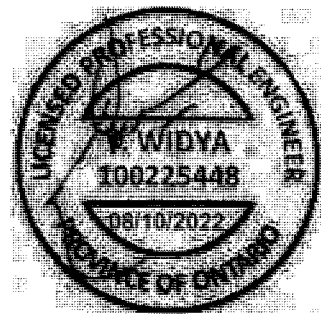
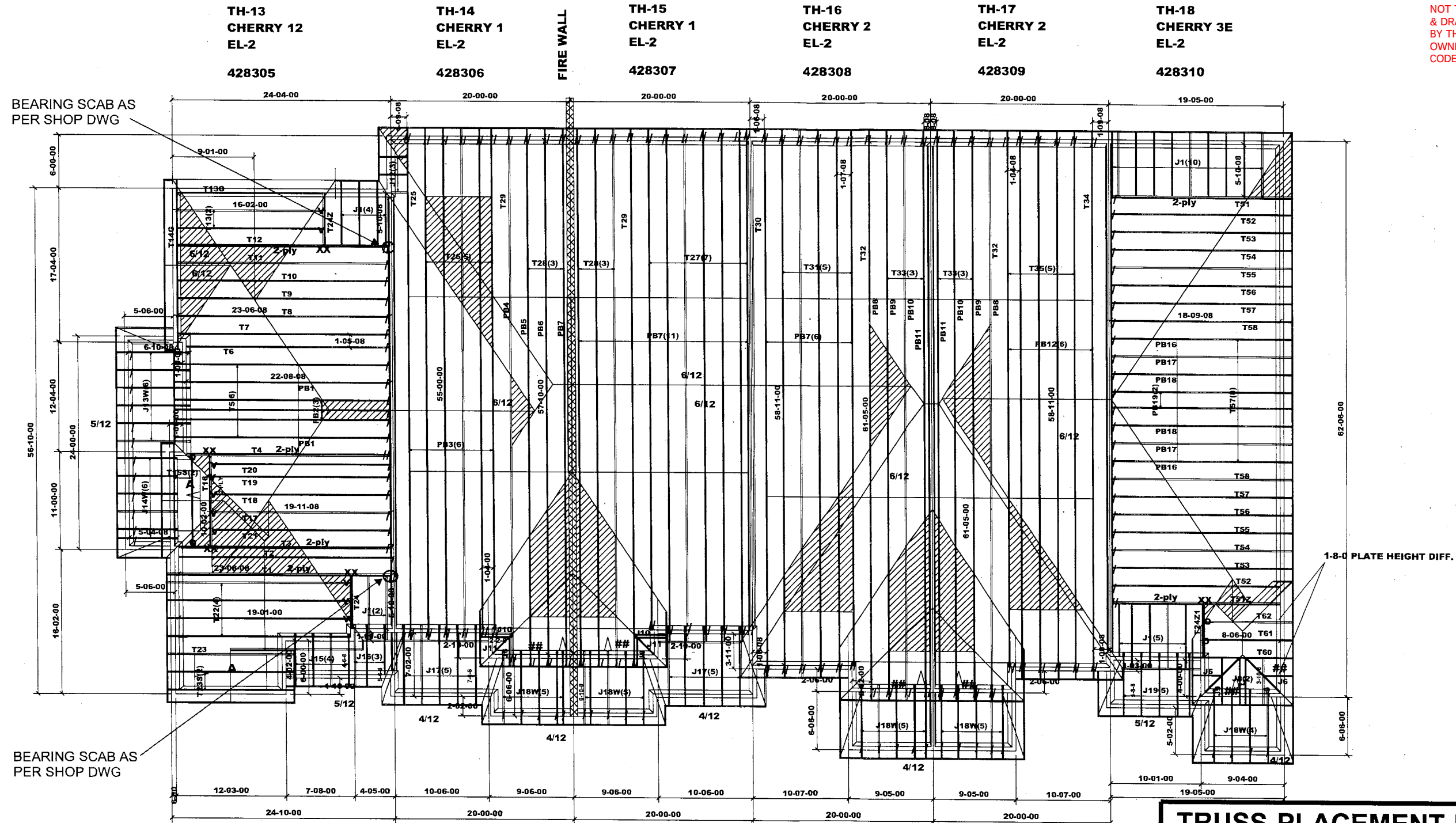


NOT THE GRANTING OF A PERMIT NOR REVIEWING OF SPECS & DRAWINGS NOR INSPECTIONS MADE DURING INSTALLATION BY THE OFFICIAL HAVING JURISDICTION SHALL RELIEVE THE OWNER FROM REQUIREMENTS OF THE ONTARIO BUILDING CODE AND ANY OTHER REFERENCED REQUIREMENTS.



TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.

9/12 ROOF PITCH UNLESS NOTED OTHERWISE

A- 12" RAISED CEILING

1-8-0 RAISE PLATE AND CEILING

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL [PART: 9]
Ss = 33.4 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 26.7 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5A- (I)
HTS16 - (T)
H8-(H)

DENOTES:
CONVENTIONAL FRAMING



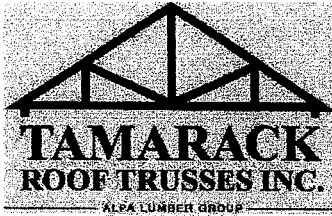
M15081

E2085161 - E22085241

Job Track: 52956	Builder / Location: GREEN PARK HOMES / CAMBRIDGE
Layout ID: 428250	Project: BARLASSINA
Plan Log: 206506	Date: 2022-08-06 Designer: YPG

Model / Elevation: BLOCK 120 / TH-13 - TH-18 (EL:2)	Mitek ver 8.5.3.233
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE	

DELIVERY SHIPLIST



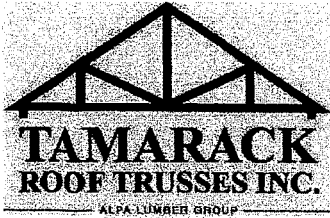
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-25,TH-13 (CHERRY 12 ,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428305
 Ref #
 Page: 1 of 3
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	9 /12	23-08-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	229.42 145.33		
	1	T2 Half Hip	9 /12	23-08-08	5-01-04	2 x 4		1-06-04 5-01-04	95.24 60.00		
	1 2-ply	T3 Half Hip Girder	9 /12	23-08-08	5-07-08	2 x 4 2 x 6		1-06-04 5-07-08	235.61 145.67		
	1 2-ply	T4 Half Hip Girder	9 /12	22-08-08	10-10-00	2 x 4 2 x 6		1-06-04 10-10-00	304.95 190.00		
	6	T5 Piggyback Base	9 /12	22-08-08	10-10-00	2 x 4	1-03-08	1-06-04 10-10-00	814.16 510.00		
	1	T6 Half Hip	9 /12	22-08-08	9-10-00	2 x 4		1-06-04 9-10-00	121.18 76.00		
	1	T7 Half Hip	9 /12	23-06-08	9-01-04	2 x 4		10-12 9-01-04	118.81 73.67		
	1	T8 Half Hip	9 /12	23-06-08	8-01-04	2 x 4		10-12 8-01-04	113.18 71.00		
	1	T9 Half Hip	9 /12	23-06-08	7-01-04	2 x 4		10-12 7-01-04	110.64 70.67		
	1	T10 Half Hip	9 /12	23-06-08	6-01-04	2 x 4		10-12 6-01-04	101.97 64.00		
	1	T11 Half Hip	9 /12	23-06-08	5-01-04	2 x 4		10-12 5-01-04	94.33 59.83		
	1 2-ply	T12 Half Hip Girder	9 /12	23-06-08	4-01-04	2 x 4 2 x 6		10-12 4-01-04	211.3 127.33		
	2	T13 Common	9 /12	16-02-00	7-03-04	2 x 4		10-12 1-06-04	139.14 89.67		
	1	T13G GABLE	9 /12	16-02-00	7-03-04	2 x 4		10-12 1-06-04	67.17 43.83		

DELIVERY SHIPLIST



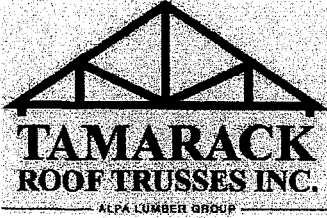
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-25,TH-13 (CHERRY 12 ,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428305
 Ref #
 Page: 2 of 3
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T14G GABLE	6 /12	16-06-00	5-03-08	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	62.19 38.83		
	2	T15S Roof Special	9 /12	10-02-00	5-04-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	103.29 73.33		
	1 2-ply	T16 Half Hip Girder	9 /12	10-02-00	4-01-12	2 x 4 2 x 6		1-06-04 4-01-12	95.83 65.33		
	1	T17 Half Hip	9 /12	19-11-08	7-07-08	2 x 4		3-07-00 7-07-08	91.36 57.00		
	1	T18 Half Hip	9 /12	19-11-08	8-07-08	2 x 4		3-07-00 8-07-08	107.13 68.00		
	1	T19 Half Hip	9 /12	19-11-08	9-07-08	2 x 4		3-07-00 9-07-08	115.76 72.67		
	1	T20 Half Hip	9 /12	19-11-08	10-04-00	2 x 4		3-07-00 10-04-00	119.9 74.50		
	1	T21 Half Hip	9 /12	19-11-08	6-07-08	2 x 4		3-07-00 6-07-08	95.31 60.33		
	4	T22 Common	9 /12	19-01-00	8-08-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	367.61 232.00		
	1	T23 Common	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	48.96 31.67		
	2	T23S Roof Special	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	112.41 77.67		
	1 2-ply	T24 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T24Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	2	PB1 Piggyback	9 /12	10-03-08	1-00-00	2 x 4		1-00-00	47.72 31.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-25,TH-13 (CHERRY 12 ,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428305
 Ref #
 Page: 3 of 3
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	PB2 Piggyback	9 /12	10-03-08	2-00-00	2 x 4		2-00-00	99.06 66.00		
	6	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	6	J13W Jack-Open	5 /12	6-10-08	3-08-10	2 x 4	1-03-08	5-06 3-03-12	128.63 80.00		
	6	J14W Jack-Open	5 /12	5-04-08	3-01-02	2 x 4	1-03-08	5-06 2-08-04	91.81 60.00		
	4	J15 Jack-Open	5 /12	4-06-08	2-08-15	2 x 4	1-03-08	5-06 2-04-01	53.29 34.67		
	3	J16 Jack-Open	5 /12	5-06-08	3-01-15	2 x 4	1-03-08	5-06 2-09-01	47.09 30.00		

TOTAL # TRUSS= 75

TOTAL BFT OF ALL TRUSSES= 3019.67

BFT.

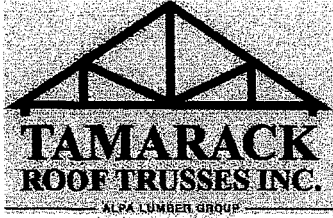
TOTAL WEIGHT OF ALL TRSSES 4762.02 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
38	Hardware	H2.5A	
4	Hardware	HGUS26-2	
10	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 54

DELIVERY SHIPLIST

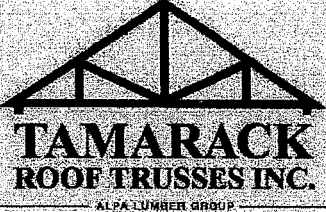


Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-26,TH-14 (CHERRY 1,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428306
 Ref #
 Page: 1 of 2
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T25 Piggyback Base Girder	6 /12	55-00-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	389.76 235.33		
	5	T26 Piggyback Base	6 /12	55-00-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1783.46 1076.67		
	3	T28 Piggyback Base	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1145.32 712.00		
	1	T29 Piggyback Base Girder	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	403.91 241.33		
	6	PB3 Piggyback	6 /12	13-08-00	3-05-00	2 x 4			197.69 121.00		
	1	PB4 Piggyback	6 /12	19-08-00	1-04-06	2 x 4			56.06 35.83		
	1	PB5 Piggyback	6 /12	19-08-00	2-10-06	2 x 4			56.53 36.83		
	1	PB6 Piggyback	6 /12	19-08-00	4-04-06	2 x 4			63.79 40.83		
	1	PB7 Piggyback	6 /12	19-08-00	4-11-00	2 x 4			57.58 35.67		
	1	J6 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	9.23 7.00		
	1	J10 Jack-Open	9 /12	2-02-08	3-02-02	2 x 4	1-03-08	1-06-04 3-02-02	10.47 7.67		
	1	J11 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 5-01	1-06-04 2-10-05	9.61 7.67		
	3	J12 Jack-Open	9 /12	1-09-08	2-10-06	2 x 4	1-03-08	1-06-04 2-10-06	27.43 21.00		
	5	J17 Jack-Open	4 /12	7-06-08	3-03-00	2 x 4	1-03-08	3-15 2-10-02	111.99 72.50		

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: BLOCK 119 , BLOCK 120 Lot #: Elevation: TH-26,TH-14 (CHERRY 1,EL-2)	Job Track: 52956 PlanLog: 206506 Layout ID: 428306 Ref # Page: 2 of 2 Date: 08-15-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

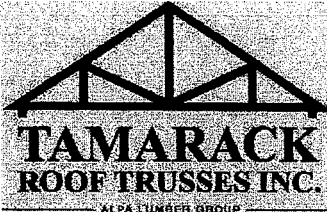
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

TOTAL # TRUSS= 36 TOTAL BFT OF ALL TRUSSES= 2712.16 BFT. TOTAL WEIGHT OF ALL TRSSES 4422.45 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
45	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 45

		DELIVERY SHIPLIST	
TAMARACK ROOF TRUSSES INC. KALPA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: BLOCK 119 , BLOCK 120 Lot #: Elevation: TH-27 ,TH-15 (CHERRY 1,EL-2)	Job Track: 52956 PlanLog: 206506 Layout ID: 428307 Ref # Page: 1 of 1 Date: 08-15-2022 Designer: Sales Rep: Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	T27 Piggyback Base	6 /12	55-00-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	2445.02 1467.67		
	3	T28 Piggyback Base	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1145.32 712.00		
	1	T29 Piggyback Base Girder	6 /12	57-10-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	403.91 241.33		
	11	PB7 Piggyback	6 /12	19-08-00	4-11-00	2 x 4			633.33 392.33		
	1	J6 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	9.23 7.00		
	1	J10 Jack-Open	9 /12	2-02-08	3-02-02	2 x 4	1-03-08	1-06-04 3-02-02	10.47 7.67		
	1	J11 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 5-01	1-06-04 2-10-05	9.61 7.67		
	5	J17 Jack-Open	4 /12	7-06-08	3-03-00	2 x 4	1-03-08	3-15 2-10-02	111.99 72.50		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

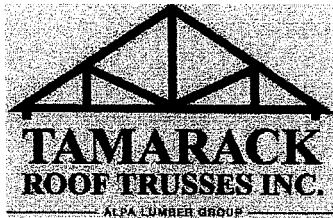
TOTAL # TRUSS= 35 TOTAL BFT OF ALL TRUSSES= 2969 BFT. TOTAL WEIGHT OF ALL TRSSES 4868.5 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
49	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 49

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-28 ,TH-16 (CHERRY 2,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428308
 Ref #
 Page: 1 of 1
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T30 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	399.03 241.33		
	5	T31 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1970.6 1183.33		
	1	T32 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	438.02 263.33		
	3	T33 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1336.72 818.00		
	6	PB7 Piggyback	6 /12	19-08-00	4-11-00	2 x 4			345.45 214.00		
	1	PB8 Piggyback	6 /12	23-07-00	1-04-12	2 x 4			68.61 43.67		
	1	PB9 Piggyback	6 /12	23-07-00	2-10-12	2 x 4			68.38 43.50		
	1	PB10 Piggyback	6 /12	23-07-00	4-04-12	2 x 4			75.01 46.50		
	1	PB11 Piggyback	6 /12	23-07-00	5-10-12	2 x 4			69.91 44.33		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

TOTAL # TRUSS= 25

TOTAL BFT OF ALL TRUSSES= 2958.82

BFT.

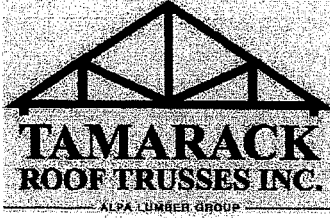
TOTAL WEIGHT OF ALL TRSSES 4871.34 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
45	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 45

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-29,TH-17 (CHERRY 2,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428309
 Ref #
 Page: 1 of 1
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T32 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	438.02 263.33		
	3	T33 Piggyback Base	6 /12	61-05-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-06-04	1336.72 818.00		
	1	T34 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	395.49 239.33		
	5	T35 Piggyback Base	6 /12	58-11-00	10-00-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1995.15 1213.33		
	1	PB8 Piggyback	6 /12	23-07-00	1-04-12	2 x 4			68.61 43.67		
	1	PB9 Piggyback	6 /12	23-07-00	2-10-12	2 x 4			68.38 43.50		
	1	PB10 Piggyback	6 /12	23-07-00	4-04-12	2 x 4			75.01 46.50		
	1	PB11 Piggyback	6 /12	23-07-00	5-10-12	2 x 4			69.91 44.33		
	6	PB12 Piggyback	6 /12	22-04-00	5-07-00	2 x 4			395.84 242.00		
	5	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	99.62 60.83		

TOTAL # TRUSS= 25

TOTAL BFT OF ALL TRUSSES= 3014.82 BFT.

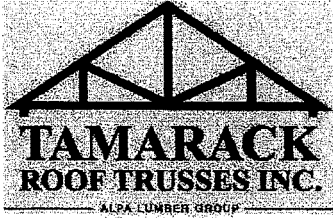
TOTAL WEIGHT OF ALL TRSSES 4942.73 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
45	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= 45

DELIVERY SHIPLIST



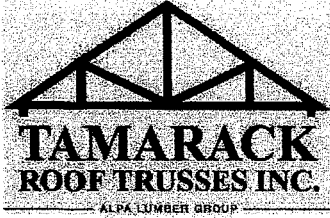
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-30 , TH-18 (CHERRY 3E,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428310
 Ref #
 Page: 1 of 2
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T24Z1 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T51 Half Hip Girder	9 /12	18-09-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	177.93 112.00		
	1 2-ply	T51Z Half Hip Girder	9 /12	18-09-08	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	177.93 112.00		
	2	T52 Half Hip	9 /12	18-09-08	5-01-04	2 x 4		1-06-04 5-01-04	150.17 97.00		
	2	T53 Half Hip	9 /12	18-09-08	6-01-04	2 x 4	1-03-08	1-06-04 6-01-04	166.98 106.67		
	2	T54 Half Hip	9 /12	18-09-08	7-01-04	2 x 4	1-03-08	1-06-04 7-01-04	176.84 113.33		
	2	T55 Half Hip	9 /12	18-09-08	8-01-04	2 x 4	1-03-08	1-06-04 8-01-04	191.55 121.67		
	2	T56 Half Hip	9 /12	18-09-08	9-01-04	2 x 4	1-03-08	1-06-04 9-01-04	202.89 127.00		
	10	T57 Half Hip	9 /12	18-09-08	10-01-04	2 x 4	1-03-08	1-06-04 10-01-04	1062.03 670.00		
	2	T58 Half Hip	9 /12	18-09-08	11-01-04	2 x 4	1-03-08	1-06-04 11-01-04	208.01 128.67		
	1	T60 Hip Girder	9 /12	8-06-00	4-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	40.88 26.67		
	1	T61 Common	9 /12	8-06-00	6-04-08	2 x 4	1-03-08 1-03-08	3-02-04 3-02-04	44.63 28.67		
	1	T62 Hip	9 /12	8-06-00	5-06-12	2 x 4	1-03-08 1-03-08	3-02-04 3-02-04	50.53 32.00		
	2	PB16 Piggyback	9 /12	7-04-03	2-00-00	2 x 4		2-00-00	45.61 31.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: BLOCK 119 , BLOCK 120
 Lot #:
 Elevation: TH-30 , TH-18 (CHERRY 3E,EL-2)

Job Track: 52956
 PlanLog: 206506
 Layout ID: 428310
 Ref #
 Page: 2 of 2
 Date: 08-15-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB17 Piggyback	9 /12	7-04-03	3-00-00	2 x 4		3-00-00	49.1 32.33		
	2	PB18 Piggyback	9 /12	7-04-03	4-00-00	2 x 4		4-00-00	53.72 36.00		
	2	PB19 Piggyback	9 /12	7-04-03	5-00-00	2 x 4		5-00-00	52.5 34.33		
	15	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	251.92 160.00		
	2	J6 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	18.46 14.00		
	2	J8 Jack-Open	9 /12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	29.83 20.33		
	2	J9 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 2-01-01	1-06-04 2-10-05	23.19 16.67		
	4	J18W Jack-Open	4 /12	6-10-08	3-00-06	2 x 4	1-03-08	3-15 2-07-07	79.69 48.67		
	5	J19 Jack-Open	5 /12	5-08-08	3-02-13	2 x 4	1-03-08	4-01 2-08-09	76.47 46.67		

TOTAL # TRUSS= 69

TOTAL BFT OF ALL TRUSSES= 2153.35

BFT.

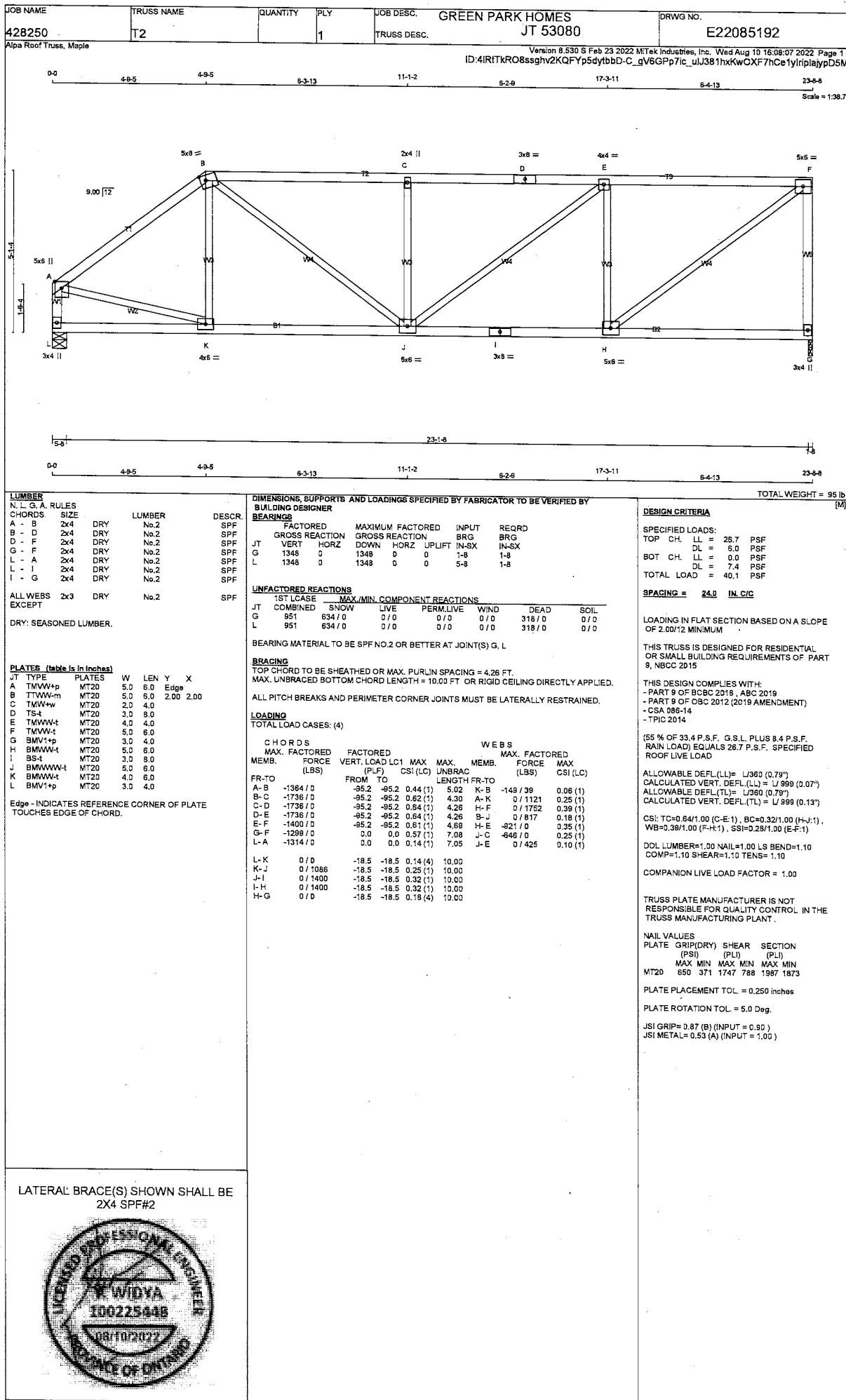
TOTAL WEIGHT OF ALL TRSSES 3389.27 LBS

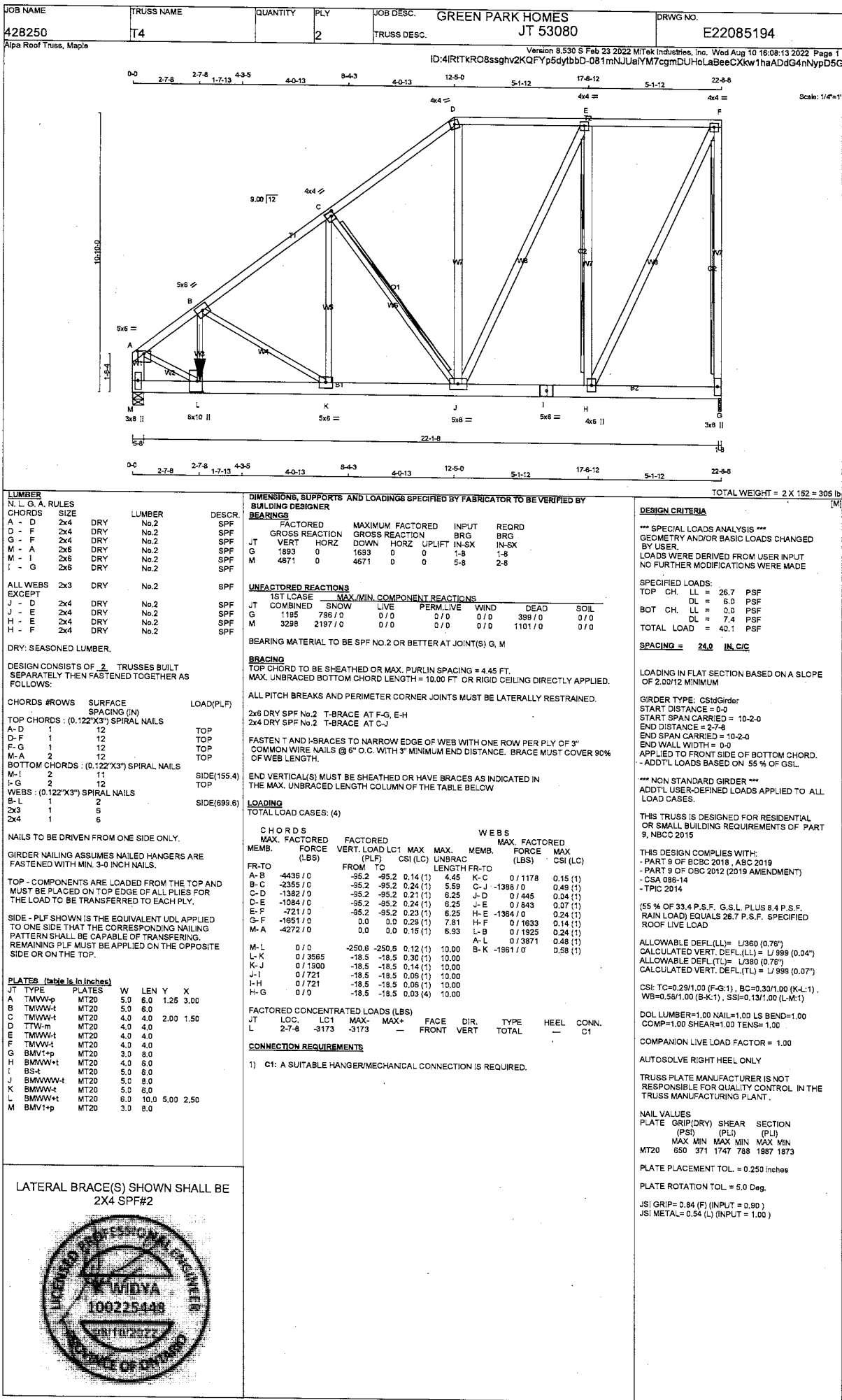
HARDWARE

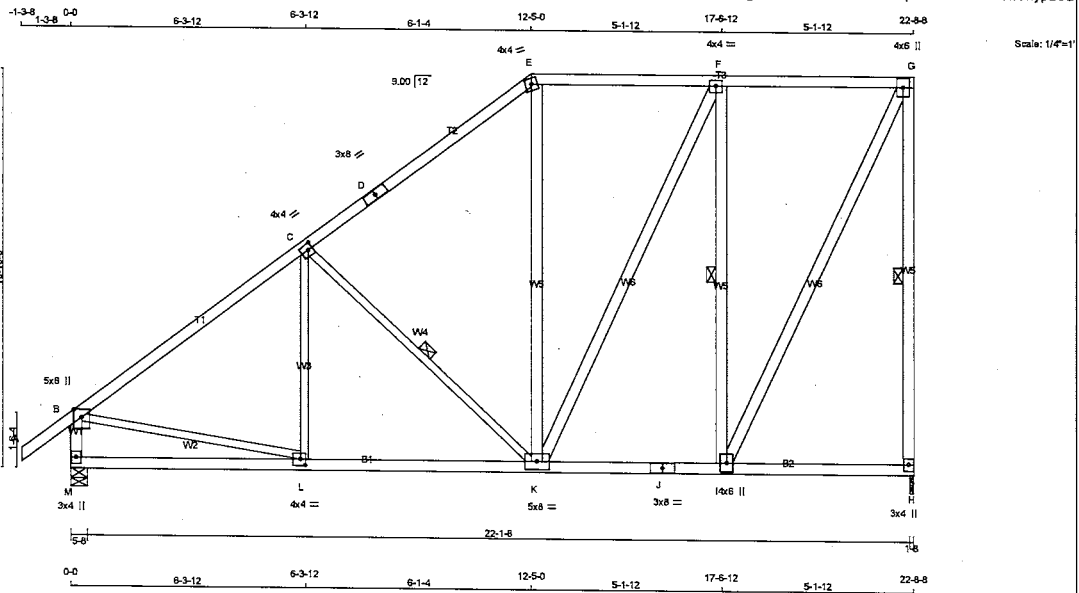
QTY	TYPE	MODEL	LENGTH
30	Hardware	H2.5A	
1	Hardware	HGUS26-2	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 33

[illegible]







LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW+p	MT20	5.0	8.0	Edge	
C TMWW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	8.0		
E TTW-m	MT20	4.0	4.0		
F TMWW-t	MT20	4.0	4.0		
G TMWW+p	MT20	4.0	8.0		
H BMV1+p	MT20	3.0	4.0		
I BMWW-t	MT20	4.0	8.0		
J BS-t	MT20	3.0	8.0		
K BMWW-t	MT20	5.0	8.0		
L BMWW-t	MT20	4.0	4.0	2.00	1.50
M BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	IN-SX
H 1351	0	1351	0
M 1440	0	1440	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	859	807/0	0/0	0/0	0/0	352/0	0/0
M	1018	881/0	0/0	0/0	0/0	335/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC) (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC) (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/39	-85.2	-95.2 0.13 (1)	10.00	L-C	-76/75	0.05 (1)
B-C	-1331/0	-85.2	-95.2 0.54 (1)	4.93	C-K	-538/0	0.27 (1)
C-D	-932/0	-85.2	-95.2 0.50 (1)	5.69	K-E	0/115	0.03 (4)
D-E	-932/0	-85.2	-95.2 0.50 (1)	5.69	K-F	0/392	0.06 (1)
E-F	-710/0	-102.7	-102.7 0.35 (1)	2.00	I-G	-1008/0	0.80 (1)
F-G	-542/0	-102.7	-102.7 0.35 (1)	2.00	I-G	0/1237	0.20 (1)
H-G	-1311/0	0.0	0.0 0.78 (1)	6.65	B-L	0/1119	0.25 (1)
M-B	-1392/0	0.0	0.0 0.15 (1)	6.90			
M-L	0/0	-18.5	-18.5 0.18 (4)	10.00			
L-K	0/1089	-18.5	-18.5 0.27 (4)	10.00			
K-J	0/542	-18.5	-18.5 0.16 (4)	10.00			
J-I	0/542	-18.5	-18.5 0.16 (4)	10.00			
I-H	0/0	-18.5	-18.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(LL) = $L/959$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.78")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.78/1.00 (S+H), BC=0.27/1.00 (K-L-4), WB=0.50/1.00 (F-I), SS=0.26/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

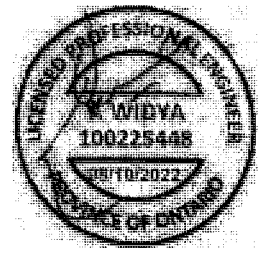
PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PL)	PL
MAX MIN MAX MIN MAX MIN		
MT20	650 371 1747 788 1987 1873	

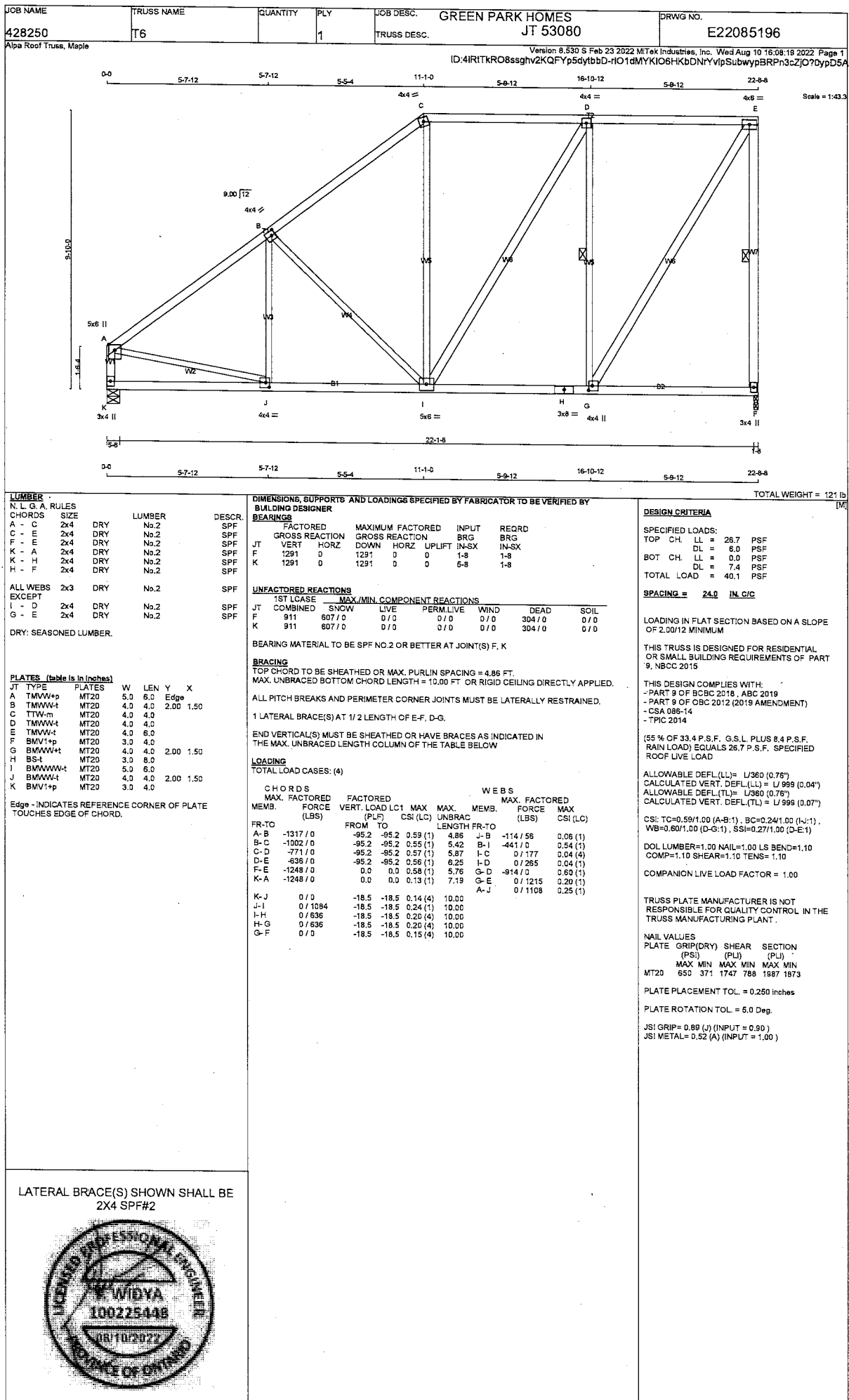
PLATE PLACEMENT TOL. = 0.250 inches

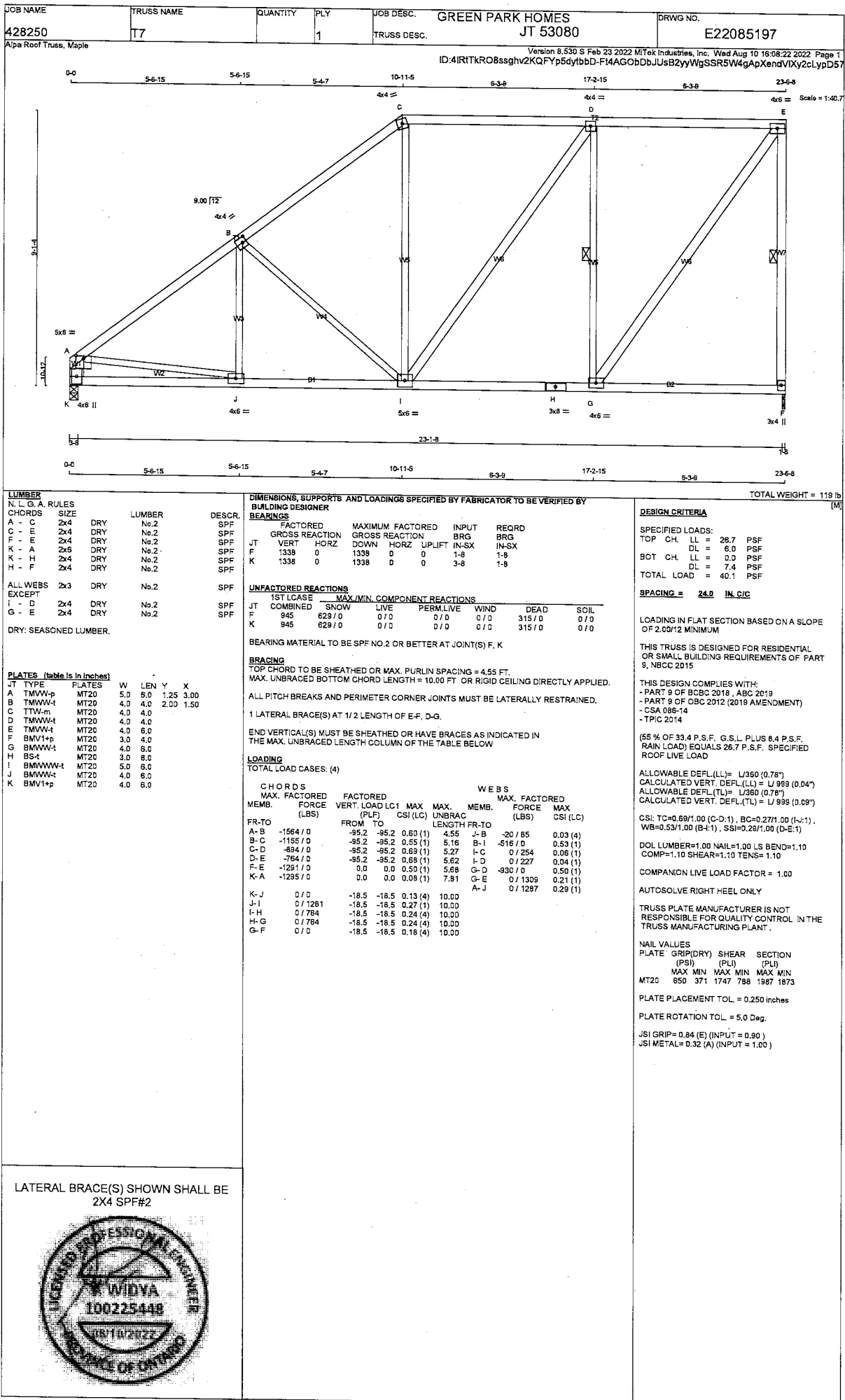
PLATE ROTATION TOL. = 5.0 Deg.

