

TH-31
CHERRY 3E
EL-1

428364

TH-32
CHERRY 2
EL-1

428365

TH-33
CHERRY 2
EL-1

428523

TH-34
CHERRY 1
EL-1

428367

FIRE WALL

TH-35
CHERRY 1
EL-1

428368

TH-36
CHERRY 12
EL-1

428370

All conventional framing to conform with
Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses
to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross
point. Vertical posts longer than 6' to have
lateral bracing so that the distance between
the post end points and lateral bracing does
not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 33.4 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 26.7 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS28-3 (SSS)
HGUS26- (X)
LUS26-2 (VV)
H2.5A - (J)
HTS16 - (T)
H8 - (H)

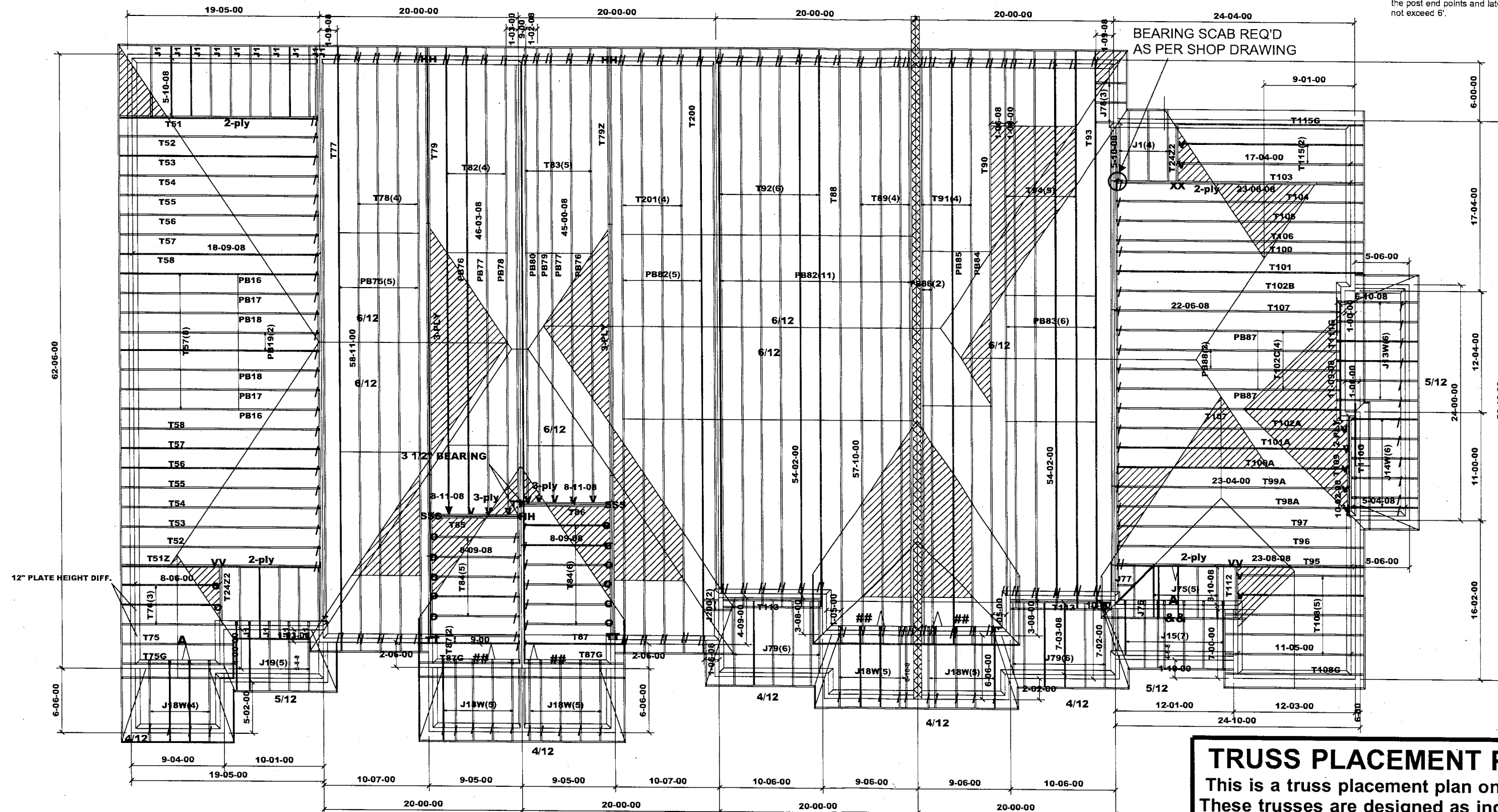
DENOTES:
CONVENTIONAL
FRAMING



TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



9/12 ROOF PITCH UNLESS NOTED OTHERWISE

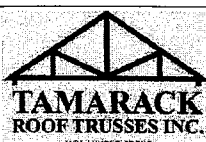
A- 12" RAISED PLATE AND CEILING

& 1-8-0 RAISED FASCIA

1-8-0 RAISE PLATE AND CEILING

E22085734 - E22085821

NOT THE GRANTING OF A PERMIT NOR REVIEWING OF SPECS
& DRAWINGS NOR INSPECTIONS MADE DURING INSTALLATION
BY THE OFFICIAL HAVING JURISDICTION SHALL RELIEVE THE
OWNER FROM REQUIREMENTS OF THE ONTARIO BUILDING
CODE AND ANY OTHER REFERENCED REQUIREMENTS.

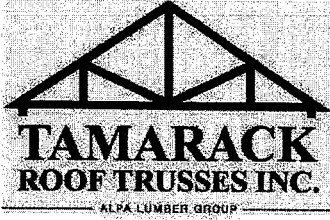


Job Track: 52956
Layout ID: 428511
Plan Log: 206546

Builder / Location: GREEN PARK HOMES / CAMBRIDGE
Project: BARLASSINA
Date: 2022-08-12
Designer: YPG

Model / Elevation: BLOCK 119 (CHERRY) / TH-31 ~ TH-36
Mitek ver 8.5.3.233
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THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED
FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



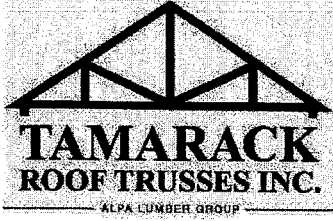
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-31)**
 Elevation: TH-7 (CHERRY 3E,EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428364
 Ref #
 Page: 1 of 2
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T24Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T51 Half Hip Girder	9 /12	18-09-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	177.93 112.00		
	1 2-ply	T51Z Half Hip Girder	9 /12	18-09-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	177.93 112.00		
	2	T52 Half Hip	9 /12	18-09-08	5-01-04	2 x 4	1-03-08	1-06-04 5-01-04	153.85 98.33		
	2	T53 Half Hip	9 /12	18-09-08	6-01-04	2 x 4	1-03-08	1-06-04 6-01-04	166.98 106.67		
	2	T54 Half Hip	9 /12	18-09-08	7-01-04	2 x 4	1-03-08	1-06-04 7-01-04	176.84 113.33		
	2	T55 Half Hip	9 /12	18-09-08	8-01-04	2 x 4	1-03-08	1-06-04 8-01-04	191.55 121.67		
	2	T56 Half Hip	9 /12	18-09-08	9-01-04	2 x 4	1-03-08	1-06-04 9-01-04	202.89 127.00		
	10	T57 Half Hip	9 /12	18-09-08	10-01-04	2 x 4	1-03-08	1-06-04 10-01-04	1062.03 670.00		
	2	T58 Half Hip	9 /12	18-09-08	11-01-04	2 x 4	1-03-08	1-06-04 11-01-04	208.02 128.67		
	1	T75 Common	9 /12	8-06-00	4-06-04	2 x 4	1-03-08 1-03-08	6-04 6-04	29.36 20.00		
	1	T75G GABLE	9 /12	8-06-00	4-06-04	2 x 4	1-03-08 1-03-08	6-04 6-04	31.13 20.67		
	3	T76 Common	9 /12	8-06-00	4-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	114.84 77.50		
	2	PB16 Piggyback	9 /12	8-02-13	2-08-00	2 x 4		2-08-00	53.17 33.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-31)**
 Elevation: TH-7 (CHERRY 3E, EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428364
 Ref #
 Page: 2 of 2
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB17 Piggyback	9 /12	8-02-13	3-08-00	2 x 4		3-08-00	57.01 37.33		
	2	PB18 Piggyback	9 /12	8-02-13	4-08-00	2 x 4		4-08-00	61.89 39.67		
	2	PB19 Piggyback	9 /12	8-02-13	5-08-00	2 x 6 2 x 4		5-08-00	74.76 46.67		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	4	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	78.76 48.67		
	5	J19 Jack-Open	5 /12	5-08-08	3-02-13	2 x 4	1-03-08	4-01 2-08-09	76.47 46.67		

TOTAL # TRUSS= 65

TOTAL BFT OF ALL TRUSSES= 2157.52

BFT.

TOTAL WEIGHT OF ALL TRSSES 3404.98 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
32	Hardware	H2.5A	
2	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 35

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-32)**
 Elevation: TH-8 (CHERRY 2,EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428365
 Ref #
 Page: 1 of 2
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T77 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	388.95 236.33		
	4	T78 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1598.57 970.67		
	1 3-ply	T79 Hip Girder	6 /12	58-11-00	9-05-14	2 x 6 2 x 8	1-03-08 1-03-08	1-02-00 1-02-00	1371.41 812.00		
	4	T82 Hip	6 /12	46-03-08	10-11-14	2 x 6	1-03-08	1-02-00 7-05-12	1311.23 782.67		
	5	T84 Jack-Closed	9 /12	8-09-08	9-09-06	2 x 4		3-02-04 9-09-06	254.62 160.83		
	1 3-ply	T85 Flat Girder	0 /12	8-11-08	7-03-08	2 x 6		7-03-08 7-03-08	246.32 154.50		
	2	T87 Monopitch	9 /12	8-09-08	8-01-06	2 x 4	1-03-08	1-06-04 8-01-06	91.92 58.67		
	1	T87G GABLE	9 /12	8-09-08	8-01-06	2 x 4	1-03-08	1-06-04 8-01-06	47.61 30.83		
	5	PB75 Piggyback	6 /12	22-04-00	5-07-00	2 x 4			329.86 201.67		
	1	PB76 Piggyback	6 /12	19-07-08	1-06-00	2 x 4			55.92 35.17		
	1	PB77 Piggyback	6 /12	19-07-08	3-00-00	2 x 4			56.54 36.17		
	1	PB78 Piggyback	6 /12	19-07-08	4-06-00	2 x 4			64.27 41.00		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	98.45 60.83		

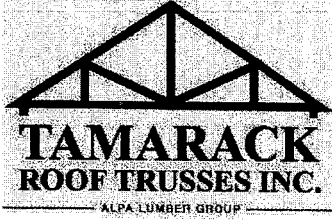
TOTAL # TRUSS= 36

TOTAL BFT OF ALL TRUSSES= 3581.34

BFT.

TOTAL WEIGHT OF ALL TRSSES 5915.69 LBS

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-32)**
 Elevation: TH-8 (CHERRY 2,EL-1)

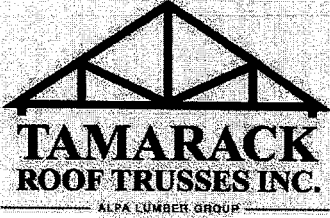
Job Track: 52956
 PlanLog: 206513
 Layout ID: 428365
 Ref #
 Page: 2 of 2
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
40	Hardware	H2.5A	
4	Hardware	H8	
1	Hardware	HGUS28-3	
2		HTS16	
4	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **56**

DELIVERY SHIPLIST



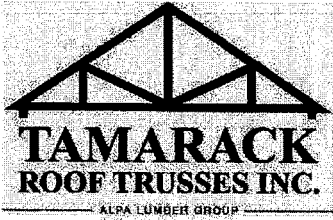
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 (TH-31~36)
 Lot #:
 Elevation: TH-33 (CHERRY 2,EL-1)

Job Track: 52956
 PlanLog: 206546
 Layout ID: 428523
 Ref #
 Page: 1 of 2
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 3-ply	T79Z Hip Girder	6 /12	58-11-00	9-07-12	2 x 8	1-03-08 1-03-08	1-02-00 1-02-00	1432 854.00		
	5	T83 Hip	6 /12	45-00-08	10-11-14	2 x 6	1-03-08	1-02-00 8-01-04	1627.02 975.00		
	6	T84 Jack-Closed	9 /12	8-09-08	9-09-06	2 x 4		3-02-04 9-09-06	305.55 193.00		
	1 3-ply	T86 Flat Girder	0 /12	8-11-08	7-11-00	2 x 6 2 x 8		7-11-00 7-11-00	272.41 169.00		
	1	T87 Monopitch	9 /12	8-09-08	8-01-06	2 x 4	1-03-08	1-06-04 8-01-06	45.96 29.33		
	1	T87G GABLE	9 /12	8-09-08	8-01-06	2 x 4	1-03-08	1-06-04 8-01-06	47.61 30.83		
	1	T200 Piggyback Base Girder	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	400.77 244.00		
	4	T201 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1610.53 969.33		
	1	PB76 Piggyback	6 /12	19-07-08	1-06-00	2 x 4			55.92 35.17		
	1	PB77 Piggyback	6 /12	19-07-08	3-00-00	2 x 4			56.54 36.17		
	1	PB79 Piggyback	6 /12	19-07-08	4-00-12	2 x 4			62.56 40.50		
	1	PB80 Piggyback	6 /12	19-07-08	4-10-14	2 x 4			57.44 35.67		
	5	PB82 Piggyback	6 /12	18-10-00	4-08-08	2 x 4			274.76 170.00		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	98.45 60.83		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 (TH-31~36)
 Lot #:
 Elevation: TH-33 (CHERRY 2,EL-1)

Job Track: 52956
 PlanLog: 206546
 Layout ID: 428523
 Ref #
 Page: 2 of 2
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J200 Jack-Open	9 /12	1-06-08	2-08-02	2 x 4	1-03-08	1-06-04 2-08-02	16.76 12.67		

TOTAL # TRUSS= 40

TOTAL BFT OF ALL TRUSSES= 3855.5

BFT.

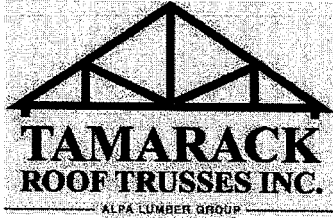
TOTAL WEIGHT OF ALL TRSSES 6364.29 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
6	Hardware	LUS24	
42	Hardware	H2.5A	
2	Hardware	H8	
1	Hardware	HGUS28-3	
4		HTS16	

TOTAL NUMBER OF ITEMS= 60

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-34)**
 Elevation: TH-10 (CHERRY 1,EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428367
 Ref #
 Page: 1 of 1
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T88 Piggyback Base	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	400.75 245.00		
	4	T89 Piggyback Base	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1639.64 988.00		
	6	T92 Piggyback Base	6 /12	54-02-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	2192.17 1312.00		
	1	T113 Half Hip Girder	12 /12	10-00-00	2-09-02	2 x 4 2 x 6		1-08-12 2-09-02	50.76 33.00		
	11	PB82 Piggyback	6 /12	18-10-00	4-08-08	2 x 4			604.47 374.00		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	98.45 60.83		
	6	J79 Jack-Open	4 /12	7-03-08	3-02-00	2 x 4	1-03-08	3-15 2-09-02	129.22 84.00		

TOTAL # TRUSS= 34

TOTAL BFT OF ALL TRUSSES= 3096.83

BFT.

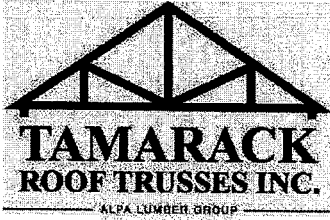
TOTAL WEIGHT OF ALL TRSSES 5115.47 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
50	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 50

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-35)**
 Elevation: TH-11 (CHERRY 1,EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428368
 Ref #
 Page: 1 of 1
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T90 Roof Special	6 /12	57-10-00	10-09-04	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	404.53 244.33		
	4	T91 Piggyback Base	6 /12	57-10-00	10-11-02	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1613.12 961.33		
	1	T93 Piggyback Base Girder	6 /12	54-02-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	367.04 222.67		
	5	T94 Piggyback Base	6 /12	54-02-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1805.23 1096.67		
	1	T113 Half Hip Girder	12 /12	10-00-00	2-09-02	2 x 4 2 x 6		1-08-12 2-09-02	50.76 33.00		
	6	PB83 Piggyback	6 /12	12-10-00	3-02-08	2 x 4			184.59 117.00		
	1	PB84 Piggyback	6 /12	15-09-00	1-06-00	2 x 4			43.96 27.83		
	1	PB85 Piggyback	6 /12	15-09-00	3-00-00	2 x 4			42.33 26.83		
	2	PB86 Piggyback	6 /12	15-09-00	3-11-04	2 x 4			76.81 46.67		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	98.45 60.83		
	3	J78 Jack-Open	9 /12	1-09-08	2-10-06	2 x 4	1-03-08	1-06-04 2-10-06	27.43 21.00		
	6	J79 Jack-Open	4 /12	7-03-08	3-02-00	2 x 4	1-03-08	3-15 2-09-02	129.22 84.00		

TOTAL # TRUSS= 36

TOTAL BFT OF ALL TRUSSES= 2942.16

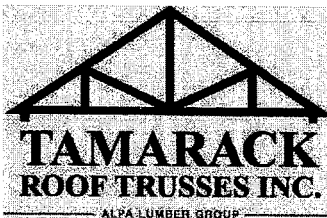
BFT.

TOTAL WEIGHT OF ALL TRSSES 4843.48 LBS

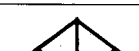
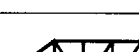
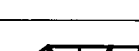
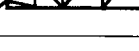
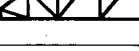
HARDWARE

QTY	TYPE	MODEL	LENGTH
50	Hardware	H2.5A	

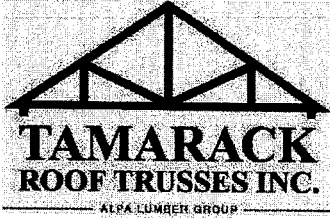
TOTAL NUMBER OF ITEMS= 50

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: BLOCK 121 (CHERRY) Lot #: <i>Block 119 (TH-36)</i> Elevation: TH-12 (CHERRY 12,EL-1)	Job Track: 52956 PlanLog: 206513 Layout ID: 428370 Ref # Page: 1 of 3 Date: 08-16-2022 Designer: Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T24Z2 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T95 Hip Girder	9 /12	23-08-08	6-01-02	2 x 4 2 x 6	1-03-08	1-06-04 2-04-02	248.85 157.33		
	1	T96 Hip	9 /12	23-08-08	7-07-02	2 x 4	1-03-08	1-06-04 3-04-02	111.74 71.33		
	1	T97 Hip	9 /12	23-08-08	9-01-02	2 x 4		1-06-04 3-04-02	115.57 71.50		
	1	T98A Hip	9 /12	23-04-00	10-07-02	2 x 4		1-09-10 3-04-02	129.77 81.83		
	1	T99A Common	9 /12	23-04-00	11-03-14	2 x 4		1-09-10 3-04-02	110.24 69.33		
	1	T100 Half Hip	9 /12	23-08-08	7-08-04	2 x 4	1-03-08	1-06-04 7-08-04	117.41 75.00		
	1	T100A Half Hip	9 /12	23-04-00	7-08-04	2 x 4		1-09-10 7-08-04	114.72 73.67		
	1	T101 Half Hip	9 /12	23-08-08	8-08-04	2 x 4	1-03-08	1-06-04 8-08-04	119.37 74.83		
	1	T101A Half Hip	9 /12	23-04-00	8-08-04	2 x 4		1-09-10 8-08-04	116.66 73.50		
	1	T102A Half Hip	9 /12	23-04-00	9-08-04	2 x 4		1-09-10 9-08-04	122.56 76.67		
	1	T102B Half Hip	9 /12	22-08-08	9-08-04	2 x 4		2-03-04 9-08-04	121.14 74.83		
	4	T102C Piggyback Base	9 /12	22-06-08	9-08-04	2 x 4		2-04-12 9-08-04	483.08 299.33		
	1 2-ply	T103 Half Hip Girder	9 /12	23-08-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	225.23 139.67		

DELIVERY SHIPLIST



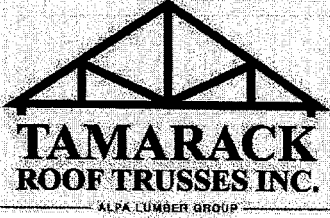
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: *Block 119 (TH-36)*
 Elevation: TH-12 (CHERRY 12,EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428370
 Ref #
 Page: 2 of 3
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T104 Half Hip	9 /12	23-08-08	5-01-04	2 x 4	1-03-08	1-06-04 5-01-04	97.08 60.67		
	1	T105 Half Hip	9 /12	23-08-08	6-01-04	2 x 4	1-03-08	1-06-04 6-01-04	105.08 66.00		
	1	T106 Half Hip	9 /12	23-08-08	7-01-04	2 x 4	1-03-08	1-06-04 7-01-04	111.49 71.33		
	2	T107 Half Hip	9 /12	22-06-08	10-08-04	2 x 4		2-04-12 10-08-04	267.33 167.33		
	5	T108 Common	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	244.83 158.33		
	1	T108G GABLE	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	52.6 34.67		
	1 2-ply	T109 Common Girder	9 /12	10-02-00	5-04-00	2 x 4 2 x 6		1-06-04 1-06-04	96.34 65.67		
	1	T110G GABLE	9 /12	10-02-00	5-04-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	46.81 32.50		
	1	T111G GABLE	9 /12	11-09-08	9-07-00	2 x 4		1-06-04 8-09-10	68.03 44.00		
	1 2-ply	T112 Jack-Closed Girder	9 /12	3-10-08	6-01-02	2 x 4 2 x 6	1-03-08	3-02-04 6-01-02	53.84 34.33		
	2	T115 Common	9 /12	17-04-00	8-00-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	161.32 102.00		
	1	T115G GABLE	9 /12	17-04-00	8-00-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	82.93 53.00		
	2	PB87 Piggyback	9 /12	12-09-13	2-00-00	2 x 4		2-00-00	81.48 54.00		
	2	PB88 Piggyback	9 /12	12-09-13	3-00-00	2 x 4		3-00-00	87.02 56.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 121 (CHERRY)
 Lot #: **Block 119 (TH-36)**
 Elevation: TH-12 (CHERRY 12,EL-1)

Job Track: 52956
 PlanLog: 206513
 Layout ID: 428370
 Ref #
 Page: 3 of 3
 Date: 08-16-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	6	J13W Jack-Open	5 /12	6-10-08	3-08-10	2 x 4	1-03-08	5-06 3-03-12	126.93 77.00		
	6	J14W Jack-Open	5 /12	5-04-08	3-01-02	2 x 4	1-03-08	5-06 2-08-04	91.81 60.00		
	7	J15 Jack-Open	5 /12	5-06-08	3-01-15	2 x 4	1-03-08	5-06 2-09-01	109.88 70.00		
	5	J75 Jack-Open	9 /12	3-10-08	5-01-02	2 x 4	1-03-08	2-02-04 5-01-02	80.09 50.83		
	1	J76 Jack-Open	9 /12	1-09-07	3-06-05	2 x 4	1-03-08 2-01-01	2-02-04 3-06-05	12.77 8.83		
	1	J77 Jack-Open	9 /12	1-08-00	3-06-05	2 x 4		2-04-02 3-07-02	8.35 6.17		

TOTAL # TRUSS= 74

TOTAL BFT OF ALL TRUSSES= 2691.15

BFT.

TOTAL WEIGHT OF ALL TRSSES 4247.16 LBS

HARDWARE

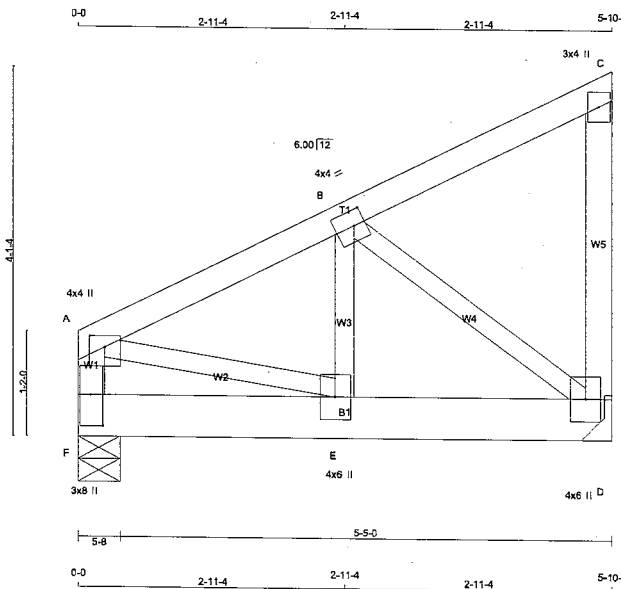
QTY	TYPE	MODEL	LENGTH
35	Hardware	H2.5A	
1	Hardware	HGUS26-2	
9	Hardware	LJS26DS	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 46

JOB NAME 428511	TRUSS NAME T24Z2	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085763
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Alpa Roof Truss, Maple

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Scale = 1:19.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1 12	TOP	
A - C 1 12	TOP	
C - D 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2 12	SIDE(217.9)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50 2.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TMV+p	MT20	3.0	4.0	
D	BMVW1+p	MT20	4.0	6.0	
E	BMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	8.0	4.25 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	VERT	DOWN	UP-LIFT	IN-SX
D	1614 0	1614 0	0	5-8 1-8
			MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
F	1140	759 / 0	0 / 0	0 / 0
D	1140	759 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO			LENGTH	FR-TO			
F-A	-1053 / 0	0.0	0.0	0.06 (1)	7.81	A-E	0 / 1271	0.16 (1)	
A-B	-1363 / 0	-95.2	-95.2	0.08 (1)	6.25	E-B	0 / 1223	0.15 (1)	
B-C	-11 / 0	-95.2	-95.2	0.06 (1)	6.25	B-D	-1546 / 0	0.18 (1)	
D-C	-115 / 0	0.0	0.0	0.01 (1)	7.81				
F-E	0 / 0	-454.4	-454.4	0.13 (1)	10.00				
E-D	0 / 1229	-454.4	-454.4	0.21 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.7 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CSNtGirder

START DISTANCE = 0-0

START SPAN CARRIED = 17-4-0

END DISTANCE = 5-10-8

END SPAN CARRIED = 17-4-0

END WALL WIDTH = 0-0

APPLIED TO FRONT SIDE OF BOTTOM CHORD.

-ADDT'L LOADS BASED ON 55 % OF GSL.

(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/399 (0.01")

ALLOWABLE DEFL.(TL) = L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/399 (0.01")

CSI: TC=0.06/1.00 (A-B:1), BC=0.21/1.00 (D-E:1).

WB=0.18/1.00 (B-D:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

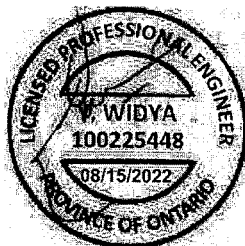
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (A) (INPUT = 0.90)

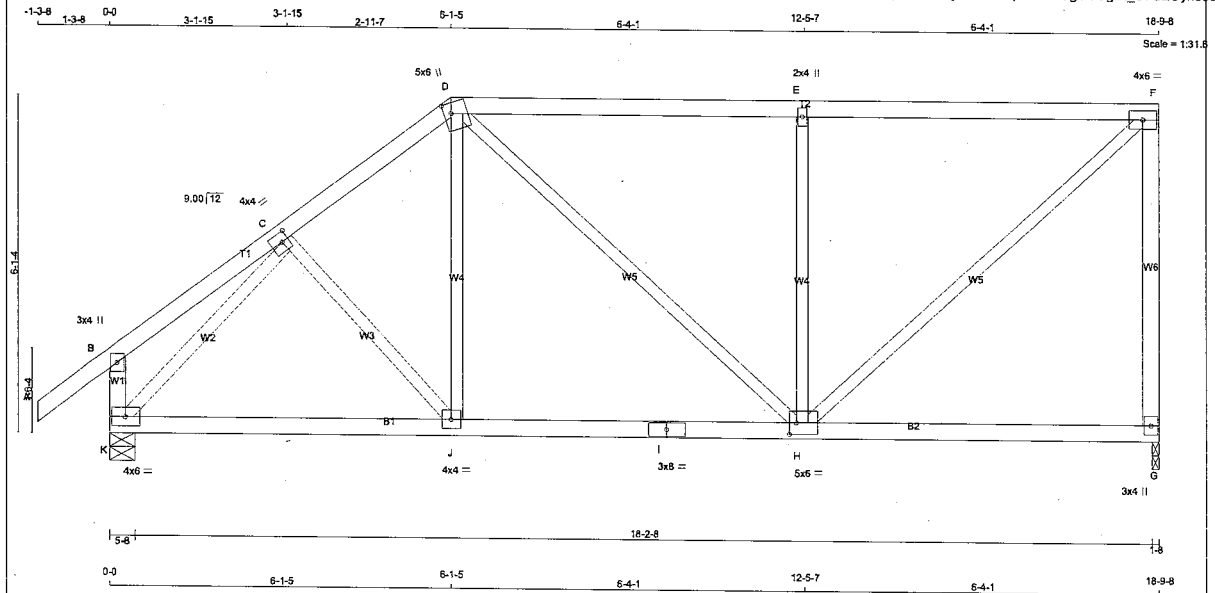
JSI METAL= 0.28 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



[illegible]

JOB NAME 428511	TRUSS NAME T52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085766																																																																																																							
Alpa Roof Truss, Maple																																																																																																												
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Aug 15 13:18:12 2022 Page 1 ID:T06XqL09rYKBxLLs9y_Z1eyrbTx-OLHBk15HSibmm8tcU6emJ4nyzH8PFIikWLhmyncc																																																																																																												
1-3-8 1-3-8 0-0 4-9-5 4-9-5 7-0-1 11-9-7 7-0-1 18-9-8 Scale = 1:31.6																																																																																																												
TOTAL WEIGHT = 2 X 77' = 154 lb																																																																																																												
LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF F - E 2x4 DRY No.2 SPF J - B 2x4 DRY No.2 SPF H - F 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQ'D JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX F 1068 0 1068 0 0 1-8 1-8 J 1200 0 1200 0 0 5-8 1-8 UNFACTORED REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL F 754 502 / 0 0 / 0 0 / 0 252 / 0 0 / 0 J 844 576 / 0 0 / 0 0 / 0 268 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) F, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>LC1 MAX. CSI (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. CSI (LC)</th><th></th></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 39</td><td>-95.2 -95.2</td><td>0.13 (1)</td><td>10.00</td><td>I-C</td><td>-81 / 81</td><td>0.03 (1)</td></tr><tr><td>B-C</td><td>-1013 / 0</td><td>-95.2 -95.2</td><td>0.41 (1)</td><td>5.65</td><td>C-G</td><td>0 / 372</td><td>0.08 (1)</td></tr><tr><td>C-D</td><td>-1113 / 0</td><td>-95.2 -95.2</td><td>0.89 (1)</td><td>4.26</td><td>G-D</td><td>-827 / 0</td><td>0.32 (1)</td></tr><tr><td>D-E</td><td>-1113 / 0</td><td>-95.2 -95.2</td><td>0.89 (1)</td><td>4.26</td><td>G-E</td><td>0 / 1348</td><td>0.30 (1)</td></tr><tr><td>E-F</td><td>-1015 / 0</td><td>0.0 0.0</td><td>0.45 (1)</td><td>7.77</td><td>B-I</td><td>0 / 833</td><td>0.19 (1)</td></tr><tr><td>J-B</td><td>-1165 / 0</td><td>0.0 0.0</td><td>0.12 (1)</td><td>7.39</td><td></td><td></td><td></td></tr><tr><td>J-I</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.13 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>I-H</td><td>0 / 807</td><td>-18.5 -18.5</td><td>0.29 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>H-G</td><td>0 / 807</td><td>-18.5 -18.5</td><td>0.29 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>G-F</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.22 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></table>				CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		FR-TO		FROM TO		FR-TO				A-B	0 / 39	-95.2 -95.2	0.13 (1)	10.00	I-C	-81 / 81	0.03 (1)	B-C	-1013 / 0	-95.2 -95.2	0.41 (1)	5.65	C-G	0 / 372	0.08 (1)	C-D	-1113 / 0	-95.2 -95.2	0.89 (1)	4.26	G-D	-827 / 0	0.32 (1)	D-E	-1113 / 0	-95.2 -95.2	0.89 (1)	4.26	G-E	0 / 1348	0.30 (1)	E-F	-1015 / 0	0.0 0.0	0.45 (1)	7.77	B-I	0 / 833	0.19 (1)	J-B	-1165 / 0	0.0 0.0	0.12 (1)	7.39				J-I	0 / 0	-18.5 -18.5	0.13 (4)	10.00				I-H	0 / 807	-18.5 -18.5	0.29 (4)	10.00				H-G	0 / 807	-18.5 -18.5	0.29 (4)	10.00				G-F	0 / 0	-18.5 -18.5	0.22 (4)	10.00			
CHORDS				WEBS																																																																																																								
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J-B	-1165 / 0	0.0 0.0	0.12 (1)	7.39																																																																																																								
J-I	0 / 0	-18.5 -18.5	0.13 (4)	10.00																																																																																																								
I-H	0 / 807	-18.5 -18.5	0.29 (4)	10.00																																																																																																								
H-G	0 / 807	-18.5 -18.5	0.29 (4)	10.00																																																																																																								
G-F	0 / 0	-18.5 -18.5	0.22 (4)	10.00																																																																																																								
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 4.1 PSF SPACING = 24.0 IN./C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFLECT (LL)= L/360 (0.63") CALCULATED VERT. DEFLECT (LL) = L/999 (0.04") ALLOWABLE DEFLECT (TL)= L/360 (0.63") CALCULATED VERT. DEFLECT (TL) = L/999 (0.10") CSI: TC=0.89/1.00 (D-E:1), BC=0.29/1.00 (G-I:4), WB=0.32/1.00 (D-G:1), SS=0.32/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.84 (B) (INPUT = 0.90) JSI METAL = 0.54 (B) (INPUT = 1.00)																																																																																																												
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2																																																																																																												

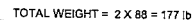


LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 83 = 167 lb	
N. L. G. A. RULES					BEARINGS										DESIGN CRITERIA	
CHORDS SIZE LUMBER DESCR.					FACTORED MAXIMUM FACTORED INPUT REQ'D										SPECIFIED LOADS:	
A - D 2x4 DRY No.2 SPF					GROSS REACTION GROSS REACTION BRG BRG										TOP CH. LL = 26.7 PSF	
D - F 2x4 DRY No.2 SPF					JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										DL = 6.0 PSF	
G - F 2x4 DRY No.2 SPF					K 1068 0 1068 0 0 1-8 1-8										BOT CH. LL = 0.0 PSF	
K - I 2x4 DRY No.2 SPF					G 1200 0 1200 0 0 5-8 1-8										DL = 7.4 PSF	
I - G 2x4 DRY No.2 SPF															TOTAL LOAD = 40.1 PSF	
ALL WEBS 2x3 DRY No.2 SPF					UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C	
EXCEPT					1ST CASE MAX./MIN. COMPONENT REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM	
DRY: SEASONED LUMBER.					JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL											
					G 754 502 / 0 0 / 0 0 / 0 0 / 0 252 / 0 0 / 0											
					K 844 575 / 0 0 / 0 0 / 0 0 / 0 268 / 0 0 / 0											

PLATES (table is in inches)				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K				THIS TRUSS COMPLIES WITH:				
JT TYPE	PLATES	W	LEN	Y	X	BRACING				- PART 9 OF NBC 2015, ABC 2019		
B	TMV+p	MT20	3.0	4.0		TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.				- PART 9 OF OBC 2012 (2019 AMENDMENT)		
C	TMWW-i	MT20	4.0	4.0	2.00	1.50	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				- CSA 086-14	
D	TTWW+m	MT20	5.0	6.0	2.25	1.50	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.				- TPIC 2014	
E	TMW+w	MT20	2.0	4.0			LOADING				(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. (ROOF LIVE LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD	
F	TMW-i	MT20	4.0	6.0			TOTAL LOAD CASES: (4)				ALLOWABLE DEFL.(LL) = L/360 (0.83")	
G	BMV-i+p	MT20	3.0	4.0			CHORDS				CALCULATED VERT. DEFL.(LL) = L/999 (0.03")	
H	BMWW-i	MT20	5.0	6.0	2.50	1.50	MEMB. FORCE (LBS)				ALLOWABLE DEFL.(TL) = L/2880 (0.03")	
I	BS-i	MT20	3.0	8.0			MAX. FACTORED				CALCULATED VERT. DEFL.(TL) = L/999 (0.07")	
J	BMW-i	MT20	4.0	4.0			VERT. LOAD LC1 MAX				CSI: TC=0.74/1.00 (F-G:1), BC=0.23/1.00 (H-J:4), WS=0.44/1.00 (E-H:1), SS=0.29/1.00 (E-F:1)	
K	BMW-i	MT20	4.0	6.0			FROM TO				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10	
							LENGTH FR-TO				COMP=1.10 SHEAR=1.10 TENS=1.10	
							MEMB. FORCE (LBS)				COMPANION LIVE LOAD FACTOR = 1.00	
							MAX. FACTORED				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	
							CSI (LC)				NAIL VALUES	
							FR-TO				PLATE GRIP(DRY) SHEAR SECTION	
							MEMB. FORCE (LBS)				(PSI) (PLI) (PLI)	
							MAX. FACTORED				MAX MIN MAX MIN MAX MIN	
							CSI (LC)				MT20 650 371 1747 788 1987 1673	
							FR-TO				PLATE PLACEMENT TOL. = 0.250 inches	
							MEMB. FORCE (LBS)				PLATE ROTATION TOL. = 5.0 Deg.	
							MAX. FACTORED				JSI: GRIP= 0.89 (C) (INPUT = 0.90)	
							CSI (LC)				JSI: METAL= 0.40 (C) (INPUT = 1.00)	

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2				NAIL VALUES			
				PLATE GRIP(DRY) SHEAR SECTION			
				(PSI) (PLI) (PLI)			
				MAX MIN MAX MIN MAX MIN			
				MT20 650 371 1747 788 1987 1673			
				PLATE PLACEMENT TOL. = 0.250 inches			
				PLATE ROTATION TOL. = 5.0 Deg.			
				JSI: GRIP= 0.89 (C) (INPUT = 0.90)			
				JSI: METAL= 0.40 (C) (INPUT = 1.00)			





DESIGN CRITERIA

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.63")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL)= $L/360$ (0.63")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.54/1.00 (D-E:1), BC=0.28/1.00 (J-K:4)
WB=0.62/1.00 (E-H:1), SSI=0.26/1.00 (E-F:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

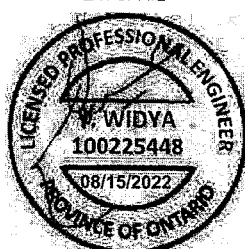
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

Alpa Roof Truss, Maple

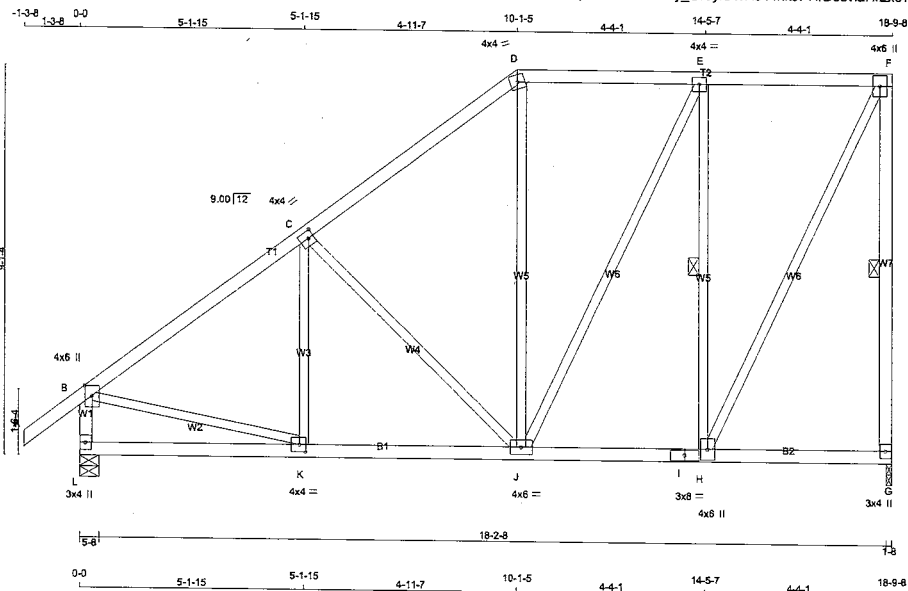
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Aug 15 13:18:16 2022 Page 1
ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-H6WiaO8nWX5CE/AN/yjiTvx/VuX5hvdugMUyqXync6b



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428511	T56	1	1	TRUSS DESC.	JT 53080	E22085770

Alpa Roof Truss, Maple

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LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS	SIZE		
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	Edge	
C	TMWW-i	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-i	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+i	MT20	4.0	6.0		
I	BS-i	MT20	3.0	8.0		
J	BMWWW-i	MT20	4.0	6.0		
K	BMWW-i	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
SPF	VERT	GROSS REACTION	BRG	BRG
JT	DOWN	HORZ	UPLIFT	IN-SX
G	1068	0	1068	0
L	1200	0	1200	0

UNFACTORED REACTIONS	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
G	754	502 / 0	0 / 0	0 / 0
L	844	576 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX. UNBRACED	FR-TO	MEMB.	MAX. FACTORED	MAX
TOTAL LOAD CASES: (4)											
	FR-TO	FORCE (LBS)	FROM TO	(PLF)	CSI (LC)	MAX	LENGTH	FR-TO	FORCE (LBS)	MAX	CSI (LC)
	A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	K-C	-91 / 57	0.04 (1)		
	B-C	-1037 / 0	-95.2	-95.2	0.33 (1)	5.76	C-J	-412 / 0	0.41 (1)		
	C-D	-745 / 0	-95.2	-95.2	0.32 (1)	6.25	J-D	0 / 93	0.03 (4)		
	D-E	-567 / 0	-95.2	-95.2	0.23 (1)	6.25	J-E	0 / 308	0.07 (1)		
	E-F	-435 / 0	-95.2	-95.2	0.22 (1)	6.25	H-E	-786 / 0	0.42 (1)		
	G-F	-1035 / 0	0.0	0.0	0.40 (1)	6.18	H-F	0 / 977	0.22 (1)		
	L-B	-1180 / 0	0.0	0.0	0.12 (1)	7.39	B-K	0 / 880	0.20 (1)		
	L-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00					
	K-J	0 / 857	-18.5	-18.5	0.20 (1)	10.00					
	J-I	0 / 435	-18.5	-18.5	0.11 (4)	10.00					
	I-H	0 / 435	-18.5	-18.5	0.11 (4)	10.00					
	H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")
CSI: TC=0.40/1.00 (F-G:1), BC=0.20/1.00 (J-K:1), WB=0.42/1.00 (E-H:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
	650	371	1747	788

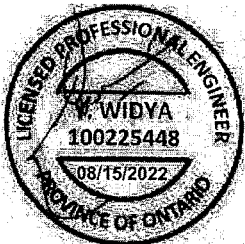
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

JSI METAL= 0.58 (B) (INPUT = 1.00)

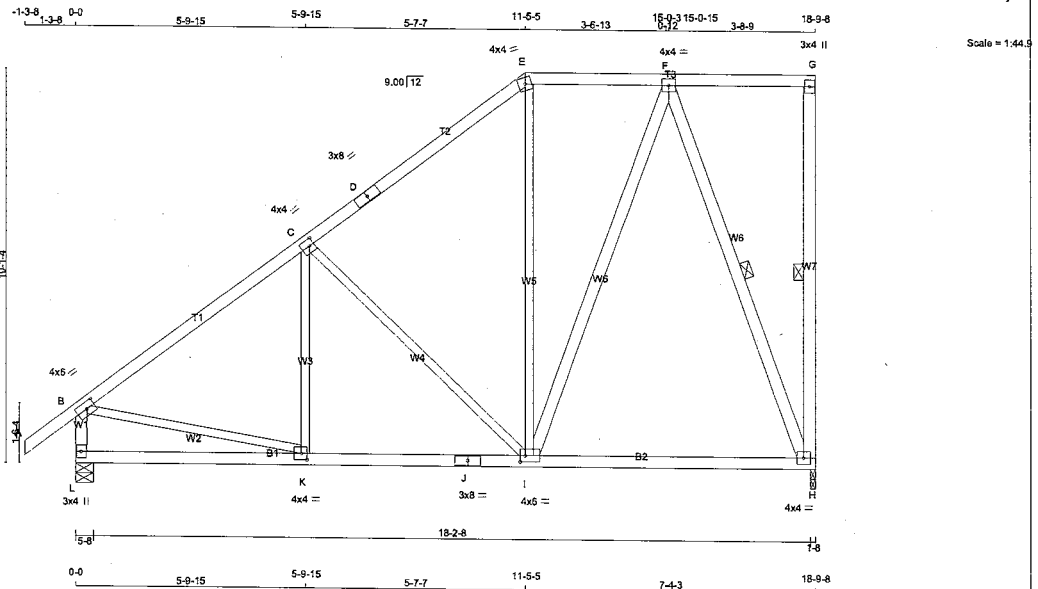
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T57	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085771
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Alpa Roof Truss, Maple

