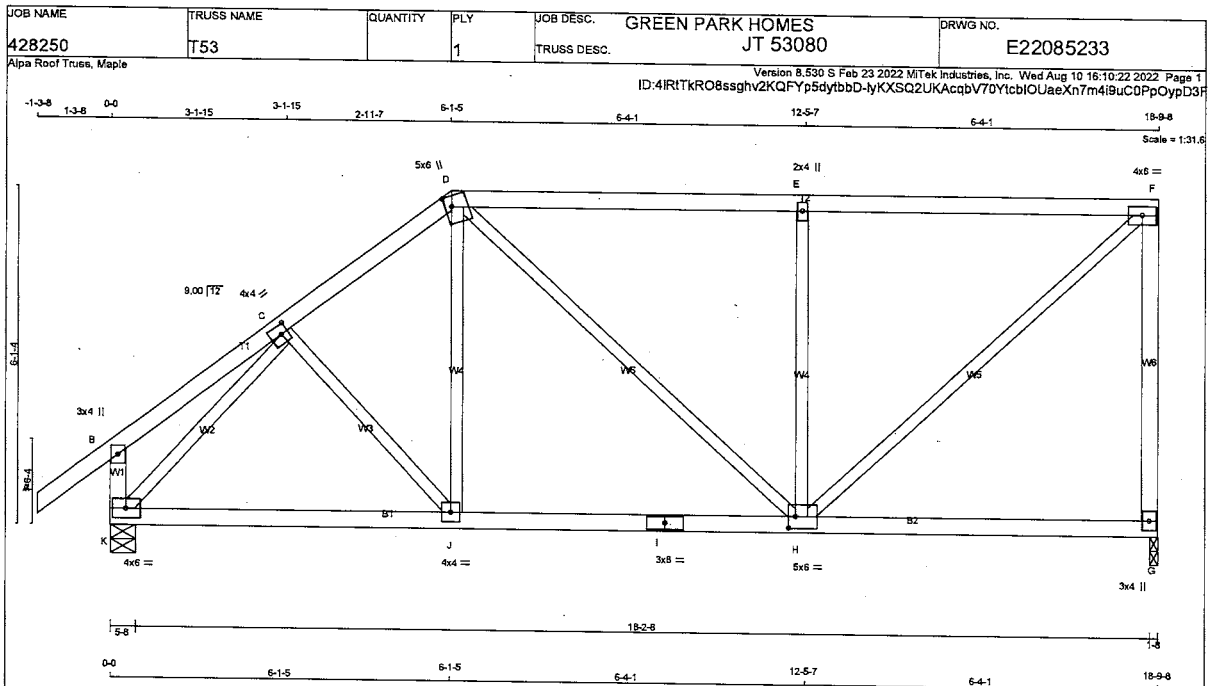
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Aug 10 16:09:57 2022 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF 17.3 PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

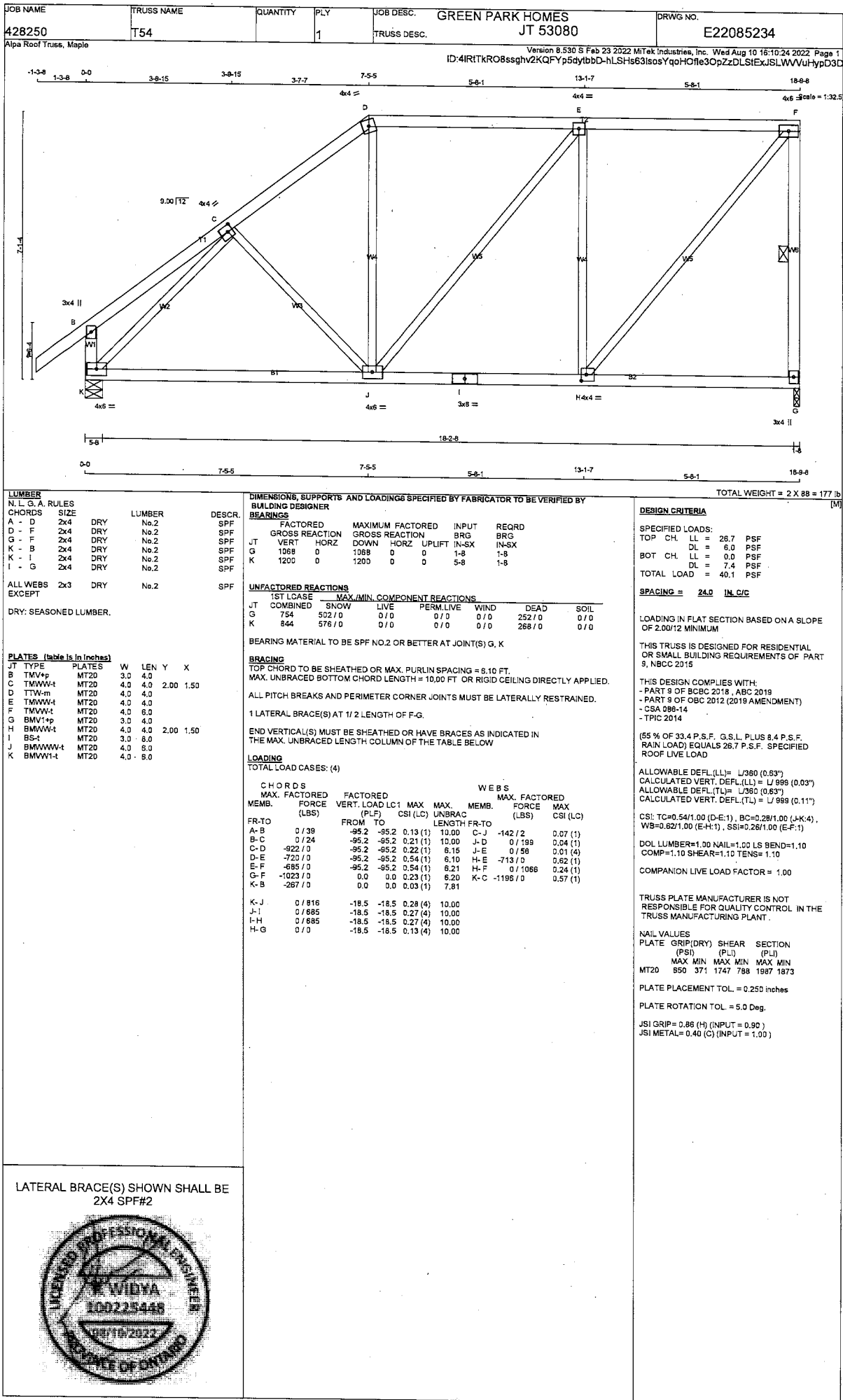
LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2







LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 83 = 167 lb									
N.L.G.A. RULES										BEARINGS										DESIGN CRITERIA									
CHORDS SIZE LUMBER DESCR.																				SPECIFIED LOADS:									
A - D 2x4 DRY No.2 SPF										FACTORED MAXIMUM FACTORED INPUT REQ'D										TOP CH. LL = 28.7 PSF									
D - F 2x4 DRY No.2 SPF										GROSS REACTION GROSS REACTION BRG BRG										DL = 6.0 PSF									
K - B 2x4 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF									
K - I 2x4 DRY No.2 SPF										G 1068 0 1068 0 0 1-8 1-8										DL = 7.4 PSF									
I - G 2x4 DRY No.2 SPF										K 1200 0 1200 0 0 5-8 1-8										TOTAL LOAD = 40.1 PSF									
ALL WEBS 2x3 DRY No.2 SPF										UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C									
EXCEPT										1ST LCASE MAX/MIN COMPONENT REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
DRY: SEASONED LUMBER.										JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015									
										G 754 502 / 0 0 / 0 0 / 0 252 / 0 0 / 0										* THIS DESIGN COMPLIES WITH:									
										K 844 576 / 0 0 / 0 0 / 0 268 / 0 0 / 0										- PART 9 OF CBC 2018, ABC 2019									
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K										- PART 9 OF OBC 2012 (2015 AMENDMENT)									
										BRACING										- CSA 086-14									
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.										- TPIC 2014									
										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD									
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.										ALLOWABLE DEFL.(LL) = L/360 (0.53")									
										LOADING										CALCULATED VERT. DEFL.(LL) = L/999 (0.03")									
										TOTAL LOAD CASES: (4)										ALLOWABLE DEFL.(TL) = L/360 (0.53")									
										CHORDS										CALCULATED VERT. DEFL.(TL) = L/999 (0.07")									
										MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) LC1 MAX. UNBRACED LENGTH (LC) FR-TO										CSI TC=0.74/1.00 (F-G-1), BC=0.23/1.00 (H-J-4), WB=0.44/1.00 (E-H-1), SSI=0.26/1.00 (E-F-1)									
										A-B 0 / 39 -95.2 -95.2 0.13 (1) 10.00 C-J -36 / 27 0.01 (1)										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
										B-C 0 / 19 -95.2 -95.2 0.13 (1) 10.00 J-D 0 / 152 0.04 (4)										COMP=1.10 SHEAR=1.10 TENS=1.10									
										C-D -984 / 0 -95.2 -95.2 0.14 (1) 6.12 D-H 0 / 127 0.03 (1)										COMPANION LIVE LOAD FACTOR = 1.00									
										D-E -868 / 0 -95.2 -95.2 0.89 (1) 5.32 H-E -748 / 0 0.44 (1)										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
										E-F -868 / 0 -95.2 -95.2 0.89 (1) 5.32 F-H 0 / 1171 0.26 (1)										NAIL VALUES									
										G-F -1019 / 0 0.0 0.0 0.74 (1) 7.76 K-C -1207 / 0 0.41 (1)										PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
										K-B -245 / 0 0.0 0.0 0.03 (1) 7.81										MAX MIN MAX MIN MAX MIN									
										WEBS										MT20 650 371 1747 788 1987 1873									
										MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH (LC) FR-TO										PLATE PLACEMENT TOL. = 0.250 inches									
										K-J 0 / 795 -18.5 -18.5 0.22 (4) 10.00										PLATE ROTATION TOL. = 5.0 Deg.									
										J-I 0 / 773 -18.5 -18.5 0.23 (4) 10.00										JSI GRIP= 0.89 (C) (INPUT = 0.90)									
										I-H 0 / 773 -18.5 -18.5 0.23 (4) 10.00										JSI METAL= 0.40 (C) (INPUT = 1.00)									
										H-G 0 / 0 -18.5 -18.5 0.17 (4) 10.00																			



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ID:4IRiTkRO8ssghv2KQFYp5dytbbD-5w7QV85d9jE7hG?zLRCm?SBWUJYVBRD4u1Tk9UcwpD3A

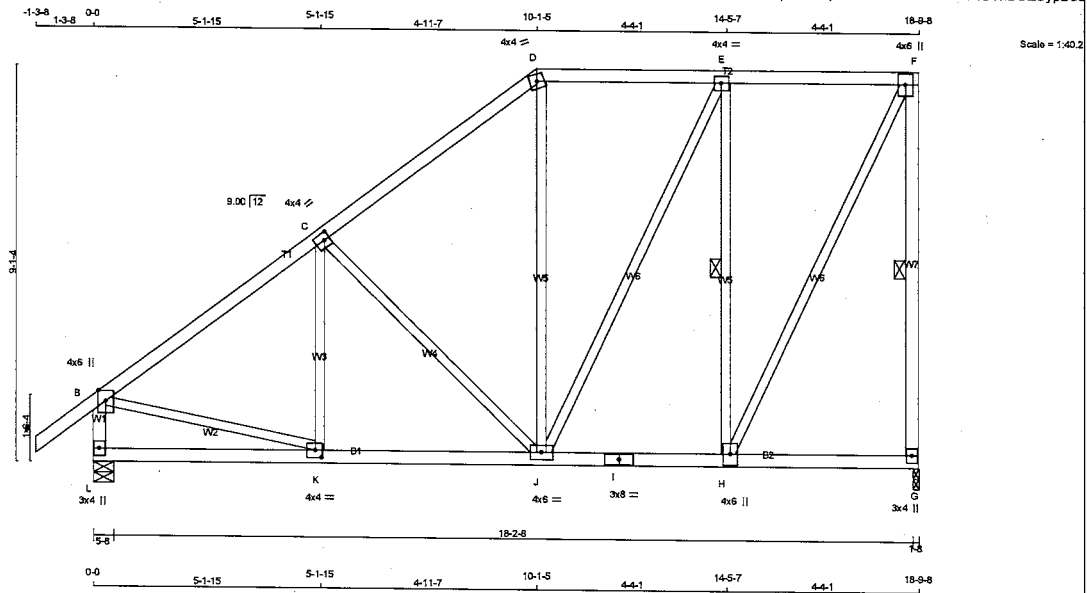


TOTAL WEIGHT = 2 X 96 = 192 lb

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LBS)	MAX. UNBRAC. CS1 (LBS)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0/39	-95.2	-95.2 0.13 (1)	10.00	K-C	-137/28	0.05 (1)
B-C	-1035/0	-95.2	-95.2 0.34 (1)	5.72	C-D	-309/0	0.22 (1)
D-E	-95.2/0	-95.2	-95.2 0.32 (1)	6.21	J-J	0/150	0.04 (4)
E-F	-643/0	-95.2	-95.2 0.41 (1)	6.25	J-E	0/181	0.04 (1)
F-F	-547/0	-95.2	-95.2 0.41 (1)	6.25	H-F	-742/0	0.23 (1)
G-F	-1030/0	0.0	0.0 0.31 (1)	6.18	H-F	0/1008	0.04 (1)
L-B	-1164/0	0.0	0.0 0.12 (1)	7.38	B-K	0/885	0.20 (1)
L-K		-18.5	-18.5 0.08 (4)	10.00			
K-J	0/855	-18.5	-18.5 0.18 (1)	10.00			
J-K	0/547	-18.5	-18.5 0.16 (4)	10.00			
I-H	0/547	-18.5	-18.5 0.16 (4)	10.00			
H-G	0/0	-18.5	-18.5 0.11 (4)	10.00			

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	Edge	
C	TMWW+4	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW+4	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+4	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW+4	MT20	4.0	6.0		
K	BMWW+4	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1058	0	1058	0
L	1200	0	1200	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
L	844	576 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
		FROM	TO				
FR-TO							
A-B	0 / 39	-95.2	-95.2	0.13 (1)	K-C	-91 / 57	0.04 (1)
B-C	-1037 / 0	-95.2	-95.2	0.33 (1)	C-J	-412 / 0	0.41 (1)
C-D	-745 / 0	-95.2	-95.2	0.32 (1)	J-D	0 / 93	0.03 (4)
D-E	-567 / 0	-95.2	-95.2	0.23 (1)	J-E	0 / 308	0.07 (1)
E-F	-435 / 0	-95.2	-95.2	0.22 (1)	H-E	-786 / 0	0.42 (1)
G-F	-1036 / 0	0.0	0.0	0.40 (1)	H-F	0 / 977	0.22 (1)
L-B	-1160 / 0	0.0	0.0	0.12 (1)	B-K	0 / 880	0.20 (1)
L-K	0 / 0	-18.5	-18.5	0.12 (4)			
K-J	0 / 857	-18.5	-18.5	0.20 (1)			
J-I	0 / 435	-18.5	-18.5	0.11 (4)			
I-H	0 / 435	-18.5	-18.5	0.11 (4)			
H-G	0 / 0	-18.5	-18.5	0.08 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	DL	PSF
	28.7	6.0	0.0	7.4
	40.1			

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")
CSI: TC=0.40/1.00 (F-G-1), BC=0.20/1.00 (J-K-1), WB=0.42/1.00 (E-H-1), SS=0.20/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 L.B.BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

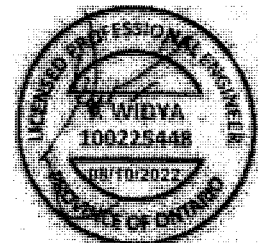
PLATE GRIP(ORY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	850	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

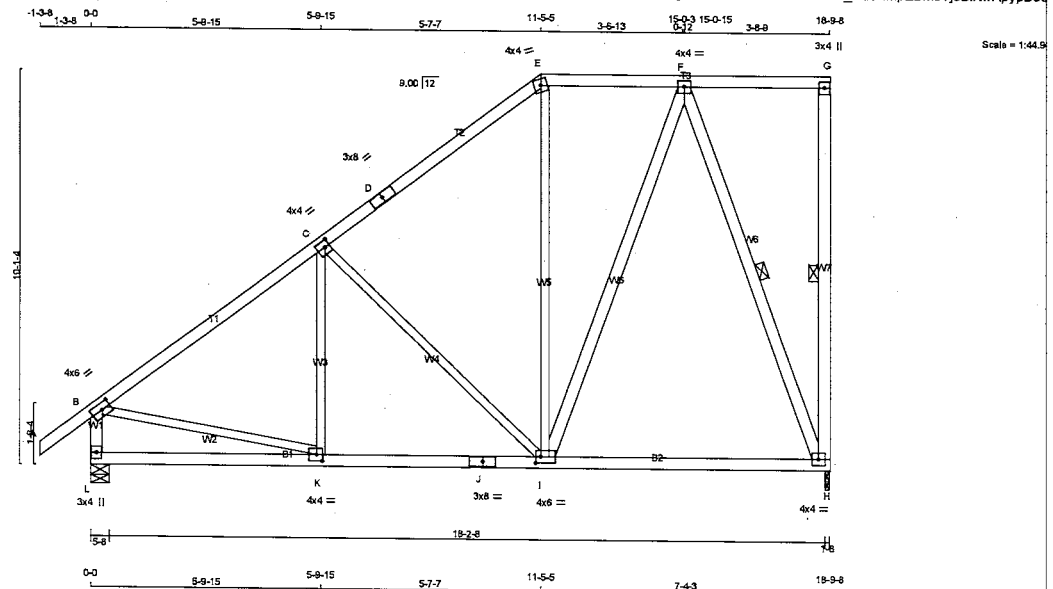
JSI GRIP= 0.80 (B) (INPUT = 0.80)
JSI METAL= 0.56 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRTKRO8ssghv2KQFP5dytbbD-SuxJY9mzFIPo1ux7_nxVvMpZBm6YjeBIRwApyD38



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
I - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-t	MT20	4.0	8.0	2.00	2.75
C TMWW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	8.0		
E TTW-m	MT20	4.0	4.0		
F TMWW-t	MT20	4.0	4.0		
G TMV-p	MT20	3.0	4.0		
H BMWW-t	MT20	4.0	4.0		
I BMWW-t	MT20	4.0	8.0	2.00	1.50
J BS-t	MT20	3.0	8.0		
K BMWW-t	MT20	4.0	4.0	2.00	1.75
L BMV-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	DOWN	HORIZ	UPLIFT	IN-SX
H 1113	0	1113	0	1-8
L 1210	0	1210	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	790	502/0	0/0	0/0	0/0	0/0	287/0	0/0
L	853	576/0	0/0	0/0	0/0	0/0	277/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0/39	-95.2	-95.2	0.13 (1)	10.00	K-C	-69/53	0.04 (1)	0.04 (1)
B-C	-1036/0	-95.2	-95.2	0.43 (1)	5.60	C-I	-495/0	0.88 (1)	0.88 (1)
C-D	-673/0	-95.2	-95.2	0.41 (1)	6.25	I-E	0/67	0.02 (4)	0.02 (4)
D-E	-673/0	-95.2	-95.2	0.41 (1)	6.25	E-F	0/474	0.08 (1)	0.08 (1)
E-F	-605/0	-102.7	-102.7	0.18 (1)	2.00	F-H	-978/0	0.58 (1)	0.58 (1)
F-G	0/0	-102.7	-102.7	0.23 (1)	10.00	B-K	0/878	0.20 (1)	0.20 (1)
G-H	-145/0	0.0	0.0	0.07 (1)	6.25				
L-B	-1184/0	0.0	0.0	0.12 (1)	7.39				
L-K	0/0	-18.5	-18.5	0.13 (4)	10.00				
K-J	0/860	-18.5	-18.5	0.28 (4)	10.00				
J-I	0/860	-18.5	-18.5	0.28 (4)	10.00				
I-H	0/345	-18.5	-18.5	0.24 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(LL) = $L/369$ (0.62")
ALLOWABLE DEFL.(TL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(TL) = $L/989$ (0.06")