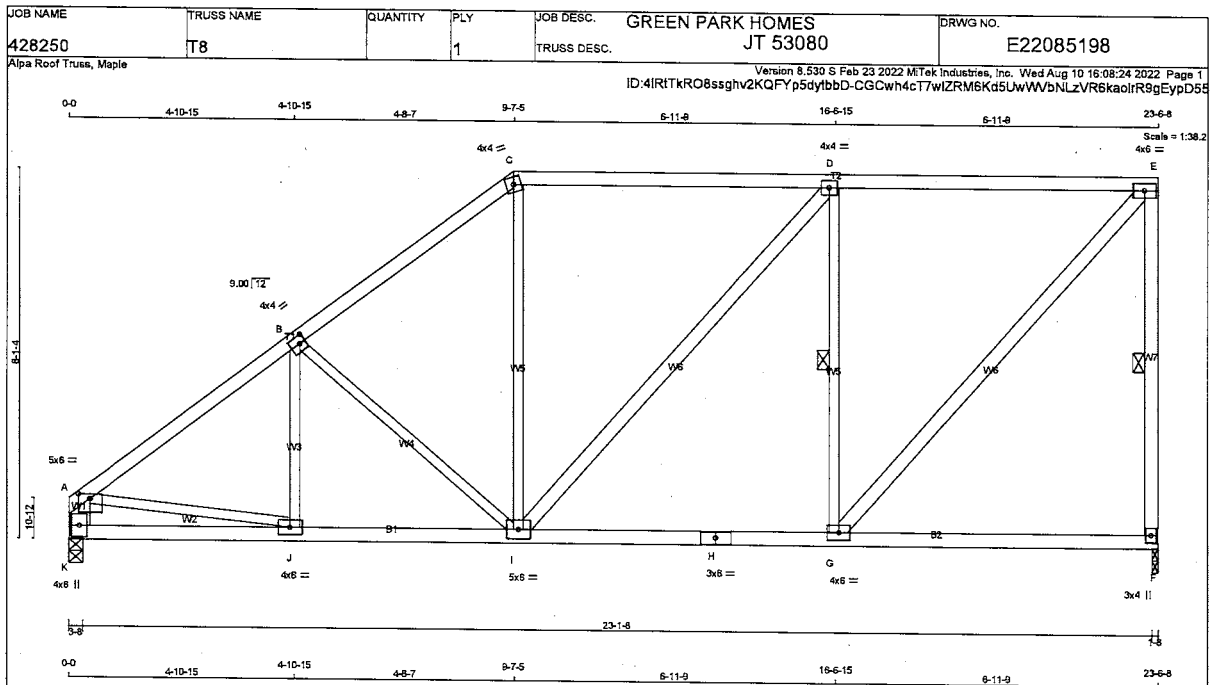
JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2









LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 113 lb	
N L G A, RULES	SIZE	LUMBER	DESCR.	BEARINGS										(M)	
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD							
C - E	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG							
F - E	2x4	DRY	No.2	SPF	JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
K - A	2x4	DRY	No.2	SPF	F	1338	0	1338	0	0	1-8	1-8			
K - H	2x4	DRY	No.2	SPF	K	1338	0	1338	0	0	3-8	1-8			
H - F	2x4	DRY	No.2	SPF											
ALL WEBS	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS										
EXCEPT				SPF	1ST LCASE	MAX/MIN. COMPONENT REACTIONS									
I - D	2x4	DRY	No.2	SPF	JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
G - E	2x4	DRY	No.2	SPF	F	845	629 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0			
					K	845	629 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0			

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015

- PART 9 OF CBC 2012 (2015 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

BRACING				LOADING									
				TOTAL LOAD CASES: (4)									
				CHORDS									
				MEMB. MAX. FACTORED FORCE (LBS)									
				FR-TO									
				A-B									
				B-C									
				C-D									
				D-E									
				E-F									
				F-G									
				G-H									
				H-I									
				I-J									
				J-K									

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS)

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

WEBS

MEMB. MAX. FACTORED FORCE (LBS)

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

WIDYA 100225448

08/10/2022

PROFESSIONAL ENGINEER

ONTOARIO

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

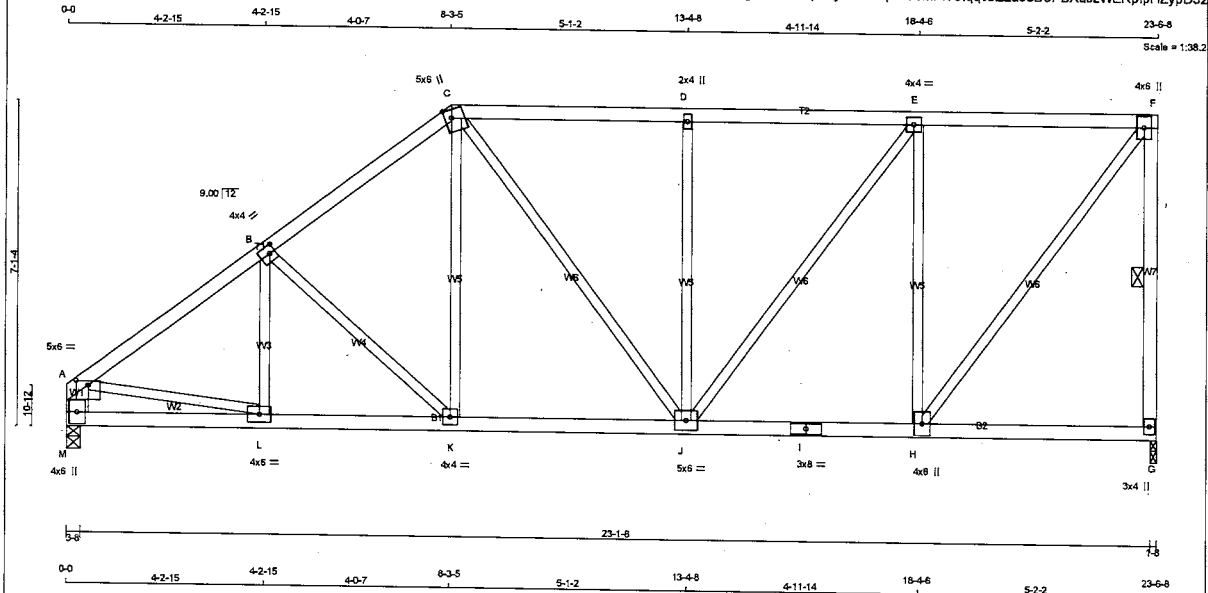
MT20 550 371 1747 788 1987 1673

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (E) (INPUT = 0.90)

JSI METAL = 0.33 (H) (INPUT = 1.00)



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	RECORD	SPECIFIED LOADS:	
A - C	2x4	DRY	No.2	SPF	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 26.7 PSF	
C - F	2x4	DRY	No.2	SPF	SPF	VERT	DOWN	HORIZ	IN-SX	DL = 6.0 PSF	
M - A	2x4	DRY	No.2	SPF	SPF	G	1338	0	0	BOT CH. LL = 0.0 PSF	
M - I	2x4	DRY	No.2	SPF	SPF	M	1338	0	0	DL = 7.4 PSF	
I - G	2x4	DRY	No.2	SPF	SPF					TOTAL LOAD = 40.1 PSF	
ALL WEBS	2x3	DRY	No.2	SPF	SPF	UNFACTORED REACTIONS					
EXCEPT						1ST LOSE	MAX./MIN. COMPONENT REACTIONS			SPACING = 24.0 IN. C/C	
DRY, SEASONED LUMBER.						JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
						G	945	629 / 0	0 / 0	0 / 0	315 / 0
						M	945	629 / 0	0 / 0	0 / 0	315 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING				CHORDS				WEBS			
TOTAL LOAD CASES: (4)				MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1	MAX. MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. UNBRACED LENGTH FR-TO	
				A-B	-1593 / 0	-55.2	-55.2 0.32 (1)	L-B	-97 / 36	0.02 (1)	
				B-C	-1354 / 0	-55.2	-55.2 0.30 (1)	B-K	-320 / 0	0.17 (1)	
				C-D	-1172 / 0	-55.2	-55.2 0.35 (1)	K-C	0 / 306	0.07 (1)	
				D-E	-1172 / 0	-55.2	-55.2 0.37 (1)	A-L	0 / 1307	0.28 (1)	
				E-F	-436 / 0	-55.2	-55.2 0.35 (1)	H-F	0 / 1381	0.31 (1)	
				G-F	-1299 / 0	0.0	0.0 0.30 (1)	C-J	0 / 182	0.04 (1)	
				M-A	-1302 / 0	0.0	0.0 0.28 (1)	H-E	-893 / 0	0.87 (1)	
				M-L	0 / 0	-18.5	-18.5 0.07 (4)	J-D	-519 / 0	0.45 (1)	
				L-K	0 / 1295	-18.5	-18.5 0.25 (1)	J-E	0 / 570	0.13 (1)	
				K-J	0 / 1063	-18.5	-18.5 0.22 (1)				
				J-I	0 / 836	-18.5	-18.5 0.20 (1)				
				I-H	0 / 836	-18.5	-18.5 0.20 (1)				
				H-G	0 / 0	-18.5	-18.5 0.11 (4)				

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (0.78")

CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.05")

ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (0.78")

CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.09")

CSI: TC=0.37/1.00 (D-E-1), BC=0.25/1.00 (K-L-1), WB=0.87/1.00 (E-H-1), SSI=0.23/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				PLATE GRIP(DRY) SHEAR SECTION			
				(PSI)	(PLI)	(PLI)	
				MAX	MIN	MAX	MIN
MT20	650	371	1747	798	1987	1873	

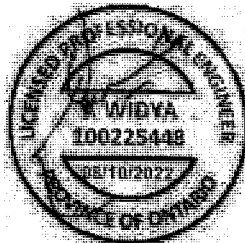
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (H) (INPUT = 0.90)

JSI METAL = 0.38 (F) (INPUT = 1.00)

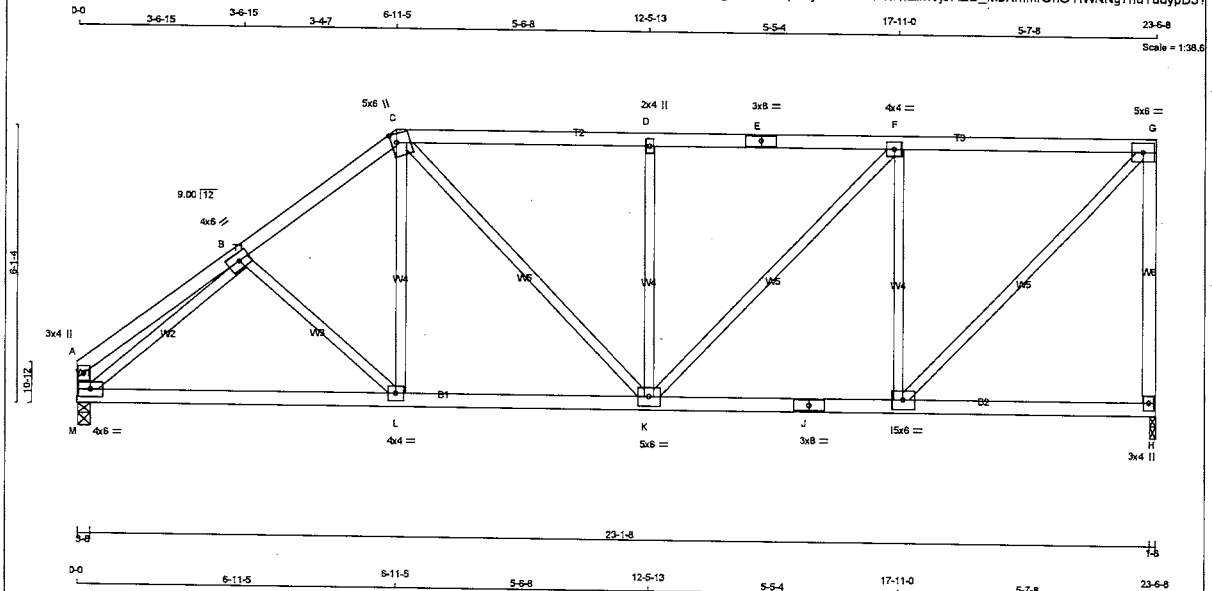
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085200
428250	T10		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Aug 10 15:08:30 2022 Page 1
ID:4IR1KRO8ssghv2KQFYp5dytbbD-0PZBx7hEimVj9H2U_MbKmmrOnOYiWNNg7nuTuuyPD57



LUMBER	N. L. G. A. RULES	SIZE	LUMBER
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - J	2x4	DRY	No.2
M - A	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMV+p	MT20	3.0	4.0	
B TMWW+1	MT20	4.0	6.0	
C TWW+m	MT20	5.0	6.0	2.25 1.50
D TWW+w	MT20	2.0	4.0	
E TS+1	MT20	3.0	6.0	
F TMWW+1	MT20	4.0	4.0	
G TMWW+1	MT20	5.0	6.0	
H BMV1+p	MT20	3.0	4.0	
I BMWW+1	MT20	5.0	6.0	
J BS+1	MT20	3.0	6.0	
K BMWW+1	MT20	5.0	6.0	
L BMWW+1	MT20	4.0	4.0	
M BMWW+1	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT GROSS REACTION				
H VERT	1338	0	0	1-8
M VERT	1338	0	0	1-8
JT GROSS REACTION				
H VERT	1338	0	0	1-8
M VERT	1338	0	0	1-8
UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS	
JT COMBINED				
H 945	628 / 0	0 / 0	0 / 0	315 / 0
M 945	628 / 0	0 / 0	0 / 0	315 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH FR-TO		
A-B	0 / 21	-95.2	-95.2	0.17 (1)	10.00	B-L
B-C	-1451 / 0	-95.2	-95.2	0.20 (1)	5.22	L-C
C-D	-1401 / 0	-95.2	-95.2	0.44 (1)	4.96	I-G
D-E	-1401 / 0	-95.2	-95.2	0.45 (1)	4.93	C-K
E-F	-1401 / 0	-95.2	-95.2	0.43 (1)	5.54	K-D
F-G	-1044 / 0	-95.2	-95.2	0.43 (1)	4.93	I-F
H-G	-1298 / 0	0.0	0.0	0.94 (1)	7.09	K-F
M-A	-130 / 0	0.0	0.0	0.01 (1)	7.81	M-B
M-L	0 / 1263	-18.5	-18.5	0.31 (1)	10.00	
L-K	0 / 1146	-18.5	-18.5	0.30 (4)	10.00	
K-J	0 / 1044	-18.5	-18.5	0.24 (1)	10.00	
J-I	0 / 1044	-18.5	-18.5	0.24 (1)	10.00	
I-H	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (0.78")
CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.06")
ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (0.78")
CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.11")

CSI: TC=0.94/1.00 (G-H-1), BC=0.31/1.00 (L-M-1),
WB=0.60/1.00 (B-M-1), SSI=0.25/1.00 (F-G-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

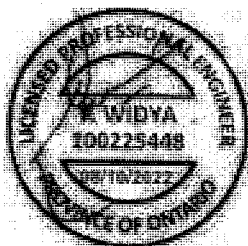
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 798 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

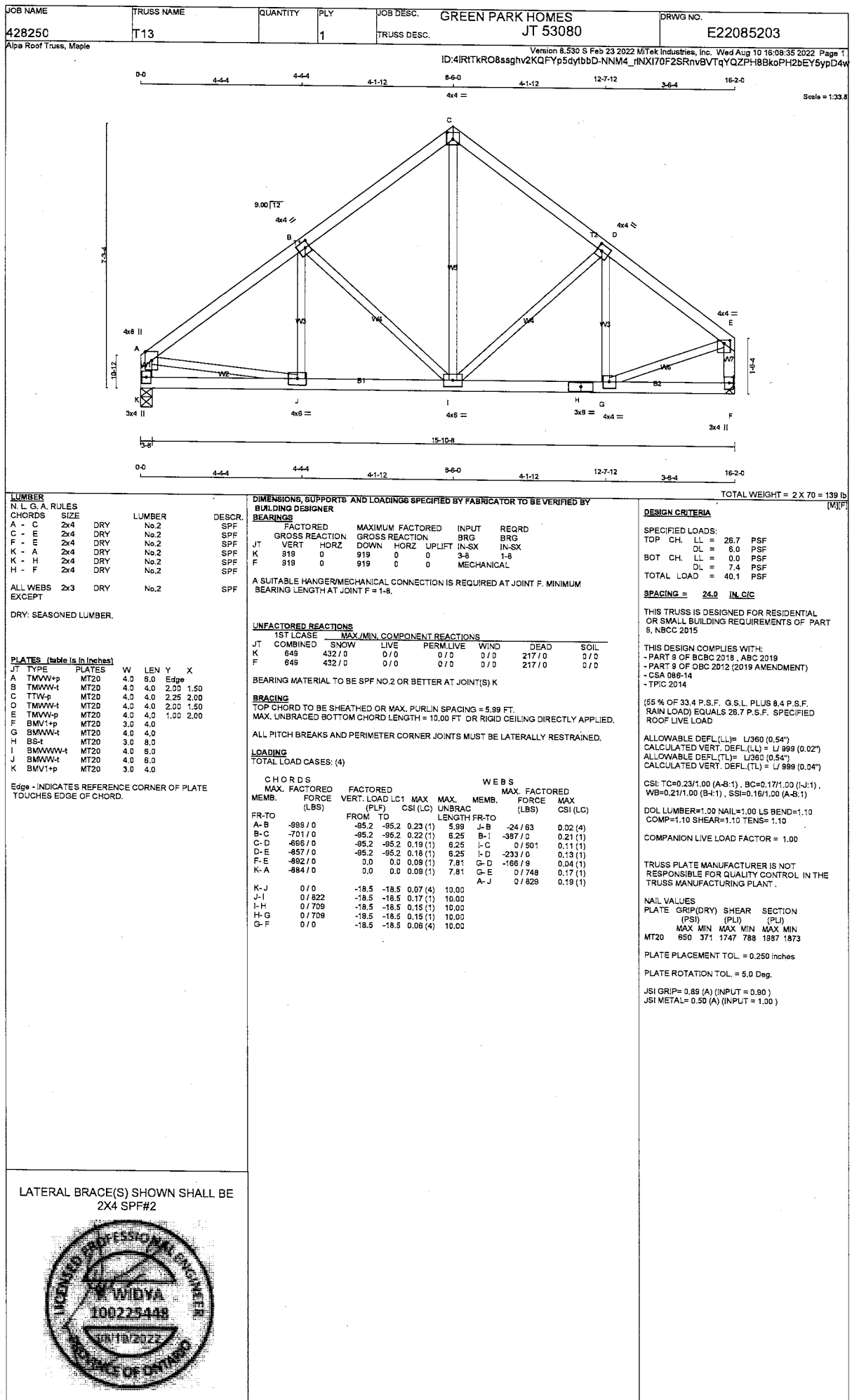




Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRAC. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO		LENGTH FR-TO			
A-B	-1511/0	-95.2	-95.2 0.84 (1)	4.53	K-B	0/96	0.03 (4)
C-B	-1711/0	-95.2	-95.2 0.55 (1)	4.43	B-J	0/845	0.15 (1)
D-C	-1711/0	-95.2	-95.2 0.56 (1)	4.41	J-C	-618/0	0.24 (1)
E-F	-1711/0	-95.2	-95.2 0.55 (1)	4.41	F-E	0/497	0.11 (1)
G-F	-1326/0	-95.2	-95.2 0.52 (1)	4.82	H-E	-937/0	0.36 (1)
E-F	-1293/0	0.0	0.0 0.57 (1)	7.10	F-H	0/1699	0.38 (1)
L-A	-1298/0	0.0	0.0 0.13 (1)	7.09	A-K	0/1213	0.27 (1)
L-K	0/0	-18.5	-18.5 0.15 (4)	10.00			
K-J	0/1207	-18.5	-18.5 0.28 (1)	10.00			
D-I	0/1326	-18.5	-18.5 0.30 (1)	10.00			
I-H	0/1326	-18.5	-18.5 0.30 (1)	10.00			
H-G	0/0	-18.5	-18.5 0.16 (4)	10.00			

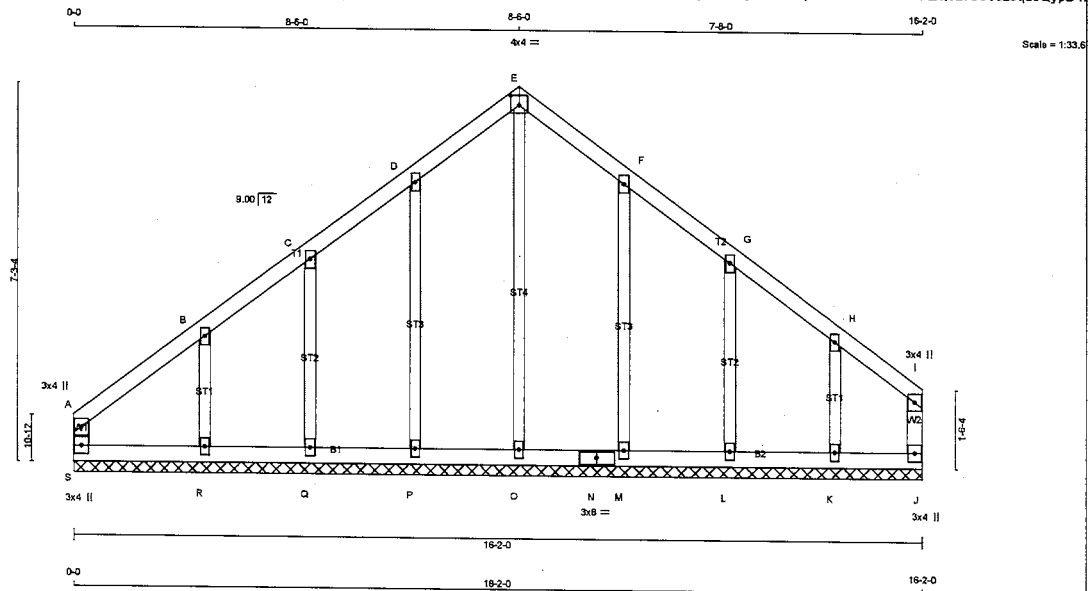
JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.59 (A) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	T13G		1	TRUSS DESC.	JT 53080	E22085204

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Aug 10 16:08:38 2022 Page 1
ID:4IRtKRO8ssghv2KQFYp5dytbbD-ny2DdsnfQDVa6WAOS1kC5SAZQcLVC6VsZ0qu9QypD4f



Scale = 1:33.6

LUMBER	N. L. Q. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
S - A	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
J - J	2x4	DRY	No.2	SPF	
S - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2'-0" OC.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TTW+p	MT20	3.0	4.0			
B, C, D, F, G, H	TTW+w	MT20	2.0	4.0			
E	TTW+p	MT20	4.0	4.0	2.25	2.00	
I	TTW+p	MT20	3.0	4.0			
J	BMV1+p	MT20	3.0	4.0			
K, L, M, O, P, Q, R	BMV1+w	MT20	2.0	4.0			
N	BS-t	MT20	3.0	6.0			
S	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1	MAX. FACTORED FORCE (LBS)	LC1
FR-TO		FROM	TO	UNBRAC	FR-TO				
A-B	-139 / 0	0.0	0.0	0.04 (1)	7.61	Q-E	-162 / 0	0.15 (1)	
B-C	-46 / 0	-95.2	-95.2	0.06 (1)	6.25	P-D	-193 / 0	0.10 (1)	
C-D	-47 / 0	-95.2	-95.2	0.06 (1)	6.25	Q-C	-177 / 0	0.05 (1)	
D-E	-35 / 0	-95.2	-95.2	0.04 (1)	6.25	R-B	-219 / 0	0.04 (1)	
E-F	-32 / 0	-95.2	-95.2	0.05 (1)	6.25	M-F	-190 / 0	0.10 (1)	
F-G	-32 / 0	-95.2	-95.2	0.06 (1)	6.25	L-G	-192 / 0	0.05 (1)	
G-H	-37 / 0	-95.2	-95.2	0.05 (1)	6.25	K-H	-160 / 0	0.03 (1)	
H-I	-39 / 0	-95.2	-95.2	0.05 (1)	6.25				
I-J	-50 / 0	-95.2	-95.2	0.04 (1)	6.25				
J-K	-106 / 0	0.0	0.0	0.04 (1)	7.61				
S-R	0 / 41	-18.5	-18.5	0.03 (4)	10.00				
R-Q	0 / 33	-18.5	-18.5	0.03 (4)	10.00				
Q-P	0 / 29	-18.5	-18.5	0.02 (4)	10.00				
P-O	0 / 26	-18.5	-18.5	0.02 (4)	10.00				
O-N	0 / 26	-18.5	-18.5	0.02 (4)	10.00				
N-M	0 / 26	-18.5	-18.5	0.02 (4)	10.00				
M-L	0 / 29	-18.5	-18.5	0.02 (4)	10.00				
L-K	0 / 34	-18.5	-18.5	0.02 (4)	10.00				
K-J	0 / 40	-18.5	-18.5	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B-1), BC=0.03/1.00 (J-K-1), WB=0.15/1.00 (E-O-1), SS=0.08/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	550	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

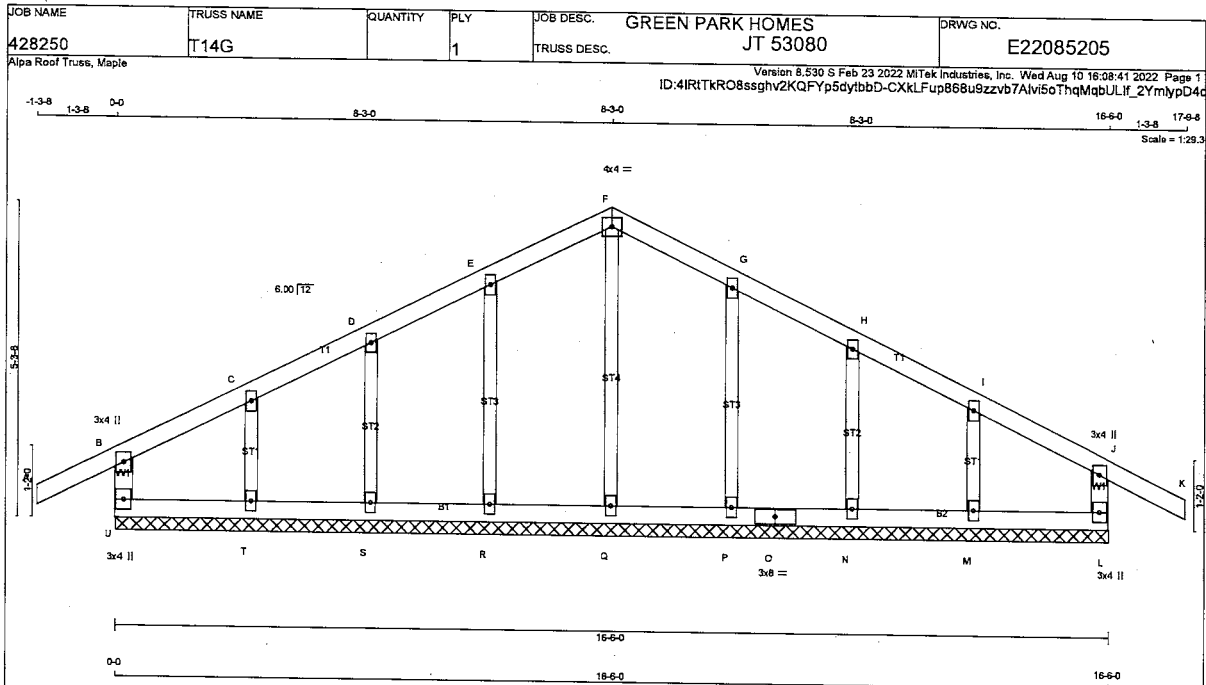
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (E) (INPUT = 0.90)

JSI METAL= 0.12 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
U - B	2x4	DRY	No.2
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, G, H, I					
C	TMV+rw	MT20	2.0	4.0	
F	TTW+p	MT20	4.0	4.0	
J	TMV+p	MT20	3.0	4.0	
L	BMV1+p	MT20	3.0	4.0	
M, N, P, Q, R, S, T					
M	BMV1+rw	MT20	2.0	4.0	
O	BS+1	MT20	3.0	8.0	
U	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 9.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LENGTH (FR-TO)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
U-B	-248 / 0	0.0	0.02 (1)	Q-F	-187 / 0	7.81	0.08 (1)
A-B	0 / 29	-95.2	-95.2 0.12 (1)	R-E	-193 / 0	10.00	0.05 (1)
B-C	-28 / 0	-95.2	-95.2 0.08 (1)	S-D	-188 / 0	6.25	0.04 (1)
C-D	-20 / 0	-95.2	-95.2 0.05 (1)	T-C	-195 / 0	6.25	0.03 (1)
D-E	-14 / 0	-95.2	-95.2 0.04 (1)	P-G	-193 / 0	6.25	0.05 (1)
E-F	-11 / 0	-95.2	-95.2 0.05 (1)	N-H	-186 / 0	6.25	0.04 (1)
F-G	-11 / 0	-95.2	-95.2 0.05 (1)	M-I	-195 / 0	6.25	0.03 (1)
G-H	-14 / 0	-95.2	-95.2 0.04 (1)				
H-I	-20 / 0	-95.2	-95.2 0.05 (1)				
I-J	-28 / 0	-95.2	-95.2 0.06 (1)				
J-K	0 / 29	-95.2	-95.2 0.12 (1)				
L-K	-248 / 0	0.0	0.02 (1)				
U-T	0 / 24	-18.5	-18.5 0.02 (4)				
T-S	0 / 17	-18.5	-18.5 0.02 (4)				
S-R	0 / 13	-18.5	-18.5 0.02 (4)				
R-Q	0 / 10	-18.5	-18.5 0.02 (4)				
Q-P	0 / 10	-18.5	-18.5 0.02 (4)				
P-O	0 / 13	-18.5	-18.5 0.02 (4)				
O-N	0 / 13	-18.5	-18.5 0.02 (4)				
N-M	0 / 17	-18.5	-18.5 0.02 (4)				
M-L	0 / 24	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. G/G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B-1), BC=0.02/1.00 (T-U-4), WB=0.08/1.00 (F-Q-1), SS=0.09/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

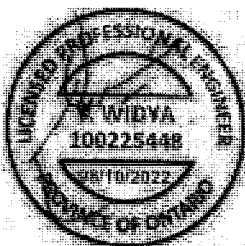
PLATE PLACEMENT TOL = 0.250 inches

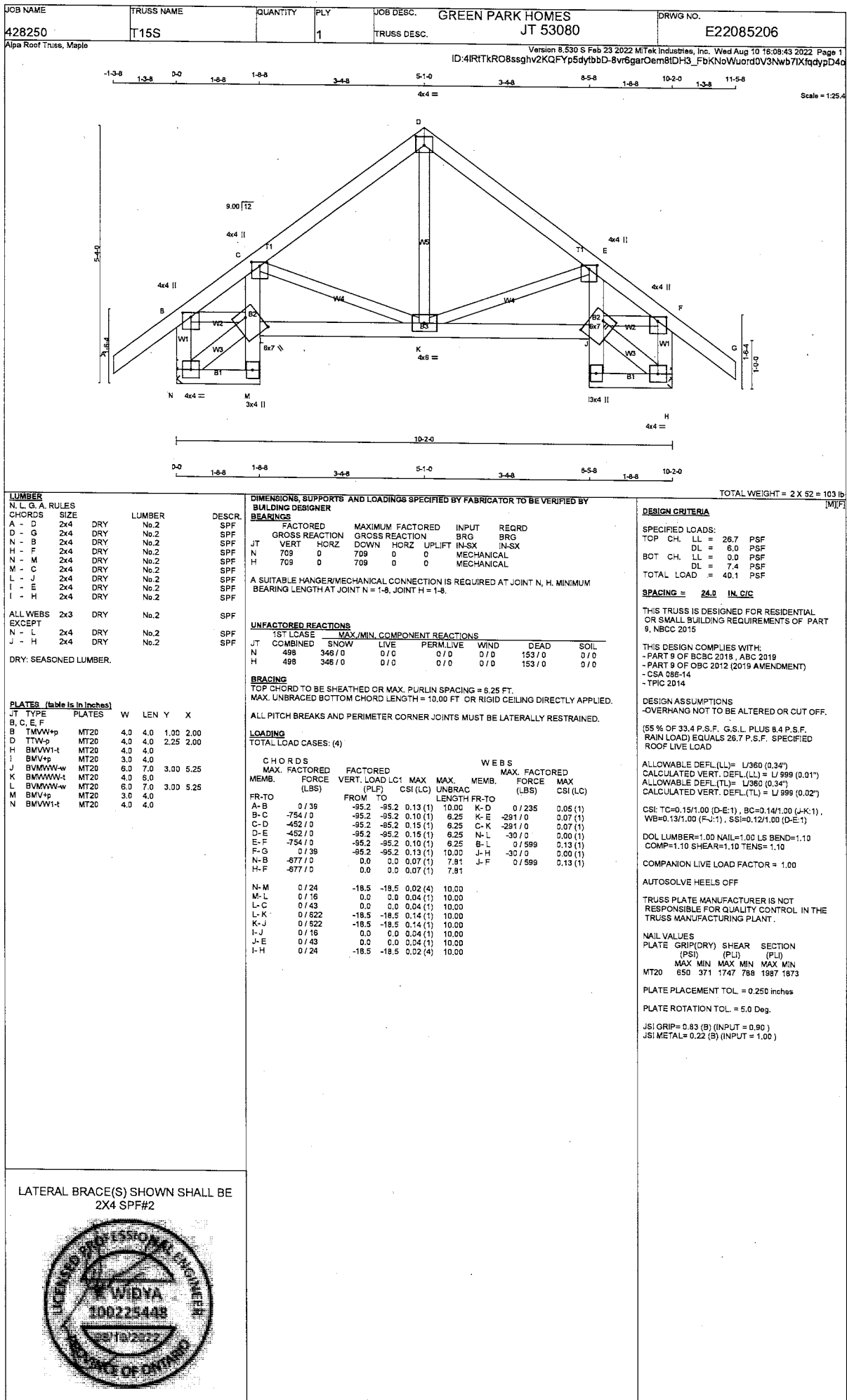
PLATE ROTATION TOL = 5.0 Deg.