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LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	6.0	2.00	2.75
C	TMVW-I	MT20	4.0	4.0	2.00	1.50
D	TS-I	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-I	MT20	4.0	4.0		
G	TMV-p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWV-I	MT20	4.0	6.0	2.00	1.50
J	BS-I	MT20	3.0	8.0		
K	BMVW-I	MT20	4.0	4.0	2.00	1.75
L	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	VERT 1113	DOWN 0	UPLIFT 1-8	IN-SX
L	1210	0	0	1-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	790	502 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
L	853	576 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT.	FACTORED	FROM	TO	LC1	MAX	UNBRAC	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT.
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	K-C	-69 / 63	0.04 (1)						
B-C	-1035 / 0	-95.2	-95.2	0.43 (1)	5.60	C-I	-495 / 0	0.65 (1)						
C-D	-673 / 0	-95.2	-95.2	0.41 (1)	6.25	I-E	0 / 67	0.02 (4)						
D-E	-673 / 0	-95.2	-95.2	0.41 (1)	6.25	I-F	0 / 474	0.08 (1)						
E-F	-505 / 0	-102.7	-102.7	0.18 (1)	2.00	F-H	-978 / 0	0.56 (1)						
F-G	0 / 0	-102.7	-102.7	0.23 (1)	10.00	B-K	0 / 878	0.20 (1)						
H-G	-145 / 0	0.0	0.0	0.07 (1)	6.25									
L-B	-1184 / 0	0.0	0.0	0.12 (1)	7.39									
L-K	0 / 0	-18.5	-18.5	0.13 (4)	10.00									
K-J	0 / 860	-18.5	-18.5	0.28 (4)	10.00									
J-I	0 / 860	-18.5	-18.5	0.28 (4)	10.00									
I-H	0 / 345	-18.5	-18.5	0.24 (4)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.43/1.00 (B-C:1), BC=0.28/1.00 (I-K:4), WB=0.68/1.00 (C-I:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

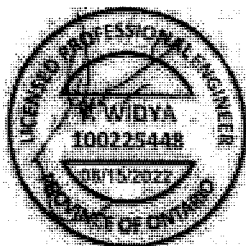
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)

JSI METAL= 0.29 (K) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

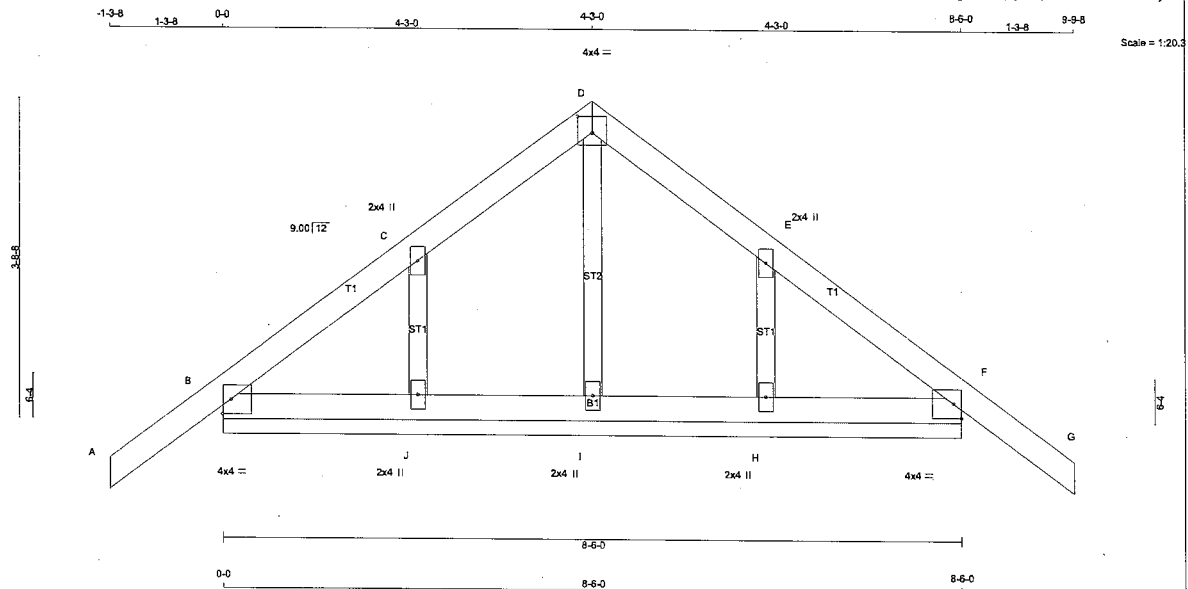


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428511	T58	1		TRUSS DESC.	JT 53080	E22085772
Alpa Roof Truss, Maple						Version 8.530 S Feb 23 2022 M/Tek Industries, Inc. Mon Aug 15 13:18:19 2022 Page 1
						ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-hhCqCQAgosTm5CvyO4GP5YZBv5YguP3KMJICRsync
Scale = 1:50.3						
LUMBER						
N.L.G.A. RULES						
CHORDS	SIZE	LUMBER	DESCR.			
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - F	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
K - B	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
H - E	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						
PLATES (table is in inches)						
JT TYPE	PLATES	W	LEN	Y	X	
B TMVW+p	MT20	4.0	5.0	Edge		
C TMWW-t	MT20	4.0	4.0	2.00	1.50	
D TS-I	MT20	3.0	8.0			
E TTW-m	MT20	4.0	4.0			
F TMVW-t	MT20	4.0	4.0			
G BMV1+p	MT20	3.0	4.0			
H BMWWW-I	MT20	4.0	10.0			
I BS-I	MT20	3.0	8.0			
J BMWW-t	MT20	4.0	4.0	2.00	1.75	
K BMV1+p	MT20	3.0	4.0			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.						
DESIGN CRITERIA						
SPECIFIED LOADS:						
TOP CH. LL = 26.7 PSF						
DL = 8.0 PSF						
BOT CH. LL = 0.0 PSF						
DL = 7.4 PSF						
TOTAL LOAD = 40.1 PSF						
SPACING = 24.0 IN./C/C						
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM						
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015						
THIS DESIGN COMPLIES WITH:						
- PART 9 OF BCBC 2015, ABC 2019						
- PART 9 OF OBC 2012 (2019 AMENDMENT)						
- CSA 086-14						
- TPIC 2014						
(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD						
ALLOWABLE DEFL.(LL)= L/360 (0.63")						
CALCULATED VERT. DEFL.(LL) = U/999 (0.03")						
ALLOWABLE DEFL.(TL)= L/360 (0.63")						
CALCULATED VERT. DEFL.(TL) = U/999 (0.07")						
CSI: TC=0.65/1.00 (F-G-I), BC=0.25/1.00 (H-J-4), WB=0.32/1.00 (C-H-I), SS=0.24/1.00 (B-C-I)						
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10						
COMPANION LIVE LOAD FACTOR = 1.00						
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.						
NAIL VALUES						
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)						
MAX MIN MAX MIN MAX MIN						
MT20 550 371 1747 788 1987 1873						
PLATE PLACEMENT TOL. = 0.250 inches						
PLATE ROTATION TOL. = 5.0 Deg.						
JSI GRIP= 0.90 (B) (INPUT = 0.90)						
JSI METAL= 0.55 (B) (INPUT = 1.00)						
BEARINGS						
FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD						
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX						
G 1088 0 1088 0 0 1-8 1-8						
K 1200 0 1200 0 0 5-8 1-8						
UNFACTORED REACTIONS						
1ST CASE MAX/MIN COMPONENT REACTIONS						
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL						
G 754 502 / 0 0 / 0 0 / 0 232 / 0 0 / 0						
K 844 576 / 0 0 / 0 0 / 0 268 / 0 0 / 0						
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K						
BRACING						
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.						
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.						
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.						
1 LATERAL BRACE(S) AT 1/2 LENGTH DF F-G, C-H, E-H.						
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW						
LOADING						
TOTAL LOAD CASES: (4)						
CHORDS						
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. MAX. CSI (LC) UNBRAC FR-TO FROM TO LENGTH FR-TO						
A-B 0 / 39 -95.2 -95.2 0.13 (1) 10.00 J-C -22 / 89 0.03 (4)						
B-C -1005 / 0 -95.2 -95.2 0.55 (1) 5.44 C-H -589 / 0 0.32 (1)						
C-D -552 / 0 -95.2 -95.2 0.52 (1) 6.25 H-E -206 / 13 0.13 (1)						
D-E -552 / 0 -95.2 -95.2 0.52 (1) 6.25 H-F 0 / 842 0.14 (1)						
E-F -401 / 0 -95.2 -95.2 0.44 (1) 6.25 B-J 0 / 854 0.19 (1)						
G-F -1025 / 0 0.0 0.0 0.65 (1) 6.20						
K-B -1151 / 0 0.0 0.0 0.12 (1) 7.42						
K-J 0 / 0 -18.5 -18.5 0.18 (4) 10.00						
J-I 0 / 840 -18.5 -18.5 0.25 (4) 10.00						
I-H 0 / 840 -18.5 -18.5 0.25 (4) 10.00						
H-G 0 / 0 -18.5 -18.5 0.16 (4) 10.00						
WEBS						
MEMB. MAX. FACTORED FORCE (LBS) MAX. MAX. CSI (LC) UNBRAC FR-TO FROM TO LENGTH FR-TO						
A-B 0 / 39 -95.2 -95.2 0.13 (1) 10.00 J-C -22 / 89 0.03 (4)						
B-C -1005 / 0 -95.2 -95.2 0.55 (1) 5.44 C-H -589 / 0 0.32 (1)						
C-D -552 / 0 -95.2 -95.2 0.52 (1) 6.25 H-E -206 / 13 0.13 (1)						
D-E -552 / 0 -95.2 -95.2 0.52 (1) 6.25 H-F 0 / 842 0.14 (1)						
E-F -401 / 0 -95.2 -95.2 0.44 (1) 6.25 B-J 0 / 854 0.19 (1)						
G-F -1025 / 0 0.0 0.0 0.65 (1) 6.20						
K-B -1151 / 0 0.0 0.0 0.12 (1) 7.42						

JOB NAME 428511	TRUSS NAME T75G	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085774
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Alpine Roof Truss, Maple

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ID:To6XqL09YKbXLLs9y_Z1eyrbTx-5GtzqSDY5NrLzgeX4Dq6ABqMJcu5rEm2Hxs2Aync6V



Scale = 1/20.3

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES	A - D	2x4	DRY	No.2
	D - G	2x4	DRY	No.2
	B - F	2x4	DRY	No.2
	ALL WEBS	2x3	DRY	No.2
	ALL GABLE WEBS	2x3	DRY	No.2
	DRY: SEASONED LUMBER.			
	GABLE STUDS SPACED AT	2'-0" O.C.		

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	4.0	4.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTW+w	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB14	MT20	4.0	4.0	Edge	
H, I, J	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)	UNBRACED LENGTH (FR-TO)
A-B	0 / 34	-95.2	-95.2 0.13 (1)	I-D	-107 / 0	0.02 (1)	10.00
B-C	-35 / 0	-95.2	-95.2 0.07 (1)	J-C	-240 / 0	0.04 (1)	6.25
C-D	-49 / 0	-95.2	-95.2 0.07 (1)	H-E	-240 / 0	0.04 (1)	6.25
D-E	-49 / 0	-95.2	-95.2 0.07 (1)				
E-F	-35 / 0	-95.2	-95.2 0.07 (1)				
F-G	0 / 34	-95.2	-95.2 0.13 (1)				
B-J	0 / 39	-18.5	-18.5 0.02 (4)				10.00
J-I	0 / 27	-18.5	-18.5 0.02 (4)				10.00
I-H	0 / 27	-18.5	-18.5 0.02 (4)				10.00
H-F	0 / 39	-18.5	-18.5 0.02 (4)				10.00

TOTAL WEIGHT = 31 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (F-G:1), BC=0.02/1.00 (B-J:4), WB=0.04/1.00 (E-H:1), SS=0.09/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

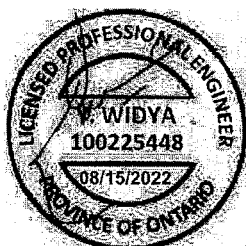
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (F) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

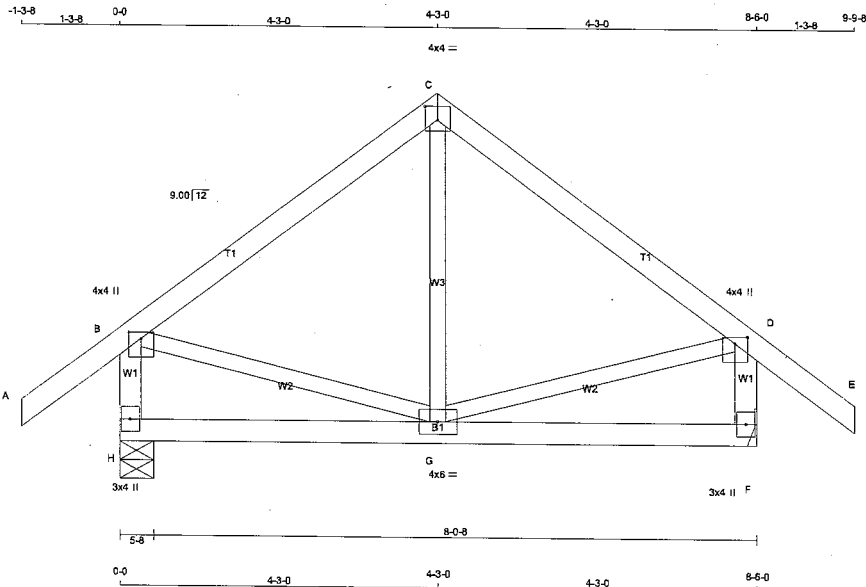
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T76	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085775
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Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Mon Aug 15 13:18:23 2022 Page 1
ID:To6XqL09rYKBxLLsBy_Z1eyrbTx-aSRLZnEAsgzCagDjdWLLFOk_glx1qlAwHxgQadync8U



Scale = 1:23.5

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE	MT20	4.0	4.0	1.00	2.00
B TMVV+p	MT20	4.0	4.0	2.25	2.00
C TTW-p	MT20	4.0	4.0	1.00	2.00
D TMVV+p	MT20	4.0	4.0	1.00	2.00
F BMV1+p	MT20	3.0	4.0		
G BMVWW+p	MT20	4.0	6.0		
H BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	VERT	DOWN	UPLIFT	IN-SX
F	615	0	0	1-8
F	615	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	431	301/0	0/0	0/0	0/0	130/0	0/0
F	431	301/0	0/0	0/0	0/0	130/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/39	-95.2	-95.2	0.13 (1)	10.00	G-C	-40/60	0.02 (4)	
B-C	-304/0	-95.2	-95.2	0.22 (1)	6.25	B-G	0/252	0.06 (1)	
C-D	-304/0	-95.2	-95.2	0.22 (1)	6.25	G-D	0/252	0.05 (1)	
D-E	0/39	-95.2	-95.2	0.13 (1)	10.00				
H-B	-584/0	0.0	0.0	0.06 (1)	7.81				
F-D	-584/0	0.0	0.0	0.06 (1)	7.81				
H-G	0/0	-18.5	-18.5	0.09 (4)	10.00				
G-F	0/0	-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.22/1.00 (B-C-1), BC=0.09/1.00 (G-H-4), WB=0.06/1.00 (D-G-1), SS=0.13/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

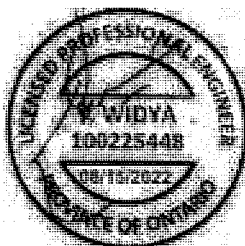
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428511	T79		3	GREEN PARK HOMES JT 52080	E22085778(2)

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mitak Industries, Inc. Mon Aug 15 13:18:30 2022 Page 2
ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-spM?WBJZDqsDwvF3Yuz_2sX8pXD1zGcxuXlIKjync6N

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
L TTWW+m	MT20	8.0	9.0	Edge	3.25
M TMWWW-t	MT20	6.0	10.0	2.75	4.50
N TS-t	MT20	6.0	7.0		
O TMWW-t	MT20	6.0	6.0		
P TMWWW-t	MT20	6.0	10.0	3.00	4.50
Q TMBW1-t	MT20	6.0	12.0	2.50	2.50
Q TP-t	MT20	4.0	6.0	3.25	2.75
S, W, AE, AH					
S BMW+w	MT20	4.0	6.0		
T, Y, AB, AG					
T BMWW-t	MT20	5.0	6.0		
U BS-t	MT18HS	7.0	10.0		
V BMWW-t	MT20	6.0	10.0	5.50	3.00
X BMWW-t	MT20	5.0	6.0	2.25	2.25
Z BS-t	MT20	6.0	7.0		
AA BMWWW-t	MT20	6.0	7.0	2.50	3.50
AC BMWW-t	MT20	5.0	6.0	2.25	2.25
AD BS-t	MT20	7.0	12.0	3.50	5.75
AF BMWW-t	MT20	6.0	10.0	5.50	3.00

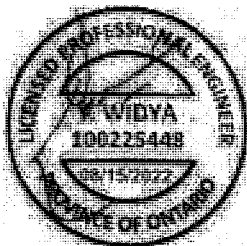
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



JOB NAME TRUSS NAME QUANTITY PLY JOB DESC. GREEN PARK HOMES

428511 **T79Z** **3** **TRUSS DESC. JT 53080**

Alpa Roof Truss, Maple

DRWG. NO. **E22085779**

Version 8.530 S Feb 23 2022 M/Tek/Drws, Inc. Mon Aug 15 13:18:33 2022 Page 1

ID: T06XqL09YKbXlLs9y_Z1eyrbTx-HC0278CLSVIEnnMzeD1WhfV9emkEGAE9OaV5yx2ync6

Scale = 1/97.1

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
A - E	2x8	DRY	No.2	SPF
E - G	2x8	DRY	No.2	SPF
G - J	2x8	DRY	No.2	SPF
J - L	2x8	DRY	No.2	SPF
L - N	2x8	DRY	No.2	SPF
N - R	2x8	DRY	No.2	SPF
R - AD	2x8	DRY	1950F 1.7E	SPF
AD - Z	2x8	DRY	1950F 1.7E	SPF
Z - U	2x8	DRY	1950F 1.7E	SPF
U - Q	2x8	DRY	1950F 1.7E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	LUMBER	DESCR.	SPF
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY

WEBS: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #/ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS: (0.122"x3") SPIRAL NAILS

MEMBER	SIZE	LOAD(PLF)
A-E	2	12
E-G	2	12
L-N	2	12
N-R	2	12
G-J	2	12
J-L	2	12

BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS

MEMBER	SIZE	LOAD(PLF)
R-AD	2	12
AD-Z	2	12
Z-U	2	12
U-Q	2	12

WEBS: (0.122"x3") SPIRAL NAILS

MEMBER	SIZE	LOAD(PLF)
M-W	2	5
M-V	2	5
2x4	1	6
2x8	2	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X	
B	TBW1-I	MT20	6.0	12.0	2.50	2.50
B	TP-I	MT20	4.0	6.0	2.75	2.75
C	TMMW-I	MT20	6.0	10.0	3.00	4.50
D	TMMW-I	MT20	5.0	6.0		
E	TS-I	MT20	6.0	7.0		
F	TMMW-I	MT20	10.0	12.0	5.50	4.50
G	TTWW-I	MT20	8.0	9.0	Edge	3.25
H	TMMW-I	MT20	4.0	6.0	2.00	2.75
I	TMMW-I	MT20	2.0	4.0		
J	TS-I	MT20	5.0	6.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORIZ	DOWN	HORIZ
B	7373	0	7736
Q	15015	0	15972

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 597 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 1114 LBS. FACTORED UPLIFT

PROVIDE FOR 134 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST CLASS	MAX/MIN. COMPONENT REACTIONS					
SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
JT	5435	3507 / 0	946 / 0	0 / 0	484 / -1213	1224 / 0
Q	11074	7272 / 0	1943 / 0	0 / 0	1090 / -2401	2497 / 0

HORIZONTAL REACTIONS

MEMBER	0 / 0	0 / 0	0 / 0	96 / -96	0 / 0	0 / 0
B						

BEARING MATERIAL TO BE SPF No.2 OR BETTER AT JOINT(S) B, Q

BEARING SIZE FACTOR = 1.15 AT JNT(S) Q (BASED ON SUPPORT DEPTH = 1-8)

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.08 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT G-A/C, L-X, M-X, L-Y, G-A/B, K-Y, H-A/B, K-AA, H-AA, I-AA

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
K TMWW-t	MT20	4.0	6.0	2.00	2.75
L TTWW+m	MT20	8.0	9.0	Edge	3.25
M TMWWW-t	MT20	10.0	12.0	5.50	4.50
N TS-t	MT20	5.0	7.0		
O TMWW-t	MT20	5.0	6.0		
P TMWWW-t	MT20	6.0	10.0	3.00	4.50
Q TMBW1-t	MT20	6.0	12.0	2.50	2.50
Q TP-t	MT20	4.0	6.0	3.25	2.75
S BWw+w	MT20	4.0	6.0		
T, Y, AB, AG					
T BMWW-t	MT20	5.0	6.0		
U BS-t	MT18HS	7.0	10.0		
V BMWW-t	MT20	6.0	10.0	5.50	3.00
W BMWW+w	MT20	5.0	12.0		
X BMWW-t	MT20	5.0	6.0	2.25	2.25
Z BS-t	MT20	6.0	7.0		
AA BMWWW-t	MT20	6.0	7.0	2.75	3.50
AC BMWW-t	MT20	5.0	6.0	2.25	2.25
AD BS-t	MT20	7.0	12.0	3.50	5.75
AE BMWW+w	MT20	6.0	12.0		
AF BMWW-t	MT20	6.0	10.0	5.50	3.00
AH BMWW+w	MT20	4.0	6.0		

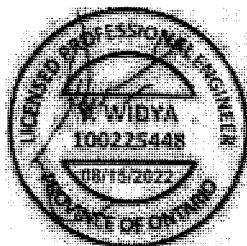
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



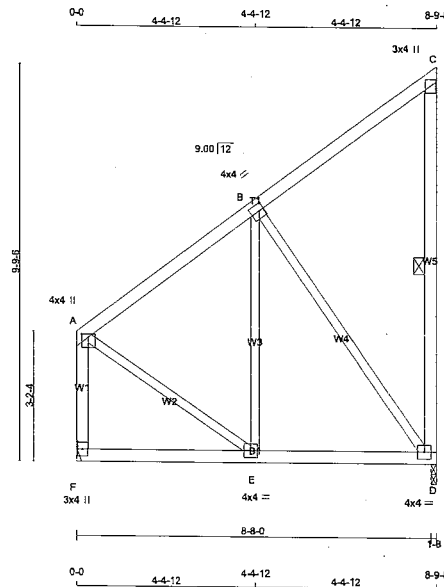
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JOB NAME 428511	TRUSS NAME T84	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085782
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Alpine Roof Truss, Maple

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Scale = 1/4" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW-t	MT20	4.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
F	500	0	500	0
D	500	0	500	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
F	353	235/0	0/0	0/0
D	353	235/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	FORCE	MAX	W E B S	MAX. FACTORED	MEMB.	FORCE	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	FORCE	MAX
FR-TO	(LBS)	FROM	TO	CS1 (LC)	UNBRAC	FR-TO	(LBS)	CS1 (LC)		
F-A	-468/0	0.0	0.0	0.08 (1)	7.81	A-E	0/265	0.05 (1)		
A-B	-245/0	-85.2	-85.2	0.24 (1)	6.25	E-B	-45/65	0.03 (1)		
B-C	-31/0	-85.2	-85.2	0.24 (1)	6.25	B-D	-382/0	0.42 (1)		
D-C	-158/0	0.0	0.0	0.07 (1)	6.25					
F-E	0/0	-18.5	-18.5	0.10 (4)	10.00					
E-D	0/222	-18.5	-18.5	0.12 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.7	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	42.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 2.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CS1: TC=0.24/1.00 (A-B:1), BC=0.12/1.00 (D-E:4), WB=0.42/1.00 (B-D:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

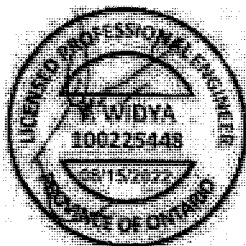
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (A) (INPUT = 0.90)

JSI METAL= 0.09 (A) (INPUT = 1.00)

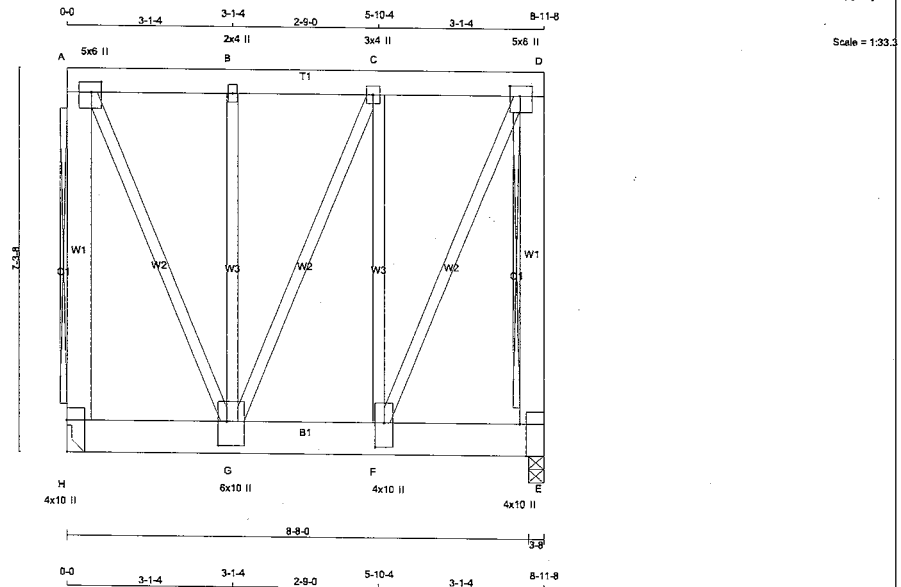
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428511	T85	3		GREEN PARK HOMES JT 53080	E22085783

Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-dLr0BwPbKHs47sc0a8sMYsXz0Mrz7knpcFyn06F



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
H - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x8	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
H-A 2	12	TOP
A-D 2	12	SIDE(35.0)
D-E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-E 4	4	SIDE(1272.3)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	5.0	6.0	2.25
B	TMW+w	MT20	2.0	4.0	
C	TMWV+H	MT20	3.0	4.0	
D	TMWV+p	MT20	5.0	6.0	2.25
E	BMV1+H	MT20	4.0	10.0	Edge 1.50
F	BMWV+H	MT20	4.0	10.0	5.50 2.00
G	BMWVW+H	MT20	6.0	10.0	5.50 2.00
H	BMV1+H	MT20	4.0	10.0	7.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPLIFT
H	9029	0	9119	-194
E	9029	0	9119	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4.0.

PROVIDE FOR 194 LBS FACTORED HORIZONTAL REACTION AT JOINT

UNFACTORED REACTIONS	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
H	8692	3855 / 0	1138 / 0	0 / 0
E	8692	3855 / 0	1138 / 0	0 / 0

HORIZONTAL REACTIONS	H	E
	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT A-H, D-E

FASTEN T AND I BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX.	FACTORED		
MEMB.	FORCE	VERT. LOAD LC1	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO	FROM	TO	UNBRAC	LENGTH	FR-TO		
H-A	-6342 / 0	0.0	0.0	0.28 (2)	7.81	F-D	0 / 6766 0.51 (2)
A-B	-2814 / 0	-57.8	-57.8	0.06 (2)	6.25	A-G	0 / 6743 0.51 (2)
B-C	-2814 / 0	-57.8	-57.8	0.07 (2)	6.25	F-C	-170 / 125 0.04 (2)
C-D	-2823 / 0	-57.8	-57.8	0.06 (2)	6.25	G-B	-181 / 97 0.05 (9)
E-D	-6356 / 0	0.0	0.0	0.28 (2)	7.81	G-C	-69 / 79 0.02 (12)
H-G	-125 / 167	-1948.0	-1948.0	0.30 (2)	6.25		
G-F	0 / 2823	-1948.0	-1948.0	0.25 (2)	10.00		
F-E	-29 / 75	-1948.0	-1948.0	0.29 (2)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0.4) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, Cp, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL COEFFICIENTS, Cpi, BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 3 X 86 = 258 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 35.1 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 58.0 PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 36.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = U/999 (0.03")
ALLOWABLE DEFL.(TL) = L/780 (0.50")
CALCULATED VERT. DEFL.(TL) = U/999 (0.04")

CSI: TC=0.28/1.00 (D-E-2), BC=0.30/1.00 (G-H-2), WS=0.51/1.00 (D-F-2), SS=0.58/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

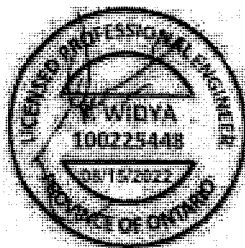
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (D) (INPUT = 0.90)
JSI METAL = 0.42 (F) (INPUT = 1.00)

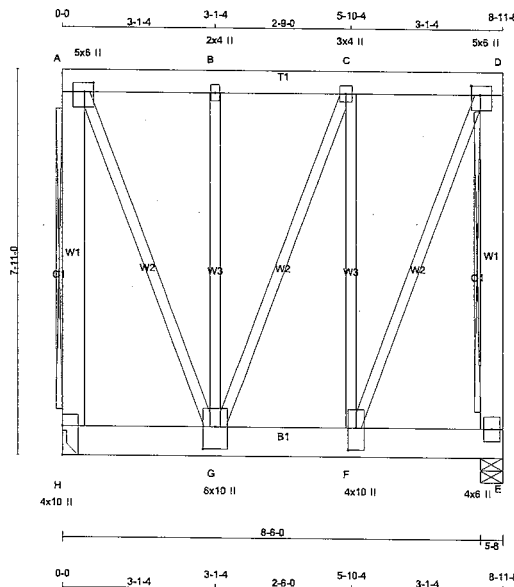
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T86	QUANTITY 3	PLY 3	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085784
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Alpa Roof Truss, Maple

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Scale = 1:36.0

LUMBER	SIZE	LUMBER	DESCR.
N.L.G.A. RULES			
CHORDS	SIZE	DRY	SPF
H - A	2x6	DRY	No.2
A - D	2x6	DRY	No.2
E - D	2x6	DRY	No.2
H - E	2x8	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY SEASONED LUMBER			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
H-A 2	12	TOP
A-D 2	12	SIDE(35.0)
D-E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-E 4	4	SIDE(1240.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.25 2.25
B	TMVW+w	MT20	2.0	4.0	
C	TMVW+h	MT20	3.0	4.0	
D	TMVW+p	MT20	5.0	6.0	2.25 2.25
E	BMV1+p	MT20	4.0	6.0	
F	BMVW+h	MT20	4.0	10.0	5.50 2.00
G	BMVWV+h	MT20	6.0	10.0	5.50 3.00
H	BMV1+t	MT20	4.0	10.0	7.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	BRG	BRG
H	8815	0	8828	-212
E	8815	0	8828	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4.0.

PROVIDE FOR 212 LBS FACTORED HORIZONTAL REACTION AT JOINT

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	8535	3705 / 0	1111 / 0
E	8535	3705 / 0	1111 / 0

HORIZONTAL REACTIONS	H	D / 0	0 / 0	152 / -152	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT A-H, D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
H-A	-8110 / 0	0.0	0.0	0.31 (2)	F-D	0 / 8438	0.48 (2)
A-B	-2489 / 0	-57.8	-67.8	0.06 (2)	A-G	0 / 8421	0.48 (2)
B-C	-2489 / 0	-57.8	-67.8	0.07 (2)	F-C	-162 / 194	0.05 (7)
C-D	-2494 / 0	-57.8	-67.8	0.06 (1)	G-B	-141 / 97	0.05 (7)
D-E	-6120 / 0	0.0	0.0	0.31 (2)	G-C	-83 / 88	0.03 (12)
H-G	-136 / 182	-1900.2	-1900.2	0.29 (2)			
G-F	0 / 2494	-1900.2	-1900.2	0.24 (2)			
F-E	-32 / 82	-1900.2	-1900.2	0.29 (2)			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-3) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL COEFFICIENTS, C_{pi}, ARE BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN. AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	35.1	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF NBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/180 (0.60")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.31/1.00 (D-E-2), BC=0.29/1.00 (G-H-2),
WB=0.48/1.00 (D-F-2), SSI=0.57/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

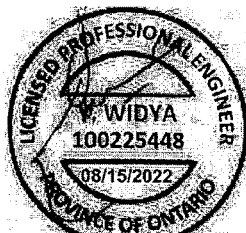
PLATE VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 1.00)

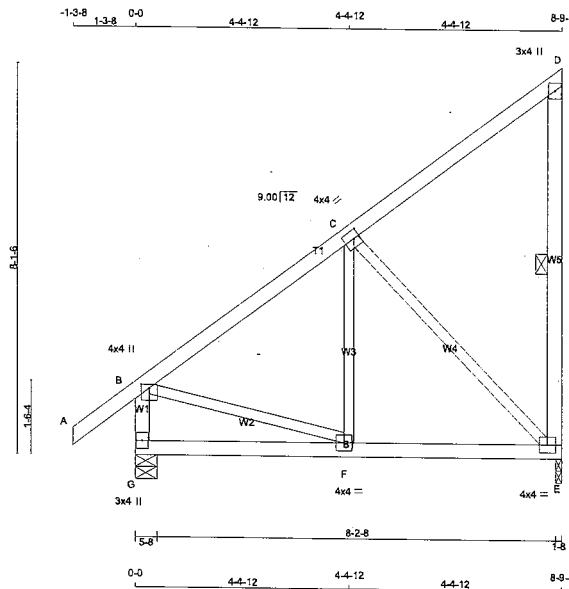
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T87	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085785
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Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-2wX9qxRTdCEfkbBhig_BU3rz4m2N?ZQk1NDaync6C



Scale = 1/8" = 1'-0"

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	
G - B	2x4	DRY	No.2	
G - E	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TMV+p	MT20	3.0	4.0	
E	BMVW-t	MT20	4.0	4.0	
F	BMVW-t	MT20	4.0	4.0	
G	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT HORZ	DOWN HORZ	UPLIFT IN-SX	IN-SX	
E 500 0	500 0	0 0	1-8	1-8
G 631 0	631 0	0 0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	353	235 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
G	443	309 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/39	-95.2	-95.2 0.13 (1)	10.00	F-C	0/86 0.03 (4)	
B-C	-349/0	-95.2	-95.2 0.24 (1)	6.25	C-E	-434/0 0.29 (1)	
C-D	-31/0	-95.2	-95.2 0.24 (1)	6.25	B-F	0/315 0.07 (1)	
E-D	-158/0	0.0	0.0 0.05 (1)	6.25			
G-B	-599/0	0.0	0.0 0.06 (1)	7.81			
G-F	0/0	-18.5	-18.5 0.10 (4)	10.00			
F-E	0/304	-18.5	-18.5 0.12 (4)	10.00			

TOTAL WEIGHT = 3 X 46 = 138 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.24/1.00 (B-C-1), BC=0.12/1.00 (E-F-4), WB=0.29/1.00 (C-E-1), SS=0.16/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

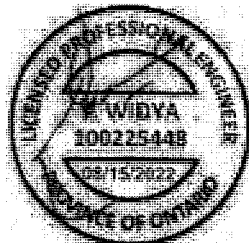
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.53 (B) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 1.00)

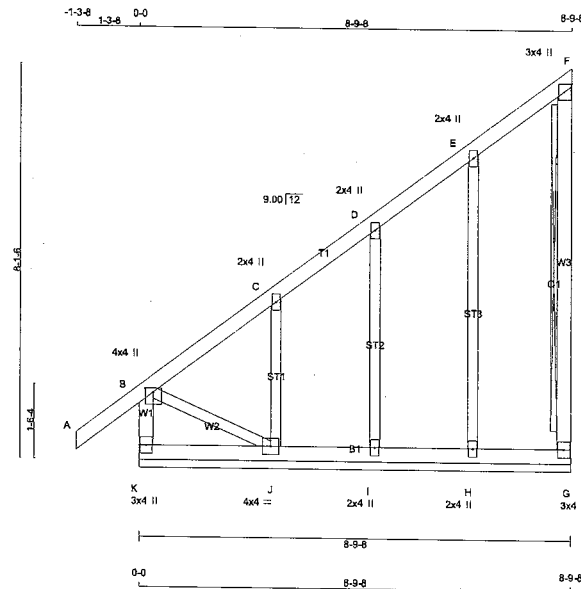
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T87G	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085786
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Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-W65X1HSSOWMM9NFQBoX00FINRQnsHjOnwDync68



Scale = 1:36.0

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E					
C TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	3.0	4.0		
G BMV1+p	MT20	3.0	4.0		
H BMV1+w	MT20	2.0	4.0		
I BMV1+w	MT20	2.0	4.0		
J BMVW1-t	MT20	4.0	4.0		
K BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
K-B	-252/0	0.0	0.0 0.03 (1)	H-E	-219/0	0.0	0.0 0.16 (1)
A-B	0/39	-95.2	-95.2 0.13 (1)	I-D	-160/0	0.0	0.06 (1)
B-C	-2/0	-95.2	-95.2 0.09 (1)	J-C	-262/0	0.0	0.06 (1)
C-D	-20/0	-95.2	-95.2 0.09 (1)	B-J	0/14	0.0	0.00 (1)
D-E	0/0	-95.2	-95.2 0.05 (1)				
E-F	-11/0	-95.2	-95.2 0.05 (1)				
G-F	-80/0	0.0	0.0 0.06 (1)				
K-J	0/0	-18.5	-18.5 0.03 (4)				
J-I	0/6	-18.5	-18.5 0.03 (4)				
I-H	0/3	-18.5	-18.5 0.02 (4)				
H-G	0/0	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 2 X 48 = 96 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.03/1.00 (I-J:4), WB=0.16/1.00 (E-H:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 371 1747 788 1987 1873	

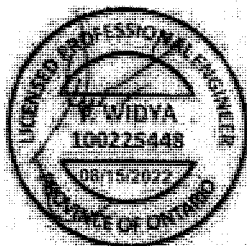
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)

JSI METAL= 0.14 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

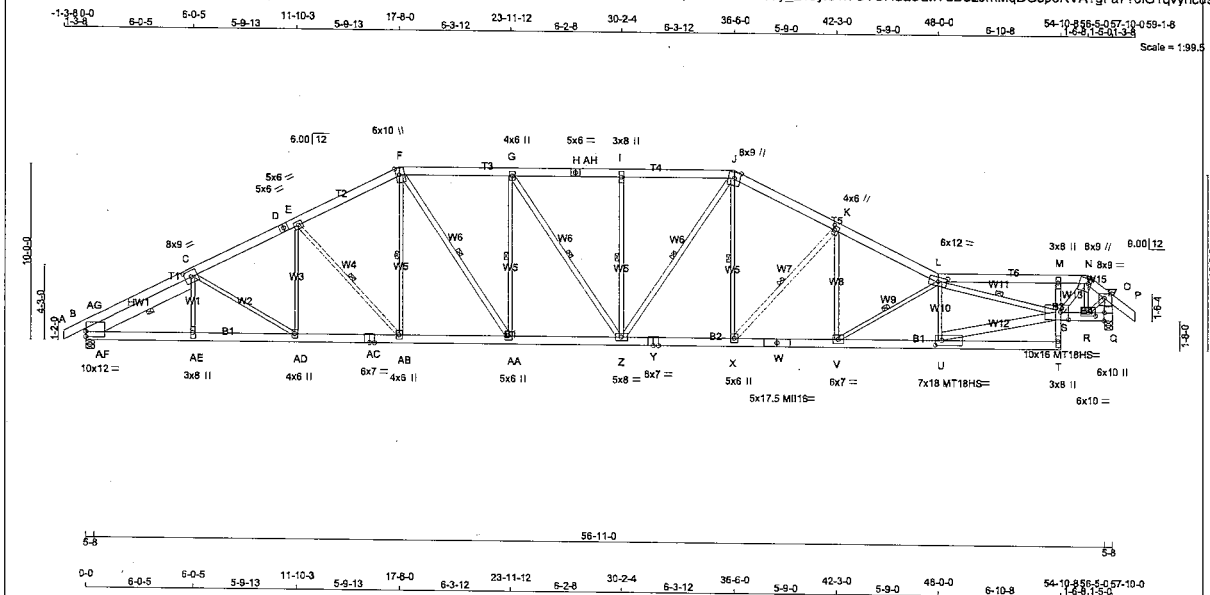


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428511	T88		1	TRUSS DESC.	JT 53080	E22085787

Alpa Roof Truss, Maple

ID:To6XqL09rYKbXLLs9y_Z1eybTx-SVCHSULw7Db2JmMqDgcp6RVA?gFa776G1qvnc69

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LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
A - D	2x6 DRY	No.2
E - F	2x6 DRY	No.2
F - H	2x6 DRY	No.2
H - J	2x6 DRY	No.2
J - L	2x6 DRY	No.2
L - N	2x6 DRY	No.2
N - P	2x6 DRY	No.2
P - Q	2x6 DRY	No.2
Q - O	2x6 DRY	No.2
S - AC	2x6 DRY	2100F 1.8E
AC - Y	2x6 DRY	2100F 1.8E
Y - W	2x6 DRY	2100F 1.8E
W - T	2x6 DRY	2100F 1.8E
T - M	2x4 DRY	No.2
S - Q	2x6 DRY	No.2
REINFORCING MEMBERS		
HW1	2x8 DRY	No.2
ALL WEBS	2x3 DRY	No.2
EXCEPT		
F - AA	2x4 DRY	No.2
G - Z	2x4 DRY	No.2
Z - J	2x4 DRY	No.2
X - K	2x4 DRY	No.2
U - S	2x6 DRY	2100F 1.8E
L - S	2x4 DRY	No.2
S - N	2x4 DRY	No.2
DRY, SEASONED LUMBER.		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
B	4911	0	4911	0
Q	4904	0	4904	0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 214 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 253 LBS FACTORED UPLIFT

PROVIDE FOR 175 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST LOASE	SNOW	MIN	LIVE	PERM	LIVE	WIND	DEAD	SOIL
JT	3625	2126 / 0	607 / 0	0 / 0	89 / 726	892 / 0	0 / 0	0 / 0
Q	3619	2131 / 0	607 / 0	0 / 0	128 / 747	881 / 0	0 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	125 / 79	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q

BRACING

FOR SECTION F-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-AB, F-AB, F-AA, G-AA, G-Z, I-Z, J-Z, J-X, K-X, L-V, L-S, C-AF.