CSI: TC=0.43/1.00 (B-C:1), BC=0.28/1.00 (I-K:4), WB=0.68/1.00 (C-1), SS=0.21/1.00 (E-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

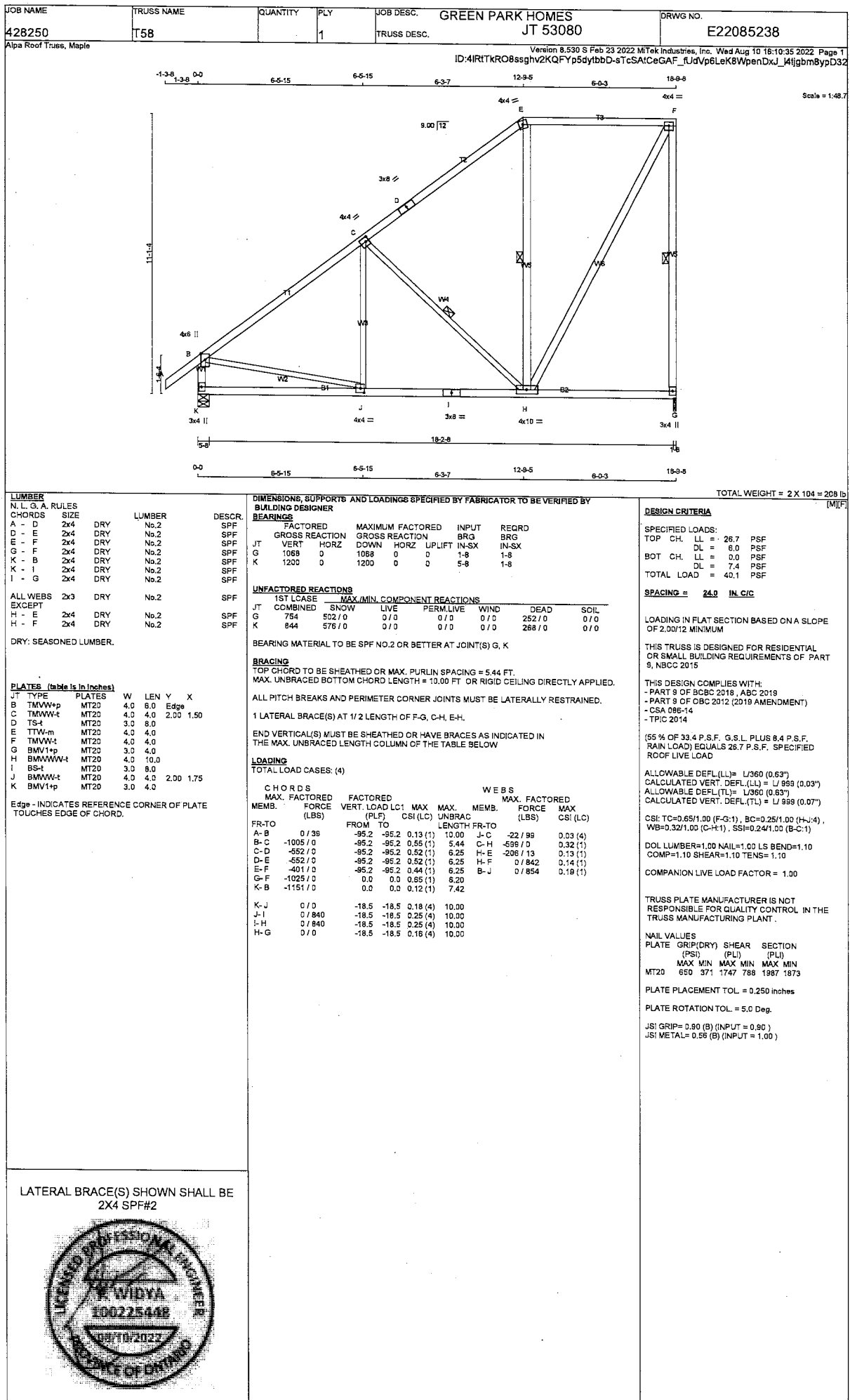
PLATE PLACEMENT TOL. = 0.250 inches

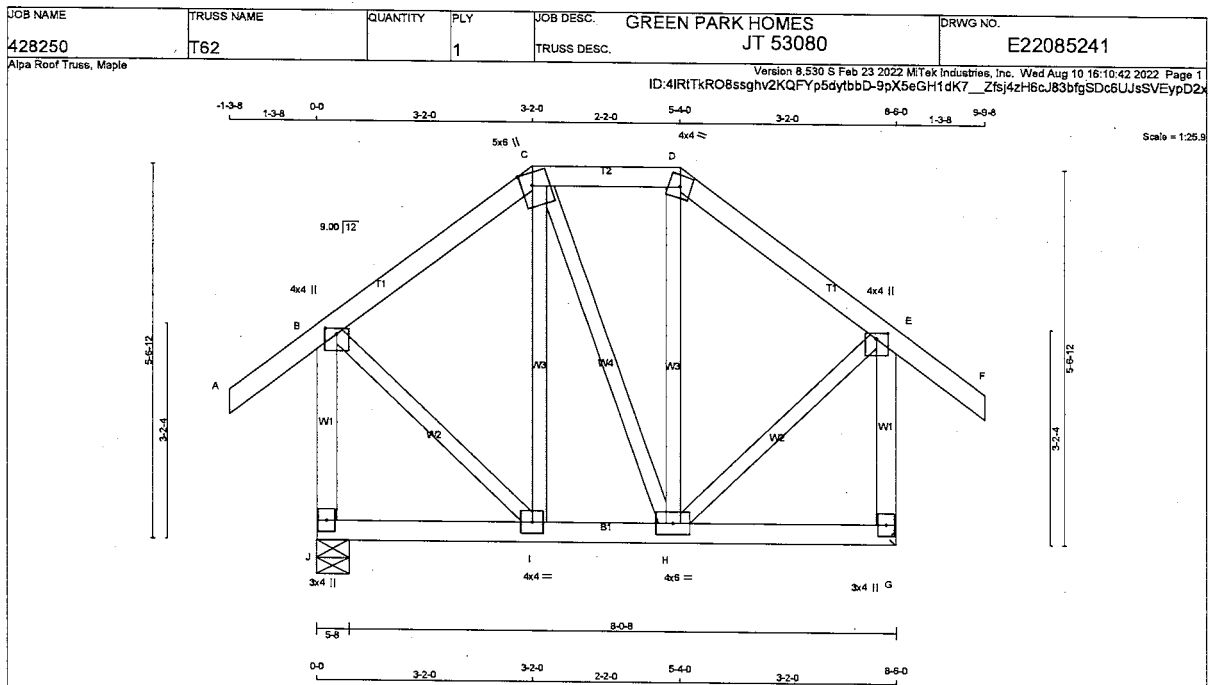
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.80)
JSI METAL= 0.28 (K) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2







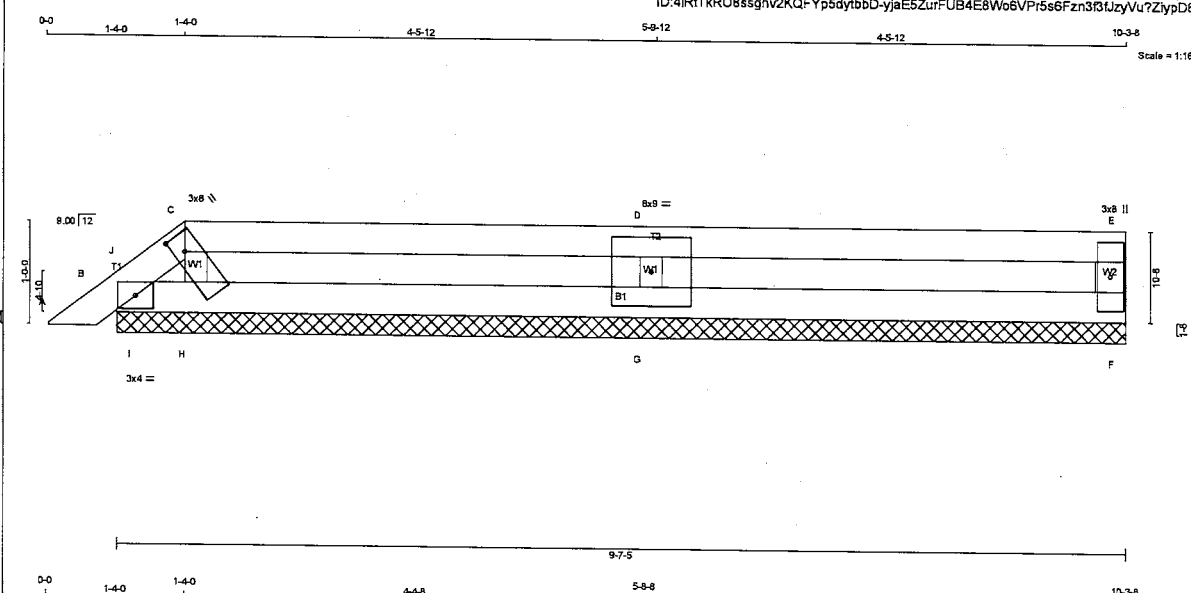
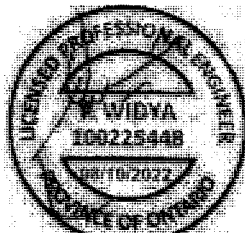
LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA										TOTAL WEIGHT = 51 lb	
N. L. G. A. RULES										BEARINGS																				[M/F]	
CHORDS																															
SIZE																															
LUMBER																															
DESCR.																															
SPF																															
No.2																															
C - D																															
2x4																															
DRY																															
No.2																															
D - F																															
2x4																															
DRY																															
No.2																															
J - B																															
2x4																															
DRY																															
No.2																															
G - E																															
2x4																															
DRY																															
No.2																															
J - G																															
2x4																															
DRY																															
No.2																															
SPF																															
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.																															
ALL WEBS																															
2x3																															
DRY																															
No.2																															
EXCEPT																															
SPF																															
SPECIFIED LOADS:																															
TOP CH. LL = 28.7 PSF																															
DL = 6.0 PSF																															
BOT CH. LL = 0.0 PSF																															
DL = 7.4 PSF																															
TOTAL LOAD = 40.1 PSF																															
SPACING = 24.0 IN/CIC																															
DRY: SEASONED LUMBER.																															

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
JT TYPE	PLATES	W	LEN Y X	1ST LCASE	MAX	MIN	COMPONENT REACTIONS				
B TMWV+p	MT20	4.0	4.0	1.00	2.00						
C TTWW+m	MT20	5.0	6.0	Edge	2.00						
D TTW-m	MT20	4.0	4.0								
E TMWV+p	MT20	4.0	4.0	1.00	2.00						
G BMV1+p	MT20	3.0	4.0								
H BMWWV-t	MT20	4.0	6.0								
I BMWV-t	MT20	4.0	4.0								
J BMV1+p	MT20	3.0	4.0								

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/38	-85.2	-85.2 0.13 (1)	I-C	-110/3	0.05 (1)	
B-C	-238/0	-85.2	-85.2 0.12 (1)	C-H	-2/0	0.00 (1)	
C-D	-185/0	-85.2	-85.2 0.08 (1)	H-D	-112/3	0.05 (1)	
D-E	-235/0	-85.2	-85.2 0.12 (1)	B-I	0/250	0.06 (1)	
E-F	0/38	-85.2	-85.2 0.13 (1)	H-E	0/249	0.06 (1)	
J-B	-589/0	0.0	0.0 0.10 (1)				
G-E	-587/0	0.0	0.0 0.10 (1)				
J-I	0/0	-18.5	-18.5 0.04 (4)				
I-H	0/186	-18.5	-18.5 0.05 (4)				
H-G	0/0	-18.5	-18.5 0.04 (4)				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
 TOTAL LOAD CASES: (4)
 ALLOWABLE DEFL.(LL) = L/360 (0.28")
 CALCULATED VERT. DEFL.(LL) = L/959 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.28")
 CALCULATED VERT. DEFL.(TL) = L/959 (0.01")
 CSI: TC=0.13/1.00 (A-B:1), BC=0.05/1.00 (H-I:4), WB=0.06/1.00 (B-I:1), SSI=0.06/1.00 (D-E:1)
 DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 1.00
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.40 (B) (INPUT = 0.50)
 JSI METAL= 0.12 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME 428250	TRUSS NAME PB1	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085175																																																																																																																																																																																													
Alpa Roof Truss, Maple																																																																																																																																																																																																		
Version 8.530 S Feb 23 2022 MTek Industries, Inc. Wed Aug 10 16:07:24 2022 Page 1 ID:4IRITKRO8ssghv2KQFYp5dytbbD-yjaE5ZurFUB4E8Wo6VP75s6Fzn3fJzyVu7ZlypD81																																																																																																																																																																																																		
																																																																																																																																																																																																		
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF F - E 2x4 DRY No.2 SPF B - F 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY, SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><thead><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQD BRG</th></tr><tr><th></th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th><th>IN-SX</th><th>IN-SX</th></tr></thead><tbody><tr><td>JT</td><td>195</td><td>0</td><td>195</td><td>0</td><td>0</td><td>9-7-5</td><td>1-8</td></tr><tr><td>F</td><td>112</td><td>0</td><td>112</td><td>0</td><td>0</td><td>9-7-5</td><td>1-8</td></tr><tr><td>H</td><td>211</td><td>0</td><td>211</td><td>0</td><td>0</td><td>9-7-5</td><td>1-8</td></tr><tr><td>G</td><td>628</td><td>0</td><td>628</td><td>0</td><td>0</td><td>9-7-5</td><td>1-8</td></tr></tbody></table> UNFACTORED REACTIONS <table><thead><tr><th></th><th>1ST LCASE</th><th>SNOW</th><th>LIVE</th><th>PERM. LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr></thead><tbody><tr><td>JT</td><td>COMBINED</td><td>91/0</td><td>0/0</td><td>0/0</td><td>0/0</td><td>47/0</td><td>0/0</td></tr><tr><td>F</td><td></td><td>75/0</td><td>0/0</td><td>0/0</td><td>0/0</td><td>1/0</td><td>0/0</td></tr><tr><td>B</td><td></td><td>80/0</td><td>0/0</td><td>0/0</td><td>0/0</td><td>74/0</td><td>0/0</td></tr><tr><td>H</td><td></td><td>289/0</td><td>0/0</td><td>0/0</td><td>0/0</td><td>143/0</td><td>0/0</td></tr></tbody></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><thead><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. LC1 (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. LC1 (LC)</th><th></th></tr></thead><tbody><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td>FR-TO</td><td></td><td>LENGTH</td><td>FR-TO</td></tr><tr><td>A-B</td><td>0/15</td><td>-95.2</td><td>-95.2 0.02 (1)</td><td>10.00</td><td>H-C</td><td>-132/0</td><td>0.02 (1)</td></tr><tr><td>B-J</td><td>-48/5</td><td>-95.2</td><td>-95.2 0.02 (1)</td><td>6.25</td><td>G-D</td><td>-533/0</td><td>0.07 (1)</td></tr><tr><td>J-C</td><td>-47/0</td><td>-95.2</td><td>-95.2 0.01 (1)</td><td>6.25</td><td>I-J</td><td>-34/0</td><td>0.00 (1)</td></tr><tr><td>C-D</td><td>0/0</td><td>-95.2</td><td>-95.2 0.32 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>D-E</td><td>0/0</td><td>-95.2</td><td>-95.2 0.32 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>E-F</td><td>-162/0</td><td>0.0</td><td>0.0 0.02 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>B-I</td><td>0/24</td><td>-18.5</td><td>-18.5 0.00 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>I-H</td><td>0/24</td><td>-18.5</td><td>-18.5 0.05 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>H-G</td><td>0/0</td><td>-18.5</td><td>-18.5 0.09 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>G-F</td><td>0/0</td><td>-18.5</td><td>-18.5 0.09 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></tbody></table>					FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	JT	195	0	195	0	0	9-7-5	1-8	F	112	0	112	0	0	9-7-5	1-8	H	211	0	211	0	0	9-7-5	1-8	G	628	0	628	0	0	9-7-5	1-8		1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	91/0	0/0	0/0	0/0	47/0	0/0	F		75/0	0/0	0/0	0/0	1/0	0/0	B		80/0	0/0	0/0	0/0	74/0	0/0	H		289/0	0/0	0/0	0/0	143/0	0/0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)		FR-TO		FROM	TO	FR-TO		LENGTH	FR-TO	A-B	0/15	-95.2	-95.2 0.02 (1)	10.00	H-C	-132/0	0.02 (1)	B-J	-48/5	-95.2	-95.2 0.02 (1)	6.25	G-D	-533/0	0.07 (1)	J-C	-47/0	-95.2	-95.2 0.01 (1)	6.25	I-J	-34/0	0.00 (1)	C-D	0/0	-95.2	-95.2 0.32 (1)	10.00				D-E	0/0	-95.2	-95.2 0.32 (1)	10.00				E-F	-162/0	0.0	0.0 0.02 (1)	7.81				B-I	0/24	-18.5	-18.5 0.00 (1)	10.00				I-H	0/24	-18.5	-18.5 0.05 (4)	10.00				H-G	0/0	-18.5	-18.5 0.09 (4)	10.00				G-F	0/0	-18.5	-18.5 0.09 (4)	10.00			
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JT	195	0	195	0	0	9-7-5	1-8																																																																																																																																																																																											
F	112	0	112	0	0	9-7-5	1-8																																																																																																																																																																																											
H	211	0	211	0	0	9-7-5	1-8																																																																																																																																																																																											
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G-F	0/0	-18.5	-18.5 0.09 (4)	10.00																																																																																																																																																																																														
TOTAL WEIGHT = 2 X 24 = 48 lb																																																																																																																																																																																																		
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 40.1 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.32/1.00 (C-D-1), BC=0.09/1.00 (F-G-4), WB=0.07/1.00 (D-G-1), SI=0.21/1.00 (D-E-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.25 (C) (INPUT = 0.90) JSI METAL = 0.03 (D) (INPUT = 1.00)		LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2 																																																																																																																																																																																																

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

GREEN PARK HOMES
JT 53080

DRWG NO.

428250

PB2

1

TRUSS DESC.

E22085176

Alpa Roof Truss, Maple

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ID:4IRtRkRO8ssghv2KQFYp5dytbD-u5h?VFv5n6RoTSgBEwS.JAHBdtbIWxzYFQpN5ebypD8?

D=0

2-8-0

2-8-0

3-9-12

6-5-12

3-9-12

10-3-8

Scale = 1:16.8

C

5x8

T1

W1

B1

G

F

4x4

E

9'-0"

2'-0"

2'-0"

3'-9-12

6'-5-12

3'-9-12

10'-3-8

I

H

D

2x4

4x6

3x4

9'-7-5

D=0

2-8-0

2-8-0

3-9-12

6-5-12

3-9-12

10-3-8

LUMBER

N L G A RULES

CHORDS SIZE

LUMBER

DESCR.

SPF

A - C

2x4

DRY

No.2

SPF

C - E

2x4

DRY

No.2

SPF

F - H

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B TMBH

MT20

3.0

4.0

C TTWW-m

MT20

5.0

6.0

Edge 2.00

D TMVW-t

MT20

2.0

4.0

E TMVW-t

MT20

4.0

4.0

F BMV1+p

MT20

3.0

4.0

G BMVWW1-t

MT20

4.0

6.0

H BMV1+w

MT20

2.0

4.0

EDGE - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

GROSS REACTION

GROSS REACTION

BRG

BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 166 0 166 0 0 9-7-5 1-8

B 191 0 191 0 0 9-7-5 1-8

H 244 0 244 0 0 9-7-5 1-8

G 546 0 546 0 0 9-7-5 1-8

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

F 118 78 / 0 0 / 0 0 / 0 40 / 0 0 / 0

B 133 100 / 0 0 / 0 0 / 0 33 / 0 0 / 0

H 174 106 / 0 0 / 0 0 / 0 68 / 0 0 / 0

G 384 261 / 0 0 / 0 0 / 0 124 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

VERT. LOAD

LC1

MAX

CS1 (LC)

UNBRAC LENGTH FR-TO

MEMB.