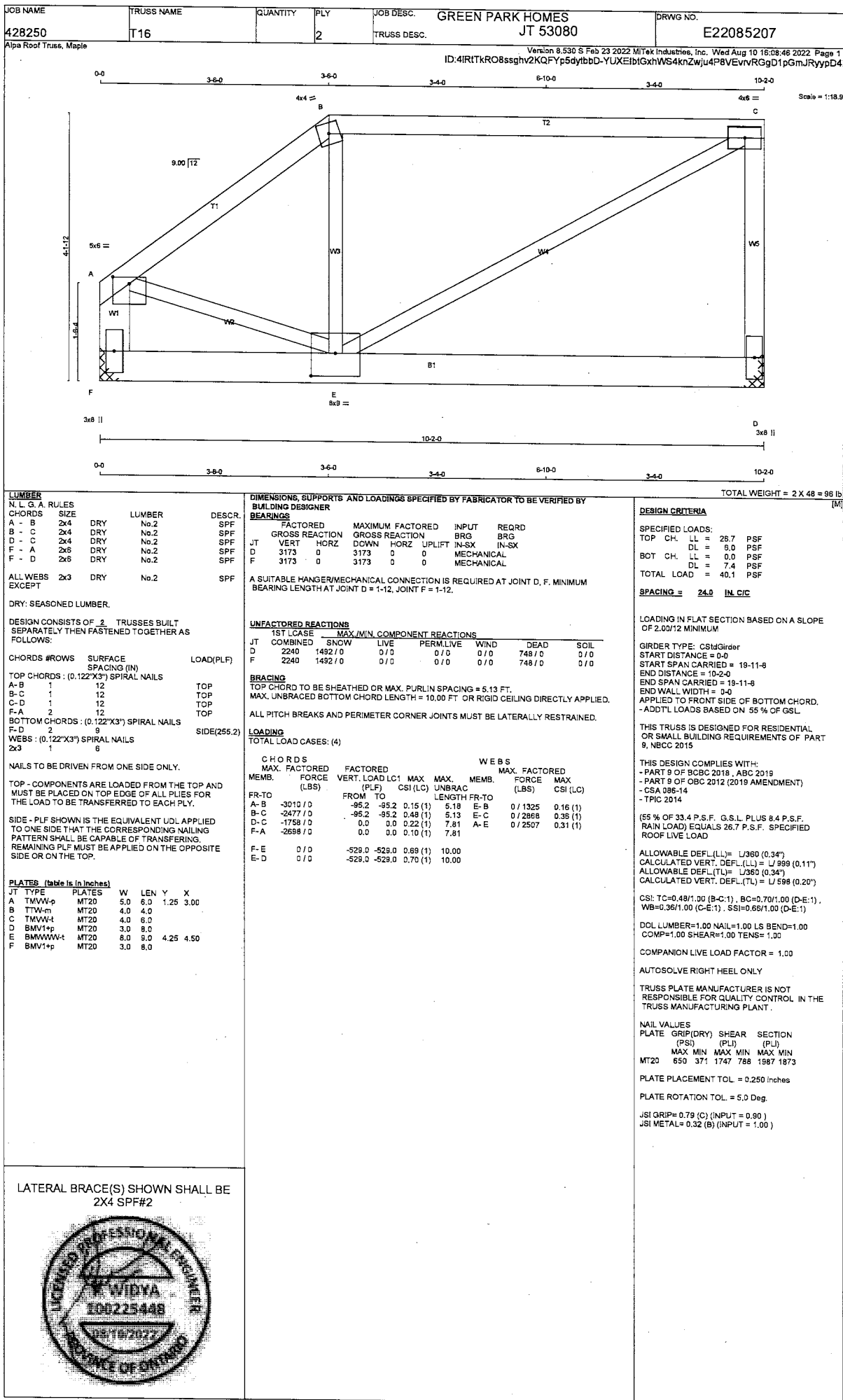
JSI GRIP= 0.39 (F) (INPUT = 0.90)

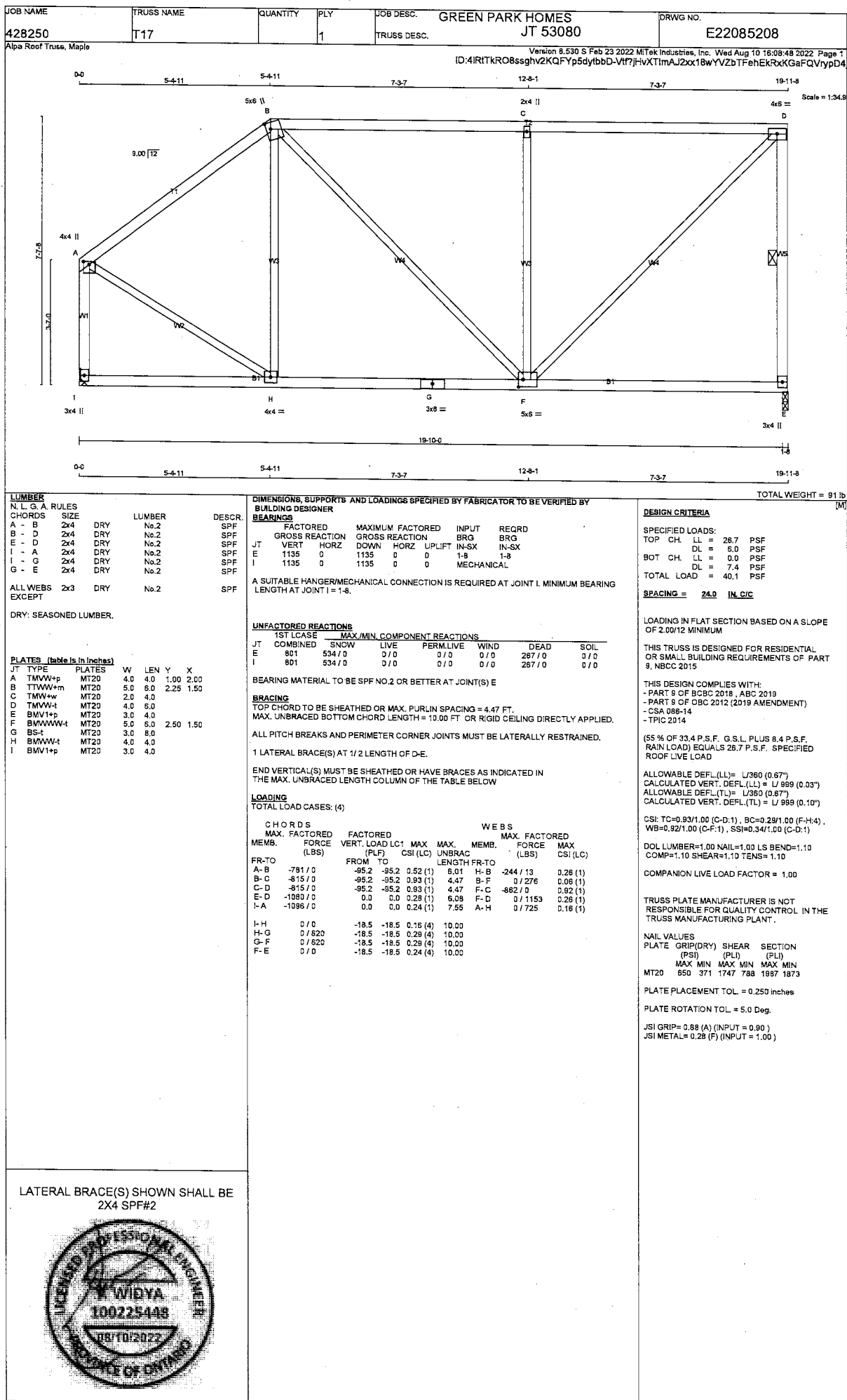
JSI METAL= 0.08 (J) (INPUT = 1.00)

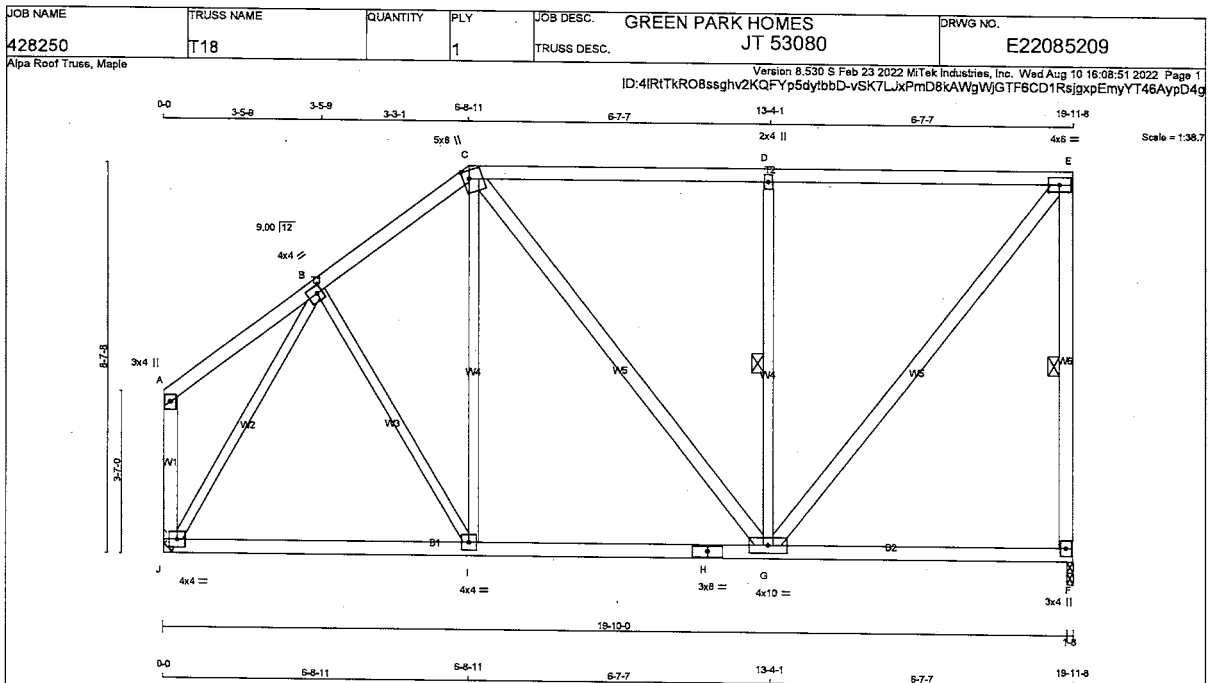
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2











LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - A	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
C - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0	
B	TMWW+t	MT20	4.0	4.0	2.00 1.50
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMWW+m	MT20	2.0	4.0	
E	TMWW+t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWWW+t	MT20	4.0	10.0	
H	BS-t	MT20	3.0	8.0	
I	BMWW+t	MT20	4.0	4.0	
J	BMWW+t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	BRG	BRG	BRG
F	1135	0	1135	0	0	1-8	1-8
J	1135	0	1135	0	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	801	534 / 0 0 / 0 0 / 0 0 / 0 267 / 0 0 / 0
J	801	534 / 0 0 / 0 0 / 0 0 / 0 267 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (CSI) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO				
A-B	0 / 22	-95.2	-95.2 0.17 (1)	10.00	B-I	0 / 114	0.03 (4)
B-C	-797 / 0	-95.2	-95.2 0.18 (1)	6.25	I-C	0 / 86	0.03 (4)
C-D	-674 / 0	-95.2	-95.2 0.74 (1)	5.64	C-G	0 / 89	0.01 (1)
D-E	-674 / 0	-95.2	-95.2 0.75 (1)	5.64	G-D	-783 / 0	0.37 (1)
F-E	-1084 / 0	0.0	0.0 0.37 (1)	6.07	G-E	0 / 1080	0.17 (1)
J-A	-125 / 0	0.0	0.0 0.03 (1)	7.81	J-B	-1110 / 0	0.88 (1)
J-I	0 / 560	-18.5	-18.5 0.23 (4)	10.00			
I-H	0 / 519	-18.5	-18.5 0.24 (4)	10.00			
H-G	0 / 519	-18.5	-18.5 0.24 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA S86-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.67")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.67")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.75/1.00 (D-E:1), BC=0.24/1.00 (G-I:4), WB=0.88/1.00 (B-J:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

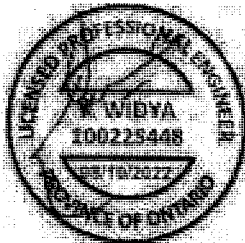
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.80)
JSI METAL= 0.34 (B) (INPUT = 1.00)

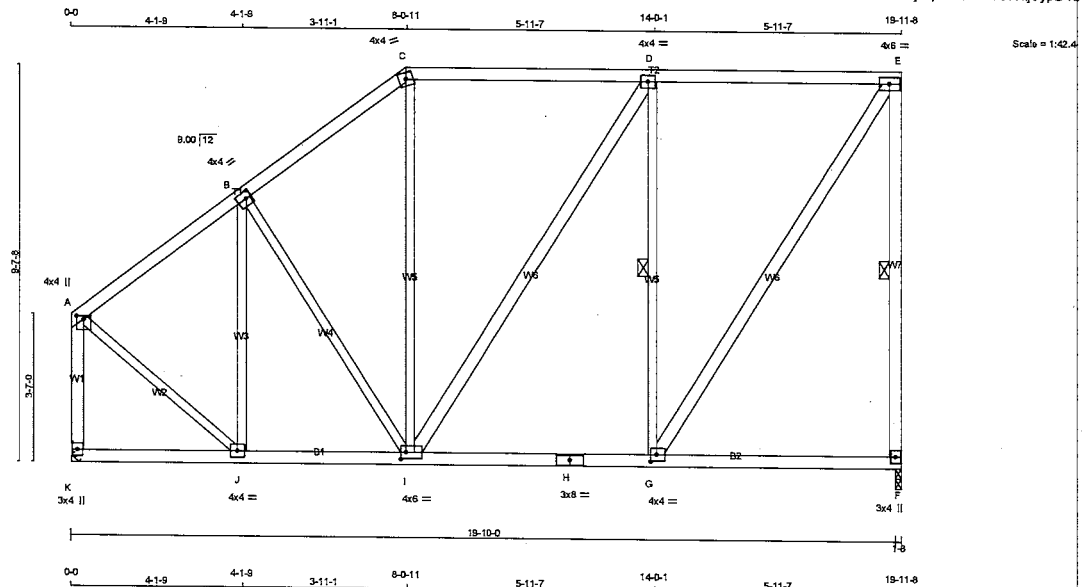
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085210
428250	T19		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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ID:4IRTKRO8ssghv2KQFYp5dytbbD-K10GzKzI28XJ1z05001yqkZ63lu8GYCvMlVypD4d



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVW-t	MT20	4.0	4.0	2.00 1.75
H	BS-t	MT20	3.0	8.0	
I	BMVWV-t	MT20	4.0	8.0	2.00 1.50
J	BMVW-t	MT20	4.0	4.0	
K	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	VERT	HORZ	DOWN	HORZ
K	1135	0	1135	0
K	1135	0	1135	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
F	801	534/0	0/0	0/0
K	801	534/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LENGTH FR-TO
A-B	-746/0	-95.2	-95.2	0.27 (1)	6.25	J-B	-406/0	0.30 (1)	10.00
B-C	-754/0	-95.2	-95.2	0.27 (1)	6.25	B-I	-59/0	0.07 (1)	10.00
C-D	-688/0	-95.2	-95.2	0.59 (1)	6.25	I-C	0/91	0.03 (4)	10.00
D-E	-561/0	-95.2	-95.2	0.59 (1)	6.25	I-D	0/51	0.01 (1)	10.00
E-F	-1081/0	0.0	0.0	0.48 (1)	6.05	G-D	-747/0	0.46 (1)	10.00
K-A	-1103/0	0.0	0.0	0.24 (1)	7.54	G-E	0/1039	0.17 (1)	10.00
K-J	0/0	-18.5	-18.5	0.07 (4)	10.00	A-J	0/787	0.18 (1)	10.00
J-I	0/819	-18.5	-18.5	0.14 (1)	10.00				
I-H	0/561	-18.5	-18.5	0.21 (4)	10.00				
H-G	0/561	-18.5	-18.5	0.21 (4)	10.00				
G-F	0/0	-18.5	-18.5	0.17 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSL: TC=0.59/1.00 (C-D-1), BC=0.21/1.00 (G-H-4), WB=0.46/1.00 (D-G-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

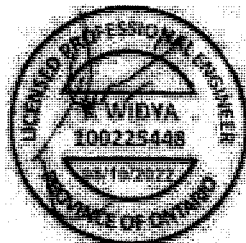
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

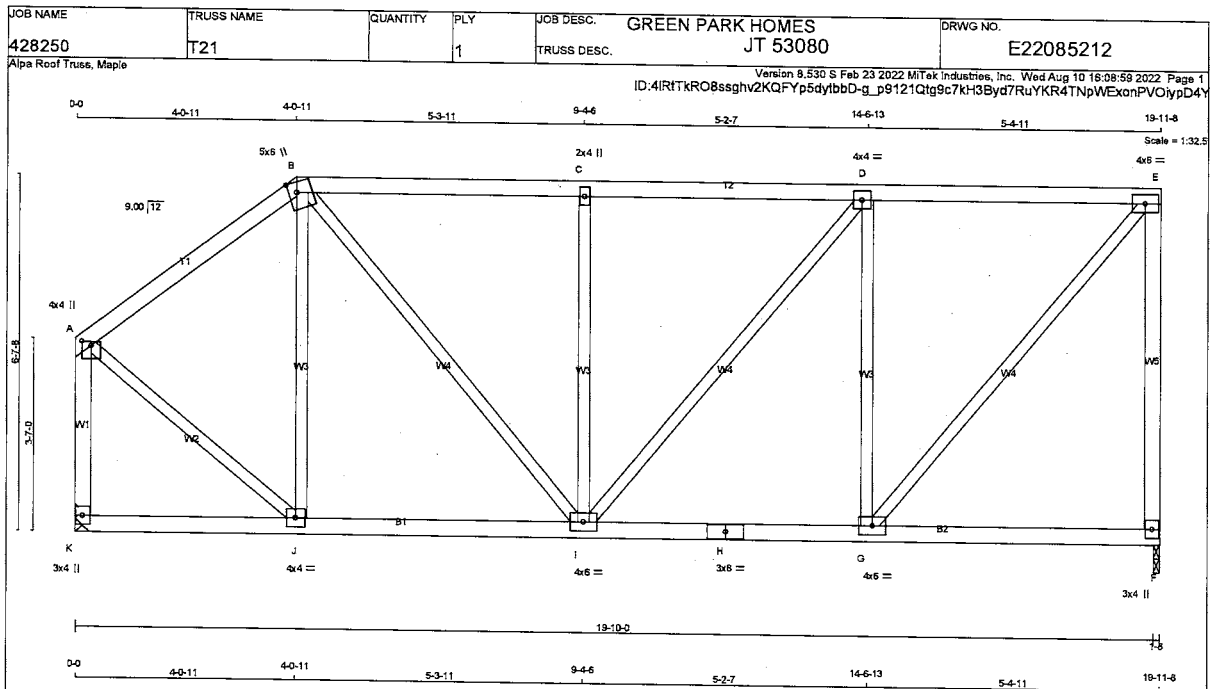
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 95 lb				
N. L. G. A. RULES					BUILDING DESIGNER										(M)				
CHORDS SIZE					LUMBER					DESCR.					DESIGN CRITERIA				
A - B 2x4 DRY					No.2					SPF					FACTORED				
B - E 2x4 DRY					No.2					SPF					GROSS REACTION				
F - E 2x4 DRY					No.2					SPF					BRG BRG				
K - A 2x4 DRY					No.2					SPF					JT				
K - H 2x4 DRY					No.2					SPF					VERT				
H - F 2x4 DRY					No.2					SPF					HORZ				
ALL WEBS 2x3 DRY					No.2					SPF					DOWN				
EXCEPT															HORZ				
DRY: SEASONED LUMBER.															UPLIFT				
															IN-SX				
															IN-SX				
															1-8				
															1-8				
															MECHANICAL				
															A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.				
															SPECIFIED LOADS:				
															TOP CH. LL = 26.7 PSF				
															DL = 6.0 PSF				
															BOT CH. LL = 0.0 PSF				
															DL = 7.4 PSF				
															TOTAL LOAD = 40.1 PSF				
															SPACING = 24.0 IN. C/C				

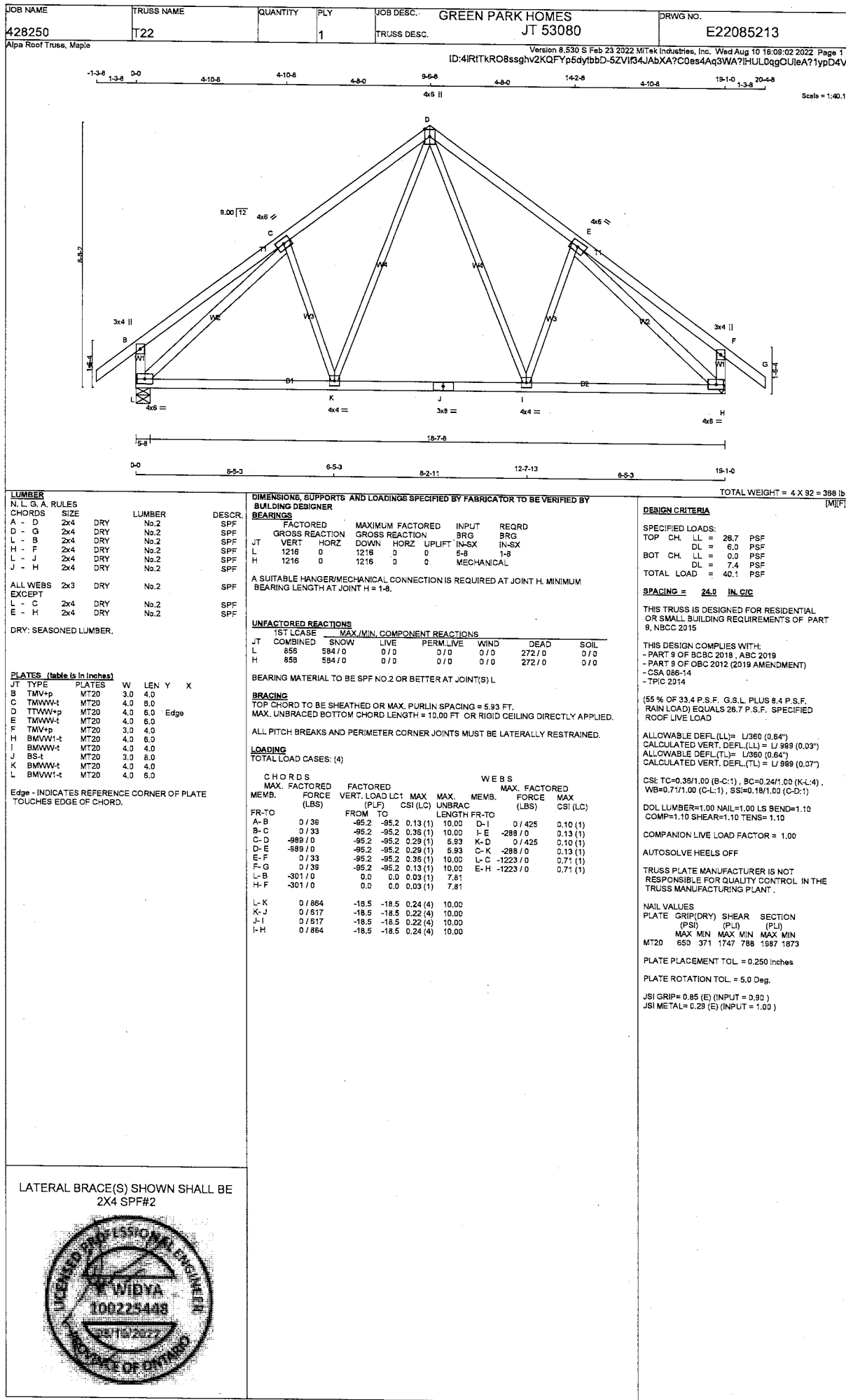
PLATES (Table is in inches)				UNFACTORED REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
JT TYPE	PLATES	W	LEN Y X	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
A	TMWW+P	MT20	4.0 4.0 1.00 2.00	F	801	534 / 0	0 / 0	0 / 0	287 / 0	0 / 0	
B	TTWW+M	MT20	5.0 6.0 2.25 1.75	K	801	534 / 0	0 / 0	0 / 0	287 / 0	0 / 0	
C	TMWW+M	MT20	2.0 4.0	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F							
D	TMWW+M	MT20	4.0 4.0								
E	TMWW+M	MT20	4.0 4.0								
F	BMV+P	MT20	3.0 4.0								
G	BMWW+M	MT20	4.0 6.0								
H	BS+M	MT20	3.0 6.0								
I	BMWW+M	MT20	4.0 6.0								
J	BMWW+M	MT20	4.0 4.0								
K	BMV+P	MT20	3.0 4.0								

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-745 / 0	-95.2	-95.2 0.28 (1)	5.25	J-B	-385 / 0	0.26 (1)
B-C	-934 / 0	-95.2	-95.2 0.37 (1)	5.87	A-J	0 / 755	0.17 (1)
C-D	-935 / 0	-95.2	-95.2 0.39 (1)	5.84	G-E	0 / 1164	0.26 (1)
D-E	-755 / 0	-95.2	-95.2 0.39 (1)	6.25	B-I	0 / 539	0.12 (1)
E-F	-1094 / 0	0.0	0.0 1.00 (1)	7.55	G-D	-775 / 0	0.56 (1)
K-A	-1107 / 0	0.0	0.0 0.24 (1)	7.52	I-C	-542 / 0	0.39 (1)
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00	I-D	0 / 293	0.06 (1)
J-I	0 / 588	-18.5	-18.5 0.15 (1)	10.00			
I-H	0 / 755	-18.5	-18.5 0.19 (1)	10.00			
H-G	0 / 755	-18.5	-18.5 0.19 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

TOTAL LOAD CASES: (4)				ALLOWABLE DEFL. (LL) = U/360 (0.67")			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	ALLOWABLE VERT. DEFL. (LL) = U/999 (0.03")	ALLOWABLE DEFL. (TL) = U/360 (0.67")	ALLOWABLE VERT. DEFL. (TL) = U/999 (0.06")	
FR-TO		FROM	TO	CS: TC=1.00/1.00 (E-F-1), BC=0.19/1.00 (G-H-1), WB=0.56/1.00 (D-G-1), SS=0.24/1.00 (D-E-1)			
A-B	-745 / 0	-95.2	-95.2 0.28 (1)	5.25			
B-C	-934 / 0	-95.2	-95.2 0.37 (1)	5.87			
C-D	-935 / 0	-95.2	-95.2 0.39 (1)	5.84			
D-E	-755 / 0	-95.2	-95.2 0.39 (1)	6.25			
E-F	-1094 / 0	0.0	0.0 1.00 (1)	7.55			
K-A	-1107 / 0	0.0	0.0 0.24 (1)	7.52			
K-J	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
J-I	0 / 588	-18.5	-18.5 0.15 (1)	10.00			
I-H	0 / 755	-18.5	-18.5 0.19 (1)	10.00			
H-G	0 / 755	-18.5	-18.5 0.19 (1)	10.00			
G-F	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

NAIL VALUES				COMPANION LIVE LOAD FACTOR = 1.00			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN MAX MIN MAX MIN	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
MT20	650	371	1747	788	1987	1973	
PLATE PLACEMENT TOL. = 0.250 inches							
PLATE ROTATION TOL. = 5.0 Deg.							
JSI GRIP= 0.89 (E) (INPUT = 0.90)							
JSI METAL= 0.27 (H) (INPUT = 1.00)							

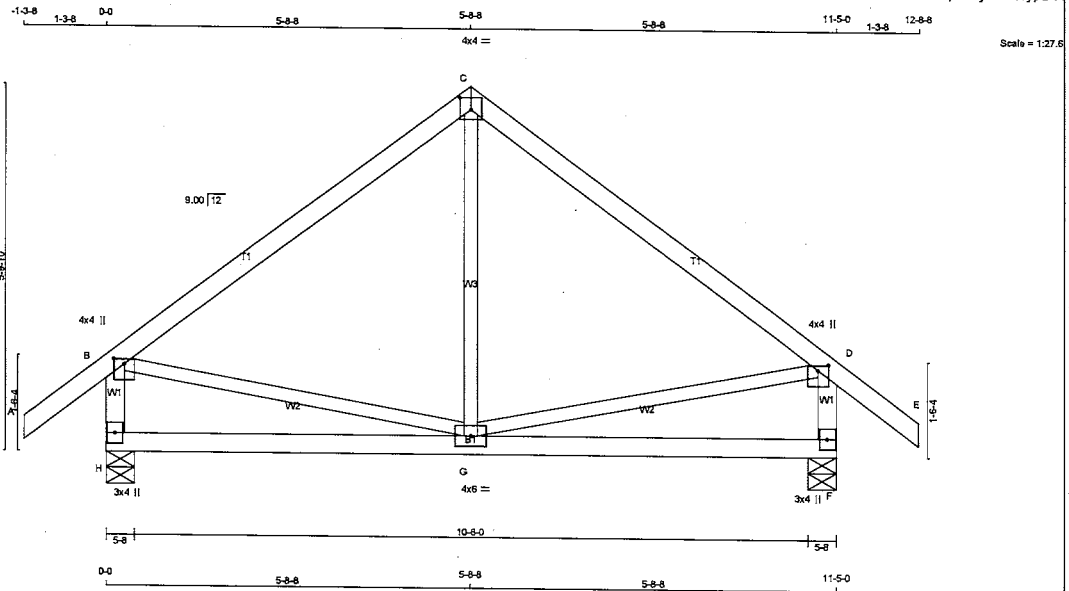
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085214
428250	T23		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

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Scale = 1:27.6

TOTAL WEIGHT = 49 lb (M/F)

LUMBER	N L G A RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+p	MT20	4.0	4.0	2.25	2.00
D	TMWW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWV-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
H 780	0	780	0	0
F 780	0	780	0	0

UNFACTORED REACTIONS	1ST CASE	MAX / MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM. LIVE
H 548	379 / 0	0 / 0	0 / 0
F 548	379 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F.
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1
TOTAL LOAD CASES: (4)										
	FR-TO									
	A-B	0 / 39								
	B-C	-439 / 0								
	C-D	-439 / 0								
	D-E	0 / 39								
	H-B	-740 / 0								
	F-D	-740 / 0								
	H-G	0 / 0								
	G-F	0 / 0								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.40/1.00 (C-D:1), BC=0.17/1.00 (F-G:4),
WB=0.08/1.00 (D-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.20 L8 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

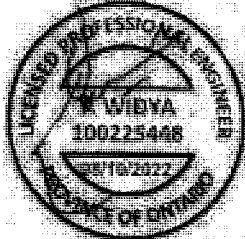
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

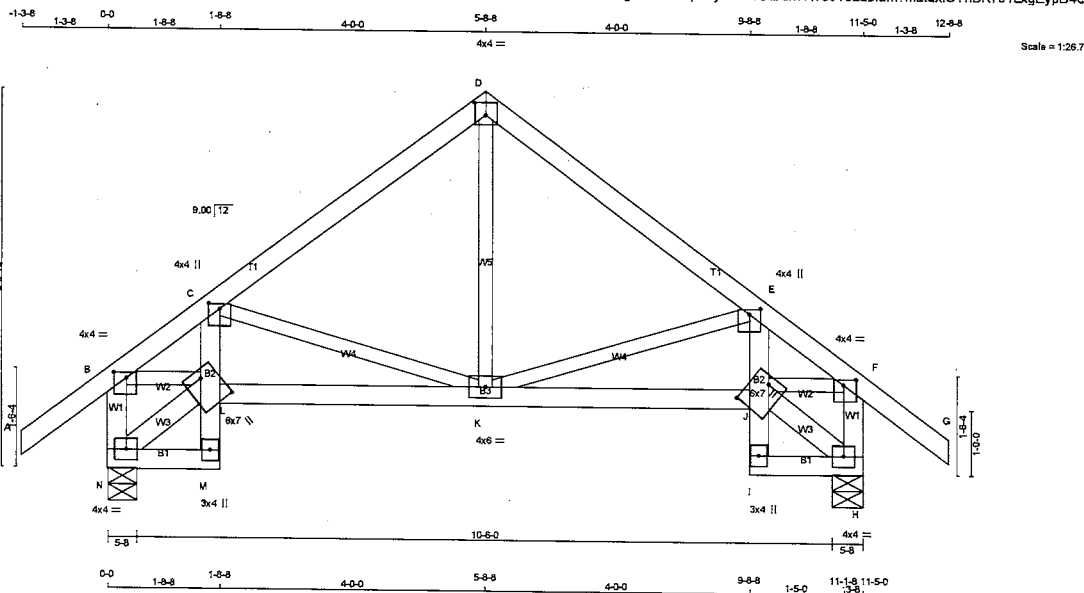
JSI GRIP= 0.66 (D) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

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ID:4IRtKRO8ssghv2KQFYp5dytbbD-RXIAIn7R?89T5zubfdm?matuxlCYhDR7e1LxgEypD4C



Scale = 1:26.7

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	4.0	4.0	1.00	2.25
C	TMWW-p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-p	MT20	4.0	4.0	1.00	2.00
F	TMWW-p	MT20	4.0	4.0	1.00	2.25
H	BMVW-t	MT20	4.0	4.0		
I	BMV-p	MT20	3.0	4.0		
J	BMVWW-w	MT20	6.0	7.0	3.00	5.50
K	BMVWW-t	MT20	4.0	6.0		
L	BMVWW-w	MT20	6.0	7.0	3.00	5.50
M	BMV-p	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
N	780	780	0	0
H	780	780	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	548	379 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	548	379 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
(LBS)	VERT.	LOAD	LC1	(LBS)	FORCE	MAX.	CS1 (LC)
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	K-D	0 / 267
B-C	-892 / 0	-95.2	-95.2	0.14 (1)	8.25	K-E	-366 / 0
C-D	-520 / 0	-95.2	-95.2	0.20 (1)	8.25	C-K	-366 / 0
D-E	-520 / 0	-95.2	-95.2	0.20 (1)	8.25	N-L	-35 / 0
E-F	-892 / 0	-95.2	-95.2	0.14 (1)	8.25	B-L	0 / 723
F-G	0 / 39	-95.2	-95.2	0.13 (1)	10.00	J-H	-35 / 0
N-B	-745 / 0	0.0	0.0	0.08 (1)	7.81	J-F	0 / 723
H-F	-745 / 0	0.0	0.0	0.08 (1)	7.81		
N-M	0 / 28	-18.5	-18.5	0.02 (4)	10.00		
M-L	0 / 16	0.0	0.0	0.05 (1)	10.00		
L-C	0 / 46	0.0	0.0	0.05 (1)	10.00		
L-K	0 / 750	-18.5	-18.5	0.17 (1)	10.00		
K-J	0 / 750	-18.5	-18.5	0.17 (1)	10.00		
I-J	0 / 18	0.0	0.0	0.05 (1)	10.00		
J-E	0 / 46	0.0	0.0	0.05 (1)	10.00		
I-H	0 / 28	-18.5	-18.5	0.02 (4)	10.00		

TOTAL WEIGHT = 2 X 56 = 112 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPI 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = U/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = U/999 (0.03")
CS1: TC=0.20/1.00 (D-E:1), BC=0.17/1.00 (J-K:1), WB=0.16/1.00 (F-J:1), SS=0.14/1.00 (D-E:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

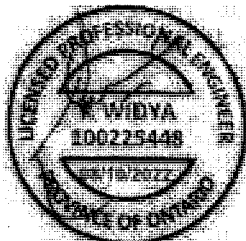
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.80)
JSI METAL= 0.25 (F) (INPUT = 1.00)

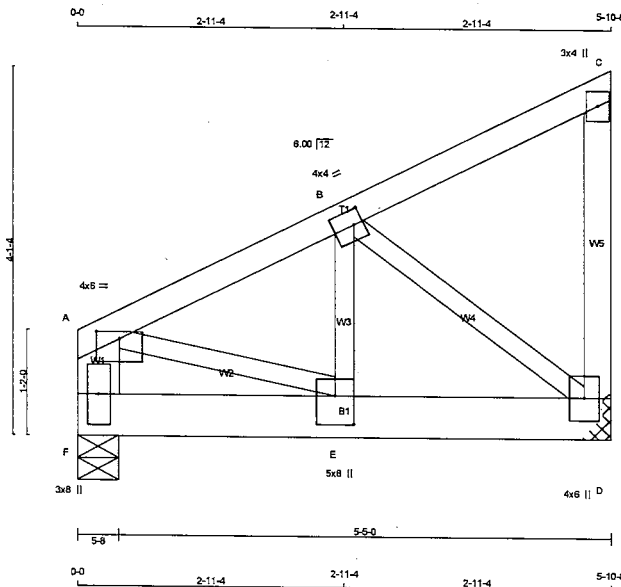
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME T24	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085216
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Alpa Roof Truss, Maple

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Scale = 1:19.5

TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	9	SIDE(242.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	4.0	6.0	1.00	3.00
B	TMWV+1	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMWV+1	MT20	4.0	6.0		
E	BMWV+1	MT20	5.0	6.0	3.75	2.50
F	BMV+1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
F	1760	0	1760	0
D	1760	0	1760	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
F	1243	828/0
D	1243	828/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PLITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	CS1(LC)	UNBRAC	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	CS1(LC)
FR-TO															
F-A	-1189/0	0.0	0.0	0.04 (1)	7.81	A-E	0/1386	0.17 (1)							
A-B	-1488/0	-95.2	-95.2	0.06 (1)	6.25	E-B	0/1366	0.17 (1)							
B-C	-11/0	-95.2	-95.2	0.05 (1)	6.25	B-D	-1687/0	0.20 (1)							
D-C	-116/0	0.0	0.0	0.01 (1)	7.81										
F-E	0/0	-504.1	-504.1	0.14 (1)	10.00										
E-D	0/1340	-504.1	-504.1	0.23 (1)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

GIROER TYPE, CstbGirder
START DISTANCE = 0-0
START SPAN CARRIED = 19-1-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 19-1-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 65 % OF GSI.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.06/1.00 (A-B-1), BC=0.23/1.00 (D-E-1), WB=0.20/1.00 (B-C-1), SS=0.28/1.00 (E-F-1)

DOL LUMBER=1.30 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

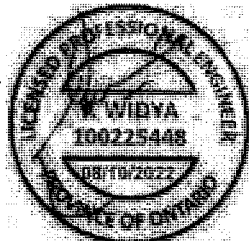
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1973

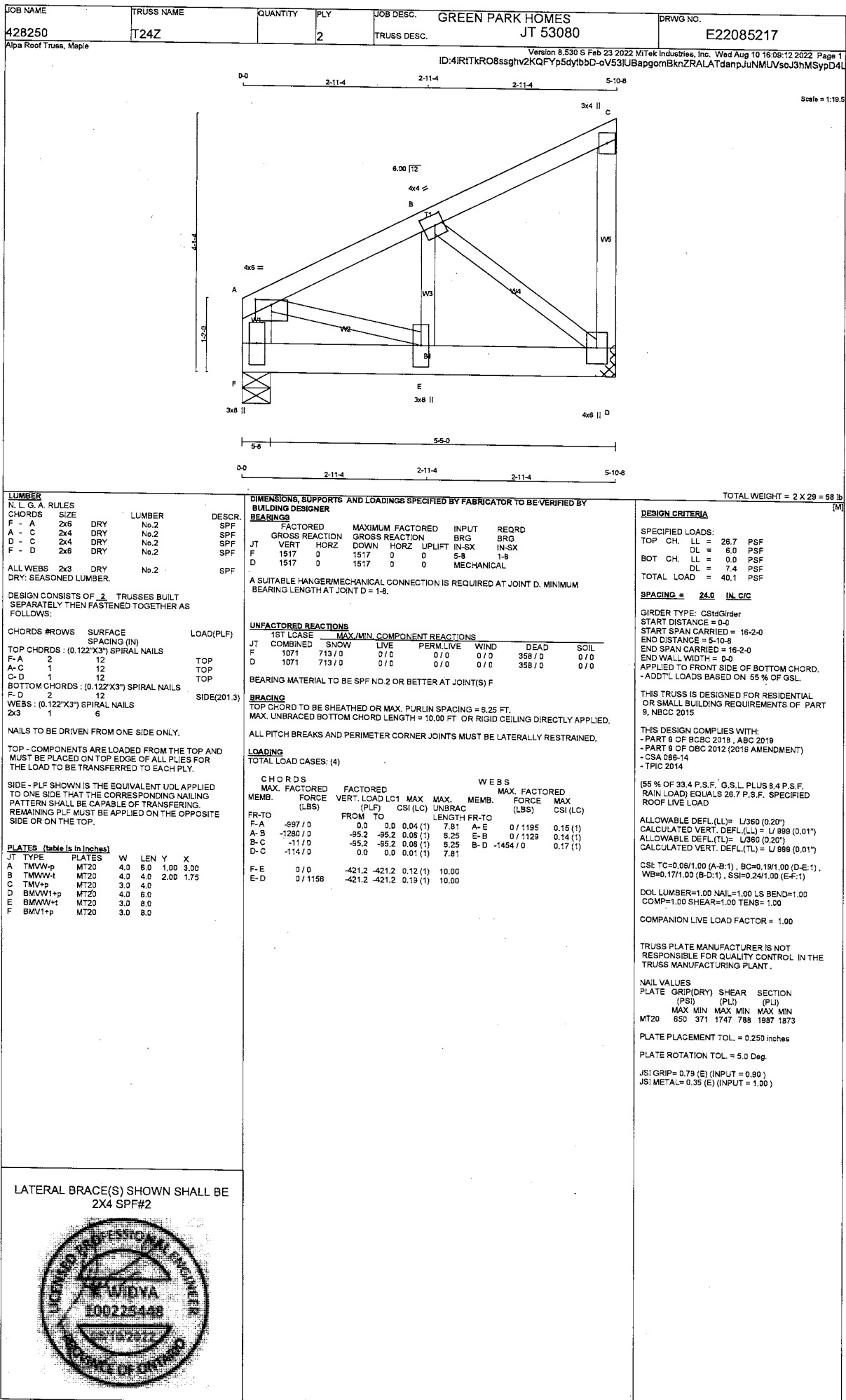
PLATE PLACEMENT TOL = 0.250 inches

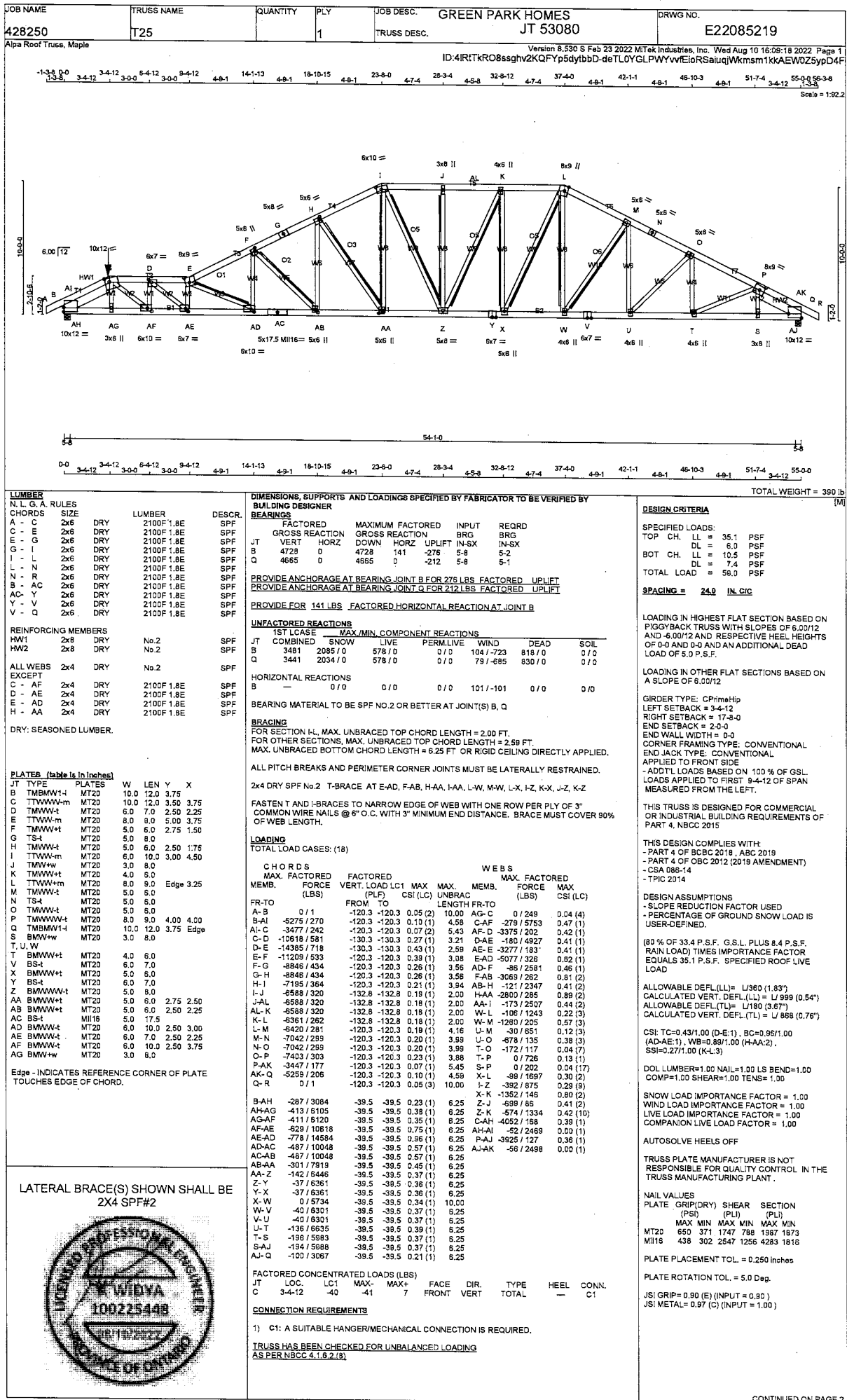
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.50)
JSI METAL= 0.30 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2







JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428250	T25		1	GREEN PARK HOMES JT 53080	E22085219(2)
Alpine Roof Truss, Maple					

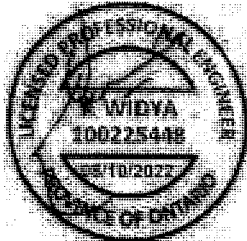
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CONNECTION REQUIREMENTS

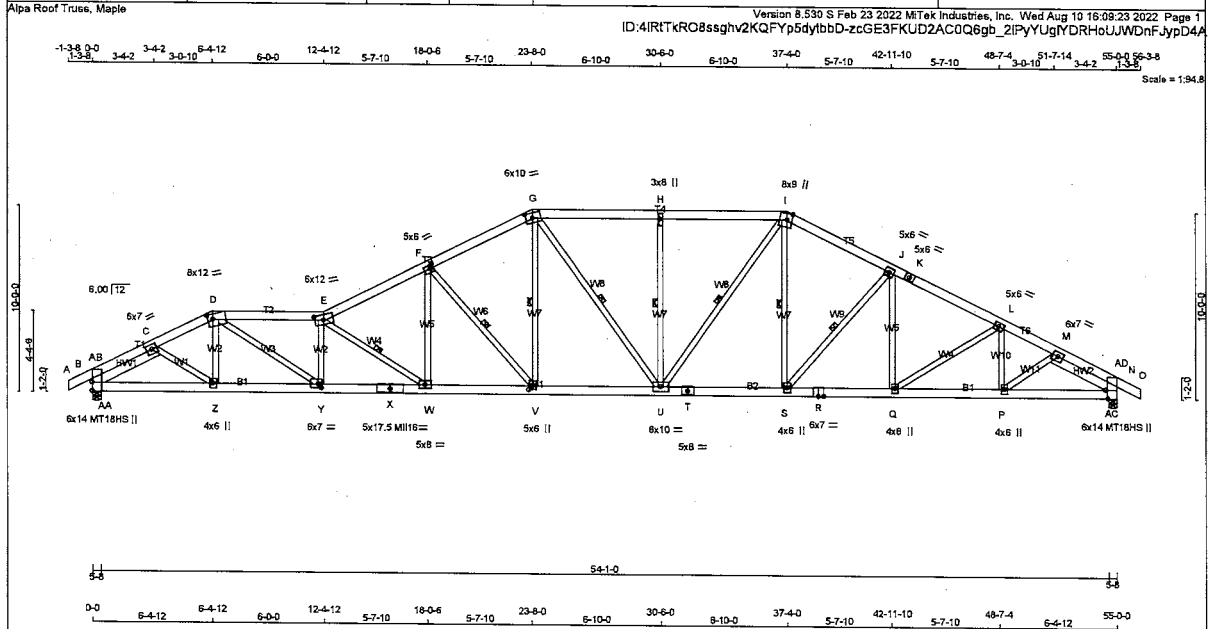
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (00-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 8.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



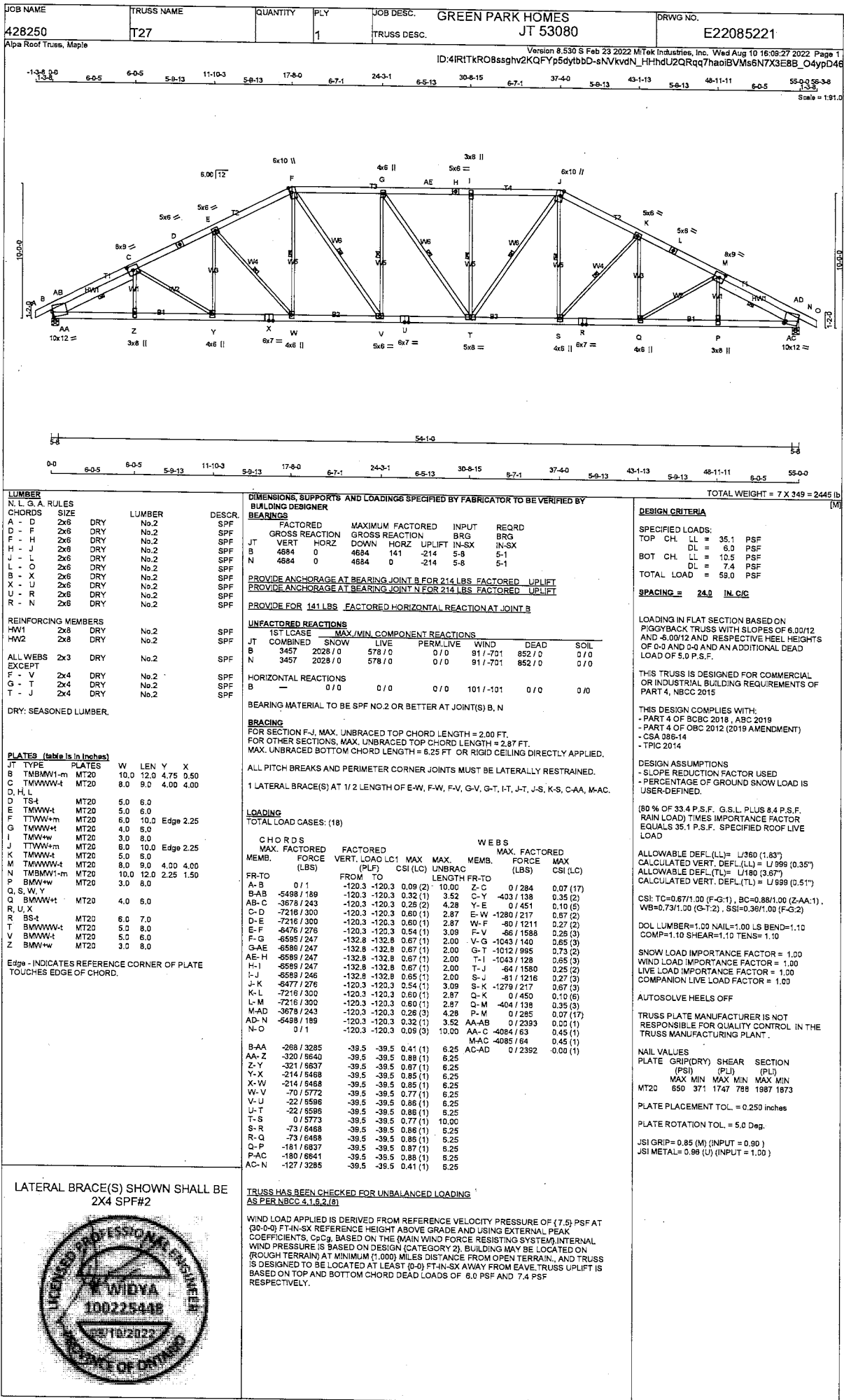
JOB NAME 428250	TRUSS NAME T26	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. E22085220
Alpa Roof Truss, Maple		Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Aug 10 16:09:23 2022 Page 1			



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES										BEARINGS										SPECIFIED LOADS:									
CHORDS SIZE										FACTORED MAXIMUM FACTORED INPUT REQD										TOP CH. LL = 35.1 PSF									
A - D 2x6 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										DL = 6.0 PSF									
D - E 2x6 DRY 2100F 1.8E SPF										B 4637 0 4637 141 -269 5-8 5-0										BOT CH. LL = 10.5 PSF									
E - G 2x6 DRY No.2 SPF										N 4656 0 4656 0 -211 5-8 5-1										DL = 7.4 PSF									
G - I 2x6 DRY No.2 SPF										PROVIDE ANCHORAGE AT BEARING JOINT B FOR 269 LBS FACTORED UPLIFT										TOTAL LOAD = 59.0 PSF									
I - K 2x6 DRY No.2 SPF										PROVIDE ANCHORAGE AT BEARING JOINT N FOR 211 LBS FACTORED UPLIFT																			
K - O 2x6 DRY No.2 SPF										PROVIDE FOR 141 LBS FACTORED HORIZONTAL REACTION AT JOINT B																			
B - X 2x6 DRY 2100F 1.8E SPF										UNFACTORED REACTIONS										LOADING IN HIGHEST FLAT SECTION BASED ON									
X - T 2x6 DRY 2100F 1.8E SPF										1ST LCASE MAX/MIN COMPONENT REACTIONS										PIGGYBACK TRUSS WITH SLOPES OF 6.00/12									
T - R 2x6 DRY 2100F 1.8E SPF										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS									
R - N 2x6 DRY 2100F 1.8E SPF										B 3420 2029/0 578/0 0/0 102/-716 815/0 0/0										OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD									
REINFORCING MEMBERS										N 3435 2029/0 578/0 0/0 78/-684 830/0 0/0										LOAD OF 5.0 P.S.F.									
HW1 2x6 DRY No.2 SPF										HORIZONTAL REACTIONS										LOADING IN OTHER FLAT SECTIONS BASED ON									
HW2 2x6 DRY No.2 SPF										B 0/0 0/0 0/0 101/-101 0/0 0/0										A SLOPE OF 6.00/12									
ALL WEBS 2x4 DRY No.2 SPF										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N										THIS TRUSS IS DESIGNED FOR COMMERCIAL									
EXCEPT										BRACING										OR INDUSTRIAL BUILDING REQUIREMENTS OF									
F - V 2x4 DRY 2100F 1.8E SPF										FOR SECTION G-I, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.										PART 4, NBCC 2015									
DRY, SEASONED LUMBER.										FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.33 FT.										THIS DESIGN COMPLIES WITH:									
										MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.										- PART 4 OF NBCC 2018, ABC 2019									
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.										- PART 4 OF OBC 2012 (2019 AMENDMENT)									
										1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-W, F-V, G-V, H-U, H-I, I-S, J-S.										- CSA 086-14									
										LOADING										- TPIC 2014									
										TOTAL LOAD CASES: (18)										DESIGN ASSUMPTIONS									
										CHORDS										- SLOPE REDUCTION FACTOR USED									
										MEMB. MAX. FACTORED FORCE (LBS)										- PERCENTAGE OF GROUND SNOW LOAD IS									
										VERT. LOAD LC1 MAX. FACTORED (PLF) (LBS)										USER-DEFINED.									
										FROM TO LENGTH FR-TO										(80% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F.									
										A-B 0/120.3 -120.3 0.09 (2) 10.30 C-Z 0/865 0.14 (1)										RAIN LOAD) TIMES IMPORTANCE FACTOR									
										B-AB -4882/173 -120.3 -120.3 0.27 (1) 3.73 D-D -212/88 0.03 (8)										EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE									
										AB-C -2812/166 -120.3 -120.3 0.21 (1) 4.79 D-Y -194/5228 0.84 (1)										LOAD									
										C-D -7346/407 -120.3 -120.3 0.36 (1) 3.00 Y-E -2787/188 0.49 (1)										ALLOWABLE DEFL.(LL) = U360 (1.63)									
										D-E -10919/549 -120.3 -120.3 0.44 (2) 3.1 E-W -3426/256 0.77 (2)										CALCULATED VERT. DEFL.(LL) = U 898 (0.48")									
										E-F -3152/435 -120.3 -120.3 0.77 (1) 2.33 W-F -72/2207 0.35 (2)										ALLOWABLE DEFL.(TL) = U180 (0.67)									
										F-G -7176/355 -120.3 -120.3 0.58 (1) 2.86 F-V -2554/307 0.68 (2)										CALCULATED VERT. DEFL.(TL) = U 974 (0.68")									
										G-H -6554/290 -132.8 -132.8 0.78 (1) 2.00 V-G -150/2487 0.40 (2)										CSI: TC=0.79/1.00 (H+1), BC=0.62/1.00 (W-Y:1),									
										H-I -6554/290 -132.8 -132.8 0.79 (1) 2.00 G-U -634/980 0.46 (2)										WB=0.84/1.00 (D-Y:1), SSI=0.36/1.00 (H:3)									
										I-J -6415/271 -120.3 -120.3 0.53 (1) 3.1 U-H -1097/93 0.48 (1)										DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10									
										J-K -7137/293 -120.3 -120.3 0.80 (1) 2.90 H-I -127/1865 0.30 (2)										COMP=1.10 SHEAR=1.10 TENS=1.10									
										K-L -7137/293 -120.3 -120.3 0.80 (1) 3.90 S-I -83/1244 0.20 (3)										SNOW LOAD IMPORTANCE FACTOR = 1.00									
										L-M -7331/307 -120.3 -120.3 0.40 (1) 4.02 S-J -1302/218 0.48 (3)										WIND LOAD IMPORTANCE FACTOR = 1.00									
										M-AD -2909/149 -120.3 -120.3 0.22 (1) 3.72 Q-J 0/478 0.08 (3)										LIVE LOAD IMPORTANCE FACTOR = 1.00									
										AD-N -5043/133 -120.3 -120.3 0.31 (1) 3.67 Q-L -474/145 0.29 (3)										COMPANION LIVE LOAD FACTOR = 1.00									
										N-O 0/1 -120.3 -120.3 0.09 (3) 10.00 P-L -400/66 0.06 (7)																			

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF 17.5 PSF AT (30-0-0) FT-N-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe}, BASED ON THE MAIN WIND FORCE RESISTING SYSTEM, INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.00) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-N-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.



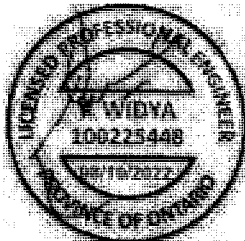
JOB NAME	TRUSS NO.	QUANTITY	PLY	JOB DESCRIPTION	GREEN PARK HOMES	DRWG NO.
428250	T29		1	TRUSS DESC.	JT 53080	E22085223
Alpa Roof Truss, Maple						
Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:08:37 2022 Page 1 ID:4IRITkROBsgghv2KQFYp5dytbbD-Zl6W?1VGWLxDhaBMPw1_v7ueOLqjdPXXicWjVypD3y						
Scale = 1/8"=1'-0"						
LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x6 DRY No.2 SPF B - F 2x6 DRY No.2 SPF G - H 2x6 DRY No.2 SPF J - L 2x6 DRY No.2 SPF K - O 2x6 DRY No.2 SPF P - R 2x6 DRY No.2 SPF S - T 2x6 DRY No.2 SPF U - V 2x6 DRY No.2 SPF W - X 2x6 DRY No.2 SPF Y - Z 2x6 DRY No.2 SPF AA - AD 2x6 DRY 2100F 1.8E SPF AE - AF 2x6 DRY 2100F 1.8E SPF AG - AH 2x6 DRY 2100F 1.8E SPF AI - AJ 2x6 DRY 2100F 1.8E SPF AK - AL 2x6 DRY 2100F 1.8E SPF AM - AN 2x6 DRY 2100F 1.8E SPF AO - AP 2x6 DRY 2100F 1.8E SPF AQ - AR 2x6 DRY 2100F 1.8E SPF AS - AT 2x6 DRY 2100F 1.8E SPF AU - AV 2x6 DRY 2100F 1.8E SPF AW - AX 2x6 DRY 2100F 1.8E SPF AY - AZ 2x6 DRY 2100F 1.8E SPF BA - BB 2x6 DRY 2100F 1.8E SPF BC - BD 2x6 DRY 2100F 1.8E SPF BE - BF 2x6 DRY 2100F 1.8E SPF BG - BH 2x6 DRY 2100F 1.8E SPF BI - BJ 2x6 DRY 2100F 1.8E SPF BK - BL 2x6 DRY 2100F 1.8E SPF BM - BN 2x6 DRY 2100F 1.8E SPF BO - BP 2x6 DRY 2100F 1.8E SPF BQ - BR 2x6 DRY 2100F 1.8E SPF BS - BT 2x6 DRY 2100F 1.8E SPF BU - BV 2x6 DRY 2100F 1.8E SPF BW - BX 2x6 DRY 2100F 1.8E SPF BY - BZ 2x6 DRY 2100F 1.8E SPF CA - CB 2x6 DRY 2100F 1.8E SPF CC - CD 2x6 DRY 2100F 1.8E SPF CE - CF 2x6 DRY 2100F 1.8E SPF CG - CH 2x6 DRY 2100F 1.8E SPF CI - CJ 2x6 DRY 2100F 1.8E SPF CK - CL 2x6 DRY 2100F 1.8E SPF CM - CN 2x6 DRY 2100F 1.8E SPF CO - CP 2x6 DRY 2100F 1.8E SPF CQ - CR 2x6 DRY 2100F 1.8E SPF CS - CT 2x6 DRY 2100F 1.8E SPF CU - CV 2x6 DRY 2100F 1.8E SPF CW - CX 2x6 DRY 2100F 1.8E SPF CY - CZ 2x6 DRY 2100F 1.8E SPF DA - DB 2x6 DRY 2100F 1.8E SPF DC - DD 2x6 DRY 2100F 1.8E SPF DE - DF 2x6 DRY 2100F 1.8E SPF DG - DH 2x6 DRY 2100F 1.8E SPF DI - DJ 2x6 DRY 2100F 1.8E SPF DK - DL 2x6 DRY 2100F 1.8E SPF DM - DN 2x6 DRY 2100F 1.8E SPF DO - DP 2x6 DRY 2100F 1.8E SPF DQ - DR 2x6 DRY 2100F 1.8E SPF DS - DT 2x6 DRY 2100F 1.8E SPF DU - DV 2x6 DRY 2100F 1.8E SPF DW - DX 2x6 DRY 2100F 1.8E SPF DY - DZ 2x6 DRY 2100F 1.8E SPF EA - EB 2x6 DRY 2100F 1.8E SPF EC - ED 2x6 DRY 2100F 1.8E SPF EE - EF 2x6 DRY 2100F 1.8E SPF EG - EH 2x6 DRY 2100F 1.8E SPF EI - EJ 2x6 DRY 2100F 1.8E SPF EK - EL 2x6 DRY 2100F 1.8E SPF EM - EN 2x6 DRY 2100F 1.8E SPF EO - EP 2x6 DRY 2100F 1.8E SPF EQ - ER 2x6 DRY 2100F 1.8E SPF ES - ET 2x6 DRY 2100F 1.8E SPF EU - EV 2x6 DRY 2100F 1.8E SPF EW - EX 2x6 DRY 2100F 1.8E SPF EY - EZ 2x6 DRY 2100F 1.8E SPF FA - FB 2x6 DRY 2100F 1.8E SPF FC - FD 2x6 DRY 2100F 1.8E SPF FE - FF 2x6 DRY 2100F 1.8E SPF FG - FH 2x6 DRY 2100F 1.8E SPF FI - FJ 2x6 DRY 2100F 1.8E SPF FK - FL 2x6 DRY 2100F 1.8E SPF FM - FN 2x6 DRY 2100F 1.8E SPF FO - FP 2x6 DRY 2100F 1.8E SPF FQ - FR 2x6 DRY 2100F 1.8E SPF FS - FT 2x6 DRY 2100F 1.8E SPF FU - FV 2x6 DRY 2100F 1.8E SPF FW - FX 2x6 DRY 2100F 1.8E SPF FY - FZ 2x6 DRY 2100F 1.8E SPF GA - GB 2x6 DRY 2100F 1.8E SPF GC - GD 2x6 DRY 2100F 1.8E SPF GE - GF 2x6 DRY 2100F 1.8E SPF GH - GI 2x6 DRY 2100F 1.8E SPF GJ - GK 2x6 DRY 2100F 1.8E SPF GL - GM 2x6 DRY 2100F 1.8E SPF GN - GO 2x6 DRY 2100F 1.8E SPF GP - GR 2x6 DRY 2100F 1.8E SPF GS - GT 2x6 DRY 2100F 1.8E SPF GU - GV 2x6 DRY 2100F 1.8E SPF GW - GX 2x6 DRY 2100F 1.8E SPF GY - GZ 2x6 DRY 2100F 1.8E SPF HA - HB 2x6 DRY 2100F 1.8E SPF HC - HD 2x6 DRY 2100F 1.8E SPF HE - HF 2x6 DRY 2100F 1.8E SPF HG - HH 2x6 DRY 2100F 1.8E SPF HI - HJ 2x6 DRY 2100F 1.8E SPF HK - HL 2x6 DRY 2100F 1.8E SPF HM - HN 2x6 DRY 2100F 1.8E SPF HO - HP 2x6 DRY 2100F 1.8E SPF HQ - HR 2x6 DRY 2100F 1.8E SPF HS - HT 2x6 DRY 2100F 1.8E SPF HU - HV 2x6 DRY 2100F 1.8E SPF HW - HX 2x6 DRY 2100F 1.8E SPF HY - HZ 2x6 DRY 2100F 1.8E SPF IA - IB 2x6 DRY 2100F 1.8E SPF IC - ID 2x6 DRY 2100F 1.8E SPF IE - IF 2x6 DRY 2100F 1.8E SPF IG - IH 2x6 DRY 2100F 1.8E SPF II - IJ 2x6 DRY 2100F 1.8E SPF IK - IL 2x6 DRY 2100F 1.8E SPF IM - IN 2x6 DRY 2100F 1.8E SPF IO - IP 2x6 DRY 2100F 1.8E SPF IQ - IR 2x6 DRY 2100F 1.8E SPF IS - IT 2x6 DRY 2100F 1.8E SPF IU - IV 2x6 DRY 2100F 1.8E SPF IW - IX 2x6 DRY 2100F 1.8E SPF IY - IZ 2x6 DRY 2100F 1.8E SPF JA - JB 2x6 DRY 2100F 1.8E SPF JC - JD 2x6 DRY 2100F 1.8E SPF JE - JF 2x6 DRY 2100F 1.8E SPF JG - JH 2x6 DRY 2100F 1.8E SPF JJ - JK 2x6 DRY 2100F 1.8E SPF JL - JM 2x6 DRY 2100F 1.8E SPF JN - JO 2x6 DRY 2100F 1.8E SPF JP - JR 2x6 DRY 2100F 1.8E SPF JS - JT 2x6 DRY 2100F 1.8E SPF JU - JV 2x6 DRY 2100F 1.8E SPF JW - JX 2x6 DRY 2100F 1.8E SPF JY - JZ 2x6 DRY 2100F 1.8E SPF KA - KB 2x6 DRY 2100F 1.8E SPF KC - KD 2x6 DRY 2100F 1.8E SPF KE - KF 2x6 DRY 2100F 1.8E SPF KG - KH 2x6 DRY 2100F 1.8E SPF KI - KJ 2x6 DRY 2100F 1.8E SPF KK - KL 2x6 DRY 2100F 1.8E SPF KM - KN 2x6 DRY 2100F 1.8E SPF KO - KP 2x6 DRY 2100F 1.8E SPF KQ - KR 2x6 DRY 2100F 1.8E SPF KS - KT 2x6 DRY 2100F 1.8E SPF KU - KV 2x6 DRY 2100F 1.8E SPF KW - KX 2x6 DRY 2100F 1.8E SPF KY - KZ 2x6 DRY 2100F 1.8E SPF LA - LB 2x6						

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



JOB NAME 428250	TRUSS NO. T30	QUANTITY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085224
Alpa Roof Truss, Maple				
Version 8.530 S Feb 23 2022 MITK Industries, Inc. Wed Aug 10 16:09:43 2022 Page 1 ID:4IRITR0B8sgshvZKQFyP5dytbbD-OSTnG5J1WBIMPVeWmBPRDANIBplu7ILQwd3qxypD3s				

LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER DESCR. SPF
A - E 2x6 DRY No.2 SPF
E - G 2x8 DRY No.2 SPF
G - I 2x6 DRY No.2 SPF
I - K 2x6 DRY No.2 SPF
K - M 2x6 DRY 1650F 1.5E SPF
M - O 2x6 DRY 1650F 1.5E SPF
O - P 2x6 DRY 1650F 1.5E SPF
P - R 2x6 DRY No.2 SPF
R - AC 2x6 DRY 2100F 1.8E SPF
AC - Y 2x6 DRY 2100F 1.8E SPF
Y - V 2x6 DRY 2100F 1.8E SPF
V - Q 2x6 DRY 2100F 1.8E SPF

REINFORCING MEMBERS
HW1 2x8 DRY No.2 SPF
HW2 2x8 DRY No.2 SPF

ALL WEBS EXCEPT T - P 2x4 DRY 1650F 1.5E SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATE	W	LEN	Y	X
B	TBMWW-I	MT20	10.0	12.0	3.75
C	TMMWW-V	MT20	6.0	10.0	2.25 4.25
D	TMMWW-T	MT20	5.0	6.0	
E	TS-I	MT20	5.0	6.0	
F	TMMWW-t	MT20	5.0	6.0	
G	TTWW-m	MT20	8.0	9.0	4.25 3.00
H	TMMWW-t	MT20	3.0	8.0	
I	TS-I	MT20	5.0	6.0	
J	TMMWW-t	MT20	4.0	6.0	
K	TTWW-m	MT20	8.0	9.0	Edge 3.25
L	TMMWW-t	MT20	4.0	6.0	3.00 1.00
M	TS-I	MT20	5.0	8.0	
N	TMMWW-t	MT20	4.0	6.0	2.75 1.00
O	TTWW-m	MT20	8.0	9.0	4.75 4.00
P	TTWW-m	MT18H	9.0	16.0	3.00 6.00
Q	TBMWW-I	MT20	10.0	12.0	3.75 Edge
R	SMWW-t	MT20	3.0	8.0	
S	BMWW-t	MT20	6.0	10.0	2.50 2.75
T	BMWW-t	MT20	5.0	8.0	2.50 3.75
V	BS-I	MT16	5.0	17.5	
W	BMWW-t	MT20	5.0	6.0	
X	BMWW-t	MT20	5.0	6.0	
Y	BS-I	MT20	6.0	7.0	
Z	BA AD	MT20	4.0	6.0	
AA	BMWW-W	MT20	6.0	10.0	
AC	BS-I	MT20	6.0	7.0	
AE	BMWW-t	MT20	4.0	6.0	
AF	BMWW-w	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 35.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.4 PSF
TOTAL LOAD = 59.0 PSF
SPACING = 24.0 IN/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.0/12 AND -6.0/12 AND RESPECTIVE HEEL HEIGHTS OF 0'-0" AND 0'-0" AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.0/12

BRACING
FOR SECTION G-K, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-AA,
2x4 DRY SPF No.2 T-BRACE AT F-AB, G-AB, K-X, N-W, O-U, K-Z, G-AA, J-Z, H-AA
2x4 DRY SPF No.2 I-BRACE AT L-X**

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
TOTAL LOAD CASES: (18)

CHORDS			WEBS		
MEMB.	FORCE (LBS)	FACTORED	MEMB.	FORCE (LBS)	FACTORED
A-B	0/1	-120.3	A-F-C	0/254	0.04(4)
B-AH	-5429/204	-120.3	C-AE	0/1002	0.18(1)
AH-C	-3451/164	-120.3	D-AD	-250/84	0.06(8)
C-D	-8051/310	-120.3	E-AD	-27/575	0.10(2)
D-E	-7745/308	-120.3	F-AB	-1218/209	0.56(2)
E-F	-7745/308	-120.3	G-H	-84/1255	0.22(2)
F-G	-7137/288	-120.3	H-K	-139/2157	0.38(3)
G-H	-7408/252	-132.8	I-L	-2321/261	0.85(3)
H-I	-7408/252	-132.8	J-M	-48/1799	0.32(3)
I-J	-7408/252	-132.8	K-N	-2417/223	0.64(3)
J-K	-7864/345	-120.3	L-O	-3528/232	0.87(1)
K-L	-7864/345	-120.3	M-P	-4005/221	0.48(1)
L-M	-8240/400	-120.3	N-Q	-278/7433	0.95(1)
M-N	-8240/400	-120.3	O-P	0/679	0.12(1)
N-O	-1115/468	-120.3	P-AJ	-65/1385	0.24(2)
O-P	-12918/577	-120.3	Q-R	-80/1932	0.34(3)
P-AJ	-3451/215	-120.3	R-Q	-80/1932	0.34(3)
AJ-Q	-5289/242	-120.3	AA-H	-1053/129	0

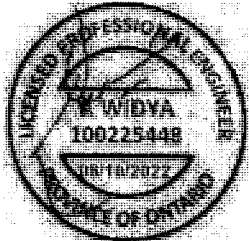
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	T30		1	TRUSS DESC.	JT 53080	E22085224(2)
Alpa Roof Truss, Maple						
Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:09:47 2022 Page 2						
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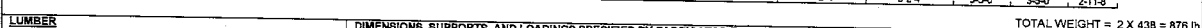
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (0-0)-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, Cp0g, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF

PLATES (table is in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TWBMW+I	MT20	10.0	12.0	3.50	
C	TWMMW+I	MT20	5.0	7.0	2.25	3.50
D, F, H, K, M	TWMMW+I					
E	TS+I	MT20	5.0	6.0		
G	TWVMW+m	MT20	8.0	9.0	4.25	3.00
I	TMW+V	MT20	3.0	8.0		
J	TS+I	MT20	5.0	6.0		
N	TWVMW+m	MT20	8.0	9.0	Edge	3.25
O	TWMMW+I	MT20	5.0	6.0	4.50	3.25
P	TMV+P	MT20	6.0	8.0	3.50	3.50
Q	TWVMW+I	MT20	3.0	8.0		
R	TWMMW+I	MT20	6.0	7.0		
S	TS+I	MT20	8.0	9.0	4.00	4.25
U	BMV+V	MT20	8.0	8.0	Edge	3.50
V	BMV+V	MT18H8	10.0	20.0	5.00	11.00
W	BMW+V	MT20	10.0	10.0		
X	BMW+V	MT20	5.0	10.0	3.00	3.00
AA	AF, AF, AI				2.75	2.00
Y	BMW+V	MT20	5.0	6.0		
Z	BS+I	ML16	5.0	17.5		
AB	BMW+V	MT20	5.0	6.0		
AC	BS+I	ML16	5.0	17.5		
AD	BMW+V	MT20	10.0	10.0		
AE	BMW+V	MT20			3.00	2.25
AG	BS+I	MT18H8	6.0	14.0		

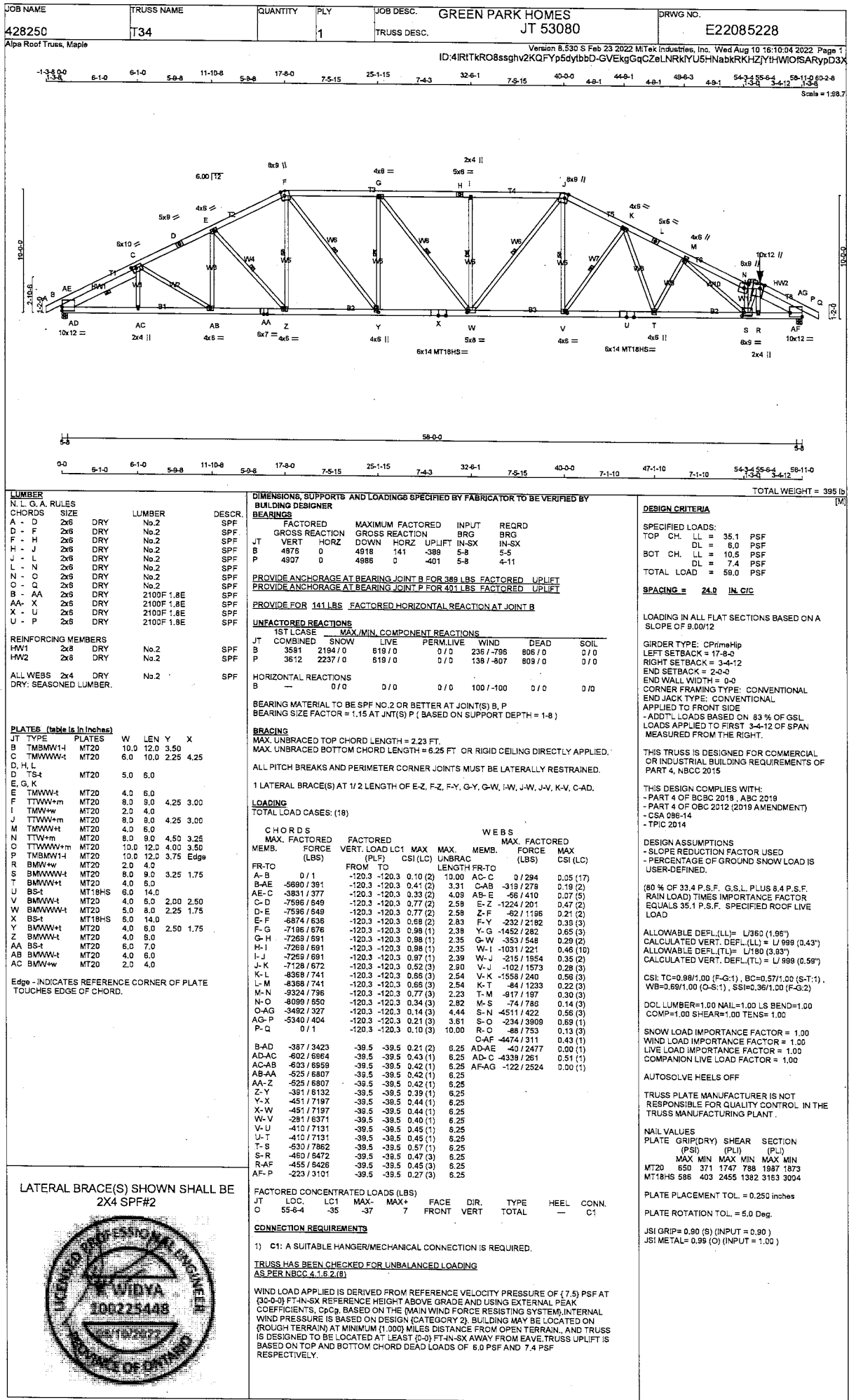
JOB NAME 428250	TRUSS NAME T32	QUANTITY	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085226(2)
Alpa Roof Truss, Maple					
Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 15:09:57 2022 Page 2 ID:4IRITkRO8ssghv2KQFYp5dytbbD-z9J4CtkpDVSN5fCa7ko7yleSACPWU8pSaQLypD3e					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (30-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, $C_p C_g$, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (8-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

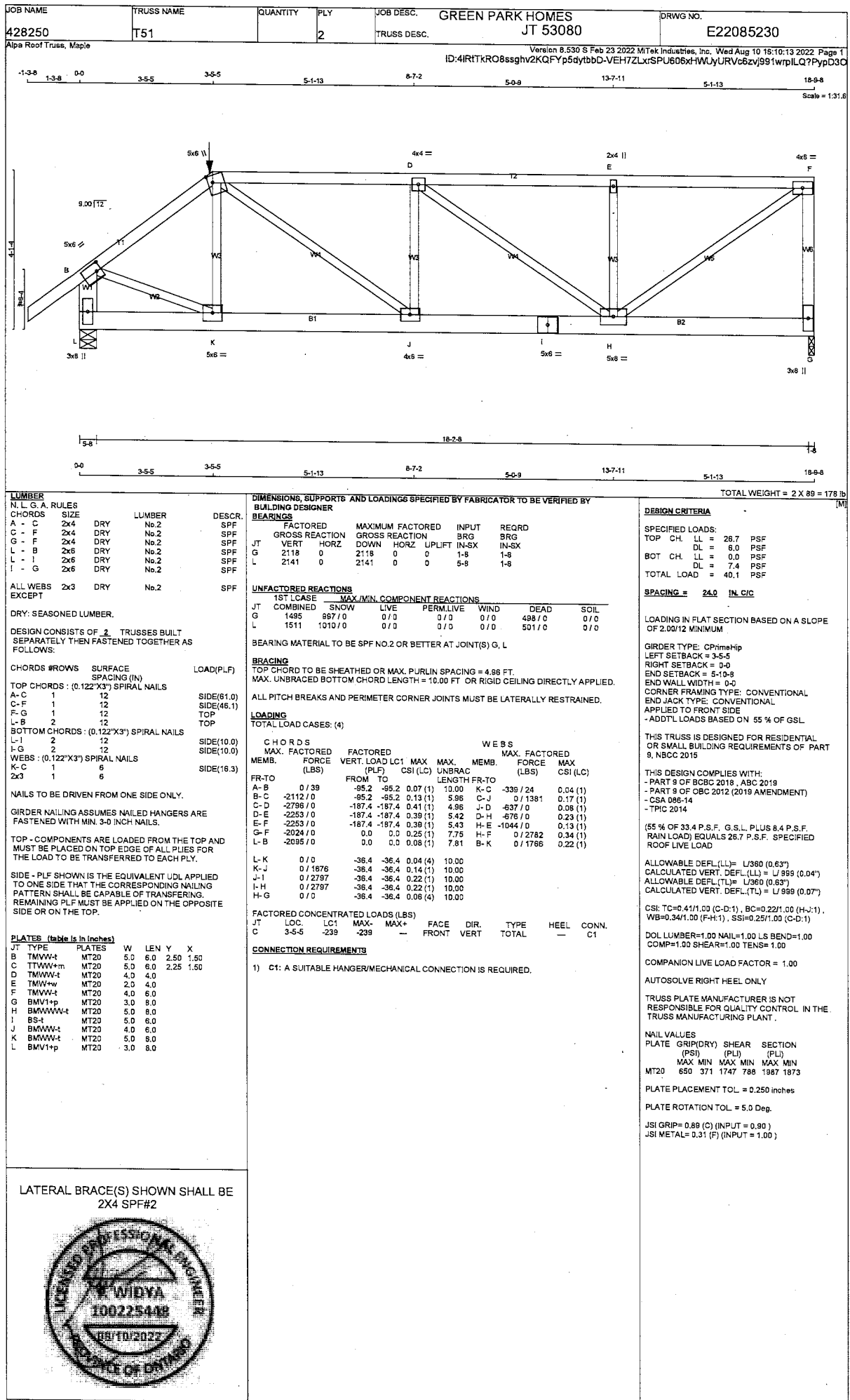
LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2