END TRUSS(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	MEMB.	FORCE
(LBS)	(PLF)	(LBS)	(LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 1	A-E	0 / 291
B-AG	-5715 / 182	C-AD	-392 / 176
AG-C	-3818 / 239	AD-E	0 / 440
C-D	-7668 / 300	E-AB	-1269 / 216
D-E	-7668 / 300	AB-F	-81 / 1201
E-F	-6946 / 275	F-AA	-82 / 1860
F-G	-7141 / 249	AA-G	-1238 / 150
G-AH	-7327 / 288	G-Z	-808 / 1300
AH-H	-7327 / 288	Z-J	-994 / 122
H-I	-7327 / 288	J-X	-65 / 1321
I-J	-7327 / 288	X-K	-124 / 2048
J-K	-7598 / 331	K-L	-28 / 1559
K-L	-9150 / 386	L-M	-2436 / 184
L-M	-6617 / 382	M-N	-2307 / 185
M-N	-6675 / 384	N-O	-359 / 10395
N-O	-4145 / 226	O-P	-3713 / 771
O-P	0 / 53	P-Q	-320 / 5629
Q-O	-4983 / 269	Q-R	-1870 / 79
		R-O	-124 / 3886
		AF-AG	0 / 2509
		AF-C	-4365 / 67
B-AF	-260 / 3411		
AF-AE	-345 / 7013		
AD-AC	-238 / 6871		
AC-AB	-238 / 6871		
AB-AA	-95 / 6192		
AA-Z	-50 / 7141		
Z-Y	-26 / 6782		
Y-X	-26 / 6782		
X-W	-205 / 8229		
W-V	-205 / 8229		
V-U	-358 / 10223		
U-T	-5 / 76		
T-S	0 / 162		
S-M	-847 / 190		
S-R	-88 / 3149		
R-Q	-5 / 11		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 35.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 59.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF CBC 2018, ABC 2018
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.47")
ALLOWABLE DEFL.(TL) = L/180 (3.86")
CALCULATED VERT. DEFL.(TL) = L/999 (0.67")

CSI: TC=0.79/1.00 (K-L-1), BC=0.56/1.00 (U-V-1), WB=0.90/1.00 (N-S-1), SS=0.34/1.00 (F-G-2)

DDL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
COMPAION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873
MT18HS 586 403 2455 1382 3153 3004
MI16 438 302 2547 1256 4283 1816

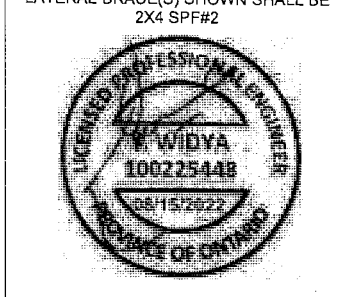
PLATE PLACEMENT TOL. = 0.250 inches

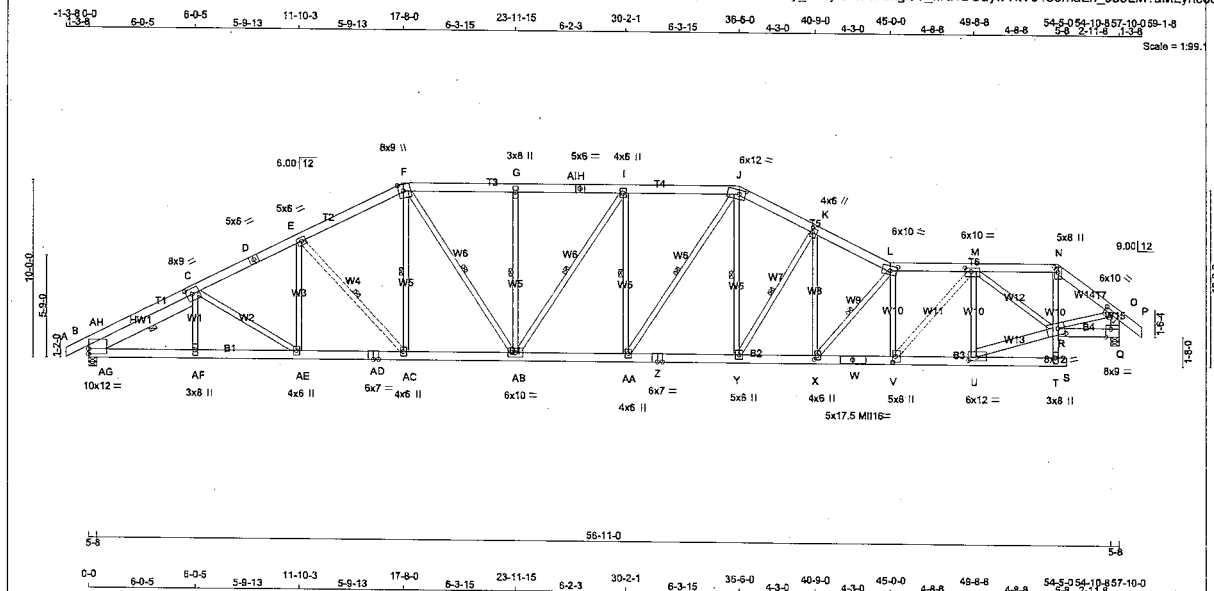
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)

JSI METAL= 1.00 (Y) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 4 X 411 = 1646 lb			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				DESIGN CRITERIA			
A - D	2x6	DRY	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:			
D - F	2x6	DRY	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 35.1 PSF			
F - H	2x6	DRY	SPF	JT VERT	DOWN	HORIZ	UPLIFT	LL = 6.0 PSF			
H - J	2x6	DRY	SPF	B 4914	0	4914	214	DL = 10.5 PSF			
J - L	2x6	DRY	SPF	Q 4928	0	4928	0	DL = 7.4 PSF			
L - N	2x6	DRY	SPF	PROVIDE ANCHORAGE AT BEARING JOINT B FOR 214 LBS FACTORED UPLIFT				TOTAL LOAD = 59.0 PSF			
N - P	2x6	DRY	SPF	PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 244 LBS FACTORED UPLIFT				SPACING = 24.0 IN. C/C			
Q - O	2x6	DRY	SPF	PROVIDE FOR 194 LBS FACTORED HORIZONTAL REACTION AT JOINT				LOADING IN HIGHEST FLAT SECTION BASED ON			
B - AD	2x6	DRY	SPF	UNFACTORED REACTIONS				PIGGYBACK TRUSS WITH SLOPES OF 6.00/12			
AD - Z	2x6	DRY	SPF	1ST CASE	MAX. MIN. COMPONENT REACTIONS			AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS			
Z - W	2x6	DRY	SPF	JT COMBINED	SNOW	LIVE	PERM. LIVE	OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD			
W - S	2x6	DRY	SPF	B 3628	2127/0	608/0	0/0	LOAD OF 5.0 P.S.F.			
R - Q	2x6	DRY	SPF	Q 3640	2130/0	621/0	0/0	LOADING IN OTHER FLAT SECTIONS BASED ON			
REINFORCING MEMBERS				HORIZONTAL REACTIONS				A SLOPE OF 6.00/12			
HW1	2x8	DRY	SPF	B	0/0	0/0	0/0	THIS TRUSS IS DESIGNED FOR COMMERCIAL			
ALL WEBS 2x4 DRY				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q				OR INDUSTRIAL BUILDING REQUIREMENTS OF			
EXCEPT				BRACING				PART 4, NBCC 2015			
U - R 2x6 DRY				FOR SECTION F-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.48 FT.				- PART 4 OF NBC 2015, ABC 2019			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.				- PART 4 OF NBC 2012 (2019 AMENDMENT)			
				MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT				- CSA 086-14			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				- TPIC 2014			
				1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-A, F-A, J-Y, K-Y, L-X, J-AA, F-AB, I-AA, G-AB, I-AB, C-AG.				DESIGN ASSUMPTIONS			
				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN				- SLOPE REDUCTION FACTOR USED			
				THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				- PERCENTAGE OF GROUND SNOW LOAD IS			
				LOADING				USER-DEFINED.			
				TOTAL LOAD CASES: (18)				(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.			
								RAIN LOAD) TIMES IMPORTANCE FACTOR			
								EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE			
								LOAD			
								ALLOWABLE DEFL.(LL) = L/360 (1.33")			
								CALCULATED VERT. DEFL.(LL) = U/999 (0.42")			
								ALLOWABLE DEFL.(TL) = L/180 (3.86")			
								CALCULATED VERT. DEFL.(TL) = U/999 (0.80")			
								CS1: TC=0.68/1.00 (I-J-1), BC=0.51/1.00 (V-X-1),			
								WB=0.92/1.00 (M-R-1), SS=0.34/1.00 (I-J-3)			
								DO1 LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS=1.10			
								SNOW LOAD IMPORTANCE FACTOR = 1.00			
								WIND LOAD IMPORTANCE FACTOR = 1.00			
								LIVE LOAD IMPORTANCE FACTOR = 1.00			
								COMPANION LIVE LOAD FACTOR = 1.00			
								AUTOSOLVE HEELS OFF			
								TRUSS PLATE MANUFACTURER IS NOT			
								RESPONSIBLE FOR QUALITY CONTROL IN THE			
								TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 786 1987 1873			
								MI15 438 302 2547 1256 4283 1816			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.90 (C) (INPUT = 0.90)			
								JSI METAL= 0.99 (AD) (INPUT = 1.00)			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBW1-1	MT20	10.0	12.0	3.50	
C TMBW1-1	MT20	8.0	9.0	3.75	4.00
D TS-1	MT20	5.0	6.0		
E TMBW1-1	MT20	5.0	6.0		
F TMBW1-1	MT20	8.0	9.0		
G TMBW1-1	MT20	3.0	8.0		
H TS-1	MT20	5.0	6.0		
I TMBW1-1	MT20	4.0	6.0		
J TMBW1-1	MT20	6.0	12.0	2.75	4.25
K TMBW1-1	MT20	4.0	6.0	3.00	1.00
L TMBW1-1	MT20	6.0	10.0	3.50	4.50
M TMBW1-1	MT20	6.0	10.0	2.50	4.00
N TMBW1-1	MT20	5.0	8.0	3.50	2.50
O TMBW1-1	MT20	6.0	10.0	2.50	5.00
P BWMW1-1	MT20	8.0	9.0	Edge	3.50
Q BWMW1-1	MT20	6.0	12.0	4.50	4.50
R BWMW1-1	MT20	3.0	8.0		
S BWMW1-1	MT20	6.0	12.0	2.25	5.25
T BWMW1-1	MT20	5.0	8.0	4.00	1.75
U BWMW1-1	MT20	5.0	8.0		
V BWMW1-1	MT20	4.0	6.0	2.75	2.00
W BWMW1-1	MT20	5.0	6.0		
X BWMW1-1	MT20	6.0	7.0		
Y BWMW1-1	MT20	4.0	6.0		
Z BWMW1-1	MT20	6.0	10.0		
AA BWMW1-1	MT20	3.0	8.0		
AB BWMW1-1	MT20	6.0	10.0		
AC BWMW1-1	MT20	8.0	7.0		
AD BWMW1-1	MT20	3.0	8.0		
AE BWMW1-1	MT20	6.0	10.0		
AF BWMW1-1	MT20	3.0	8.0		

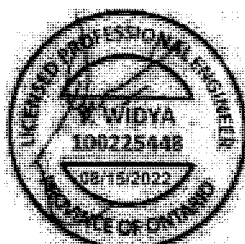
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

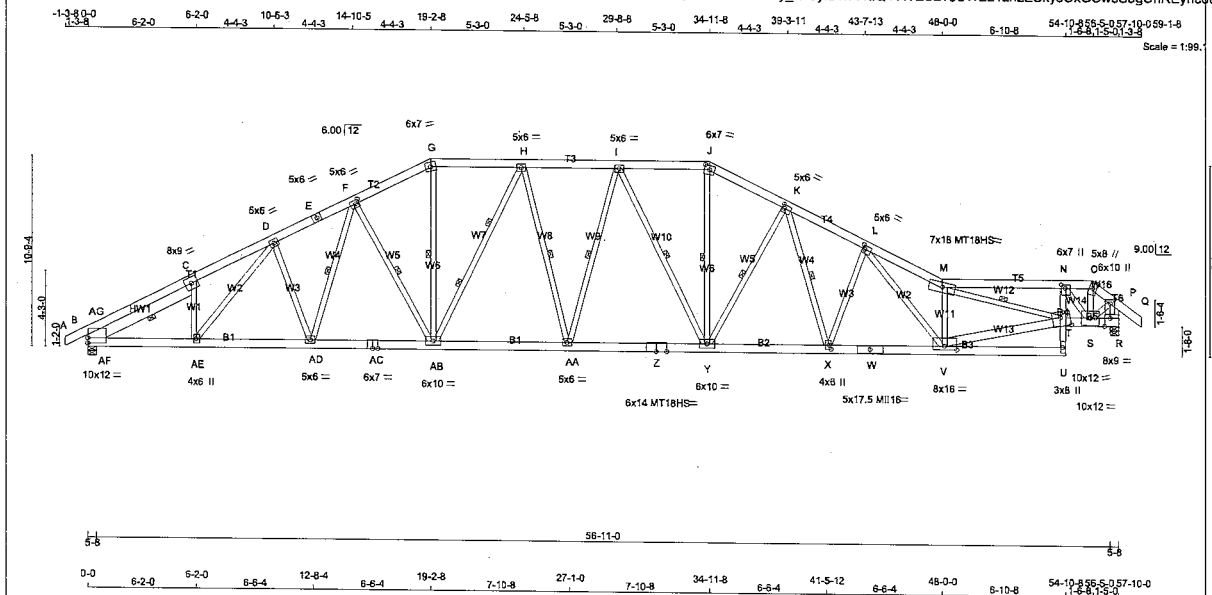
CHORDS	FACTORED	WEBS	FACTORED
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0/1	A-F	0/291
B-AH	-5719/182	C-AE	-391/177
AH-C	-3823/239	A-E	0/442
C-D	-7677/289	E-AC	-1272/217
D-E	-7677/289	AC-F	-81/1209
E-F	-6951/274	Y-J	-146/2126
F-G	-7143/248	Y-K	-2173/251
G-AI	-7144/248	X-K	-83/1678
AI-H	-7144/248	X-L	-1872/180
H-I	-7144/248	V-L	-2856/123
I-J	-7347/288	V-M	-81/3884
J-K	-7594/344	U-M	-2075/142
K-L	-8593/379	T-R	0/112
L-M	-8912/416	R-N	-64/2843
M-N	-4504/267	R-O	-125/4493
N-O	-5437/267	U-R	-220/5778
O-P	0/53	M-R	-2409/123
Q-O	-4855/269	AA-J	-65/1329
		F-AB	-81/1850
		AA-I	-894/137
		AB-G	-997/122
		AB-I	-1324/786
		AG-AH	0/2506
		AG-C	-4389/65

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_s BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.





LUMBER										TOTAL WEIGHT = 413 lb														
N. L. G. A. RULES										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY														
CHORDS										BUILDING DESIGNER														
SIZE										BEARINGS														
LUMBER										DESIGN CRITERIA														
DESCR.										SPECIFIED LOADS:														
A - E	2x6	DRY	No.2	SPF	FACTORED					MAXIMUM FACTORED					INPUT					REQD				
E - G	2x6	DRY	No.2	SPF	GROSS REACTION					GROSS REACTION					BRG					BRG				
G - J	2x6	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX						TOP CH. DL = 35.1 PSF						
J - M	2x6	DRY	No.2	SPF	R	4785	0	4785	183	-356	5-8	5-8						BOT CH. LL = 10.5 PSF						
M - O	2x6	DRY	No.2	SPF	R	4794	0	4794	0	-377	5-8	5-3						DL = 7.4 PSF						
O - Q	2x6	DRY	No.2	SPF	PROVIDE ANCHORAGE AT BEARING JOINT B FOR 355 LBS FACTORED UPLIFT										TOTAL LOAD = 59.0 PSF									
Q - R	2x6	DRY	No.2	SPF	PROVIDE ANCHORAGE AT BEARING JOINT R FOR 377 LBS FACTORED UPLIFT										SPACING = 24.0 IN. C/C									
R - P	2x6	DRY	No.2	SPF	PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT B										LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 9.00/12									
S - AC	2x6	DRY	2100F 1.8E	SPF	UNFACTORED REACTIONS										THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015									
AC - Z	2x6	DRY	2100F 1.8E	SPF	1ST CASE										MAX/MIN COMPONENT REACTIONS									
Z - W	2x6	DRY	2100F 1.8E	SPF	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL												
W - U	2x6	DRY	2100F 1.8E	SPF	R	3525	2126 / 0	607 / 0	0 / 0	193 / -753	791 / 0	0 / 0												
U - N	2x4	DRY	No.2	SPF	B	3530	2131 / 0	607 / 0	0 / 0	170 / -779	792 / 0	0 / 0												
T - R	2x6	DRY	No.2	SPF	HORIZONTAL REACTIONS										THIS DESIGN COMPLIES WITH:									
										B										- PART 4 OF CBC 2018, ABC 2019				
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, R										- PART 4 OF CBC 2012 (2019 AMENDMENT)				
																				- CSA 086-14				

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBW1-I	MT20	10.0	12.0	3.50	
C TMBW1-I	MT20	8.0	9.0		
D H-I					
E TMBW1-I	MT20	5.0	6.0		
F TMBW1-I	MT20	5.0	6.0	2.50	2.50
G TMBW1-I	MT20	6.0	7.0		
J TMBW1-I	MT20	6.0	7.0	2.75	3.50
K TMBW1-I	MT20	5.0	6.0	2.50	1.50
L TMBW1-I	MT20	5.0	6.0	2.50	1.75
M TMBW1-I	MT18HS	7.0	18.0		
N TMBW1-I	MT20	6.0	7.0	2.50	2.75
O TMBW1-I	MT20	5.0	6.0	Edge	
P TMBW1-I	MT20	6.0	10.0	2.75	2.75
R TMBW1-I	MT20	8.0	9.0	Edge	3.50
S TMBW1-I	MT20	10.0	12.0		
T TMBW1-I	MT20	10.0	12.0	6.75	6.00
U TMBW1-I	MT20	3.0	8.0		
V TMBW1-I	MT20	8.0	16.0	2.50	8.00
W BS-I	MT18	5.0	17.5		
X TMBW1-I	MT20	6.0	6.0	2.75	1.25
Y TMBW1-I	MT20	6.0	10.0		
Z BS-I	MT18HS	6.0	14.0		
AA TMBW1-I	MT20	5.0	6.0		
AB TMBW1-I	MT20	6.0	10.0		
AC BS-I	MT20	6.0	7.0		
AD TMBW1-I	MT20	5.0	6.0		
AE TMBW1-I	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

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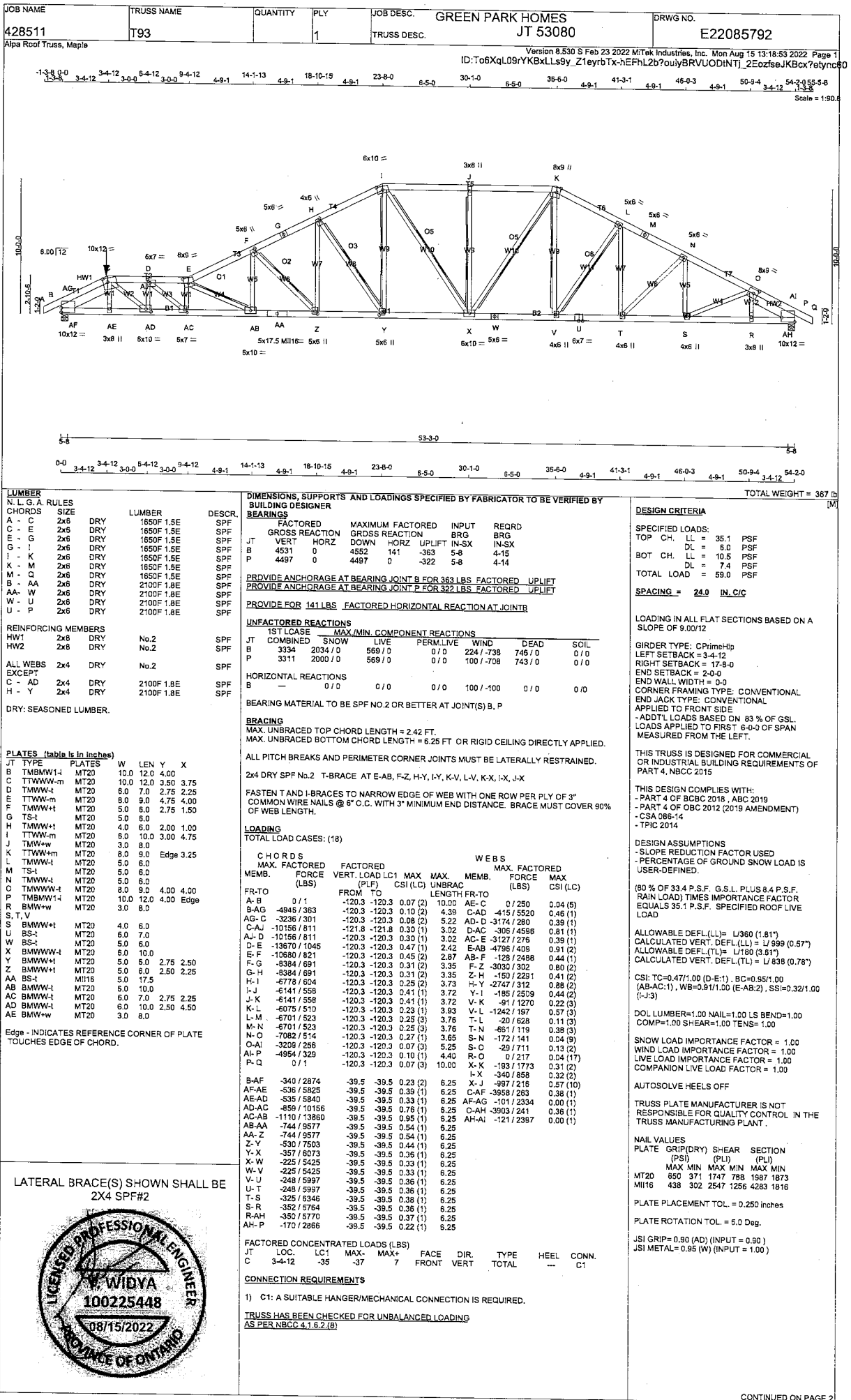
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[illegible]



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428511	T93		1	GREEN PARK HOMES JT 53080	E22085792(2)
Alpa Roof Truss, Maple					

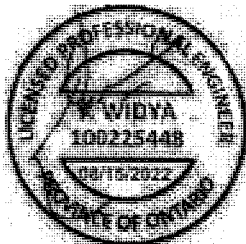
Version 8.530 S Feb 23 2022 Mittek Industries, Inc. Mon Aug 15 13:18:53 2022 Page 2
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CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2

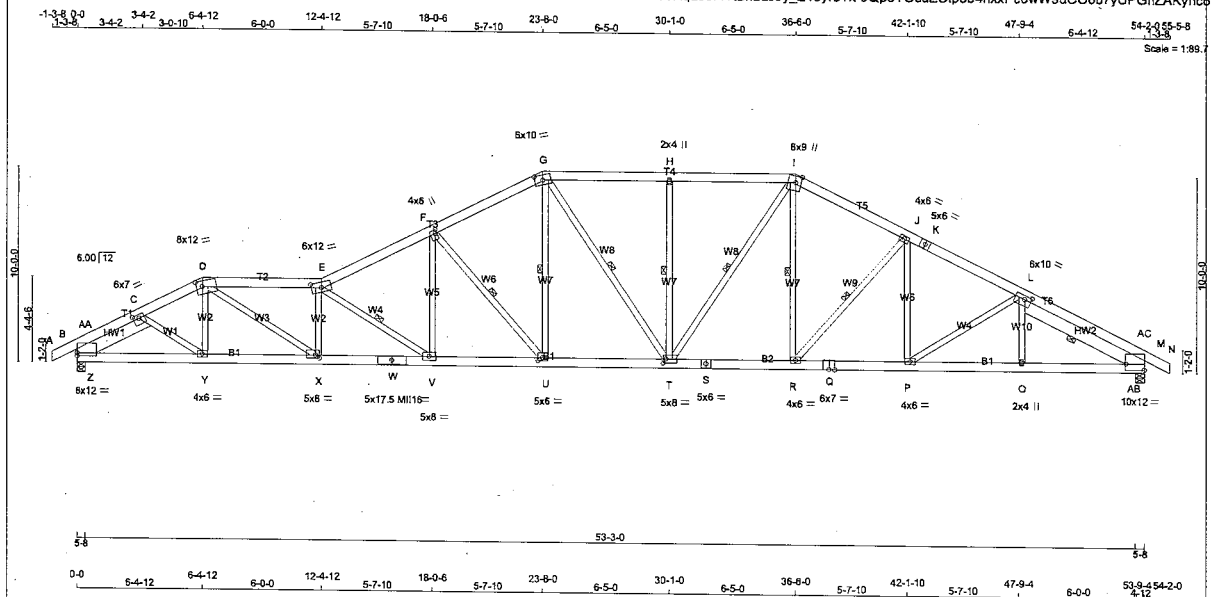


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	
428511	T94		1	TRUSS DESC.	JT 53080	E22085793	

Alpa Roof Truss, Maple

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LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x6	DRY	No.2	SPF
E	2x6	DRY	1650F 1.5E	SPF
F - G	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
J - K	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF
O - W	2x6	DRY	2100F 1.8E	SPF
X - S	2x6	DRY	2100F 1.8E	SPF
T - M	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x8	DRY	No.2
HW2	2x8	DRY	No.2
ALL WEBS EXCEPT F - U	2x4	DRY	2100F 1.8E

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TBMWW-1	MT20	8.0	12.0	2.00
C TBMWW-1	MT20	8.0	7.0	2.25 3.50
D TTMWW-1	MT20	8.0	12.0	3.25 3.75
E TTMWW-1	MT20	6.0	12.0	3.25 6.00
F TTMWW-1	MT20	4.0	6.0	2.25 1.00
G TTMWW-1	MT20	6.0	10.0	3.00 4.75
H TTMWW-1	MT20	2.0	4.0	
I TTMWW-1	MT20	8.0	9.0	Edge 3.25
J TTMWW-1	MT20	4.0	6.0	
K TS-1	MT20	5.0	6.0	
L TBMWW-1	MT20	6.0	10.0	2.25 4.00
M TBMWW-1	MT20	10.0	12.0	3.75 Edge
O BMWW-1	MT20	2.0	4.0	
P, R, Y				
P BMWW-1	MT20	4.0	6.0	
Q BS-1	MT20	6.0	7.0	
S BS-1	MT20	5.0	6.0	
T BMWW-1	MT20	5.0	8.0	2.50 1.75
U BMWW-1	MT20	5.0	6.0	2.00 2.75
V BMWW-1	MT20	5.0	8.0	
W BS-1	MT16	5.0	17.5	
X BMWW-1	MT20	5.0	8.0	2.00 2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT HORIZ	VERT HORIZ	UPLIFT IN-SX	IN-SX
B	4566 0	4566 141	-268 5-8	4-15
M	4584 0	4584 0	-211 5-8	4-15

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 268 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 211 LBS FACTORED UPLIFT

PROVIDE FOR 141 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED		
B	3357	1998 / 0
M	3381	1998 / 0

HORIZONTAL REACTIONS
B 0 / 0
M 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M

BRACING

FOR SECTION G-I, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-V, F-U, G-U, G-T, H-T, I-T, I-R, J-R, L-AB.

LOADING

TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC	MEMB.	MAX. FACTORED
FR-TO									
A-B	0 / 1	-120.3	-120.3	0.09 (2)	10.00	C-Y	0 / 832	0.13 (1)	
B-AA	-5031 / 215	-120.3	-120.3	0.16 (1)	3.81	Y-D	-175 / 104	0.03 (8)	
AA-C	-3279 / 196	-120.3	-120.3	0.10 (1)	4.63	D-X	-190 / 5086	0.82 (1)	
C-D	-7229 / 408	-120.3	-120.3	0.33 (1)	3.06	X-E	-2720 / 187	0.48 (1)	
D-E	-10890 / 545	-120.3	-120.3	0.54 (2)	2.81	E-V	-3379 / 255	0.76 (2)	
E-F	-8940 / 432	-120.3	-120.3	0.75 (1)	2.38	V-F	-71 / 2189	0.35 (2)	
F-G	-6977 / 352	-120.3	-120.3	0.57 (1)	2.82	F-U	-2937 / 306	0.67 (2)	
G-H	-6338 / 292	-132.8	-132.8	0.68 (1)	2.00	U-G	-152 / 2462	0.40 (2)	
H-I	-6338 / 292	-132.8	-132.8	0.68 (1)	2.00	G-T	-660 / 921	0.46 (2)	
I-J	-6262 / 270	-120.3	-120.3	0.50 (1)	3.17	T-H	-1026 / 87	0.45 (1)	
J-K	-6965 / 293	-120.3	-120.3	0.57 (1)	2.95	H-I	-126 / 1766	0.29 (2)	
K-L	-6965 / 293	-120.3	-120.3	0.57 (1)	2.95	R-I	-84 / 1237	0.20 (3)	
L-AC	-3598 / 243	-120.3	-120.3	0.27 (3)	4.31	R-J	-1305 / 215	0.48 (3)	
AC-M	-5348 / 176	-120.3	-120.3	0.32 (1)	3.55	P-J	0 / 507	0.08 (3)	
M-N	0 / 1	-120.3	-120.3	0.09 (3)	10.00	P-L	-516 / 142	0.31 (3)	
B-Z	-252 / 2914	-39.5	-39.5	0.20 (1)	6.25	Z-AA	-40 / 2397	0.05 (17)	
Z-Y	-421 / 5789	-39.5	-39.5	0.36 (1)	6.25	Z-C	-3861 / 228	0.34 (1)	
Y-X	-379 / 6455	-39.5	-39.5	0.39 (1)	6.25	L-AB	-3985 / 56	0.47 (1)	
X-W	-544 / 10792	-39.5	-39.5	0.62 (1)	6.25	AB-AC	0 / 2283	0.00 (1)	
W-V	-544 / 10792	-39.5	-39.5	0.62 (1)	6.25				
V-U	-334 / 8032	-39.5	-39.5	0.42 (1)	6.25				
U-T	-136 / 6241	-39.5	-39.5	0.34 (1)	6.25				
T-S	0 / 5596	-39.5	-39.5	0.31 (1)	10.00				
S-R	0 / 5596	-39.5	-39.5	0.31 (1)	10.00				
R-Q	-62 / 6266	-39.5	-39.5	0.34 (1)	6.25				
Q-P	-62 / 6266	-39.5	-39.5	0.34 (1)	6.25				
P-O	-173 / 6497	-39.5	-39.5	0.36 (1)	6.25				
O-AB	-171 / 6501	-39.5	-39.5	0.37 (1)	6.25				
AB-M	-125 / 3215	-39.5	-39.5	0.17 (1)	6.25				

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2 (B)WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} C_{pi} BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT IN-SX AWAY FROM EAVE TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 35.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 56.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.81)
CALCULATED VERT. DEFL.(LL) = U/999 (0.47)
ALLOWABLE DEFL.(TL) = L/800 (3.61)
CALCULATED VERT. DEFL.(TL) = U/978 (0.86)

CSI: TD=0.75/1.00 (E-F:1), BC=0.52/1.00 (V-X:1), WB=0.52/1.00 (D-X:1), SS=0.33/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

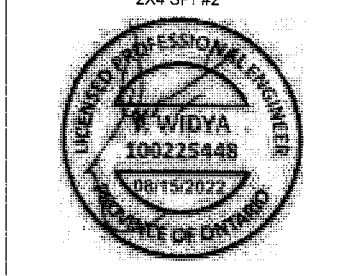
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
MT16 438 302 2547 1256 4293 1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.99 (S) (INPUT = 1.00)

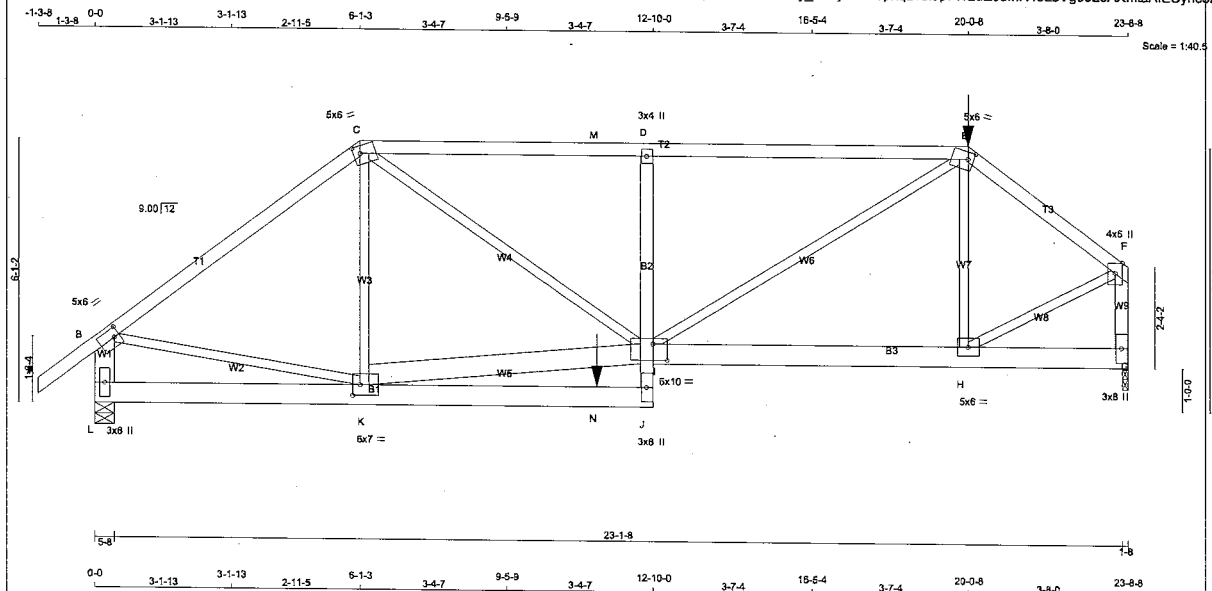
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085794
428511	T95		2	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - I	2x6	DRY	No.2	SPF

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	SIDE(45.0)
E-F	12	SIDE(59.4)
G-F	12	TOP
L-B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-J	12	SIDE(4.9)
I-G	12	SIDE(4.9)
J-D	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

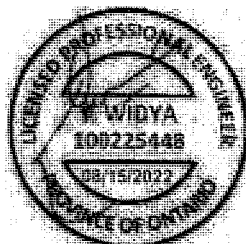
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-4	MT20	5.0	6.0	2.50	1.50
C TTWW-m	MT20	5.0	6.0	2.00	1.75
D TMV+p	MT20	3.0	4.0		
E TTWW-m	MT20	5.0	6.0	2.00	1.50
F TMWV+p	MT20	4.0	6.0	Edge	
G BMV1+p	MT20	3.0	8.0		
H BMWV-4	MT20	5.0	6.0		
I BVWVWV-4	MT20	6.0	10.0	4.50	3.75
J BVV+p	MT20	3.0	8.0		
K BMWVWV-4	MT20	6.0	7.0	3.00	2.00
L BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	BRG	BRG
L	2036	0	2036	0
G	2158	0	2158	0

UNFACTORED REACTIONS	1ST LCASE	MAX MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	1435	970 / 0 0 / 0 0 / 0 0 / 0 465 / 0 0 / 0
G	1524	1012 / 0 0 / 0 0 / 0 0 / 0 512 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO				
A-B	0 / 39	-95.2	-95.2	0.07 (1)	10.00	K-C	-223 / 39	0.06 (1)	
B-C	-2154 / 0	-95.2	-95.2	0.41 (1)	5.55	K-I	0 / 1674	0.11 (1)	
C-M	-3133 / 0	-95.2	-95.2	0.67 (1)	4.35	C-I	0 / 1740	0.22 (1)	
M-D	-3133 / 0	-139.8	-139.8	0.67 (1)	4.35	I-E	0 / 1908	0.24 (1)	
D-E	-3146 / 0	-139.8	-139.8	0.71 (1)	4.33	H-E	-593 / 0	0.11 (1)	
E-F	-1945 / 0	-95.2	-95.2	0.14 (1)	6.13	B-K	0 / 1748	0.22 (1)	
L-B	-2025 / 0	0.0	0.0	0.07 (1)	7.81	H-F	0 / 1744	0.22 (1)	
G-F	-2141 / 0	0.0	0.0	0.14 (1)	7.58				
L-K	0 / 0	-18.5	-18.5	0.11 (1)	10.00				
K-N	0 / 62	-18.5	-18.5	0.27 (1)	10.00				
N-J	0 / 82	-27.2	-27.2	0.27 (1)	10.00				
J-I	0 / 837	0.0	0.0	0.10 (1)	10.00				
I-D	-1066 / 0	0.0	0.0	0.12 (1)	7.81				
I-H	0 / 1539	-27.2	-27.2	0.15 (1)	10.00				
H-G	0 / 0	-27.2	-27.2	0.09 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
E	20-0-8	-138	-138		FRONT	VERT	TOTAL		C1
N	11-6-8	-744	-744		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-1-3
RIGHT SETBACK = 3-8-0
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADDTL LOADS BASED ON 55 % OF G.S.L.
LOADS APPLIED TO FIRST 12-2-0 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/899 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/899 (0.13")

CSI: TC=0.71/1.00 (D-E:1), BC=0.27/1.00 (J-K:1), WB=0.24/1.00 (E-I:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

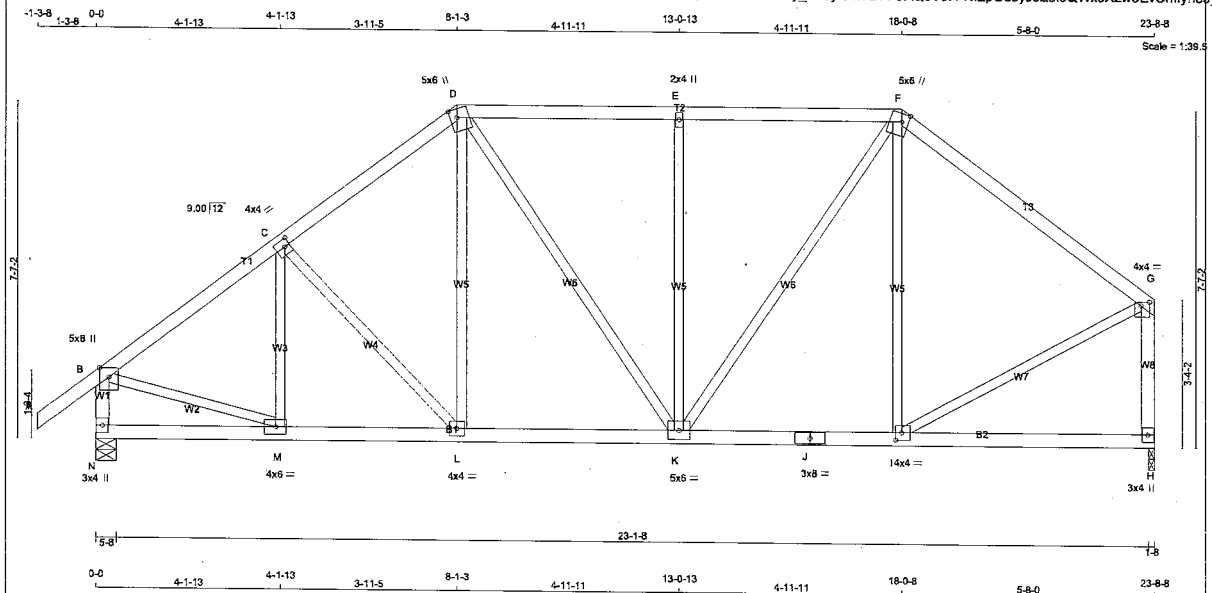
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL. IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI: GRIP= 0.81 (B) (INPUT = 0.90)
JSI: METAL= 0.45 (F) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TMVW+m	MT20	5.0	6.0	2.25	1.75
E TMVW-w	MT20	2.0	4.0		
F TMVW+m	MT20	5.0	6.0	2.25	1.75
G TMVW-p	MT20	4.0	4.0	1.00	2.25
H BMV1+p	MT20	3.0	4.0		
I BMVW-t	MT20	4.0	4.0	2.00	1.75
J BS-1	MT20	3.0	6.0		
K BMVW-t	MT20	5.0	6.0		
L BMVW-t	MT20	4.0	4.0		
M BMVW-t	MT20	4.0	4.0		
N BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	HORZ	UPLIFT	IN-SX
JT	1479	0	5-8
N	1479	0	5-8
H	1348	0	1-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
N	1042	708 / 0 0 / 0 0 / 0 0 / 0 334 / 0 0 / 0
H	951	634 / 0 0 / 0 0 / 0 0 / 0 318 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 39	-95.2 -95.2 0.13 (1)	10.00	M-C	-245 / 0	0.08 (1)	
B-C	-1372 / 0	-95.2 -95.2 0.22 (1)	5.33	C-L	-190 / 0	0.11 (1)	
C-D	-1266 / 0	-95.2 -95.2 0.21 (1)	5.49	L-D	0 / 226	0.05 (1)	
D-E	-1124 / 0	-95.2 -95.2 0.31 (1)	5.51	D-K	0 / 235	0.05 (1)	
E-F	-1124 / 0	-95.2 -95.2 0.31 (1)	5.51	K-E	-578 / 0	0.61 (1)	
F-G	-1016 / 0	-95.2 -95.2 0.41 (1)	5.67	K-F	0 / 563	0.13 (1)	
N-B	-1445 / 0	0.0 0.0 0.15 (1)	6.80	I-F	-308 / 0	0.32 (1)	
H-G	-1305 / 0	0.0 0.0 0.25 (1)	7.07	B-M	0 / 1164	0.26 (1)	
				I-G	0 / 915	0.21 (1)	
N-M	0 / 0	-18.5 -18.5 0.06 (4)	10.00				
M-L	0 / 1118	-18.5 -18.5 0.22 (1)	10.00				
L-K	0 / 992	-18.5 -18.5 0.20 (1)	10.00				
K-J	0 / 807	-18.5 -18.5 0.21 (1)	10.00				
J-I	0 / 807	-18.5 -18.5 0.21 (1)	10.00				
I-H	0 / 0	-18.5 -18.5 0.14 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. ROOF LIVE LOAD)

ALLOWABLE DEFL. (LL) = L/360 (0.79")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.79")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.41/1.00 (F-G:1), BC=0.22/1.00 (L-M:1), WB=0.61/1.00 (E-K:1), SS=0.23/1.00 (D-E:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

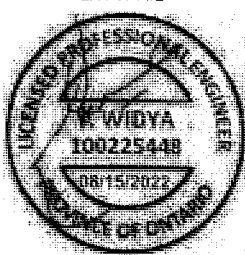
PLATE PLACEMENT TOL. = 0.250 inches

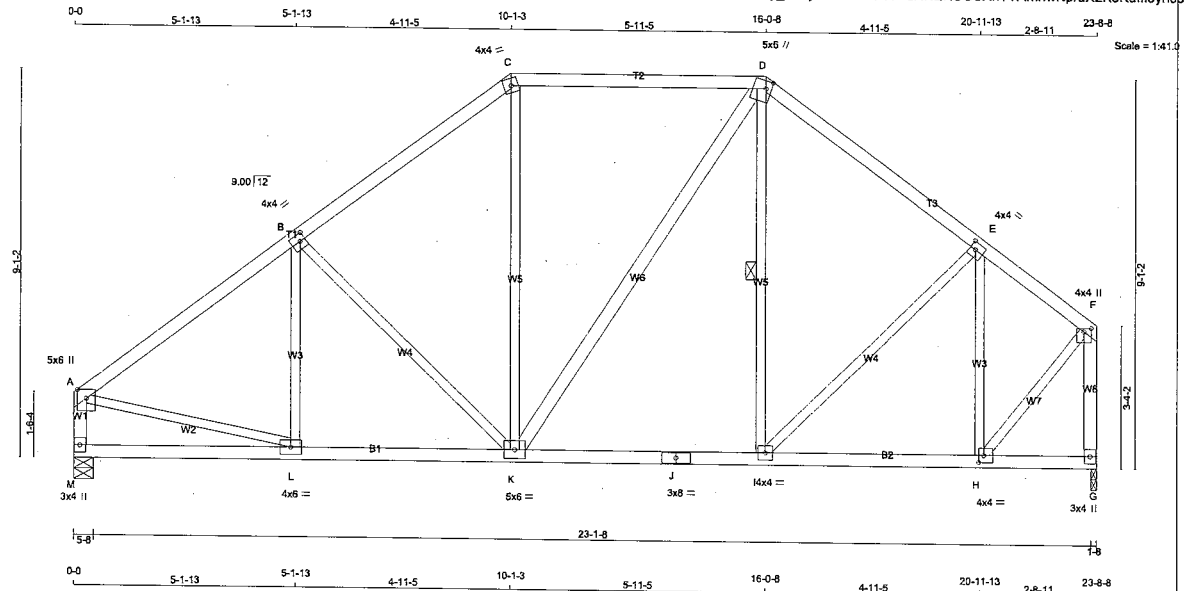
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)

JSI METAL= 0.55 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMWW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
M	VERT	DOWN	UPLIFT	IN-SX
G	1348	0	1348	0
	0	1348	0	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	951	634 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
G	951	634 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	-1391 / 0	-95.2	-95.2 0.34 (1)	5.14	L-B	-164 / 29	0.07 (1)
B-C	-1151 / 0	-95.2	-95.2 0.33 (1)	5.54	B-K	-353 / 0	0.35 (1)
C-D	-893 / 0	-95.2	-95.2 0.44 (1)	5.88	K-C	0 / 212	0.05 (4)
D-E	-1030 / 0	-95.2	-95.2 0.28 (1)	5.85	K-D	0 / 165	0.03 (1)
E-F	-914 / 0	-95.2	-95.2 0.23 (1)	6.25	I-D	-9 / 74	0.03 (4)
M-A	-1307 / 0	0.0	0.0 0.14 (1)	7.06	I-E	0 / 181	0.04 (1)
G-F	-1331 / 0	0.0	0.0 0.25 (1)	7.02	H-E	-687 / 0	0.30 (1)
					A-L	0 / 1170	0.26 (1)
M-L	0 / 0	-18.5	-18.5 0.11 (4)	10.00	H-F	0 / 1027	0.23 (1)
L-K	0 / 1140	-18.5	-18.5 0.24 (1)	10.00			
K-J	0 / 802	-18.5	-18.5 0.19 (1)	10.00			
J-I	0 / 802	-18.5	-18.5 0.19 (1)	10.00			
I-H	0 / 688	-18.5	-18.5 0.17 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA D86-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = U/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = U/999 (0.07")

