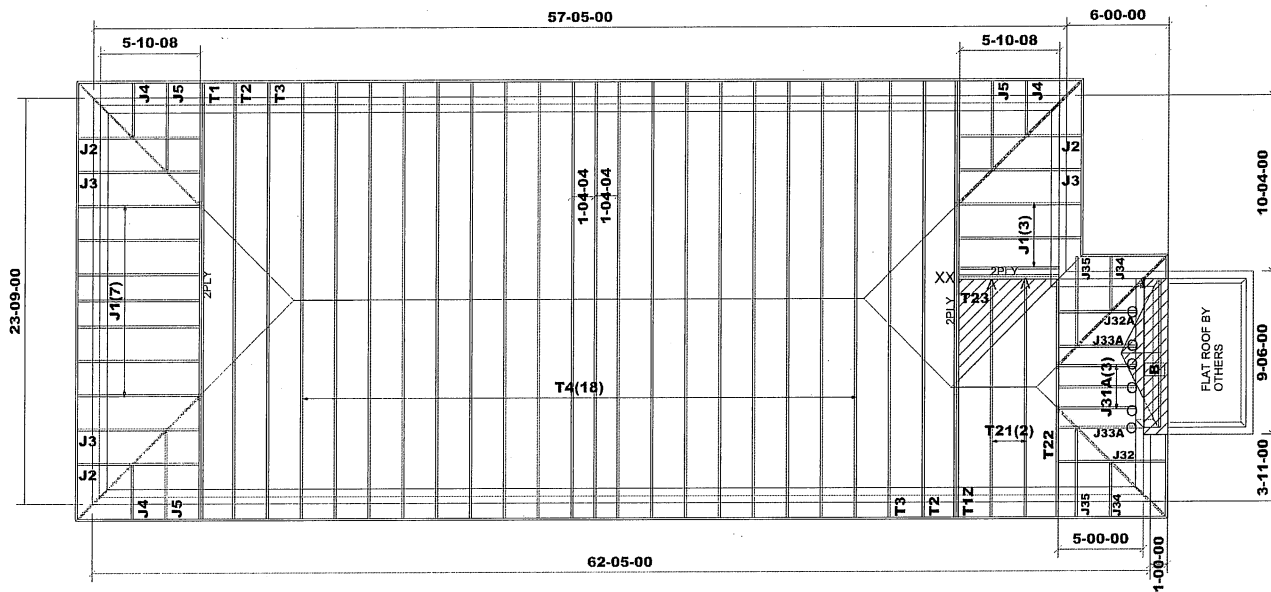




FINISH O.H: 12"
 Heel: R.T.M.C
 CLADDING ALLOWANCE: 5"
 2X6 EXTERIOR WALLS
 ASPHALT SHINGLES
 2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
 ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4" SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM TO THE RELEVANT SECTION OF THE LATEST EDITION OF THE ONTARIO BUILDING CODE (RESIDENTIAL PART 9)

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
 OCCUPANCY: RESIDENTIAL | PART: 9
 S_s = 33.4 psf | S_r = 8.4 psf

DESIGN LOADS:
 TC_{SL} = 26.7 psf
 TC_{DL} = 6.0 psf
 BC_{LL} = 0.0 psf
 BC_{DL} = 7.4 psf

HARDWARE
 LUS24(O)
 LUS26DS(V)
 LUS26-2(VV)
 HUS26-2(XX)

ALL ROOF PITCHES ARE 6/12 UNLESS OTHERWISE NOTED

 DENOTES:
 CONVENTIONAL FRAMING

DWG# TR22080160 TO TR22080168
 DWG# TR22080194 TO TR22080203



STRUCTURAL COMPONENTS ONLY
 DWG# TR22080206

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

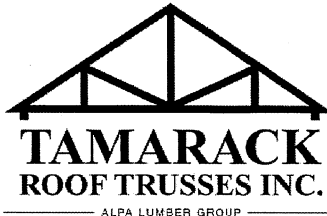
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



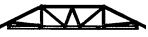
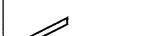
Job Track: 52956
 Layout ID: 427574
 Plan Log: 206398
 Builder / Location: GREENPARK HOMES / CAMBRIDGE
 Project: BARLASSINA
 Date: 8/02/22
 Designer: AT

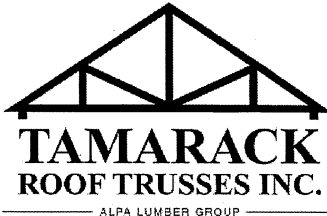
Model / Elevation: GARDEN 4 / 3
 Milek ver 8.5.3.233
 THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

M15038

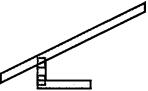
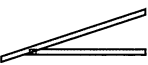
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
	Builder:	GREENPARK HOMES	PlanLog:	206398
	Project:	BARLASSINA	Layout ID:	427546
	Location:	CAMBRIDGE	Ref #	
Model:	GARDEN 4	Page:	1 of 2	
Lot #:		Date:	08-03-2022	
Elevation:	1	Designer:		
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	206.7 130.00		
	1	T2 Hip	6 /12	22-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	94.14 59.33		
	1	T3 Hip	6 /12	22-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	96.52 60.17		
	14	T4 Common	6 /12	22-11-00	6-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1280.52 807.33		
	1 3-ply	T5 Common Girder	6 /12	22-11-00	6-10-12	2 x 4 2 x 6	1-05-00 1-05-00	1-02-00 1-02-00	341.92 217.50		
	11	T6 Common	10 /12	17-02-00	8-09-09	2 x 4	1-03-08 1-03-08	1-07-11 1-07-12	910.59 595.83		
	1 2-ply	T7 Common Girder	12 /12	12-07-00	8-02-00	2 x 4 2 x 6	1-05-00 1-03-08	1-10-08 1-10-08	159.29 106.00		
	1	T8 Common	12 /12	12-07-00	8-02-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	66.06 43.33		
	2	T8S Roof Special	12 /12	12-07-00	8-02-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	136.81 89.00		
	2	G6 GABLE	10 /12	17-02-00	8-09-08	2 x 4	1-03-08 1-03-08	1-07-10 1-07-11	173.41 113.67		
	7	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		

DELIVERY SHIPLIST		
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER	Job Track: 52956
	Builder: GREENPARK HOMES	PlanLog: 206398
	Project: BARLASSINA	Layout ID: 427546
	Location: CAMBRIDGE	Ref #
	Model: GARDEN 4	Page: 2 of 2
	Lot #:	Date: 08-03-2022
	Elevation: 1	Designer:
		Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	5	J6 Monopitch	4 /12	6-05-08	2-10-11	2 x 4	1-03-08	3-15 2-05-12	83.95 53.33		
	2	J7 Monopitch	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	28.9 18.67		

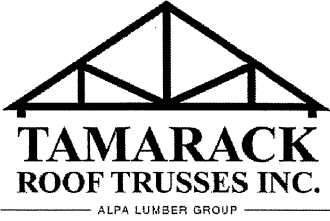
TOTAL # TRUSS= 61 TOTAL BFT OF ALL TRUSSES= 2422.16 BFT. TOTAL WEIGHT OF ALL TRSSES 3780.99 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
17	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 17