MAX. FACTORED FORCE (LBS)

MAX CS1 (LC)

A-B

0 / 15

-95.2

-95.2

0.02 (1)

10.00

H-C

-168 / 0

0.02 (1)

B-J

-36 / 0

-95.2

-95.2

0.01 (1)

8.25

C-G

-33 / 0

0.01 (1)

J-C

-53 / 0

-95.2

-95.2

0.04 (1)

8.25

G-D

-453 / 0

0.07 (1)

C-D

0 / 2

-95.2

-95.2

0.23 (1)

10.00

G-E

-2 / 0

0.00 (1)

D-E

0 / 2

-95.2

-95.2

0.23 (1)

10.00

I-J

-119 / 0

0.00 (1)

F-E

-138 / 0

0.0

0.0

0.01 (1)

7.81

B-I

0 / 40

-18.5

-18.5

0.05 (1)

10.00

I-H

0 / 40

-18.5

-18.5

0.05 (1)

10.00

H-G

0 / 29

-18.5

-18.5

0.07 (4)

10.00

G-F

0 / 0

-18.5

-18.5

0.07 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCS 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA S88-14

- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-D-1), BC=0.07/1.00 (G-H-4), WB=0.07/1.00 (D-G-1), CSI=0.18/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)

JSI METAL= 0.10 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

PROFESSIONAL ENGINEER

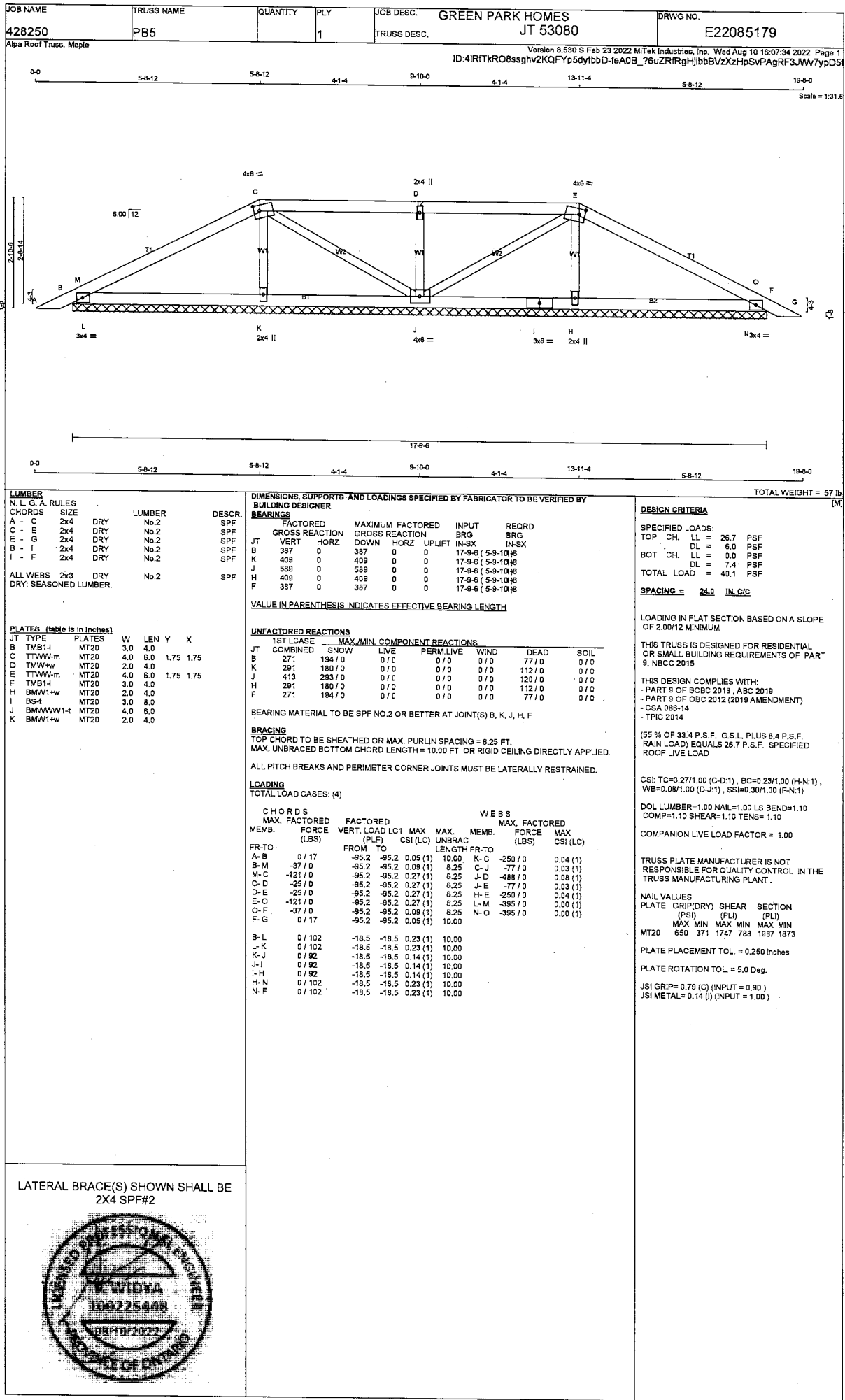
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2019/2022

PROVINCE OF ONTARIO

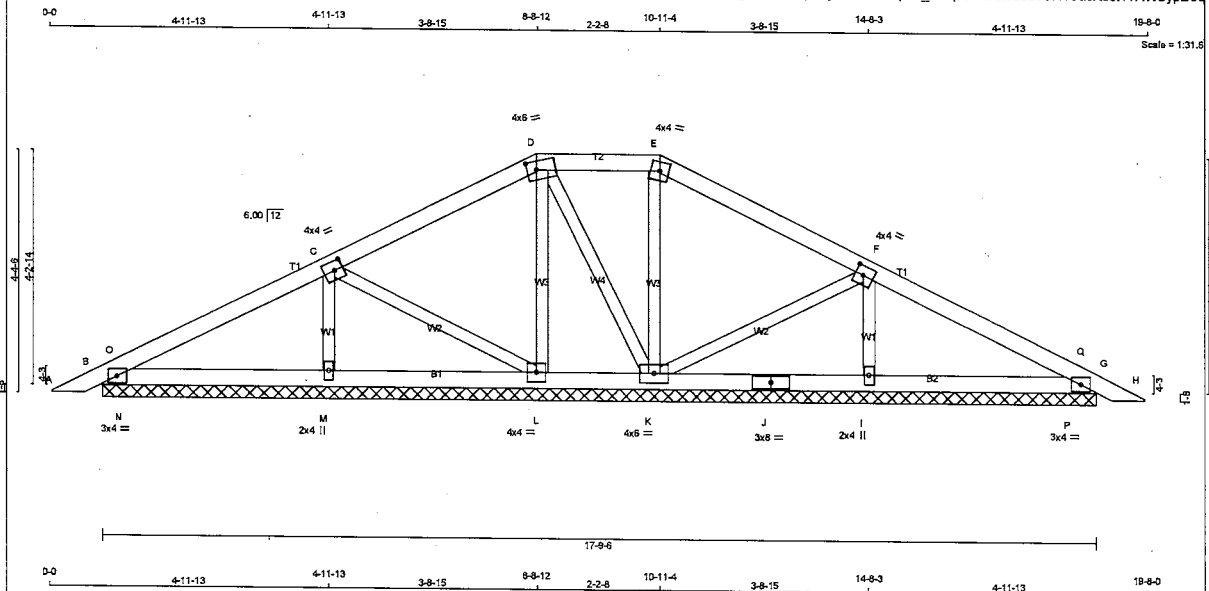
[illegible]



JOB NAME 428250	TRUSS NAME PB6	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085180
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Alpa Roof Truss, Maple

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ID:4IRITkRO8ssghv2KQFYp5dytbbD-4Ds9p72_BUqE180Ink8u6b9VH1VacXetx1YAWSypD5q



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
B - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TMBW14	MT20	4.0	4.0	2.00	1.75
D	TMBW14	MT20	4.0	4.0	1.75	2.00
E	TMBW14	MT20	4.0	4.0		
F	TMBW14	MT20	4.0	4.0	2.00	1.75
G	TMB14	MT20	3.0	4.0		
H	TMBW14	MT20	2.0	4.0		
I	BS-1	MT20	3.0	8.0		
J	BS-1	MT20	4.0	8.0		
K	BS-1	MT20	4.0	4.0		
L	BS-1	MT20	2.0	4.0		
M	BS-1	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
B	300	0	0	17-9-6 (5-9-10) 1/8
M	483	0	0	17-9-6 (5-9-10) 1/8
L	229	0	0	17-9-6 (5-9-10) 1/8
K	418	0	0	17-9-6 (5-9-10) 1/8
I	461	0	0	17-9-6 (5-9-10) 1/8
G	291	0	0	17-9-6 (5-9-10) 1/8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
B	210	151/0	0/0	0/0	59/0	0/0
M	342	225/0	0/0	0/0	117/0	0/0
L	153	102/0	0/0	0/0	63/0	0/0
K	293	207/0	0/0	0/0	86/0	0/0
I	326	212/0	0/0	0/0	114/0	0/0
G	204	145/0	0/0	0/0	58/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, L, K, I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
A-B	0/17	-95.2	-95.2	0.05 (1)	10.00	M-C	-371/0	0.08 (1)	
B-O	-91/0	-95.2	-95.2	0.02 (4)	6.25	C-L	-74/0	0.02 (1)	
O-C	-75/0	-95.2	-95.2	0.22 (1)	6.25	L-D	-151/0	0.04 (1)	
C-D	-41/0	-95.2	-95.2	0.22 (1)	6.25	D-K	-89/0	0.03 (1)	
D-E	0/28	-95.2	-95.2	0.08 (1)	10.00	K-E	-250/0	0.07 (1)	
E-F	-2/3	-95.2	-95.2	0.22 (1)	10.00	F-F	-98/0	0.03 (1)	
F-Q	-55/0	-95.2	-95.2	0.22 (1)	6.25	I-F	-349/0	0.05 (1)	
Q-G	-71/0	-95.2	-95.2	0.02 (4)	6.25	N-O	-156/5	0.00 (1)	
G-H	0/17	-95.2	-95.2	0.05 (1)	10.00	P-Q	-156/6	0.00 (1)	
B-N	0/84	-18.5	-18.5	0.10 (1)	10.00				
N-M	0/84	-18.5	-18.5	0.10 (1)	10.00				
M-L	0/84	-18.5	-18.5	0.08 (1)	10.00				
L-K	0/16	-18.5	-18.5	0.03 (4)	10.00				
K-J	0/67	-18.5	-18.5	0.08 (4)	10.00				
J-I	0/67	-18.5	-18.5	0.10 (1)	10.00				
I-P	0/87	-18.5	-18.5	0.10 (1)	10.00				
P-G	0/87	-18.5	-18.5	0.10 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
LL = 6.0 PSF
BOT CH. LL = 0.0 PSF
LL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (E-F-1), BC=0.10/1.00 (M-N-1), WB=0.07/1.00 (E-K-1), SSI=0.15/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

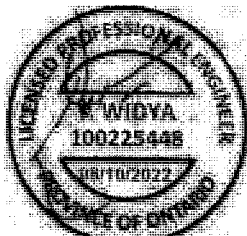
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1897 1873

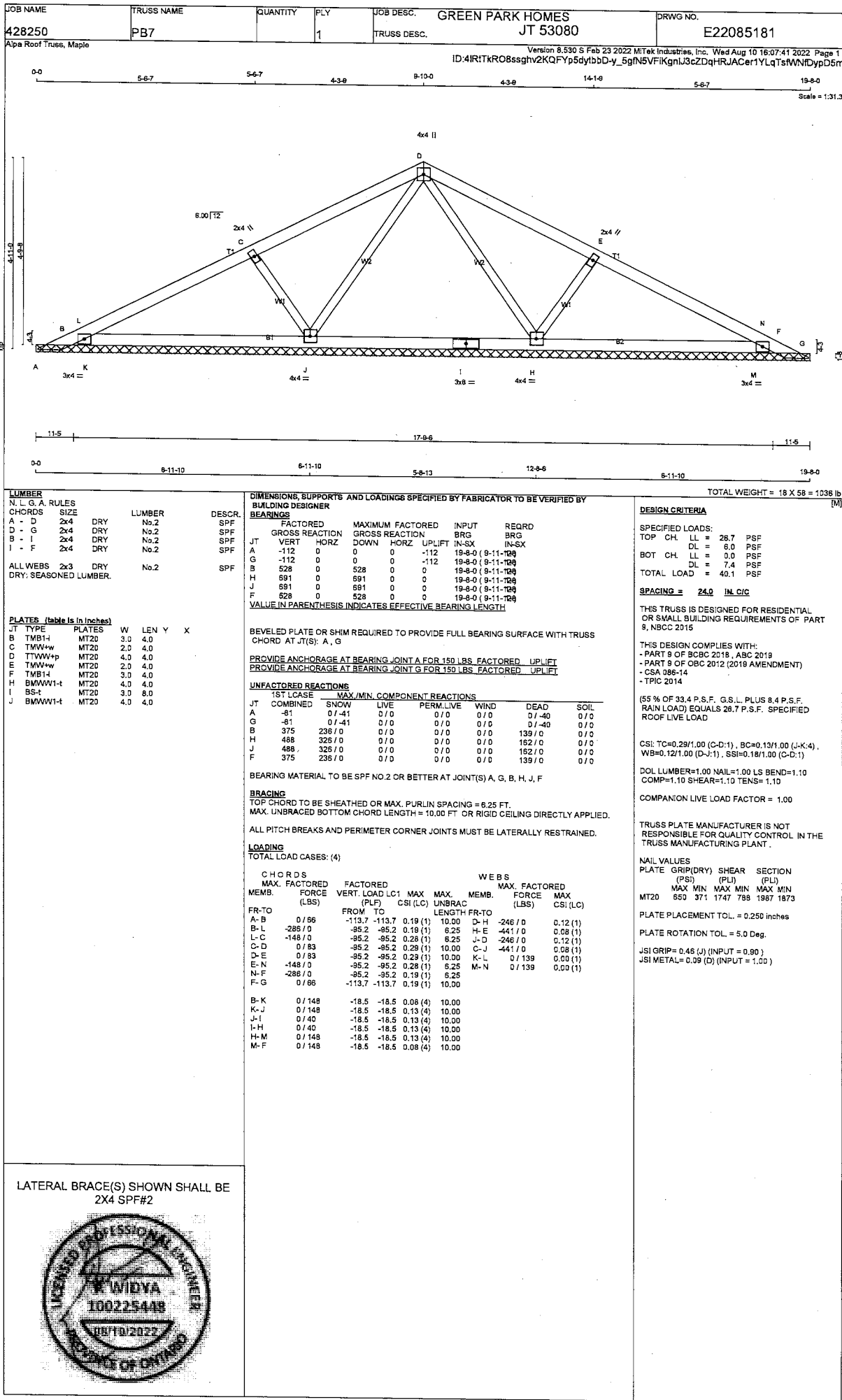
PLATE PLACEMENT TOL. = 0.250 inches

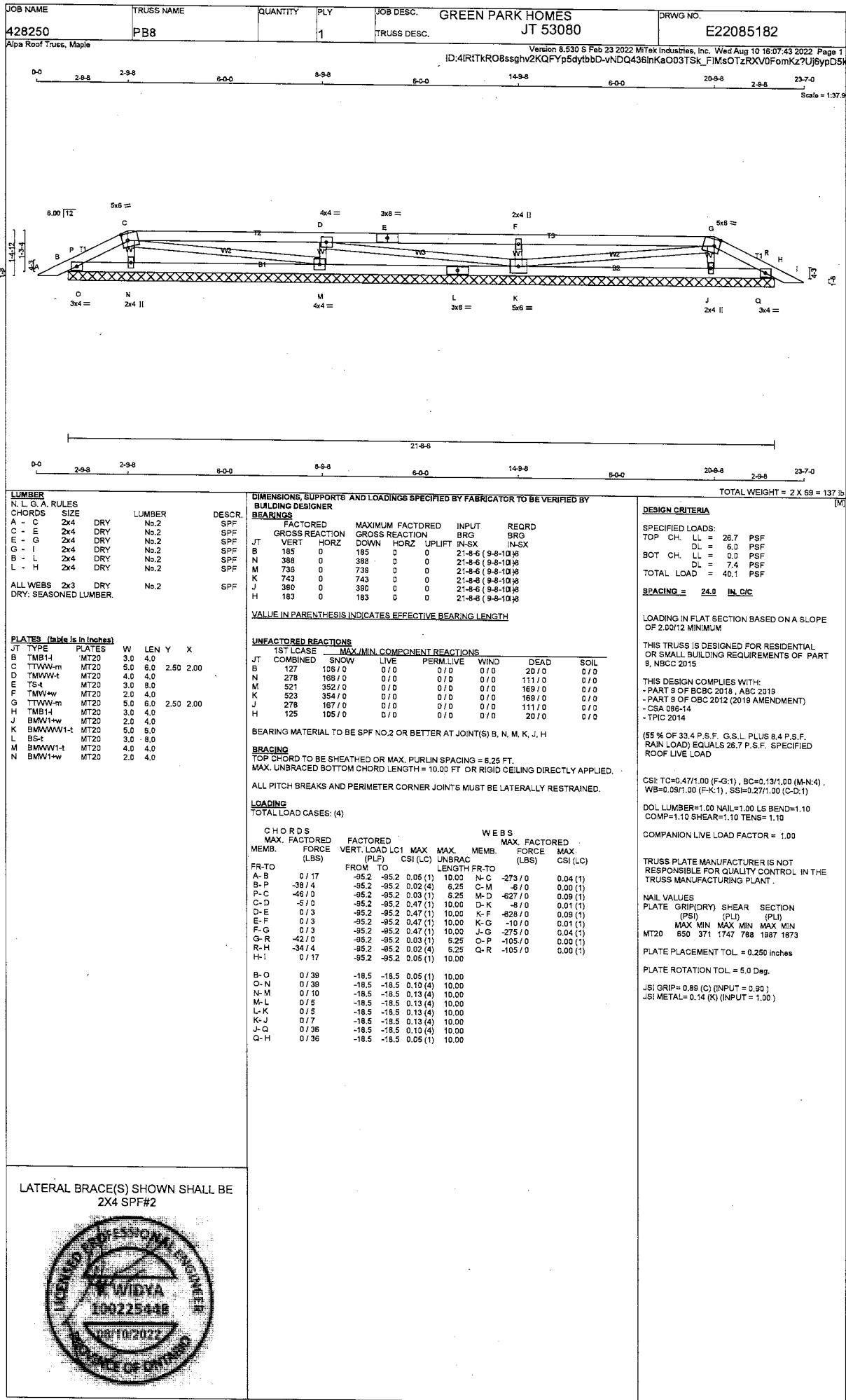
PLATE ROTATION TOL. = 5.0 Deg.