CS1: TC=0.44/1.00 (C-D-1), BC=0.24/1.00 (K-L-1), WB=0.35/1.00 (B-K-1), SS=0.22/1.00 (C-O-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(ORY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

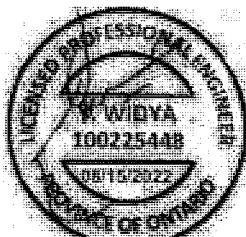
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)

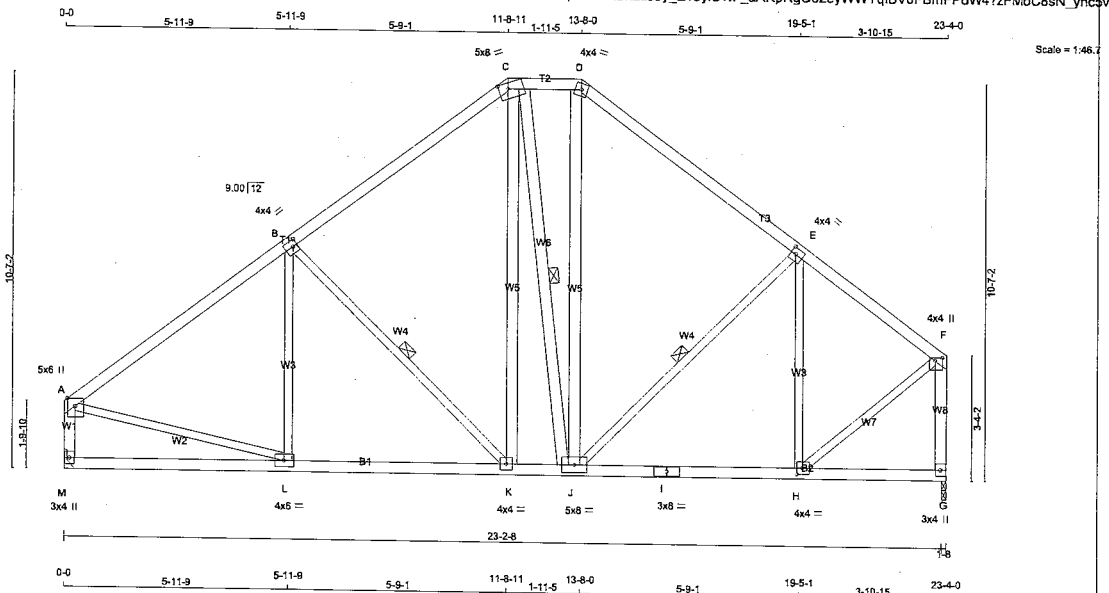
JSI METAL= 0.54 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - C	2x4	DRY	No.2
C - J	2x4	DRY	No.2
J - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	8.0	Edge
B	TMWW-t	MT20	4.0	4.0	2.00 1.50
C	TTWW-m	MT20	5.0	8.0	1.75 3.00
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.50
F	TMWW+p	MT20	4.0	4.0	1.00 2.25
G	BMV1+p	MT20	3.0	4.0	
H	BMWW-t	MT20	4.0	4.0	2.00 1.75
I	BS-t	MT20	3.0	8.0	
J	BMWWW-t	MT20	5.0	8.0	
K	BMWW-t	MT20	4.0	4.0	
L	BMWW-t	MT20	4.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
M	1327 0	1327 0	0	IN-SX	
G	1327 0	1327 0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
M	936	624 / 0	0 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
G	936	624 / 0	0 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, C-J, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-1295 / 0	L-B	-136 / 57
B-C	-980 / 0	B-K	-453 / 0
C-D	-737 / 0	C-K	0 / 368
D-E	-950 / 0	C-J	-103 / 0
E-F	-933 / 0	J-D	0 / 238
F-G	-1282 / 0	J-E	-71 / 0
G-F	-1301 / 0	H-E	-504 / 0
		A-L	0 / 1101
		H-F	0 / 989
M-L	0 / 0		
L-K	0 / 1068		
K-J	0 / 755		
J-I	0 / 783		
I-H	0 / 783		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (0.78")
CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.03")
ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (0.78")
CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.07")

CSI: TC=0.47/1.00 (A-B-1), BC=0.26/1.00 (K-L-1), WB=0.32/1.00 (E-H-1), SS=0.22/1.00 (A-B-1)
DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.52 (A) (INPUT = 1.00)

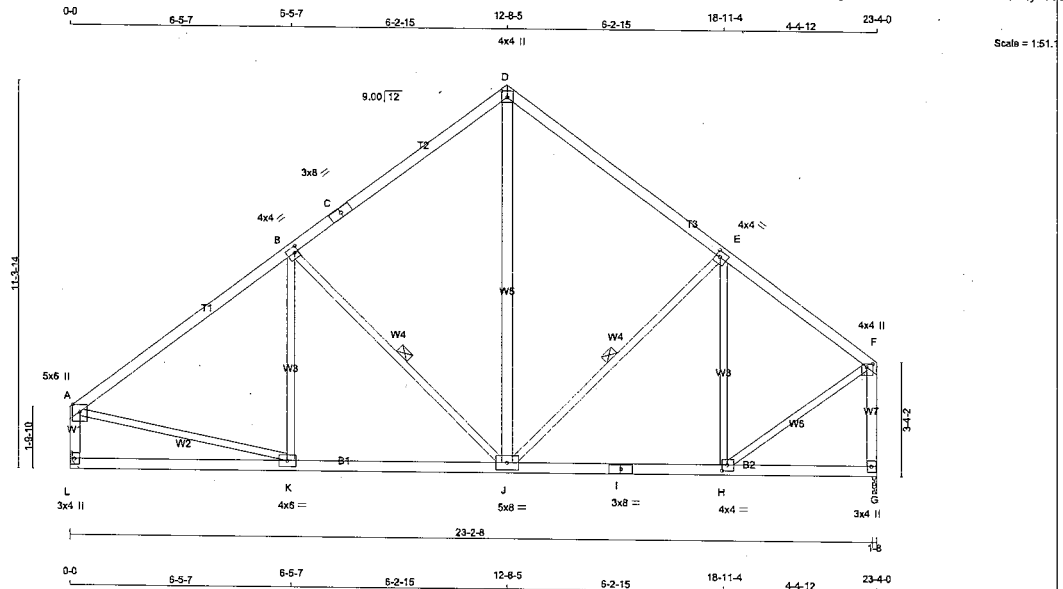
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T99A	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085798
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Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-Smkj0nh0vMmp8g61sv1FoPJPI1sFkPJW0stQvQyncs



Scale = 1/51

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT	2x4	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge
B	TMWW-i	MT20	4.0	4.0	2.00 1.50
C	TS-i	MT20	3.0	8.0	
D	TTW+p	MT20	4.0	4.0	2.25 2.00
E	TMWW-i	MT20	4.0	4.0	2.00 1.50
F	TMWW+p	MT20	4.0	4.0	1.00 2.25
G	BMV1+p	MT20	3.0	4.0	
H	BMWW-i	MT20	4.0	4.0	2.00 1.75
I	BS-i	MT20	3.0	8.0	
J	BMWWW-i	MT20	5.0	8.0	
K	BMWW-i	MT20	4.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	VERT HORZ	DOWN HORZ	UPLIFT	IN-SX
G	1327 0	1327 0	0 0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	936	624 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
G	936	624 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-J, E-J.

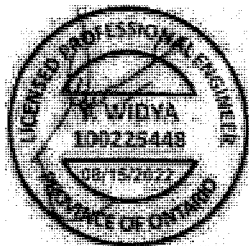
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE	(PLF)	CS1 (LC)	UNBRAC	MEMB.	FORCE	MAX	CS1 (LC)
FR-TO	(LBS)	FROM TO	LENGTH	FR-TO	(LBS)	CS1 (LC)		
A-B	-1288 / 0	-95.2	-95.2	0.56 (1)	4.95	K-B	-114 / 68	0.08 (1)
B-C	-925 / 0	-95.2	-95.2	0.52 (1)	5.67	B-J	-518 / 0	0.28 (1)
C-D	-925 / 0	-95.2	-95.2	0.52 (1)	5.67	J-D	0 / 594	0.10 (1)
D-E	-914 / 0	-95.2	-95.2	0.46 (1)	5.82	J-E	-157 / 0	0.08 (1)
E-F	-969 / 0	-95.2	-95.2	0.39 (1)	5.80	H-E	-451 / 0	0.33 (1)
L-A	-1277 / 0	0.0	0.0	0.14 (1)	7.13	A-K	0 / 1094	0.25 (1)
G-F	-1297 / 0	0.0	0.0	0.25 (1)	7.09	H-F	0 / 987	0.22 (1)
L-K	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
K-J	0 / 1096	-18.5	-18.5	0.26 (1)	10.00			
J-I	0 / 814	-18.5	-18.5	0.21 (1)	10.00			
I-H	0 / 814	-18.5	-18.5	0.21 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2018

- PART 9 OF CBC 2012 (2015 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.

RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED

ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")

CALCULATED VERT. DEFL.(LL) = L/999 (0.03")

ALLOWABLE DEFL.(TL) = L/360 (0.78")

CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.56/1.00 (A-B:1), BC=0.28/1.00 (J-K:1),

WB=0.33/1.00 (E-H:1), SS=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN THE

TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

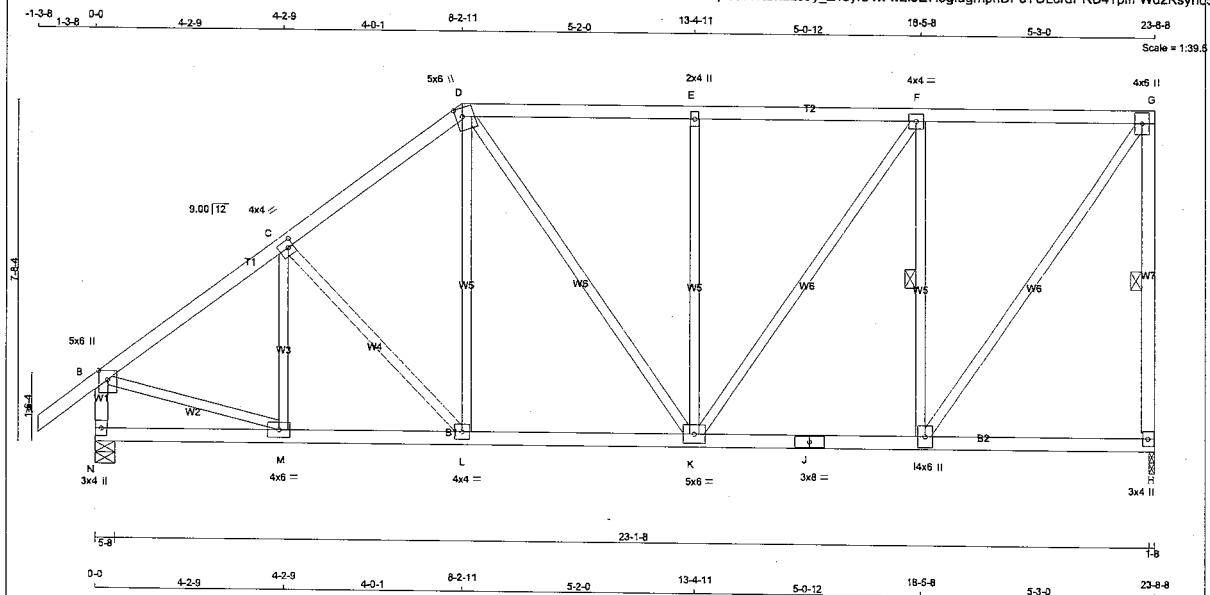
JSI GRIP = 0.87 (F) (INPUT = 0.90)

JSI METAL = 0.52 (A) (INPUT = 1.00)

JOB NAME 428511	TRUSS NAME T100	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085799
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Alpha Roof Truss, Maple

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ID: To6XqL09rYKBxLLs9y_Z1eyrbTx-wzI5E7iegfugmphDPcYULcrdPRD4TpfFWdzReyndSt



LUMBER	N. L. O. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (Table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWW+P	MT20	5.0	6.0	Edge	
C	TMWW+I	MT20	4.0	4.0	2.00	1.50
D	TTWW+M	MT20	5.0	6.0	2.25	1.75
E	TMWW+W	MT20	2.0	4.0		
F	TMWW+I	MT20	4.0	4.0		
G	TMWW+P	MT20	4.0	6.0		
H	BMV1+P	MT20	3.0	4.0		
I	BMWW+I	MT20	4.0	6.0		
J	BS-I	MT20	3.0	6.0		
K	BMWW+I	MT20	5.0	6.0		
L	BMWW+I	MT20	4.0	6.0		
M	BMWW+I	MT20	4.0	6.0		
N	BMV1+P	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	BRG	BRG
H	1348	0	0	1-8
N	1479	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
H	951	634 / 0	0 / 0	0 / 0
N	1042	708 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

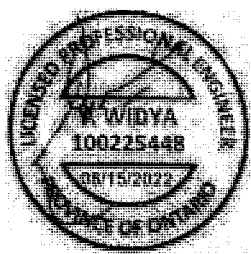
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	MEMB.	CHORDS	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX.	UNBRAC	FR-TO	MEMB.	MAX. FACTORED	FORCE	MAX
TOTAL LOAD CASES: (4)														
	A-B	0 / 39	-95.2	-95.2	0.13	(1)	10.00	M-C	-239 / 0	0.08	(1)			
	B-C	-1374 / 0	-95.2	-95.2	0.30	(1)	5.19	C-L	-200 / 0	0.12	(1)			
	C-D	-1260 / 0	-95.2	-95.2	0.30	(1)	5.37	L-D	0 / 235	0.05	(1)			
	D-E	-1098 / 0	-95.2	-95.2	0.36	(1)	5.55	B-M	0 / 1164	0.26	(1)			
	E-F	-1098 / 0	-95.2	-95.2	0.36	(1)	5.52	I-G	0 / 1356	0.31	(1)			
	F-G	-785 / 0	-95.2	-95.2	0.36	(1)	6.25	D-K	0 / 184	0.04	(1)			
	H-G	-1308 / 0	0.0	0.0	0.35	(1)	5.65	I-F	-997 / 0	0.37	(1)			
	N-B	-1444 / 0	0.0	0.0	0.15	(1)	6.80	K-E	-527 / 0	0.57	(1)			
	N-M	0 / 0	-18.5	-18.5	0.07	(4)	10.00	K-F	0 / 553	0.12	(1)			
	M-L	0 / 1120	-18.5	-18.5	0.23	(1)	10.00							
	L-K	0 / 986	-18.5	-18.5	0.20	(1)	10.00							
	K-J	0 / 785	-18.5	-18.5	0.19	(1)	10.00							
	J-I	0 / 785	-18.5	-18.5	0.19	(1)	10.00							
	I-H	0 / 0	-18.5	-18.5	0.12	(4)	10.00							

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")
CSI: TC=0.38/1.00 (E-F:1), BC=0.23/1.00 (L-M:1), WB=0.57/1.00 (E-K:1), SS=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	798	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

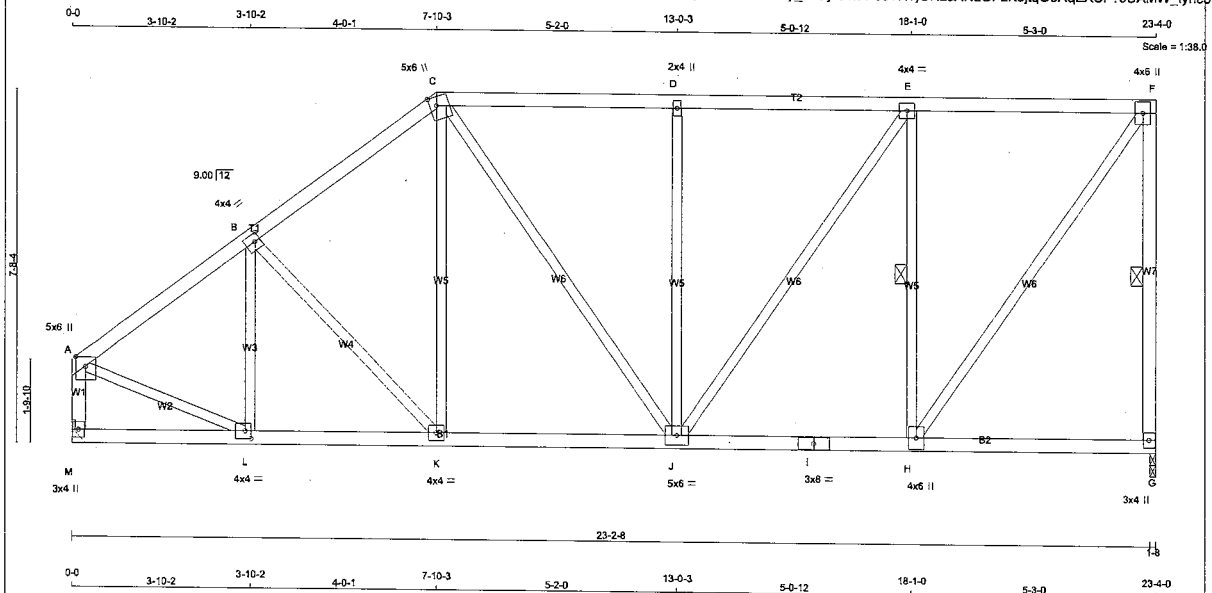
JSI GRIP= 0.84 (M) (INPUT = 0.90)

JSI METAL= 0.58 (B) (INPUT = 1.00)

JOB NAME 428511	TRUSS NAME T100A	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085800
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Alpa Roof Truss, Maple

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ID:To6XqL09rYKbLs9y_Z1eyrbTx-P9sTR7jGRz0XNzGPzK3JlqOoAqZXCf?oUAMW_lync5s



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge
B	TMWW-i	MT20	4.0	4.0	2.00 1.50
C	TTWW+m	MT20	5.0	6.0	2.25 1.75
D	TMWW-w	MT20	2.0	4.0	
E	TMWW-i	MT20	4.0	4.0	
F	TMWW+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWW+i	MT20	4.0	6.0	
I	BS-i	MT20	3.0	8.0	
J	BMWWW-i	MT20	5.0	6.0	
K	BMWW-i	MT20	4.0	4.0	
L	BMWW-i	MT20	4.0	4.0	2.00 1.50
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	BRG	BRG
G	1327 0	1327 0	0 0	1-8 1-8
M	1327 0	1327 0	0 0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	936	624 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
M	936	624 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MAX. FACTORED CSI (LC)
A-B	-1249 / 0	-95.2	-95.2 0.27 (1)	5.43	K-C	0 / 177	0.04 (4)
B-C	-1203 / 0	-95.2	-95.2 0.27 (1)	5.51	L-B	-318 / 0	0.10 (1)
C-D	-1068 / 0	-95.2	-95.2 0.36 (1)	5.52	B-K	-119 / 0	0.07 (1)
D-E	-1068 / 0	-95.2	-95.2 0.37 (1)	5.58	A-L	0 / 1093	0.26 (1)
E-F	-770 / 0	-95.2	-95.2 0.36 (1)	6.25	H-F	0 / 1330	0.30 (1)
G-F	-1287 / 0	0.0	0.0 0.34 (1)	5.69	C-J	0 / 220	0.08 (1)
M-A	-1295 / 0	0.0	0.0 0.14 (1)	7.10	H-E	-976 / 0	0.36 (1)
					J-D	-527 / 0	0.57 (1)
					J-E	0 / 527	0.12 (1)
M-L	0 / 0	-18.5	-18.5 0.06 (4)	10.00			
L-K	0 / 1021	-18.5	-18.5 0.21 (1)	10.00			
K-J	0 / 942	-18.5	-18.5 0.22 (1)	10.00			
J-I	0 / 770	-18.5	-18.5 0.19 (1)	10.00			
I-H	0 / 770	-18.5	-18.5 0.19 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.37/1.00 (D-E:1), BC=0.21/1.00 (K-L:1), WB=0.57/1.00 (D-J:1), SI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

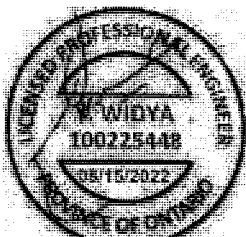
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

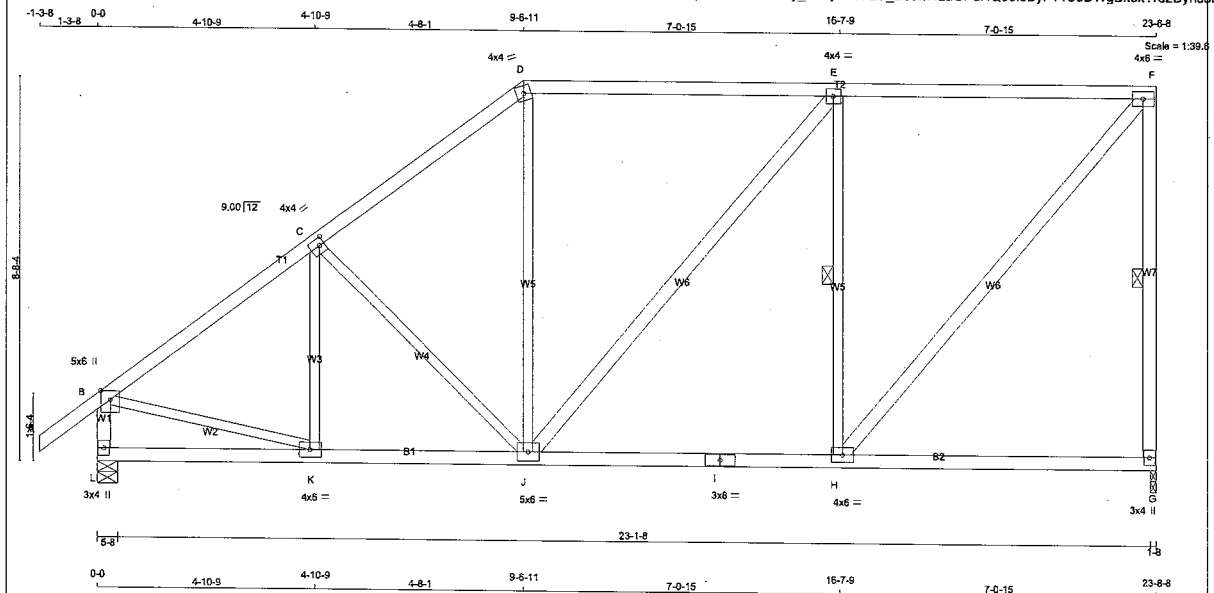
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (L) (INPUT = 0.90)
JSI METAL= 0.48 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER N.L.G.A. RULES SIZE LUMBER DESCR. TOTAL WEIGHT = 119 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	SPF
D - F	2x4	DRY	SPF
G - F	2x4	DRY	SPF
L - B	2x4	DRY	SPF
L - I	2x4	DRY	SPF
I - G	2x4	DRY	SPF

ALL WEBS	SIZE	LUMBER	DESCR.
EXCEPT			
J - E	2x4	DRY	SPF
H - F	2x4	DRY	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE	MT20	5.0	8.0	Edge	
B TMVW+p	MT20	4.0	4.0	2.00	1.50
C TMVW-i	MT20	4.0	4.0		
D TMVW-m	MT20	4.0	4.0		
E TMVW-i	MT20	4.0	4.0		
F TMVW-i	MT20	4.0	4.0		
G BMV1+p	MT20	3.0	4.0		
H BMVW-i	MT20	4.0	4.0		
I SS-i	MT20	3.0	8.0		
J BMVW-i	MT20	5.0	6.0		
K BMVW-i	MT20	4.0	6.0		
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT GROSS REACTION				
G VERT	1348	0	1348	0
L VERT	1479	0	1479	0

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED			
G	951	634 / 0	0 / 0
L	1042	708 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED	MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED
TOTAL LOAD CASES: (4)								
	FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)
	A-B	0 / 39	-95.2	-95.2 0.13 (1)	10.00	K-C	-186 / 15	0.07 (1)
	B-C	-1388 / 0	-95.2	-95.2 0.43 (1)	5.01	C-J	-308 / 0	0.27 (1)
	C-D	-1186 / 0	-95.2	-95.2 0.41 (1)	5.33	J-D	0 / 273	0.06 (1)
	D-E	-823 / 0	-95.2	-95.2 0.89 (1)	4.49	J-E	0 / 66	0.01 (1)
	E-F	-881 / 0	-95.2	-95.2 0.89 (1)	4.59	H-E	-888 / 0	0.43 (1)
	F-G	-1295 / 0	0.0	0.0 0.45 (1)	5.58	H-F	0 / 1362	0.22 (1)
	L-B	-1440 / 0	0.0	0.0 0.15 (1)	6.81	B-K	0 / 1170	0.26 (1)
	L-K	0 / 0	-18.5	-18.5 0.09 (4)	10.00			
	K-J	0 / 1136	-18.5	-18.5 0.25 (1)	10.00			
	J-I	0 / 881	-18.5	-18.5 0.31 (4)	10.00			
	I-H	0 / 881	-18.5	-18.5 0.31 (4)	10.00			
	H-G	0 / 0	-18.5	-18.5 0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.7 PSF
DL = 5.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.89/1.00 (D-E:1), BC=0.31/1.00 (H-J:4), WB=0.43/1.00 (E-H:1), SS=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
	650	371	1747	788

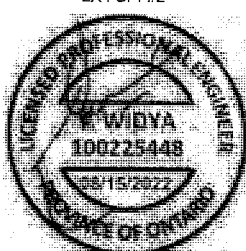
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

.JSI GRIP = 0.82 (F) (INPUT = 0.90)

.JSI METAL = 0.57 (B) (INPUT = 1.00)

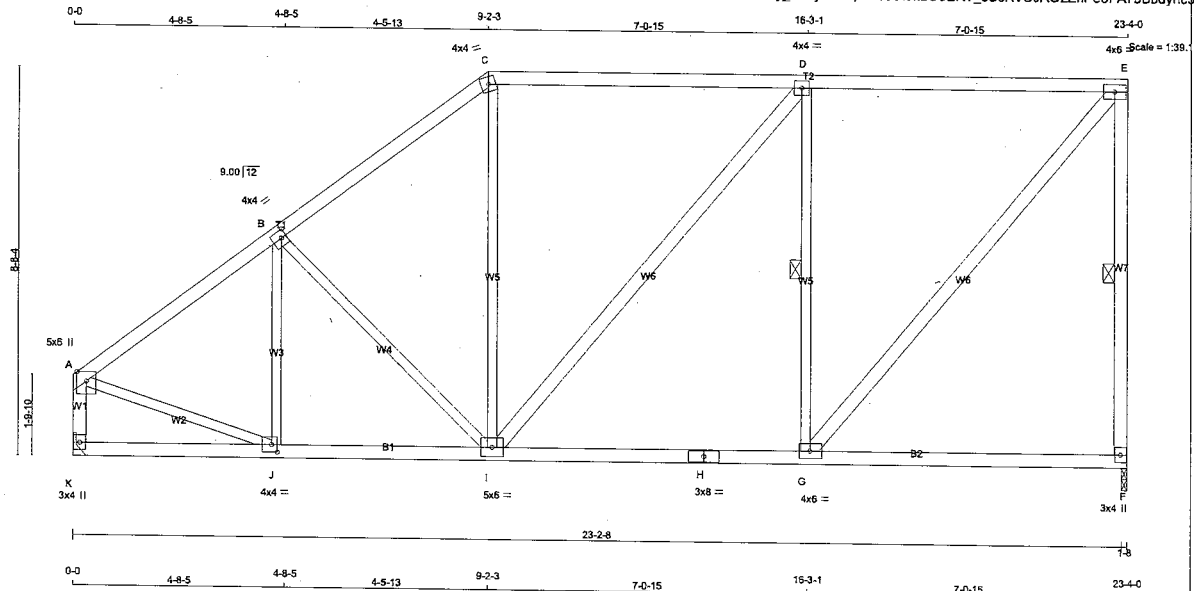
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T101A	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085802
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Alpa Roof Truss, Maple

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ID: To6XqL09rYKBxLLs9y_Z1eyrbTx-pkXc3UI9ku06ER?_eScRVs0AO2ZnPe8FA7bBbdyncsp



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
K - A	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
I - D	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVW-t	MT20	4.0	6.0	
H	BS-t	MT20	3.0	8.0	
I	BMVW-t	MT20	5.0	6.0	
J	BMVW-t	MT20	4.0	4.0	2.00 1.50
K	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
F 1327	0	0	1-8
K 1327	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM. LIVE
F 936	624 / 0	0 / 0	0 / 0
K 936	624 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. LENGTH UNBRACED	FR-TO	WES	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
A-B	-1279 / 0	-95.2	-95.2	0.38 (1)	5.22	J-B	-255 / 0	0.10 (1)		
B-C	-1135 / 0	-95.2	-95.2	0.37 (1)	5.47	B-I	-234 / 0	0.20 (1)		
C-D	-887 / 0	-95.2	-95.2	0.89 (1)	4.59	I-C	0 / 242	0.05 (1)		
D-E	-863 / 0	-95.2	-95.2	0.89 (1)	4.64	I-D	0 / 39	0.01 (1)		
E-F	-1274 / 0	0.0	0.0	0.44 (1)	5.71	G-D	-867 / 0	0.42 (1)		
K-A	-1291 / 0	0.0	0.0	0.14 (1)	7.10	G-E	0 / 1334	0.21 (1)		
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00	A-J	0 / 1103	0.25 (1)		
J-I	0 / 1049	-18.5	-18.5	0.23 (1)	10.00					
I-H	0 / 863	-18.5	-18.5	0.30 (4)	10.00					
H-G	0 / 863	-18.5	-18.5	0.30 (4)	10.00					
G-F	0 / 0	-18.5	-18.5	0.23 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.89/1.00 (C-D:1), BC=0.30/1.00 (G-I:4), WB=0.42/1.00 (D-G:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

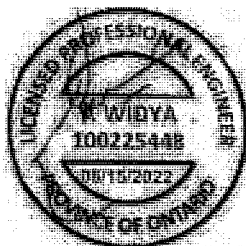
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

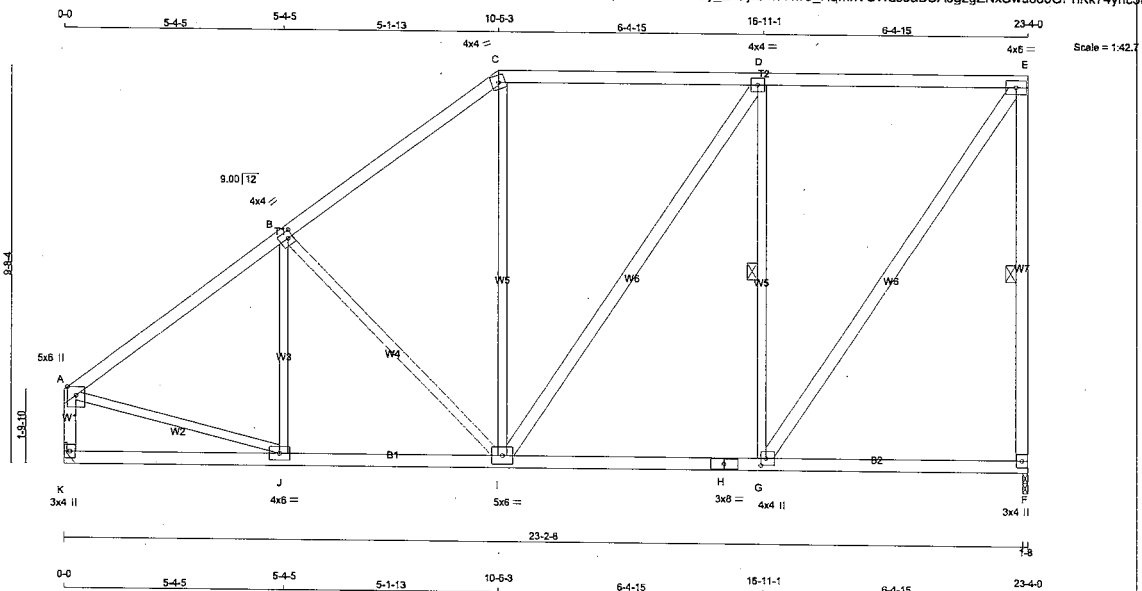
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085803
428511	T102A		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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ID: To6XqL09rYKBxLs9y_Z1eyr0Tx-Hw5_HqmnVCWzsbA8g2NkSwu8300PnKk74yncso



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-l	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-l	MT20	4.0	4.0		
E	TMWW-l	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-l	MT20	4.0	4.0	2.00	1.50
H	BS-l	MT20	3.0	8.0		
I	BMWW-l	MT20	5.0	6.0		
J	BMWW-l	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	IN-SX
F	1327	0	1327	0
K	1327	0	1327	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	936	624 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
K	936	624 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
ALL, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC) (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. (LC) (LBS)	
FR-TO		FROM	TO	FR-TO			
A-B	-1293 / 0	-95.2	-95.2	0.52 (1)	J-B	-185 / 26	0.10 (1)
B-C	-1062 / 0	-95.2	-95.2	0.50 (1)	B-I	-354 / 0	0.41 (1)
C-D	-820 / 0	-95.2	-95.2	0.71 (1)	I-C	0 / 203	0.05 (4)
D-E	-718 / 0	-95.2	-95.2	0.70 (1)	I-D	0 / 184	0.03 (1)
F-E	-1279 / 0	0.0	0.0	0.57 (1)	G-D	-910 / 0	0.57 (1)
K-A	-1285 / 0	0.0	0.0	0.14 (1)	G-E	0 / 1269	0.20 (1)
					A-J	0 / 1103	0.25 (1)
K-J	0 / 0	-18.5	-18.5	0.12 (4)			
J-I	0 / 1063	-18.5	-18.5	0.23 (1)			
I-H	0 / 718	-18.5	-18.5	0.25 (4)			
H-G	0 / 718	-18.5	-18.5	0.25 (4)			
G-F	0 / 0	-18.5	-18.5	0.19 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.71/1.00 (C-D-1), BC=0.25/1.00 (G-J-4), WB=0.57/1.00 (D-G-1), SSI=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

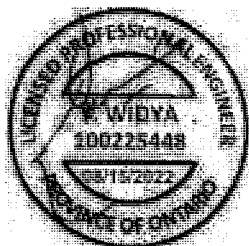
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

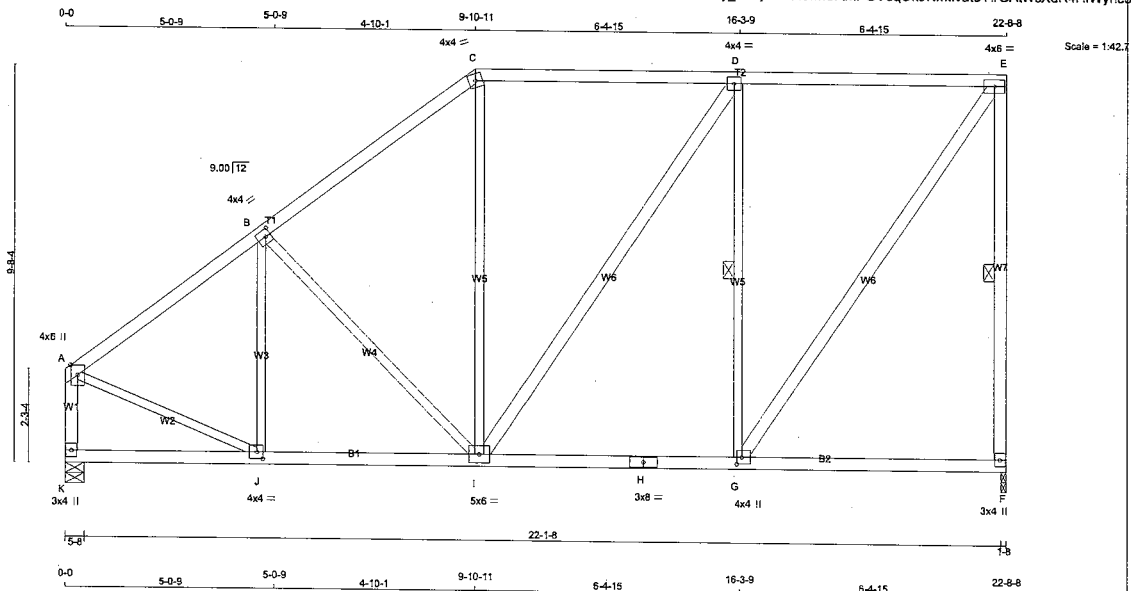
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
I - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A - TMWV+p	MT20	4.0	5.0	Edge	
B - TMWV-t	MT20	4.0	4.0	2.00	1.50
C - TTW-m	MT20	4.0	4.0		
D - TMWV-t	MT20	4.0	4.0		
E - TMWV-t	MT20	4.0	4.0		
F - BMV1+p	MT20	3.0	4.0		
G - BMWV-t	MT20	4.0	4.0	2.00	1.50
H - BS-t	MT20	3.0	8.0		
I - BMWVW-t	MT20	5.0	9.0		
J - BMWV-t	MT20	4.0	4.0	2.00	1.75
K - BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION	JT	GROSS REACTION
F	1291 0	1291 0	0 0	0 0	0 0	1-8	1-8
K	1291 0	1291 0	0 0	0 0	0 0	1-8	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	911	807 / 0	0 / 0	0 / 0	0 / 0	304 / 0	0 / 0
K	911	807 / 0	0 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

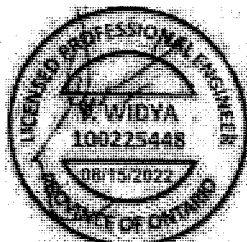
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRACED	MEMB.	FORCE (LBS)	MAX	LC1 MAX
A-B	-1138 / 0	-95.2	-95.2	0.44 (1)	5.36	J-B	-285 / 0	0.15 (1)	
B-C	-998 / 0	-95.2	-95.2	0.44 (1)	5.65	B-I	-244 / 0	0.28 (1)	
C-D	-772 / 0	-95.2	-95.2	0.70 (1)	5.51	I-C	0 / 161	0.04 (4)	
D-E	-694 / 0	-95.2	-95.2	0.70 (1)	5.74	I-D	0 / 141	0.02 (1)	
E-F	-1243 / 0	0.0	0.0	0.56 (1)	5.76	G-D	-875 / 0	0.55 (1)	
K-A	-1255 / 0	0.0	0.0	0.15 (1)	7.17	G-E	0 / 1226	0.20 (1)	
						A-J	0 / 1015	0.23 (1)	
K-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
J-I	0 / 939	-18.5	-18.5	0.21 (1)	10.00				
I-H	0 / 694	-18.5	-18.5	0.25 (4)	10.00				
H-G	0 / 694	-18.5	-18.5	0.25 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.19 (4)	10.00				

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2016
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")
CSI: TC=0.70/1.00 (C-D-1), BC=0.25/1.00 (G-I-4), WB=0.55/1.00 (D-G-1), SSI=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	550 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

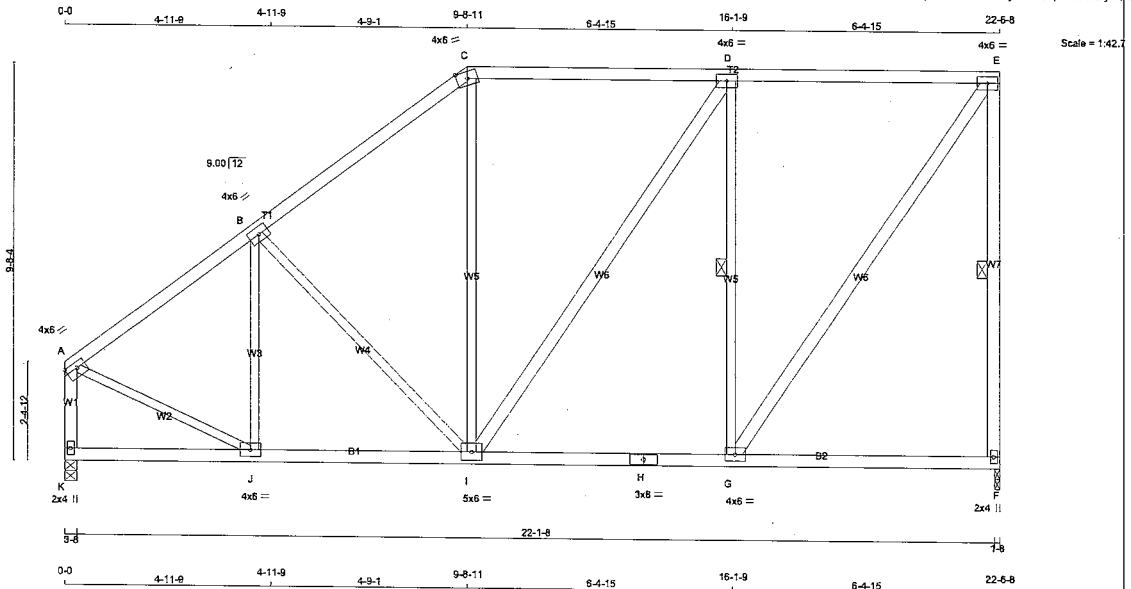
JSI GRIP = 0.89 (A) (INPUT = 0.90)

JSI METAL = 0.56 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085805
428511	T102C		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-hVn6vsofo7uXj2lthhNfBuMfyWLQiq5IZOkOynd5l



LUMBER	SIZE	LUMBER	DESCR.
N.L.G.A. RULES			
CHORDS	SIZE		
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
I - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A - TMWV-I	MT20	4.0	6.0	1.50 Edge
B - TMWV-I	MT20	4.0	6.0	
C - TTW-m	MT20	4.0	6.0	Edge
D - TMWV-I	MT20	4.0	6.0	
E - TMWV-I	MT20	4.0	6.0	
F - BMV1+p	MT20	2.0	4.0	
G - BMWV-I	MT20	4.0	6.0	
H - BS-I	MT20	3.0	6.0	
I - BMWV-I	MT20	5.0	6.0	
J - BMWV-I	MT20	4.0	6.0	
K - BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
F	1350	0	1-8	1-8
K	1309	0	3-8	1-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	960	603 / 0	0 / 0
K	927	603 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

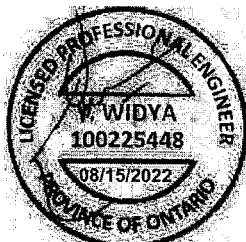
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	WEBS
TOTAL LOAD CASES: (4)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED MEMB. FORCE (LBS)
	VERT. (PLF)	MAX. FACTORED MEMB. FORCE (LBS)
	FROM TO	FROM TO
	FR-TO	FR-TO
	A-B	-1125 / 0
	B-C	-1017 / 0
	C-D	-798 / 0
	D-E	-722 / 0
	E-F	-1303 / 0
	F-K	-1275 / 0
	K-J	0 / 0
	J-I	0 / 929
	I-H	0 / 722
	H-G	0 / 722
	G-F	0 / 0

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 8.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.75")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.75")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: TC=0.76/1.00 (C-D-1), BC=0.25/1.00 (G-I-4), WB=0.58/1.00 (D-G-1), SSI=0.32/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