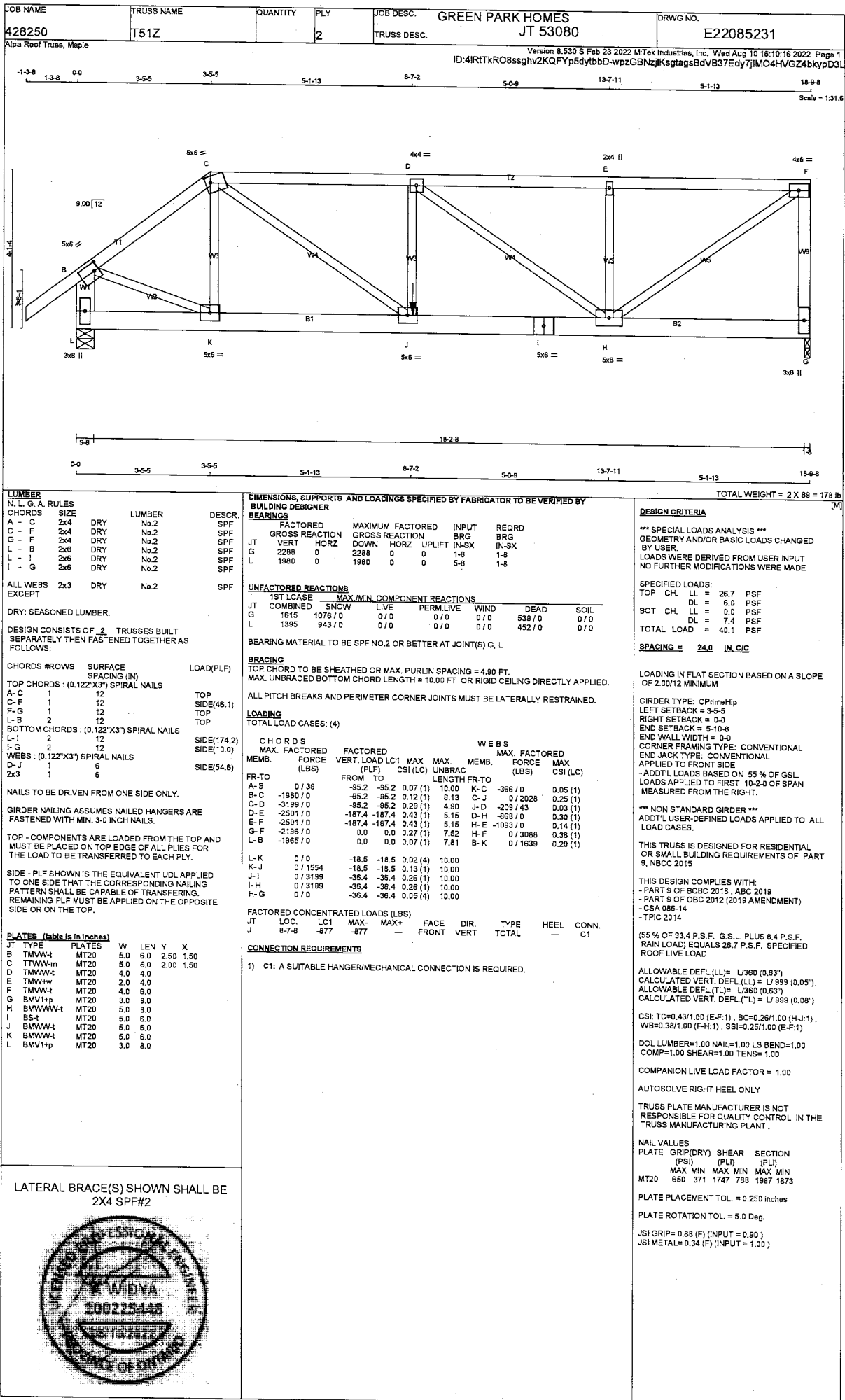


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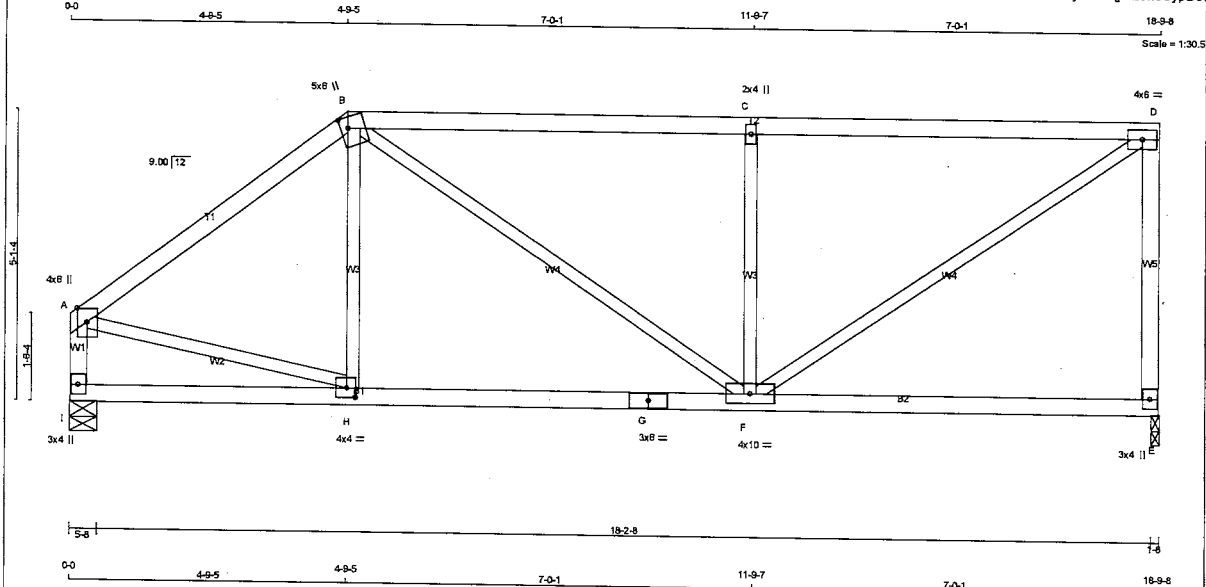


JOB NAME 428250	TRUSS NAME T52	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC. JT 53080	DRWG NO. E22085232
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Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Aug 10 16:10:19 2022 Page 1

ID:4IRITkRO8ssghv2KQFyp5dytbbD-KOeOp07c1FEfk1ORil2uhms1yKIZZlqjCEokC3ypD3



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
I - A	2x4	DRY	No.2
I - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	Edge
B	TTWW+m	MT20	5.0	6.0	2.25 1.50
C	TMVW+p	MT20	2.0	4.0	
D	TMVW+p	MT20	4.0	6.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMVWWH+p	MT20	4.0	10.0	
G	BS+p	MT20	3.0	6.0	
H	BMVW+p	MT20	4.0	4.0	2.00 1.75
I	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
E	VERT 1068	DOWN 1068	0	1-8	1-8		
I	VERT 1068	DOWN 1068	0	5-8	1-8		

UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
E	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	754	502/0	0/0	0/0	0/0	252/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1013 / 0	-95.2 -95.2	0.41 (1)	5.65	H-B	-81 / 81	0.03 (1)
B-C	-1113 / 0	-95.2 -95.2	0.89 (1)	4.26	B-F	0 / 372	0.08 (1)
C-D	-1113 / 0	-95.2 -95.2	0.89 (1)	4.26	F-C	-827 / 0	0.32 (1)
E-D	-1015 / 0	0.0 0.0	0.45 (1)	7.77	F-D	0 / 1348	0.30 (1)
I-A	-1035 / 0	0.0 0.0	0.11 (1)	7.72	A-H	0 / 833	0.19 (1)
I-H	0 / 0	-18.5 -18.5	0.13 (4)	10.00			
H-G	0 / 807	-18.5 -18.5	0.29 (4)	10.00			
G-F	0 / 807	-18.5 -18.5	0.29 (4)	10.00			
F-E	0 / 0	-18.5 -18.5	0.22 (4)	10.00			

TOTAL WEIGHT = 2 X 75 = 150 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2019, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.53")
CALCULATED VERT. DEFL. (LL) = $L/998$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.53")
CALCULATED VERT. DEFL. (TL) = $L/998$ (0.10")

CSI: TC=0.89/1.00 (C-D-1), BC=0.29/1.00 (F-H-4), WB=0.32/1.00 (C-F-1), SSI=0.32/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

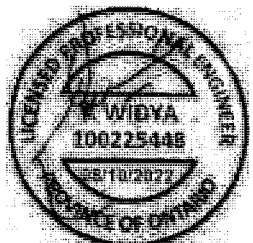
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 .650 .371 .1747 .788 .1987 .1873

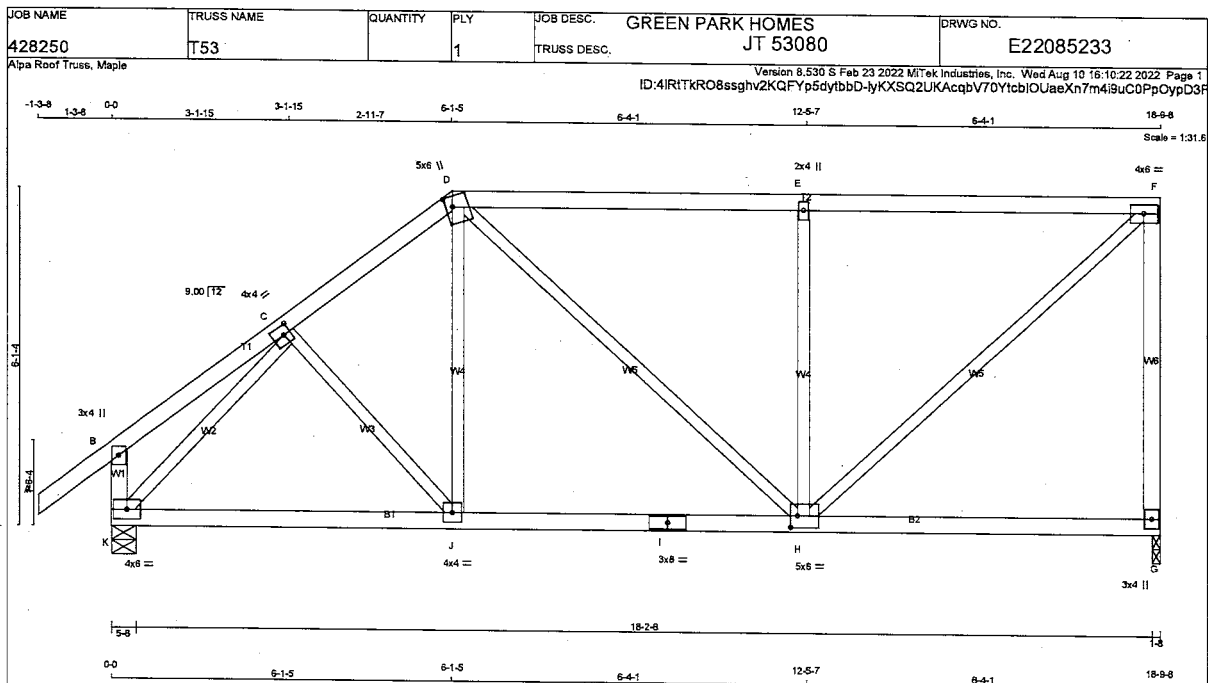
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	SPF
D - F	2x4	DRY	SPF
G - F	2x4	DRY	SPF
K - B	2x4	DRY	SPF
K - I	2x4	DRY	SPF
I - G	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	4.0	2.00	1.50
D	TMVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+1	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW+1	MT20	5.0	6.0	2.50	1.50
I	BS-1	MT20	3.0	8.0		
J	BMVW+1	MT20	4.0	4.0		
K	BMVW+1	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP
G	1068	0	1-8
K	1200	0	1-8

UNFACTORED REACTIONS

1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
			PERM
G	754	502 / 0	0 / 0
K	844	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	FACTORED	MAX
	FORCE	VERT. LOAD	LC1		FORCE	VERT. LOAD	LC1
	(LBS)	(PLF)	(LC)		(LBS)	(PLF)	(LC)
FR-TO	0 / 39	-95.2	-95.2	FR-TO	-38 / 27	0.01 (1)	
A-B	0 / 19	-95.2	-95.2	C-J	0 / 152	0.04 (4)	
B-C	-984 / 0	-95.2	-95.2	D-H	0 / 127	0.03 (1)	
C-D	-866 / 0	-95.2	-95.2	E-I	-748 / 0	0.44 (1)	
D-E	-866 / 0	-95.2	-95.2	F-J	0 / 1171	0.26 (1)	
E-F	-1019 / 0	0.0	0.0	G-K	-1207 / 0	0.41 (1)	
F-G	-245 / 0	0.0	0.0				
G-H							
H-I							
I-J							
J-K							

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.63")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")

ALLOWABLE DEFL.(TL) = $L/380$ (0.63")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI TC=0.74/1.00 (F-G-1), BC=0.23/1.00 (H-J-4), WS=0.44/1.00 (E-H-1), SS=0.28/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (PS)	(ORY)	SHEAR	SECTION	
	(PS)	(PL)	(PL)	(PL)	
MAX	MIN	MAX	MIN	MAX	
MT20	850	371	1747	788	1987

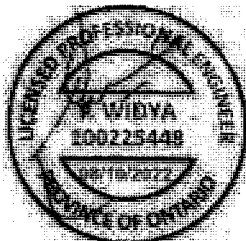
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.80)

JSI METAL= 0.40 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

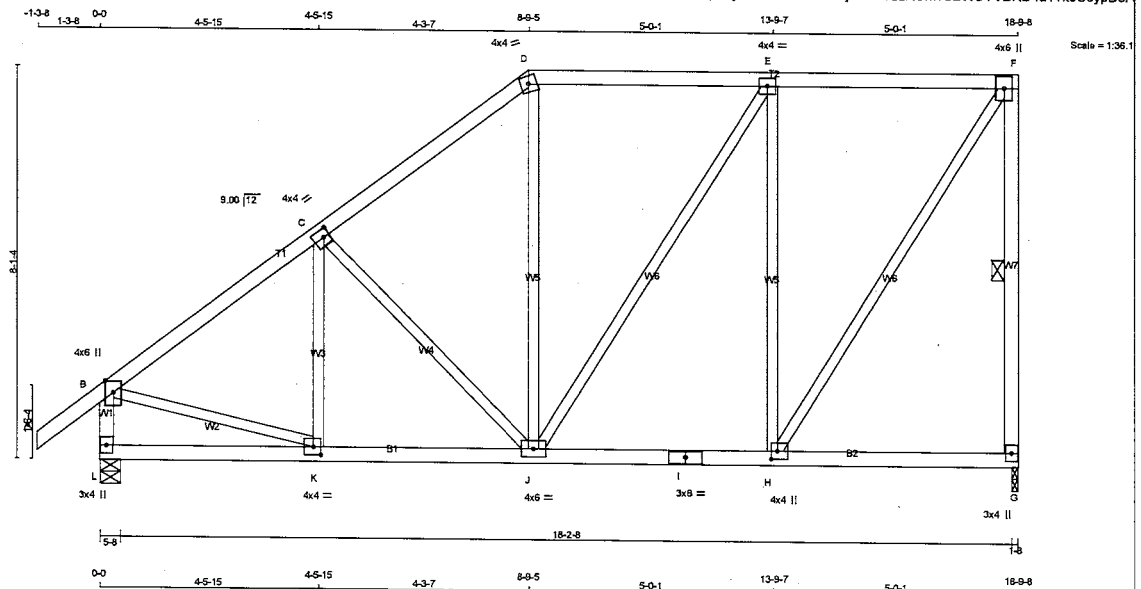


[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428250	T55		1	TRUSS DESC.	JT 53080	E22085235

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:10:27 2022 Page 1
ID:4IRITKRO8ssghv2KQFYp5dytbbD-5w7QV85d9JE7hG7zLRCm7SBWUYVBRD4u1Tk9UcypD3A



LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
O - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY, SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
B TMWW+p	MT20	4.0	8.0	Edge
C TMWW+t	MT20	4.0	4.0	2.00 1.50
D TTW-m	MT20	4.0	4.0	
E TMWW-t	MT20	4.0	4.0	
F TMWW+p	MT20	4.0	6.0	
G BMV1+p	MT20	3.0	4.0	
H BMWW+t	MT20	4.0	4.0	2.00 1.50
I BS-t	MT20	3.0	8.0	
J BMWWW-t	MT20	4.0	6.0	
K BMWW-t	MT20	4.0	4.0	2.00 1.75
L BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
G	1068	0	1068	0
L	1200	0	1200	0

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERK. LIVE
G	764	502 / 0	0 / 0
L	844	578 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	FORCE (LBS)
FR-TO		
A-B	0 / 39	
B-C	-1038 / 0	
C-D	-833 / 0	
D-E	-643 / 0	
E-F	-547 / 0	
G-F	-1030 / 0	
L-B	-1164 / 0	
L-K	0 / 0	
K-J	0 / 855	
J-I	0 / 547	
I-H	0 / 547	
H-G	0 / 0	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.63")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.41/1.00 (D-E-1), BC=0.19/1.00 (J-K-1), WB=0.94/1.00 (E-H-1), SSI=0.23/1.00 (E-F-1)

DOL LUMBER: 1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

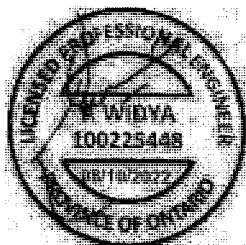
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.65 (B) (INPUT = 1.00)

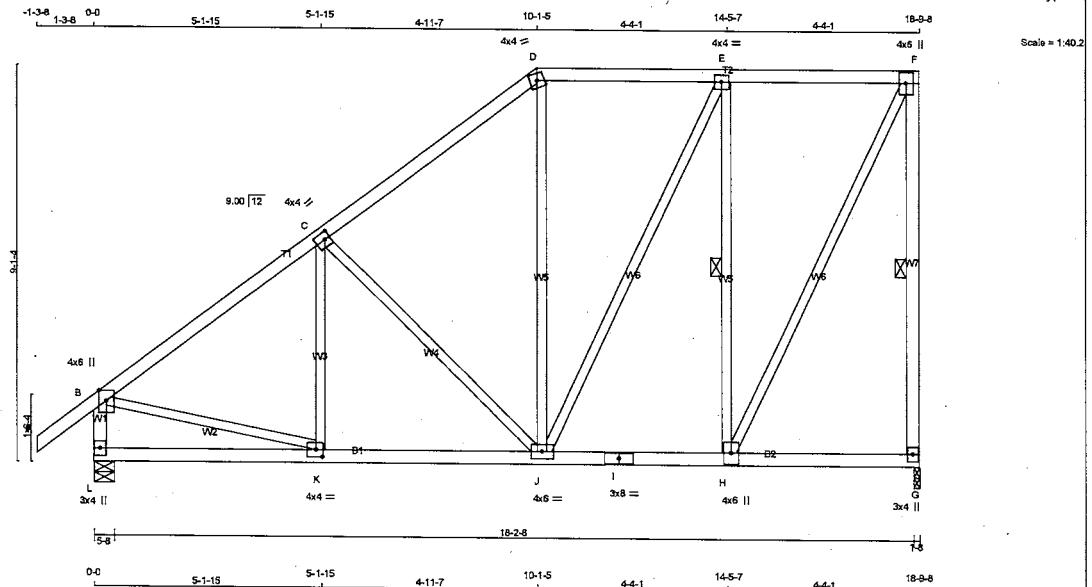
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	E22085236
428250	T56		1	TRUSS DESC.	JT 53080		

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Wed Aug 10 16:10:29 2022 Page 1
ID:4IRtKRO8ssghv2KQFYp5dytbbD-1JFAwp7thKVqxZ9MSrEE5IG7MAJvFeBVnDGZUypD38



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVV+p	MT20	4.0	8.0	Edge		
C TMWW+L	MT20	4.0	4.0	2.00	1.50	
D TTW+m	MT20	4.0	4.0			
E TMWW+L	MT20	4.0	4.0			
F TMVV+p	MT20	4.0	6.0			
G BMV1+p	MT20	3.0	4.0			
H BMWW+L	MT20	4.0	5.0			
I BS+L	MT20	3.0	8.0			
J BMWW+L	MT20	4.0	4.0	2.00	1.75	
K BMWW+L	MT20	4.0	4.0			
L BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
G	1088	0	1088	0
L	1200	0	1200	0

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
G	754	502 / 0	0 / 0
L	844	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 39	-95.2	-95.2 0.13 (1)	10.00	K-C	-91 / 57 0.04 (1)	
B-C	-1037 / 0	-95.2	-95.2 0.33 (1)	5.76	C-J	-412 / 0 0.41 (1)	
C-D	-745 / 0	-95.2	-95.2 0.32 (1)	6.25	J-D	0 / 93 0.03 (4)	
D-E	-567 / 0	-95.2	-95.2 0.23 (1)	6.25	J-E	0 / 308 0.07 (1)	
E-F	-435 / 0	-95.2	-95.2 0.22 (1)	6.25	H-E	-786 / 0 0.42 (1)	
G-F	-1035 / 0	0.0	0.0 0.40 (1)	6.18	H-F	0 / 977 0.22 (1)	
L-B	-1160 / 0	0.0	0.0 0.12 (1)	7.39	B-K	0 / 880 0.20 (1)	
L-K	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
K-J	0 / 857	-18.5	-18.5 0.20 (1)	10.00			
J-I	0 / 435	-18.5	-18.5 0.11 (4)	10.00			
I-H	0 / 435	-18.5	-18.5 0.11 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = U/999 (0.03")
ALLOWABLE DEFL.(TL) = L/380 (0.63")
CALCULATED VERT. DEFL.(TL) = U/999 (0.06")
CSI: TC=0.42/1.00 (F-G-1), BC=0.20/1.00 (J-K-1), WB=0.42/1.00 (E-H-1), SS=0.20/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

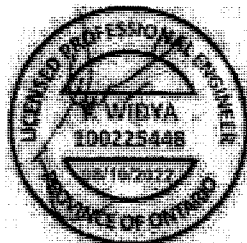
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (P.LI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

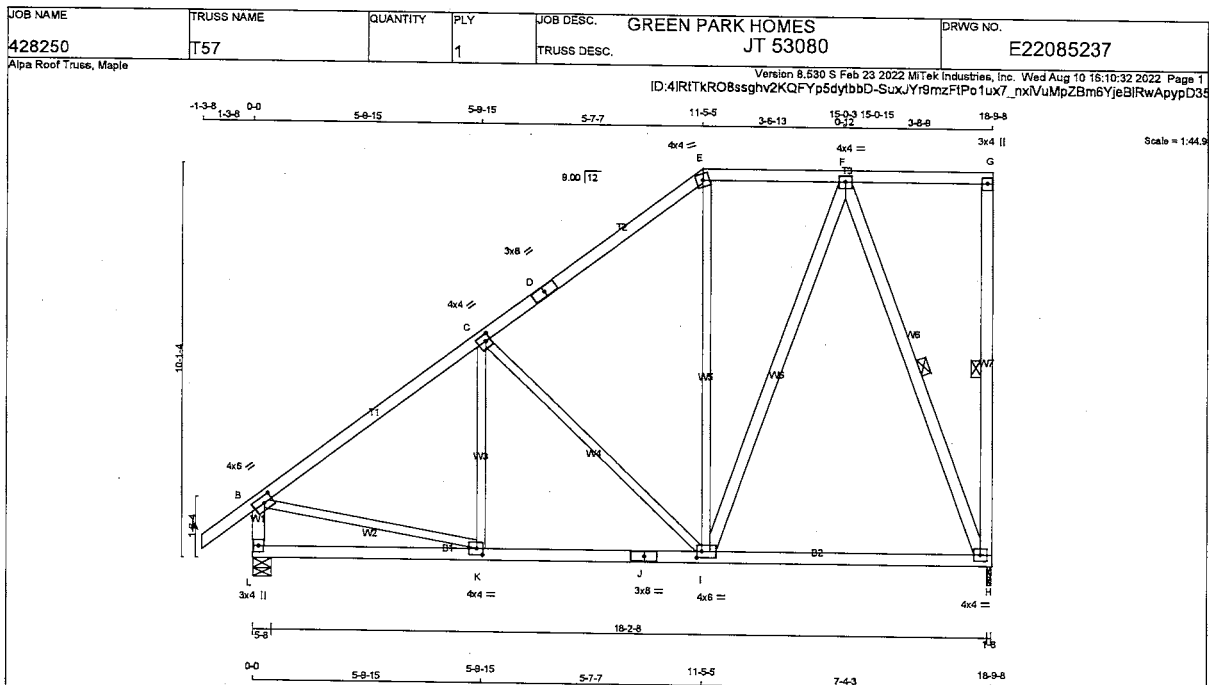
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER

N L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
I - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-4	MT20	4.0	8.0	2.00	2.75
C TMWV-4	MT20	4.0	4.0	2.00	1.50
D TS-4	MT20	3.0	8.0		
E TTW-m	MT20	4.0	4.0		
F TMWV-4	MT20	4.0	4.0		
G TMV-p	MT20	3.0	4.0		
H BMWV-4	MT20	4.0	4.0		
I BMWV-4	MT20	4.0	6.0	2.00	1.50
J BS-4	MT20	3.0	8.0		
K BMWV-4	MT20	4.0	4.0	2.00	1.75
L BMV-4	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT GROSS REACTION	GROSS REACTION	BRG	BRG
H 1113 0	DOWN	HORZ	UPLIFT
L 1210 0	DOWN	HORZ	UPLIFT

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM	WIND	DEAD	SOIL
JT COMBINED	790	502/0	0/0	0/0	287/0	0/0
L 853	575/0	0/0	0/0	0/0	277/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/39	-95.2	-95.2 0.13 (1)	10.00	K-C	-49/53	0.04 (1)
B-C	-1036/0	-95.2	-95.2 0.43 (1)	5.80	C-I	-495/0	0.68 (1)
C-D	-673/0	-95.2	-95.2 0.41 (1)	6.25	I-E	0/67	0.02 (4)
D-E	-673/0	-95.2	-95.2 0.41 (1)	6.25	I-F	0/474	0.08 (1)
E-F	-905/0	-102.7	-102.7 0.18 (1)	2.00	F-H	-976/0	0.56 (1)
F-G	0/0	-102.7	-102.7 0.23 (1)	10.00	B-K	0/878	0.20 (1)
G-H	-145/0	0.0	0.0 0.07 (1)	6.25			
L-B	-1184/0	0.0	0.0 0.12 (1)	7.38			
L-K	0/0	-18.5	-18.5 0.13 (4)	10.00			
K-J	0/960	-18.5	-18.5 0.28 (4)	10.00			
J-I	0/960	-18.5	-18.5 0.28 (4)	10.00			
I-H	0/345	-18.5	-18.5 0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION (LL) = $L/999$ (0.02")

CALCULATED VERT. DEF. (LL) = $L/999$ (0.02")

ALLOWABLE DEFLECTION (TL) = $L/960$ (0.63")

CALCULATED VERT. DEF. (TL) = $L/999$ (0.09")

CSI: TC=0.43/1.00 (B-C-1), BC=0.28/1.00 (I-K-4), WB=0.59/1.00 (C-I-1), SSI=0.21/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

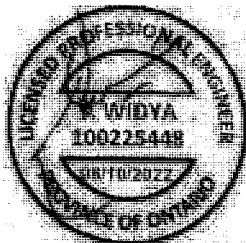
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.28 (K) (INPUT = 1.00)

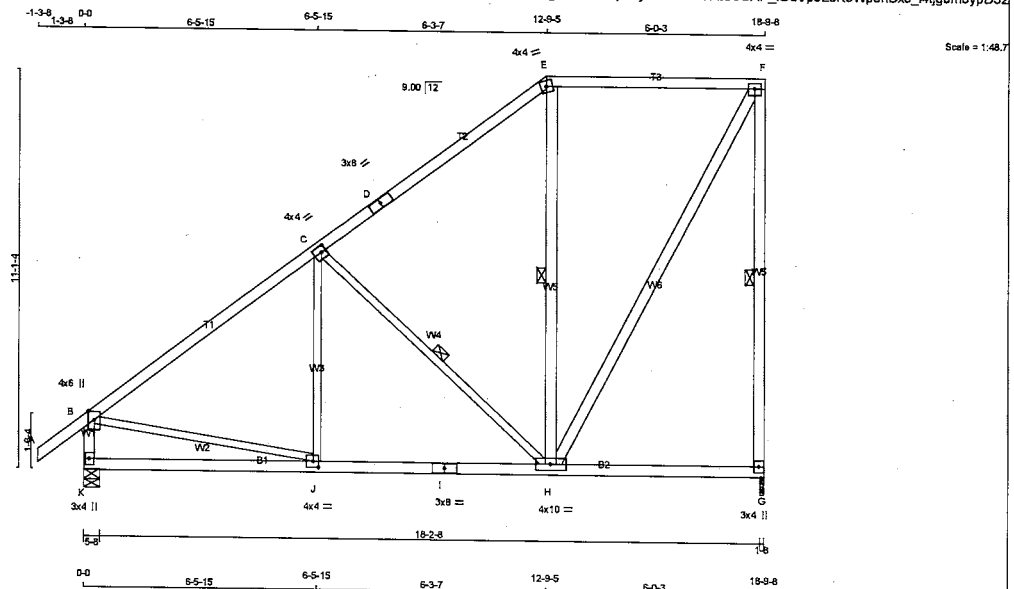
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME T58	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085238
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Alpa Roof Truss, Maple

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ID:4IRtKRO8ssghv2KQFYp5dytbbD-sTcSAteGAF_fJdVp6LeK8WpenDxJ_4figbm8ypD32



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2		SPF
D - E	2x4	DRY	No.2		SPF
E - F	2x4	DRY	No.2		SPF
G - F	2x4	DRY	No.2		SPF
K - B	2x4	DRY	No.2		SPF
K - I	2x4	DRY	No.2		SPF
I - G	2x4	DRY	No.2		SPF

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	4.0	6.0	Edge		
C TMWW-t	MT20	4.0	4.0	2.00	1.50	
D TS-4	MT20	3.0	8.0			
E TTW-m	MT20	4.0	4.0			
F TMW-t	MT20	4.0	4.0			
G BMV1+p	MT20	3.0	4.0			
H BMWW-t	MT20	4.0	10.0			
I BS-t	MT20	3.0	8.0			
J BMWW-t	MT20	4.0	4.0	2.00	1.75	
K BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
G	VERT	HORZ	DOWN	HORZ
K	1200	0	1088	0

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
G	754	502 / 0	0 / 0
K	844	578 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	WEBS
MEMB.	MAX. FACTORED	MAX. FACTORED
FR-TO	FORCE (LBS)	FORCE (LBS)
A-B	0 / 39	0 / 39
B-C	-1005 / 0	-1005 / 0
C-D	-552 / 0	-552 / 0
D-E	-552 / 0	-552 / 0
E-F	-401 / 0	-401 / 0
F-G	-1025 / 0	-1025 / 0
K-B	-1151 / 0	-1151 / 0
K-J	0 / 0	0 / 0
J-I	0 / 840	0 / 840
I-H	0 / 840	0 / 840
H-G	0 / 0	0 / 0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 089-14
- TPI-C 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.65/1.00 (F-G:1), BC=0.25/1.00 (H-J:4), WB=0.32/1.00 (C-H:1), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

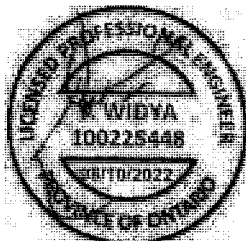
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.96 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES JT 53080	DRWG NO.
428250	T60	1		TRUSS DESC.		E22085239

Alpa Roof Truss, Maple

ID:4IRITkROBssghv2KQFYp5dytbbD-orkCbYDuonViuomuwXNBPZCfTawwnxVNK19hrDypD30

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Scale = 1:21.3

Detailed description of the truss diagram: The diagram shows a symmetrical roof truss. The main span is 31 feet wide at the base. The peak height is 14 feet. The truss is composed of several members labeled T1, T2, W1, W2, W3, B1, B2, B3, A, B, E, F, G, H, I, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z. Connections are shown with bolts and gusset plates. Dimensions are given in feet and inches. A scale of 1:21.3 is indicated.