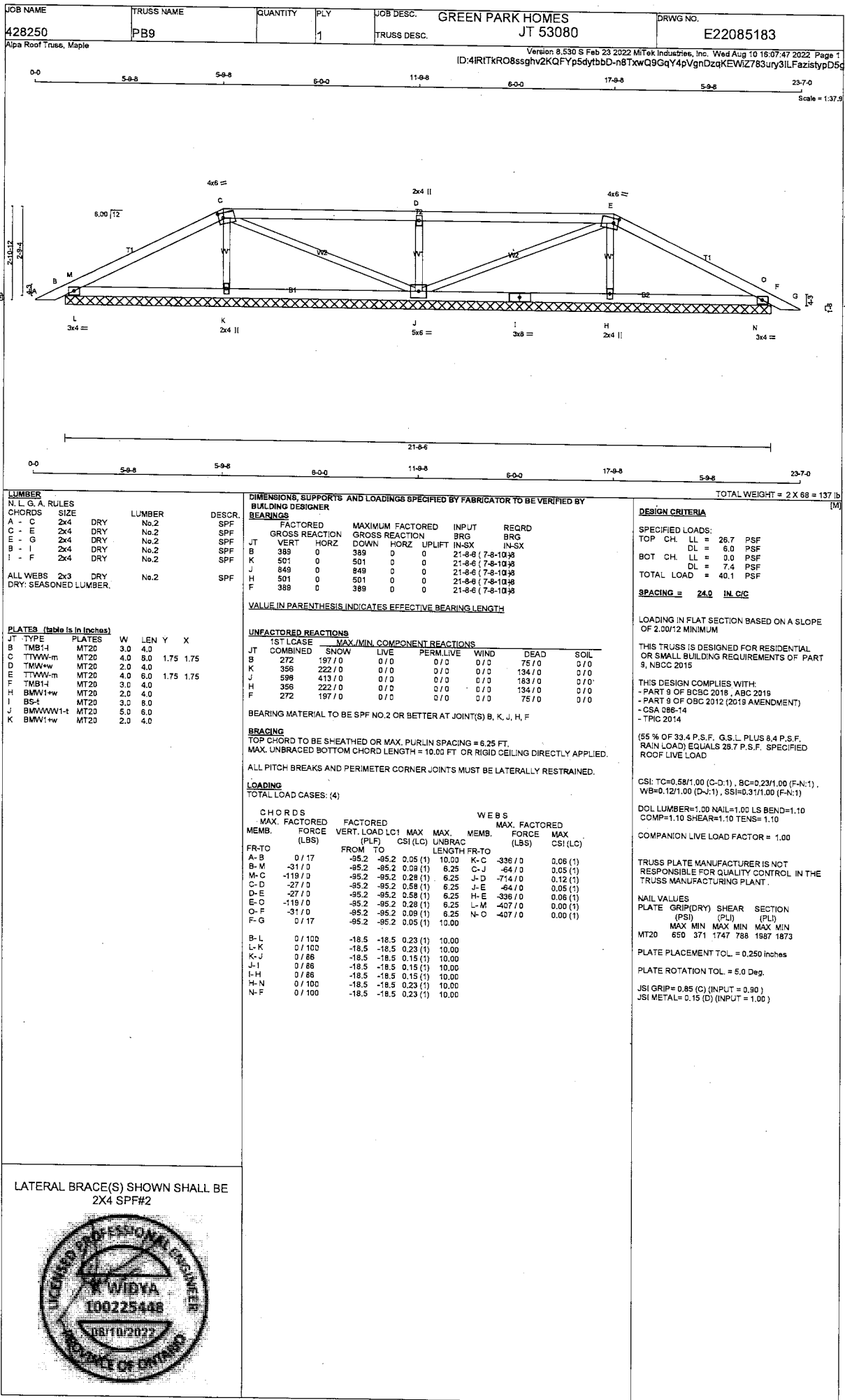
JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





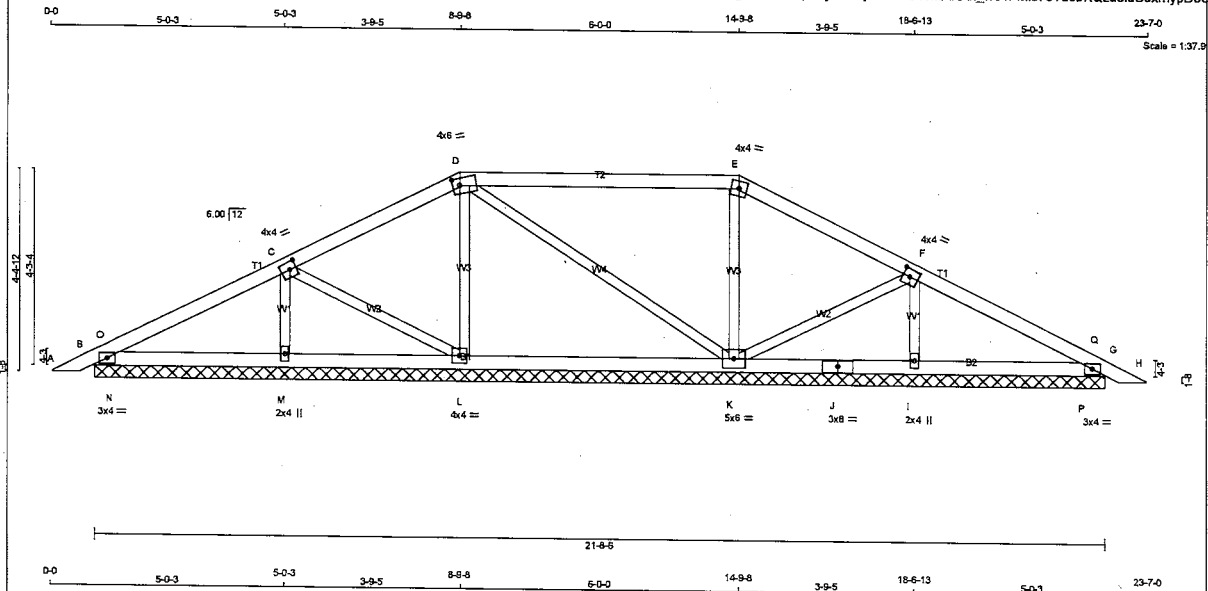




JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	PB10		1	TRUSS DESC.	JT 53080	E22085184

Alpa Roof Truss, Maple

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ID:4IRtkRO8ssghv2KQFYp5dytbbD-JahK6BWMAXKk_wc4FMic7eTdsbKQzueiuSoxmyDSe



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
B - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY
DRY SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TWB14	MT20	3.0	4.0		
C	TWW14	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TTWW-m	MT20	4.0	4.0		
F	TWW14	MT20	4.0	4.0	2.00	1.75
G	TWB14	MT20	3.0	4.0		
I	BMW14w	MT20	2.0	4.0		
J	BS-1	MT20	3.0	8.0		
K	BMWW14	MT20	5.0	8.0		
L	BMWW14	MT20	4.0	4.0		
M	BMW14w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
B	313	0	313	0	0	21-8-6 (5-8-10-8)			
M	497	0	497	0	0	21-8-6 (5-8-10-8)			
L	467	0	467	0	0	21-8-6 (5-8-10-8)			
K	583	0	583	0	0	21-8-6 (5-8-10-8)			
I	467	0	467	0	0	21-8-6 (5-8-10-8)			
G	301	0	301	0	0	21-8-6 (5-8-10-8)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	MAX / MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	219	157 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
M	350	239 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
L	332	209 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
K	412	274 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
I	328	222 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
G	211	150 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, L, K, I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-95.2 -95.2 0.05 (1)	10.00	M-C	-397 / 0	0.06 (1)	
B-O	-115 / 0	-95.2 -95.2 0.03 (4)	6.25	C-L	-50 / 0	0.02 (1)	
O-C	-98 / 0	-95.2 -95.2 0.23 (1)	6.25	L-D	-355 / 0	0.10 (1)	
C-D	-86 / 0	-95.2 -95.2 0.23 (1)	6.25	D-K	-68 / 0	0.06 (1)	
D-E	0 / 5	-95.2 -95.2 0.58 (1)	10.00	K-E	-418 / 0	0.11 (1)	
E-F	-27 / 0	-95.2 -95.2 0.23 (1)	6.25	F-F	-84 / 0	0.03 (1)	
F-Q	-69 / 0	-95.2 -95.2 0.23 (1)	6.25	I-F	-367 / 0	0.06 (1)	
Q-G	-88 / 0	-95.2 -95.2 0.03 (4)	6.25	N-O	-150 / 13	0.00 (1)	
G-H	0 / 17	-95.2 -95.2 0.05 (1)	10.00	P-Q	-151 / 13	0.00 (1)	
B-N	0 / 103	-18.5 -18.5 0.10 (1)	10.00				
N-M	0 / 103	-18.5 -18.5 0.11 (1)	10.00				
M-L	0 / 103	-18.5 -18.5 0.11 (4)	10.00				
L-K	0 / 50	-18.5 -18.5 0.10 (4)	10.00				
K-J	0 / 80	-18.5 -18.5 0.10 (4)	10.00				
J-I	0 / 80	-18.5 -18.5 0.10 (1)	10.00				
I-P	0 / 80	-18.5 -18.5 0.10 (1)	10.00				
P-G	0 / 80	-18.5 -18.5 0.10 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF BCBC 2018, ABC 2019
-PART 9 OF OBC 2012 (2016 AMENDMENT)
-CSA D86-14
-TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.58/1.00 (D-E:1), BC=0.11/1.00 (L-M:4), WB=0.11/1.00 (E-K:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

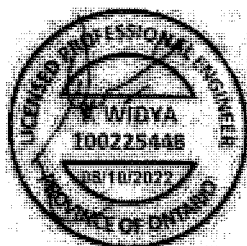
MT20 550 371 1747 788 1597 1673

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JISI GRIP= 0.79 (D) (INPUT = 0.80)
JISI METAL= 0.17 (D) (INPUT = 1.00)

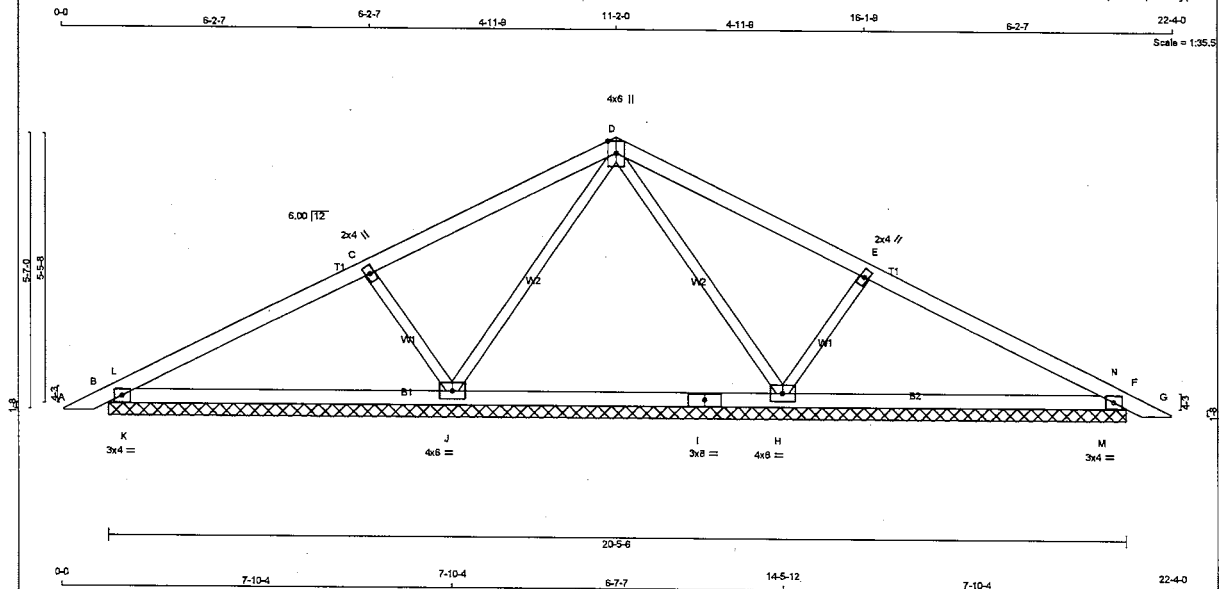
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME PB12	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085186
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Alpa Roof Truss, Maple

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ID:4IRtKRO8ssghv2KQFYp5dytbbD-Ygyb9GHy0ShSvOIRVT7rOuXnHcJqIOX4qv79PypDSY



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMB14	MT20	3.0	4.0	
C TMW+p	MT20	2.0	4.0	
D TTWW+p	MT20	4.0	8.0	Edge
E TMW+p	MT20	2.0	4.0	
F TMB14	MT20	3.0	4.0	
H BMWW1-t	MT20	4.0	6.0	
I BS-t	MT20	3.0	8.0	
J BMWW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
B	431	0	0	20-5-6 (11-11-18)
H	812	0	0	20-5-6 (11-11-18)
J	812	0	0	20-5-6 (11-11-18)
F	431	0	0	20-5-6 (11-11-18)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS	1ST LOASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
B	303	210 / 0	0 / 0 0 / 0 93 / 0 0 / 0
H	573	382 / 0	0 / 0 0 / 0 192 / 0 0 / 0
J	573	382 / 0	0 / 0 0 / 0 192 / 0 0 / 0
F	303	210 / 0	0 / 0 0 / 0 93 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX. UNBRAC	MEMB.	MAX. FACTORED	MAX. UNBRAC
FR-TO	FORCE (LBS)	VERT. (PLF)	CS (LC)	LENGTH	FR-TO	FORCE (LBS)	CS (LC)
A-B	0 / 17	-95.2	-95.2 0.05 (1)	10.00	D-H	-261 / 0	0.18 (1)
B-L	-270 / 0	-95.2	-95.2 0.09 (4)	6.25	H-E	-533 / 0	0.11 (1)
L-C	-209 / 0	-95.2	-95.2 0.41 (1)	6.25	J-D	-261 / 0	0.18 (1)
C-D	0 / 61	-95.2	-95.2 0.41 (1)	10.00	C-J	-533 / 0	0.11 (1)
D-E	0 / 61	-95.2	-95.2 0.41 (1)	10.00	K-L	-117 / 92	0.00 (1)
E-N	-209 / 0	-95.2	-95.2 0.41 (1)	6.25	M-N	-117 / 92	0.00 (1)
N-F	-270 / 0	-95.2	-95.2 0.09 (4)	6.25			
F-G	0 / 17	-95.2	-95.2 0.05 (1)	10.00			
B-K	0 / 213	-18.5	-18.5 0.12 (1)	10.00			
K-J	0 / 213	-18.5	-18.5 0.20 (4)	10.00			
J-I	0 / 63	-18.5	-18.5 0.19 (4)	10.00			
I-H	0 / 63	-18.5	-18.5 0.19 (4)	10.00			
H-M	0 / 213	-18.5	-18.5 0.20 (4)	10.00			
M-F	0 / 213	-18.5	-18.5 0.12 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (D-E:1), BC=0.23/1.00 (J-K:4), WB=0.18/1.00 (D-H:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

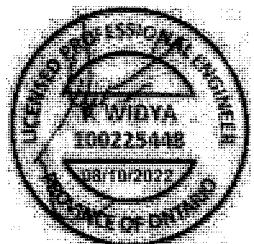
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

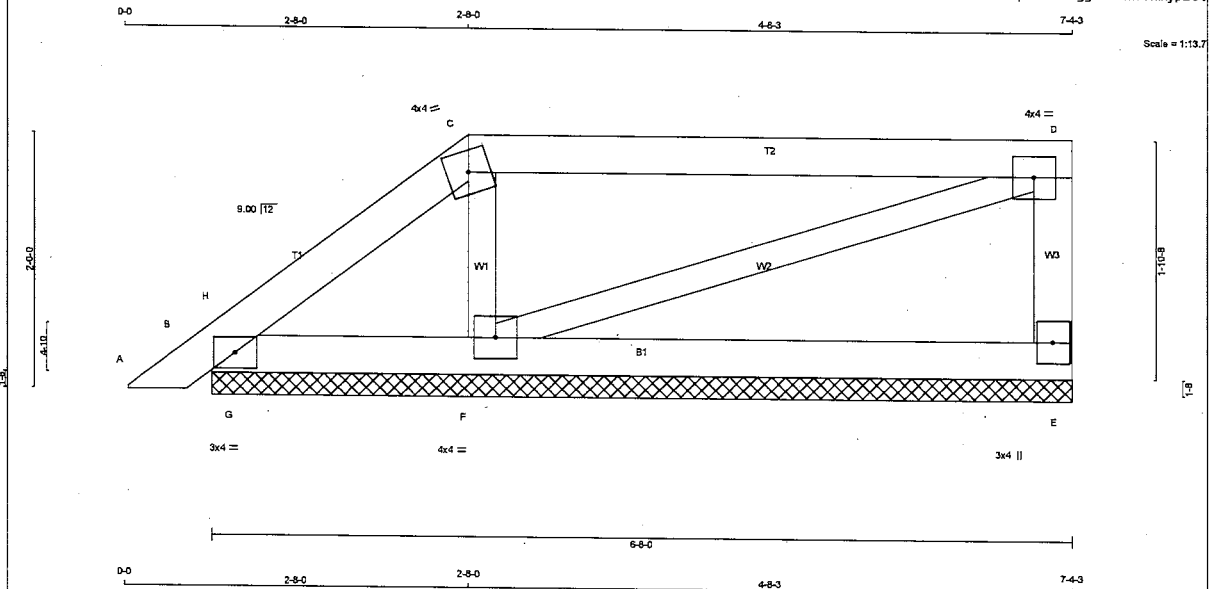
JSI GRIP= 0.40 (J) (INPUT = 0.80)
JSI METAL= 0.11 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRtKRO8ssghv2KQFYp5dytbbD-yf5D8I9ExTGJMK6e1qT1W3wUgg12Hzmo8nikypD5V



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	252	0	252	0	0	6-8-0	1-8	
B	177	0	177	0	0	6-8-0	1-8	
F	374	0	374	0	0	6-8-0	1-8	

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
E	184	128 / 0	0 / 0	0 / 0	56 / 0	0 / 0
B	122	99 / 0	0 / 0	0 / 0	23 / 0	0 / 0
F	297	160 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 15	-95.2 -95.2 0.02 (1)	10.00	F-C -271 / 0 0.04 (1)
B-H	-14 / 10	-95.2 -95.2 0.02 (1)	6.25	F-D 0 / 18 0.00 (1)
H-C	-48 / 0	-95.2 -95.2 0.04 (1)	6.25	G-H -145 / 0 0.00 (1)
C-D	-17 / 0	-95.2 -95.2 0.35 (1)	6.25	
E-D	-229 / 0	0.0 0.0 0.02 (1)	7.81	
B-G	0 / 35	-18.5 -18.5 0.05 (1)	10.00	
G-F	0 / 35	-18.5 -18.5 0.09 (4)	10.00	
F-E	0 / 0	-18.5 -18.5 0.09 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (C-D:1), BC=0.08/1.00 (F-G:4), WB=0.04/1.00 (C-F:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

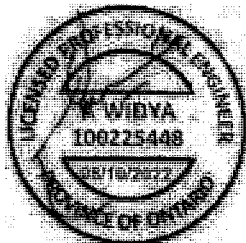
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

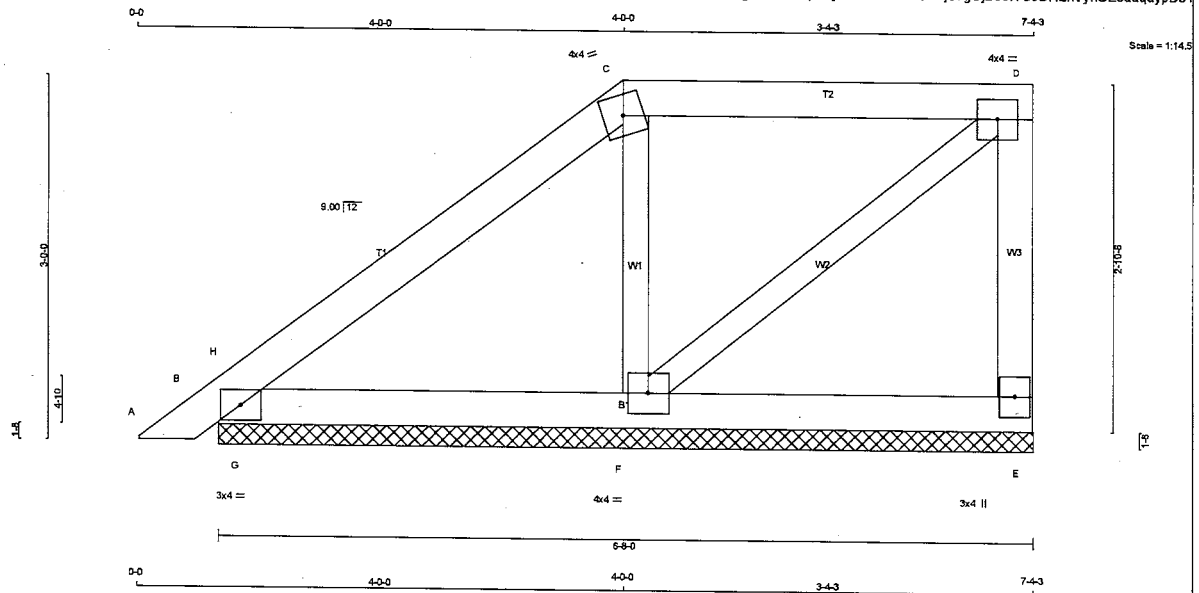
JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRITkRO8ssghv2KQFYp5dytbbD-veiretKQmYzYgGjD33IYScS7ILnVyhGE6duqypD5T