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: GARDEN 4
 Lot #:
 Elevation: 2

Job Track: 52956
 PlanLog: 206398
 Layout ID: 427556
 Ref #
 Page: 1 of 3
 Date: 08-03-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	206.7 130.00		
	1	T2 Hip	6 /12	22-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	94.14 59.33		
	1	T3 Hip	6 /12	22-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	96.52 60.17		
	17	T4 Common	6 /12	22-11-00	6-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1554.92 980.33		
	1	T9 Hip	10 /12	22-11-00	5-10-09	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	97.01 61.67		
	1	T10 Hip	10 /12	22-11-00	6-08-09	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	106.38 67.17		
	1 2-ply	T11 Hip Girder	10 /12	22-11-00	8-00-09	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	277.09 174.67		
	1	T11Z Hip	10 /12	22-11-00	8-00-09	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	138.54 87.33		
	1	T12 Half Hip	10 /12	10-01-00	7-09-14	2 x 4	1-03-08	1-07-10 7-09-14	58.25 38.00		
	1	T13 Half Hip	10 /12	10-01-00	6-05-09	2 x 4	1-03-08	1-07-10 6-05-09	52.87 34.17		
	1	T14 Half Hip	10 /12	10-01-00	5-01-09	2 x 4	1-03-08	1-07-10 5-01-09	57.89 39.00		
	1	T15 Half Hip	10 /12	10-01-00	3-10-14	2 x 4	1-03-08	1-07-10 3-10-14	52.14 36.17		
	1 2-ply	T16 Jack-Open Girder	8 /12	6-02-08	8-00-09	2 x 4 2 x 6	1-03-08	3-10-14 8-00-09	95.93 61.00		
	3	T17 Common	10 /12	12-07-00	6-10-09	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	181.21 118.50		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: GARDEN 4
 Lot #:
 Elevation: 2

Job Track: 52956
 PlanLog: 206398
 Layout ID: 427556
 Ref #
 Page: 2 of 3
 Date: 08-03-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T18 Hip Girder	10 / 12	12-07-00	5-07-02	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	136.85 91.33		
	1 2-ply	T19 Half Hip Girder	10 / 12	4-09-00	4-08-05	2 x 4 2 x 6		1-07-10 4-08-05	62.62 42.00		
	1	T20 Common	10 / 12	8-08-00	5-02-15	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	40.32 26.83		
	2	T20S Roof Special	10 / 12	8-08-00	5-02-15	2 x 4	1-05-00 1-03-08	1-07-10 1-07-10	91.38 63.33		
	7	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	2	J2 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	1	J21 Jack-Open	10 / 12	3-08-00	4-08-05	2 x 4	1-03-08	1-07-10 4-08-05	13.19 8.67		
	1	J22 Jack-Open	10 / 12	1-10-15	3-02-12	2 x 4	1-03-08 1-09-01	1-07-10 3-02-12	10.59 7.33		
	1	J23 Jack-Open	10 / 12	1-10-15	3-02-12	2 x 4	1-03-08 1-01	1-07-10 3-02-12	8.69 6.00		

TOTAL # TRUSS= 60

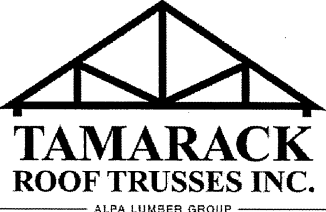
TOTAL BFT OF ALL TRUSSES= 2321

BFT.

TOTAL WEIGHT OF ALL TRSSES 3635.42 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
3	Hardware	LJS26DS	

	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
	Builder:	GREENPARK HOMES	PlanLog:	206398
	Project:	BARLASSINA	Layout ID:	427556
	Location:	CAMBRIDGE	Ref #	
	Model:	GARDEN 4	Page:	3 of 3
	Lot #:		Date:	08-03-2022
	Elevation:	2	Designer:	
		Sales Rep:	Rick DiCiano	

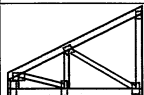
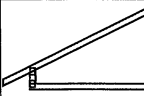
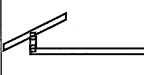
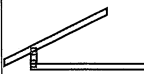
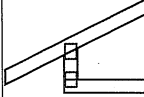
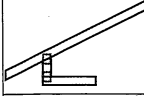
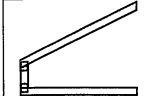
HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
1	Hardware	LUS26	

TOTAL NUMBER OF ITEMS= 10

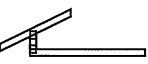
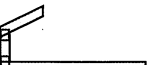
DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
	Builder:	GREENPARK HOMES	PlanLog:	206398
	Project:	BARLASSINA	Layout ID:	427574
	Location:	CAMBRIDGE	Ref #	
	Model:	GARDEN 4	Page:	1 of 2
	Lot #:		Date:	08-03-2022
	Elevation:	3	Designer:	
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	206.7 130.00		
	1 2-ply	T1Z Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	206.7 130.00		
	2	T2 Hip	6 /12	22-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	188.29 118.67		
	2	T3 Hip	6 /12	22-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	193.05 120.33		
	18	T4 Common	6 /12	22-11-00	6-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1646.39 1038.00		
	2	T21 Common	6 /12	12-07-00	4-03-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	101.11 64.00		
	1	T22 Hip Girder	6 /12	12-07-00	3-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	51.1 33.67		
	1 2-ply	T23 Jack-Open Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.83 37.67		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	3	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	34.75 22.00		
	3	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	42.4 26.00		
	3	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	21.06 14.00		
	3	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	28.71 18.00		
	3	J31A Jack-Open	6 /12	4-06-08	3-08-00	2 x 4		1-04-12 3-08-00	36.57 24.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: GARDEN 4 Lot #: Elevation: 3	Job Track: 52956 PlanLog: 206398 Layout ID: 427574 Ref # Page: 2 of 2 Date: 08-03-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	J32 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 3-02-09	1-02-00 2-00-12	10.58 6.67		
	1	J32A Jack-Open	6 /12	4-06-08	2-00-12	2 x 4		1-04-12 3-08-00	8.09 5.33		
	2	J33A Jack-Open	6 /12	4-06-08	3-00-12	2 x 4		1-04-12 3-08-00	21.28 13.33		
	2	J34 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J35 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		

TOTAL # TRUSS= 64 TOTAL BFT OF ALL TRUSSES= 1929.67 BFT. TOTAL WEIGHT OF ALL TRSSES 3055.74 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LJS26DS	
1	Hardware	HGUS26-2	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 9

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427546	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:27 2022 Page 2
ID:2Omi0yVrZwt1e1q0AL1nK7z24Xg-Yth8b2ozG9AyPjnYRDHpgeXwv2QVaCIRbhHA51yrvTY

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	14-11-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
S	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	11-5-8	-21	-21	---	FRONT	VERT	TOTAL	---	C1
W	14-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	18-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	20-11-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

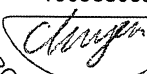
CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER

8/02/22

C. M. HEYENS

100505065



PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR22080165 PG 2/2

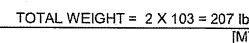
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:40 2022 Page 1