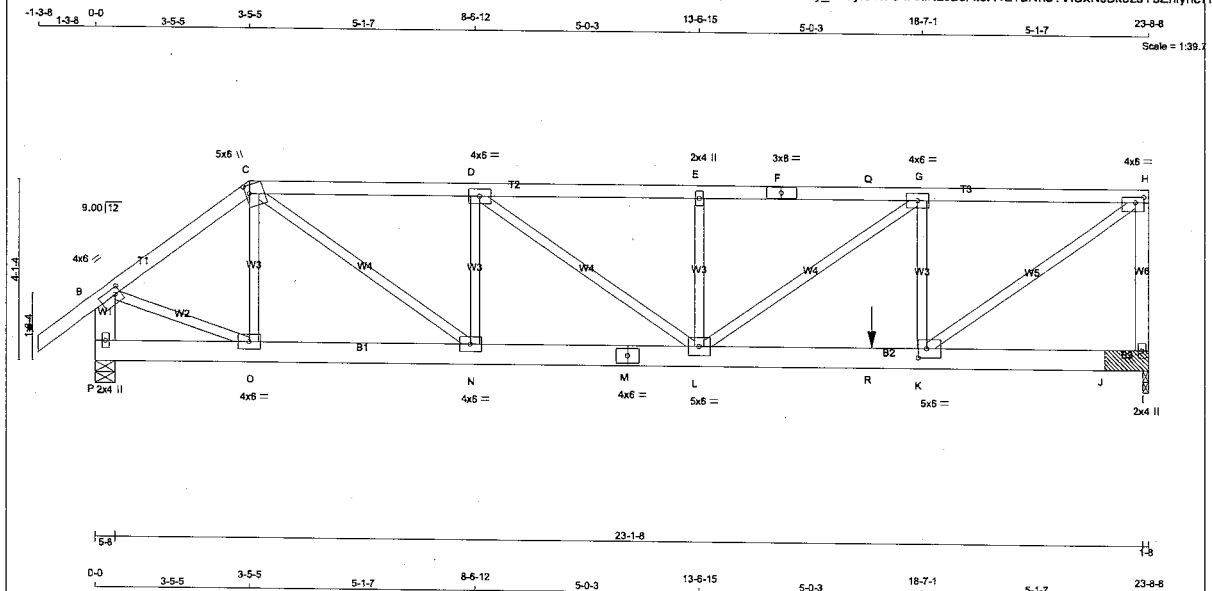
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.30 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085806
428511	T103		2	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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ID: To6XqL09rYKBxLLs9y_Z1eyrbTx-94AxIKaJa5A16X4L.TaNH37V:CxNs8KbzsYbZhyncrK



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-F 1	12	TOP
F-H 1	12	SIDE(46.1)
H-I 1	12	TOP
P-B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-M 2	12	TOP
M-I 2	12	SIDE(10.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

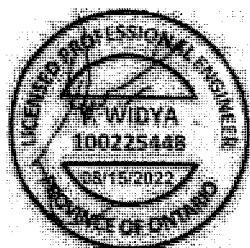
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWW-I	MT20	4.0	6.0	1.75	1.50	
C TTWW+m	MT20	5.0	6.0	2.25	1.00	
D TMWW-I	MT20	4.0	6.0			
E TMWW+m	MT20	2.0	4.0			
F TS-I	MT20	3.0	8.0			
G TMWW-I	MT20	4.0	6.0			
H TMWW-I	MT20	4.0	6.0	1.50	2.25	
I BMV1+p	MT20	2.0	4.0			
K BMWW-I	MT20	5.0	6.0	2.50	2.25	
L BMWW-I	MT20	5.0	6.0			
M BS-I	MT20	4.0	6.0			
N BMWW-I	MT20	4.0	6.0			
O BMWW-I	MT20	4.0	6.0			
P BMV1+p	MT20	2.0	4.0			

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	3166	3166	0	1-8
P	2007	2007	0	1-8 & BLOCK

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	2235	1489 / 0	0 / 0	0 / 0	0 / 0
P	1414	956 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P
2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. I ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 4" C.C. 6 NAILS TOTAL.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)									
CHORDS						WEBS					
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LOAD (LC1)	MAX. FORCE (LBS)	UNBRACED LENGTH (FT)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LOAD (LC1)	MAX. FORCE (LBS)	UNBRACED LENGTH (FT)
FR-TO		FROM TO				FR-TO		FROM TO			
A-B	0 / 39		-95.2	-95.2	0.07 (1)	10.00	O-C	-363 / 0	0.05 (1)		
B-C	-1994 / 0		-95.2	-95.2	0.12 (1)	8.09	C-N	0 / 2036	0.25 (1)		
C-D	-3230 / 0		-95.2	-95.2	0.25 (1)	4.93	N-D	-1210 / 0	0.15 (1)		
D-E	-4119 / 0		-95.2	-95.2	0.29 (1)	4.43	D-L	0 / 1108	0.14 (1)		
E-F	-4119 / 0		-95.2	-95.2	0.41 (1)	4.26	L-E	-402 / 0	0.05 (1)		
F-G	-4119 / 0		-95.2	-95.2	0.41 (1)	4.26	L-G	0 / 399	0.05 (1)		
G-Q	-4119 / 0		-187.4	-187.4	0.41 (1)	4.26	K-G	-1187 / 0	0.15 (1)		
H-H	-3798 / 0		-187.4	-187.4	0.42 (1)	4.40	K-H	0 / 4700	0.58 (1)		
I-H	-3168 / 0		0.0	0.0	0.39 (1)	6.52	B-O	0 / 1667	0.21 (1)		
P-B	-1995 / 0		0.0	0.0	0.07 (1)	7.81					
P-O	0 / 0		-18.5	-18.5	0.03 (4)	10.00					
O-N	0 / 1581		-18.5	-18.5	0.14 (1)	10.00					
N-M	0 / 3230		-18.5	-18.5	0.31 (1)	10.00					
M-L	0 / 3230		-18.5	-18.5	0.31 (1)	10.00					
L-R	0 / 3798		-18.5	-18.5	0.64 (1)	10.00					
R-K	0 / 3798		-36.4	-36.4	0.64 (1)	10.00					
K-J	0 / 0		-36.4	-36.4	0.18 (1)	10.00					
J-I	0 / 0		-36.4	-36.4	0.18 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)	JT LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	17-5-8	-1657	-1657	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS INPUT BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-5-5
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADDTL LOADS BASED ON 55 % OF G.S.L.
LOADS APPLIED TO FIRST 6-3-0 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.42/1.00 (B-H-1), BC=0.64/1.00 (K-L-1), WB=0.58/1.00 (H-K-1), SS=0.46/1.00 (K-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

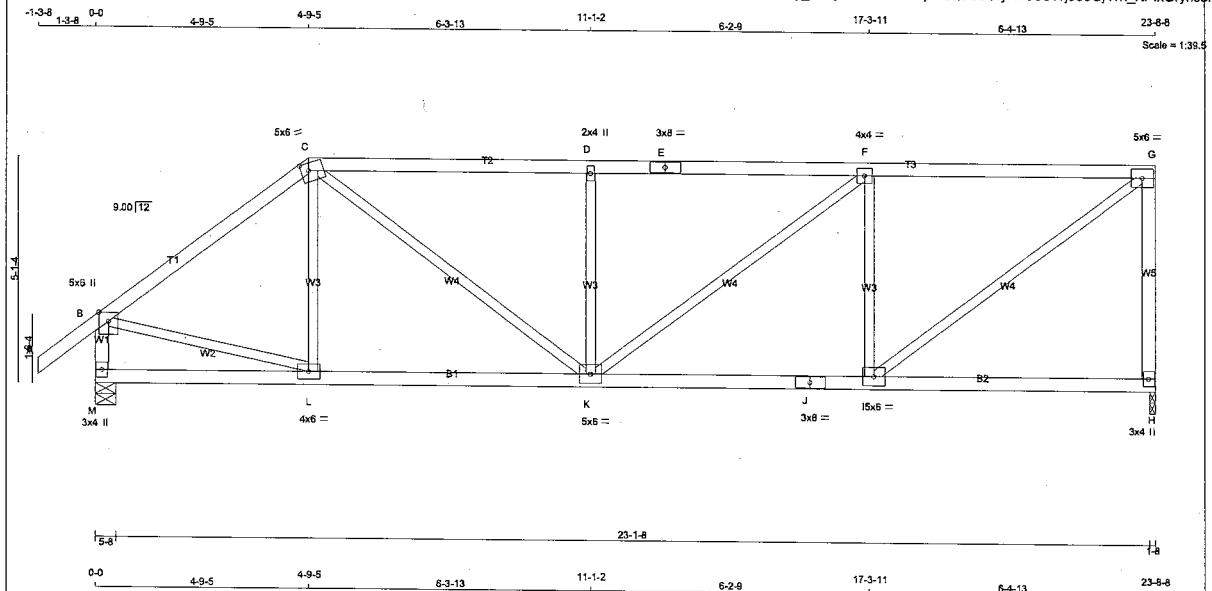
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)

JOB NAME 428511	TRUSS NAME T104	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085807
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Alpa Roof Truss, Maple

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LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - B	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMWV+p	MT20	5.0	6.0	Edge
C TTWV-m	MT20	5.0	6.0	2.00 2.00
D TMWV-w	MT20	2.0	4.0	
E TS-1	MT20	3.0	8.0	
F TMWV-1	MT20	4.0	4.0	
G TMWV-1	MT20	5.0	6.0	
H BMV1+p	MT20	3.0	4.0	
I BMWV-t	MT20	5.0	6.0	
J BS-1	MT20	3.0	8.0	
K BMWV-1	MT20	5.0	6.0	
L BMWV-1	MT20	4.0	6.0	
M BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT GROSS REACTION			BRG	BRG
H VERT HORZ	DOWN	UP/LIFT	IN-SX	IN-SX
H 1348 0	1348 0	0 1-8	1-8	1-8
M 1479 0	1479 0	0 5-8	1-10	1-10

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	951	634 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
M	1042	708 / 0	0 / 0	0 / 0	0 / 0	334 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED	FORCE	MAX	CS1 (LC)
FR-TO													
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	L-C	-149 / 39	0.06 (1)					
B-C	-1364 / 0	-95.2	-95.2	0.44 (1)	5.02	B-L	0 / 1121	0.25 (1)					
C-D	-1736 / 0	-95.2	-95.2	0.62 (1)	4.30	I-G	0 / 1752	0.39 (1)					
D-E	-1736 / 0	-95.2	-95.2	0.64 (1)	4.26	C-K	0 / 817	0.18 (1)					
E-F	-1736 / 0	-95.2	-95.2	0.64 (1)	4.26	I-F	-921 / 0	0.35 (1)					
F-G	-1400 / 0	-95.2	-95.2	0.61 (1)	4.59	K-D	-646 / 0	0.25 (1)					
H-G	-1299 / 0	0.0	0.0	0.57 (1)	7.08	K-F	0 / 425	0.10 (1)					
M-B	-1446 / 0	0.0	0.0	0.15 (1)	6.80								
M-L	0 / 0	-18.5	-18.5	0.14 (4)	10.00								
L-K	0 / 1086	-18.5	-18.5	0.25 (1)	10.00								
K-J	0 / 1400	-18.5	-18.5	0.32 (1)	10.00								
J-I	0 / 1400	-18.5	-18.5	0.32 (1)	10.00								
I-H	0 / 0	-18.5	-18.5	0.18 (4)	10.00								

TOTAL WEIGHT = 97 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL) = L/360 (0.79")

CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.84/1.00 (D-F:1), BC=0.32/1.00 (H-K:1),

WB=0.39/1.00 (G-I:1), SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 550 371 1747 788 1987 1873

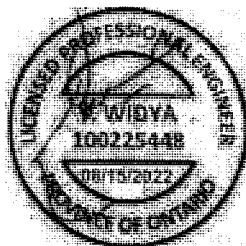
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)

JSI METAL = 0.56 (B) (INPUT = 1.00)

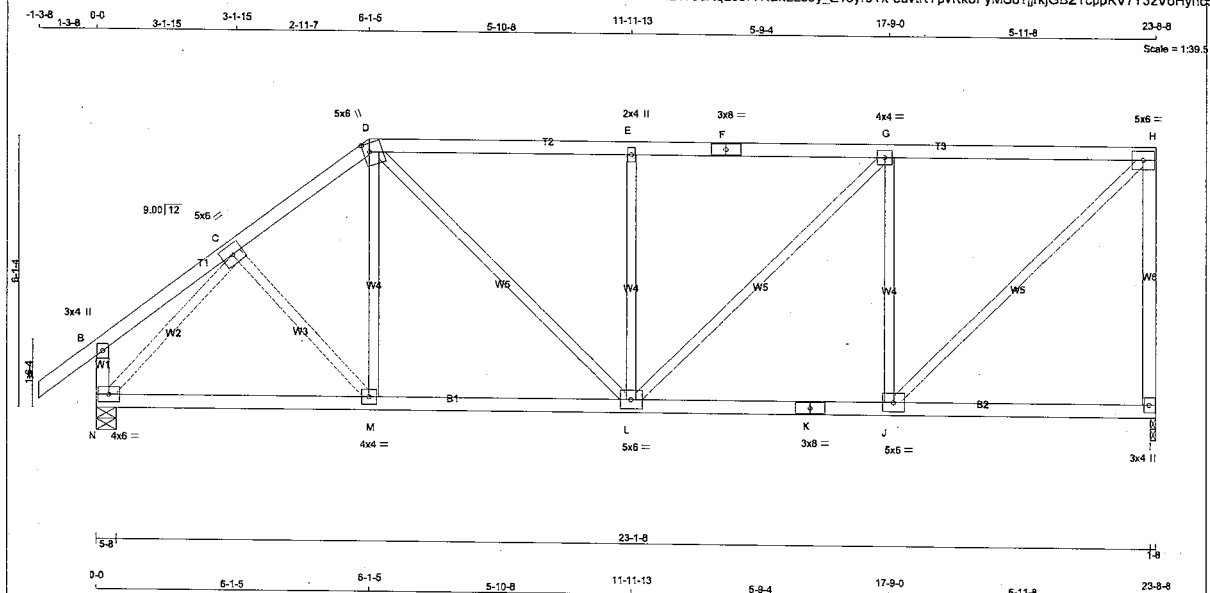
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T105	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085808
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Alpa Roof Truss, Maple

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ID:To6XqL09YKbXLLs9y_Z1eyrbTx-euvtkYpvKk8FyMS8?jrkjGB2TcppKV7Y32Vohync5j



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ
I	1348	0	1348	0
N	1479	0	1479	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
I	951	634 / 0
N	1042	708 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
FR-TO							FR-TO		
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	C-M	0 / 53	0.02 (4)	
B-C	0 / 18	-95.2	-95.2	0.13 (1)	10.00	M-D	0 / 104	0.04 (4)	
C-D	-1354 / 0	-95.2	-95.2	0.15 (1)	5.42	N-C	-1577 / 0	0.53 (1)	
D-E	-1433 / 0	-95.2	-95.2	0.50 (1)	4.82	J-H	0 / 1538	0.35 (1)	
E-F	-1433 / 0	-95.2	-95.2	0.52 (1)	4.79	D-L	0 / 513	0.12 (1)	
F-G	-1433 / 0	-95.2	-95.2	0.52 (1)	4.79	J-G	-950 / 0	0.56 (1)	
G-H	-1100 / 0	-95.2	-95.2	0.50 (1)	5.32	L-E	-600 / 0	0.35 (1)	
H-I	-1303 / 0	0.0	0.0	0.94 (1)	7.07	L-G	0 / 472	0.11 (1)	
N-B	-246 / 0	0.0	0.0	0.03 (1)	7.81				
N-M	0 / 1038	-18.5	-18.5	0.26 (1)	10.00				
M-L	0 / 1068	-18.5	-18.5	0.26 (1)	10.00				
L-K	0 / 1100	-18.5	-18.5	0.26 (1)	10.00				
K-J	0 / 1100	-18.5	-18.5	0.26 (1)	10.00				
J-I	0 / 0	-18.5	-18.5	0.16 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. DL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.10")

CSI: TC=0.94/1.00 (H-I:1), BC=0.26/1.00 (L-M:1), WB=0.56/1.00 (G-J:1), SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

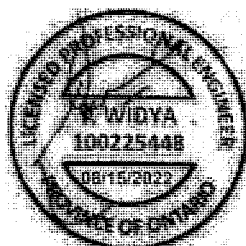
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1967 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

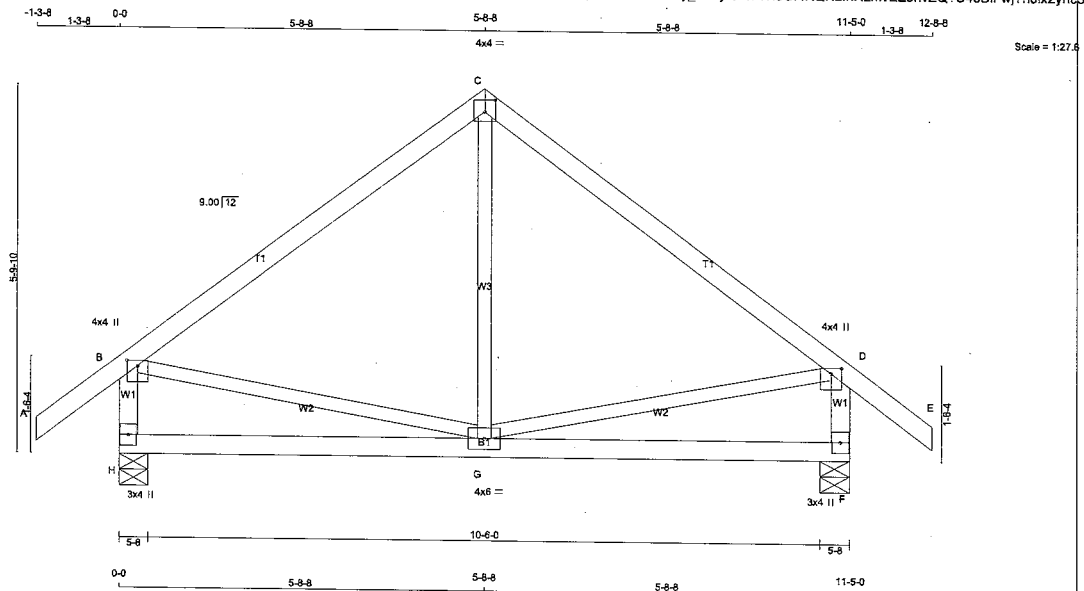


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Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eybTx-Wf8OAvtQNzfrRzmveZonvZQ?U40BIFwJTh0ix2yncsf



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS	SIZE		
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMWW+p	MT20	4.0	4.0	1.00 2.00
C TTW-p	MT20	4.0	4.0	2.25 2.00
D TMWW+p	MT20	4.0	4.0	1.00 2.00
F BMV1+p	MT20	3.0	4.0	
G BMWWV-1	MT20	4.0	6.0	
H BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	BRG	BRG
H	780 0	780 0	0 5-8	1-8
F	780 0	780 0	0 5-8	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	548	379 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0
F	548	379 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE (LBS)	FACTORED	VERT. LOAD (PLF)	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	WEBS	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	G-C	-17 / 94	0.03 (4)						
B-C	-439 / 0	-95.2	-95.2	0.40 (1)	6.25	B-G	0 / 359	0.08 (1)						
C-D	-439 / 0	-95.2	-95.2	0.40 (1)	6.25	G-D	0 / 359	0.08 (1)						
D-E	0 / 39	-95.2	-95.2	0.13 (1)	10.00									
H-B	-740 / 0	0.0	0.0	0.08 (1)	7.81									
F-D	-740 / 0	0.0	0.0	0.08 (1)	7.81									
H-G	0 / 0	-18.5	-18.5	0.17 (4)	10.00									
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.40/1.00 (B-C:1), BC=0.17/1.00 (G-H:4),
WB=0.08/1.00 (D-G:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

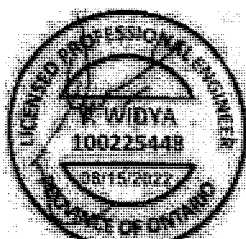
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

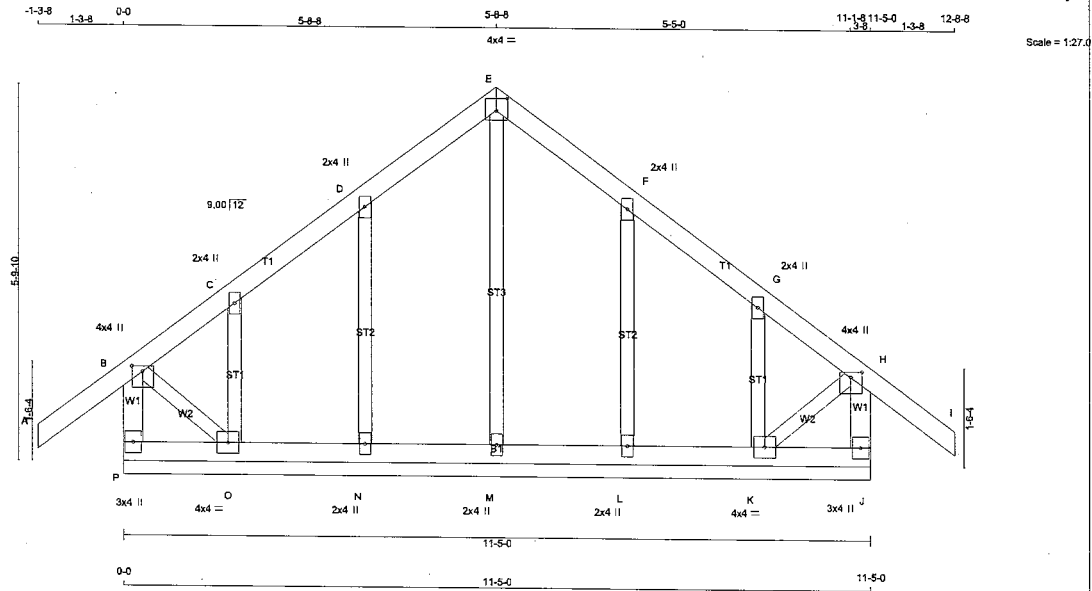
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	4.0	2.25	2.00
H TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMWW1-t	MT20	4.0	4.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMWW1-t	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
P-B	-283 / 0	0.0	0.0 0.03 (1)	M-E	-144 / 0	0.07 (1)	
A-B	0 / 39	-95.2	-95.2 0.13 (1)	N-D	-230 / 0	0.06 (1)	
B-C	-51 / 0	-95.2	-95.2 0.12 (1)	O-C	-102 / 0	0.02 (1)	
C-D	-6 / 0	-95.2	-95.2 0.06 (1)	L-F	-230 / 0	0.06 (1)	
D-E	-23 / 0	-95.2	-95.2 0.06 (1)	K-G	-102 / 0	0.02 (1)	
E-F	-23 / 0	-95.2	-95.2 0.06 (1)	B-O	0 / 21	0.00 (1)	
F-G	-6 / 0	-95.2	-95.2 0.06 (1)	K-H	0 / 21	0.00 (1)	
G-H	-51 / 0	-95.2	-95.2 0.12 (1)				
H-I	0 / 39	-95.2	-95.2 0.13 (1)				
J-H	-283 / 0	0.0	0.0 0.03 (1)				
P-O	0 / 0	-18.5	-18.5 0.02 (4)				
O-N	0 / 13	-18.5	-18.5 0.02 (4)				
N-M	0 / 8	-18.5	-18.5 0.01 (4)				
M-L	0 / 8	-18.5	-18.5 0.01 (4)				
L-K	0 / 13	-18.5	-18.5 0.02 (4)				
K-J	0 / 0	-18.5	-18.5 0.02 (4)				

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (N-O:4), WB=0.07/1.00 (E-M:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747 788 1987 1873

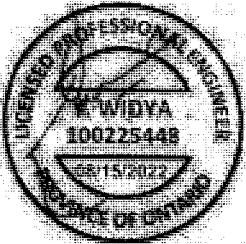
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (H) (INPUT = 0.90)

JSI METAL = 0.12 (F) (INPUT = 1.00)

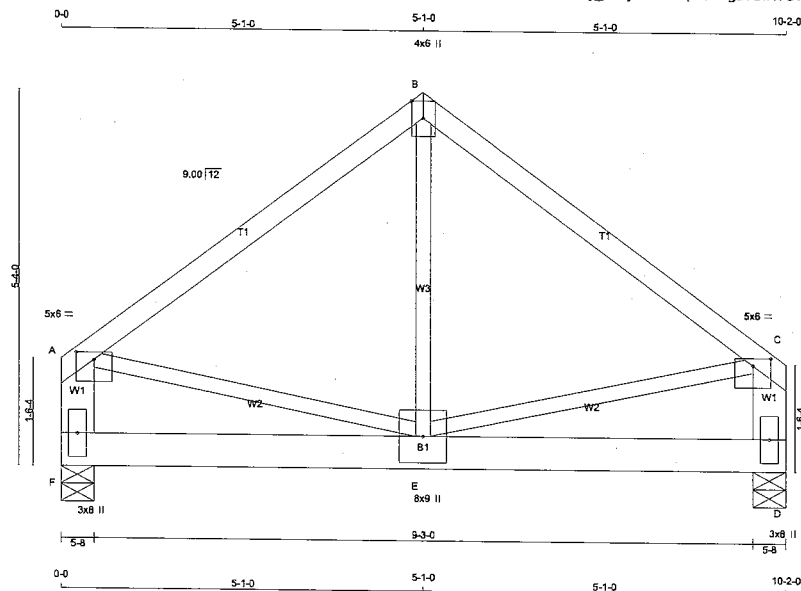
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T109	QUANTITY 2	PLY 4x6 II	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085813
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Alpa Roof Truss, Maple

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ID:To6XqL09rYKbXLLs9y_Z1eyrbTx-xEqWoxvlgU1GIRVUvhLUXC2YGHv2yYH99fENYNyn5c



Scale = 1/24.8

LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1 12		TOP
B-C 1 12		TOP
F-A 2 12		TOP
D-C 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 8		SIDE(303.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.25 3.00
B	TTW-p	MT20	4.0	6.0	Edge
C	TMVW-p	MT20	5.0	6.0	1.25 3.00
D	BMV1-p	MT20	3.0	6.0	
E	BMVWWH-p	MT20	5.0	6.0	
F	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
F	3661	0	3661	0
D	3661	0	3661	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2584	1721/0	0/0	0/0	0/0	863/0	0/0
D	2584	1721/0	0/0	0/0	0/0	863/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC2 MAX (PLF)	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC2 MAX (PLF)
FR-TO						FR-TO				
A-B	-2828/0	-95.2	-95.2	0.30 (1)	5.14	E-B	0/2910	0.36 (1)		
B-C	-2828/0	-95.2	-95.2	0.30 (1)	5.14	A-E	0/2316	0.29 (1)		
F-A	-2433/0	0.0	0.0	0.09 (1)	7.81	E-C	0/2316	0.29 (1)		
D-C	-2433/0	0.0	0.0	0.09 (1)	7.81					
F-E	0/0	-624.9	-624.9	0.67 (1)	10.00					
E-D	0/0	-624.9	-624.9	0.67 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CS1dGirder

START DISTANCE = 0-0

START SPAN CARRIED = 23-4-0

END DISTANCE = 10-2-0

END SPAN CARRIED = 23-4-0

END WALL WIDTH = 0-0

APPLIED TO FRONT SIDE OF BOTTOM CHORD.

-ADDTL. LOADS BASED ON 55% OF GSL.

(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

-PART 9 OF BCBC 2018, ABC 2019

-PART 9 OF CBC 2012 (2019 AMENDMENT)

-CSA 086-14

-TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.34")

CALCULATED VERT. DEFL. (LL) = L/999 (0.05")

ALLOWABLE DEFL. (TL) = L/360 (0.34")

CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CS1: TC=0.30/1.00 (A-B-1), BC=0.67/1.00 (E-F-1),

WB=0.36/1.00 (B-E-1), SS=0.65/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 550 371 1747 788 1987 1873

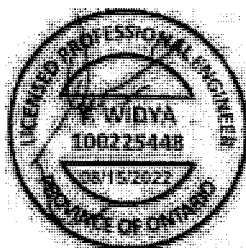
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (B) (INPUT = 0.90)

JSI METAL= 0.35 (B) (INPUT = 1.00)

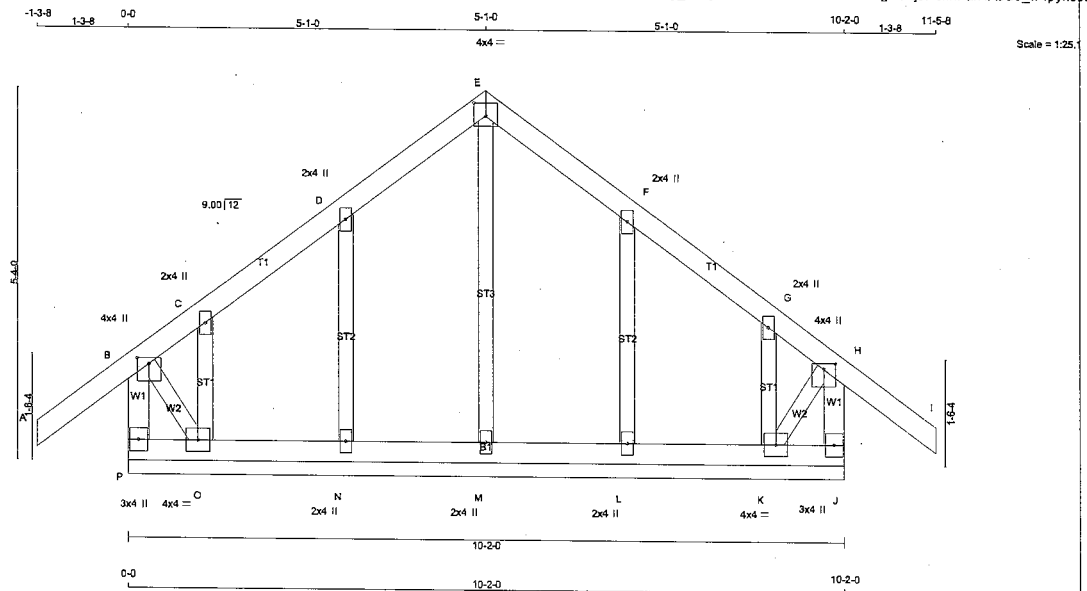
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME T110G	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085814
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Alpa Roof Truss, Maple

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TOTAL WEIGHT = 47 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	4.0	2.25	2.00
H TMW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMW1-i	MT20	4.0	4.0		
L, M, N					
L BMV1+w	MT20	2.0	4.0		
O BMW1-i	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-277 / 0	0.0	0.0 0.03 (1)	M-E	-153 / 0	0.06 (1)	
A-B	0 / 39	-95.2	-95.2 0.13 (1)	N-D	-230 / 0	0.05 (1)	
B-C	-60 / 0	-95.2	-95.2 0.12 (1)	O-C	-42 / 0	0.01 (1)	
C-D	0 / 2	-95.2	-95.2 0.06 (1)	L-F	-230 / 0	0.05 (1)	
D-E	-15 / 0	-95.2	-95.2 0.06 (1)	K-G	-42 / 0	0.01 (1)	
E-F	-15 / 0	-95.2	-95.2 0.06 (1)	B-O	0 / 13	0.00 (1)	
F-G	0 / 2	-95.2	-95.2 0.06 (1)	K-H	0 / 13	0.00 (1)	
G-H	-60 / 0	-95.2	-95.2 0.12 (1)				
H-I	0 / 39	-95.2	-95.2 0.13 (1)				
J-H	-277 / 0	0.0	0.0 0.03 (1)				
P-O	0 / 0	-18.5	-18.5 0.01 (4)				
O-N	0 / 7	-18.5	-18.5 0.02 (4)				
N-M	0 / 1	-18.5	-18.5 0.02 (4)				
M-L	0 / 1	-18.5	-18.5 0.02 (4)				
L-K	0 / 7	-18.5	-18.5 0.02 (4)				
K-J	0 / 0	-18.5	-18.5 0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	26.7	PSF
LL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (H-I:1), BC=0.02/1.00 (K-L:4), WB=0.06/1.00 (E-M:1), SS=0.08/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

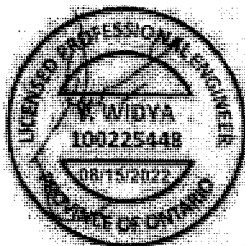
PLATE PLACEMENT TOL. = 0.250 inches

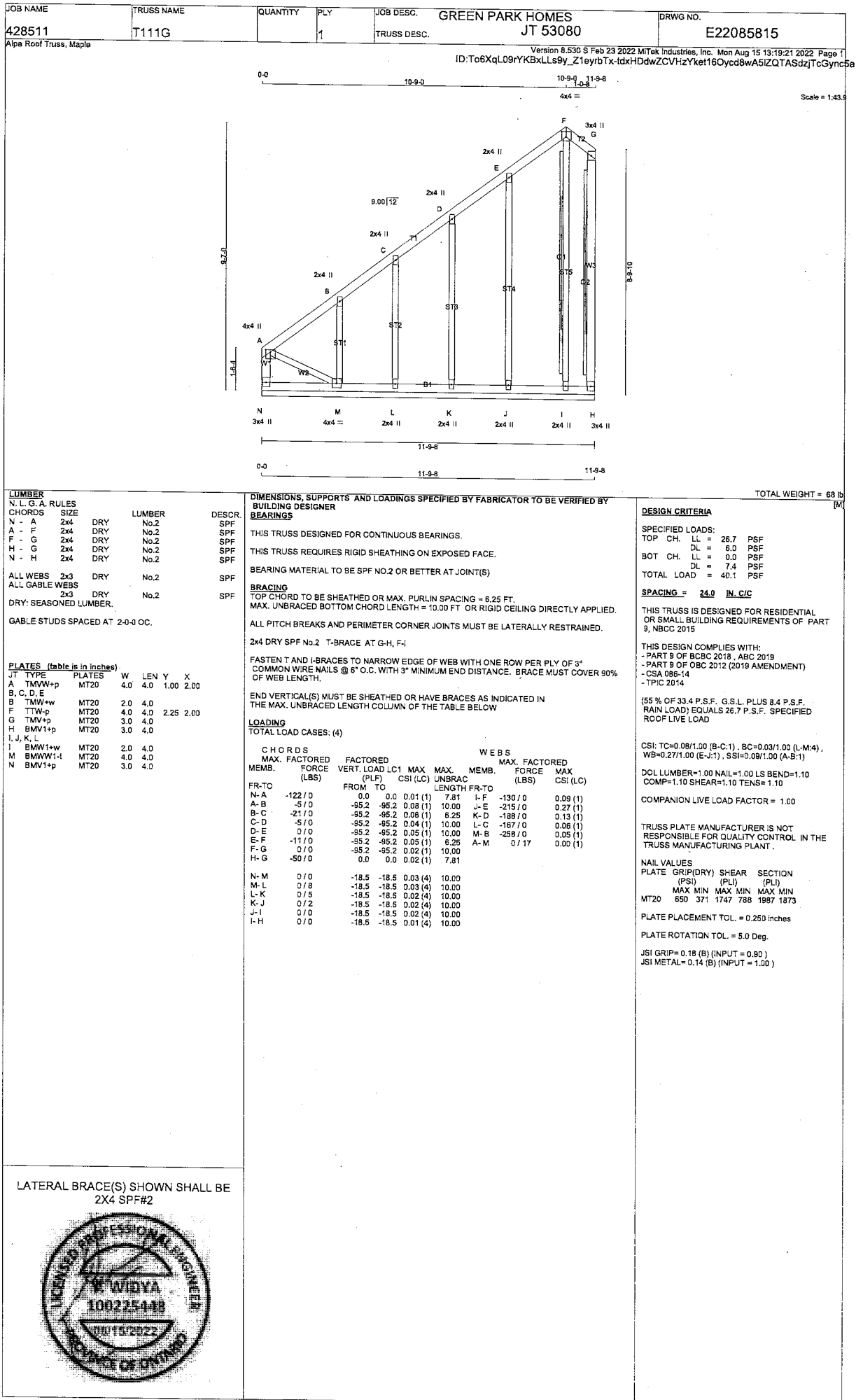
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)

JSI METAL= 0.12 (D) (INPUT = 1.00)

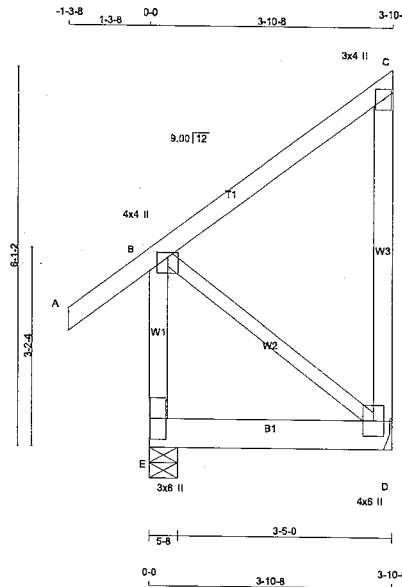
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	
428511	T112		2	JT 53080		E22085816	
Alpa Roof Truss, Maple							

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Scale = 1/28.

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
E - B 1	12	TOP
A - C 1	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
E - D 2	12	SIDE(133.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT HORZ	DOWN HORZ	UPLIFT IN-SX	IN-SX	
E 870 0	870 0	0 5-8	1-8	
D 738 0	738 0	0 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E 612	421/0	0/0	0/0	0/0	191/0	0/0	
D 522	347/0	0/0	0/0	0/0	174/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED L1 MAX. (LC) (LC)	MAX. FACTORED UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED L1 MAX. (LC) (LC)
FR-TO								
E-B	-316/0	0.0	0.0	0.03 (1)	7.81	B-D	0/0	0.00 (1)
A-B	0/39	-95.2	-95.2	0.11 (7)	10.00			
B-C	0/0	-95.2	-95.2	0.13 (1)	10.00			
D-C	-184/0	0.0	0.0	0.06 (1)	7.81			
E-D	0/0	-286.2	-286.2	0.20 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS

TOTAL WEIGHT = 2 X 27 = 54 b

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CSdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-5-0
END DISTANCE = 3-10-8
END SPAN CARRIED = 11-5-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.13/1.00 (B-C:1), BC=0.20/1.00 (D-E:1), WB=0.00/1.00 (B-D:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

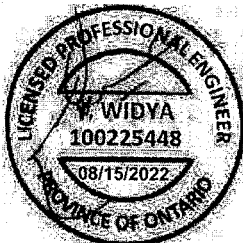
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.12 (B) (INPUT = 0.90)
JSI METAL = 0.03 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

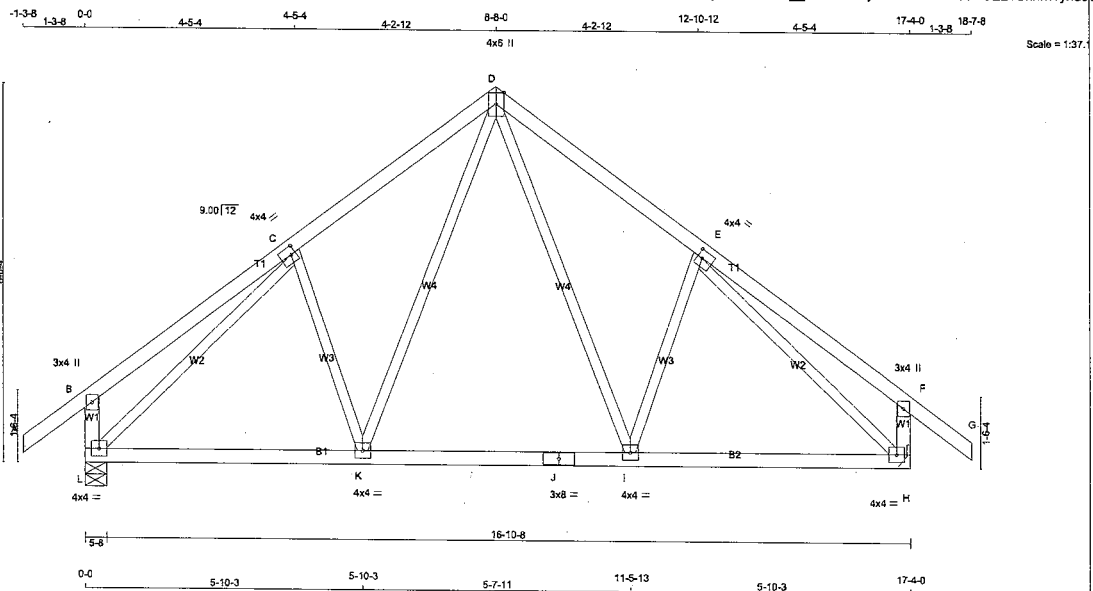




JOB NAME 428511	TRUSS NAME T115	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085818
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Alpa Roof Truss, Maple

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Scale = 1/32"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMVW-i	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	3.0	4.0		
H	BMVW-i	MT20	4.0	4.0		
I	BMVW-i	MT20	4.0	4.0		
J	BS-i	MT20	3.0	8.0		
K	BMVW-i	MT20	4.0	4.0		
L	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	VERT	HORZ	UPLIFT	IN-SX
H	1117	0	0	5-8
H	1117	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
L	786	537/0	0/0
H	786	537/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX.	FACTORED	FORCE	MAX
FR-TO												
A-B	0/39	-95.2	-95.2	0.13	(1)	10.00	D-I	0/373	0.08	(1)		
B-C	0/29	-95.2	-95.2	0.29	(1)	10.00	I-E	-249/0	0.09	(1)		
C-D	-880/0	-95.2	-95.2	0.23	(1)	6.25	K-D	0/373	0.08	(1)		
D-E	-880/0	-95.2	-95.2	0.23	(1)	6.25	C-K	-249/0	0.09	(1)		
E-F	0/29	-95.2	-95.2	0.29	(1)	10.00	L-C	-1095/0	0.71	(1)		
F-G	0/39	-95.2	-95.2	0.13	(1)	10.00	E-H	-1095/0	0.71	(1)		
L-B	-287/0	0.0	0.0	0.03	(1)	7.81						
H-F	-287/0	0.0	0.0	0.03	(1)	7.81						
L-K	0/764	-18.5	-18.5	0.20	(4)	10.00						
K-J	0/553	-18.5	-18.5	0.18	(4)	10.00						
J-I	0/553	-18.5	-18.5	0.18	(4)	10.00						
I-H	0/764	-18.5	-18.5	0.20	(4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = U/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = U/999 (0.05")

CSI: TC=0.29/1.00 (B-C:1), BC=0.20/1.00 (K-L:4), WB=0.71/1.00 (C-L:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

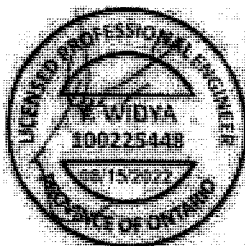
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

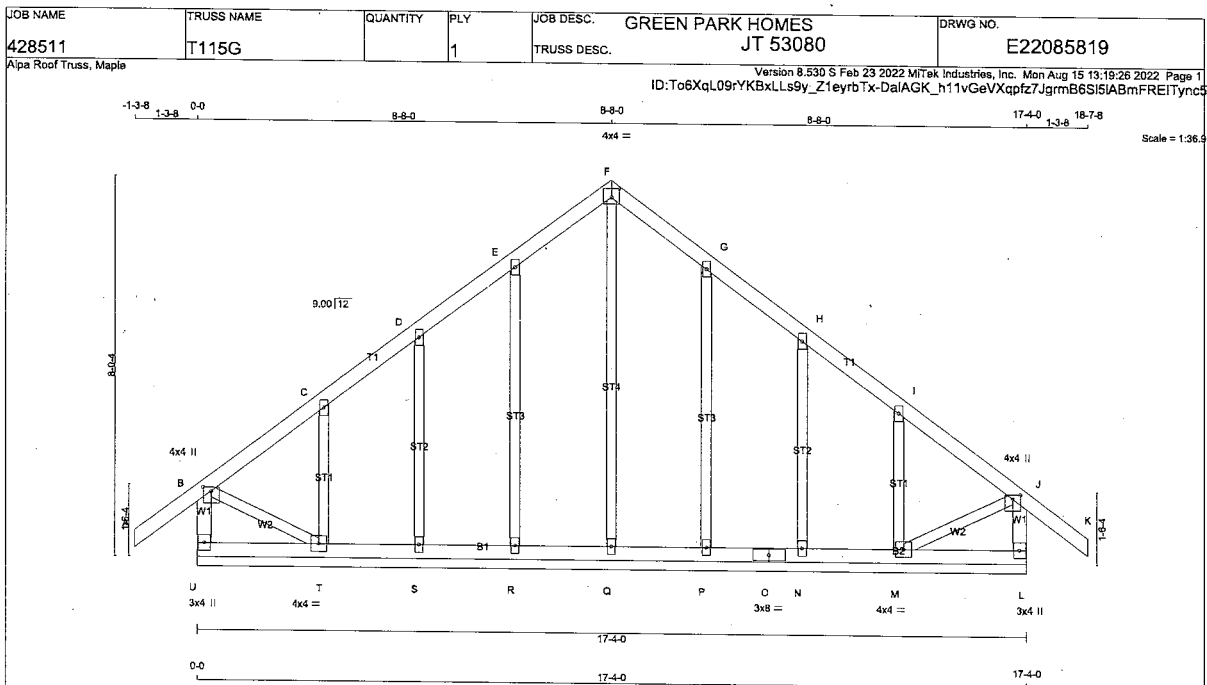
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