LUMBER	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	MT20	3.0	4.0	
B TMB1-t	MT20	3.0	4.0	
C TTW-m	MT20	4.0	4.0	
D TMVV-t	MT20	4.0	4.0	
E BMV1-p	MT20	3.0	4.0	
F BMVV1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS	REACTION	GROSS	REACTION	UPLIFT	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
E	204	0	204	0	0	8-8-0	1-8
B	288	0	288	0	0	8-8-0	1-8
F	340	0	340	0	0	8-8-0	1-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	143	101/0	0/0	0/0	0/0	43/0
E	188	136/0	0/0	0/0	0/0	52/0
B	242	151/0	0/0	0/0	0/0	91/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO							
A-B	0/15	-95.2	-95.2	0.02 (1)	10.00	F-C	-257/0	0.04 (1)	
B-H	-16/28	-95.2	-95.2	0.05 (1)	6.25	F-D	0/47	0.01 (1)	
H-C	-67/0	-95.2	-95.2	0.13 (1)	6.25	G-H	-295/0	0.00 (1)	
C-D	-37/0	-95.2	-95.2	0.18 (1)	6.25				
E-D	-189/0	0.0	0.0	0.03 (1)	7.81				
B-G	0/47	-18.5	-18.5	0.11 (1)	10.00				
G-F	0/47	-18.5	-18.5	0.11 (1)	10.00				
F-E	0/0	-18.5	-18.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1), BC=0.11/1.00 (F-G:1), WB=0.04/1.00 (C-F:1), SSI=0.23/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

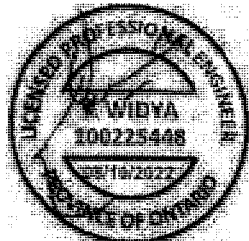
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.80)
JSI METAL= 0.08 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

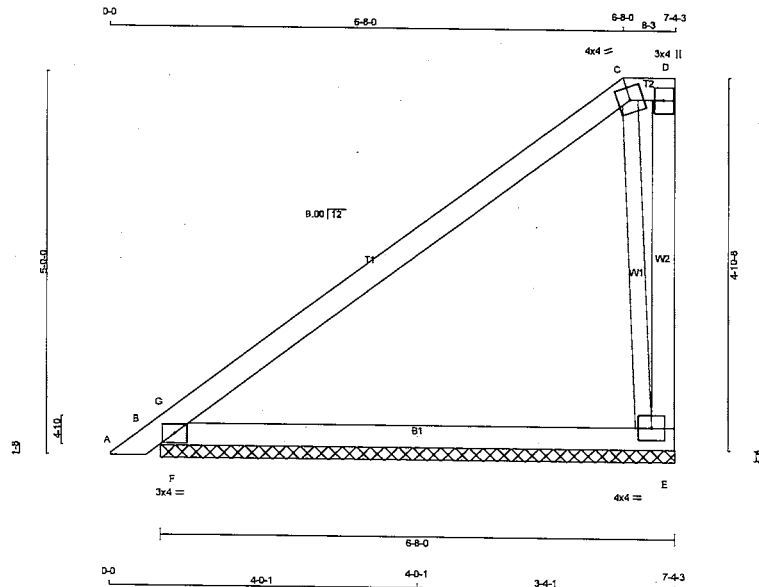


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES JT 53080	DRWG NO. E22085189		
428250	PB18	1		TRUSS DESC.				
Alpa Roof Truss, Maple								
						Version 8.530 S Feb 23 2022 M/Tek Industries, Inc. Wed Aug 10 16:08:02 2022 Page 1 ID:4IRtKRO8ssghv2KQFYp5dytbD-r1tc3YLgI9zho_QBLU5mdthrV6?jzszyHQ6_uYvpD5R		
Scale = 1:18								
TOTAL WEIGHT = 2 X 27 = 54 lb								
LUMBER	N.L.G.A. RULES	SIZE	DRIY	No.2	DESCR.	SPF		
A - C	C	2x4	DRIY	No.2	SPF			
C - D	D	2x4	DRIY	No.2	SPF			
E - D	D	2x4	DRIY	No.2	SPF			
B - E	E	2x4	DRIY	No.2	SPF			
ALL WEBS	2x3	DRIY	No.2	SPF				
DRIY SEASONED LUMBER,								
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								
BEARINGS								
FACTORED GROSS REACTION MAXIMUM FACTORED BRG INPUT REQRD								
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX IN-SX		
E	132	0	132	0	0	6-8-0 1-8		
B	335	0	335	0	0	6-8-0 1-8		
F	346	0	346	0	0	6-8-0 1-8		
UNFACTORED REACTIONS								
1ST CASE MAX/MIN COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD SOIL		
E	92	70 / 0	0 / 0	0 / 0	0 / 0	22 / 0 0 / 0		
B	235	165 / 0	0 / 0	0 / 0	0 / 0	69 / 0 0 / 0		
F	246	152 / 0	0 / 0	0 / 0	0 / 0	94 / 0 0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F								
BRACING								
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.								
MAX. UNBRADED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.								
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.								
LOADING								
TOTAL LOAD CASES: (4)								
CHORDS								
MEMB. FORCE MAX. FACTORED VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FORCE MAX.								
(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)								
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO		
A-B	-95.2	-95.2	0.02 (1)	10.00	F-C	-251 / 0	0.06 (1)	
B-H	-15 / 170	-95.2	-95.2	0.15 (1)	6.25	F-D	0 / 78	0.02 (1)
H-C	-67 / 0	-95.2	-95.2	0.25 (1)	6.25	G-H	-596 / 0	0.00 (1)
C-D	-37 / 0	-95.2	-95.2	0.07 (1)	6.25			
E-D	-164 / 0	0.0	0.0	0.04 (1)	7.81			
B-G	0 / 44	-18.5	-18.5	0.21 (1)	10.00			
G-F	0 / 44	-18.5	-18.5	0.21 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.13 (1)	10.00			
DESIGN CRITERIA								
SPECIFIED LOADS:								
TOP CH. LL = 28.7 PSF								
DL = 6.0 PSF								
BOT CH. LL = 0.0 PSF								
DL = 7.4 PSF								
TOTAL LOAD = 40.1 PSF								
SPACING = 24.0 IN./C/C								
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM								
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015								
THIS DESIGN COMPLIES WITH:								
- PART 9 OF CBC 2015, ABC 2019								
- PART 9 OF OBC 2012 (2019 AMENDMENT)								
- CSA 086-14								
- TPIC 2014								
(55 % OF 33.4 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD								
CSI: TC=0.25/1.00 (C-H:1), BC=0.21/1.00 (B-G:1), WB=0.06/1.00 (C-F:1), SS=0.44/1.00 (B-G:1)								
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10								
COMPANION LIVE LOAD FACTOR = 1.00								
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .								
NAIL VALUES								
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)								
MT20 MAX MIN MAX MIN MAX MIN								
850 371 1747 788 1987 1873								
PLATE PLACEMENT TOL = 0.250 inches								
PLATE ROTATION TOL = 5.0 Deg.								
JSI GRIP= 0.28 (B) (INPUT = 0.90) JSI METAL= 0.10 (C) (INPUT = 1.00)								
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2								

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	PB19		1	TRUSS DESC.	JT 53080	E22085190

Alpa Roof Truss, Maple

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ID:4IRtKrOBssghv2KQFYp6dytbbD-FcYkhaNYb4LGR9g0cFTFVJEQJ0uACB7OOKNvqypD5C



Scale = 1/22.8

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B	MT20	3.0	4.0	
C	TTW-m	4.0	4.0	
D	TMV+p	3.0	4.0	
E	BMVV-1	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	DOWN	HORZ	UPLIFT	IN-SX
E	379	0	0	6-8-0
B	434	0	0	6-8-0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	268	178 / 0	0 / 0	0 / 0	89 / 0
B	305	209 / 0	0 / 0	0 / 0	96 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH		(LBS)	CSI (LC)	CSI (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 15	-95.2	-95.2	0.02 (1)	10.00	C-E	-260 / 0	0.09 (1)	
B-G	-78 / 242	-95.2	-95.2	0.19 (1)	8.25	F-G	-720 / 54	0.00 (1)	
G-C	-56 / 0	-95.2	-95.2	0.47 (1)	8.25				
C-D	0 / 0	-95.2	-95.2	0.01 (1)	10.00				
E-D	-30 / 0	0.0	0.0	0.01 (1)	7.81				
B-F	0 / 36	-18.5	-18.5	0.29 (1)	10.00				
F-E	0 / 36	-18.5	-18.5	0.31 (1)	10.00				

TOTAL WEIGHT = 2 X 26 = 53 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (C-G:1), BC=0.31/1.00 (E-F:1), WB=0.09/1.00 (C-E:1), SS=0.59/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

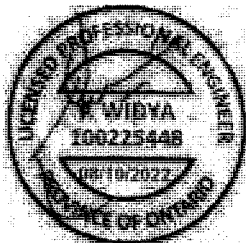
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.99)
JSI METAL= 0.13 (C) (INPUT = 1.00)

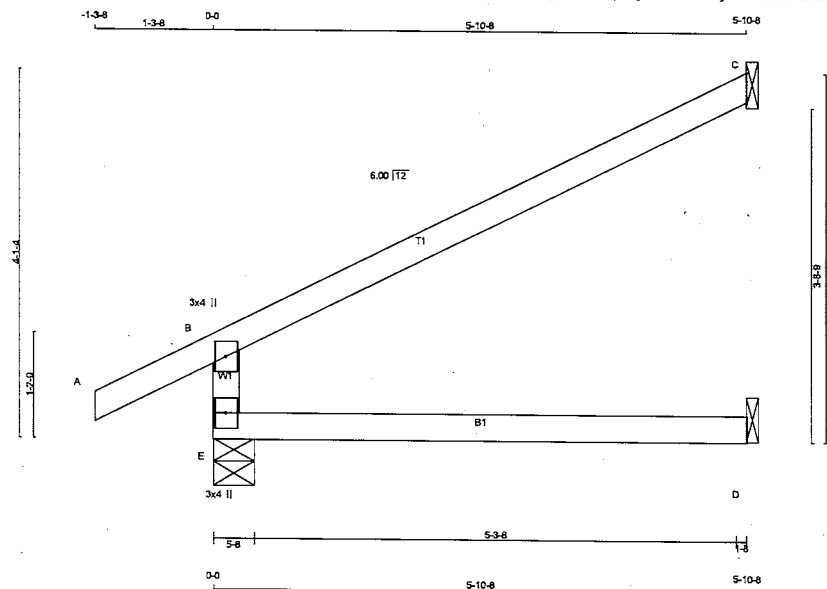
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME J1	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085161
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Alpa Roof Truss, Maple

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ID:4IRItkRO8ssghv2KQFYp5dytbbD-MICWayWOYzrH0NeroUMaKJv7HTTrfoOqBusz?ypD6W



Scale = 1:19.4

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (Table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
E	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT
JT	VERT	HORZ	IN-SX	IN-SX
E	542	0	0	5-8
C	210	0	0	1-8
D	45	0	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW
E	380	269/0
C	144	119/0
D	36	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (P)	LC1	MAX CSl (C)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSl (C)	
FR-TO		FROM TO							
E-B	-479/0	0.0	0.0	0.13 (4)	7.81				
A-B	0/29	-95.2	-95.2	0.12 (1)	10.00				
B-C	-31/0	-95.2	-95.2	0.56 (1)	6.25				
E-D	0/0	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ASC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.56/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (N/A:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

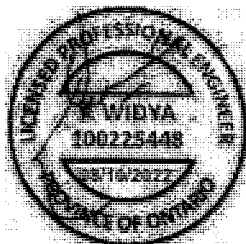
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1573

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

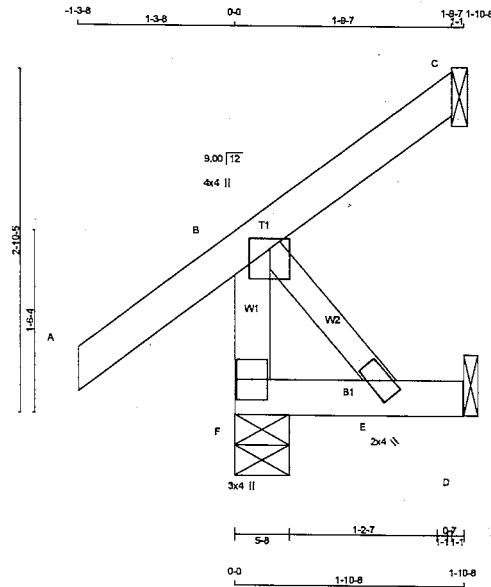
JSI GRIP= 0.19 (E) (INPUT = 0.80)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRITkRO8ssghv2KQFYp5dytbbD-J8KH7eXe4aXCfHdWvO2QKPLL5B2mZhhVny2uypD8U



Scale = 1:14.5

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	284	0	284	0	5-8
C	34	0	34	0	1-8
D	17	0	17	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS						
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	197	150/0	0/0	0/0	0/0	48/0
C	24	19/-28	0/0	0/0	0/0	4/0
D	14	0/0	0/0	0/0	0/0	14/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		MEMB.		WEBS	
MAX. FACTORED	FACTORED	FORCE	VERT. LOAD	MAX. FACTORED	FORCE
(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO	FROM TO	FR-TO	FROM TO	FR-TO	FROM TO
A-B	-287/0	0.0	0.0	0.03(1)	7.81
B-C	0/39	-95.2	-95.2	0.13(1)	10.00
F-E	-30/0	-95.2	-95.2	0.12(1)	6.25
E-D	0/0	-18.5	-18.5	0.02(4)	10.00
				-18.5	0.02(4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA D86-14
- TPIG 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

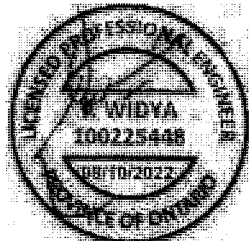
NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	650	371	1747	798	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 1.00)

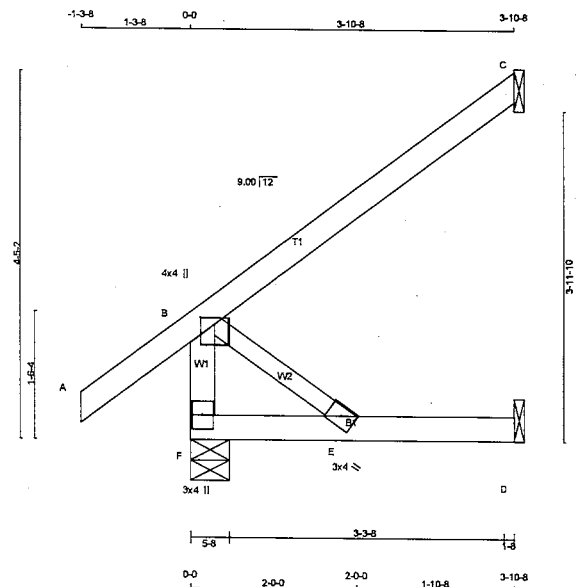
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME J8	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085163
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Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Wed Aug 10 16:05:57 2022 Page 1
ID:4IRITkRO8ssghv2KQFYp5dyIbbD-FWR1QKZvcBnvV_yc1KQWw9Vg3vryESn_ips36nypD6S



Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
F	352	0	0	1-8
C	184	0	0	1-8
D	36	0	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
JT	246	177/0 0/0 0/0 0/0 68/0 0/0
F	127	104/0 0/0 0/0 0/0 23/0 0/0
C	29	0/0 0/0 0/0 0/0 29/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CS1 (LC) UNBRAC LENGTH FR-TO	MEMB. FORCE MAX (LBS)
FR-TO				
F-B	-316/0	0.0 0.0 0.02(1)	7.81	B-E 0/0 0.00(1)
A-B	0/39	-95.2 -95.2 0.15(5)	10.00	
B-C	0/0	-95.2 -95.2 0.24(1)	12.00	
F-E	0/0	-18.5 -18.5 0.08(4)	10.00	
E-D	0/0	-18.5 -18.5 0.08(4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.24/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

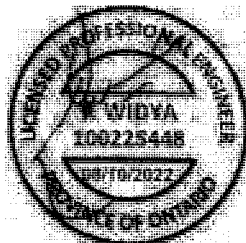
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

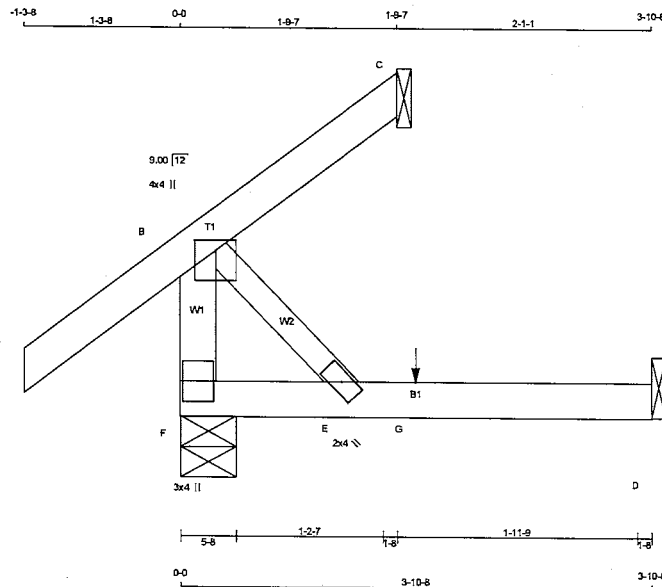
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME J9	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085164
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Alpa Roof Truss, Maple

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ID:4IRITkRO8ssghv2KQFyp5dytbbD-BvZor0a98p1dkI679IT_8aa14X0IMH-IC6LABfypD6C



Scale = 1:14.5

LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER
F - B 2x4 DRY No.2
A - C 2x4 DRY No.2
F - D 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BNVW+p	MT20	2.0	4.0		
F BNV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
JT	303	0	303	0	5-8	1-8		
F	34	0	77	0	4-1	1-8		
C	36	0	40	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS								
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F		212	150/0	0/0	0/0	0/0	62/0	0/0
C		24	48/-28	0/0	0/0	0/0	4/0	0/0
D		29	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

MEMB.	CHORDS		FACTORED		W E B S		FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CSI (LC)	MEMB. FORCE (LBS)	MAX. CSI (LC)
FR-TO								
F-B	-267/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/38	-95.2	-95.2	0.15 (5)	10.00			
B-C	-30/0	-95.2	-95.2	0.14 (5)	6.25			
F-E	0/0	-18.5	-18.5	0.07 (4)	10.00			
E-G	0/0	-18.5	-18.5	0.08 (4)	10.00			
G-D	0/0	-18.5	-18.5	0.08 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
G	1-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = U/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = U/999 (0.01")

CSI: TC=0.15/1.00 (A-B-S), BC=0.09/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.09/1.00 (A-S-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1973