ID:2OmioVrZwt1e1q0AL1nK7z24Xg-xi5?YynDbr7et3WS9enePQ6OsjKqyfrsBCKqkCyrWw9

The diagram shows a roof truss structure with the following dimensions and labels:

- Top chord dimensions: 1-3-8, 0-0, 5-10-8, 5-10-8, 5-7-0, 11-5-8, 1-9-4, 13-2-12, 1-9-0, 14-11-12, 2-0-12, 17-0-8, 22-11-0, 24-2-8, 5-10-8, 1-3-8.
- Bottom chord dimensions: 1-3-8, 5-10-8, 5-7-0, 1-9-4, 13-2-12, 1-9-0, 2-0-12, 17-0-8, 22-11-0, 24-2-8, 5-10-8, 1-3-8.
- Scale: 1/4" = 1'-0"



TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 29	-95.2	-95.2	0.07 (1)	10.00	M-C	-246 / 14	0.03 (1)
B-C	-3071 / 0	-95.2	-95.2	0.40 (1)	4.85	I-E	-255 / 81	0.03 (1)
C-D	-4052 / 0	-95.2	-95.2	0.45 (1)	6.25	B-M	0 / 2768	0.34 (1)
D-O	-4537 / 0	-95.2	-95.2	0.48 (1)	4.03	I-F	0 / 3546	0.44 (1)
Q-P	-4537 / 0	-95.2	-95.2	0.48 (1)	4.03	L-D	-1142 / 0	0.16 (1)
P-E	-4537 / 0	-95.2	-95.2	0.48 (1)	4.03	D-K	0 / 295	0.04 (1)
E-F	-3934 / 0	-95.2	-95.2	0.45 (1)	4.35	K-E	0 / 1366	0.17 (1)
F-G	0 / 29	-95.2	-95.2	0.07 (1)	10.00	C-L	0 / 1747	0.22 (1)
N-B	-2141 / 0	0.0	0.0	0.08 (1)	7.81			
H-F	-2628 / 0	0.0	0.0	0.09 (1)	7.81			
N-M	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
M-L	0 / 2739	-18.5	-18.5	0.20 (1)	10.00			
L-K	0 / 4438	-18.5	-18.5	0.34 (1)	10.00			
K-Q	0 / 3510	-18.5	-18.5	0.28 (1)	10.00			
Q-R	0 / 3510	-18.5	-18.5	0.28 (1)	10.00			
R-J	0 / 3510	-18.5	-18.5	0.28 (1)	10.00			
J-I	0 / 3510	-18.5	-18.5	0.28 (1)	10.00			
I-S	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
S-T	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
T-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

JT	LOC.	LCI	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-0-8	-370	-370	---	FRONT	VERT	TOTAL	---	C1
I	16-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
K	12-8-8	-800	-800	---	FRONT	VERT	TOTAL	---	C1
O	13-2-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
P	14-11-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
Q	13-2-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	14-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	18-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	20-11-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.40 (I) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427574	T1Z	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:40 2022 Page 2
ID:2Omi0yVrZwt1e1q0AL1nK7z24Xg-xl5?YynDbr7et3WS9enePQ6OsiQkyfRsBCKqkCyrvW9

PLATES (table is in inches)

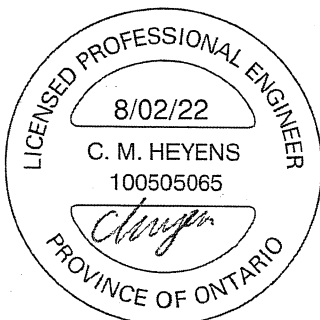
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

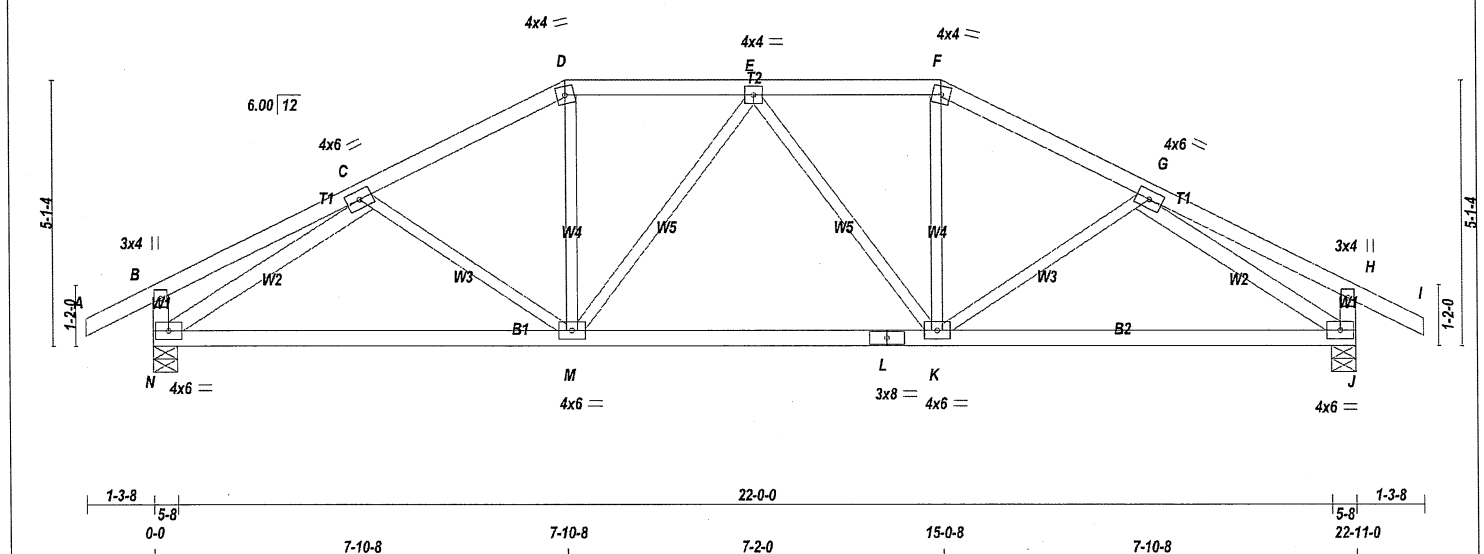
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22080200 PG 2/2

JOB NAME 427546	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:28 2022 Page 1
ID:2Om0yVrZwt1e1q0AL1nK7z24Xg-03EWpOpb1Tlp1tMk?wo2Ds3ATSkFJcjaqL0jdTyrVTX
Scale = 1:42.5



LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - C	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMVWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMVWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
N	1432	0	1432	0	0	5-8	5-8	5-8	5-8
J	1432	0	1432	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
N	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0	
J	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX. (LBS)	CS1 (LC)	MAX. (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO						
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00	C-M	-131 / 21	0.05 (1)			
B-C	0 / 18	-95.2	-95.2	0.22 (1)	10.00	M-D	0 / 431	0.10 (1)			
C-D	-1606 / 0	-95.2	-95.2	0.20 (1)	5.03	M-E	-253 / 0	0.16 (1)			
D-E	-1428 / 0	-95.2	-95.2	0.17 (1)	5.30	E-K	-253 / 0	0.16 (1)			
E-F	-1428 / 0	-95.2	-95.2	0.17 (1)	5.30	K-F	0 / 431	0.10 (1)			
F-G	-1606 / 0	-95.2	-95.2	0.20 (1)	5.03	K-G	-131 / 21	0.05 (1)			
G-H	0 / 18	-95.2	-95.2	0.22 (1)	10.00	N-C	-1877 / 0	0.52 (1)			
H-I	0 / 29	-95.2	-95.2	0.12 (1)	10.00	G-J	-1877 / 0	0.52 (1)			
N-B	-277 / 0	0.0	0.0	0.03 (1)	7.81						
J-H	-277 / 0	0.0	0.0	0.03 (1)	7.81						
N-M	0 / 1525	-18.5	-18.5	0.39 (1)	10.00						
M-L	0 / 1576	-18.5	-18.5	0.38 (1)	10.00						
L-K	0 / 1576	-18.5	-18.5	0.38 (1)	10.00						
K-J	0 / 1525	-18.5	-18.5	0.39 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CS1: TC=0.22/1.00 (B-C:1), BC=0.39/1.00 (J-K:1), WB=0.52/1.00 (G-J:1), SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

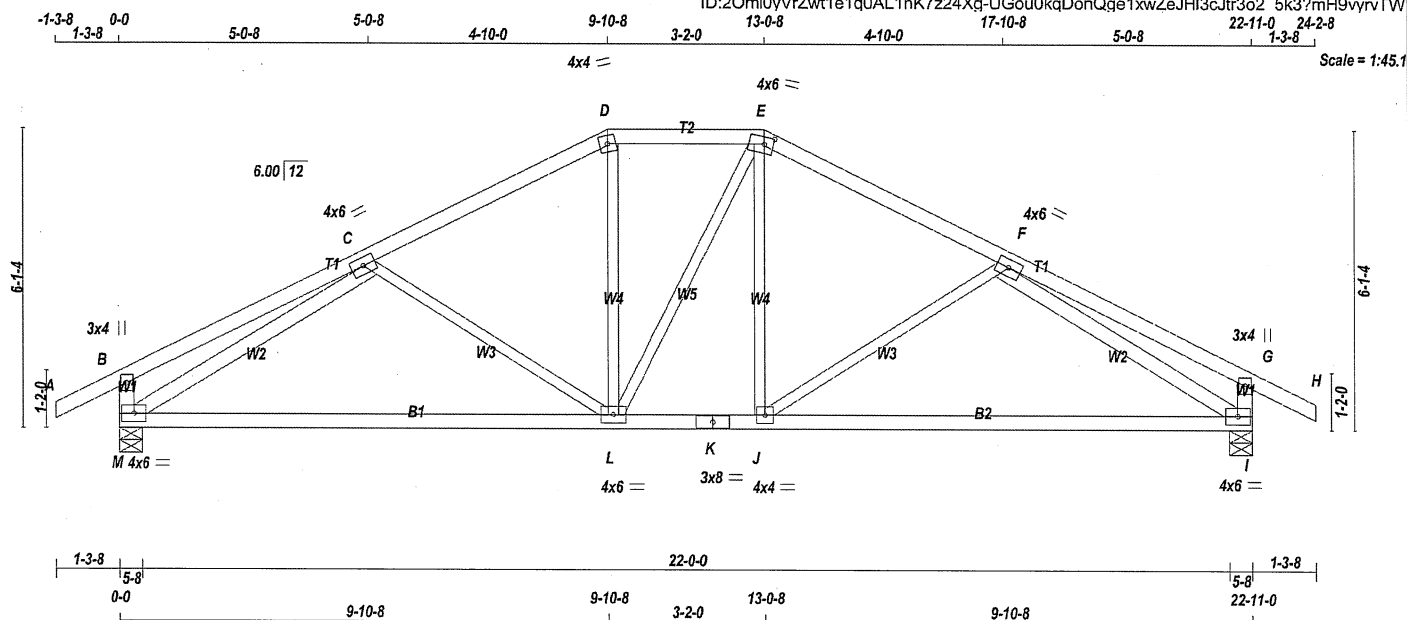
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080166



TOTAL WEIGHT = 97 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWV-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TTWV-m	MT20	4.0	6.0	1.75 2.25
F	TMWV-t	MT20	4.0	6.0	
G	TMV+p	MT20	3.0	4.0	
I	BMWV-1-t	MT20	4.0	6.0	
J	BMWV-t	MT20	4.0	4.0	
K	BS-t	MT20	3.0	8.0	
L	BMWVVV-t	MT20	4.0	6.0	
M	BMWV-1-t	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1432	0	1432	0	0	5-8	5-8
I	1432	0	1432	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SC
M	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0
I	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M. I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO		
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00	C-L	-323 / 0 0.21 (1)
B-C	0 / 24	-95.2	-95.2	0.37 (1)	10.00	L-D	0 / 314 0.07 (1)
C-D	-1460 / 0	-95.2	-95.2	0.31 (1)	5.08	L-E	0 / 1 0.00 (1)
D-E	-1289 / 0	-95.2	-95.2	0.14 (1)	5.15	J-E	0 / 313 0.07 (1)
E-F	-1460 / 0	-95.2	-95.2	0.31 (1)	5.08	J-F	-323 / 0 0.21 (1)
F-G	0 / 24	-95.2	-95.2	0.37 (1)	10.00	M-C	-1870 / 0 0.83 (1)
G-H	0 / 29	-95.2	-95.2	0.12 (1)	10.00	F-I	-1870 / 0 0.83 (1)
M-B	-311 / 0	0.0	0.0	0.03 (1)	7.81		
I-G	-311 / 0	0.0	0.0	0.03 (1)	7.81		
M-L	0 / 1552	-18.5	-18.5	0.49 (4)	10.00		
L-K	0 / 1288	-18.5	-18.5	0.49 (4)	10.00		
K-J	0 / 1288	-18.5	-18.5	0.49 (4)	10.00		
J-I	0 / 1551	-18.5	-18.5	0.49 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	26.7	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 861 (0.32")

CSI: TC=0.37/1.00 (F-G:1) , BC=0.49/1.00 (L-M:4)
 , WB=0.83/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.85 (K) (INPUT = 1.00)