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4 DRY	No.2	SPF			
C - D	2x4 DRY	No.2	SPF			
D - E	2x4 DRY	No.2	SPF			
E - F	2x4 DRY	No.2	SPF			
F - G	2x4 DRY	No.2	SPF			
G - H	2x4 DRY	No.2	SPF			
H - I	2x4 DRY	No.2	SPF			
I - J	2x4 DRY	No.2	SPF			
J - K	2x4 DRY	No.2	SPF			
K - L	2x4 DRY	No.2	SPF			
L - M	2x4 DRY	No.2	SPF			
M - N	2x4 DRY	No.2	SPF			
N - O	2x4 DRY	No.2	SPF			
O - P	2x4 DRY	No.2	SPF			
P - Q	2x4 DRY	No.2	SPF			
Q - R	2x4 DRY	No.2	SPF			
R - S	2x4 DRY	No.2	SPF			
S - T	2x4 DRY	No.2	SPF			
T - U	2x4 DRY	No.2	SPF			
U - V	2x4 DRY	No.2	SPF			
V - W	2x4 DRY	No.2	SPF			
W - X	2x4 DRY	No.2	SPF			
X - Y	2x4 DRY	No.2	SPF			
Y - Z	2x4 DRY	No.2	SPF			

ALL WEBS	EXCEPT	DRY: SEASONED LUMBER.
2x3	DRY	No.2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQ'D									
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	BRG	IN-SX	
I	814	0	814	0	0	5-8	1-8		
G	814	0	814	0	0	5-8	1-8		

UNFACTORED REACTIONS									
1ST LOOSE MAX/MIN COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL	
I	573	392 / 0	0 / 0	0 / 0	0 / 0	180 / 0	180 / 0	0 / 0	
G	573	392 / 0	0 / 0	0 / 0	0 / 0	180 / 0	180 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO					FR-TO				
A-B	0 / 39	-95.2	-95.2	0.14 (1)	10.00	B-H	0 / 454	0.11 (1)	
B-C	-546 / 0	-95.2	-95.2	0.29 (1)	8.25	H-E	0 / 454	0.11 (1)	
C-D	-433 / 0	-139.8	-139.8	0.01 (1)	8.25	C-H	-53 / 40	0.02 (1)	
D-E	-546 / 0	-95.2	-95.2	0.29 (1)	8.25	H-D	-53 / 40	0.02 (1)	
E-F	0 / 39	-95.2	-95.2	0.14 (1)	10.00				
F-G	-789 / 0	0.0	0.0	0.09 (1)	7.81				
G-E	-789 / 0	0.0	0.0	0.09 (1)	7.81				
I-J	0 / 0	-27.2	-27.2	0.15 (4)	10.00				
J-H	0 / 0	-27.2	-27.2	0.15 (4)	10.00				
H-K	0 / 0	-27.2	-27.2	0.15 (4)	10.00				
K-G	0 / 0	-27.2	-27.2	0.15 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11+1	-148	-148	—	FRONT	VERT	TOTAL	—	C1
D	4-6-15	-148	-148	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPPrimeHip

SIDE SETBACK = 3-11+1

END SETBACK = 3-10-8

END WALL WIDTH = 0-0

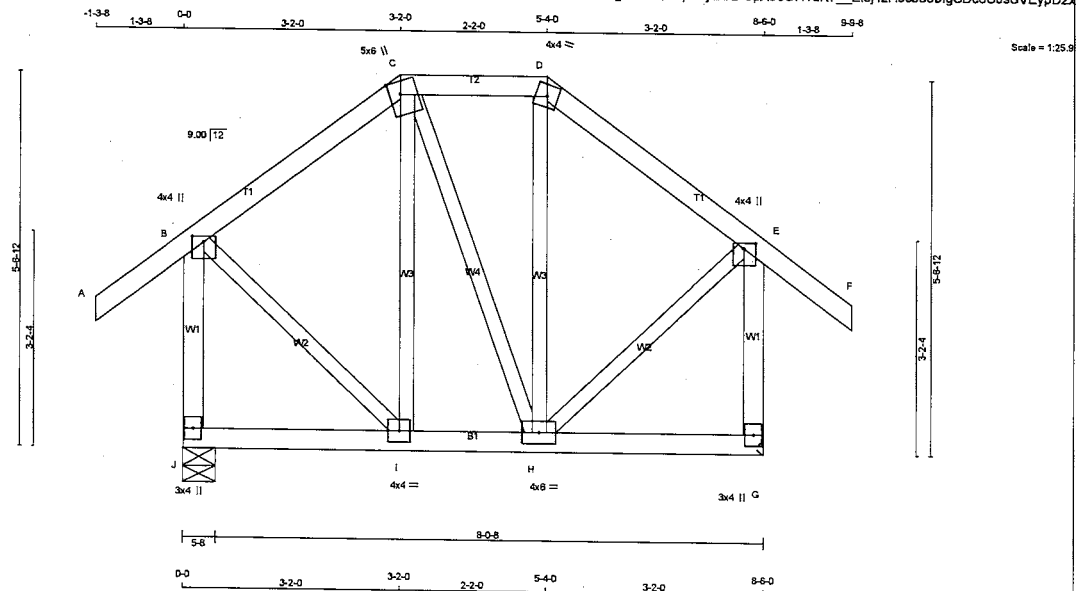
CORNER FR

[illegible]

JOB NAME 428250	TRUSS NAME T62	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC. JT 53080	DRWG NO. E22085241
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Alpa Roof Truss, Maple

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ID:4IRTKRO8ssghv2KQFyp5dytbbD-9pX5eGH1dK7_Zfsj4zH6cJ83bfgSDc6UJsSVeypD2x



TOTAL WEIGHT = 51 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2		
C - D	2x4 DRY	No.2		
D - F	2x4 DRY	No.2		
J - B	2x4 DRY	No.2		
G - E	2x4 DRY	No.2		
J - G	2x4 DRY	No.2		
ALL WEBS	2x3 DRY	No.2		
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMW+p MT20	4.0	4.0	1.00	2.00
C TTWW+m MT20	5.0	6.0	Edge	2.00
D TTW-m MT20	4.0	4.0		
E TMW+p MT20	4.0	4.0	1.00	2.00
G BMV1+p MT20	3.0	4.0		
H BMWWH+ MT20	4.0	6.0		
I BMWW+ MT20	4.0	4.0		
J BMV1+p MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	615	0	0	0
J HORZ	0	615	0	0
G VERT	615	0	0	0
G HORZ	0	615	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS
	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT	431 301/0 0/0 0/0 0/0 130/0 0/0
G	431 301/0 0/0 0/0 0/0 130/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (CSI) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (PL)	MAX. FACTORED (LC)
FR-TO		FROM TO	LENGTH FR-TO				
A-B	0/39	-95.2 -95.2	0.13 (1)	10.00	I-C	-110/3	0.05 (1)
B-C	-238/0	-95.2 -95.2	0.12 (1)	6.25	C-H	-2/0	0.00 (1)
C-D	-185/0	-95.2 -95.2	0.08 (1)	6.25	H-D	-112/3	0.05 (1)
D-E	-235/0	-95.2 -95.2	0.12 (1)	6.25	B-I	0/250	0.06 (1)
E-F	0/39	-95.2 -95.2	0.13 (1)	10.00	H-E	0/249	0.06 (1)
J-B	-589/0	0.0 0.0	0.10 (1)	7.81			
G-E	-587/0	0.0 0.0	0.10 (1)	7.81			
J-I	0/0	-18.5 -18.5	0.04 (4)	10.00			
I-H	0/186	-18.5 -18.5	0.05 (4)	10.00			
H-G	0/0	-18.5 -18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2018 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B:1), BC=0.05/1.00 (H-I:4), WB=0.06/1.00 (B-I:1), SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

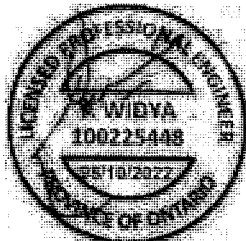
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

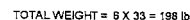
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

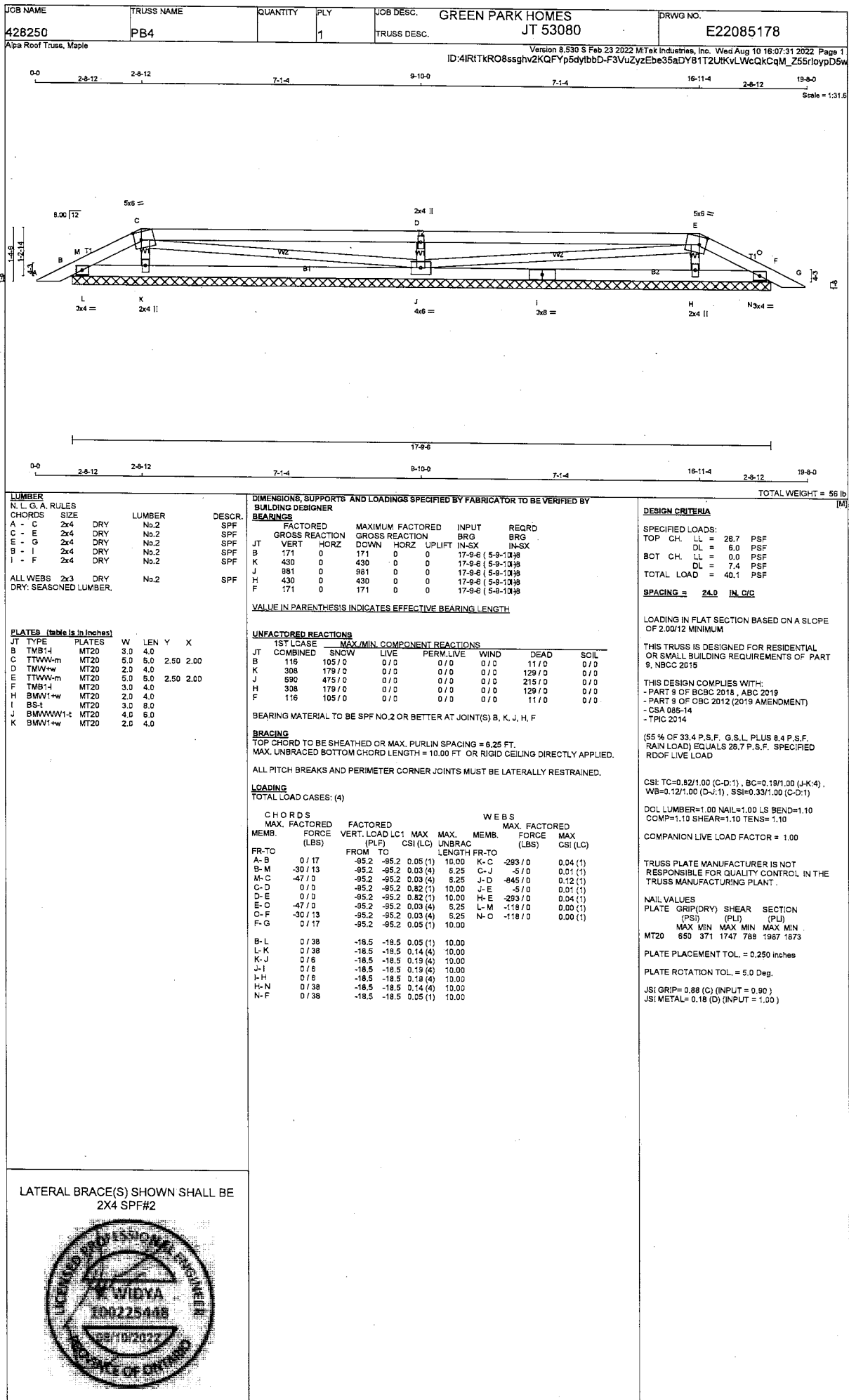
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

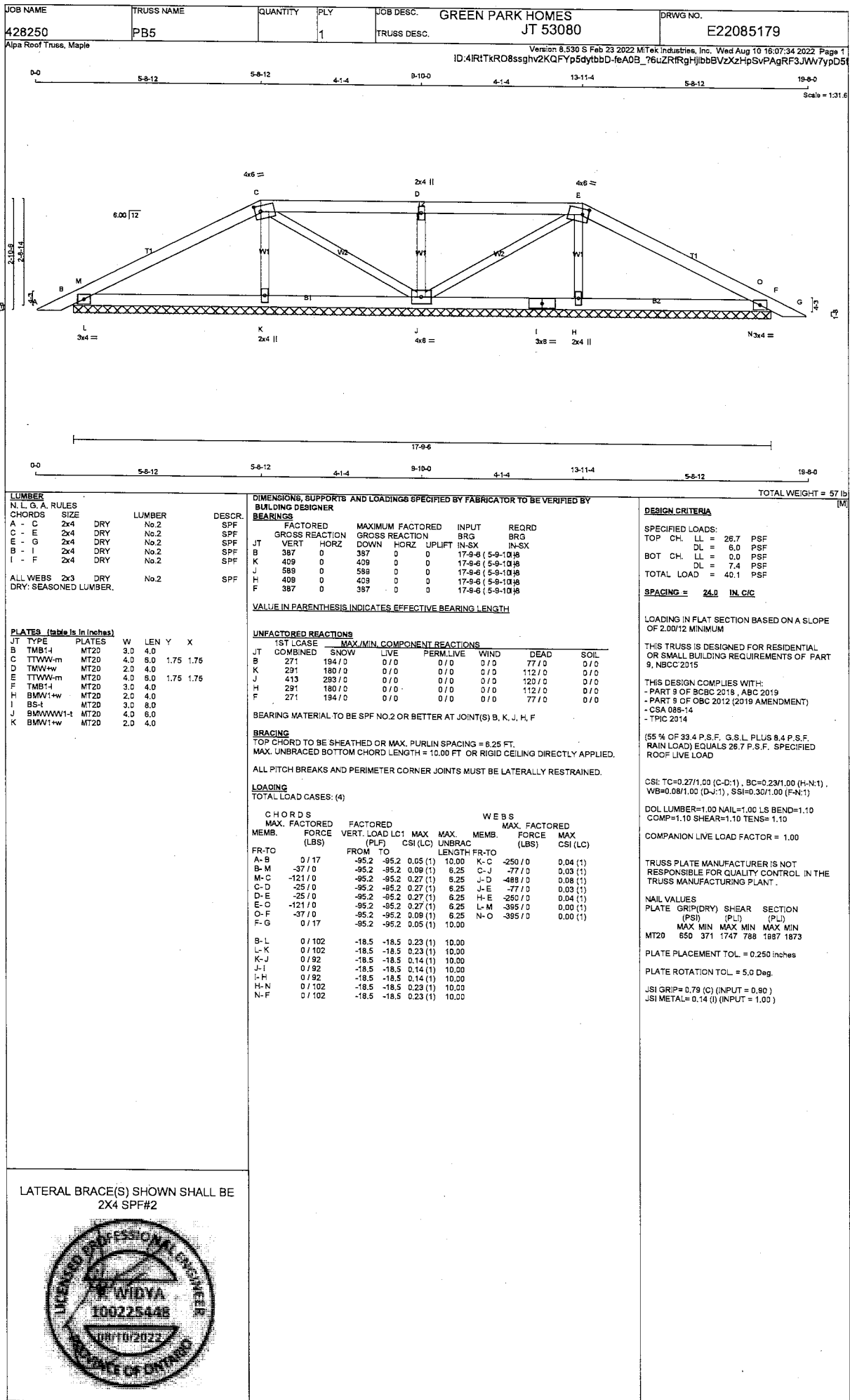


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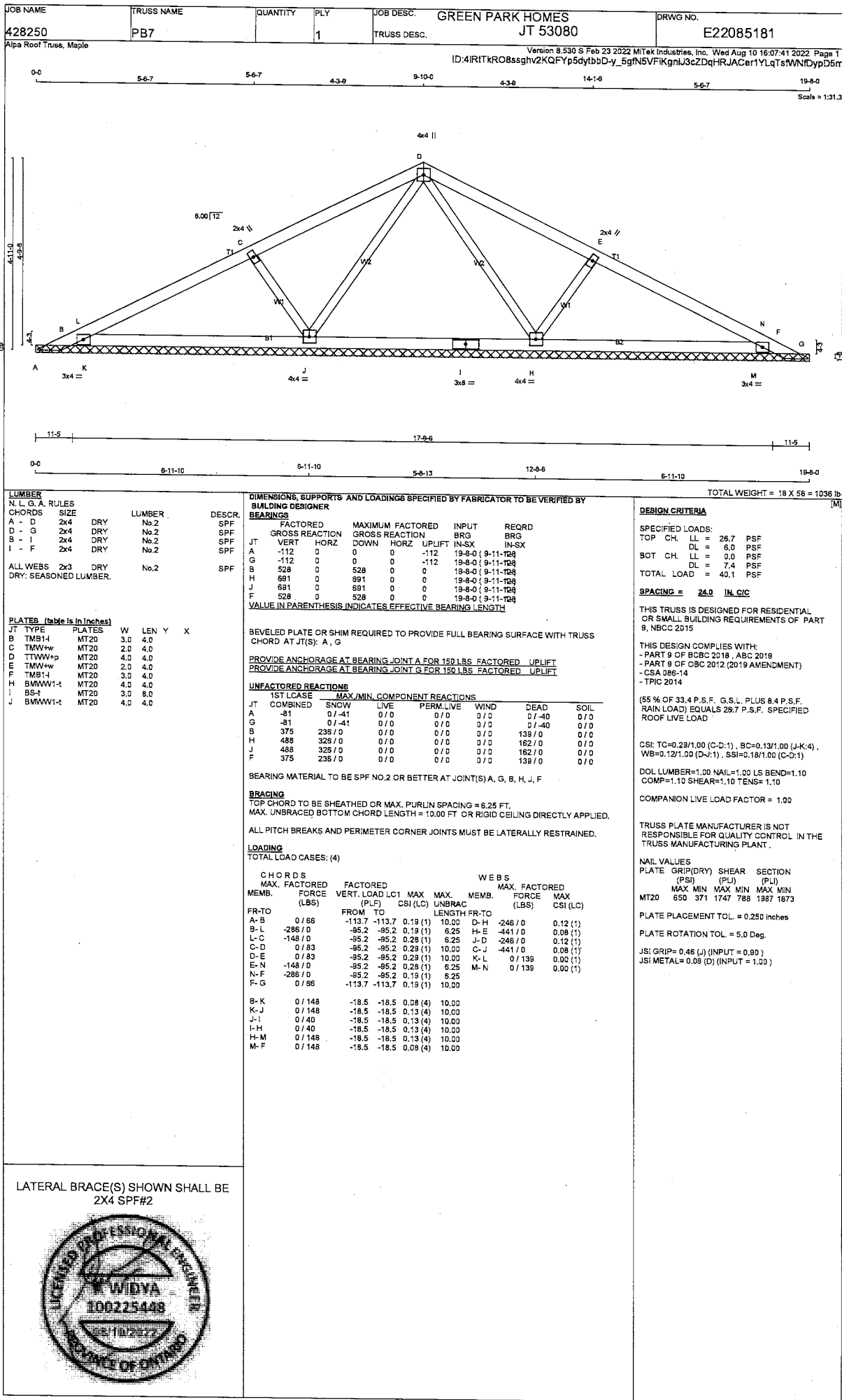


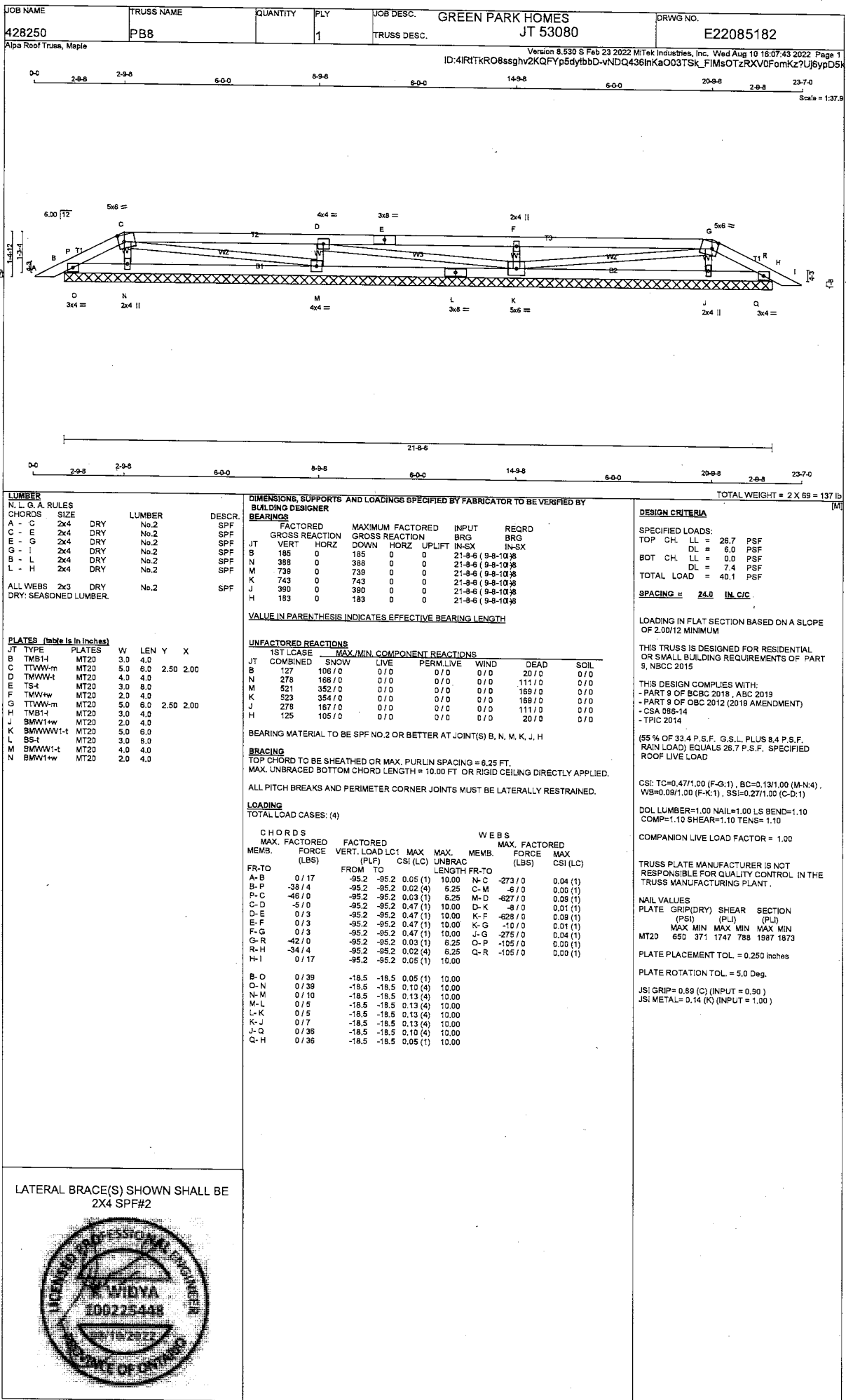
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JSI METAL= 0.10 (D) (INPUT = 1.00 )
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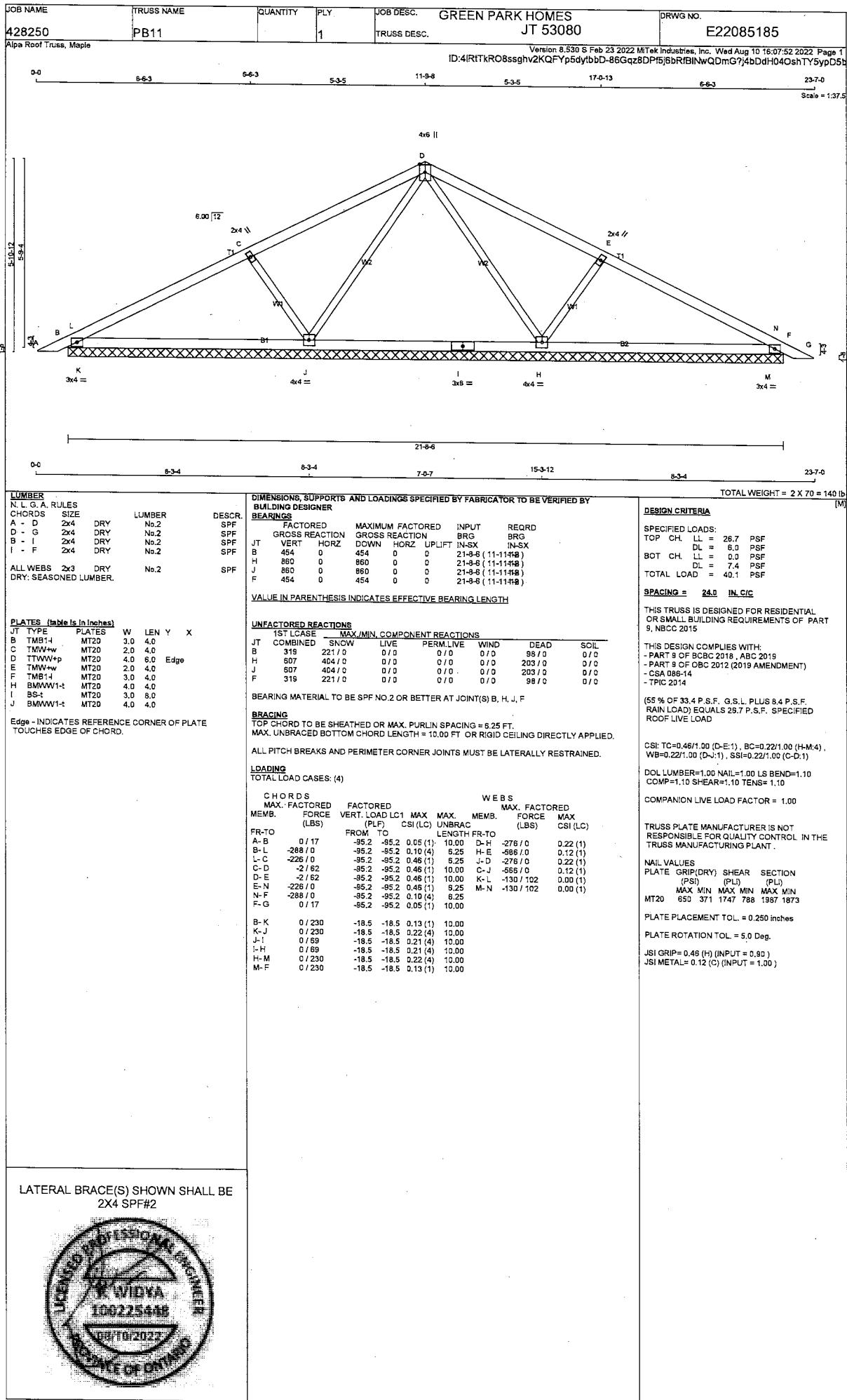


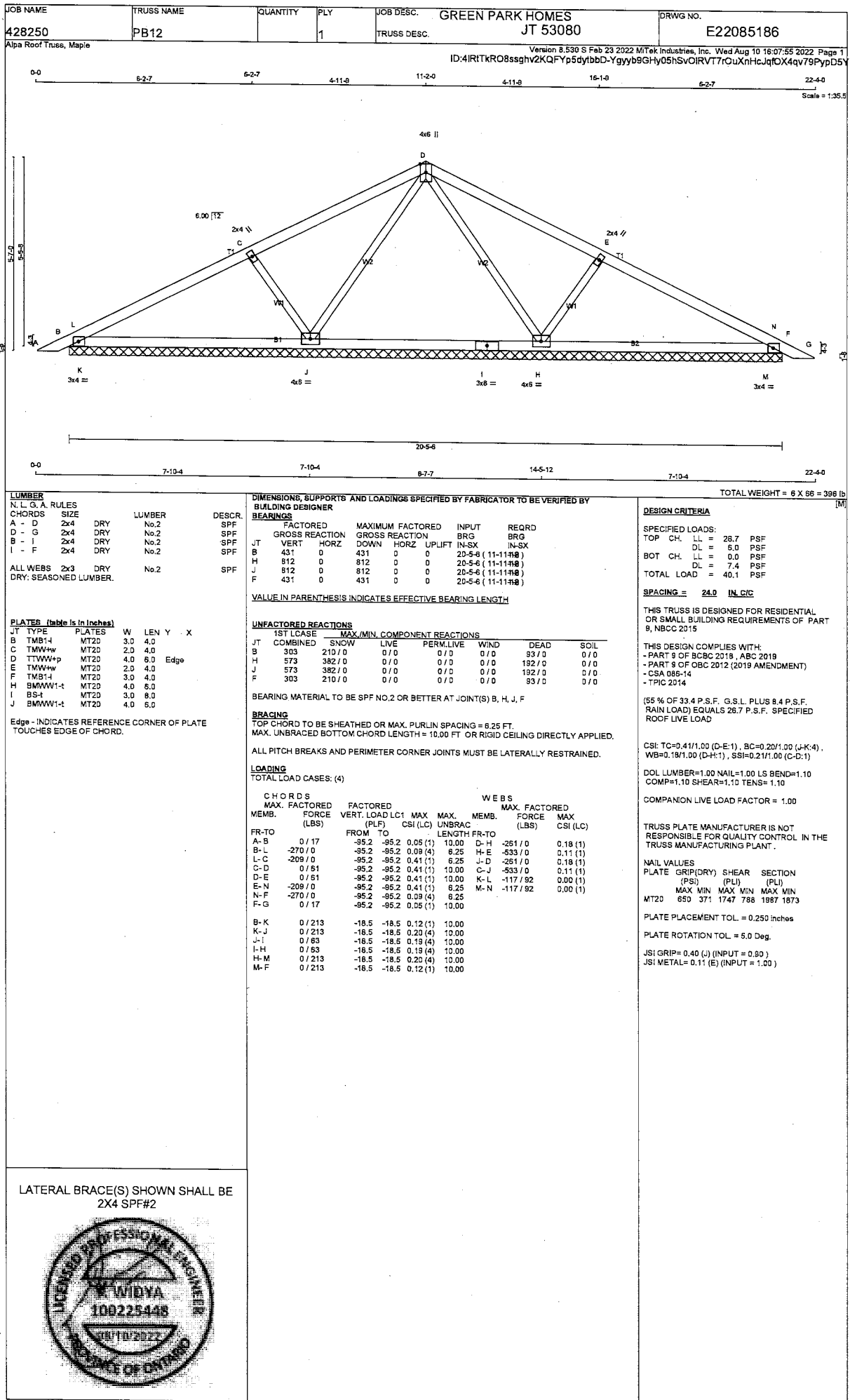
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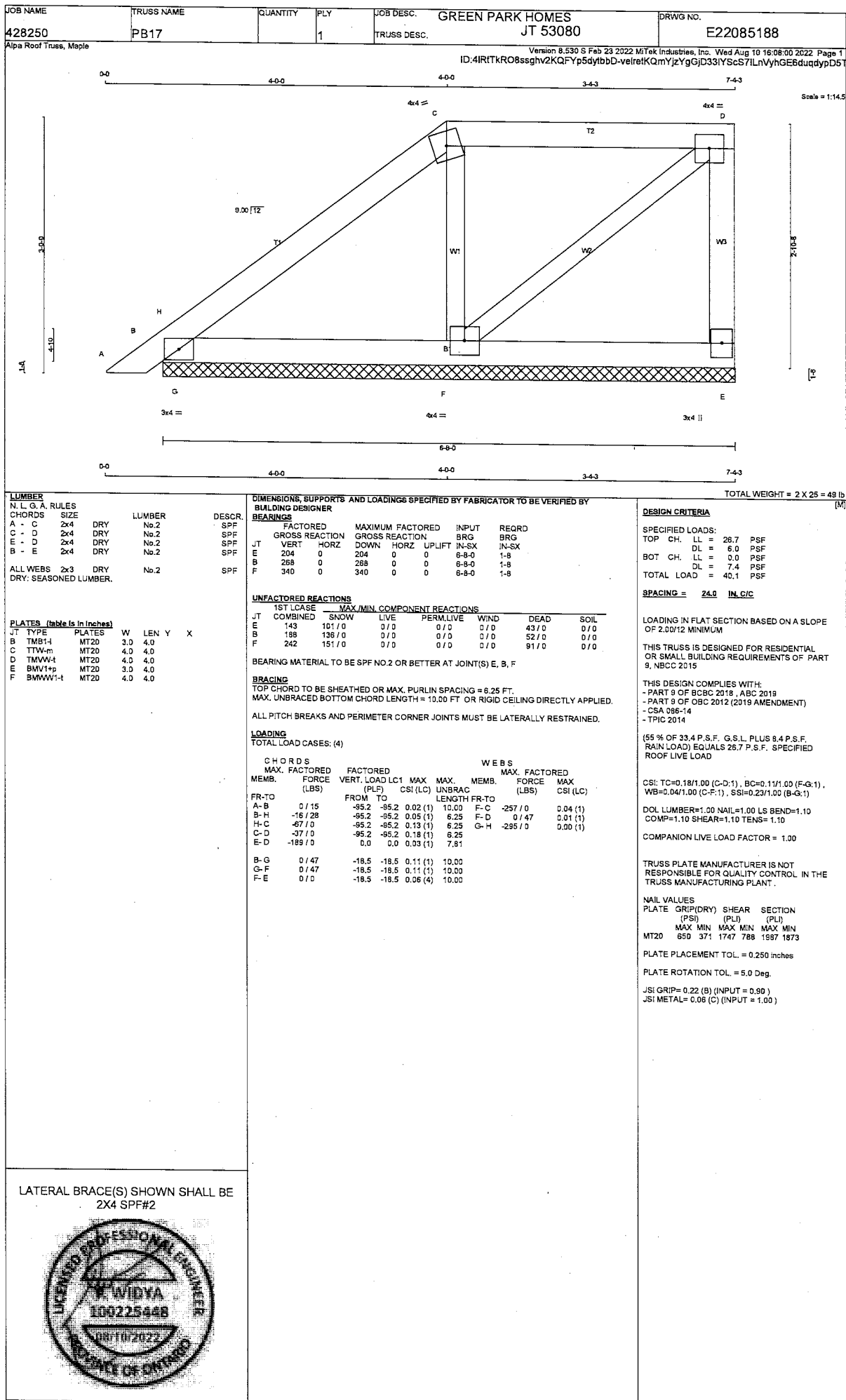




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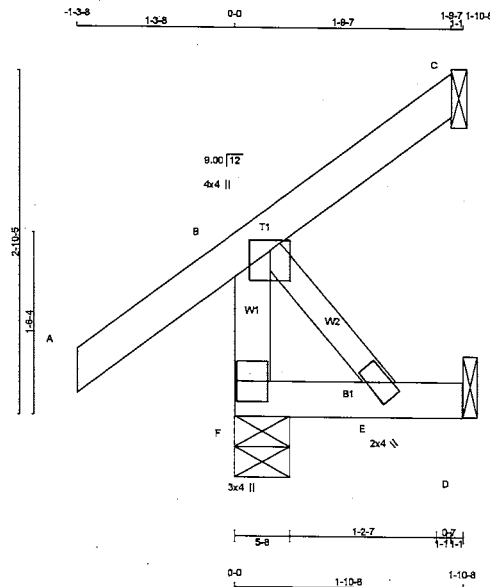


[illegible]

[illegible]

Alpa Roof Truss, Maple

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ID:4IRITKRO8ssghv2KQFYp5dytbbD-i8KH-7eXe4aXCfHoDwvO2QkPLL5B2mZhhHVny2uypD6L



Scale = 1:14.5

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	4.0	1.00 2.00
E	BMV+p	MT20	2.0	4.0	
F	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
F	284	0	284	0	0	5-8	1-8		
C	34	0	34	0	-41	1-8	1-8		
D	17	0	19	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	137	150/0	0/0	0/0	0/0	48/0	0/0
C	24	19/-28	0/0	0/0	0/0	4/0	0/0
D	14	0/0	0/0	0/0	0/0	14/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	LC1 (LBS)
F-B	-287/0	0.0	0.0 0.03 (1)	7.81	0/0	0.00 (1)	
A-B	0/39	-95.2	-95.2 0.13 (1)	10.00			
B-C	-30/0	-95.2	-95.2 0.12 (1)	6.25			
F-E	0/0	-18.5	-18.5 0.02 (4)	10.00			
E-D	0/0	-18.5	-18.5 0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPI/C 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

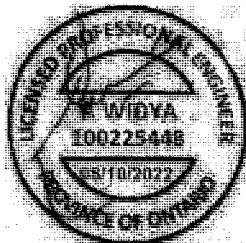
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	SECTION (PL)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

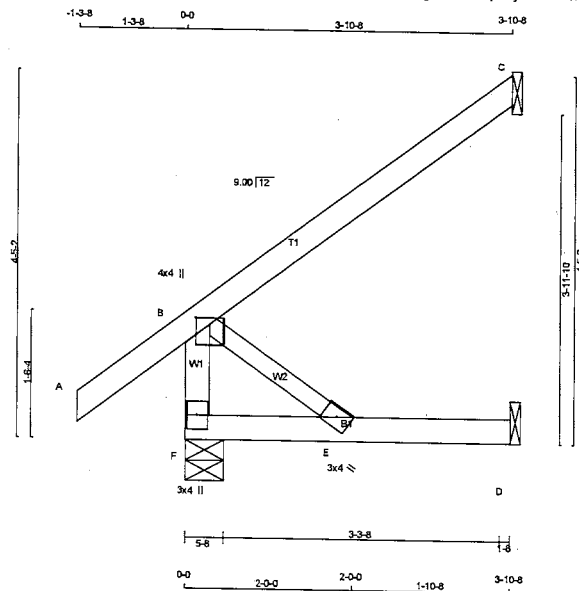
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME J8	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC. JT 53080	DRWG NO. E22085163
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Alpa Roof Truss, Maple

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ID:4IRtKRO8sgghv2KQFYp5dytbbD-FWR1QKZvcBnV_yc1KQWV9Vg3vrYESn_lps36nypD6S



Scale = 1:21.0

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	SPF
A - C	2x4	DRY	SPF
F - D	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
DRY, SEASONED LUMBER.		No.2	

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00 2.00
E	BMV+p	MT20	3.0	4.0	
F	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRO
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
F	352	352	0	0	0
C	184	184	0	0	0
D	36	40	0	0	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
F	246	177/0	0/0	0/0	0/0
C	127	104/0	0/0	0/0	0/0
D	29	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
	FORCE (LBS)		FORCE (LBS)
FR-TO		FR-TO	
F-B	-315/0	0.0	0.0
A-B	0/39	-95.2	-95.2
B-C	0/0	-95.2	-95.2
F-E	0/0	-18.5	-18.5
E-D	0/0	-18.5	-18.5

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.24/1.00 (B-C-1), BC=0.08/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.12/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

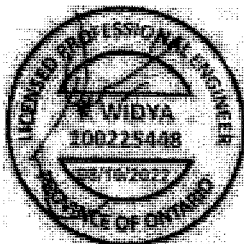
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	550	371	1747

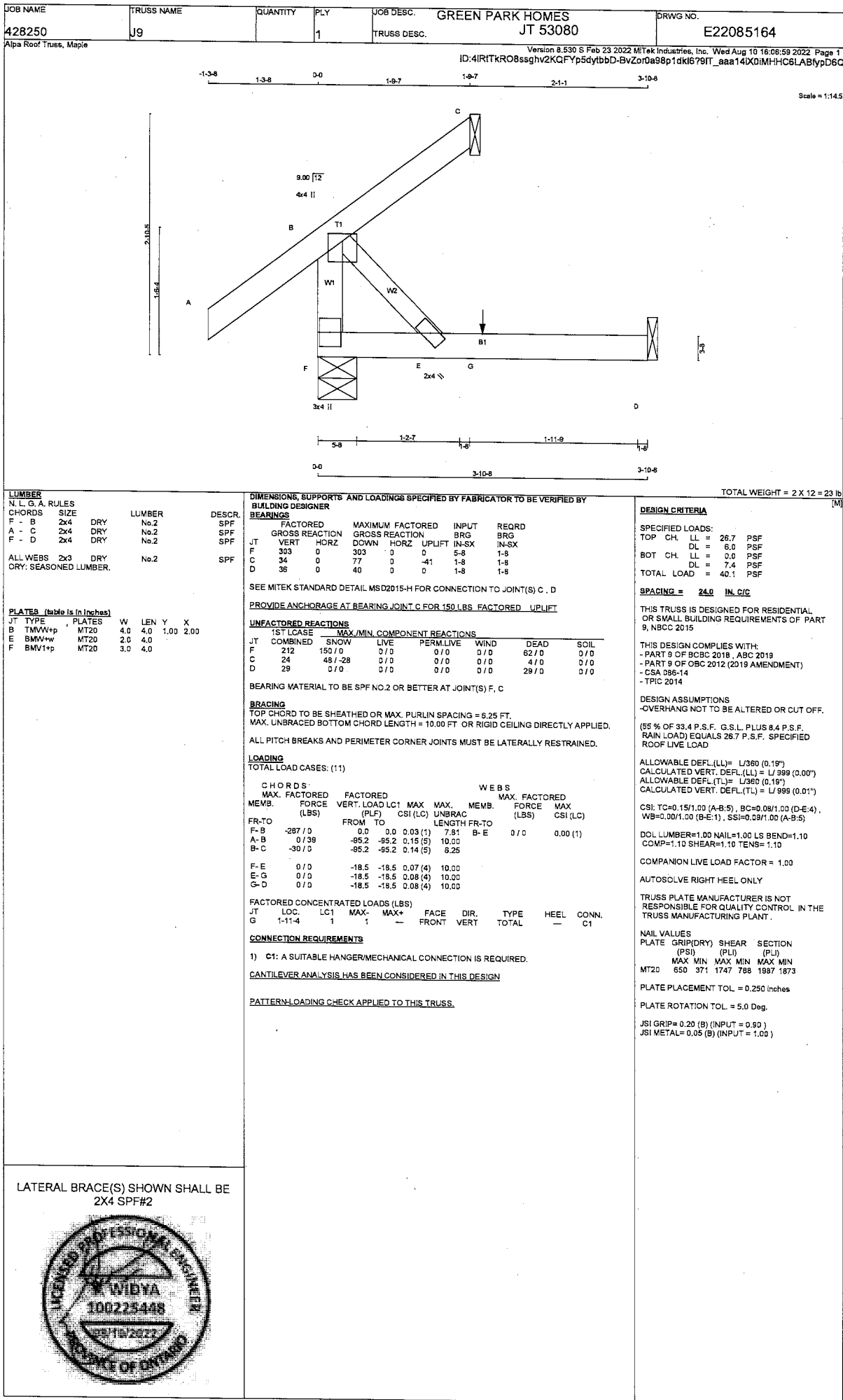
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.80)
JSI METAL = 0.08 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





JOB NAME 428250	TRUSS NAME J10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085165
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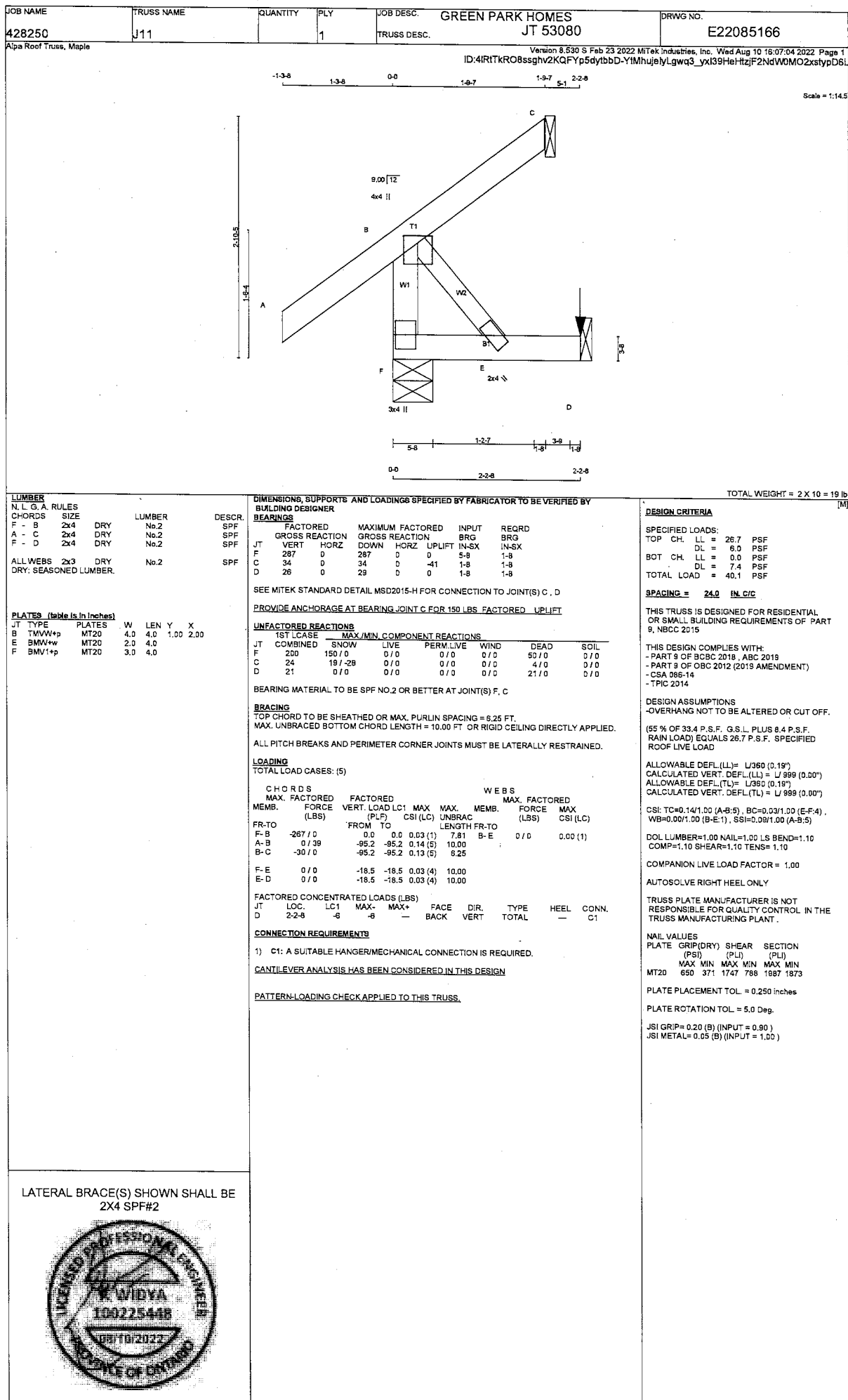
Alpa Roof Truss, Maple