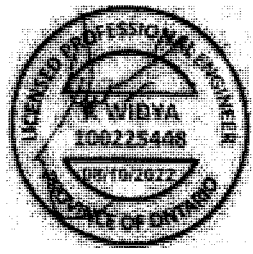
PLATE PLACEMENT TOL. = 0.250 inches

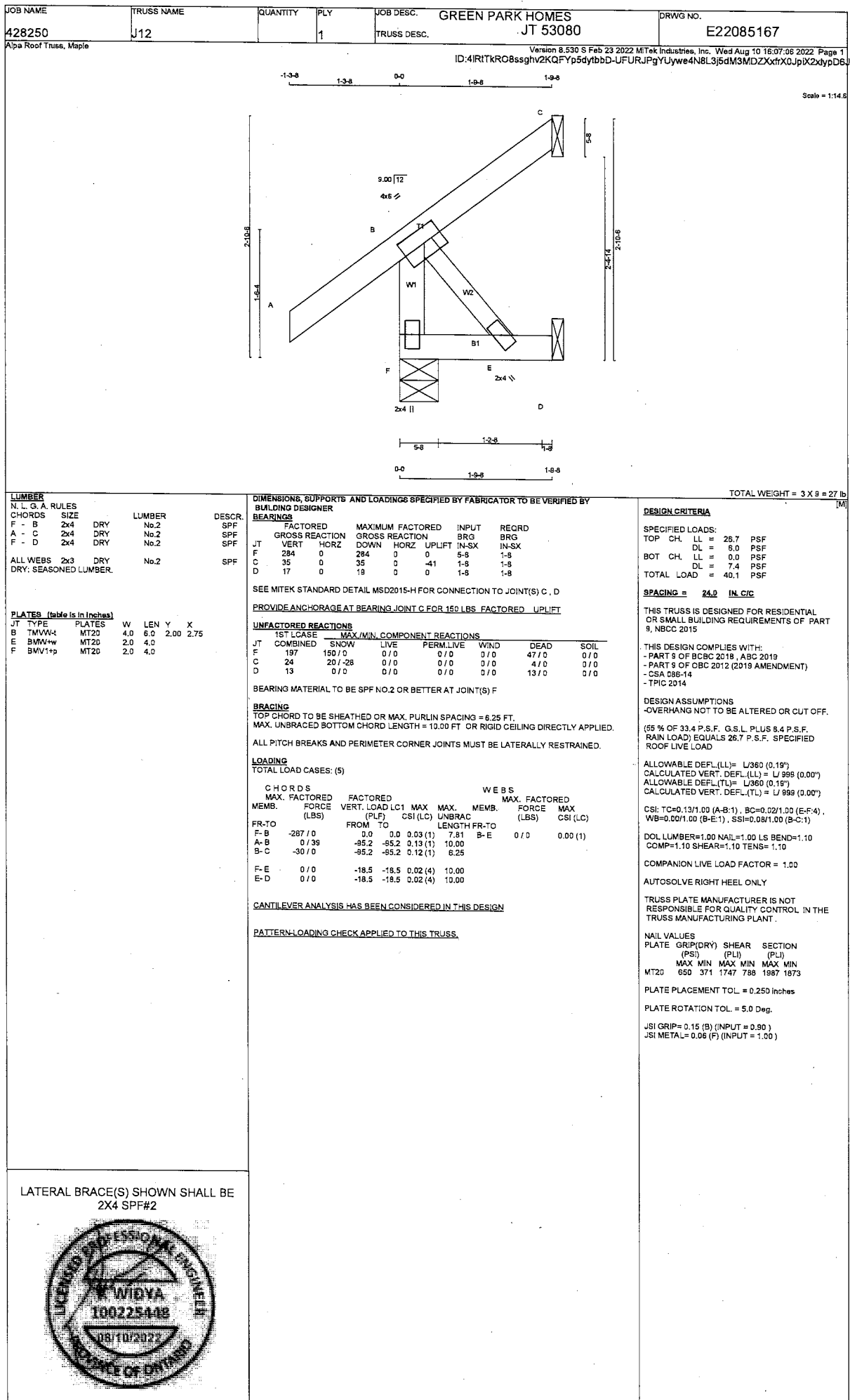
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

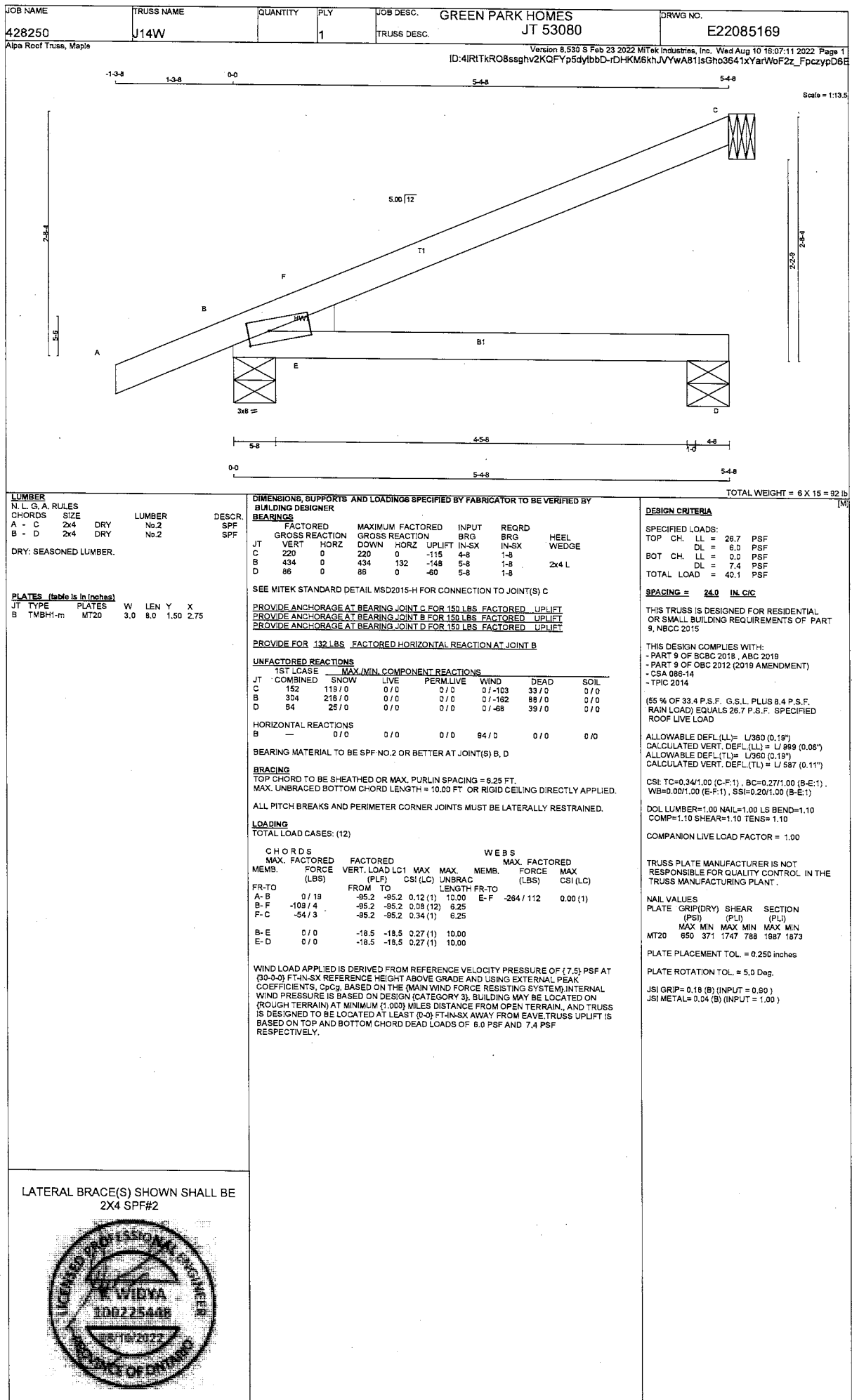
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2







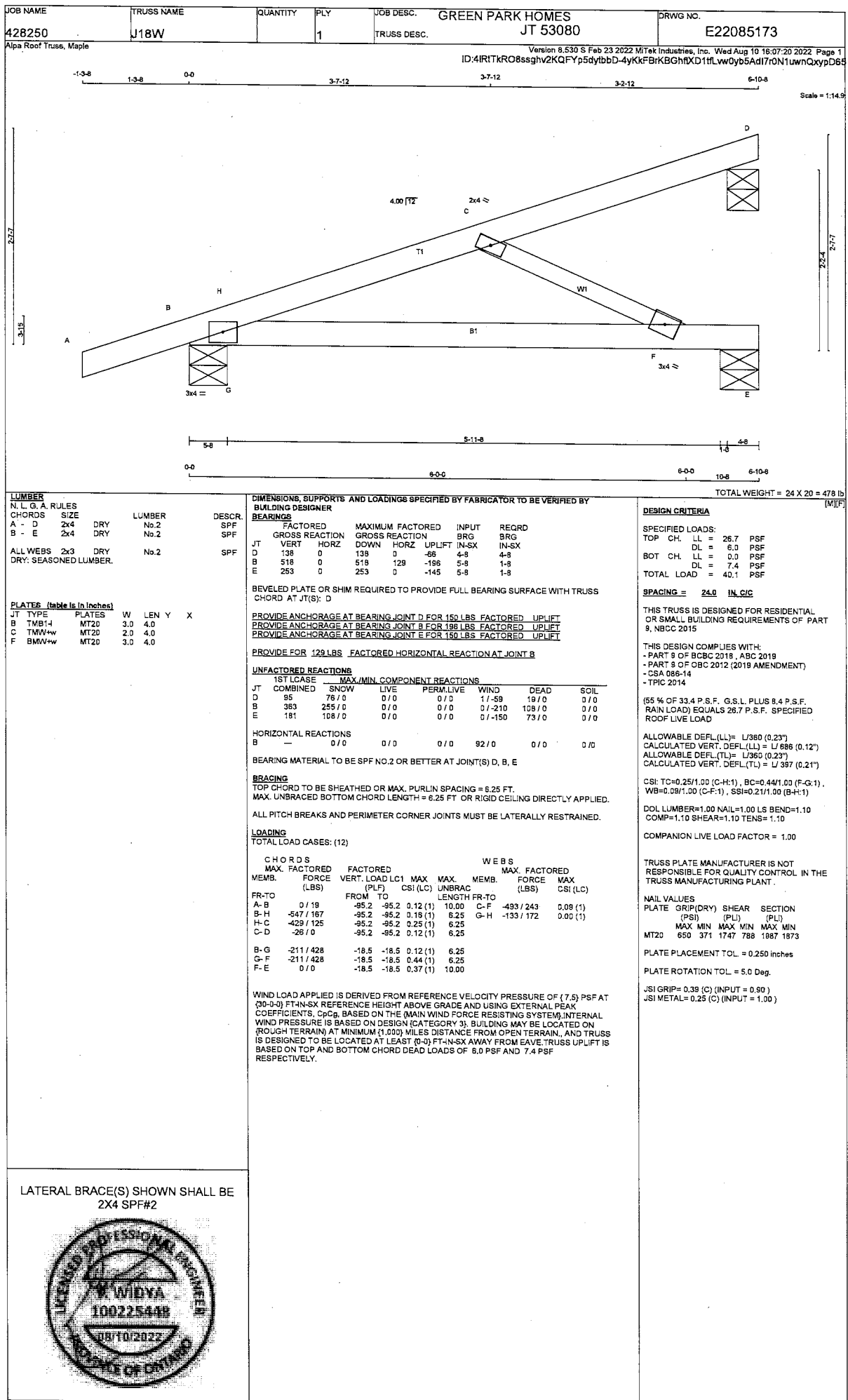
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JOB NAME 428250	TRUSS NAME J17	QUANTITY 1	PLY 3	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085172																																																										
Alpa Roof Truss, Maple																																																															
Version 8.530 S Feb 23 2022 M/Tek Industries, Inc. Wed Aug 10 16:07:17 2022 Page 1 ID:4JRTKROB8ssghv2KQFYp5dytbbD-nfbdAoRuL14u3USCXCnCJNK6ZzZ9wTPxLwi7qcyPD68																																																															
Scale: 3/4"=1'																																																															
TOTAL WEIGHT = 10 X 22 = 224 lb [M/F]																																																															
LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER A - D 2x4 DRY No.2 B - E 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY; SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQ'D BRG JT VERT HORZ D 131 0 131 0 0 4-8 IN-SX IN-SX B 556 0 556 0 0 5-8 1-8 E 288 0 288 0 0 5-8 1-8 BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL D 80 72/0 0/0 0/0 0/0 18/0 0/0 B 390 273/0 0/0 0/0 0/0 117/0 0/0 E 212 130/0 0/0 0/0 0/0 83/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table border="1"><thead><tr><th rowspan="2">MEMB.</th><th rowspan="2">FR-TO</th><th colspan="2">CHORDS</th><th colspan="2">WEBS</th></tr><tr><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th></tr></thead><tbody><tr><td>A-B</td><td>0/19</td><td>-95.2</td><td>-95.2</td><td>0.12 (1)</td><td>10.00</td></tr><tr><td>B-I</td><td>-707/0</td><td>-95.2</td><td>-95.2</td><td>0.02 (1)</td><td>6.25</td></tr><tr><td>I-C</td><td>-701/0</td><td>-95.2</td><td>-95.2</td><td>0.14 (1)</td><td>6.25</td></tr><tr><td>C-D</td><td>-8/0</td><td>-95.2</td><td>-95.2</td><td>0.11 (1)</td><td>10.00</td></tr><tr><td>B-H</td><td>0/670</td><td>-18.5</td><td>-18.5</td><td>0.25 (1)</td><td>10.00</td></tr><tr><td>H-G</td><td>0/670</td><td>-18.5</td><td>-18.5</td><td>0.25 (1)</td><td>10.00</td></tr><tr><td>G-F</td><td>0/670</td><td>-18.5</td><td>-18.5</td><td>0.53 (1)</td><td>10.00</td></tr><tr><td>F-E</td><td>0/0</td><td>-18.5</td><td>-18.5</td><td>0.43 (1)</td><td>10.00</td></tr></tbody></table>		MEMB.	FR-TO	CHORDS		WEBS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	A-B	0/19	-95.2	-95.2	0.12 (1)	10.00	B-I	-707/0	-95.2	-95.2	0.02 (1)	6.25	I-C	-701/0	-95.2	-95.2	0.14 (1)	6.25	C-D	-8/0	-95.2	-95.2	0.11 (1)	10.00	B-H	0/670	-18.5	-18.5	0.25 (1)	10.00	H-G	0/670	-18.5	-18.5	0.25 (1)	10.00	G-F	0/670	-18.5	-18.5	0.53 (1)	10.00	F-E	0/0	-18.5	-18.5	0.43 (1)	10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 40.1 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: -PART 9 OF BCBC 2015, ABC 2018 -PART 9 OF OBC 2012 (2019 AMENDMENT) -CSA 086-14 -TPIC 2014 (5% OF 33.4 P.S.F., G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.7 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.25") CALCULATED VERT. DEFL.(LL) = L/999 (0.04") ALLOWABLE DEFL.(TL)= L/360 (0.25") CALCULATED VERT. DEFL.(TL) = L/999 (0.08") CSI: TC=0.14/1.00 (C-F:1), BC=0.53/1.00 (F-G:1), WB=0.15/1.00 (C-F:1), SS=0.23/1.00 (E-F:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MT20 MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.52 (F) (INPUT = 0.80) JSI METAL= 0.29 (F) (INPUT = 1.00)	
MEMB.	FR-TO	CHORDS				WEBS																																																									
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LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2																																																															

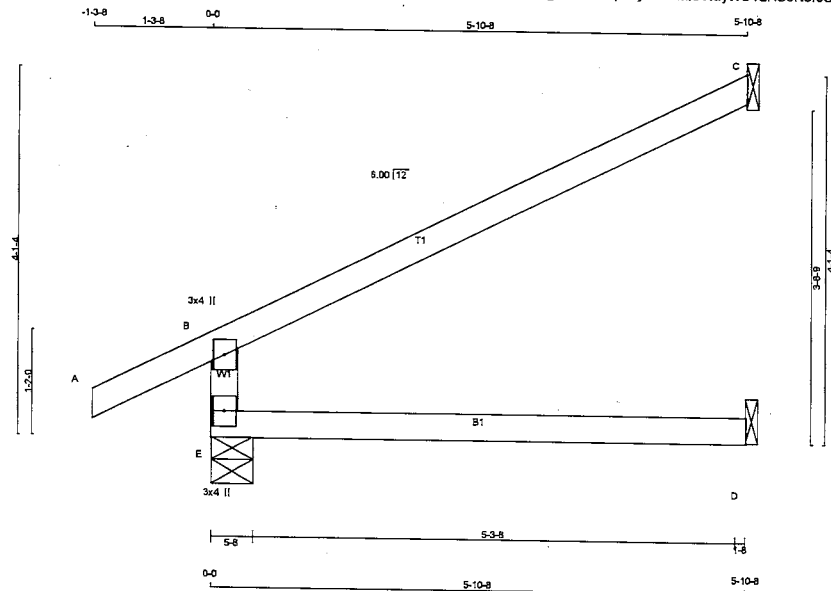


[illegible]

JOB NAME 428250	TRUSS NAME J1	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085161
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Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:06:53 2022 Page 1
ID:4IRTKRO8ssghv2KQFYp5dytbbD-MICWayWOYzHU0NeroUMaKJKv7HTifoOqBusz7ypD6W



Scale = 1/16"

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
E	542	0	542	0
C	210	0	210	0
D	45	0	50	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	VERT.	LC1	MAX	MIN	FR-TO		
E	380	269 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 / 0
C	144	118 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	0 / 0
D	38	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MIN	FR-TO	WEBS	MAX. FACTORED
MEMB.	FORCE	(PLF)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE
	(LBS)								(LBS)
E-B	-479 / 0	0.0	0.0	0.13 (4)	7.81				
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00				
B-C	-31 / 0	-95.2	-95.2	0.56 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSL TC=0.56/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

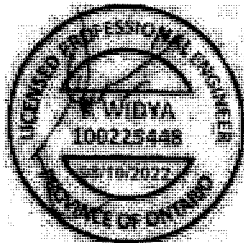
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 785 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MINIMUM NUMBER OF TOE-NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

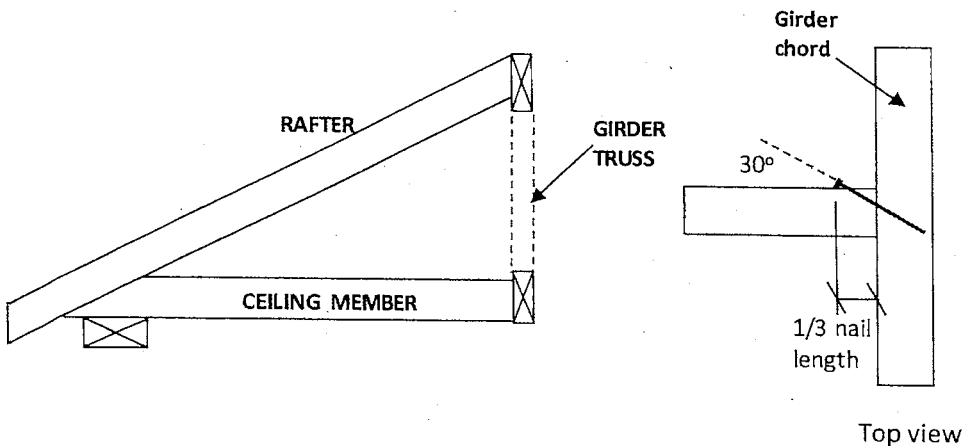


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

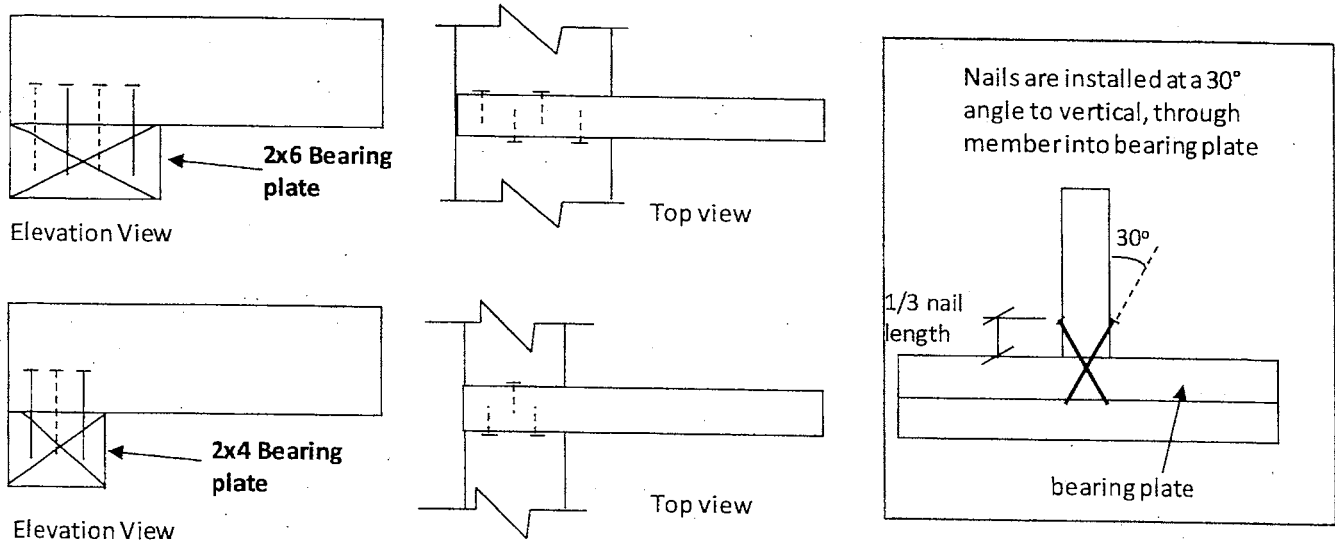
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