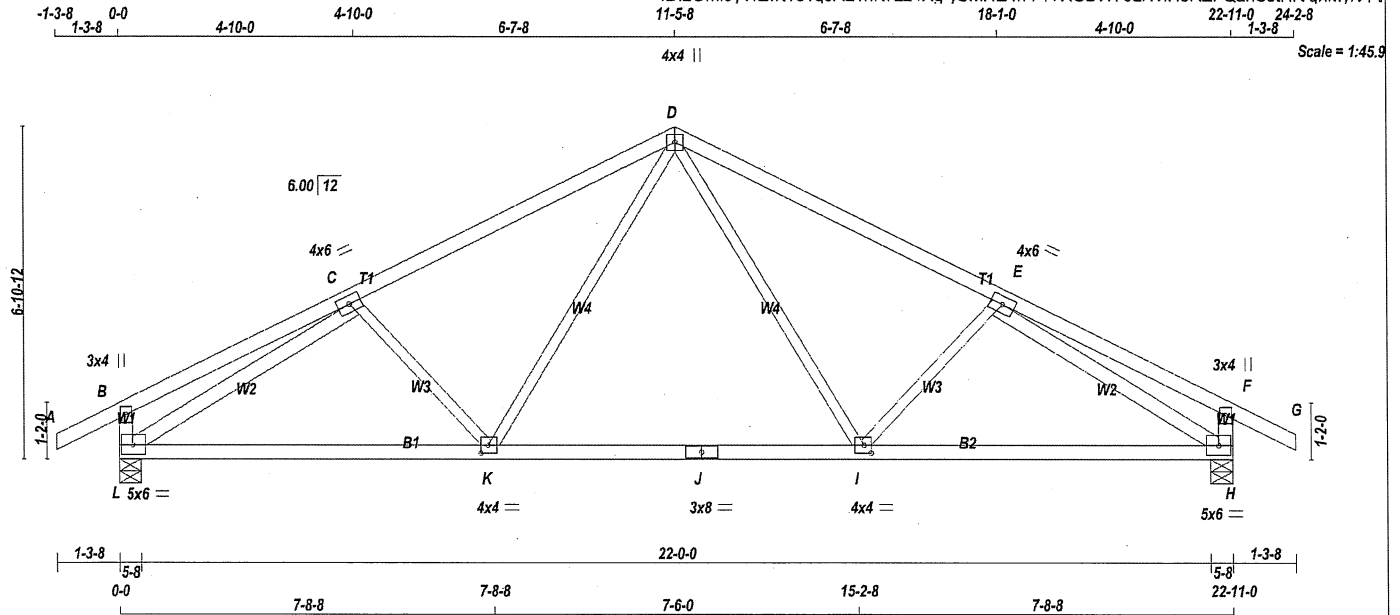


STRUCTURAL COMPONENT ONLY
DWG # TR22080167

JOB NAME 427546	TRUSS NAME T4	QUANTITY 14	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
---------------------------	-------------------------	-----------------------	-----------------	-------------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:30 2022 Page 1
ID:2Om0yVrZwt1e1q0AL1nK7z24Xg-ySMHE4rrY4YXGBW76LrWIH8RZFQanSjtHfVqhMynvTV



TOTAL WEIGHT = 14 X 91 = 1281 lb [M][F]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS				DESCR.	
EXCEPT	CHORDS	SIZE	LUMBER		
L - C	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMVW-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	4.0	2.00	1.75
L	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
L	1432	0	1432	0	0	5-8	5-8	5-8	5-8
H	1432	0	1432	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					DEAD	SOIL
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND		
L	1009	685 / 0	0 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0
H	1009	685 / 0	0 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 29	-95.2 -95.2	0.12 (1)	10.00	D-I	0 / 496	0.11 (1)
B-C	0 / 38	-95.2 -95.2	0.56 (1)	10.00	I-E	-368 / 0	0.12 (1)
C-D	-1566 / 0	-95.2 -95.2	0.57 (1)	4.58	K-D	0 / 496	0.11 (1)
D-E	-1566 / 0	-95.2 -95.2	0.57 (1)	4.58	C-K	-368 / 0	0.12 (1)
E-F	0 / 38	-95.2 -95.2	0.56 (1)	10.00	L-C	-1964 / 0	0.77 (1)
F-G	0 / 29	-95.2 -95.2	0.12 (1)	10.00	E-H	-1964 / 0	0.77 (1)
L-B	-268 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-268 / 0	0.0 0.0	0.03 (1)	7.81			
L-K	0 / 1623	-18.5 -18.5	0.39 (1)	10.00			
K-J	0 / 1127	-18.5 -18.5	0.34 (4)	10.00			
J-I	0 / 1127	-18.5 -18.5	0.34 (4)	10.00			
I-H	0 / 1623	-18.5 -18.5	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.14")

CSI: TC=0.57/1.00 (D-E:1), BC=0.39/1.00 (H-I:1), WB=0.77/1.00 (E-H:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.42 (E) (INPUT = 1.00)



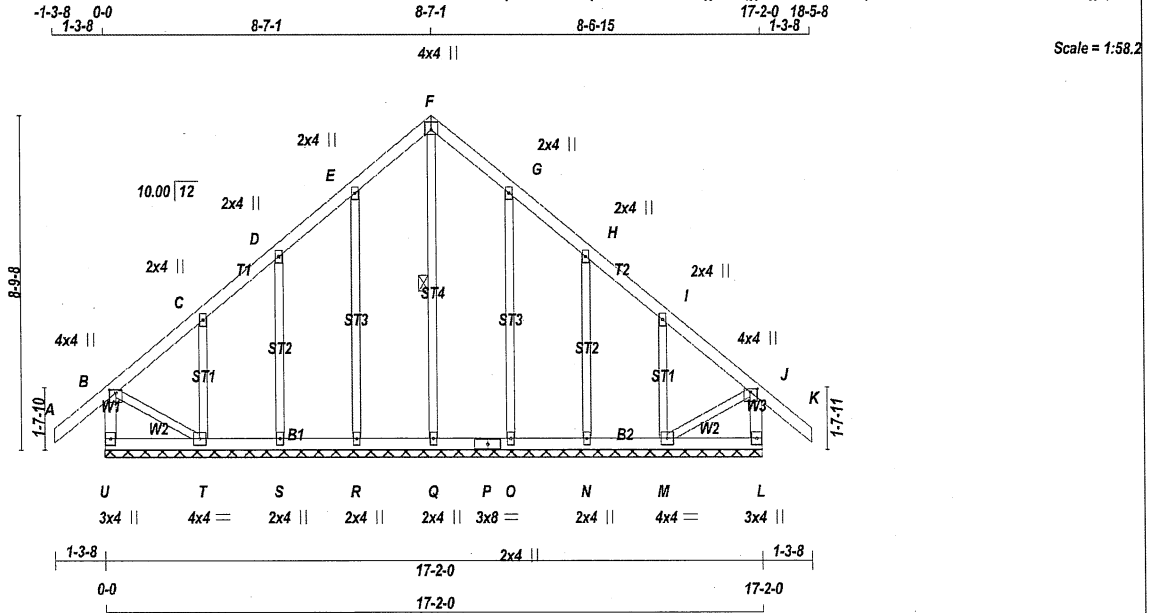
STRUCTURAL COMPONENT ONLY
DWG # TR22080168

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427546	G6	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:11 2022 Page 1