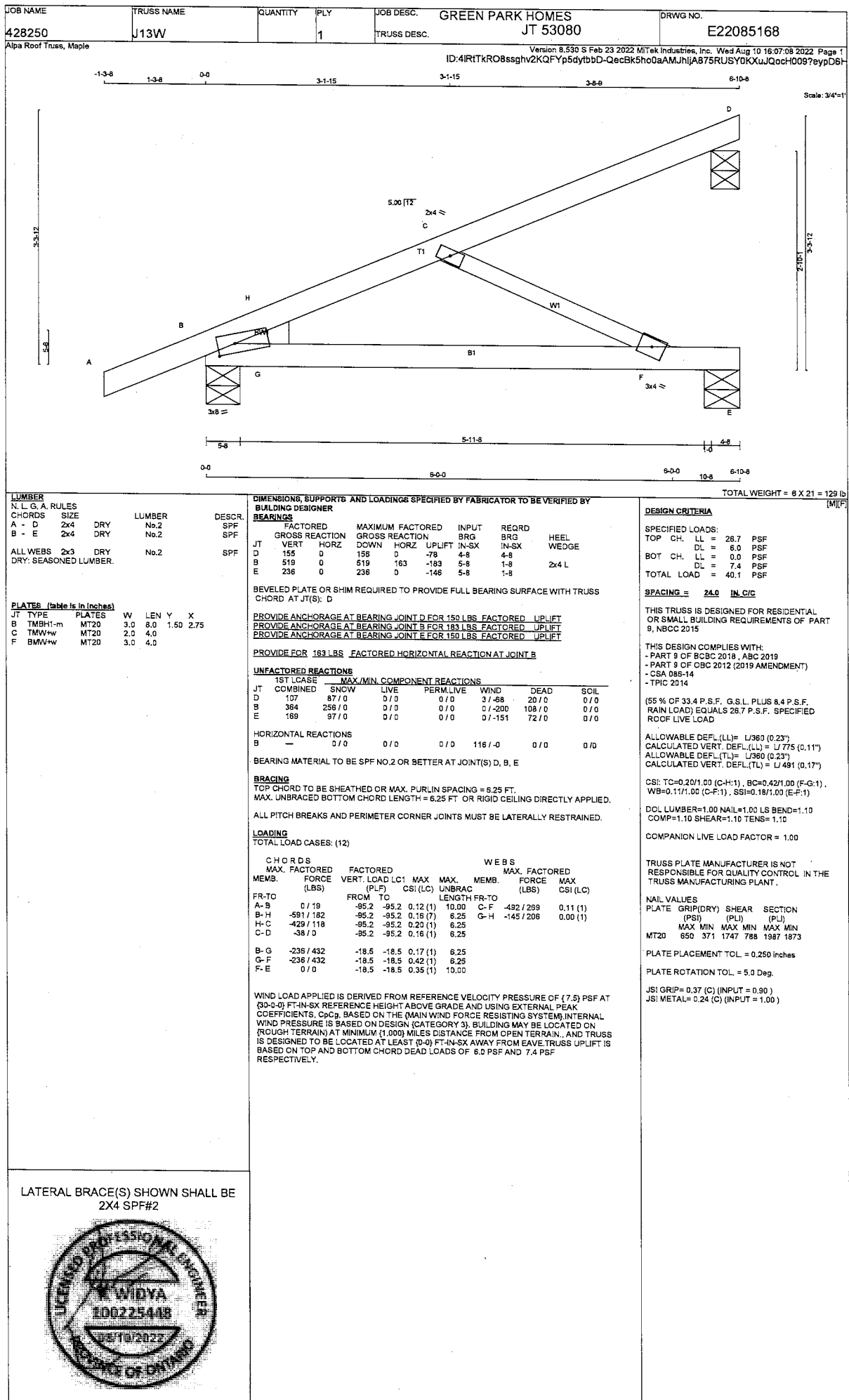
Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:07:02 2022 Page 1

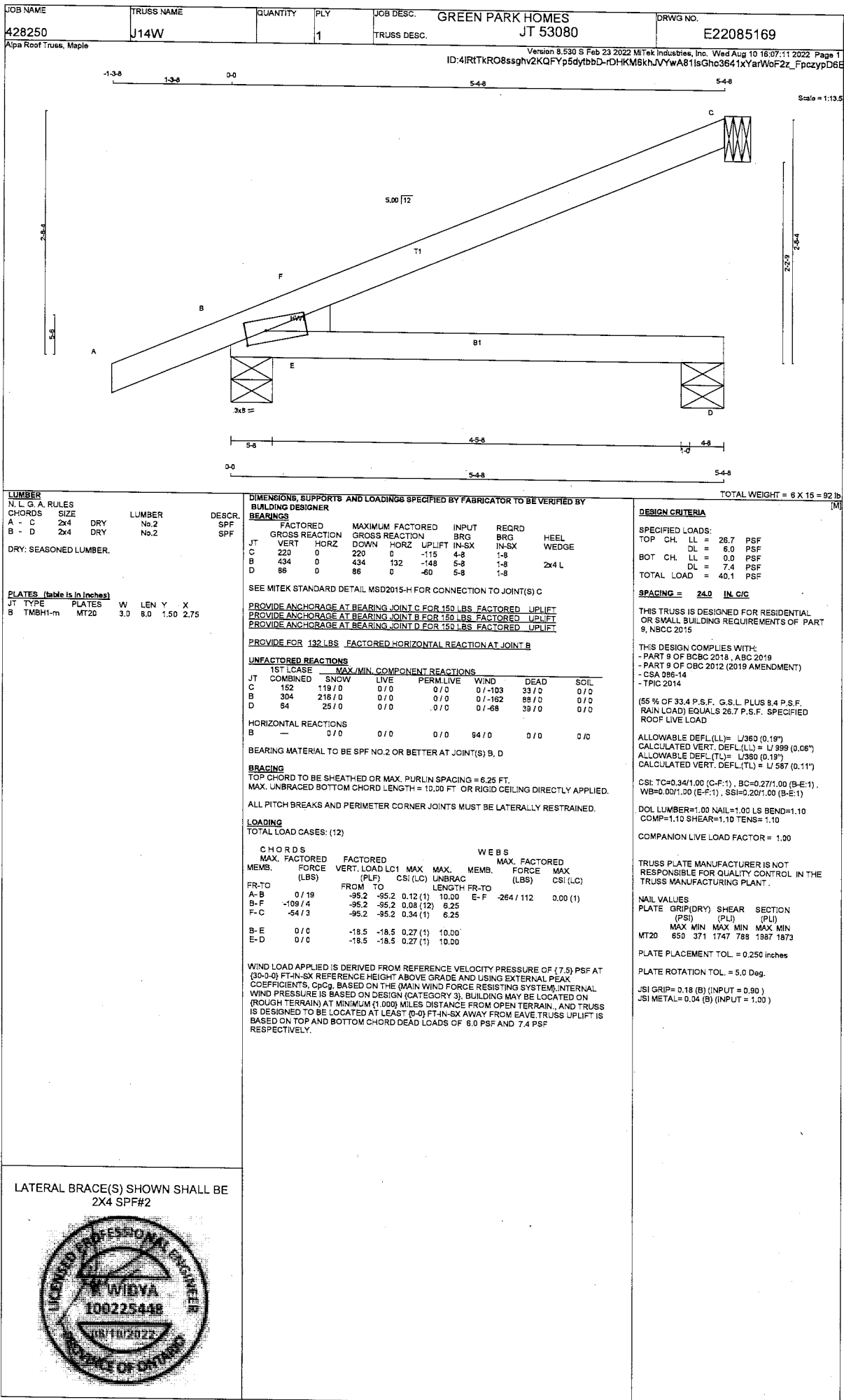
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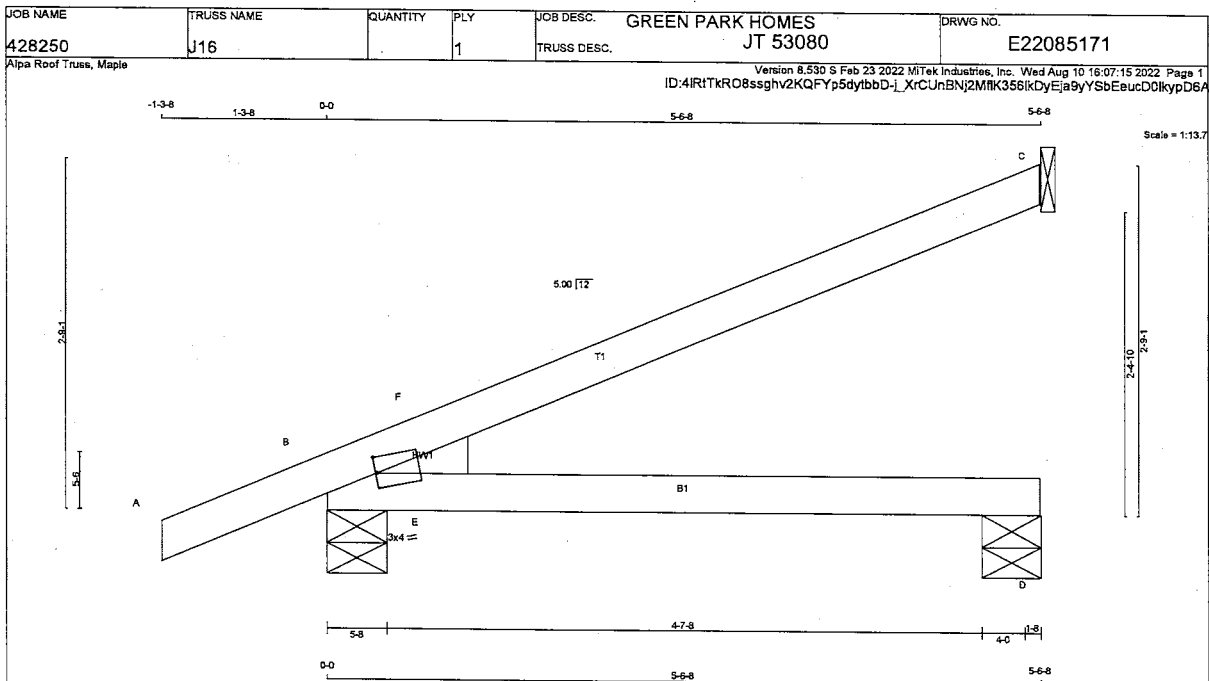
LUMBER N.L.G.A. RULES <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>CHORDS</th> <th>SIZE</th> <th>LUMBER</th> <th>DESCR.</th> </tr> <tr> <td>F - B</td> <td>2x4</td> <td>DRY</td> <td>No.2</td> </tr> <tr> <td>A - C</td> <td>2x4</td> <td>DRY</td> <td>No.2</td> </tr> <tr> <td>F - D</td> <td>2x4</td> <td>DRY</td> <td>No.2</td> </tr> </table> ALL WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER. PLATES (table is in inches) <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> <tr> <td>B</td> <td>TMVW+p</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td>1.00</td> <td>2.00</td> </tr> <tr> <td>E</td> <td>BMVW+p</td> <td>MT20</td> <td>2.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>F</td> <td>BMV1+p</td> <td>MT20</td> <td>3.0</td> <td>4.0</td> <td></td> <td></td> </tr> </table>	CHORDS	SIZE	LUMBER	DESCR.	F - B	2x4	DRY	No.2	A - C	2x4	DRY	No.2	F - D	2x4	DRY	No.2	JT	TYPE	PLATES	W	LEN	Y	X	B	TMVW+p	MT20	4.0	4.0	1.00	2.00	E	BMVW+p	MT20	2.0	4.0			F	BMV1+p	MT20	3.0	4.0			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th></th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>RECORD BRG</th> </tr> <tr> <td>JT</td> <td>VERT</td> <td>DOWN</td> <td>HORIZ</td> <td>UPLIFT</td> </tr> <tr> <td>F</td> <td>257</td> <td>0</td> <td>257</td> <td>0</td> </tr> <tr> <td>C</td> <td>105</td> <td>0</td> <td>105</td> <td>0</td> </tr> <tr> <td>D</td> <td>20</td> <td>0</td> <td>23</td> <td>0</td> </tr> </table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D UNFACTORED REACTIONS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>1ST LCASE</th> <th>SNOW</th> <th>LIVE</th> <th>PERM. LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> <tr> <td>JT</td> <td>COMBINED</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>F</td> <td>179</td> <td>133 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>48 / 0</td> <td>0 / 0</td> </tr> <tr> <td>C</td> <td>72</td> <td>58 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>13 / 0</td> <td>0 / 0</td> </tr> <tr> <td>D</td> <td>16</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>16 / 0</td> <td>0 / 0</td> </tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (5) <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th colspan="4">CHORDS</th> <th colspan="4">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>FACTORED LC1 MAX. CSI (LC)</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX. CSI (LC)</th> <th></th> </tr> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> <td>FR-TO</td> <td></td> <td></td> <td></td> </tr> <tr> <td>F-B</td> <td>-236 / 0</td> <td>0.0</td> <td>0.02 (1)</td> <td>7.81</td> <td>B-E</td> <td>0 / 0</td> <td>0.00 (1)</td> </tr> <tr> <td>A-B</td> <td>0 / 39</td> <td>-95.2</td> <td>-95.2 0.19 (5)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>B-C</td> <td>0 / 0</td> <td>-95.2</td> <td>-95.2 0.08 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>F-E</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5 0.03 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>E-D</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5 0.02 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> </table> CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	JT	VERT	DOWN	HORIZ	UPLIFT	F	257	0	257	0	C	105	0	105	0	D	20	0	23	0	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED						F	179	133 / 0	0 / 0	0 / 0	48 / 0	0 / 0	C	72	58 / 0	0 / 0	0 / 0	13 / 0	0 / 0	D	16	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		FR-TO		FROM TO		FR-TO				F-B	-236 / 0	0.0	0.02 (1)	7.81	B-E	0 / 0	0.00 (1)	A-B	0 / 39	-95.2	-95.2 0.19 (5)	10.00				B-C	0 / 0	-95.2	-95.2 0.08 (1)	10.00				F-E	0 / 0	-18.5	-18.5 0.03 (4)	10.00				E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00				TOTAL WEIGHT = 2 X 10 = 21 lb DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 40.1 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2018 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.00") CSI: TC=0.13/1.00 (A-B-5), BC=0.03/1.00 (E-F-4), WB=0.00/1.00 (B-E-1), SSI=0.08/1.00 (A-B-5) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR (PSI)</th> <th>SECTION (PL)</th> </tr> <tr> <td>MT20</td> <td>650</td> <td>371</td> <td>1747</td> </tr> </table> MAX MIN MAX MIN MAX MIN 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.18 (B) (INPUT = 0.90) JSI METAL= 0.05 (B) (INPUT = 1.00)	PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MT20	650	371	1747
CHORDS	SIZE	LUMBER	DESCR.																																																																																																																																																																															
F - B	2x4	DRY	No.2																																																																																																																																																																															
A - C	2x4	DRY	No.2																																																																																																																																																																															
F - D	2x4	DRY	No.2																																																																																																																																																																															
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																																																																																												
B	TMVW+p	MT20	4.0	4.0	1.00	2.00																																																																																																																																																																												
E	BMVW+p	MT20	2.0	4.0																																																																																																																																																																														
F	BMV1+p	MT20	3.0	4.0																																																																																																																																																																														
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG																																																																																																																																																																														
JT	VERT	DOWN	HORIZ	UPLIFT																																																																																																																																																																														
F	257	0	257	0																																																																																																																																																																														
C	105	0	105	0																																																																																																																																																																														
D	20	0	23	0																																																																																																																																																																														
1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL																																																																																																																																																																												
JT	COMBINED																																																																																																																																																																																	
F	179	133 / 0	0 / 0	0 / 0	48 / 0	0 / 0																																																																																																																																																																												
C	72	58 / 0	0 / 0	0 / 0	13 / 0	0 / 0																																																																																																																																																																												
D	16	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0																																																																																																																																																																												
CHORDS				WEBS																																																																																																																																																																														
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)																																																																																																																																																																												
FR-TO		FROM TO		FR-TO																																																																																																																																																																														
F-B	-236 / 0	0.0	0.02 (1)	7.81	B-E	0 / 0	0.00 (1)																																																																																																																																																																											
A-B	0 / 39	-95.2	-95.2 0.19 (5)	10.00																																																																																																																																																																														
B-C	0 / 0	-95.2	-95.2 0.08 (1)	10.00																																																																																																																																																																														
F-E	0 / 0	-18.5	-18.5 0.03 (4)	10.00																																																																																																																																																																														
E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00																																																																																																																																																																														
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)																																																																																																																																																																															
MT20	650	371	1747																																																																																																																																																																															

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2









LUMBER	DESCR.	SPF
N.L.G.A. RULES		
CHORDS SIZE		
A - C 2x4 DRY		No.2
B - D 2x4 DRY		No.2
DRY: SEASONED LUMBER.		

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMBH1-m MT20	3.0	4.0	1.50	0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED	MAXIMUM FACTORED	INPUT	REQD	HEEL					
GROSS REACTION	GROSS REACTION	BRG	BRG	WEDGE					
JT VERT HORZ	DOWN HORZ	UPLIFT	IN-SX	IN-SX					
C 227 0	227 0	0	1-8	1-8					
B 443 0	443 0	0	5-8	1-8					
D 88 0	88 0	0	5-8	1-8					

UNFACTORED REACTIONS									
1ST LCASE	MAX / MIN	COMPONENT	REACTIONS						
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	HEEL	WEDGE
C	157	123 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0	0 / 0	0 / 0
B	311	223 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0	0 / 0	0 / 0
D	68	25 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0	0 / 0	0 / 0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
A-B	0 / 19	-95.2	-95.2 0.12 (1)	10.00	E-F	-252 / 7	0.00 (1)		
B-F	-23 / 0	-95.2	-95.2 0.08 (4)	8.25					
F-C	0 / 3	-95.2	-95.2 0.37 (1)	10.00					
B-E	0 / 0	-18.5	-18.5 0.29 (1)	10.00					
E-D	0 / 0	-18.5	-18.5 0.29 (1)	10.00					

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL = 26.7 PSF	
DL = 6.0 PSF	
BOT CH. LL = 0.0 PSF	
DL = 7.4 PSF	
TOTAL LOAD = 40.1 PSF	

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/538$ (0.12")

CSI: TC=0.37/1.00 (C-F:1), BC=0.28/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.42 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME 428250	TRUSS NAME J17	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085172
Alpa Roof Truss, Maple					
Version 8.530 S Feb 23 2022 MTEK Industries, Inc. Wed Aug 10 16:07:17 2022 Page 1 ID:4IRtRkROBssghv2KQFyp5dytbdb-fNbdAoRuLl4u3USCXnCNJK6ZzZ9wTPxLwi7qcyDDB					

Scale: 3/4"=1'

LUMBER						DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N.L.G.A. RULES						BEARINGS			
CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A - D	2x4	DRY	No.2	SPF		JT VERT HORZ	DOWN HORZ UPLIFT IN-SX	IN-SX	
B - E	2x4	DRY	No.2	SPF		D 131 0	131 0 0 4-8	4-8	
F	BMV+w					B 556 0	556 0 0 5-8	1-8	
G	BMV+w					E 288 0	288 0 0 5-8	1-8	
ALL WEBS									
2x3									
DRY SEASONED LUMBER.									

UNFACTORED REACTIONS									
1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
O	90	72 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0		
B	390	273 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0		
E	212	130 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	VERT. LOAD (LC1)	FACTORED (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED (PLF)
FR-TO					FR-TO		
A-B	0 / 19	-95.2	-95.2	0.12 (1)	G-C	0 / 294	0.97 (1)
B-I	-707 / 0	-95.2	-95.2	0.02 (1)	C-F	-804 / 0	0.15 (1)
I-C	-701 / 0	-95.2	-95.2	0.14 (1)	H-I	-181 / 0	0.00 (1)
C-D	-8 / 0	-95.2	-95.2	0.11 (1)			
B-H	0 / 670	-18.5	-18.5	0.25 (1)			
H-G	0 / 670	-18.5	-18.5	0.25 (1)			
G-F	0 / 670	-18.5	-18.5	0.53 (1)			
F-E	0 / 0	-18.5	-18.5	0.43 (1)			

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL)= U/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL)= U/999 (0.08")

CSI: TC=0.14/1.00 (C-F), BC=0.53/1.00 (F-G), WB=0.15/1.00 (C-F), SSI=0.23/1.00 (E-F)
DOL LUMBER=1.20 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

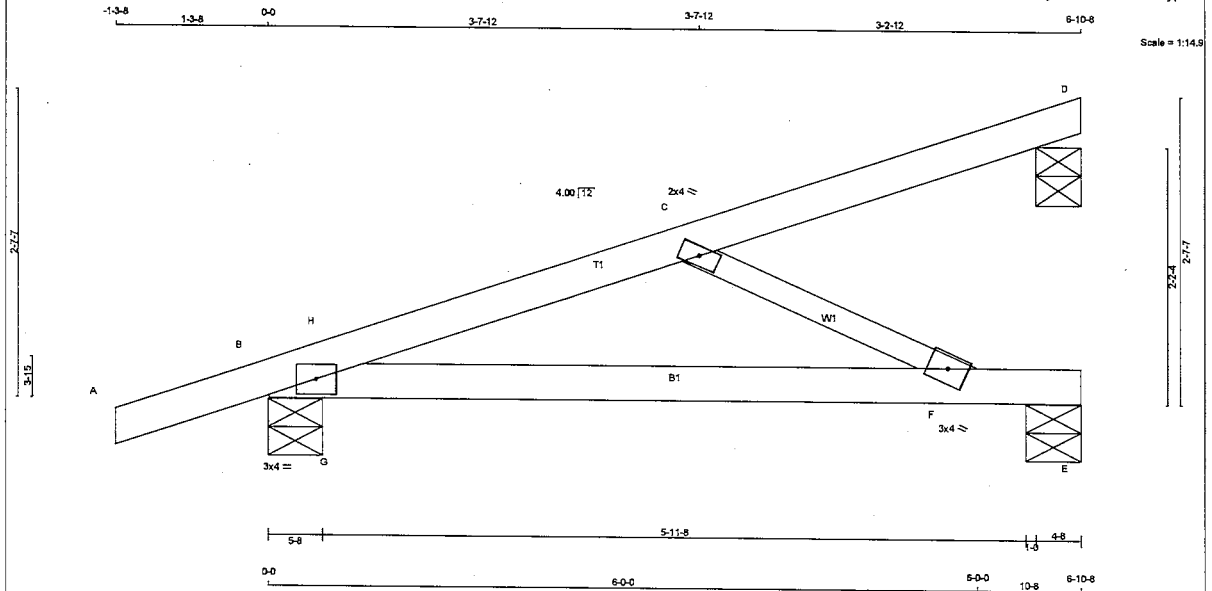
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP= 0.52 (F) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)

JOB NAME 428250	TRUSS NAME J18W	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085173
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Alpa Roof Truss, Maple

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - D 2x4 DRY No.2
B - E 2x4 DRY No.2
ALL WEBS 2x3 DRY, SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB14 MT20 3.0 4.0
C TMWrw MT20 2.0 4.0
F BMWrw MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
D	138	0	138	0	-66	4-8
B	518	0	518	129	-196	5-8
E	253	0	253	0	-145	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 196 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS. FACTORED UPLIFT

PROVIDE FOR 129 LBS. FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS						
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	95	78 / 0	0 / 0	0 / 0	0 / 0	1 / -59	19 / 0	0 / 0
B	363	255 / 0	0 / 0	0 / 0	0 / 0	0 / -210	108 / 0	0 / 0
E	181	108 / 0	0 / 0	0 / 0	0 / 0	0 / -150	73 / 0	0 / 0

HORIZONTAL REACTIONS					
B	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX. (PLF)	MAX. FACTORED VERT. LOAD LC1 MAX. (PLF)	MAX. FACTORED VERT. LOAD LC1 MAX. (PLF)
A-B	0 / 19	-95.2	-95.2	0.12 (1)
B-H	-547 / 167	-95.2	-95.2	0.18 (1)
H-C	-429 / 125	-95.2	-95.2	0.25 (1)
C-D	-26 / 0	-95.2	-95.2	0.12 (1)
B-G	-211 / 428	-18.5	-18.5	0.12 (1)
G-F	-211 / 428	-18.5	-18.5	0.44 (1)
F-E	0 / 0	-18.5	-18.5	0.37 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.5) PSF AT (10-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_{pe} , BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 3). BUILDING MAY BE LOCATED ON (ROUGH TERRAIN) AT MINIMUM (1.000) MILES DISTANCE FROM OPEN TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(LL) = $L/686$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (0.23")
CALCULATED VERT. DEFL.(TL) = $L/397$ (0.21")

CSI: $TC=0.25/1.00$ (C-H:1), $BC=0.44/1.00$ (F-G:1),
 $WB=0.09/1.00$ (C-F:1), $SSI=0.21/1.00$ (B-H:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

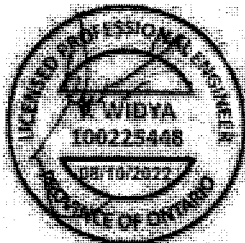
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.38 (C) (INPUT = 0.30)
JSI METAL = 0.25 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SP#2



JOB NAME 428250	TRUSS NAME J19	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES JT 53080	DRWG NO. E22085174																																																																																																																																																																																						
Alpa Roof Truss, Maple																																																																																																																																																																																											
Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Wed Aug 10 16:07:22 2022 Page 1 ID:4IRtRkRO8ssghv2KQFYp5dytbD-0KSUgtsjxM?qqMQ74NN0R1uF_KKbmzgVBpuVqypD63																																																																																																																																																																																											
Scale = 1:13.8																																																																																																																																																																																											
TOTAL WEIGHT = 5 X 15 = 75 lb																																																																																																																																																																																											
LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER No.2 A - C 2x4 DRY B - D 2x4 DRY DESCR. SPF SPF DRY: SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><thead><tr><th></th><th colspan="2">FACTORED GROSS REACTION</th><th colspan="2">MAXIMUM FACTORED GROSS REACTION</th><th colspan="2">INPUT BRG</th><th colspan="2">REQRD BRG</th></tr><tr><th></th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th><th>IN-SX</th><th>IN-SX</th><th></th></tr></thead><tbody><tr><td>JT</td><td>238</td><td>0</td><td>238</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td><td></td></tr><tr><td>C</td><td>453</td><td>0</td><td>453</td><td>0</td><td>0</td><td>5-8</td><td>1-8</td><td></td></tr><tr><td>D</td><td>87</td><td>0</td><td>87</td><td>0</td><td>0</td><td>5-8</td><td>1-8</td><td></td></tr></tbody></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C UNFACTORED REACTIONS <table><thead><tr><th></th><th colspan="2">1ST LCASE COMBINED</th><th colspan="2">SNOW</th><th colspan="2">LIVE</th><th colspan="2">PERM. LIVE</th><th colspan="2">WIND</th><th colspan="2">DEAD</th><th colspan="2">SOIL</th></tr></thead><tbody><tr><td>JT</td><td>184</td><td>129 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>35 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td></tr><tr><td>B</td><td>317</td><td>225 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>33 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>65</td><td>23 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>42 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td></tr></tbody></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><thead><tr><th colspan="2">CHORDS</th><th colspan="4">MEMB.</th><th colspan="4">WEBS</th></tr><tr><th></th><th>MAX. FACTORED FORCE (LBS)</th><th>VERT. LOAD (PLF)</th><th>LC1</th><th>MAX</th><th>MAX. UNBRACED LENGTH FR-TO</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX</th><th>CS1 (LC)</th></tr></thead><tbody><tr><td>FR-TO</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 23</td><td>-95.2</td><td>-95.2</td><td>0.12 (1)</td><td>10.00</td><td>E-F</td><td>-399 / 12</td><td>0.00 (1)</td><td></td></tr><tr><td>B-F</td><td>-24 / 51</td><td>-95.2</td><td>-95.2</td><td>0.09 (1)</td><td>6.25</td><td></td><td></td><td></td><td></td></tr><tr><td>F-C</td><td>-4 / 2</td><td>-95.2</td><td>-95.2</td><td>0.40 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>B-E</td><td>0 / 0</td><td>-18.5</td><td>-18.5</td><td>0.27 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.5</td><td>-18.5</td><td>0.28 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr></tbody></table>					FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG			VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		JT	238	0	238	0	0	1-8	1-8		C	453	0	453	0	0	5-8	1-8		D	87	0	87	0	0	5-8	1-8			1ST LCASE COMBINED		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL		JT	184	129 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0	0 / 0	0 / 0	0 / 0	B	317	225 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0	0 / 0	0 / 0	0 / 0	D	65	23 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0	0 / 0	0 / 0	0 / 0	CHORDS		MEMB.				WEBS					MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)	FR-TO										A-B	0 / 23	-95.2	-95.2	0.12 (1)	10.00	E-F	-399 / 12	0.00 (1)		B-F	-24 / 51	-95.2	-95.2	0.09 (1)	6.25					F-C	-4 / 2	-95.2	-95.2	0.40 (1)	10.00					B-E	0 / 0	-18.5	-18.5	0.27 (1)	10.00					E-D	0 / 0	-18.5	-18.5	0.28 (1)	10.00				
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DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 40.1 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.7 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/523 (0.13") CS1 TC=0.40/1.00 (C-F:1), BC=0.28/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.33/1.00 (B-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.33 (B) (INPUT = 0.30) JSI METAL= 0.08 (B) (INPUT = 1.00)																																																																																																																																																																																											
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2																																																																																																																																																																																											

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MINIMUM NUMBER OF NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

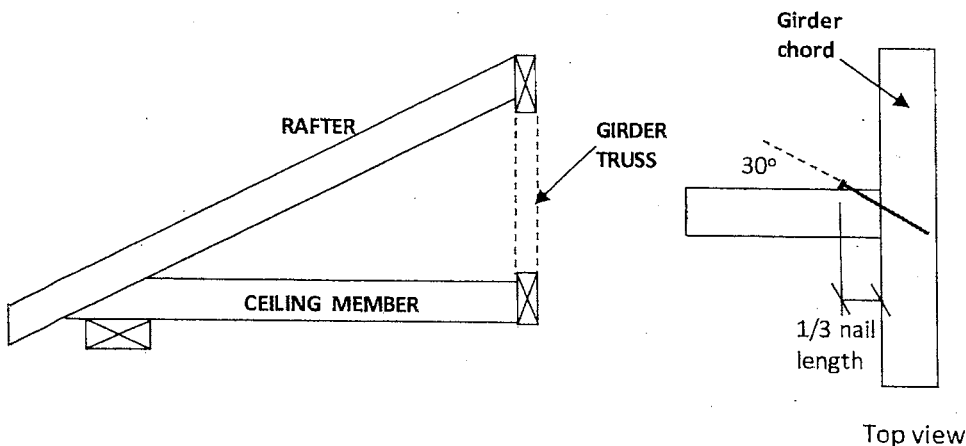


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

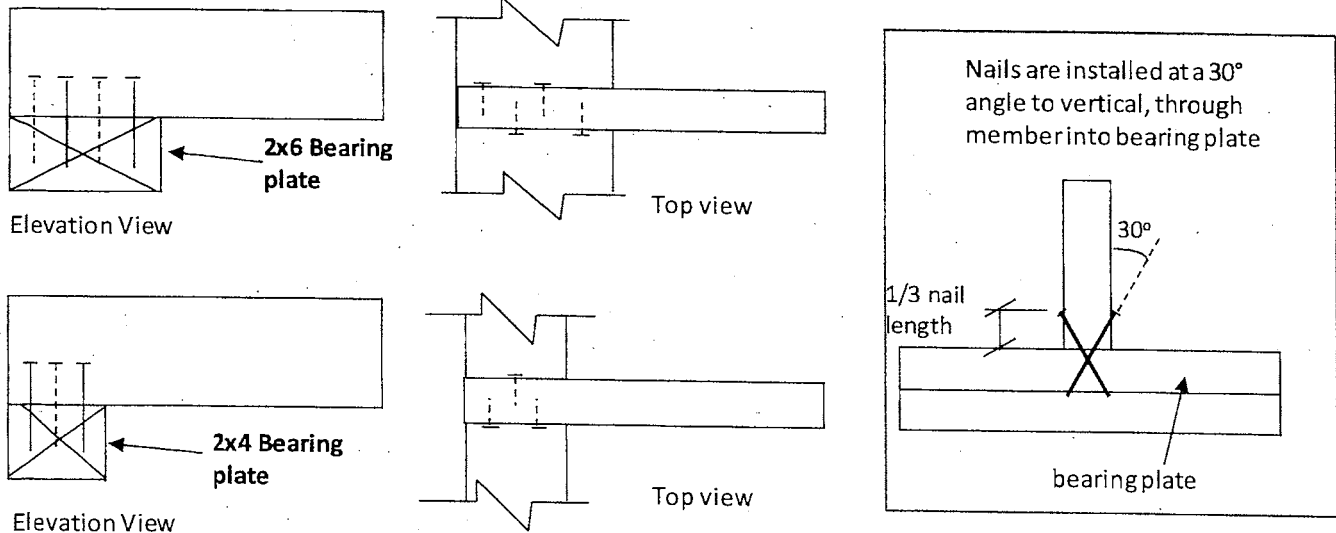
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_0 factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA Q86-19, Clause 12.9.

PEO
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LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

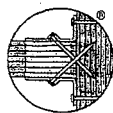
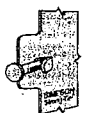
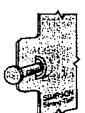
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

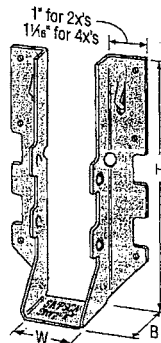
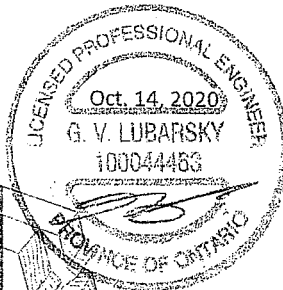
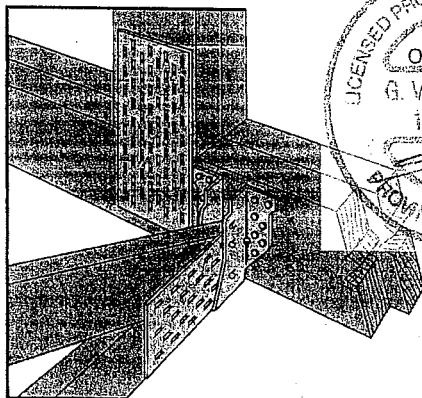
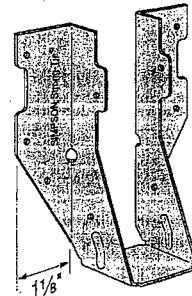
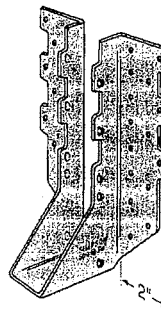
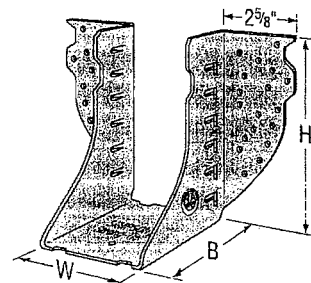
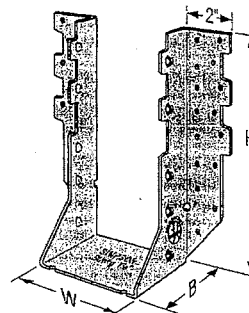
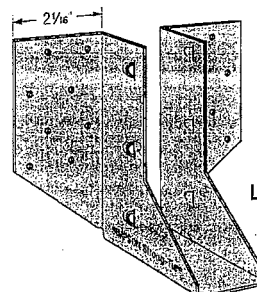
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.