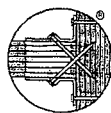
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

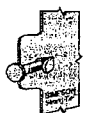
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

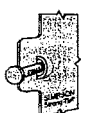
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

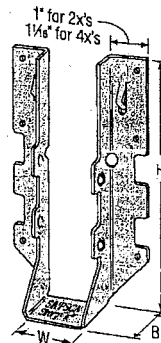
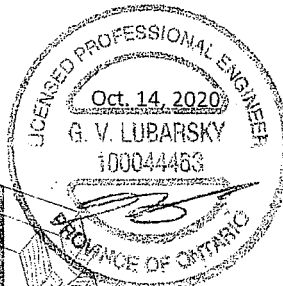
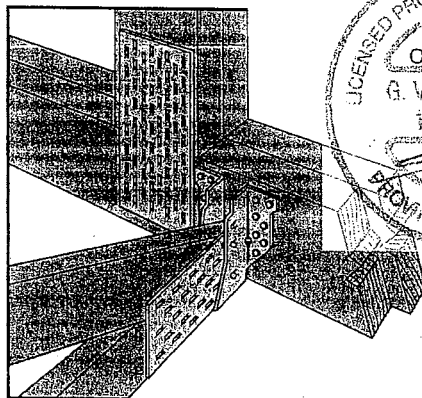
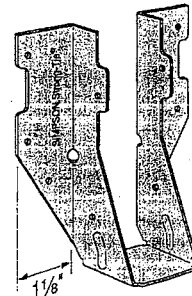
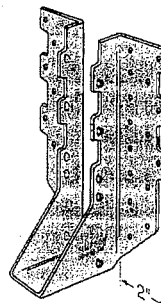
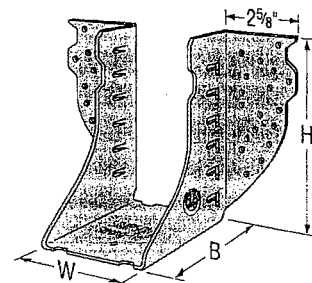
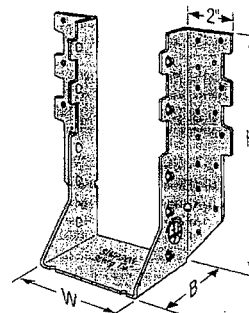
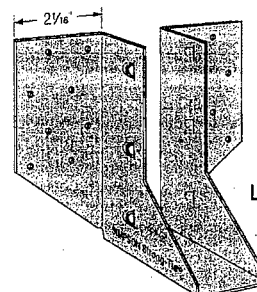


Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View (available on some models)

Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)

**LUS28****LU26L****HUS210**
(HUS26, HUS28, and HHUS similar)**HGUS28-2****HHUS210-2****LJS26DS**

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

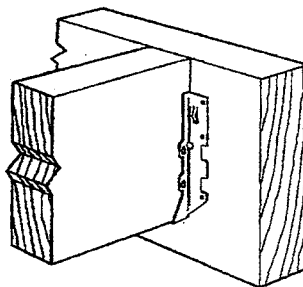
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

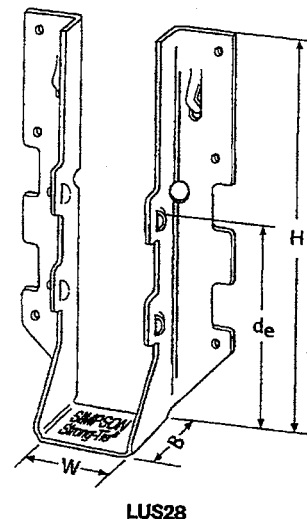
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

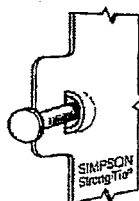


Typical LUS
Installation



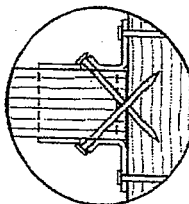
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 3/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/8	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

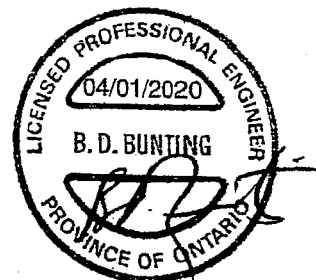


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

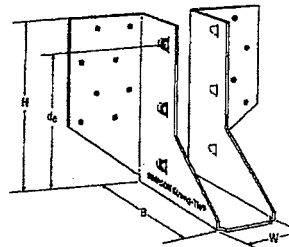
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

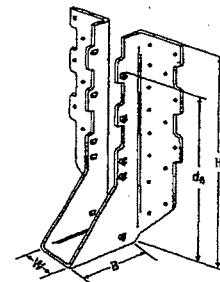
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

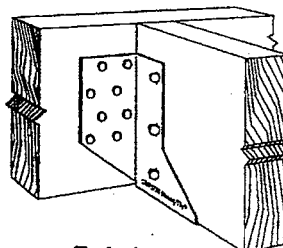
- See current catalogue for options



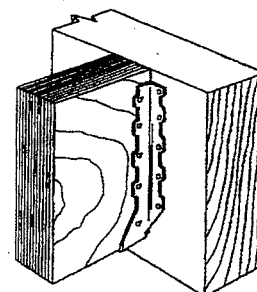
LJS26DS



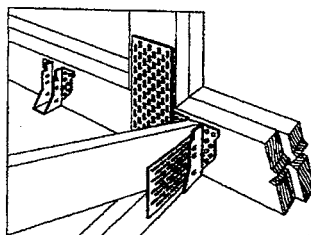
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



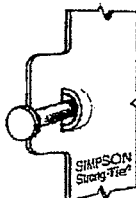
Typical HUS
Installation



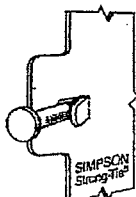
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

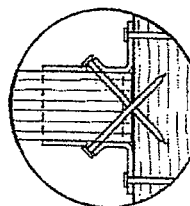
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

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T-SPECBUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

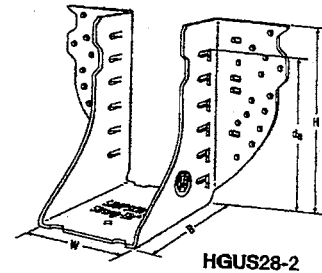
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

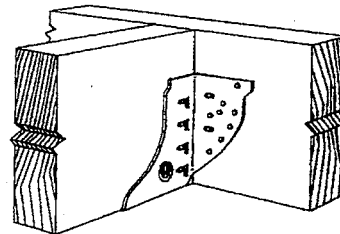
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

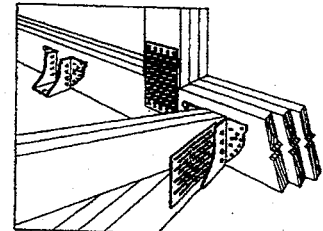
- See current catalogue for options



HGUS28-2



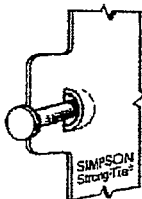
Typical HGUS
Installation



Typical HGUS Installation
(Truss Designer to
provide fastener quantity
for connecting multiple
members together)

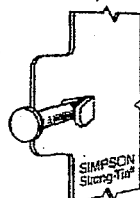
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K ₀ =1.15)	(K ₀ =1.00)	(K ₀ =1.15)	(K ₀ =1.00)
HGUS26	12	1%	5%	5	4 ½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 ¼	5 ½	4	4 ½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 ¼	5 ½	4	4 ½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 ¼	5 ½	4	4 ½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1%	7 ½	5	6 ½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 ¼	7 ½	4	6 ½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 ¼	7 ½	4	6 ½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 ¼	7 ½	4	6 ½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1%	9 ½	5	7 ½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 ¼	9 ½	4	8 ½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 ¼	9 ½	4	8 ½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 ¼	9 ½	4	8 ½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 ¼	10 ½	4	10 ½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 ¼	12 ½	4	11 ½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₀ is the distance from the seat of the hanger to the highest joist nail.

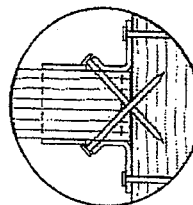


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

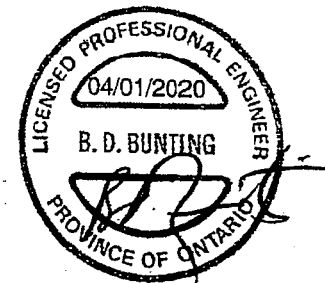
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

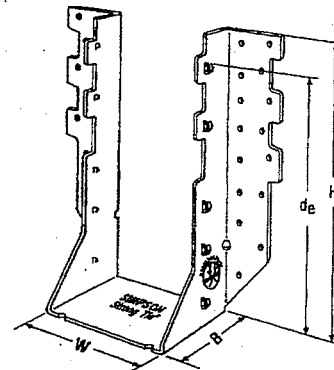
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

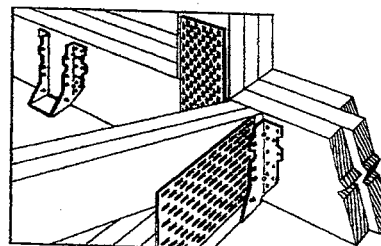
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



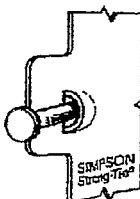
HHUS410



Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

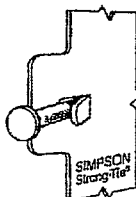
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HHUS26-2	14	3⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3⅞	9⅞	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4⅞	9	3	7⅞	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6⅞	8⅞	3	7⅞	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3⅞	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5⅞	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7⅞	9	3⅞	7⅞	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

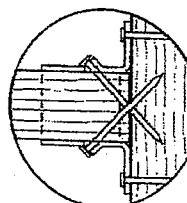


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

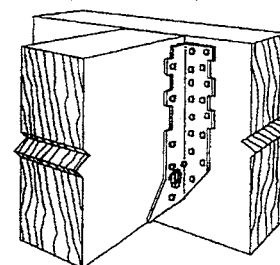
U.S. Patent 5,603,580



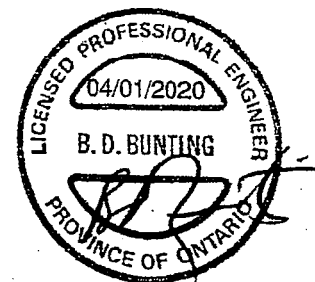
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



LIMIT STATES DESIGN

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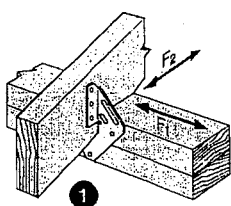
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T-SPECHHUS20 3/20 exp. 6/22

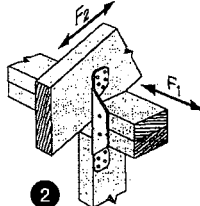
(800) 999-5099
strongtie.com

H/TSP

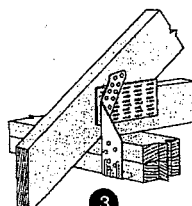
Seismic and Hurricane Ties (cont.)



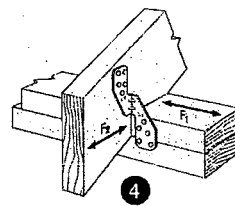
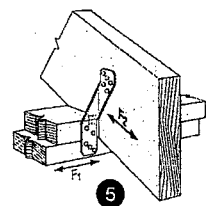
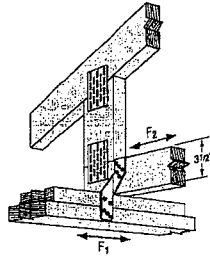
1 H1 Installation



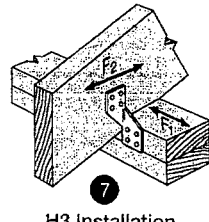
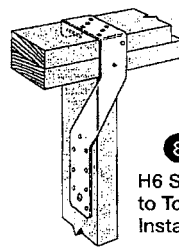
2 H2A Installation



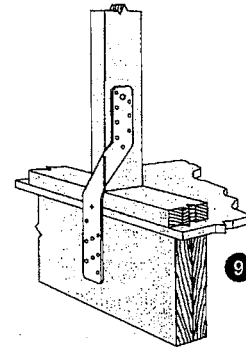
3 TSP Installation

4 H2.5A Installation
(Nails into both top plates)5 H2.5T Installation
(Nails into both top plates)

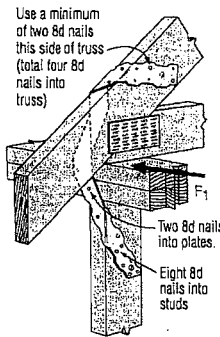
6 H2.5T Installation

7 H3 Installation
(Nails into upper top plate)

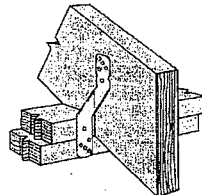
8 H6 Stud to Top Plate Installation



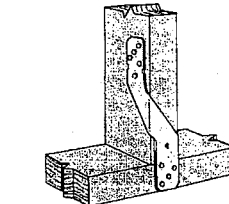
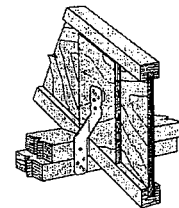
9 H6 Stud to Band Joist Installation



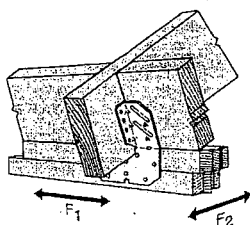
10 H7Z Installation



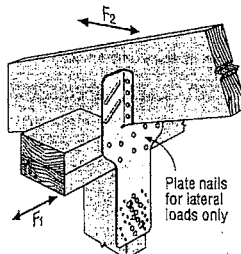
11 H8 Attaching Rafter to Double Top Plates

12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)

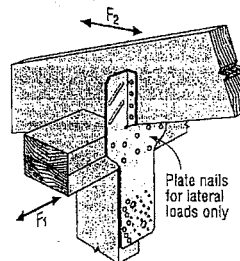
13 H8 attaching I-Joist to Double Top Plates



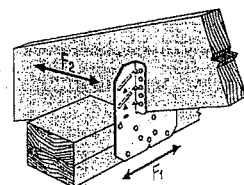
14 H10A Field-Bent Installation



15 H10S Installation

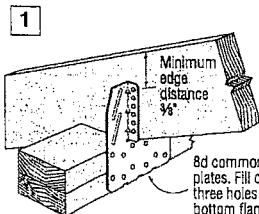


16 H10S Installation with Stud Offset

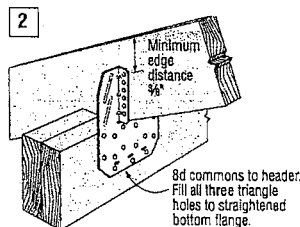


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

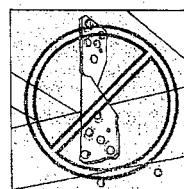


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie[®] hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

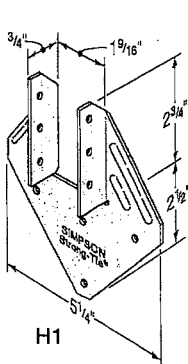
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

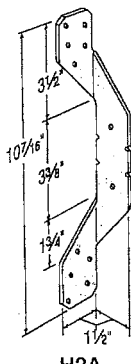
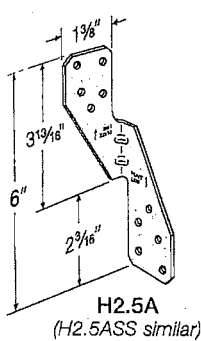
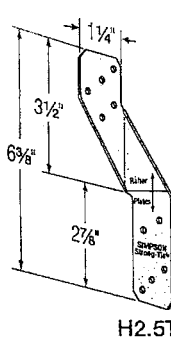
Finish: Galvanized. H7Z and H11Z — ZMAX[®] coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

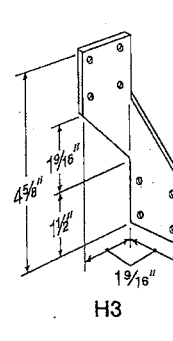
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



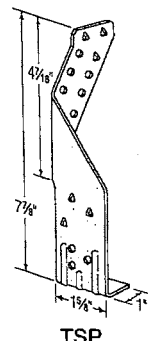
H1

H2A
(H2ASS similar)H2.5A
(H2.5ASS similar)

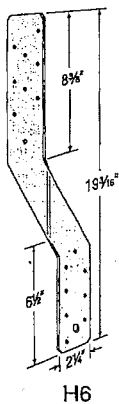
H2.5T



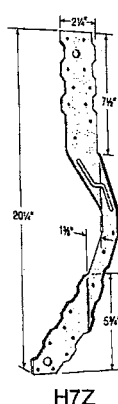
H3



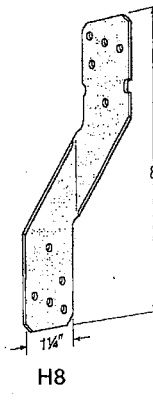
TSP



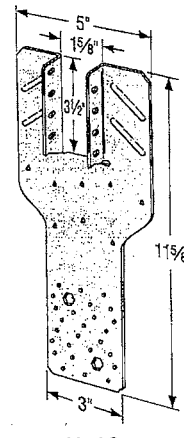
H6



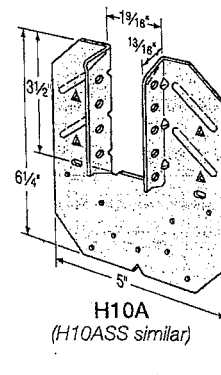
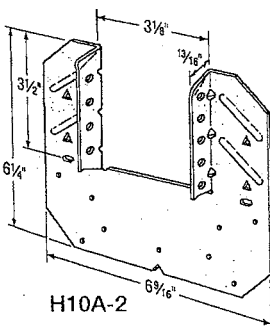
H7Z



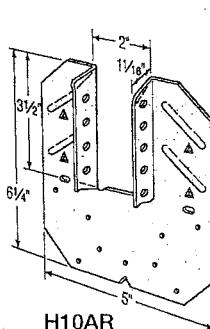
H8



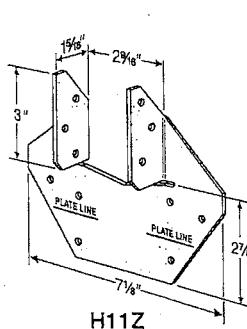
H10S

H10A
(H10ASS similar)

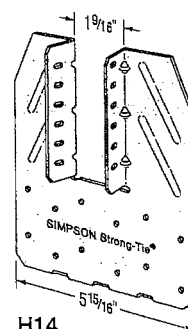
H10A-2



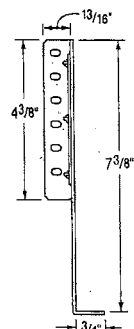
H10AR



H11Z



H14

H14
Profile

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14

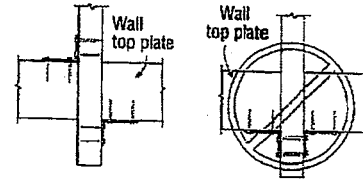
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

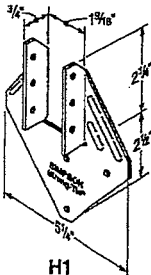
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

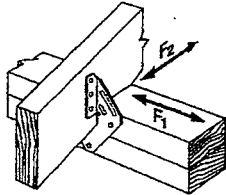


Install diagonally across from each other for minimum 2x truss.