ID:2Omi0yVrZwt1e1q0AL1nK7z24Xg-S8ggEe?mYV247Ji6bdx9MemR Z7sGKbdO9 bgrrvt6



TOTAL WEIGHT = 2 X 87 = 173 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. U - B 2x4 DRY No.2 SPF A - F 2x4 DRY No.2 SPF F - K 2x4 DRY No.2 SPF L - J 2x4 DRY No.2 SPF U - P 2x4 DRY No.2 SPF P - L 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2'-0" OC.							DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4)							DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 26.7 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 40.1 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.13/1.00 (A-B:1), BC=0.03/1.00 (S-T:4), WB=0.19/1.00 (E-R:1), SSI=0.08/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.76 (F) (INPUT = 0.90) JSI METAL= 0.13 (C) (INPUT = 1.00)																																																																																																																																																																																															
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW+p MT20 4.0 4.0 1.00 2.00 C, D, E, G, H, I C TMW+w MT20 2.0 4.0 F TTW+p MT20 4.0 4.0 1.50 2.00 J TMVW+p MT20 4.0 4.0 1.00 2.00 L BMV1+p MT20 3.0 4.0 M BMVW1-t MT20 4.0 4.0 N, O, Q, R, S N BMV1+w MT20 2.0 4.0 P BS-t MT20 3.0 8.0 T BMVW1-t MT20 4.0 4.0 U BMV1+p MT20 3.0 4.0							<table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRACED LENGTH (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRACED LENGTH (LC)</th><th></th></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>U-B</td><td>-272 / 0</td><td>0.0 0.0</td><td>0.03 (1)</td><td>Q-F</td><td>-128 / 0</td><td>0.06 (1)</td><td></td></tr><tr><td>A-B</td><td>0 / 42</td><td>-95.2 -95.2</td><td>0.13 (1)</td><td>10.00</td><td>R-E</td><td>-220 / 0</td><td>0.19 (1)</td></tr><tr><td>B-C</td><td>-28 / 0</td><td>-95.2 -95.2</td><td>0.08 (1)</td><td>6.25</td><td>S-D</td><td>-165 / 0</td><td>0.07 (1)</td></tr><tr><td>C-D</td><td>-43 / 0</td><td>-95.2 -95.2</td><td>0.08 (1)</td><td>6.25</td><td>T-C</td><td>-246 / 0</td><td>0.06 (1)</td></tr><tr><td>D-E</td><td>-25 / 0</td><td>-95.2 -95.2</td><td>0.06 (1)</td><td>6.25</td><td>O-G</td><td>-220 / 0</td><td>0.19 (1)</td></tr><tr><td>E-F</td><td>-38 / 0</td><td>-95.2 -95.2</td><td>0.06 (1)</td><td>6.25</td><td>N-H</td><td>-165 / 0</td><td>0.07 (1)</td></tr><tr><td>F-G</td><td>-38 / 0</td><td>-95.2 -95.2</td><td>0.06 (1)</td><td>6.25</td><td>M-I</td><td>-245 / 0</td><td>0.06 (1)</td></tr><tr><td>G-H</td><td>-25 / 0</td><td>-95.2 -95.2</td><td>0.06 (1)</td><td>6.25</td><td>B-T</td><td>0 / 36</td><td>0.01 (1)</td></tr><tr><td>H-I</td><td>-43 / 0</td><td>-95.2 -95.2</td><td>0.08 (1)</td><td>6.25</td><td>M-J</td><td>0 / 36</td><td>0.01 (1)</td></tr><tr><td>I-J</td><td>-28 / 0</td><td>-95.2 -95.2</td><td>0.08 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>J-K</td><td>0 / 42</td><td>-95.2 -95.2</td><td>0.13 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>L-J</td><td>-272 / 0</td><td>0.0 0.0</td><td>0.03 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>U-T</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.03 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>T-S</td><td>0 / 26</td><td>-18.5 -18.5</td><td>0.03 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>S-R</td><td>0 / 23</td><td>-18.5 -18.5</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>R-Q</td><td>0 / 20</td><td>-18.5 -18.5</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>Q-P</td><td>0 / 20</td><td>-18.5 -18.5</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>P-O</td><td>0 / 20</td><td>-18.5 -18.5</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 23</td><td>-18.5 -18.5</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 26</td><td>-18.5 -18.5</td><td>0.03 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>M-L</td><td>0 / 0</td><td>-18.5 -18.5</td><td>0.03 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></table>							CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)		FR-TO		FROM TO		FR-TO				U-B	-272 / 0	0.0 0.0	0.03 (1)	Q-F	-128 / 0	0.06 (1)		A-B	0 / 42	-95.2 -95.2	0.13 (1)	10.00	R-E	-220 / 0	0.19 (1)	B-C	-28 / 0	-95.2 -95.2	0.08 (1)	6.25	S-D	-165 / 0	0.07 (1)	C-D	-43 / 0	-95.2 -95.2	0.08 (1)	6.25	T-C	-246 / 0	0.06 (1)	D-E	-25 / 0	-95.2 -95.2	0.06 (1)	6.25	O-G	-220 / 0	0.19 (1)	E-F	-38 / 0	-95.2 -95.2	0.06 (1)	6.25	N-H	-165 / 0	0.07 (1)	F-G	-38 / 0	-95.2 -95.2	0.06 (1)	6.25	M-I	-245 / 0	0.06 (1)	G-H	-25 / 0	-95.2 -95.2	0.06 (1)	6.25	B-T	0 / 36	0.01 (1)	H-I	-43 / 0	-95.2 -95.2	0.08 (1)	6.25	M-J	0 / 36	0.01 (1)	I-J	-28 / 0	-95.2 -95.2	0.08 (1)	6.25				J-K	0 / 42	-95.2 -95.2	0.13 (1)	10.00				L-J	-272 / 0	0.0 0.0	0.03 (1)	7.81				U-T	0 / 0	-18.5 -18.5	0.03 (4)	10.00				T-S	0 / 26	-18.5 -18.5	0.03 (4)	10.00				S-R	0 / 23	-18.5 -18.5	0.02 (4)	10.00				R-Q	0 / 20	-18.5 -18.5	0.02 (4)	10.00				Q-P	0 / 20	-18.5 -18.5	0.02 (4)	10.00				P-O	0 / 20	-18.5 -18.5	0.02 (4)	10.00				O-N	0 / 23	-18.5 -18.5	0.02 (4)	10.00				N-M	0 / 26	-18.5 -18.5	0.03 (4)	10.00				M-L	0 / 0	-18.5 -18.5	0.03 (4)	10.00			
CHORDS				WEBS																																																																																																																																																																																																									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)																																																																																																																																																																																																							
FR-TO		FROM TO		FR-TO																																																																																																																																																																																																									
U-B	-272 / 0	0.0 0.0	0.03 (1)	Q-F	-128 / 0	0.06 (1)																																																																																																																																																																																																							
A-B	0 / 42	-95.2 -95.2	0.13 (1)	10.00	R-E	-220 / 0	0.19 (1)																																																																																																																																																																																																						
B-C	-28 / 0	-95.2 -95.2	0.08 (1)	6.25	S-D	-165 / 0	0.07 (1)																																																																																																																																																																																																						
C-D	-43 / 0	-95.2 -95.2	0.08 (1)	6.25	T-C	-246 / 0	0.06 (1)																																																																																																																																																																																																						
D-E	-25 / 0	-95.2 -95.2	0.06 (1)	6.25	O-G	-220 / 0	0.19 (1)																																																																																																																																																																																																						
E-F	-38 / 0	-95.2 -95.2	0.06 (1)	6.25	N-H	-165 / 0	0.07 (1)																																																																																																																																																																																																						
F-G	-38 / 0	-95.2 -95.2	0.06 (1)	6.25	M-I	-245 / 0	0.06 (1)																																																																																																																																																																																																						
G-H	-25 / 0	-95.2 -95.2	0.06 (1)	6.25	B-T	0 / 36	0.01 (1)																																																																																																																																																																																																						
H-I	-43 / 0	-95.2 -95.2	0.08 (1)	6.25	M-J	0 / 36	0.01 (1)																																																																																																																																																																																																						
I-J	-28 / 0	-95.2 -95.2	0.08 (1)	6.25																																																																																																																																																																																																									
J-K	0 / 42	-95.2 -95.2	0.13 (1)	10.00																																																																																																																																																																																																									
L-J	-272 / 0	0.0 0.0	0.03 (1)	7.81																																																																																																																																																																																																									
U-T	0 / 0	-18.5 -18.5	0.03 (4)	10.00																																																																																																																																																																																																									
T-S	0 / 26	-18.5 -18.5	0.03 (4)	10.00																																																																																																																																																																																																									
S-R	0 / 23	-18.5 -18.5	0.02 (4)	10.00																																																																																																																																																																																																									
R-Q	0 / 20	-18.5 -18.5	0.02 (4)	10.00																																																																																																																																																																																																									
Q-P	0 / 20	-18.5 -18.5	0.02 (4)	10.00																																																																																																																																																																																																									
P-O	0 / 20	-18.5 -18.5	0.02 (4)	10.00																																																																																																																																																																																																									
O-N	0 / 23	-18.5 -18.5	0.02 (4)	10.00																																																																																																																																																																																																									
N-M	0 / 26	-18.5 -18.5	0.03 (4)	10.00																																																																																																																																																																																																									
M-L	0 / 0	-18.5 -18.5	0.03 (4)	10.00																																																																																																																																																																																																									