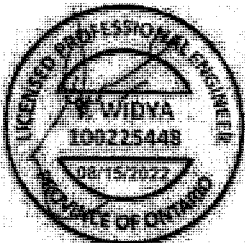
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
U - B	2x4	DRY	No.2	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				TOP CH. LL = 25.7 PSF			
A - F	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				DL = 5.0 PSF			
F - K	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				BOT CH. LL = 0.0 PSF			
L - J	2x4	DRY	No.2	BRACING				DL = 7.4 PSF			
U - O	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				TOTAL LOAD = 40.1 PSF			
O - L	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				SPACING = 24.0 IN./C			
ALL WEBS	2x3	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
ALL GABLE WEBS	2x3	DRY	No.2	LOADING				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				TOTAL LOAD CASES: (4)				- PART 9 OF CBC 2018, ABC 2019			
GABLE STUDS SPACED AT 2-0-0 OC.				CHORDS				- PART 9 OF CBC 2012 (2019 AMENDMENT)			
				MEMB. MAX. FACTORED FORCE (LBS)				- CSA 086-14			
				MEMB. FACTORED VERT. LOAD (PL)				- TPIC 2014			
				MEMB. MAX. FACTORED FORCE (LBS)				(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.7 P.S.F. SPECIFIED ROOF LIVE LOAD			
				MEMB. MAX. FACTORED FORCE (LBS)				CSI: TC=0.13/1.00 (J-K-1), BC=0.03/1.00 (S-T-4), WB=0.16/1.00 (F-Q-1), SSI=0.09/1.00 (B-C-1)			
				MEMB. MAX. FACTORED FORCE (LBS)				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
				MEMB. MAX. FACTORED FORCE (LBS)				COMPANION LIVE LOAD FACTOR = 1.00			
				MEMB. MAX. FACTORED FORCE (LBS)				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				MEMB. MAX. FACTORED FORCE (LBS)				NAIL VALUES			
				MEMB. MAX. FACTORED FORCE (LBS)				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
				MEMB. MAX. FACTORED FORCE (LBS)				MAX MIN MAX MIN MAX MIN			
				MEMB. MAX. FACTORED FORCE (LBS)				MT20 850 371 1747 788 1987 1873			
				MEMB. MAX. FACTORED FORCE (LBS)				PLATE PLACEMENT TOL. = 0.250 inches			
				MEMB. MAX. FACTORED FORCE (LBS)				PLATE ROTATION TOL. = 5.0 Deg.			
				MEMB. MAX. FACTORED FORCE (LBS)				JSI GRIP= 0.57 (T) (INPUT = 0.90)			
				MEMB. MAX. FACTORED FORCE (LBS)				JSI METAL= 0.13 (I) (INPUT = 1.00)			

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428511	T200		1	GREEN PARK HOMES JT 53080	E22085820(2)
Alpa Roof Truss, Maple					

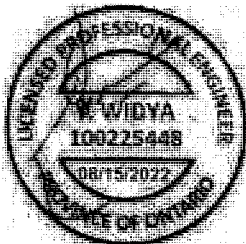
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Aug 15 13:19:28 2022 Page 2
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CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5 } PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

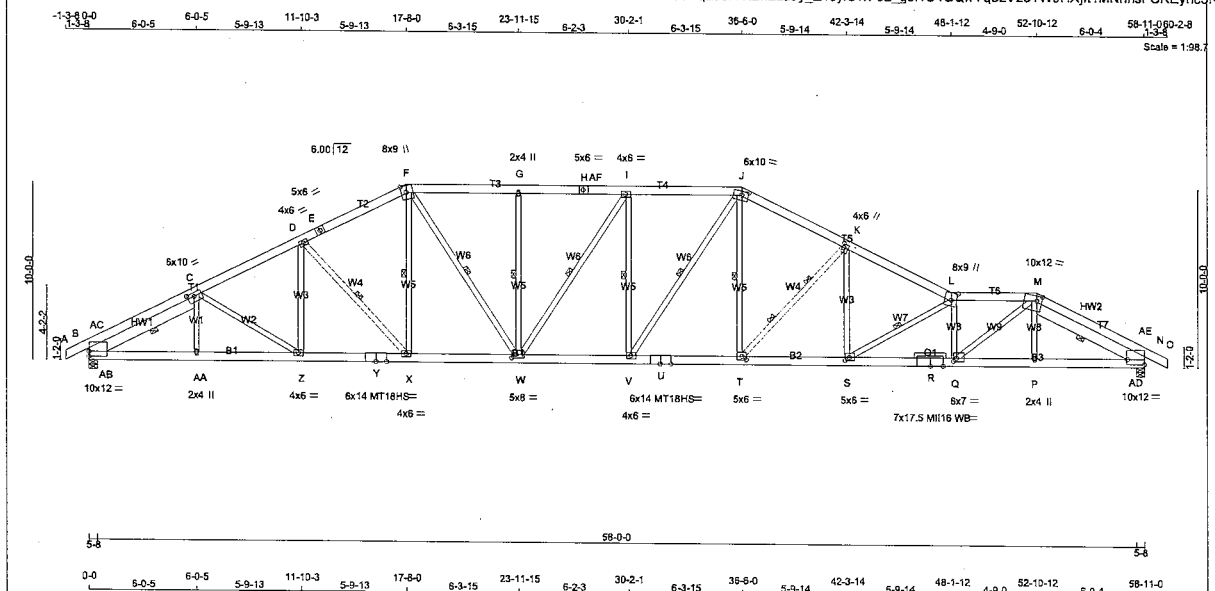
LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428511	T201		1	GREEN PARK HOMES JT 53080	E22085821

Alpa Roof Truss, Maple

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ID:To6XqL09YKbXLL69y_Z1eyrbTx-6L_g61C4GQl77q2V23TW0Hxjft1MnhsPSREyncR



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - E	2x6 DRY	No.2	SPF
E - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
J - L	2x6 DRY	No.2	SPF
L - M	2x6 DRY	No.2	SPF
M - O	2x6 DRY	No.2	SPF
O - Y	2x6 DRY	2100F 1.8E	SPF
Y - U	2x6 DRY	2100F 1.8E	SPF
U - R	2x6 DRY	2100F 1.8E	SPF
R - N	2x6 DRY	2100F 1.8E	SPF

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x8 DRY	No.2	SPF
HW2	2x8 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF
DRY, SEASONED LUMBER			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TBMW-1	MT20	10.0	12.0	3.50
C TBMW-1	MT20	6.0	10.0	2.25 4.50
D TBMW-1	MT20	4.0	6.0	
E TS-1	MT20	5.0	6.0	
F TTMW+m	MT20	8.0	9.0	Edge 3.25
G TTMW+w	MT20	2.0	4.0	
H TS-1	MT20	5.0	6.0	
I TTMW-1	MT20	4.0	6.0	
J TTMW-m	MT20	6.0	10.0	2.75 4.50
K TBMW-1	MT20	4.0	6.0	2.75 1.00
L TTMW+m	MT20	8.0	9.0	5.25 3.50
M TTMW-m	MT20	10.0	12.0	3.50 3.50
N TBMW-1	MT20	10.0	12.0	3.50 Edge
P BMW+w	MT20	2.0	4.0	
Q BMW-1	MT20	6.0	7.0	2.25 2.00
R BS-1	MT16	7.0	17.5	
S BMW-1	MT20	5.0	6.0	2.00 2.75
T BMW-1	MT20	5.0	6.0	2.25 2.75
U BS-1	MT18HS	5.0	6.0	14.0
V, X, Z				
V BMW-1	MT20	4.0	6.0	
W BMW-1	MT20	5.0	6.0	2.25 2.75
Y BS-1	MT18HS	6.0	14.0	
AA BMW+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION		GROSS REACTION	BRG	BRG
JT VERT	5001	DOWN	141	5-8
HORIZ	0	HORIZ	215	5-8
N	4982	0	-263	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 215 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 263 LBS FACTORED UPLIFT
PROVIDE FOR 141 LBS FACTORED HORIZONTAL REACTION AT JOINTS

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED			
B	3692	2165 / 0	619 / 0
N	3676	2165 / 0	619 / 0
HORIZONTAL REACTIONS			
B	0 / 0	0 / 0	101 / -101
N	0 / 0	0 / 0	101 / -101

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 1.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-X, F-X, J-T, L-S, J-V, F-W, I-V, G-W, I-W, M-AD, C-AB.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF K-T.

LOADING
TOTAL LOAD CASES: (18)

CHORDS	MAX. FACTORED	FACTORED	WELLS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	FR-TO	(LBS)
A-B	0 / 1	-120.3 -120.3 0.09 (2)	AA-C	0 / 291
B-AC	-5823 / 183	-120.3 -120.3 0.34 (1)	C-Z	-380 / 193
AC-C	-3891 / 240	-120.3 -120.3 0.27 (2)	Z-D	-5 / 436
C-D	-7851 / 301	-120.3 -120.3 0.67 (1)	D-X	-1269 / 217
D-E	-7132 / 275	-120.3 -120.3 0.61 (1)	X-F	-81 / 1207
E-F	-7132 / 275	-120.3 -120.3 0.61 (1)	F-T	-136 / 2287
F-G	-7362 / 249	-132.8 -132.8 0.67 (1)	T-K	-2746 / 292
G-AF	-7362 / 249	-132.8 -132.8 0.69 (1)	K-S	-45 / 1933
AF-H	-7362 / 249	-132.8 -132.8 0.69 (1)	S-L	-3087 / 231
H-I	-7362 / 249	-132.8 -132.8 0.69 (1)	L-Q	-3322 / 188
I-J	-7622 / 304	-132.8 -132.8 0.72 (1)	Q-M	-175 / 5379
J-K	-7972 / 351	-120.3 -120.3 0.68 (1)	M-P	0 / 383
K-L	-8841 / 421	-120.3 -120.3 0.88 (1)	P-V	-65 / 1275
L-M	-11292 / 519	-120.3 -120.3 0.83 (1)	V-W	-87 / 1951
M-AE	-3893 / 305	-120.3 -120.3 0.34 (3)	W-I	-850 / 190
AE-N	-5713 / 233	-120.3 -120.3 0.37 (3)	I-W	-997 / 122
N-O	0 / 1	-120.3 -120.3 0.09 (3)	O-I	-1426 / 746
B-AB	-266 / 3475	-39.5 -39.5 0.19 (1)	AB-AC	0 / 2550
AB-AA	-320 / 7152	-39.5 -39.5 0.40 (1)	AC-C	-4406 / 66
AA-Z	-321 / 7157	-39.5 -39.5 0.39 (1)	AD-AE	0 / 2400
Z-Y	-214 / 7036	-39.5 -39.5 0.39 (1)		
Y-X	-214 / 7036	-39.5 -39.5 0.39 (1)		
X-W	-70 / 6363	-39.5 -39.5 0.36 (1)		
W-V	0 / 7623	-39.5 -39.5 0.40 (1)		
V-U	0 / 7128	-39.5 -39.5 0.38 (1)		
U-T	0 / 7128	-39.5 -39.5 0.38 (1)		
T-S	-189 / 8839	-39.5 -39.5 0.46 (1)		
S-R	-384 / 11422	-39.5 -39.5 0.66 (1)		
R-Q	-384 / 11422	-39.5 -39.5 0.66 (1)		
Q-P	-240 / 7101	-39.5 -39.5 0.43 (1)		
P-AD	-242 / 7087	-39.5 -39.5 0.41 (1)		
AD-N	-174 / 3454	-39.5 -39.5 0.22 (1)		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM. INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.00) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 35.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 59.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018, ABC 2018
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.51")
ALLOWABLE DEFL.(TL) = L/80 (3.93")
CALCULATED VERT. DEFL.(TL) = L/972 (0.73")

CSI: TC=0.98/1.00 (K-L-1), BC=0.66/1.00 (Q-S-1), WB=0.98/1.00 (I-W-3), SS=0.34/1.00 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	650	371	1747
MT18HS	586	403	2455
MT16	438	302	2547

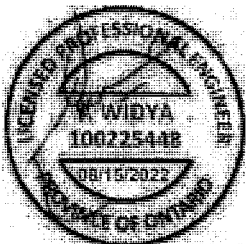
PLATE PLACEMENT TOL. = 0.250 inches

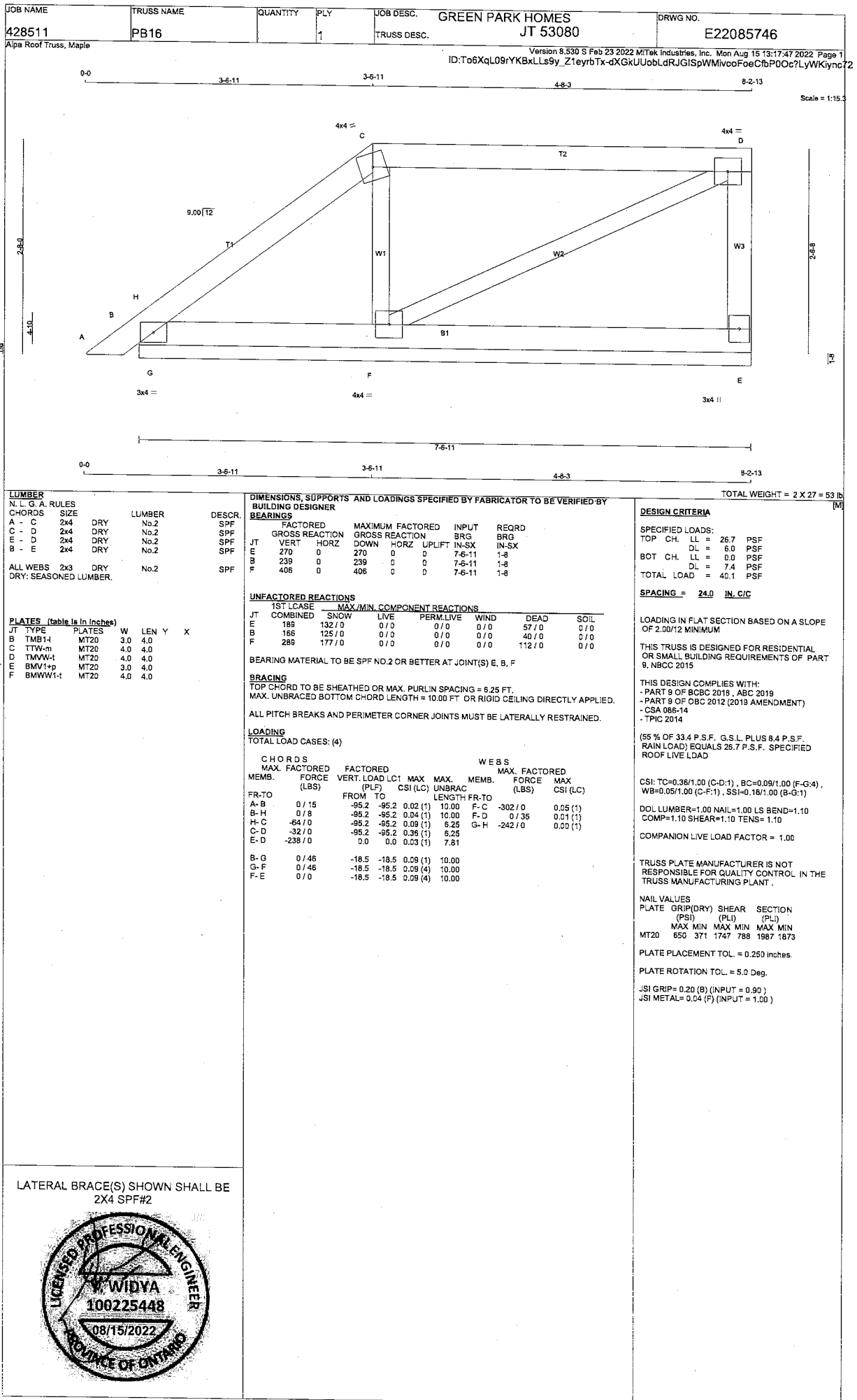
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)

JSI METAL= 0.99 (M) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

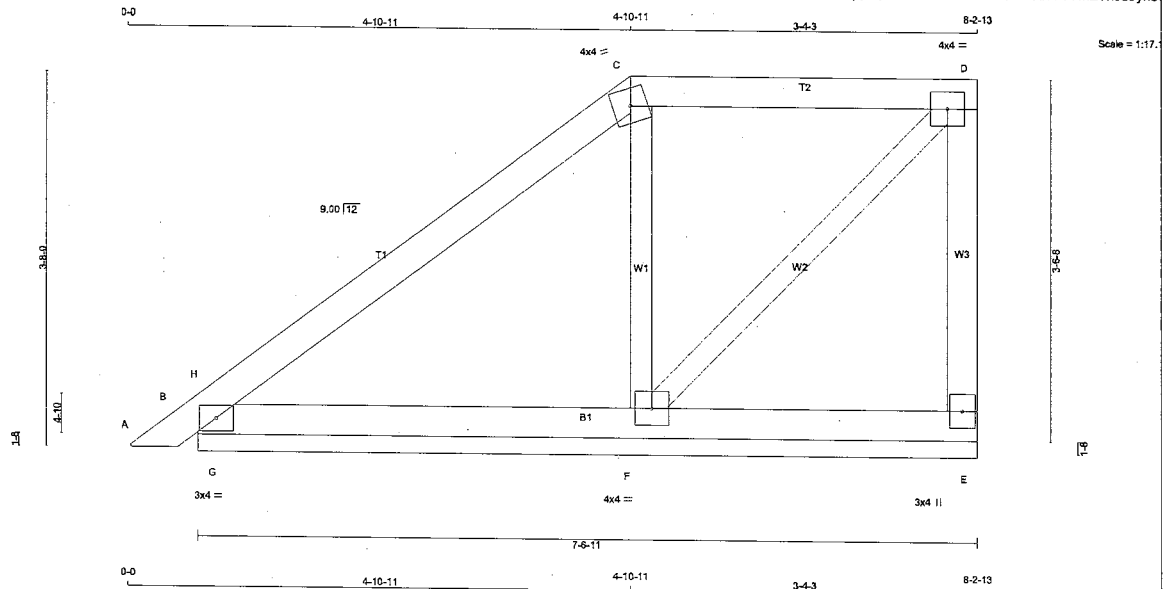




JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428511	PB17		1	TRUSS DESC.	JT 53080	E22085747

Alpa Roof Truss, Maple

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LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2		
C - D	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		
B - E	2x4	DRY	No.2		
ALL WEBS	2x3	DRY	No.2		
	DRY, SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMB1-4	MT20	3.0	4.0			
C TTW-m	MT20	4.0	4.0			
D TMVW-1	MT20	4.0	4.0			
E BMV1+p	MT20	3.0	4.0			
F BMVW1-4	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG
	VERT	HORZ				IN-SX	IN-SX
E	219	0	219	0	0	7-6-11	1-8
B	326	0	326	0	0	7-6-11	1-8
F	368	0	368	0	0	7-6-11	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	154	109 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0		
B	228	163 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0		
F	262	163 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		MAX. FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (P/LF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO					
A-B	0 / 15	-95.2	-95.2 0.02 (1)	10.00	F-C	-284 / 0	0.06 (1)
B-H	-21 / 91	-95.2	-95.2 0.11 (1)	6.25	F-D	0 / 73	0.02 (1)
H-C	-87 / 0	-95.2	-95.2 0.21 (1)	6.25	G-H	-459 / 0	0.00 (1)
C-D	-52 / 0	-95.2	-95.2 0.18 (1)	6.25			
E-D	-211 / 0	0.0	0.0 0.04 (1)	7.81			
B-G	0 / 61	-18.5	-18.5 0.18 (1)	10.00			
G-F	0 / 61	-18.5	-18.5 0.18 (1)	10.00			
F-E	0 / 0	-18.5	-18.5 0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-H-1), BC=0.18/1.00 (B-G-1), WB=0.06/1.00 (C-F-1), SSI=0.35/1.00 (B-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

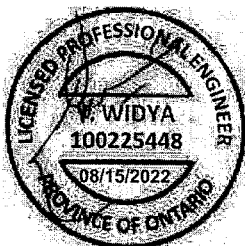
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

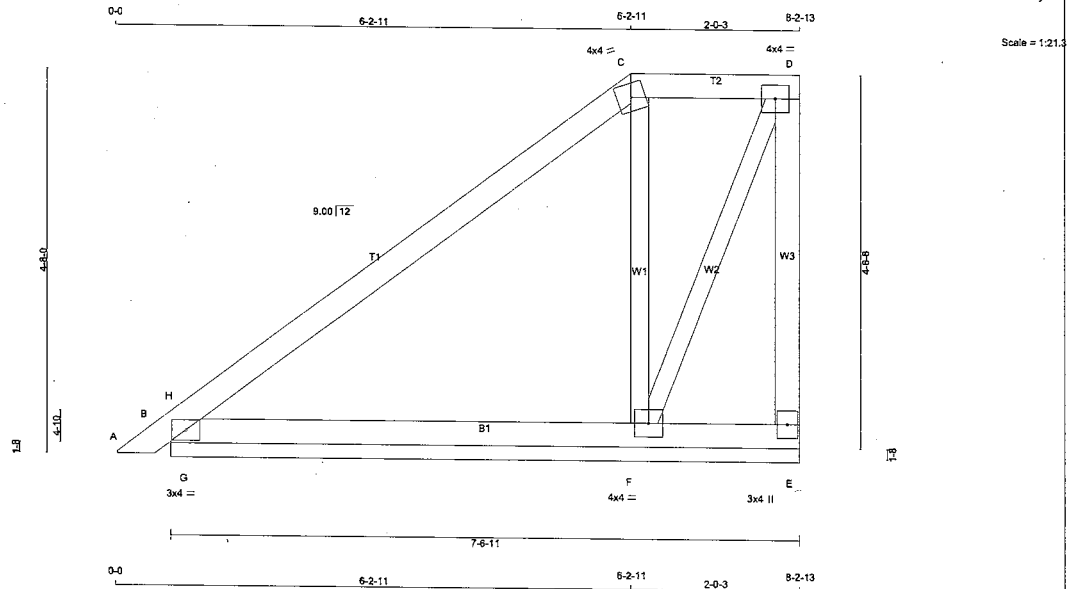
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428511	PB18		1	TRUSS DESC.	JT 53080	E22085748

Alpa Roof Truss, Maple

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ID:To6XqL09rYKbXLLs9y_Z1eyrbTx-ZwOVV9prtEh1VbcBenkNIDK81?HusvHvTIRdObync70



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
B TMB14	MT20	3.0	4.0	
C TTW-m	MT20	4.0	4.0	
D TMVW-1	MT20	4.0	4.0	
E BMV1+P	MT20	3.0	4.0	
F BMVW1-1	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	134	0	134	0
B	384	0	384	0
F	396	0	396	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
E	92	73 / 0
B	270	189 / 0
F	282	172 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX	CS1 (LC)
		(LBS)		(PLF)									(LBS)			
A-B		0 / 15	-95.2	-95.2	0.02 (1)	10.00	F-C	-284 / 0	0.09 (1)							
B-H		-10 / 290	-95.2	-95.2	0.24 (1)	6.25	F-D	0 / 102	0.02 (1)							
H-C		-77 / 0	-95.2	-95.2	0.36 (1)	6.25	G-H	-797 / 0	0.00 (1)							
C-D		-42 / 0	-95.2	-95.2	0.07 (1)	6.25										
E-D		-188 / 0	0.0	0.0	0.06 (1)	7.81										
B-G		0 / 49	-18.5	-18.5	0.30 (1)	10.00										
G-F		0 / 49	-18.5	-18.5	0.30 (1)	10.00										
F-E		0 / 0	-18.5	-18.5	0.19 (1)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAG = 40.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.38/1.00 (C-H:1), BC=0.30/1.00 (F-G:1), WB=0.09/1.00 (C-F:1), SS=0.61/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

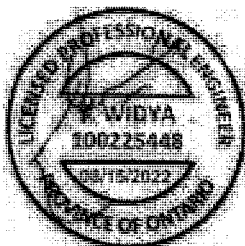
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

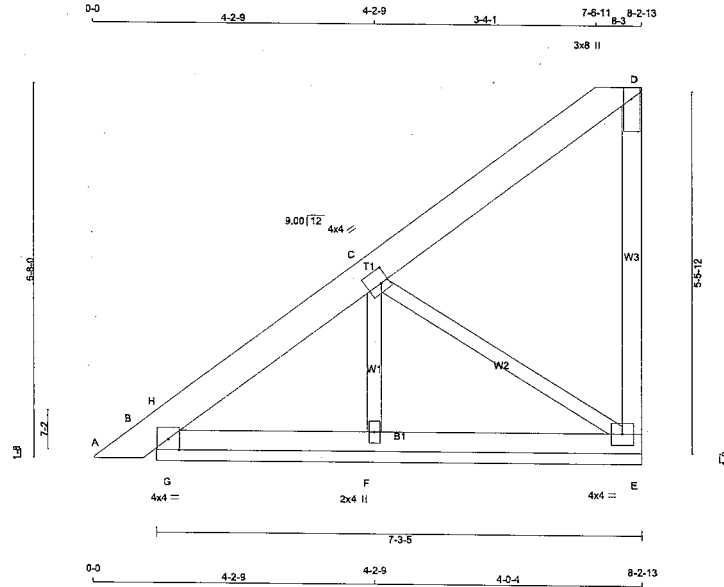
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	
A - D 2x6 DRY No.2	
E - D 2x4 DRY No.2	
G - E 2x4 DRY No.2	
ALL WEBS 2x3 DRY No.2	
DRY: SEASONED LUMBER.	

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMB1-t	MT20	4.0	4.0	Edge		
C TMMW1-t	MT20	4.0	4.0	2.50	1.50	
D TMV+tp	MT20	3.0	8.0	Edge		
E BMMW1-t	MT20	4.0	4.0			
F BMMW1+tp	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	240	0	240	0	0	7-3-5	1-8		
B	276	0	276	0	0	7-3-5	1-8		
F	387	0	387	0	0	7-3-5	1-8		

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	169	118 / 0	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0	0 / 0
B	192	143 / 0	0 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0	0 / 0
F	275	172 / 0	0 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH (FR-TO)
A-B	0 / 18	-95.2	-95.2 0.02 (1)	10.00	F-C	-300 / 0	0.05 (1)
B-H	-173 / 0	-95.2	-95.2 0.01 (4)	6.25	C-E	-108 / 0	0.04 (1)
H-C	-81 / 0	-95.2	-95.2 0.10 (1)	6.25	G-H	-21 / 26	0.00 (1)
C-D	-23 / 0	-95.2	-95.2 0.10 (1)	6.25			
E-D	-153 / 0	0.0	0.0 0.09 (1)	7.81			
B-G	0 / 91	-18.5	-18.5 0.03 (1)	10.00			
G-F	0 / 91	-18.5	-18.5 0.08 (4)	10.00			
F-E	0 / 91	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-H-1), BC=0.08/1.00 (F-G-4), WB=0.05/1.00 (C-F-1), SS=0.11/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	(PLI)
(PSI)	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

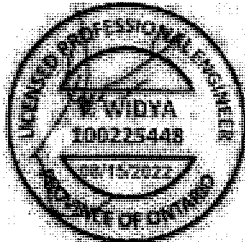
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (C) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)

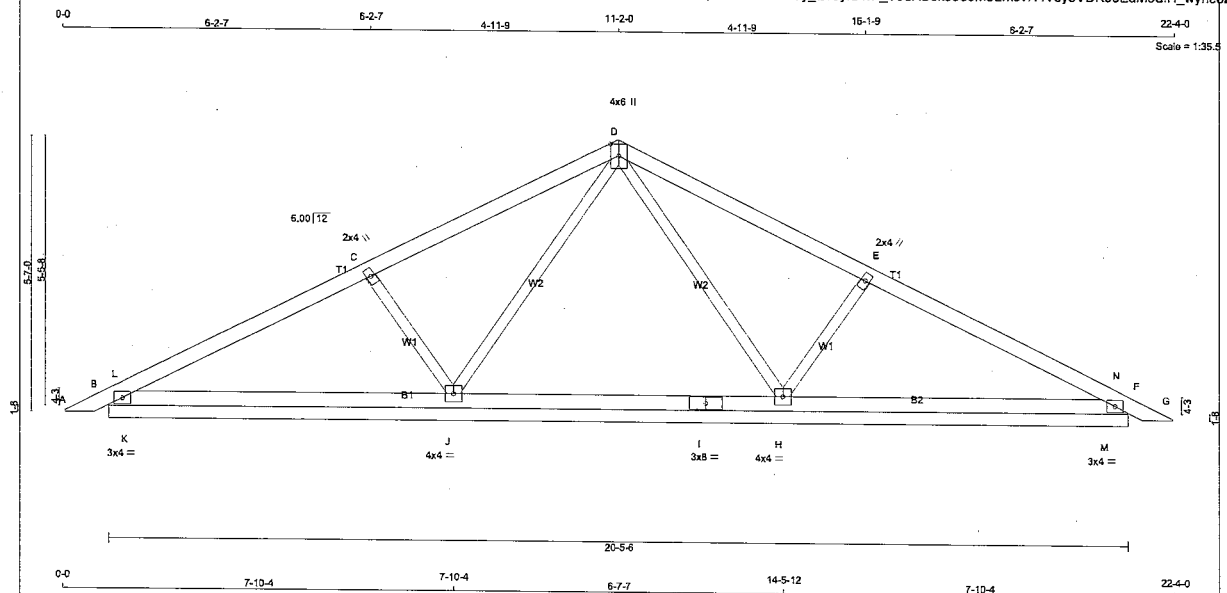
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME PB75	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085750
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Alpine Roof Truss, Maple

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ID:To6XqL09rYKBxLLs9y_Z1eyrbTx_V3dXBsk993cM3LmJvH4VsyVDK93EdM9dfH_wync6z



LUMBER	SIZE	LUMBER	DESCR.
N.L.G.A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - I	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMB1-1	MT20	3.0	4.0	
C TWW+P	MT20	2.0	4.0	
D TWW+P	MT20	4.0	6.0	Edge
E TWW+P	MT20	2.0	4.0	
F TMB1-1	MT20	3.0	4.0	
H BMWW1-1	MT20	4.0	4.0	
I BS-1	MT20	3.0	8.0	
J BMWW1-1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT GROSS REACTION				
VERT				
B	431	0	0	20-5-6 (11-11118)
H	812	0	0	20-5-6 (11-11118)
J	812	0	0	20-5-6 (11-11118)
F	431	0	0	20-5-6 (11-11118)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
B	303	210 / 0	0 / 0	0 / 0
H	573	382 / 0	0 / 0	0 / 0
J	573	382 / 0	0 / 0	0 / 0
F	303	210 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO										FR-TO				
A-B	0 / 17									D-H	-261 / 0			0.18 (1)
B-L	-270 / 0									H-E	-533 / 0			0.11 (1)
L-C	-209 / 0									J-D	-261 / 0			0.18 (1)
C-D	0 / 61									C-J	-533 / 0			0.11 (1)
D-E	0 / 61									K-L	-117 / 92			0.00 (1)
E-N	-209 / 0									M-N	-117 / 92			0.00 (1)
N-F	-270 / 0													
F-G	0 / 17													
B-K	0 / 213													
K-J	0 / 213													
J-I	0 / 63													
I-H	0 / 63													
H-M	0 / 213													
M-F	0 / 213													

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSH: TC=0.41/1.00 (C-D-1), BC=0.20/1.00 (H-M-4), WB=0.18/1.00 (D-J-1), SS=0.21/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

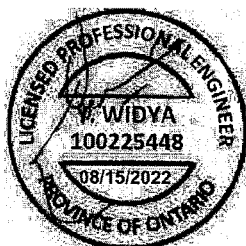
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (J) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 1.00)

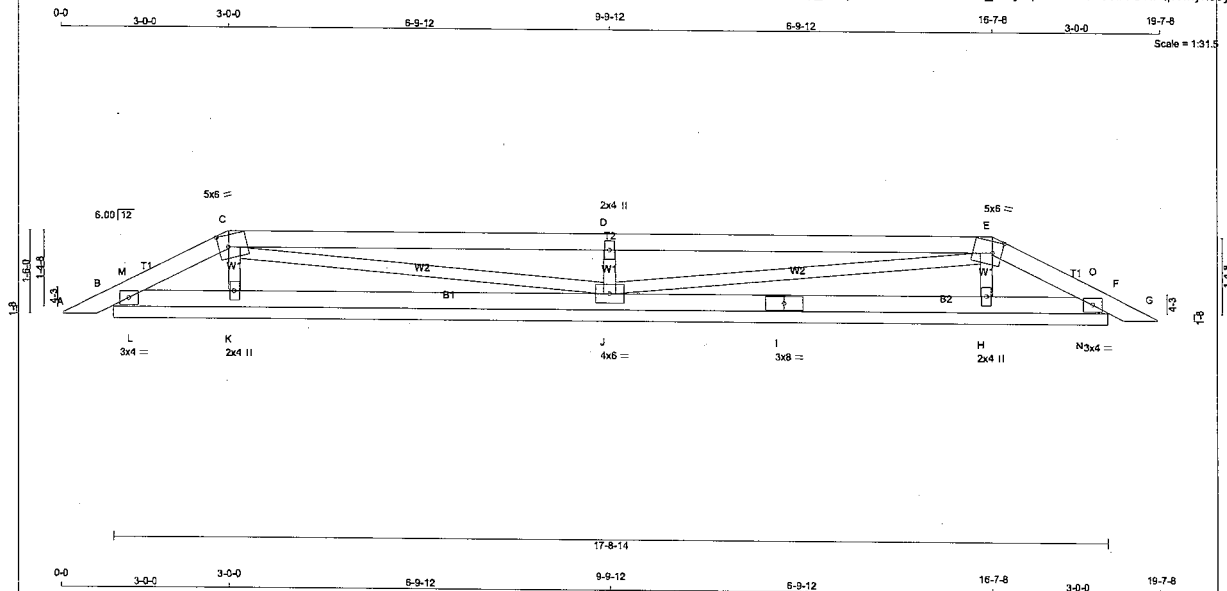
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME PB76	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085751
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Alpine Roof Truss, Maple

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ID:To6XqL09rYKbXLLs9y_Z1eyrTx-Shd?IXsMwTBT_DwytdpJ23UkxcheoirVOHPqWMyncbY



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2
	C - E	2x4	DRY	No.2
	E - G	2x4	DRY	No.2
	G - I	2x4	DRY	No.2
	I - F	2x4	DRY	No.2
	ALL WEBS	2x3	DRY	No.2
	DRY, SEASONED LUMBER.			

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	2.00
D	TTWW-m	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.00
F	TMB14	MT20	3.0	4.0		
H	BMW1-w	MT20	2.0	4.0		
I	BS-1	MT20	3.0	8.0		
J	BMWW1-1	MT20	4.0	6.0		
K	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	193	0	193	0
K	425	0	425	0
J	942	0	942	0
H	425	0	425	0
F	193	0	193	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
B	132	111/0 0/0 0/0 0/0 20/0 0/0
K	304	180/0 0/0 0/0 0/0 124/0 0/0
J	662	455/0 0/0 0/0 0/0 207/0 0/0
H	304	180/0 0/0 0/0 0/0 124/0 0/0
F	132	111/0 0/0 0/0 0/0 20/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB. FORCE (LBS)
FR-TO				
A-B	0/17	-95.2	-95.2 0.05 (1)	10.00
B-M	-33/7	-95.2	-95.2 0.02 (4)	6.25
M-C	-46/0	-95.2	-95.2 0.04 (1)	6.25
C-D	-2/0	-95.2	-95.2 0.75 (1)	10.00
D-E	-2/0	-95.2	-95.2 0.75 (1)	10.00
E-O	-46/0	-95.2	-95.2 0.04 (1)	6.25
O-F	-33/7	-95.2	-95.2 0.02 (4)	6.25
F-G	0/17	-95.2	-95.2 0.05 (1)	10.00
B-L	0/39	-18.5	-18.5 0.06 (1)	10.00
L-K	0/39	-18.5	-18.5 0.13 (4)	10.00
K-J	0/10	-18.5	-18.5 0.18 (4)	10.00
J-I	0/10	-18.5	-18.5 0.18 (4)	10.00
I-H	0/10	-18.5	-18.5 0.18 (4)	10.00
H-N	0/39	-18.5	-18.5 0.13 (4)	10.00
N-F	0/39	-18.5	-18.5 0.06 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.7 PSF
LL = 6.0 PSF
BOT CH. LL = 0.0 PSF
LL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 35.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.75/1.00 (C-D-1), BC=0.18/1.00 (J-K-4), WB=0.11/1.00 (D-J-1), SI=0.32/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

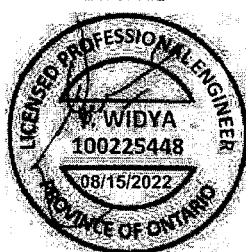
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.17 (I) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

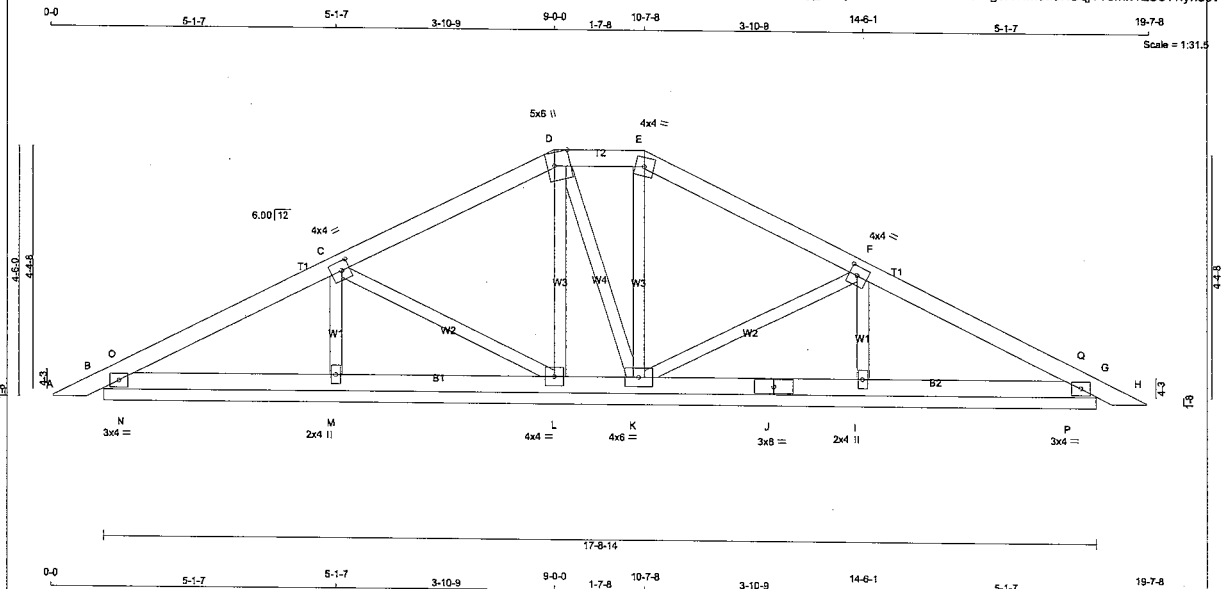


[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428511	PB78		1	TRUSS DESC.	JT 53080	E22085753

Alpa Roof Truss, Maple

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ID:To6XqL09rYKBxLLS9y_Z1eyroTx-sGJ8NZVEDOa1rgeXYIM0f6NBqjT73Mx4EeU7hynobv



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TMBW1-I	MT20	4.0	4.0	2.00	1.75	
D	TTWW+m	MT20	5.0	6.0	Edge	3.50	
E	TTW-m	MT20	4.0	4.0			
F	TMBW1-I	MT20	4.0	4.0	2.00	1.75	
G	TMB1-I	MT20	3.0	4.0			
J	BMW1+w	MT20	2.0	4.0			
K	BS-I	MT20	3.0	8.0			
L	BMWW1-I	MT20	4.0	6.0			
M	BMW1+w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
B	305	0	305	0
M	493	0	493	0
L	204	0	204	0
K	401	0	401	0
I	476	0	476	0
G	258	0	258	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	213	153 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
M	349	229 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0
L	146	89 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
K	281	200 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
I	337	219 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
G	209	149 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, L, K, I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 17		-95.2	-95.2	0.05 (1)	10.00	M-C	-377 / 0	0.06 (1)
B-O	-87 / 0		-95.2	-95.2	0.02 (4)	6.25	C-L	-84 / 0	0.03 (1)
O-C	-72 / 0		-95.2	-95.2	0.24 (1)	6.25	L-D	-126 / 0	0.04 (1)
C-D	-28 / 0		-95.2	-95.2	0.24 (1)	6.25	D-K	-93 / 0	0.03 (1)
D-E	0 / 27		-95.2	-95.2	0.05 (1)	10.00	K-E	-228 / 0	0.06 (1)
E-F	-2 / 4		-95.2	-95.2	0.24 (1)	10.00	K-F	-103 / 0	0.03 (1)
F-Q	-58 / 0		-95.2	-95.2	0.24 (1)	6.25	I-F	-360 / 0	0.06 (1)
Q-G	-72 / 0		-95.2	-95.2	0.02 (4)	6.25	N-O	-165 / 6	0.00 (1)
G-H	0 / 17		-95.2	-95.2	0.05 (1)	10.00	P-Q	-165 / 6	0.00 (1)
B-N	0 / 83		-18.5	-18.5	0.11 (1)	10.00			
N-M	0 / 83		-18.5	-18.5	0.11 (1)	10.00			
M-L	0 / 83		-18.5	-18.5	0.09 (1)	10.00			
L-K	0 / 5		-18.5	-18.5	0.03 (4)	10.00			
K-J	0 / 59		-18.5	-18.5	0.09 (4)	10.00			
J-I	0 / 59		-18.5	-18.5	0.09 (4)	10.00			
I-P	0 / 59		-18.5	-18.5	0.11 (1)	10.00			
P-G	0 / 69		-18.5	-18.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS DESIGN IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (E-F:1), BC=0.11/1.00 (M-N:1), WB=0.06/1.00 (E-K:1), SS=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

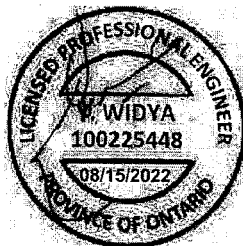
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (K) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

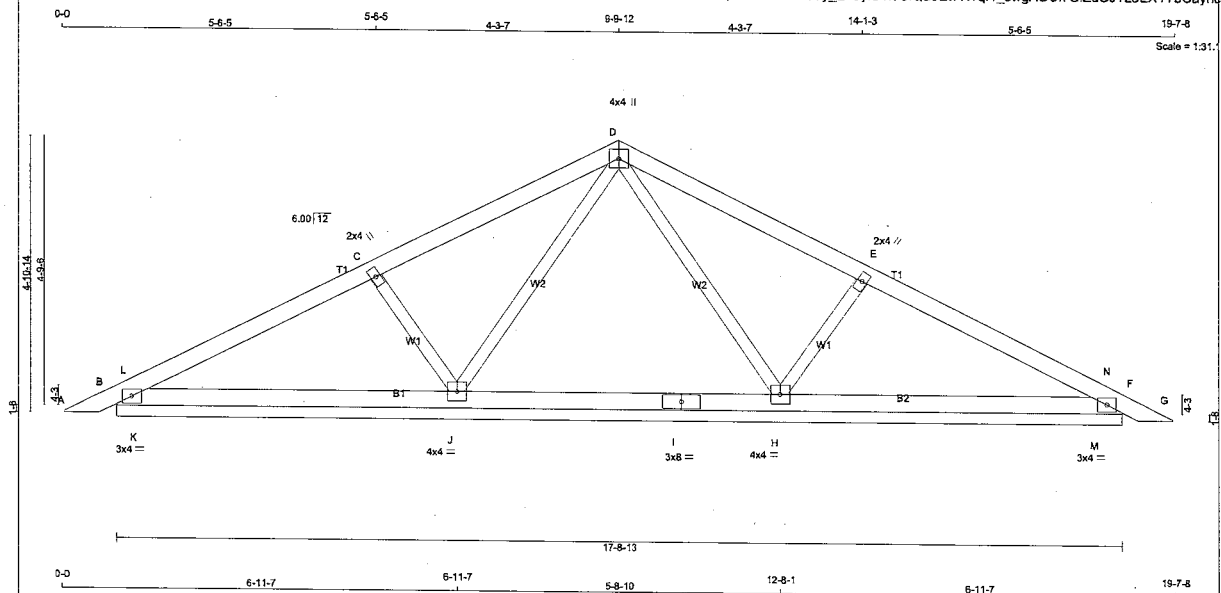


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JOB NAME 428511	TRUSS NAME PB80	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085755
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Alpha Roof Truss, Maple

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ID: To6XqL09rYKBxLLs9y_Z1eyrBtx-ofQuoEwVl7q14_owgAOUI7GZdOJTz8EXY7bCayn68t