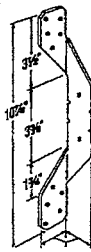
Nailing into both sides of a single ply 2x truss may cause the wood to split.



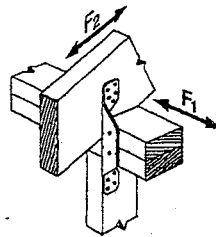
H1



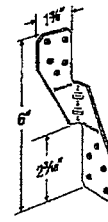
H1 Installation



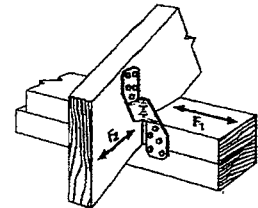
H2A



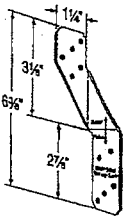
H2A Installation



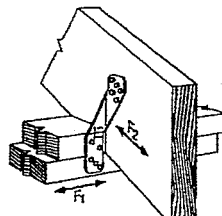
H2.5A



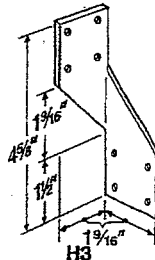
H2.5A Installation



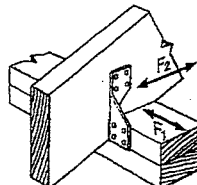
H2.5T



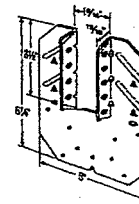
H2.5T Installation
(Nails into both top plates)



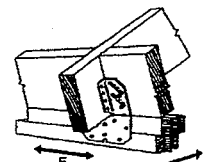
H3



H3 Installation



H10A



H10A Installation

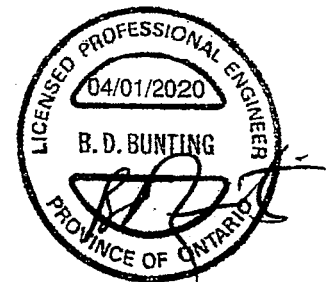
Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—						
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	740	685	300	680	485	215
H2.5A	18	(5) 8d	(5) 8d	—	830	220	75	590	155	55
H2.5T	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H3	18	(4) 8d	(4) 8d	—	835	175	240	740	160	210
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	740	180	265	615	125	190
					1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

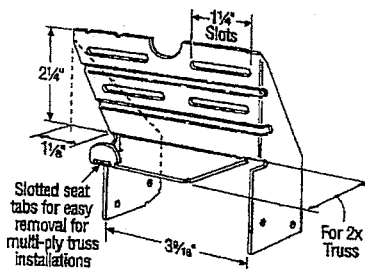
Design: Factored resistances are in accordance with CSA 086-14

Installation:

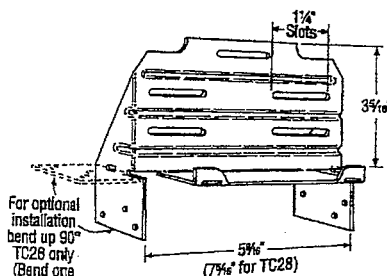
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire; 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

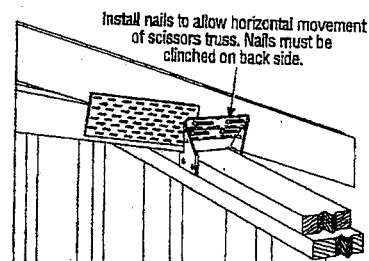
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



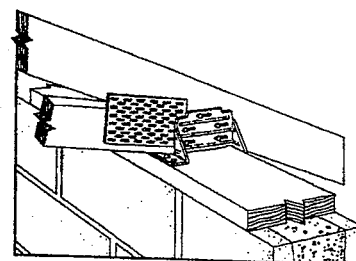
TC24
U.S. Patent 4,932,173



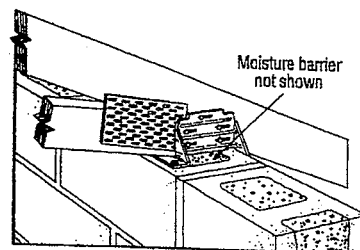
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



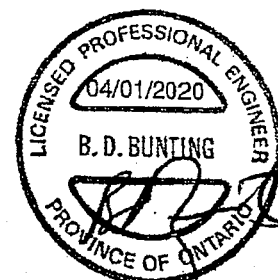
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 3/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HTU

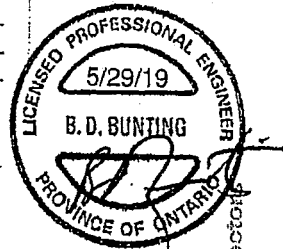
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (In.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3%	(2) 2x4	(10) 16d	(14) 10d x 1 1/2"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5 1/2"	(2) 2x4	(10) 16d	(20) 10d x 1 1/2"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3%	(2) 2x6	(20) 16d	(26) 10d x 1 1/2"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7 1/4"	(2) 2x6	(20) 16d	(32) 10d x 1 1/2"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 280.

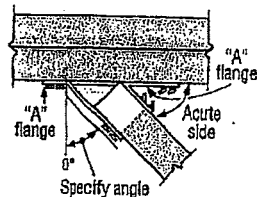


Hanger Options

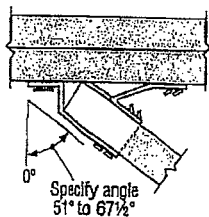
See Hanger Options Information on pp. 125-127.

Skewed Seat

- Skawable up to 67 1/2°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				(K _D =1.15)	(K _D =1.00)	(K _D =1.15)	(K _D =1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1 1/2"	1935	4110	1300	2905
	51-67 1/2	(20) 16d	(12) 10d x 1 1/2"	8.16	18.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1 1/2"	1350	3820	955	2560
	51-67 1/2	(26) 16d	(17) 10d x 1 1/2"	8.01	18.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1 1/2"	2810	4270	1985	3030
	51-67 1/2	(32) 16d	(22) 10d x 1 1/2"	12.50	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67 1/2	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67 1/2	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67 1/2	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67 1/2	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1610	3920	1140	2785
	51-67 1/2	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3980	5425	2815	3855
	51-67 1/2	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1895	3855
	51-67 1/2	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4890
	51-67 1/2	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6580	2225	4745
	51-67 1/2	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

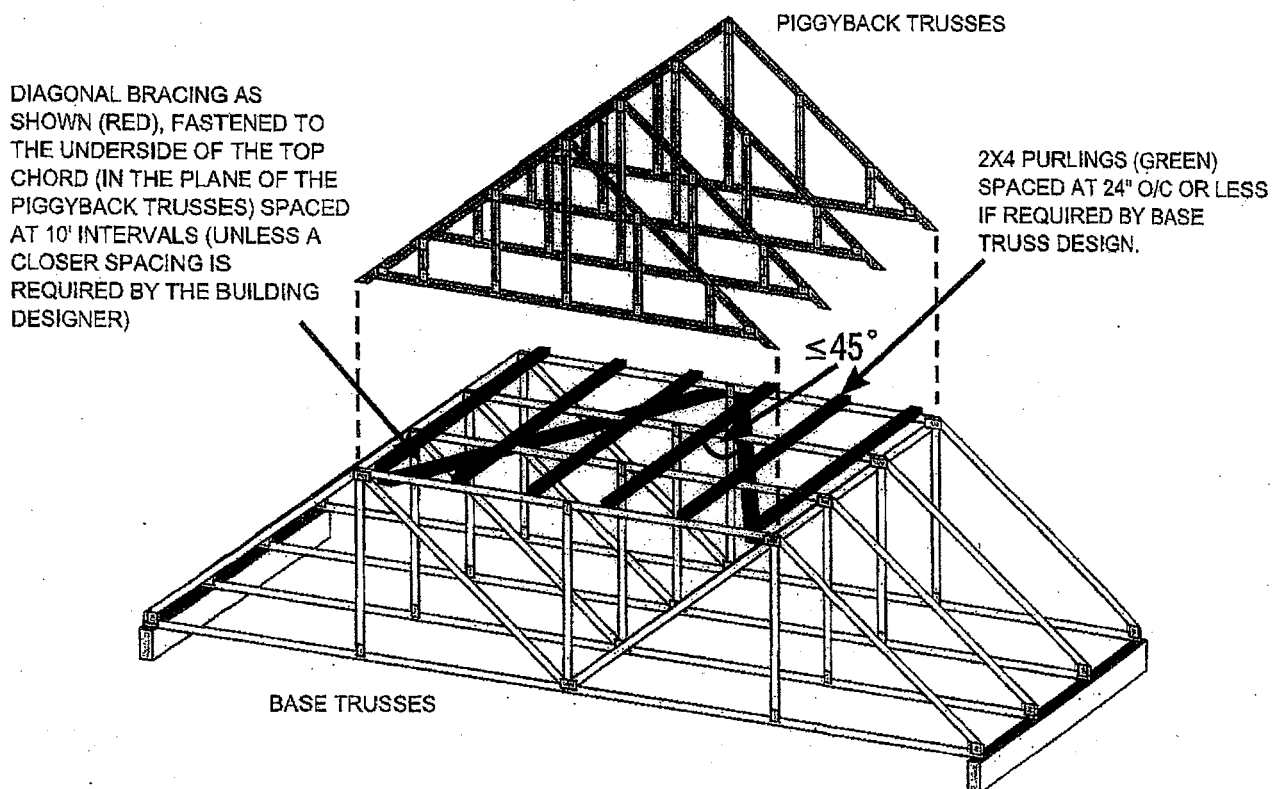
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3 1/4" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the fiat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

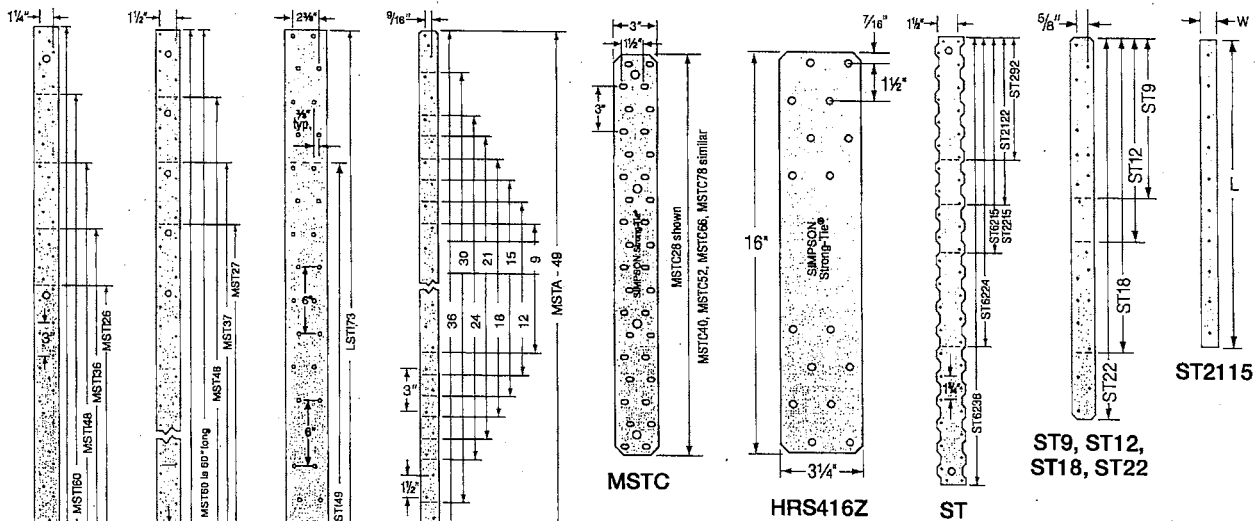
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

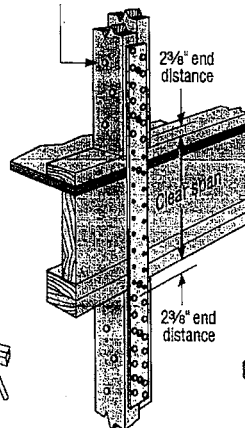
Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

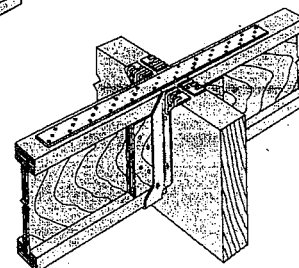


LSTA and MSTA
(Pilot holes not shown)

Stitch nailing
of double studs
by others



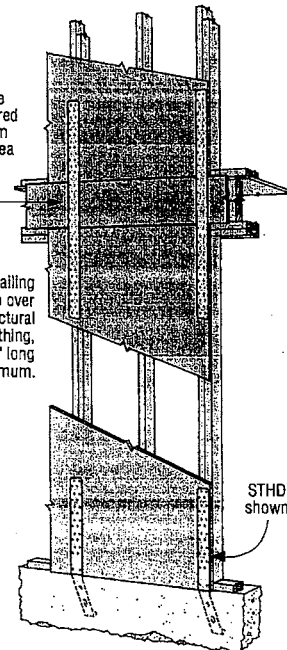
Floor-to-Floor Tie
Installation
Showing a
Clear Span



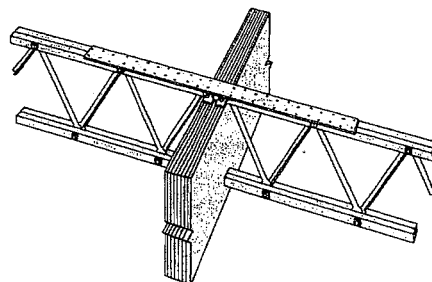
Typical MSTI Installation
(MIT hanger shown)
LSTI similar

Nails are
not required
in the rim
board area

When nailing
the strap over
wood structural
panel sheathing,
use 2 1/2\"/>



Typical Detail with
Strap Installed over
Wood Structural Panel
Sheathing



Typical LSTI Installation

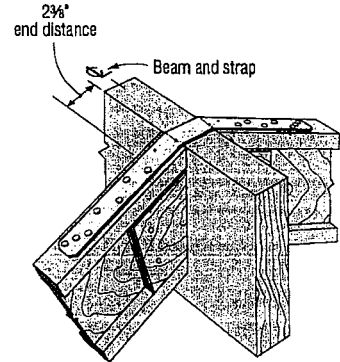
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI**Strap Ties (cont.)**

These products are available with additional corrosion protection. For more information, see p. 20.

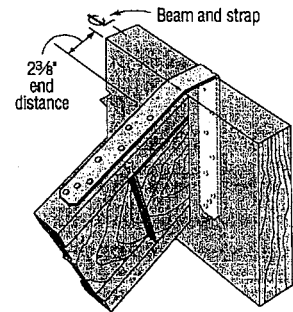


Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
LSTA9	20	1¼	9	(6) 10d	600	690	555	635
LSTA12		1¼	12	(8) 10d	800	920	735	845
LSTA15		1¼	15	(10) 10d	1000	1150	920	1060
LSTA18		1¼	18	(12) 10d	1200	1380	1105	1270
LSTA21		1¼	21	(14) 10d	1400	1610	1290	1485
LSTA24		1¼	24	(16) 10d	1600	1840	1475	1695
ST292		2½ ₈	9¾ ₈	(8) 8d	585	675	535	615
ST2122		2½ ₈	12¼ ₈	(12) 8d	940	1085	865	995
ST2115		¾	16¾ ₈	(8) 8d	670	770	615	710
ST2215		2½ ₈	16¾ ₈	(16) 8d	1335	1540	1235	1420
LSTA30	18	1¼	30	(20) 10d	2235	2465	2075	2385
LSTA36		1¼	36	(24) 10d	2465	2465	2465	2465
LSTI49		3¾	49	(32) 10d x 1½"	3115	3580	2852	3280
LSTI73		3¾	73	(48) 10d x 1½"	4670	5370	4280	4920
MSTA9		1¼	9	(6) 10d	670	770	625	715
MSTA12		1¼	12	(8) 10d	895	1030	830	955
MSTA15		1¼	15	(10) 10d	1120	1285	1040	1195
MSTA18		1¼	18	(12) 10d	1340	1545	1245	1430
MSTA21		1¼	21	(14) 10d	1565	1800	1455	1670
MSTA24		1¼	24	(16) 10d	1790	2060	1660	1910
MSTA30	16	1¼	30	(20) 10d	2470	2840	2260	2595
MSTA36		1¼	36	(24) 10d	2965	3070	2710	3070
MSTA49		1¼	49	(28) 8d	2725	2725	2545	2725
ST6215		2½ ₈	16¾ ₈	(16) 8d	1405	1615	1300	1500
ST6224		2½ ₈	23¾ ₈	(24) 8d	2305	2650	2155	2475
ST9		1¼	9	(6) 8d	525	605	490	560
ST12		1¼	11¾	(8) 8d	700	805	650	750
ST18		1¼	17¾	(12) 8d	1050	1210	975	1125
ST22		1¼	21¾	(18) 8d	1580	1790	1465	1685
					7.03	7.96	6.52	7.50



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Use half of the nails in each member being connected to achieve the listed resistances.
- Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

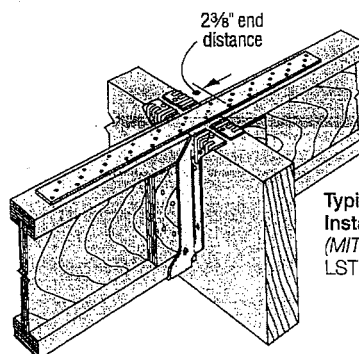
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

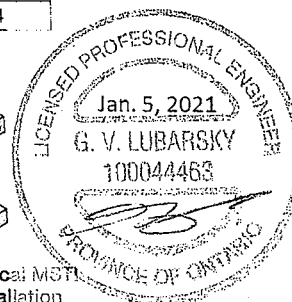
SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52		3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2½	33½	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2½	26	(22) 10d x 1 ½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2½	36	(32) 10d x 1 ½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2½	48	(44) 10d x 1 ½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2½	60	(56) 10d x 1 ½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2½	72	(68) 10d x 1 ½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2½	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2½	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2½	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z		3¼	16	(16) ¼" x 1 ½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60	10	2½	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2½	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

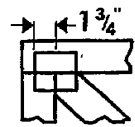


Typical MSTI
Installation
(MIT hanger shown)
LSTI similar

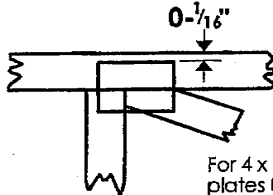


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

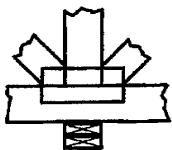
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I, or Eliminator bracing if indicated.

BEARING

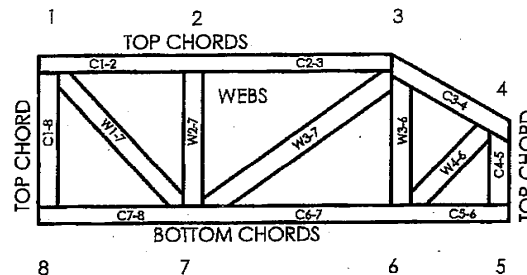


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.