8/02/22

C. M. HEYENS

100505065

Chayen

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

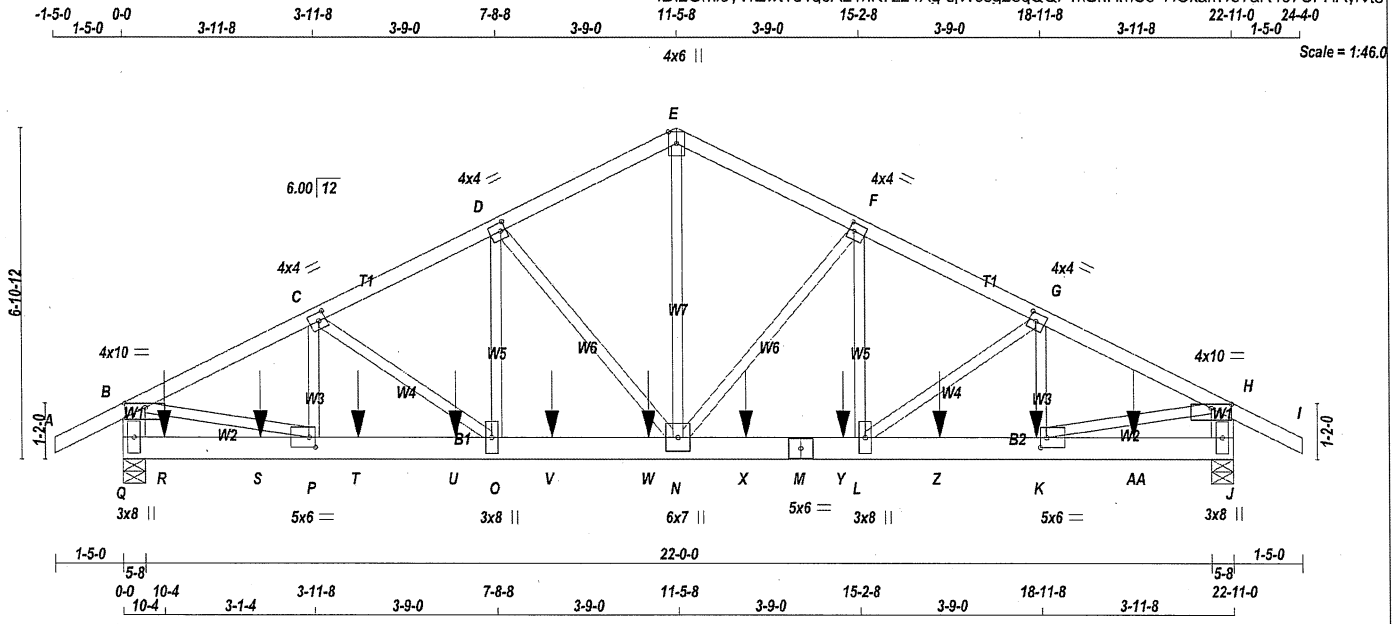
DWG # TR22080169



STRUCTURAL COMPONENT ONLY
DWG # TR22080169

JOB NAME 427546	TRUSS NAME T5	QUANTITY 1	PLY 3	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:14 2022 Page 1
ID:2Omi0yVrZwt1e1q0AL1nK7z24Xq-sjWosq2eqQQf mShHmUs HOxam1sTaR457CFHAYrvt3
Scale = 1:46.0



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
A - E 2x4 DRY No.2 SPF
E - I 2x4 DRY No.2 SPF
Q - B 2x6 DRY No.2 SPF
J - H 2x6 DRY No.2 SPF
Q - M 2x6 DRY No.2 SPF
M - J 2x6 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:
CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS
A - E 1 12 TOP
E - I 1 12 TOP
Q - B 2 12 TOP
J - H 2 12 TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS
Q - M 2 10 SIDE (284.5)
M - J 2 10 SIDE (458.6)
WEBS : (0.122"x3") SPIRAL NAILS
2x3 1 6
STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	
Q	7747	0	7747	0	5-8	5-8	
J	7124	0	7124	0	5-8	5-8	