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ORY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TWW+p	MT20	2.0	4.0		
D	TWW+p	MT20	4.0	4.0		
E	TWW+p	MT20	2.0	4.0		
F	TMB14	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	DOWN	UP	IN-SX
JT	VERT	HORZ	HORZ	IN-SX
B	383	0	383	0
H	706	0	706	0
J	706	0	706	0
F	383	0	383	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	269	188 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
H	499	332 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0
J	499	332 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0
F	269	188 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED	VERT. LOAD	LC1	MAX	MAX.	UNBRAC	W E B S	MEMB.	FORCE	FACTORED	MAX.	MAX.
FR-TO	MEMB.	(LBS)	FROM	TO	(PLP)	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	(LBS)	FROM	TO	CSI (LC)
A-B	0 / 17	-95.2	-95.2	0.05 (1)	10.00	D-H	-229 / 0	0.11 (1)						
B-L	-232 / 0	-95.2	-95.2	0.07 (4)	6.25	H-E	-462 / 0	0.08 (1)						
L-C	-173 / 0	-95.2	-95.2	0.30 (1)	6.25	J-D	-229 / 0	0.11 (1)						
C-D	0 / 59	-95.2	-95.2	0.31 (1)	10.00	C-J	-462 / 0	0.08 (1)						
D-E	0 / 59	-95.2	-95.2	0.31 (1)	10.00	K-L	-91 / 71	0.00 (1)						
E-N	-173 / 0	-95.2	-95.2	0.30 (1)	6.25	M-N	-91 / 71	0.00 (1)						
N-F	-232 / 0	-95.2	-95.2	0.07 (4)	6.25									
F-G	0 / 17	-95.2	-95.2	0.05 (1)	10.00									
B-K	0 / 178	-18.5	-18.5	0.10 (1)	10.00									
K-J	0 / 178	-18.5	-18.5	0.15 (4)	10.00									
J-I	0 / 50	-18.5	-18.5	0.14 (4)	10.00									
I-H	0 / 50	-18.5	-18.5	0.14 (4)	10.00									
H-M	0 / 178	-18.5	-18.5	0.15 (4)	10.00									
M-F	0 / 178	-18.5	-18.5	0.10 (1)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-D:1), BC=0.15/1.00 (H-M:4), WB=0.11/1.00 (D-H:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

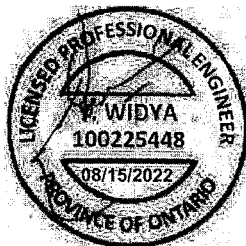
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL) (PSI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

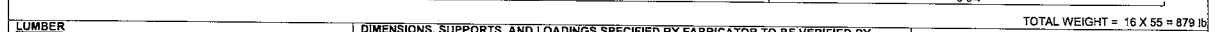
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (J) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTMW+p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMB1-t	MT20	3.0	4.0		
H	BMVWV-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	8.0		
J	BMVWV-t	MT20	4.0	4.0		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIE

LOADING

TOTAL LOAD CASES: (4)

CHORDS	WORDS
1	1
2	2
3	3
4	4

MEMB. FACTORED			FACTORED			MEMB. FACTORED		
MEMB.	FORCE	VERT. LOAD	MAX.	MAX.	MEMB.	FORCE	MAX.	
(LBS)	(PLF)	CSI (L)	UNBRAC	FR-TO	(LBS)	CSI (L)		
FR-TO	FROM TO	LENGTH	FR-TO					
B	0 1/7	-95.2 -95.2 0.05 (1)	10.00	D-H	-219/0	0.10 (1)		
B-L	-221/0	-95.2 -95.2 0.07 (1)	6.25	H-E	-44/10	0.08 (1)		
L-C	-163/0	-95.2 -95.2 0.28 (1)	6.25	J-D	-219/0	0.10 (1)		
C-D	0 5/8	-95.2 -95.2 0.29 (1)	10.00	C-J	-441/0	0.08 (1)		
D-E	0 5/8	-95.2 -95.2 0.29 (1)	10.00	K-L	-84/65	0.00 (1)		
E-N	-163/0	-95.2 -95.2 0.28 (1)	6.25	M-N	-84/65	0.00 (1)		
N-F	-221/0	-95.2 -95.2 0.07 (1)	6.25					
F-G	0 1/7	-95.2 -95.2 0.05 (1)	10.00					
B-K	0 1/69	-18.5 -18.5 0.09 (1)	10.00					
K-J	0 1/69	-18.5 -18.5 0.14 (4)	10.00					
J-I	0 1/48	-18.5 -18.5 0.13 (4)	10.00					
I-H	0 1/45	-18.5 -18.5 0.13 (4)	10.00					
H-M	0 1/69	-18.5 -18.5 0.14 (4)	10.00					
M-F	0 1/69	-18.5 -18.5 0.09 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

D. CSI: TC=0.29/1.00 (C-D:1), BC=0.14/1.00 (J-K:4),

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURED IN U.S.A.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

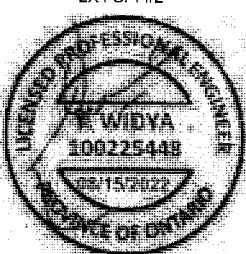
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (H) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

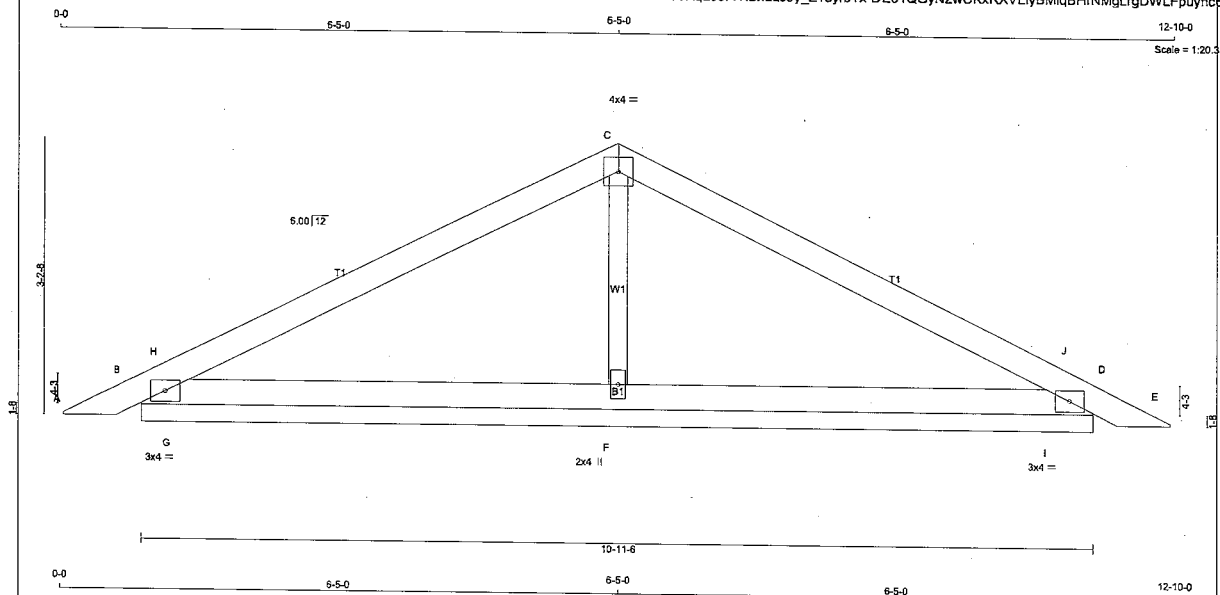
LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085757
428511	PB83		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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ID:To6XqL09rYKbXLLs9y_Z1eyrbTx-DE61QGyN2wCkRXVLyBMlqBhRNMgDLrgDWLFpuyncsq



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
		DRY, SEASONED LUMBER.			

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-4	MT20	3.0	4.0			
C	TTW-P	MT20	4.0	4.0			
D	TMB1-4	MT20	3.0	4.0			
F	BMW1-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT	REORD
	VERT	HORZ	DOWN	HORZ		
B	444	0	444	0	10-11-6	1-8
D	444	0	444	0	10-11-6	1-8
F	517	0	517	0	10-11-6	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	311	223/0	0/0	0/0	0/0	88/0	0/0
D	311	223/0	0/0	0/0	0/0	88/0	0/0
F	368	230/0	0/0	0/0	0/0	138/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 17	-95.2	-95.2	0.05 (1)	10.00	F-C	-268 / 0 0.05 (1)
B-H	-50 / 0	-95.2	-95.2	0.16 (1)	6.25	G-H	-554 / 0 0.00 (1)
H-C	-199 / 0	-95.2	-95.2	0.34 (1)	6.25	I-J	-554 / 0 0.00 (1)
C-J	-199 / 0	-95.2	-95.2	0.34 (1)	6.25		
J-D	-50 / 0	-95.2	-95.2	0.16 (1)	6.25		
D-E	0 / 17	-95.2	-95.2	0.05 (1)	10.00		
B-G	0 / 169	-18.5	-18.5	0.32 (1)	10.00		
G-F	0 / 169	-18.5	-18.5	0.32 (1)	10.00		
F-I	0 / 169	-18.5	-18.5	0.32 (1)	10.00		
I-D	0 / 169	-18.5	-18.5	0.32 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 080-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (C-J-1), BC=0.32/1.00 (D-I-1), WB=0.05/1.00 (C-F-1), SSI=0.41/1.00 (B-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

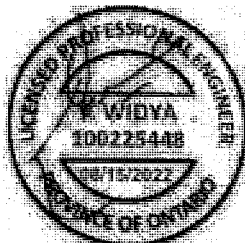
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

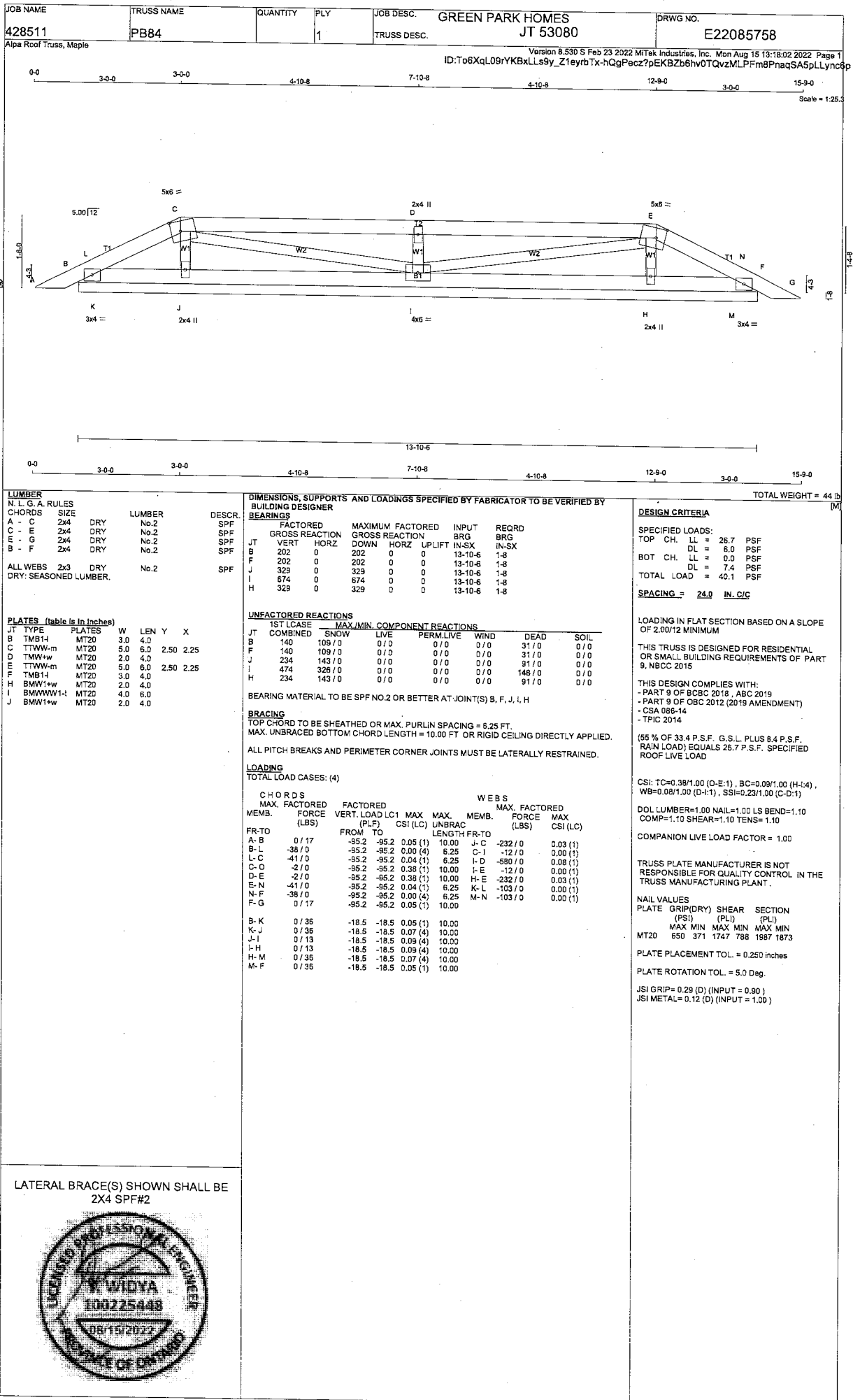
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.50)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

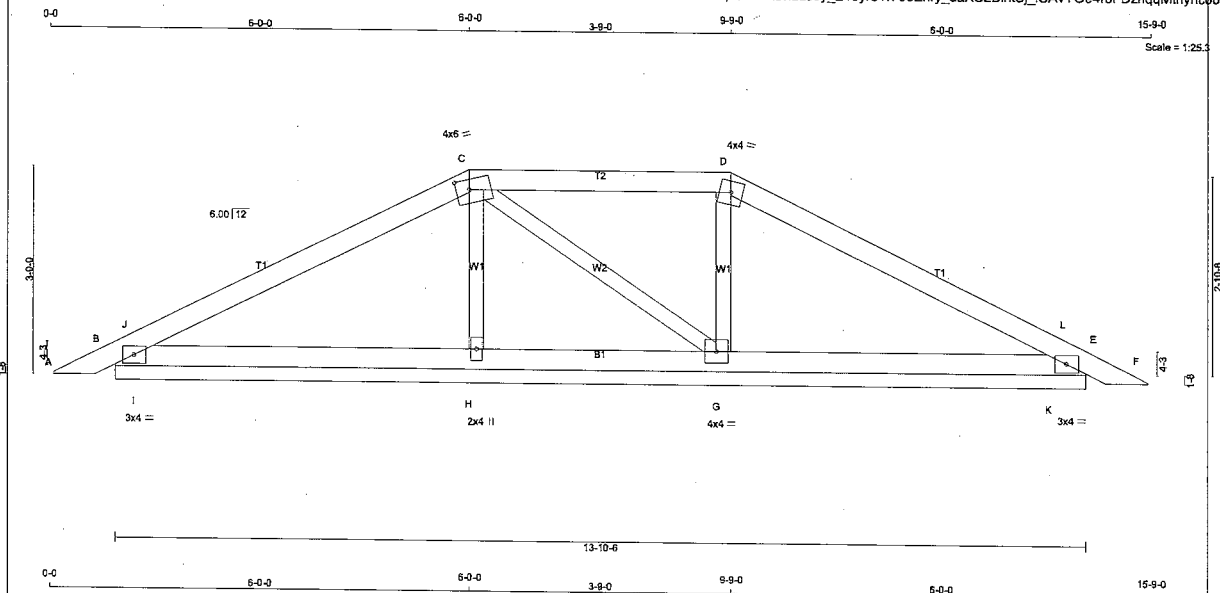




JOB NAME 428511	TRUSS NAME PB85	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085759
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Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Mon Aug 15 13:18:03 2022 Page 1
ID: To6XqL09rYKBxLLs9y_Z1eyrbTx-9cEnry_daXS2BhSj_fSAvYCe4r8FDzhqqMlnynco6



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 42 lb	
N. L. G. A. RULES				BEARINGS										DESIGN CRITERIA	
CHORDS				SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	SPECIFIED LOADS:		
A - C				2x4	DRY	SPF	DOWN		UP		IN-SX	IN-SX	TOP CH. LL = 26.7 PSF		
C - D				2x4	DRY	SPF	VERT		HORIZ				DL = 6.0 PSF		
D - F				2x4	DRY	SPF	DOWN		UP				BOT CH. LL = 0.0 PSF		
B - E				2x4	DRY	No.2	JT		E				DL = 7.4 PSF		
ALL WEBS				2x3	DRY	No.2	SPF		H				TOTAL LOAD = 40.1 PSF		
DRY, SEASONED LUMBER.							G		486						

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TM81-I	MT20	3.0	4.0	
C	TTWW-m	MT20	4.0	5.0	1.75 2.25
D	TTWW-m	MT20	4.0	4.0	
E	TM81-I	MT20	3.0	4.0	
G	BMWW1-I	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
A-B	0/17	-55.2	-95.2 0.05 (1)	10.00	H-C	-280/0	0.05 (1)
B-J	-85/0	-55.2	-95.2 0.10 (1)	6.25	C-G	-49/0	0.02 (1)
J-C	-176/0	-55.2	-95.2 0.31 (1)	6.25	G-D	-328/0	0.06 (1)
C-D	-100/0	-55.2	-95.2 0.23 (1)	6.25	I-J	-421/0	0.00 (1)
D-L	-133/0	-55.2	-95.2 0.31 (1)	6.25	K-L	-423/0	0.00 (1)
L-E	-48/0	-55.2	-95.2 0.10 (1)	6.25			
E-F	0/17	-55.2	-95.2 0.05 (1)	10.00			
B-I	0/151	-18.5	-18.5 0.25 (1)	10.00			
I-H	0/151	-18.5	-18.5 0.25 (1)	10.00			
H-G	0/140	-18.5	-18.5 0.13 (1)	10.00			
G-K	0/113	-18.5	-18.5 0.25 (1)	10.00			
K-E	0/113	-18.5	-18.5 0.25 (1)	10.00			

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-J-1), BC=0.25/1.00 (B-I-1), WB=0.06/1.00 (D-G-1), SSI=0.33/1.00 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

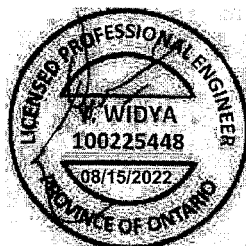
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	550	371	1747 798 1987 1873

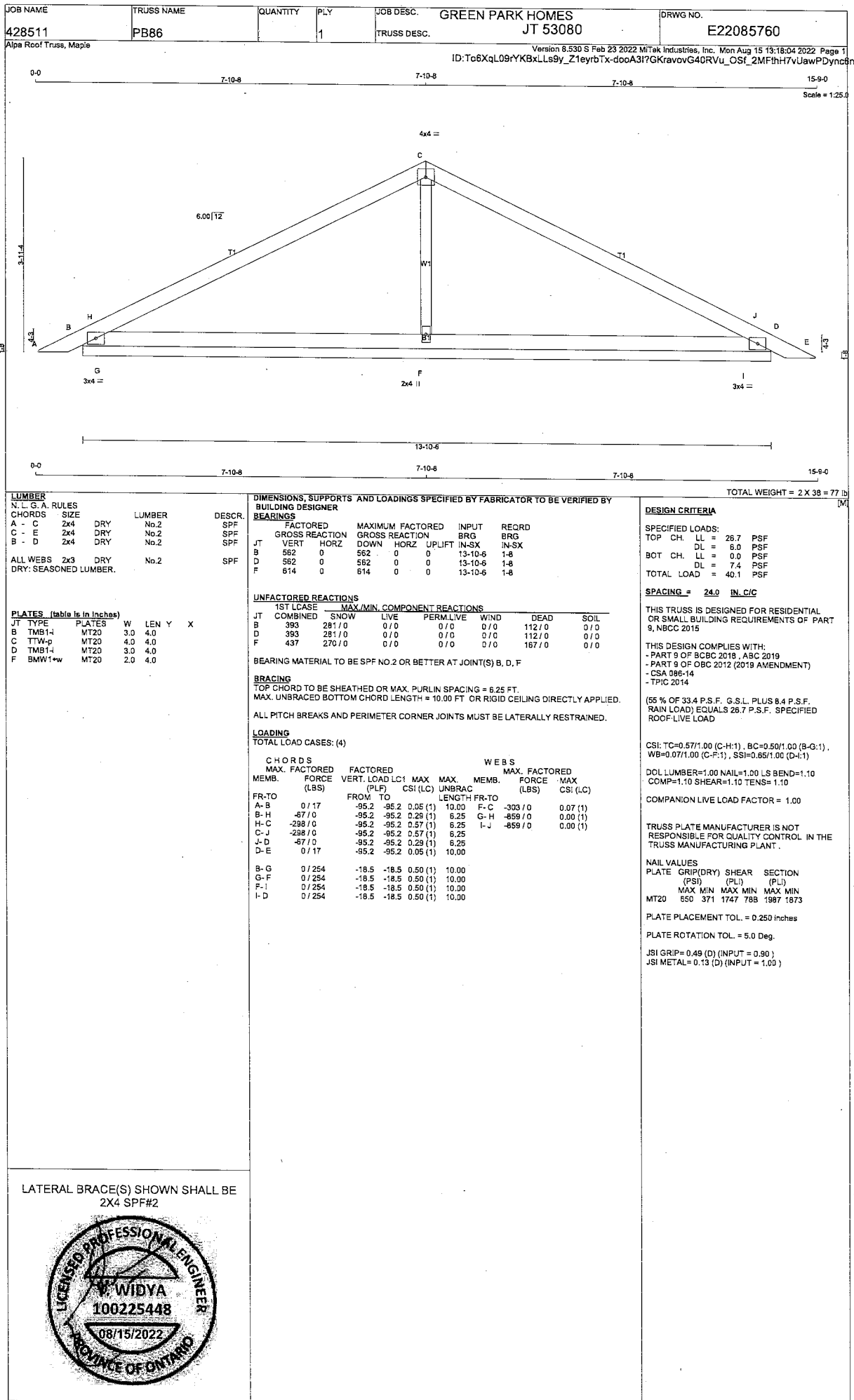
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

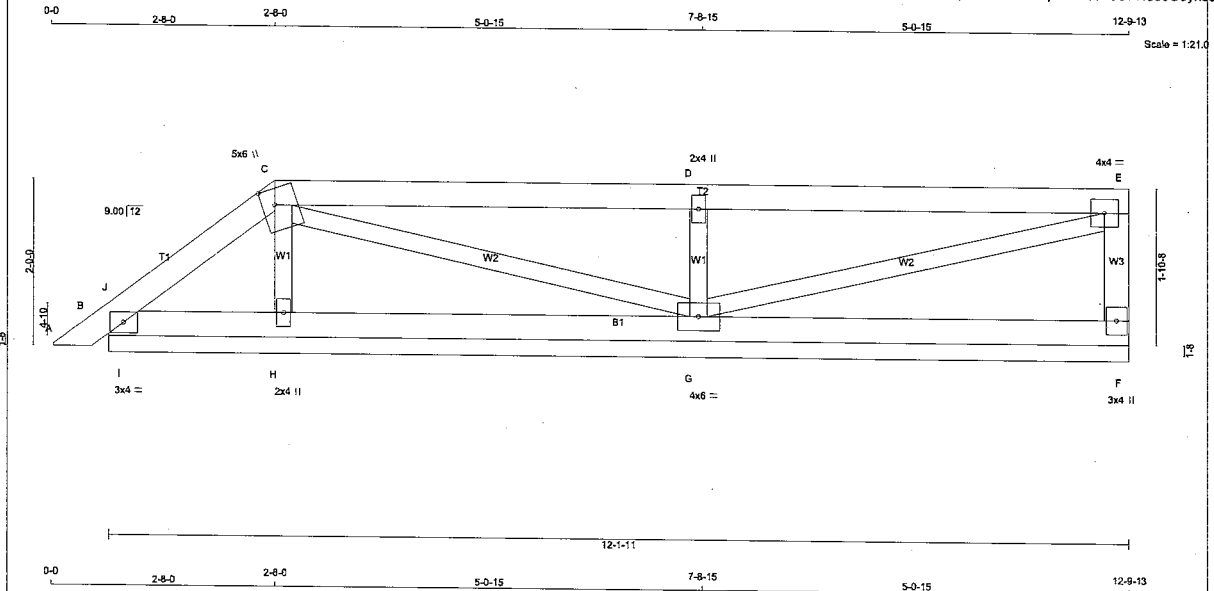




JOB NAME 428511	TRUSS NAME PB87	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085761
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Alpha Roof Truss, Maple

Version 8.530 S. Feb 23 2022 Mitak Industries, Inc. Mon Aug 15 13:18:06 2022 Page 1
ID:To6XqL09:YKbXLLs9y_Z1eyrbTx-ZBvwTz0WwSqd2DPS8sXm3pX1us8aLbUPNo30U6yncb0



LUMBER	SIZE	LUMBER	DESCR.
N.L.G.A. RULES			
CHORDS			
A - C	2x4	DRY	SPF
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			SPF

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMB1-t	MT20	3.0	4.0	
C TTMW+m	MT20	5.0	6.0	2.25 1.75
D TMW+w	MT20	2.0	4.0	
E TMW+h	MT20	4.0	4.0	
F BMV1+p	MT20	3.0	4.0	
G BMWVW1-t	MT20	4.0	6.0	
H BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	VERT 221	DOWN 0	0	12-1-11 1-8
B	188	0	0	12-1-11 1-8
H	306	0	0	12-1-11 1-8
G	720	0	0	12-1-11 1-8

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F	156	103 / 0 0 / 0 0 / 0 53 / 0 0 / 0
B	130	101 / 0 0 / 0 0 / 0 29 / 0 0 / 0
H	218	132 / 0 0 / 0 0 / 0 87 / 0 0 / 0
G	507	343 / 0 0 / 0 0 / 0 164 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO		
A-B	0 / 15	-95.2	-95.2 0.02 (1)	10.00	H-C	-215 / 0 0.03 (1)
B-J	-32 / 0	-95.2	-95.2 0.01 (1)	6.25	C-G	-29 / 0 0.01 (1)
J-C	-56 / 0	-95.2	-95.2 0.04 (1)	6.25	G-D	-604 / 0 0.09 (1)
C-D	0 / 0	-95.2	-95.2 0.42 (1)	10.00	G-E	0 / 0 0.00 (1)
D-E	0 / 0	-95.2	-95.2 0.42 (1)	10.00	I-J	-129 / 0 0.00 (1)
F-E	-184 / 0	0.0	0.0 0.02 (1)	7.81		
B-I	0 / 42	-18.5	-18.5 0.05 (1)	10.00		
I-H	0 / 42	-18.5	-18.5 0.05 (4)	10.00		
H-G	0 / 27	-18.5	-18.5 0.12 (4)	10.00		
G-F	0 / 0	-18.5	-18.5 0.12 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (D-E:1), BC=0.12/1.00 (G-H:4), WB=0.09/1.00 (D-G:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

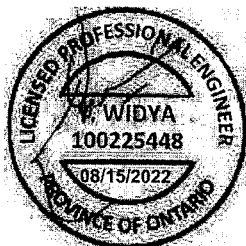
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 560 371 1747 788 1987 1873

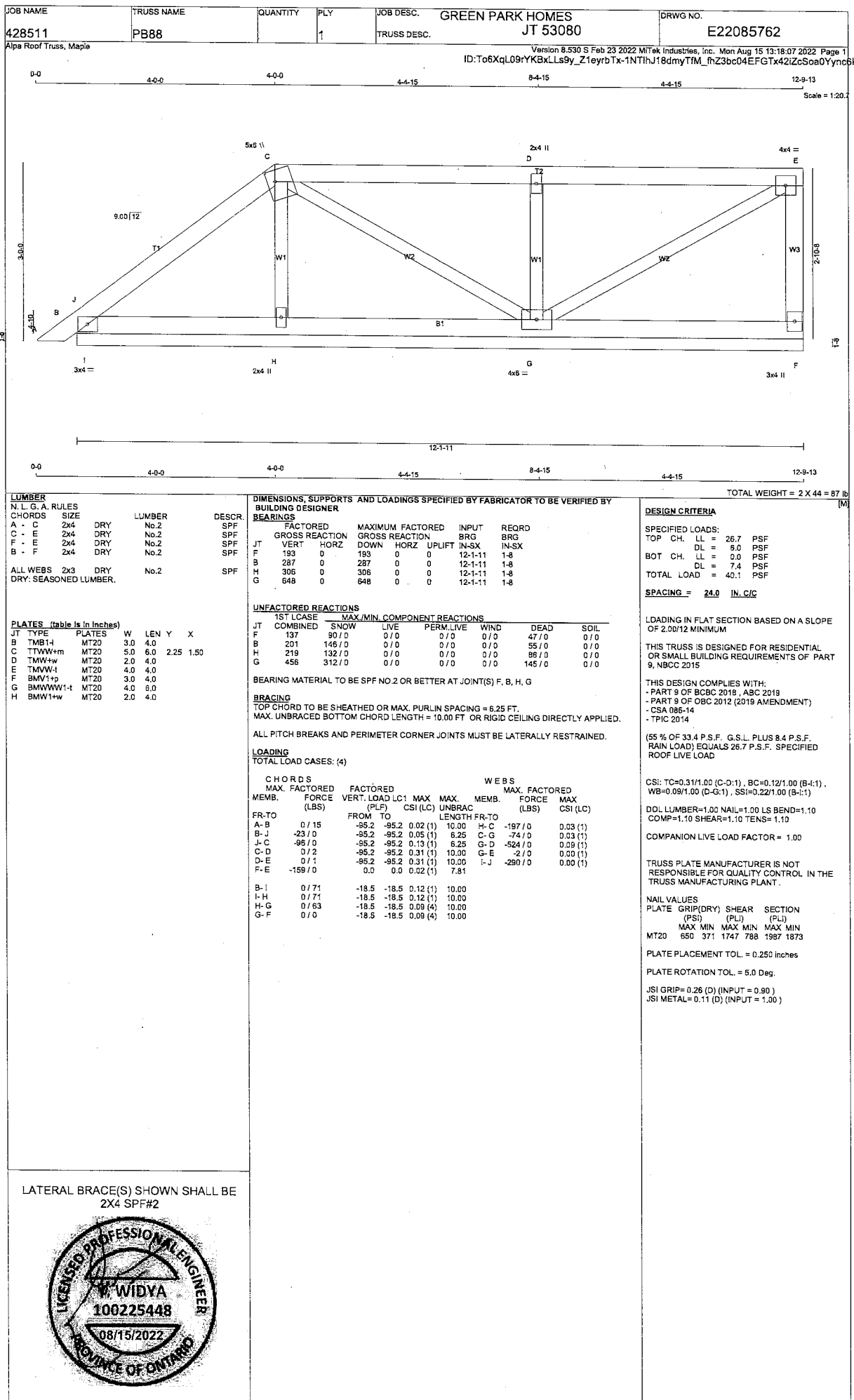
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.30 (D) (INPUT = 0.90)
JSI METAL = 0.13 (D) (INPUT = 1.00)

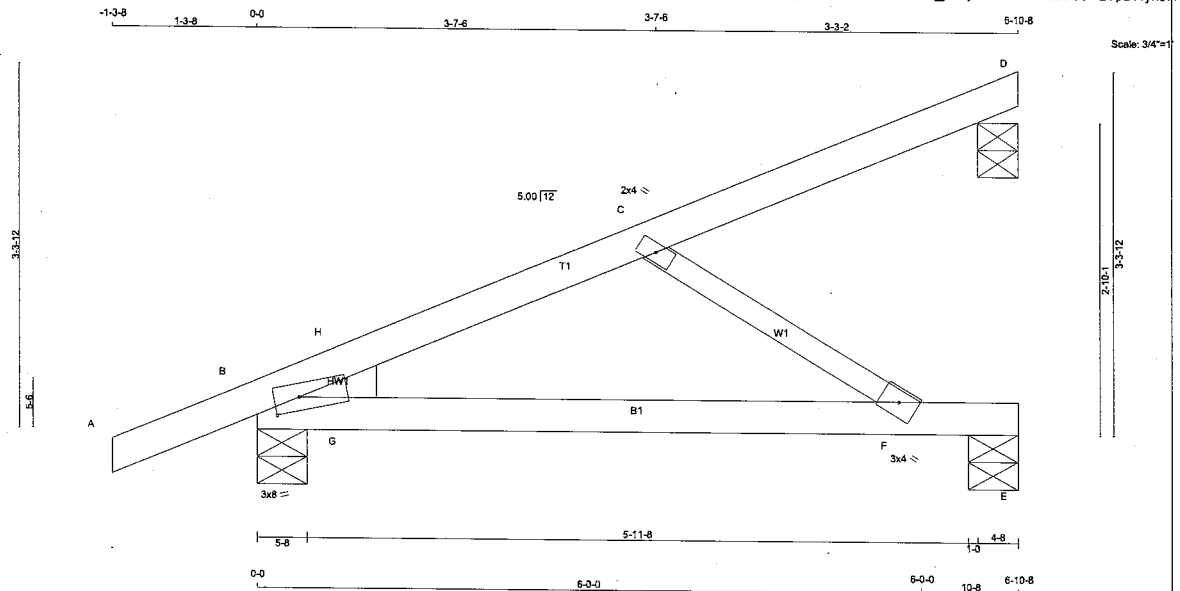
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mittek Industries, Inc. Mon Aug 15 13:17:32 2022 Page 1
ID:To6XqL09rYKbLLs9y_Z1eyrbTx-Zes3LLcB0YRz_pw8jwOhe7GM9S2OC5P2VpDI4ync7H



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
JT	TMH1-m	MT20	3.0	8.0	1.50	2.75
B	TMH1-m	MT20	2.0	4.0		
C	TMW+w	MT20	3.0	4.0		
F	BMW+w	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
D	134 0	135 0	-67 4-8	4-8
B	519 0	519 163	-183 5-8	1-8
E	257 0	257 0	-157 5-8	1-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 183 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 167 LBS FACTORED UPLIFT

PROVIDE FOR 163 LBS FACTORED HORIZONTAL REACTION AT JOINT

UNFACTORED REACTIONS	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
D	92	75/0	0/0	0/0
B	354	256/0	0/0	0/0
E	184	109/0	0/0	0/0

HORIZONTAL REACTIONS	B	0/0	0/0	0/0	116/0	0/0	0/0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)
FR-TO				
A-B	0/19	-95.2	-95.2 0.12 (1)	10.00
B-H	-502/131	-95.2	-95.2 0.17 (1)	6.25
H-C	-345/77	-95.2	-95.2 0.23 (1)	6.25
C-D	-33/0	-95.2	-95.2 0.12 (1)	6.25
B-G	-189/349	-18.5	-18.5 0.11 (1)	6.25
G-F	-189/349	-18.5	-18.5 0.42 (1)	6.25
F-E	0/0	-18.5	-18.5 0.36 (1)	10.00

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 3). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1,000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.23")
CALCULATED VERT. DEFL. (LL) = L/711 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (0.23")
CALCULATED VERT. DEFL. (TL) = L/438 (0.19")

CSI: TC=0.23/1.00 (C-H:1), BC=0.42/1.00 (F-G:1), WB=0.09/1.00 (C-F:1), SS=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

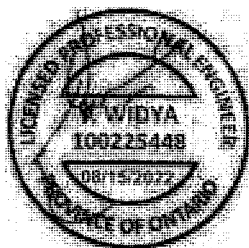
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

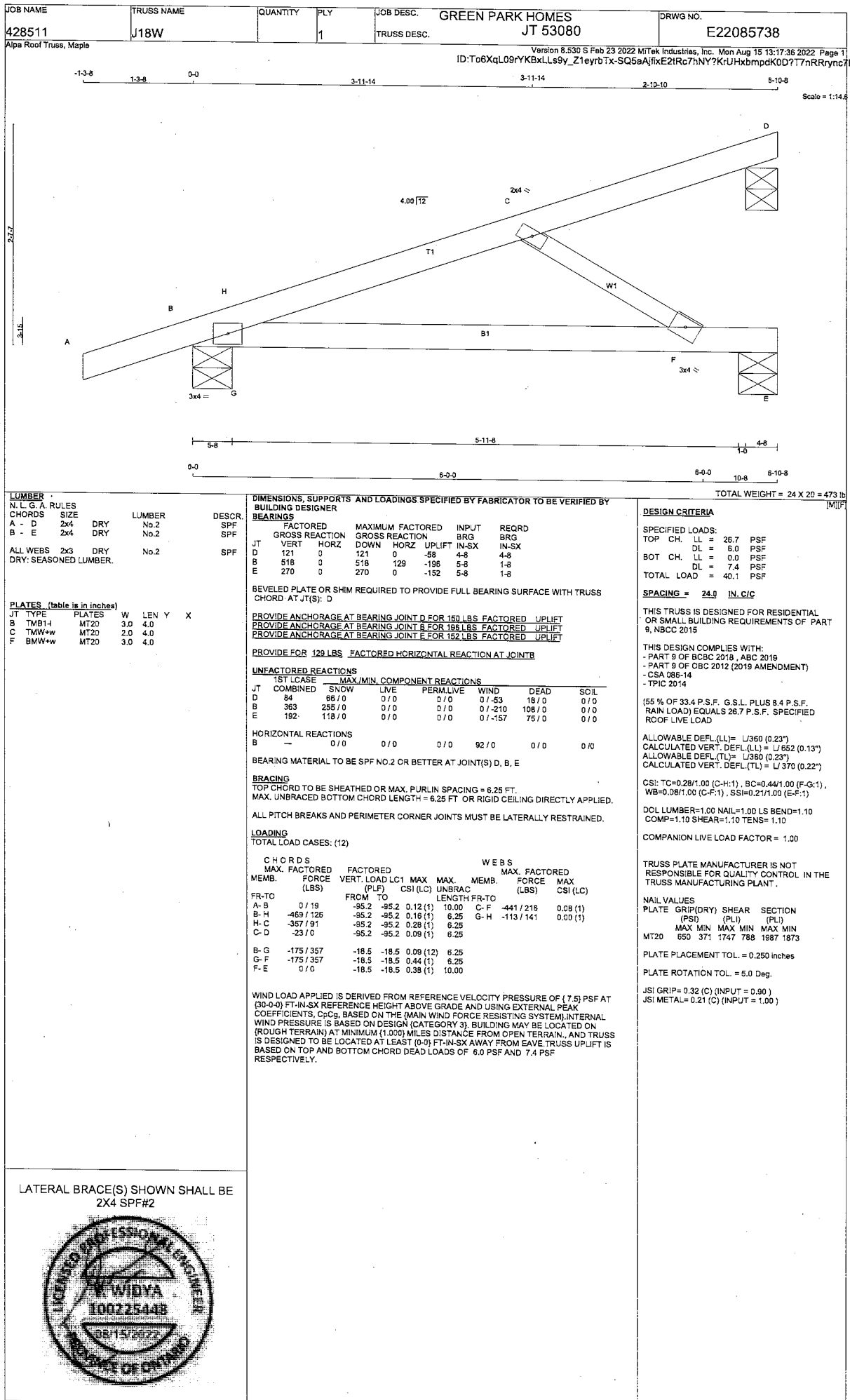
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.29 (C) (INPUT = 0.90)
JSI METAL = 0.20 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





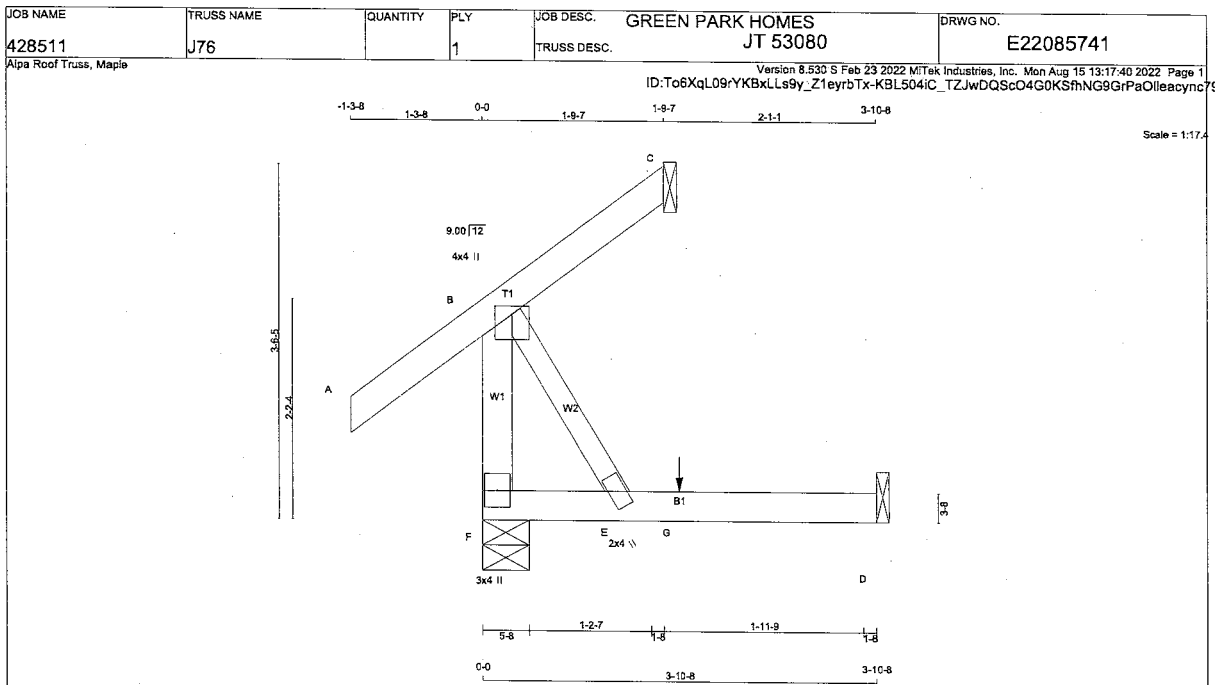




[M]

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

JOB NAME 428511	TRUSS NAME J75	QUANTITY 1	PLY 0-0	JOB DESC. GREEN PARK HOMES JT 53080	TRUSS DESC.	DRWG NO. E22085740
Alpa Roof Truss, Maple						
Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Mon Aug 15 13:17:38 2022 Page 1 ID:To6XqL09rYKBxLLs9y_Z1eyrbTx-OoDKbPhySslbhvG4Vz1oxvNlgaahoxvbxRGXVkync7B						
Scale: 1/2"=1'-0"						
TOTAL WEIGHT = 5 X 16 = 80 lb						
LUMBER						
N. L. G. A. RULES						
CHORDS SIZE LUMBER DESCR.						
F - B 2x4 DRY No.2 SPF						
A - C 2x4 DRY No.2 SPF						
F - D 2x4 DRY No.2 SPF						
ALL WEBS 2x3 DRY No.2 SPF						
DRY: SEASONED LUMBER.						
PLATES (table in inches)						
JT TYPE PLATES W LEN Y X						
B TMW+p MT20 4.0 4.0 1.00 2.00						
E BMW+w MT20 2.0 4.0						
F BMV1+p MT20 3.0 4.0						
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
BEARINGS						
FACTORED MAXIMUM FACTORED INPUT REQD						
GROSS REACTION GROSS REACTION BRG BRG						
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX						
F 352 0 352 0 0 5-8 1-8						
C 184 0 184 0 0 1-8 1-8						
D 36 0 40 0 0 1-8 1-8						
SEE MITEK STANDARD DETAIL MS02015-H FOR CONNECTION TO JOINT(S) C, D						
UNFACTORED REACTIONS						
1ST LCASE MAX/MIN COMPONENT REACTIONS						
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL						
F 246 177/0 0/0 0/0 0/0 68/0 0/0						
C 127 104/0 0/0 0/0 0/0 23/0 0/0						
D 29 0/0 0/0 0/0 0/0 29/0 0/0						
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F						
BRACING						
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.						
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.						
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.						
LOADING						
TOTAL LOAD CASES: (7)						
CHORDS WEBS						
MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FACTORED FORCE MAX						
(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)						
FR-TO FROM TO						
F-B -315/0 0.0 0.0 0.04 (1) 7.81 B-E 0/0 0.00 (1)						
A-B 0/39 -95.2 -95.2 0.15 (5) 10.00						
B-C 0/0 -95.2 -95.2 0.24 (1) 10.00						
F-E 0/0 -18.5 -18.5 0.08 (4) 10.00						
E-D 0/0 -18.5 -18.5 0.08 (4) 10.00						
CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN						
PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.						
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2						



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY-SEASONED LUMBER			

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	HORZ	DOWN	HORZ
JT	303	0	0
F	34	0	77
C	36	0	40
D	36	0	40