Double-Shear
Nailing
Top ViewDouble-Shear
Nailing
Side View;
Do not
bend tabDouble-Shear
Nailing
Side View
(available on
some models)

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)

 **LUS28****LU26L** **HUS210**
(HUS26, HUS28,
and HHUS similar) **HGUS28-2** **HHUS210-2****LJS26DS**

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

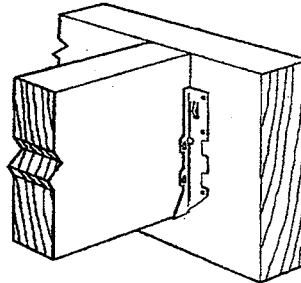
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

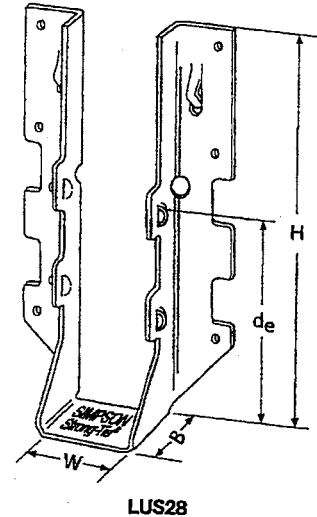
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

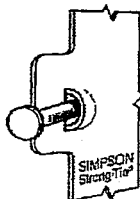


Typical LUS
Installation



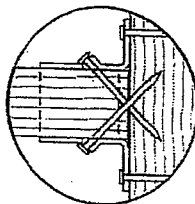
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1½	3½	1¾	1 7/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 7/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 1/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8 1/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

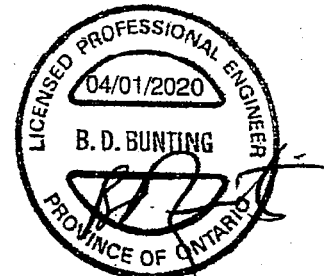


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
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HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

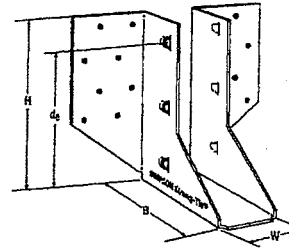
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

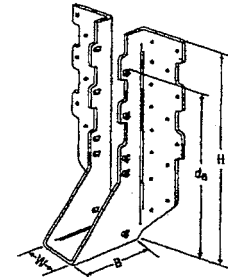
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

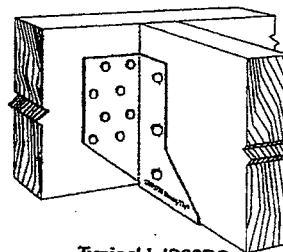
- See current catalogue for options



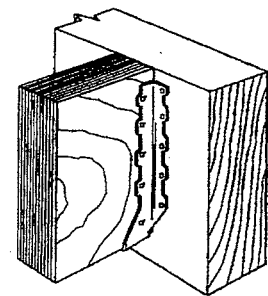
LJS26DS



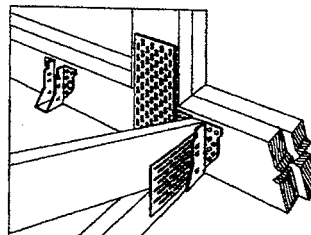
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



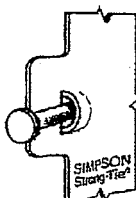
Typical HUS
Installation



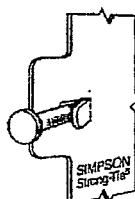
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LJS26DS	18	1⅞	5	3½	4⅞	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1⅞	5⅞	3	3⅞	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1⅞	9⅞	3	7⅞	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

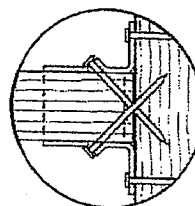
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



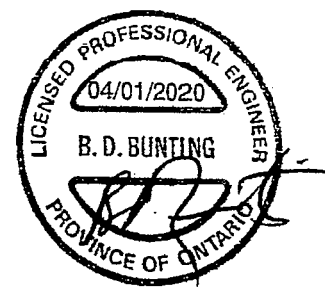
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

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T-SPECHUS20 3/20 exp. 6/22

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strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

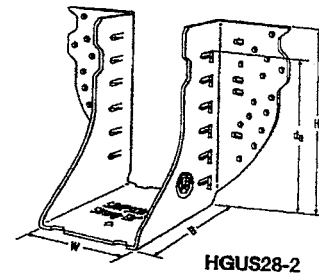
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

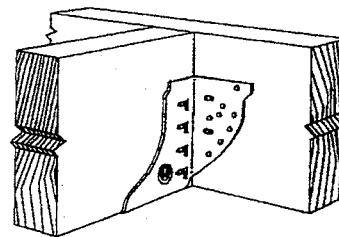
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

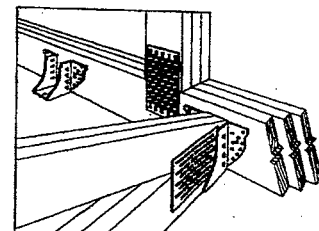
- See current catalogue for options



HGUS28-2



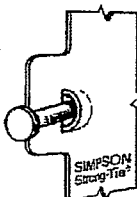
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

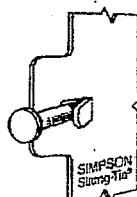
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/16	7 1/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/16	7 1/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 1/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 1/16	9 1/16	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/16	9 1/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/16	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/16	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

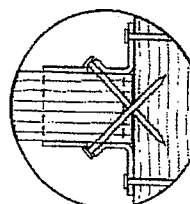


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

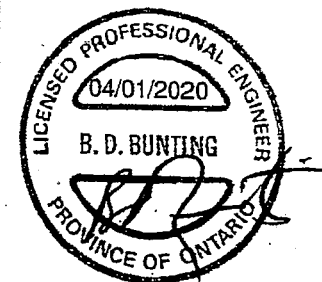
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers



All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

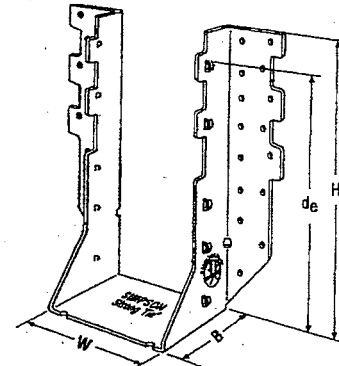
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

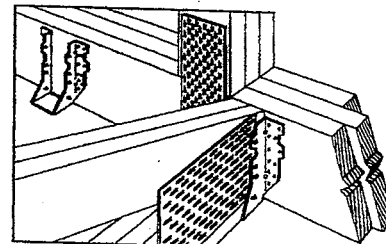
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia. x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



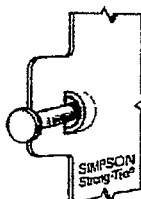
HHUS410



Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

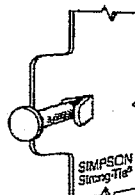
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d_e^1	Face	Joist	D.Fir-L		S-P-F	
								Uplift ($K_p=1.15$)	Normal ($K_p=1.00$)	Uplift ($K_p=1.15$)	Normal ($K_p=1.00$)
HHUS26-2	14	$3\frac{1}{16}$	$5\frac{1}{16}$	3	$3\frac{1}{16}$	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	$3\frac{1}{16}$	$7\frac{1}{32}$	3	$6\frac{1}{32}$	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	$3\frac{1}{16}$	$9\frac{1}{32}$	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	$4\frac{1}{16}$	9	3	$7\frac{1}{16}$	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	$6\frac{1}{16}$	$8\frac{1}{32}$	3	$7\frac{1}{32}$	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	$3\frac{1}{16}$	$5\frac{1}{32}$	3	$3\frac{1}{16}$	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	$3\frac{1}{16}$	$7\frac{1}{16}$	3	$6\frac{1}{16}$	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	$3\frac{1}{16}$	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	$5\frac{1}{2}$	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	$7\frac{1}{4}$	9	$3\frac{1}{16}$	$7\frac{1}{32}$	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

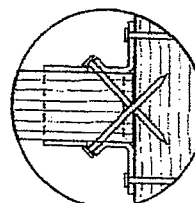


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

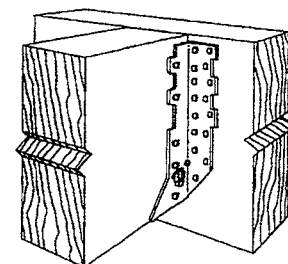
U.S. Patent 5,603,580



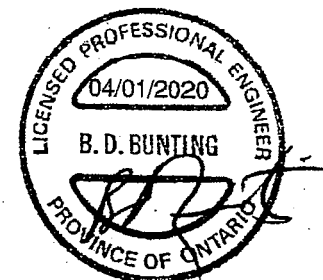
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



LIMIT STATES DESIGN

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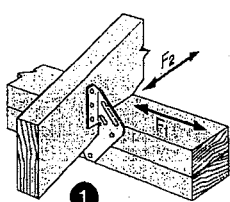
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHHUS20 3/20 exp. 6/22

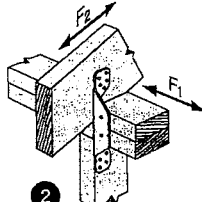
(800) 999-5099
strongtie.com

H/TSP

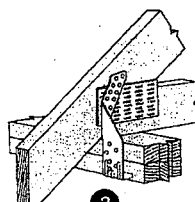
Seismic and Hurricane Ties (cont.)



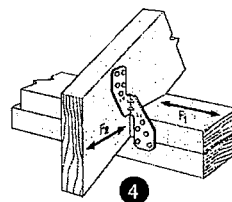
1 H1 Installation



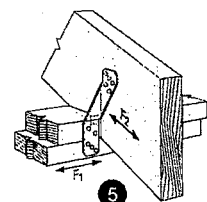
2 H2A Installation



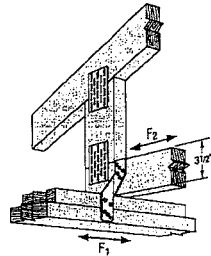
3 TSP Installation



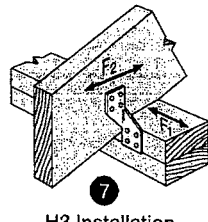
4 H2.5A Installation
(Nails into both top plates)



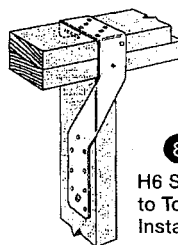
5 H2.5T Installation
(Nails into both top plates)



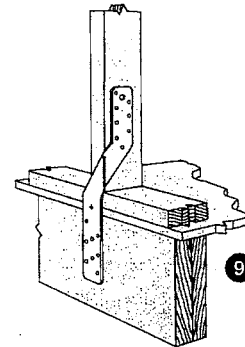
6 H2.5T Installation



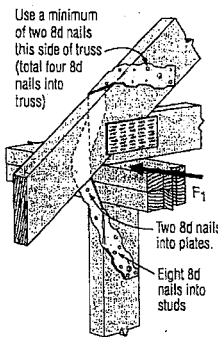
7 H3 Installation
(Nails into upper top plate)



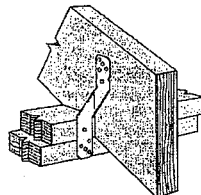
8 H6 Stud to Top Plate Installation



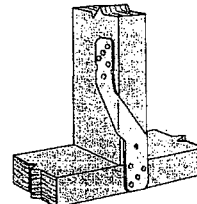
9 H6 Stud to Band Joist Installation



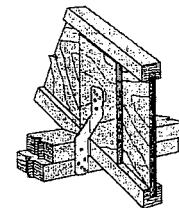
10 H7Z Installation



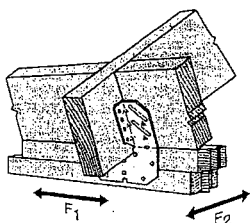
11 H8 Attaching Rafter to Double Top Plates



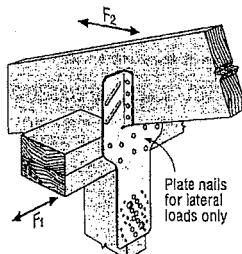
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



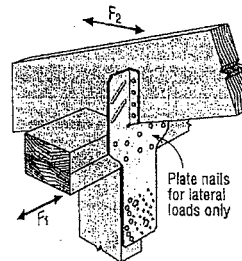
13 H8 attaching I-Joist to Double Top Plates



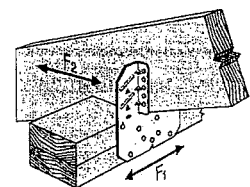
14 H10A Field-Bent Installation



15 H10S Installation

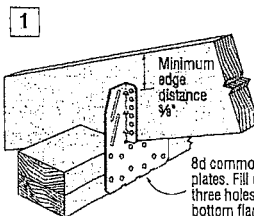


16 H10S Installation with Stud Offset

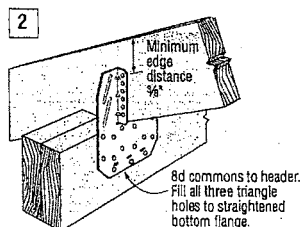


17 H10A Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

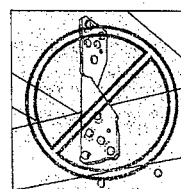


18 H14 Installation to Double Top Plates



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

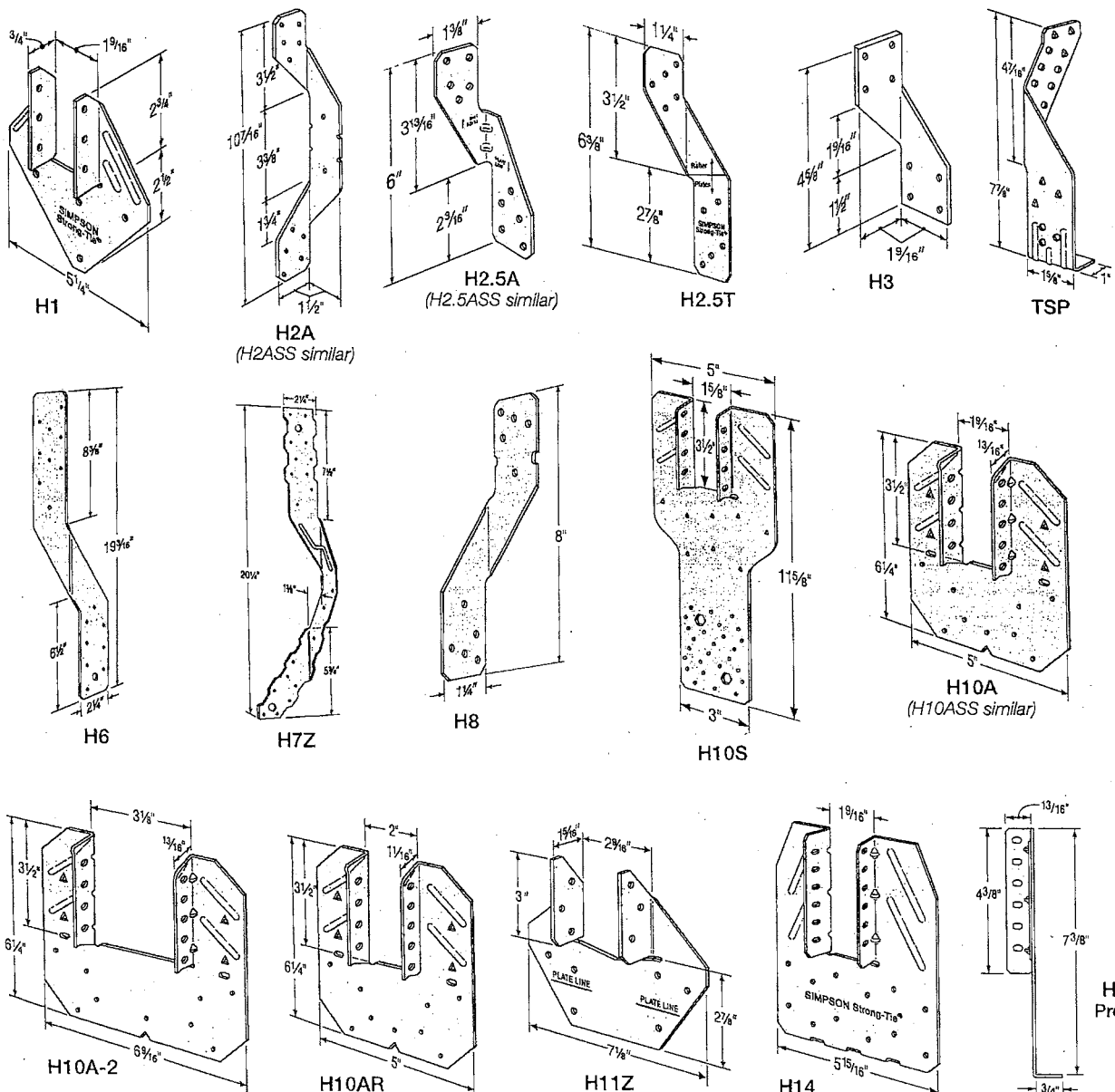
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp.20-24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

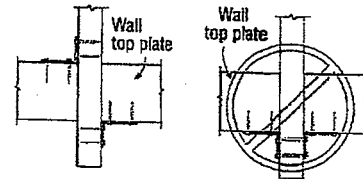
Design: • Factored resistances are in accordance with CSA 086-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

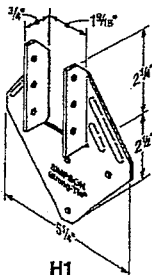
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

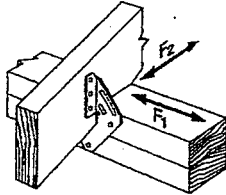


Install diagonally across from each other for minimum 2x truss.

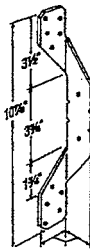
Nailing into both sides of a single ply 2x truss may cause the wood to split.



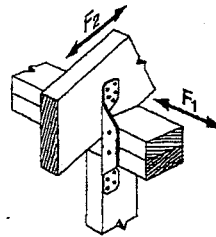
H1



H1 Installation



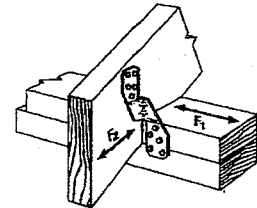
H2A



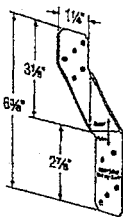
H2A Installation



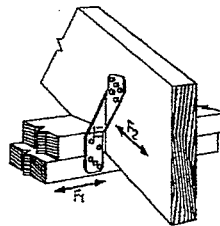
H2.5A



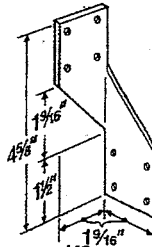
H2.5A Installation



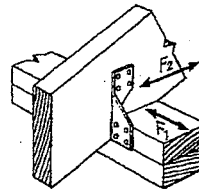
H2.5T



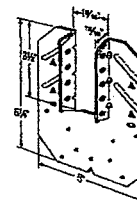
H2.5T Installation
(Nails into both top plates)



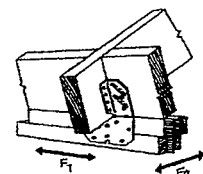
H3



H3 Installation



H10A

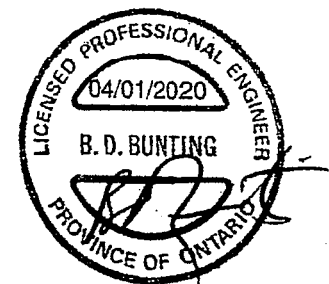


H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
		To Rafter	To Plates	To Studs	D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TC - Truss Connectors

SIMPSON
Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1/4". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

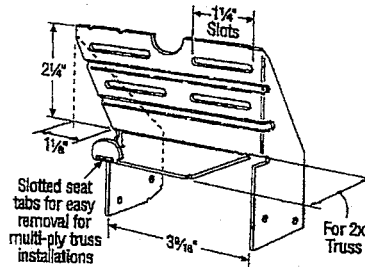
Design: Factored resistances are in accordance with CSA 086-14

Installation:

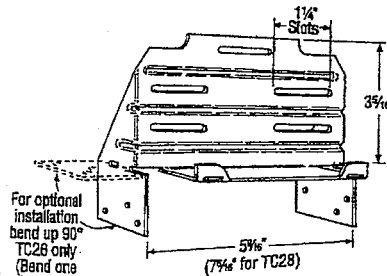
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

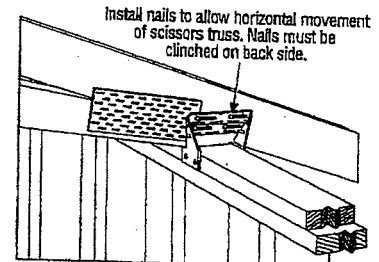
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



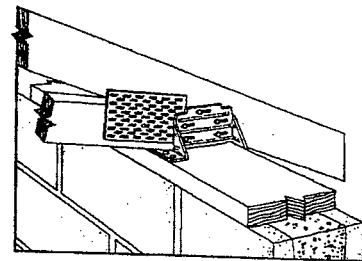
TC24
U.S. Patent 4,932,173



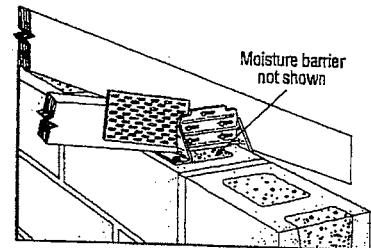
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K _o =1.15)	Uplift (K _o =1.15)
TC26	(5) 10d	(6) 10d x 1 1/2"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - 1/8" x 2 1/4" Titen screws has a factored uplift resistance of 275 lb.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HTU

SIMPSON
Strong-Tie

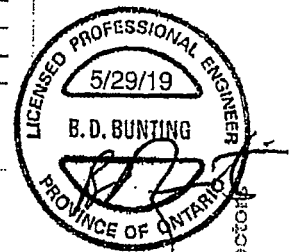
Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
			Header	Joist	D.Fir-L		S-P-F	
					Uplift	Normal	Uplift	Normal
					(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3 7/8	(2) 2x4	(10) 16d	(14) 10d x 1 1/2"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5 1/2	(2) 2x4	(10) 16d	(20) 10d x 1 1/2"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3 7/8	(2) 2x6	(20) 16d	(26) 10d x 1 1/2"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7 1/4	(2) 2x6	(20) 16d	(32) 10d x 1 1/2"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

See table footnotes on p. 280.

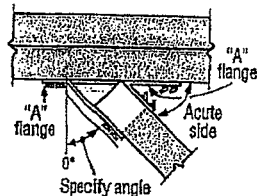


Hanger Options

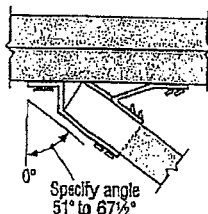
See Hanger Options Information on pp. 125-127.

Skewed Seat

- Skawable up to 67 1/2°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
		Header	Joist	D.Fir-L		S-P-F	
				Uplift	Normal	Uplift	Normal
				(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1 1/2"	1835	4110	1300	2905
	51-67 1/2	(20) 16d	(12) 10d x 1 1/2"	8.16	15.28	5.78	12.92
HTU28	< 51	(26) 16d	(20) 10d x 1 1/2"	1350	3620	955	2560
	51-67 1/2	(26) 16d	(17) 10d x 1 1/2"	8.01	16.10	4.25	11.39
HTU210	< 51	(32) 16d	(26) 10d x 1 1/2"	2810	4270	1985	3030
	51-67 1/2	(32) 16d	(22) 10d x 1 1/2"	12.50	18.99	8.83	13.48
HTU26-2	< 51	(20) 16d	(14) 10d	2075	3930	1465	2780
	51-67 1/2	(20) 16d	(12) 10d	9.23	17.48	6.52	12.37
HTU28-2	< 51	(26) 16d	(20) 10d	3785	4430	2675	3135
	51-67 1/2	(26) 16d	(17) 10d	16.84	19.71	11.90	13.95
HTU210-2	< 51	(32) 16d	(26) 10d	2795	4240	1980	3000
	51-67 1/2	(32) 16d	(22) 10d	12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
	51-67 1/2	(20) 16d	(12) 10d	9.52	16.53	6.74	11.68
HTU28-2	< 51	(26) 16d	(20) 10d	1510	3920	1140	2785
	51-67 1/2	(26) 16d	(17) 10d	7.16	17.44	5.07	12.39
HTU210-2	< 51	(32) 16d	(26) 10d	3960	5425	2815	3855
	51-67 1/2	(32) 16d	(22) 10d	17.62	24.13	12.52	17.15
HTU26-2	< 51	(20) 16d	(14) 10d	2385	5425	1695	3855
	51-67 1/2	(20) 16d	(12) 10d	10.61	24.13	7.54	17.15
HTU28-2	< 51	(26) 16d	(20) 10d	5025	6890	3570	4890
	51-67 1/2	(26) 16d	(17) 10d	22.35	30.65	15.98	21.75
HTU210-2	< 51	(32) 16d	(26) 10d	3145	6680	2225	4745
	51-67 1/2	(32) 16d	(22) 10d	13.99	29.72	9.90	21.10

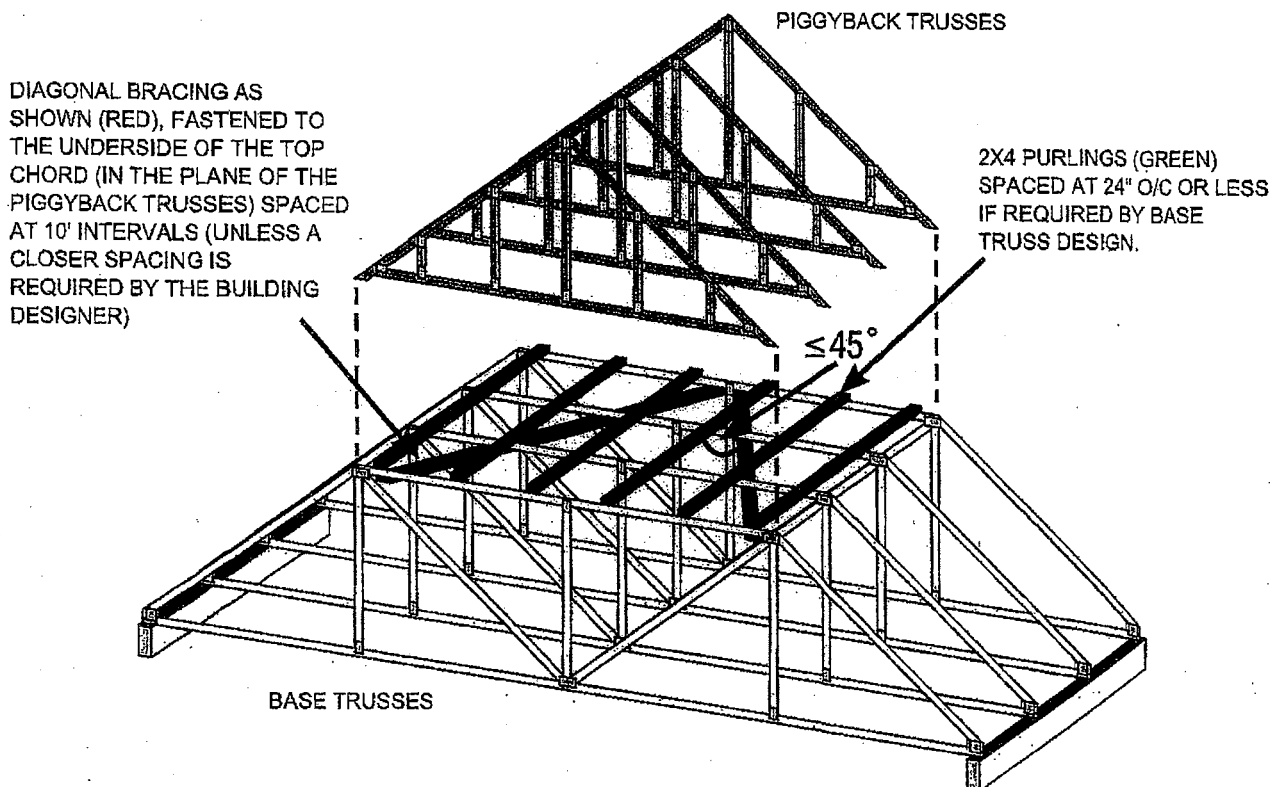
1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3 1/2" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 10d = 0.148" dia. x 3" long. See pp. 27-28 for other nail sizes and information.

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and **MSTA** — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and **MSTI** — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

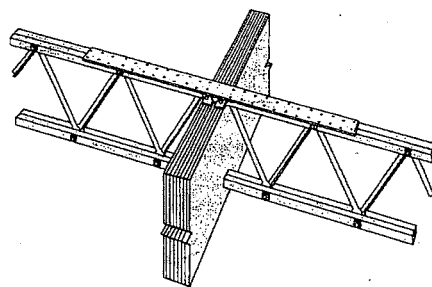
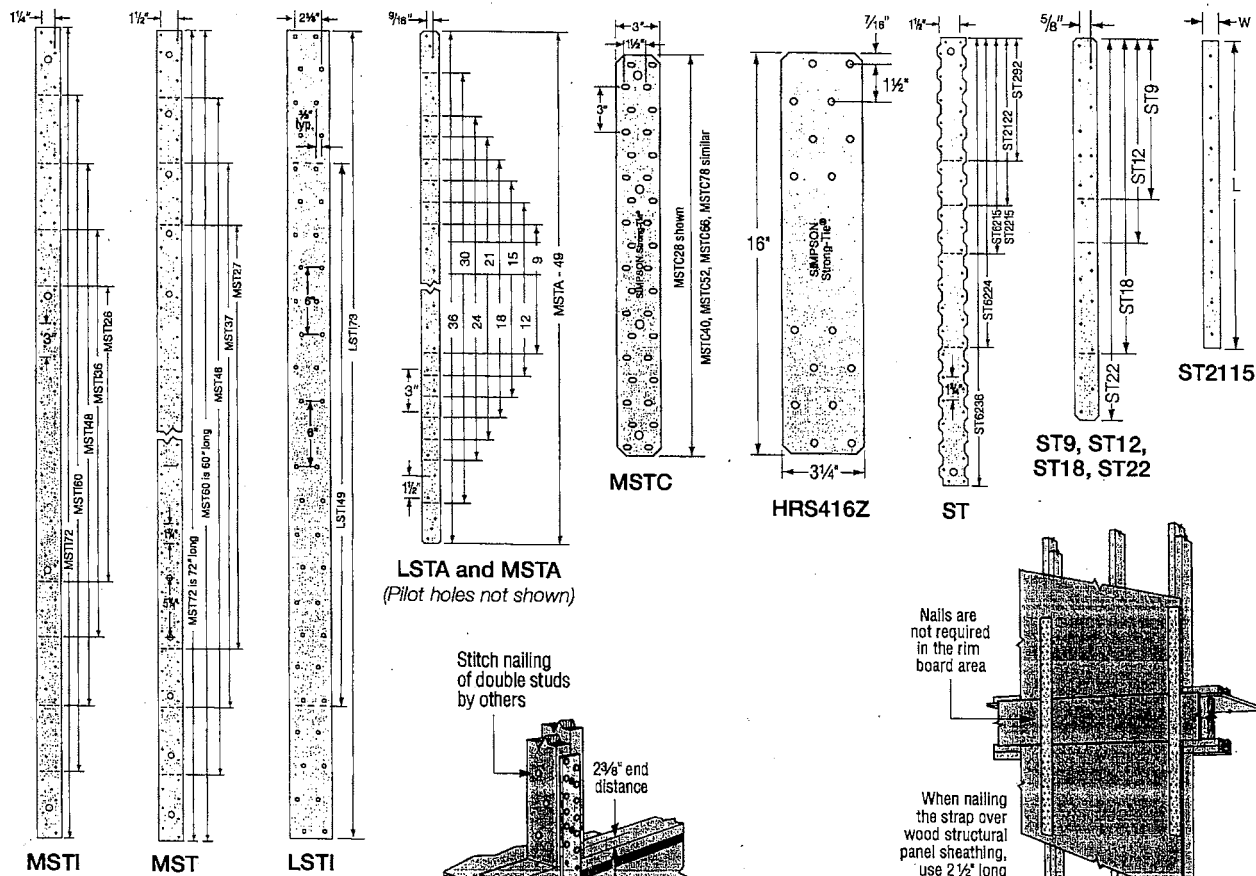
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

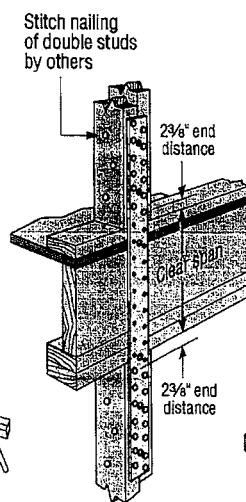
Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

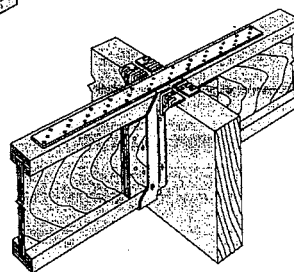
Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths



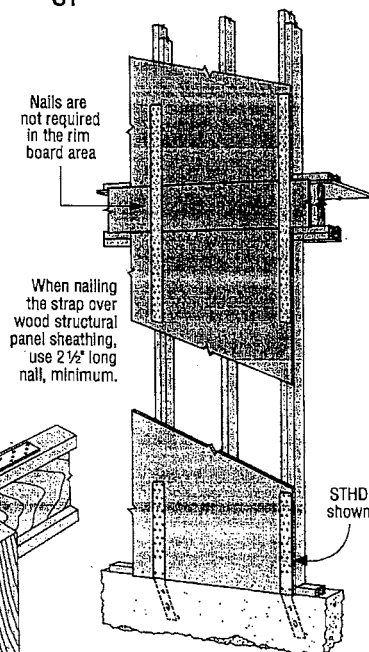
Typical LSTI Installation



Floor-to-Floor Tie Installation Showing a Clear Span



Typical MSTI Installation (MIT hanger shown) LSTI similar



Typical Detail with Strap Installed over Wood Structural Panel Sheathing

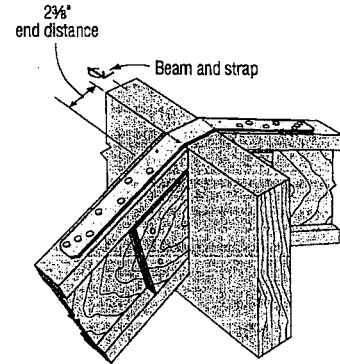
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

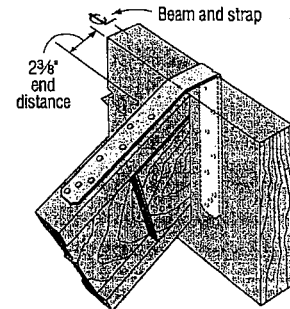


Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

	Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
			W	L		D.Fir-L		S-P-F	
						(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
						lb.	lb.	lb.	lb.
					kN	kN	kN	kN	
SS ● SS ● SS ● SS ● SS	LSTA9	20	1¼	9	(6) 10d	600	690	555	635
						2.67	3.07	2.47	2.82
	LSTA12		1¼	12	(8) 10d	800	920	735	845
						3.56	4.09	3.27	3.76
	LSTA15		1¼	15	(10) 10d	1000	1150	920	1060
						4.45	5.12	4.09	4.72
	LSTA18		1¼	18	(12) 10d	1200	1380	1105	1270
						5.34	6.14	4.92	5.65
	LSTA21		1¼	21	(14) 10d	1400	1610	1290	1485
						6.23	7.16	5.74	6.61
	LSTA24	1¼	24	(16) 10d	1600	1840	1475	1695	
					7.12	8.19	6.56	7.54	
	ST292	2⅝	9⅝	(8) 8d	585	675	535	615	
					2.60	3.00	2.38	2.74	
	ST2122	2⅝	12⅜	(12) 8d	940	1085	865	995	
					4.18	4.83	3.85	4.43	
	ST2115	¾	16⅝	(8) 8d	670	770	615	710	
					2.98	3.43	2.74	3.16	
	ST2215	2⅝	16⅝	(16) 8d	1335	1540	1235	1420	
					5.94	6.85	5.49	6.32	
SS ● SS ● SS ● SS	LSTA30	18	1¼	30	(20) 10d	2235	2465	2075	2385
						9.94	10.97	9.23	10.61
	LSTA36		1¼	36	(24) 10d	2465	2465	2465	2465
						10.97	10.97	10.97	10.97
	LSTI49		3¾	49	(32) 10d x 1½"	3115	3580	2852	3280
						13.86	15.93	12.69	14.59
	LSTI73		3¾	73	(48) 10d x 1½"	4670	5370	4280	4920
						20.77	23.89	19.04	21.89
	MSTA9		1¼	9	(6) 10d	670	770	625	715
						2.98	3.43	2.78	3.18
	MSTA12	1¼	12	(8) 10d	895	1030	830	955	
					3.98	4.58	3.69	4.25	
	MSTA15	1¼	15	(10) 10d	1120	1285	1040	1195	
					4.98	5.72	4.63	5.32	
	MSTA18	1¼	18	(12) 10d	1340	1545	1245	1430	
					5.96	6.87	5.54	6.36	
	MSTA21	1¼	21	(14) 10d	1565	1800	1455	1670	
					6.96	8.01	6.47	7.43	
MSTA24	1¼	24	(16) 10d	1790	2060	1660	1910		
				7.96	9.16	7.38	8.50		
SS ● SS	MSTA30	16	1¼	30	(20) 10d	2470	2840	2260	2595
						10.99	12.63	10.05	11.54
	MSTA36		1¼	36	(24) 10d	2965	3070	2710	3070
						13.19	13.66	12.06	13.66
	MSTA49		1¼	49	(28) 8d	2725	2725	2545	2725
						12.12	12.12	11.32	12.12
	ST6215		2⅝	16⅝	(16) 8d	1405	1615	1300	1500
						6.25	7.18	5.78	6.67
	ST6224		2⅝	23⅝	(24) 8d	2305	2650	2155	2475
						10.25	11.79	9.59	11.01
	ST9	1¼	9	(6) 8d	525	605	490	560	
					2.34	2.69	2.18	2.49	
ST12	1¼	11⅝	(8) 8d	700	805	650	750		
				3.11	3.58	2.89	3.34		
ST18	1¼	17¾	(12) 8d	1050	1210	975	1125		
				4.67	5.38	4.34	5.00		
ST22	1¼	21⅝	(18) 8d	1580	1790	1465	1685		
				7.03	7.96	6.52	7.50		



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

Straps and Ties

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI**Strap Ties (cont.)**

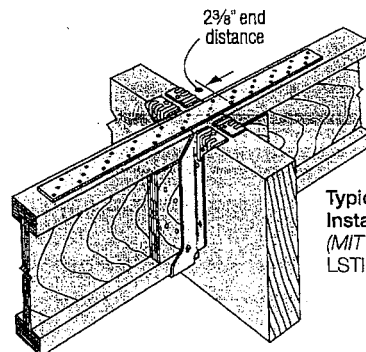
These products are available with additional corrosion protection. For more information, see p. 20.



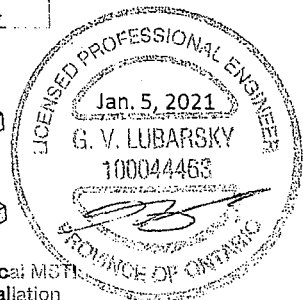
Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.)		Fasteners (Total)	Factored Tensile Resistance			
		W	L		D.Fir-L		S-P-F	
					(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52	14	3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66		3	65¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78	12	3	77¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2⅝	33⅝	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	10	2⅝	26	(22) 10d x 1½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2⅝	36	(32) 10d x 1½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2⅝	48	(44) 10d x 1½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2⅝	60	(56) 10d x 1½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2⅝	72	(68) 10d x 1½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2⅝	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2⅝	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2⅝	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z	10	3¼	16	(16) ¼" x 1½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60		2⅝	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72	10	2⅝	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. Nails: 10d = 0.148" dia. x 3" long, 10d x 1 1/2" = 0.148" dia. x 1 1/2" long, 8d = 0.131" dia. x 2 1/2" long. See pp. 22–23 for other nail sizes and information.

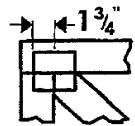


Typical MSTI
Installation
(MIT hanger shown)
LSTI similar

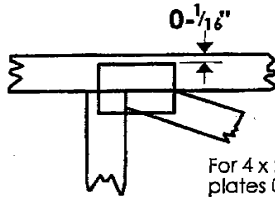


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

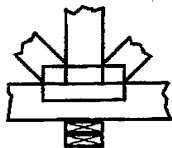
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



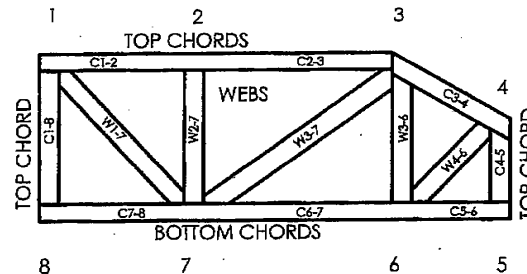
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.