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	212	150/0	0/0
F	24	48/-28	0/0
C	29	0/0	0/0
D	29	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
F-B	-267/0	0.0	B-E	0/0
A-B	0/39	-95.2	E-G	0/0
B-C	-30/0	-95.2	G-D	0/0
F-E	0/0	-18.5		
E-G	0/0	-18.5		
G-D	0/0	-18.5		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 26.7	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD	= 40.1	PSF	

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.15/1.00 (A-B-5), BC=0.08/1.00 (D-E-4),
WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

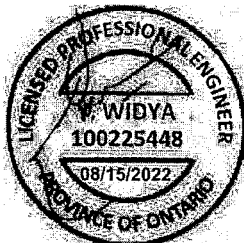
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428511	TRUSS NAME J77	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085742
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Alpe Roof Truss, Maple
Version 8.530 S Feb 23 2022 MITek Industries, inc. Mon Aug 15 13:17:42 2022 Page 1
ID: T06XqL09YKBxLLs9y_Z1eyrbTx-GaTrRmkSW4p09Xarkp6k5DX1qByfkiuts3ElFVync77

Scale = 1:17.4

LUMBER

N.L.G.A. RULES	SIZE	LUMBER	DESCR.
E - A	2x4 DRY	No.2	SPF
A - B	2x4 DRY	No.2	SPF
E - C	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY, SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWw+p	MT20	4.0	4.0	1.00	2.00
D	BMWw	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	91	0	91	0	0	1-8	1-8	
B	75	0	75	0	0	1-8	1-8	
C	15	0	17	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	54	42/0	0/0	0/0	0/0	22/0	0/0
B	52	42/0	0/0	0/0	0/0	9/0	0/0
C	12	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH FR-TO
E-A	-75/0	0.0	0.0 0.01 (1)	A-D	0/0	0.00 (1)	7.81
A-B	0/0	-95.2	-95.2 0.04 (1)				10.00
E-D	0/0	-18.5	-18.5 0.02 (4)				10.00
D-C	0/0	-18.5	-18.5 0.01 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/998 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (A-D:1), SS=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

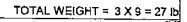
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90)
JSI METAL= 0.01 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

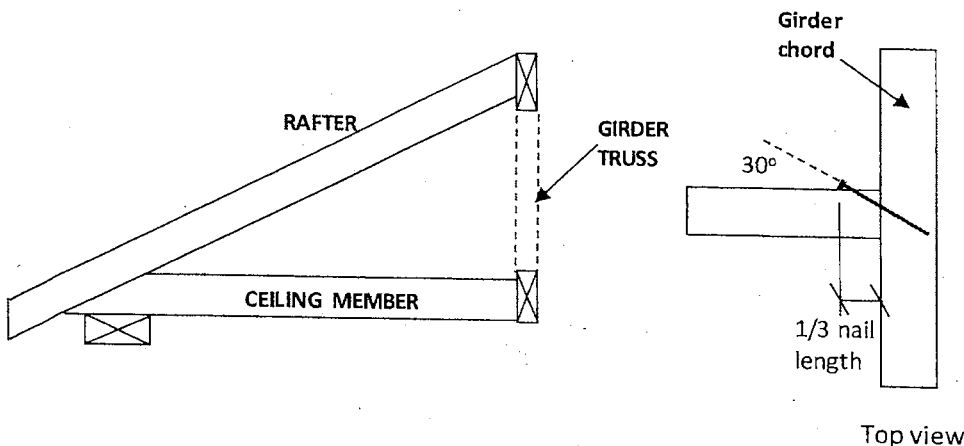


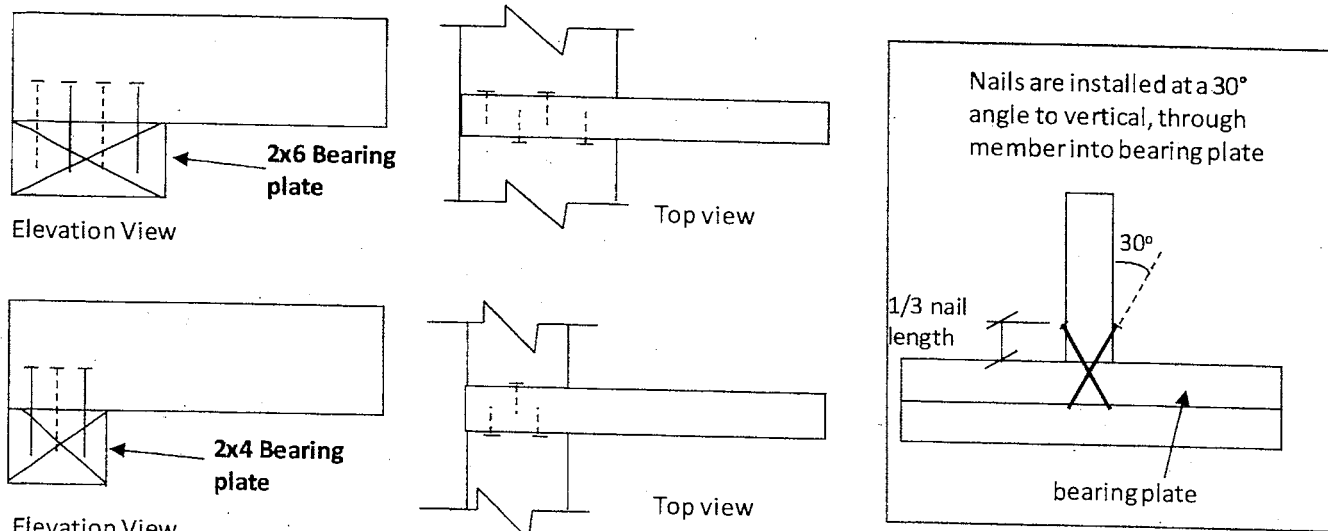
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

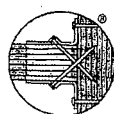
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

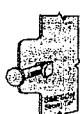
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

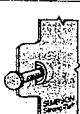
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

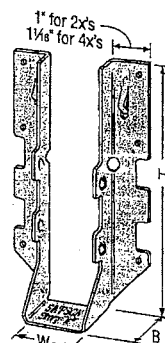
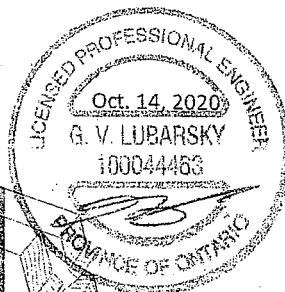
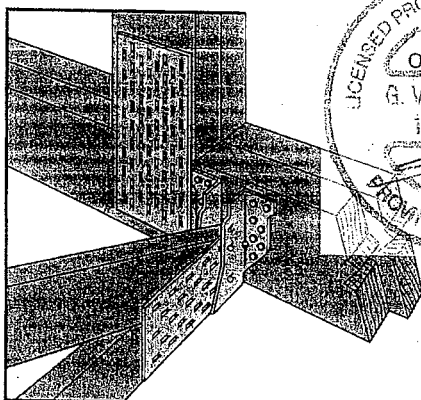


Double-Shear Nailing Side View; Do not bend tab

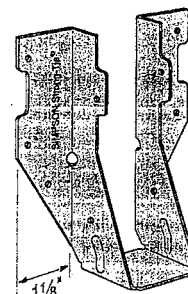


Dome Double-Shear Nailing Side View (available on some models)

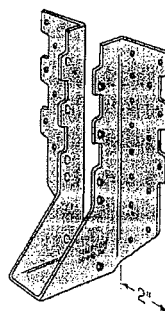
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



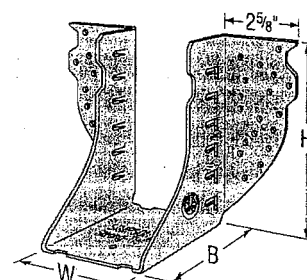
✓ LUS28



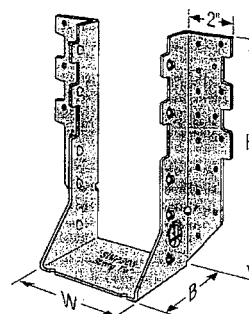
LU26L



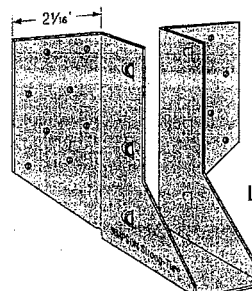
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

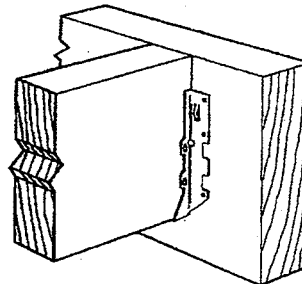
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

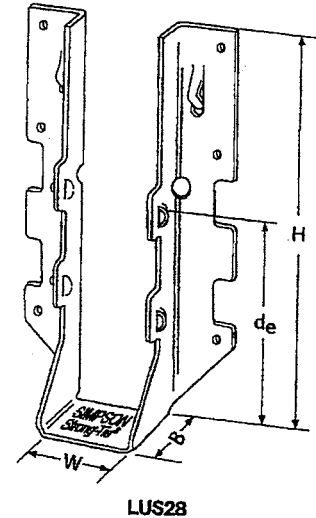
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

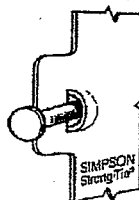


Typical LUS
Installation



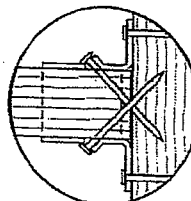
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LUS24	18	1½	3½	1¼	1 11/16	(4) 10d	(2) 10d		710	1630	645	1155
LUS24-2	18	3½	3½	2	1 11/16	(4) 16d	(2) 16d		835	2020	590	1435
LUS26	18	1½	4¾	1¼	3½	(4) 10d	(4) 10d		1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d		1720	2595	1545	2340
LUS28	18	1½	6¾	1¼	3¾	(6) 10d	(6) 10d		1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d		1720	3325	1545	2375
LUS210	18	1½	7 11/16	1¼	3¾	(8) 10d	(4) 10d		1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3195
LUS210-3	18	4¾	8 3/8	2	5¼	(8) 16d	(6) 16d		2580	3345	2320	2375

1. d₀ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

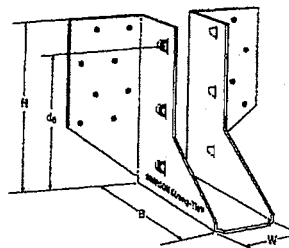
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

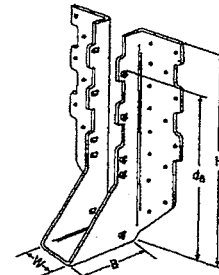
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

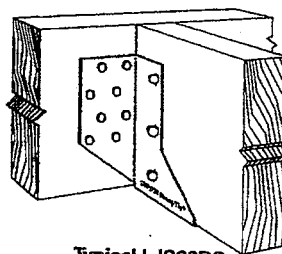
- See current catalogue for options



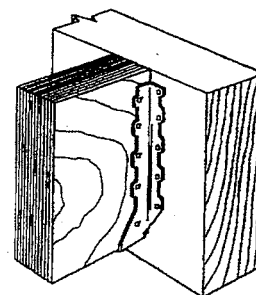
LJS26DS



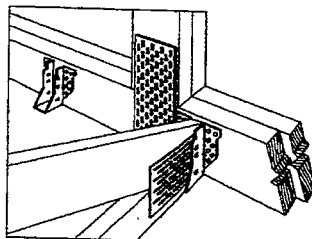
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



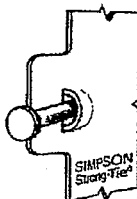
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

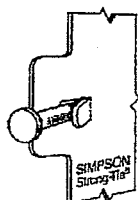
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 9/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_s is the distance from the seat of the hanger to the highest joist nail.

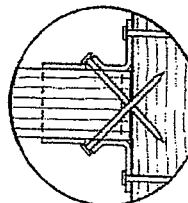


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

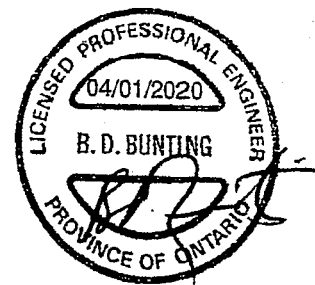
U.S. Patent
5,803,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

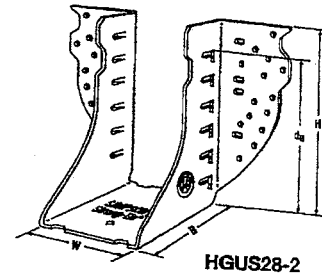
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

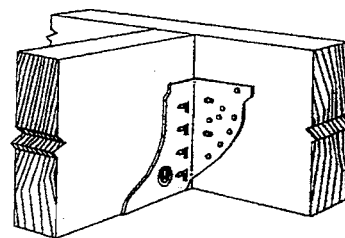
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

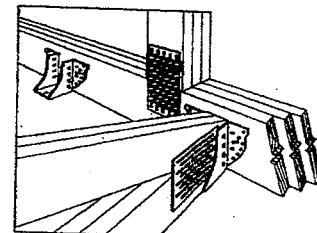
- See current catalogue for options



HGUS28-2



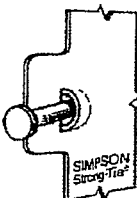
Typical HGUS Installation



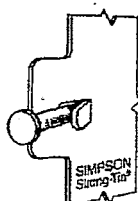
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

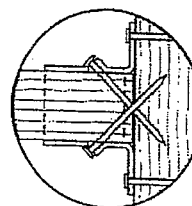
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



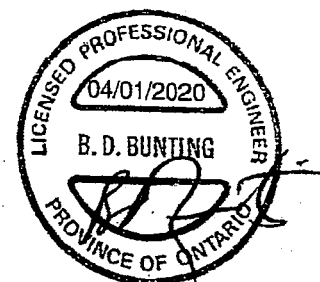
Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

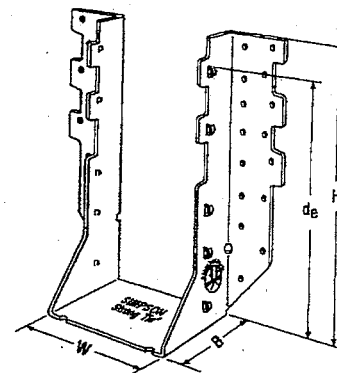
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

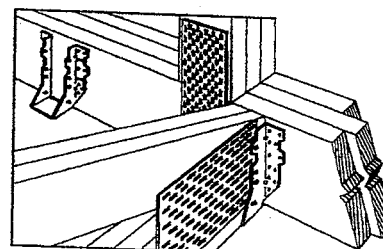
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



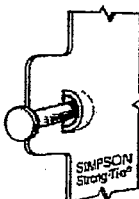
HHUS410



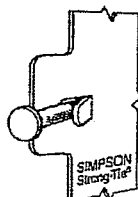
Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HHUS26-2	14	3 1/16	5 1/16	3	3 1/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/16	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 1/16	9 3/2	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/16	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 3/16	5 1/2	3	3 1/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/16	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 3/16	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

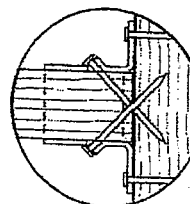
1. d_e is the distance from the seat of the hanger to the highest joist nail.



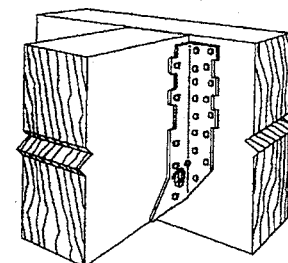
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



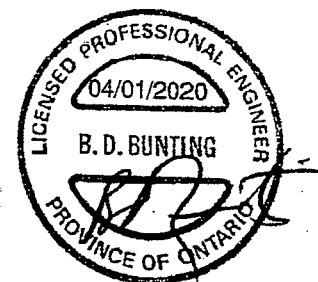
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



LIMIT
STATES
DESIGN

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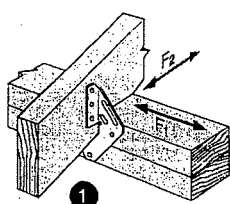
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHHUS20 3/20 exp. 6/22

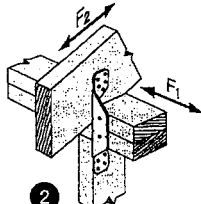
(800) 999-5099
strongtie.com

H/TSP

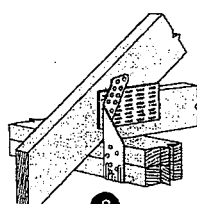
Seismic and Hurricane Ties (cont.)



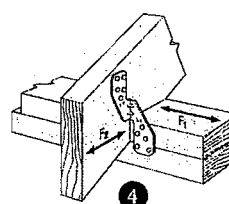
1 H1 Installation



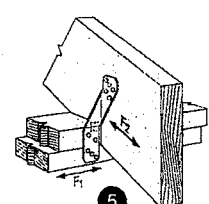
2 H2A Installation



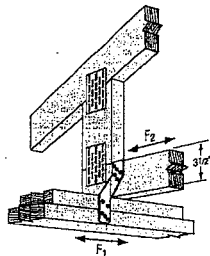
3 TSP Installation



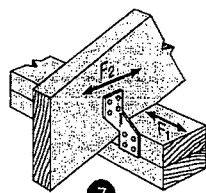
4 H2.5A Installation
(Nails into both top plates)



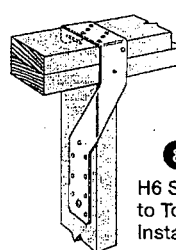
5 H2.5T Installation
(Nails into both top plates)



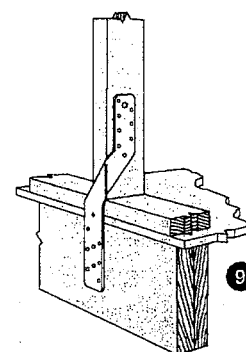
6 H2.5T Installation



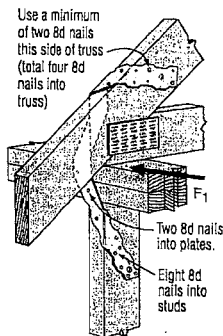
7 H3 Installation
(Nails into upper top plate)



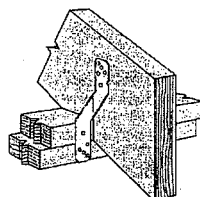
8 H6 Stud
to Top Plate
Installation



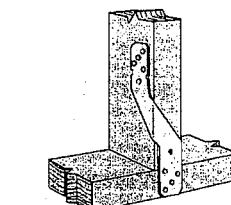
9 H6 Stud to
Band Joist
Installation



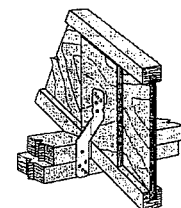
10 H7Z Installation



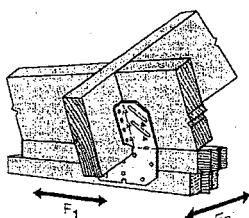
11 H8 Attaching
Rafter to Double
Top Plates



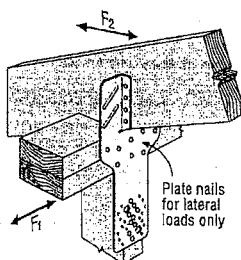
12 H8 attaching Stud to Sill
(4 8d into plate, 5 8d into stud)



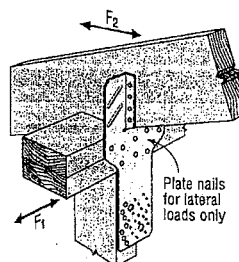
13 H8 attaching
I-Joist to Double
Top Plates



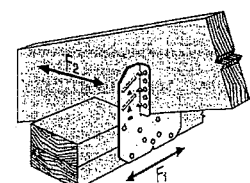
14 H10A Field-Bent
Installation



15 H10S Installation

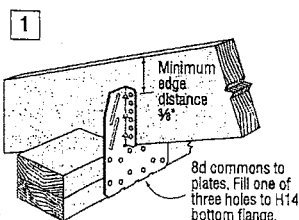


16 H10S Installation
with Stud Offset

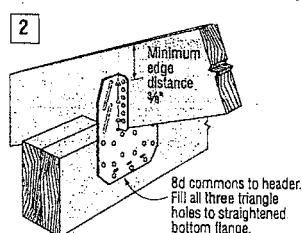


17 H10A
Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

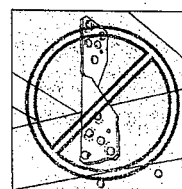


18 H14 Installation to
Double Top Plates



19 H14 Installation
to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

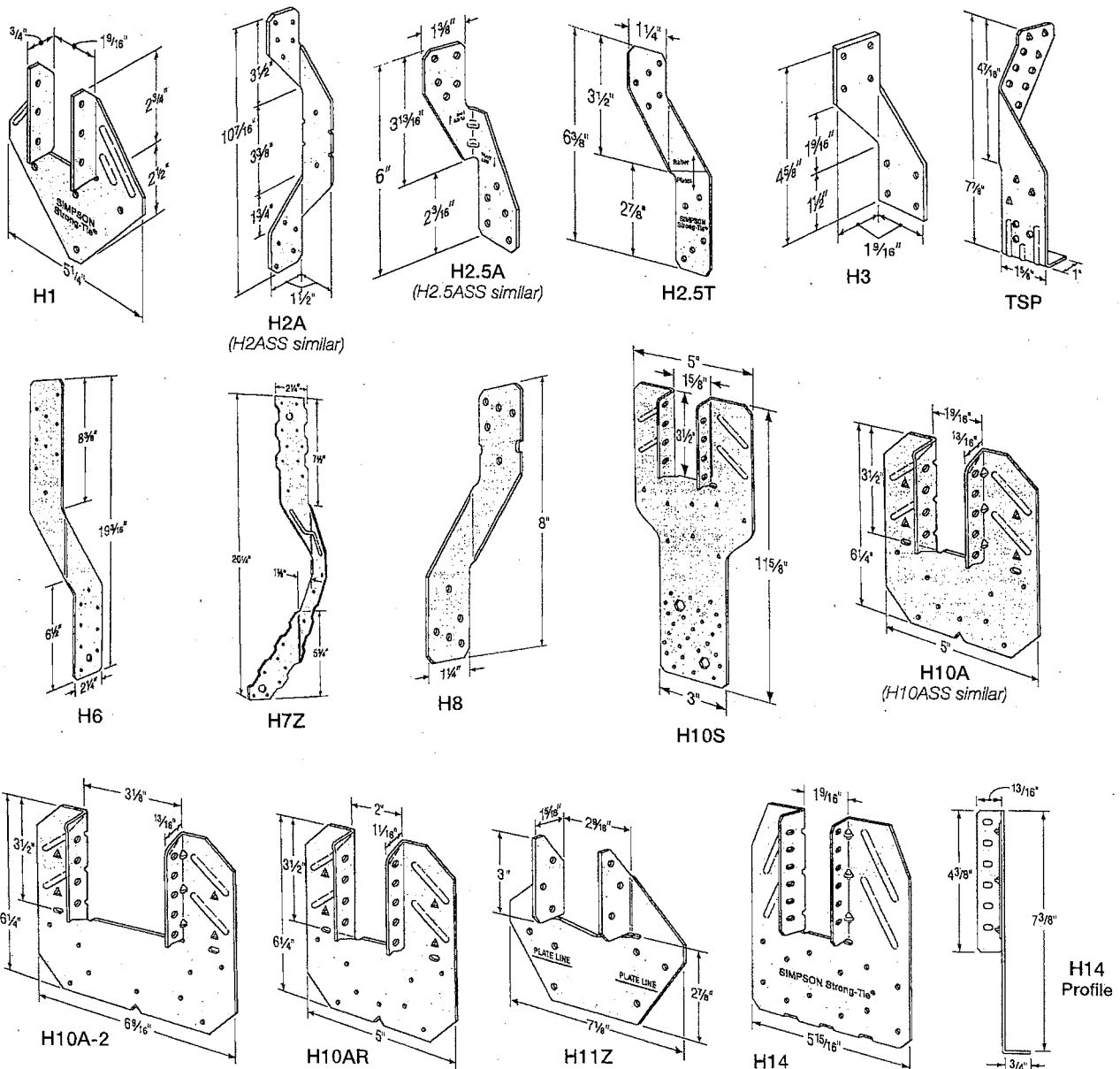
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA 086-14

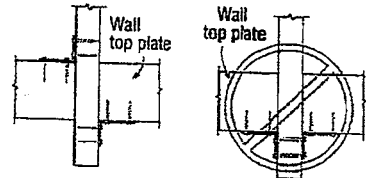
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

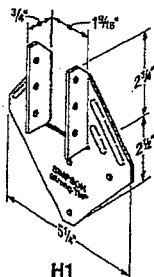
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

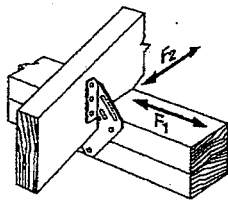


Install diagonally across from each other for minimum 2x truss.

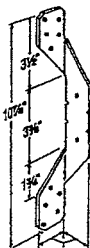
Nailing into both sides of a single ply 2x truss may cause the wood to split.



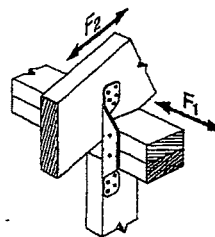
H1



H1 Installation



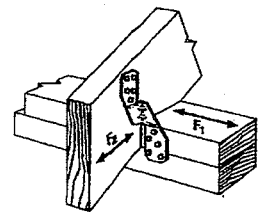
H2A



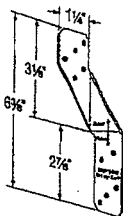
H2A Installation



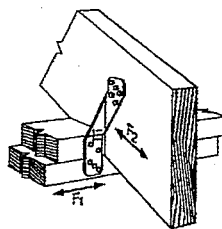
H2.5A



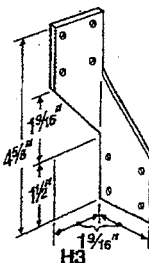
H2.5A Installation



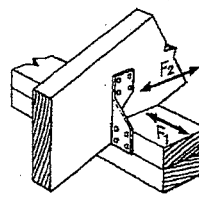
H2.5T



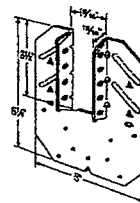
H2.5T Installation
(Nails into both top plates)



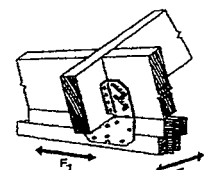
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
								(K _p =1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

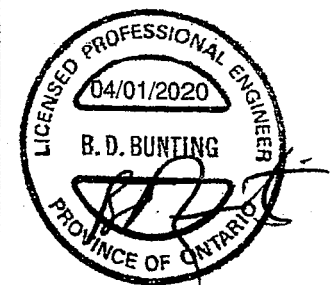


**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22



(800) 999-5099
strongtie.com

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

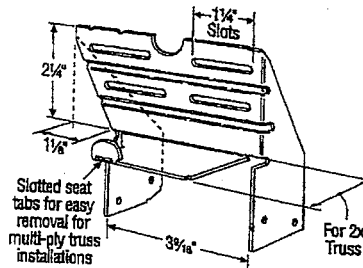
Design: Factored resistances are in accordance with CSA 086-14

Installation:

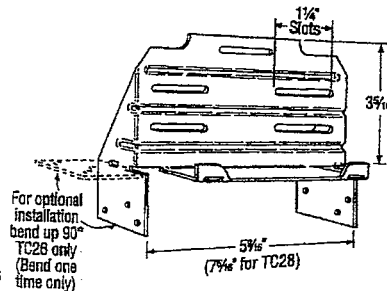
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

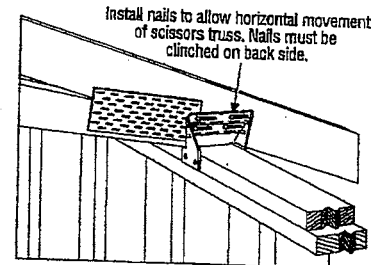
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



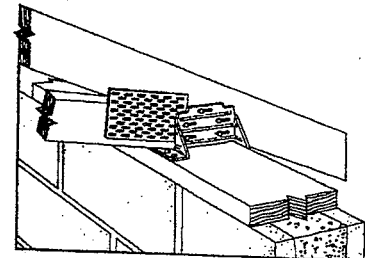
TC24
U.S. Patent 4,932,173



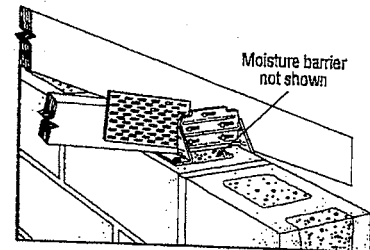
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



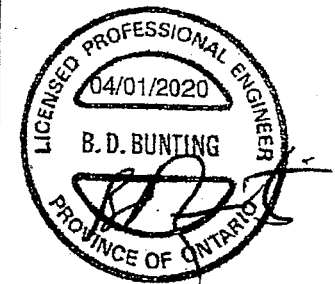
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _u =1.15)	Uplift (K _u =1.15)
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



**LIMIT
STATES
DESIGN**

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T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HTU

SIMPSON
Strong-Tie

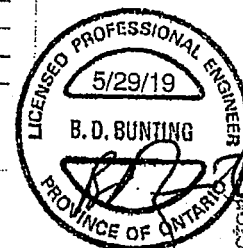
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3½"	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5½"	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470	4016	1755	2850
					10.99	17.86	7.81	12.89
HTU28 (Max.)	3½"	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150	6395	2945	4540
					18.46	28.46	13.10	20.19
HTU210 (Max.)	7½"	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150	6395	2945	4540
					18.46	28.46	13.10	20.19

See table footnotes on p. 280.

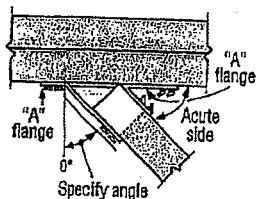


Hanger Options

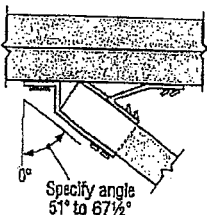
See Hanger Options information on pp. 125-127.

Skewed Seat

- Skawable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
				lb.	lb.	lb.	lb.
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1835	4110	1300	2905
	51-67½	(20) 16d	(12) 10d x 1½"	8.16	18.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1½"	1350	3620	955	2560
	51-67½	(26) 16d	(17) 10d x 1½"	8.01	18.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1½"	2810	4270	1985	3030
	51-67½	(32) 16d	(22) 10d x 1½"	12.60	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67½	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67½	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67½	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67½	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1510	3920	1140	2785
	51-67½	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2815	3855
	51-67½	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1695	3855
	51-67½	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4890
	51-67½	(26) 16d	(17) 10d	22.35	30.65	15.88	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6680	2225	4745
	51-67½	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

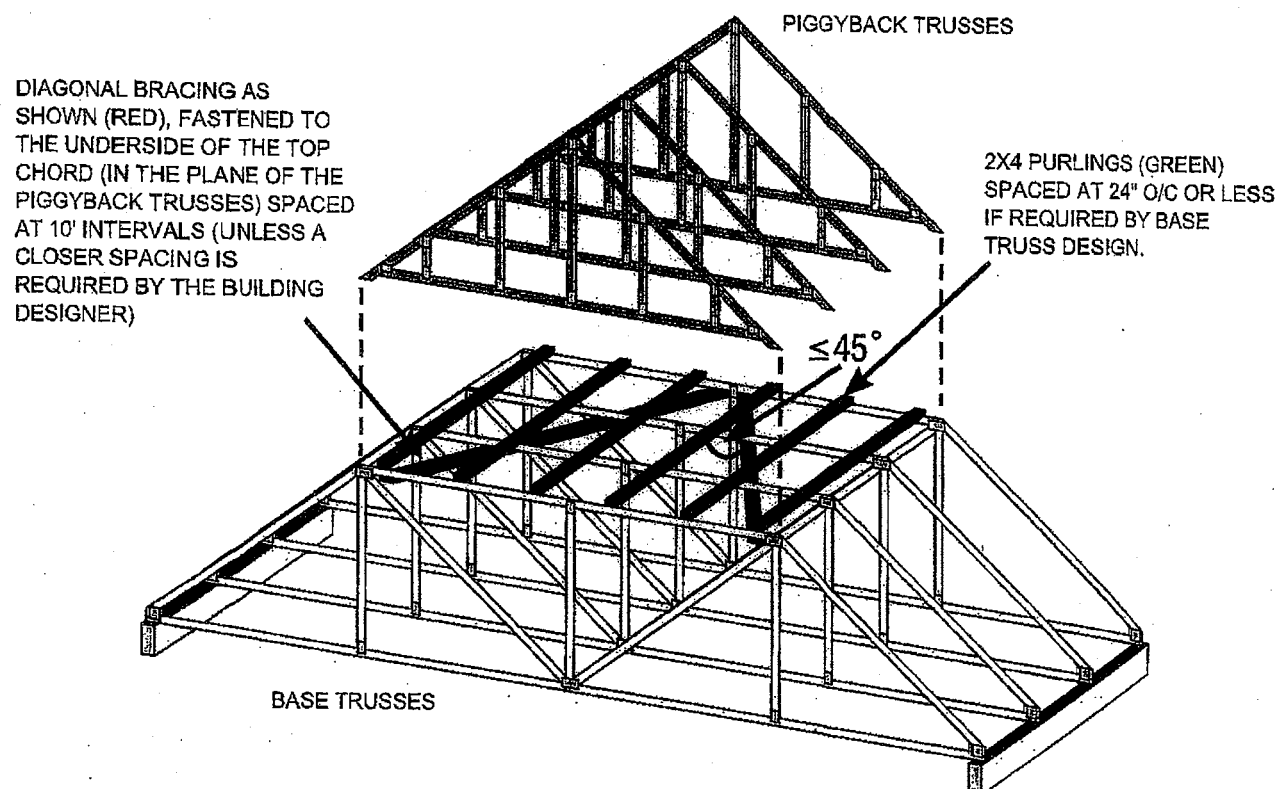
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

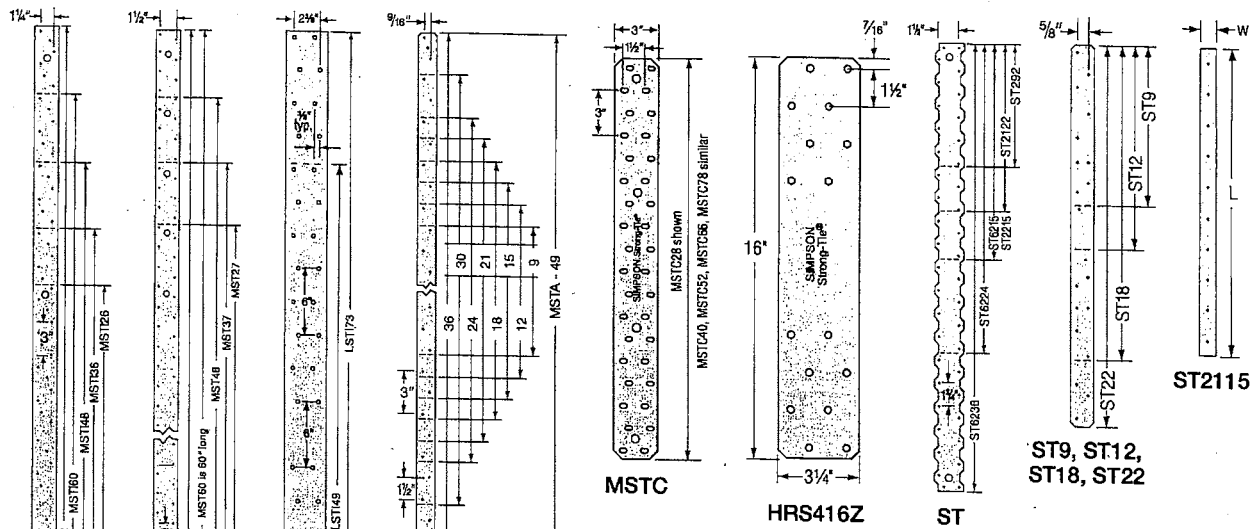
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

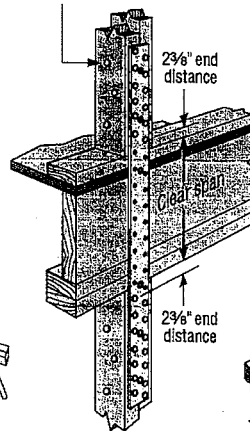
Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

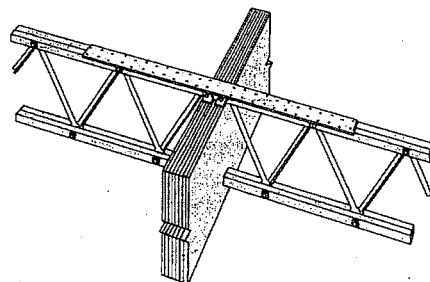


LSTA and MSTA
(Pilot holes not shown)

Stitch nailing
of double studs
by others

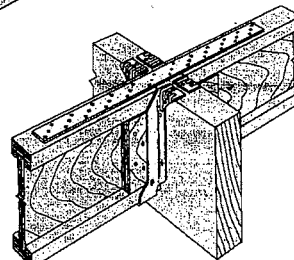


When nailing
the strap over
wood structural
panel sheathing,
use 2 1/2" long
nail, minimum.

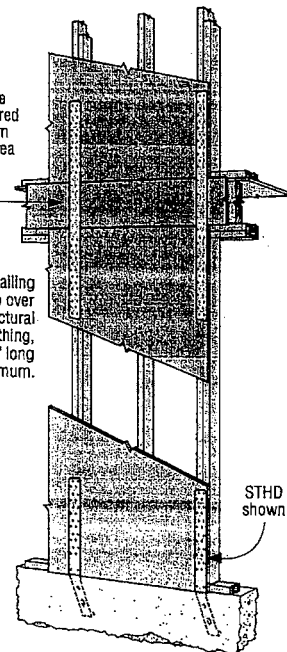


Typical LSTI Installation

Floor-to-Floor Tie
Installation
Showing a
Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Typical Detail with
Strap Installed over
Wood Structural Panel
Sheathing

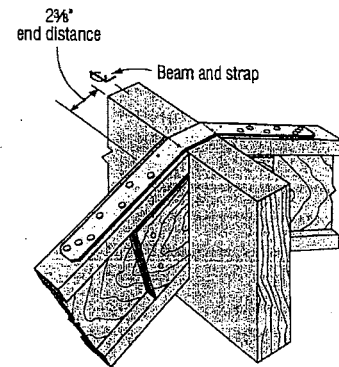
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties (cont.)

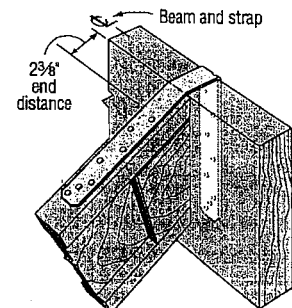
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366-370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
LSTA9	20	1 ¼	9	(6) 10d	600	690	555	635
LSTA12		1 ¼	12	(8) 10d	2.67	3.07	2.47	2.82
LSTA15		1 ¼	15	(10) 10d	800	920	735	845
LSTA18		1 ¼	18	(12) 10d	3.56	4.09	3.27	3.76
LSTA21		1 ¼	21	(14) 10d	1000	1150	920	1060
LSTA24		1 ¼	24	(16) 10d	4.45	5.12	4.09	4.72
ST292		2 ⅝	9 ⅞	(8) 8d	1200	1380	1105	1270
ST2122		2 ⅝	12 ⅜	(12) 8d	5.34	6.14	4.92	5.65
ST2115		¾	16 ⅝	(8) 8d	1400	1610	1290	1485
ST2215		2 ⅝	16 ⅝	(16) 8d	6.23	7.16	5.74	6.61
LSTA30	18	1 ¼	30	(20) 10d	1600	1840	1475	1695
LSTA36		1 ¼	36	(24) 10d	7.12	8.19	6.56	7.54
LSTI49		3 ¾	49	(32) 10d x 1 ½"	585	675	535	615
LSTI73		3 ¾	73	(48) 10d x 1 ½"	2.60	3.00	2.38	2.74
MSTA9		1 ¼	9	(6) 10d	940	1085	865	995
MSTA12		1 ¼	12	(8) 10d	4.18	4.83	3.85	4.43
MSTA15		1 ¼	15	(10) 10d	670	770	615	710
MSTA18		1 ¼	18	(12) 10d	2.98	3.43	2.74	3.16
MSTA21		1 ¼	21	(14) 10d	1335	1540	1235	1420
MSTA24		1 ¼	24	(16) 10d	5.94	6.85	5.49	6.32
MSTA30	16	1 ¼	30	(20) 10d	2235	2465	2075	2385
MSTA36		1 ¼	36	(24) 10d	9.94	10.97	9.23	10.61
MSTA49		1 ¼	49	(28) 8d	2465	2465	2465	2465
ST6215		2 ⅝	16 ⅝	(16) 8d	10.97	10.97	10.97	10.97
ST6224		2 ⅝	23 ⅝	(24) 8d	3115	3580	2852	3280
ST9		1 ¼	9	(6) 8d	13.86	15.93	12.69	14.59
ST12		1 ¼	11 ⅝	(8) 8d	4670	5370	4280	4920
ST18		1 ¼	17 ¾	(12) 8d	20.77	23.89	19.04	21.89
ST22		1 ¼	21 ⅝	(18) 8d	670	770	625	715
					2.98	3.43	2.78	3.18
				895	1030	830	955	
				3.98	4.58	3.69	4.25	
				1120	1285	1040	1195	
				4.98	5.72	4.63	5.32	
				1340	1545	1245	1430	
				5.96	6.87	5.54	6.36	
				1565	1800	1455	1670	
				6.96	8.01	6.47	7.43	
				1790	2060	1660	1910	
				7.96	9.16	7.38	8.50	
				2470	2840	2260	2595	
				10.99	12.63	10.05	11.54	
				2965	3070	2710	3070	
				13.19	13.66	12.06	13.66	
				2725	2725	2545	2725	
				12.12	12.12	11.32	12.12	
				1405	1615	1300	1500	
				6.25	7.18	5.78	6.67	
				2305	2650	2155	2475	
				10.25	11.79	9.59	11.01	
				525	605	490	560	
				2.34	2.69	2.18	2.49	
				700	805	650	750	
				3.11	3.58	2.89	3.34	
				1050	1210	975	1125	
				4.67	5.38	4.34	5.00	
				1580	1790	1465	1685	
				7.03	7.96	6.52	7.50	



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

- Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
- Use half of the nails in each member being connected to achieve the listed resistances.
- Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22-23 for other nail sizes and information.

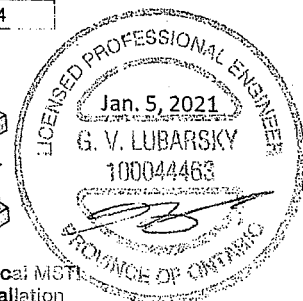
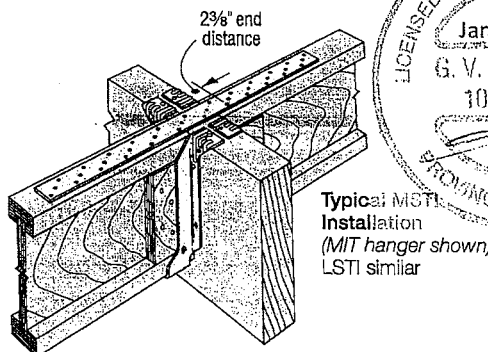
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

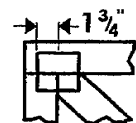
Model No.	Ga.	Dimensions (In.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52	16	3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¼	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¼	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236	14	2⅝	33⅝	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2⅝	26	(22) 10d x 1½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2⅝	36	(32) 10d x 1½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2⅝	48	(44) 10d x 1½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2⅝	60	(56) 10d x 1½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2⅝	72	(68) 10d x 1½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2⅝	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2⅝	37 ½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2⅝	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z	10	3¼	16	(16) ¼" x 1½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60		2⅝	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2⅝	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.

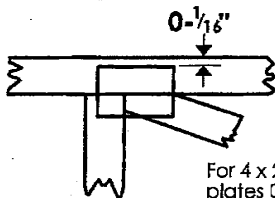


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

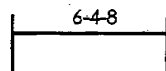


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

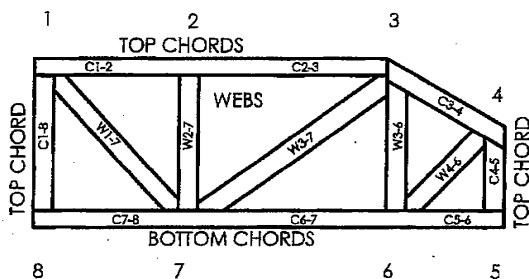
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MJL-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.