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
Q	5443	3775 / 0	0 / 0	0 / 0	0 / 0	1667 / 0	0 / 0
J	5005	3471 / 0	0 / 0	0 / 0	0 / 0	1534 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-95.2	-95.2	0.05 (1)	10.00	N-E	0 / 6432 0.48 (1)
B-C	-10221 / 0	-95.2	-95.2	0.24 (1)	3.66	N-F	-2627 / 0 0.52 (1)
C-D	-9419 / 0	-95.2	-95.2	0.18 (1)	3.85	L-F	0 / 2478 0.19 (1)
D-E	-7552 / 0	-95.2	-95.2	0.14 (1)	4.25	L-G	-798 / 0 0.09 (1)
E-F	-7552 / 0	-95.2	-95.2	0.14 (1)	4.25	K-G	0 / 432 0.03 (1)
F-G	-9387 / 0	-95.2	-95.2	0.18 (1)	3.85	D-N	-2673 / 0 0.53 (1)
G-H	-10108 / 0	-95.2	-95.2	0.23 (1)	3.68	O-D	0 / 2527 0.19 (1)
H-I	0 / 32	-95.2	-95.2	0.05 (1)	10.00	C-O	-887 / 0 0.10 (1)
Q-B	-6684 / 0	0.0	0.0	0.14 (1)	6.85	P-C	0 / 522 0.04 (1)
J-H	-6613 / 0	0.0	0.0	0.14 (1)	6.88	B-P	0 / 9318 0.70 (1)
					K-H	0 / 9215 0.69 (1)	
Q-R	0 / 0	-18.5	-18.5	0.20 (1)	10.00		
R-S	0 / 0	-18.5	-18.5	0.20 (1)	10.00		
S-P	0 / 0	-18.5	-18.5	0.20 (1)	10.00		
P-T	0 / 9145	-18.5	-18.5	0.48 (1)	10.00		
T-U	0 / 9145	-18.5	-18.5	0.48 (1)	10.00		
U-O	0 / 9145	-18.5	-18.5	0.48 (1)	10.00		
O-V	0 / 8430	-18.5	-18.5	0.52 (1)	10.00		
V-W	0 / 8430	-18.5	-18.5	0.52 (1)	10.00		
W-N	0 / 8430	-18.5	-18.5	0.52 (1)	10.00		
N-X	0 / 8401	-18.5	-18.5	0.52 (1)	10.00		
X-M	0 / 8401	-18.5	-18.5	0.52 (1)	10.00		
M-Y	0 / 8401	-18.5	-18.5	0.52 (1)	10.00		
Y-L	0 / 8401	-18.5	-18.5	0.52 (1)	10.00		
L-Z	0 / 9044	-18.5	-18.5	0.52 (1)	10.00		
Z-K	0 / 9044	-18.5	-18.5	0.52 (1)	10.00		
K-AA	0 / 0	-18.5	-18.5	0.23 (1)	10.00		
AA-J	0 / 0	-18.5	-18.5	0.23 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)		FACE	DIR.	TYPE	HEEL	CONN.
JT	LOC.					
K	18-10-4	-765	---	BACK VERT	TOTAL	C1
R	10-4	-766	---	BACK VERT	TOTAL	C1
S	2-10-4	-765	---	BACK VERT	TOTAL	C1
T	4-10-4	-765	---	BACK VERT	TOTAL	C1
U	6-10-4	-765	---	BACK VERT	TOTAL	C1
V	8-10-4	-765	---	BACK VERT	TOTAL	C1
W	10-10-4	-765	---	BACK VERT	TOTAL	C1
X	12-10-4	-765	---	BACK VERT	TOTAL	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.24/1.00 (B-C:1), BC=0.52/1.00 (K-L:1), WB=0.70/1.00 (B-P:1), SSI=0.28/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.69 (F) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080172 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427546	T5	1	3	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:14 2022 Page 2
ID:2Omi0yVrZwt1e1q0AL1nK7z24Xg-sjWosg2eqQQf mShHmUs HOxam1sTaR457CFHAyrt3

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	10.0	1.00	5.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	4.0	10.0	1.00	5.00
J	BMV1+p	MT20	3.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	1.50
L	BMWW+t	MT20	3.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	6.0	7.0		
O	BMWW+t	MT20	3.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	1.50
Q	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	14-10-4	-765	-765	---	BACK	VERT	TOTAL	---	C1
Z	16-10-4	-765	-765	---	BACK	VERT	TOTAL	---	C1
AA	20-10-4	-765	-765	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



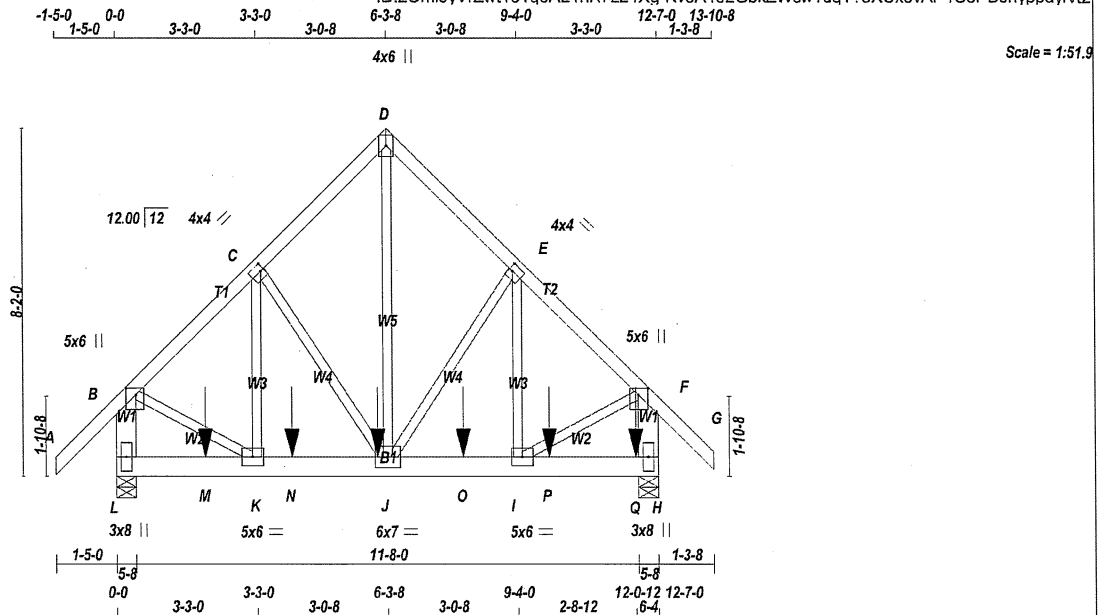
STRUCTURAL COMPONENT ONLY
DWG # TR22080172 PG 2/2

STRUCTURAL COMPONENT ONLY
DWG # TR22080173

JOB NAME 427546	TRUSS NAME T7	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
--------------------	------------------	---------------	----------	------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:15 2022 Page 1
ID:2Om0yVrZwt1e1q0AL1nK7z24Xg-Kv3A402GbKZWcw1uqT?5XUx6vAP4C5PDJnyppdyrvt2



Scale = 1:51.9

TOTAL WEIGHT = 2 X 80 = 159 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY No.2	SPF			
D - G	2x4 DRY No.2	SPF			
L - B	2x6 DRY No.2	SPF			
H - F	2x6 DRY No.2	SPF			
L - H	2x6 DRY No.2	SPF			

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1 12	TOP	
D-G 1 12	TOP	
L-B 2 12	TOP	
H-F 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-H 2 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.75	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	3.0	8.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	6.0	7.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	3708	0	3708	0
H	4541	0	4541	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2604	1811 / 0	0 / 0	0 / 0	0 / 0	793 / 0	0 / 0
H	3189	2216 / 0	0 / 0	0 / 0	0 / 0	974 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX	CS1 (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
FR-TO								FR-TO				
A-B	0 / 51	-95.2	-95.2	0.09	(1)	10.00	J-D	0 / 3206	0.40	(1)		
B-C	-3049 / 0	-95.2	-95.2	0.11	(1)	5.21	J-E	-950 / 0	0.25	(1)		
C-D	-2456 / 0	-95.2	-95.2	0.08	(1)	5.69	I-E	0 / 972	0.12	(1)		
D-E	-2456 / 0	-95.2	-95.2	0.08	(1)	5.69	C-J	-811 / 0	0.22	(1)		
E-F	-3157 / 0	-95.2	-95.2	0.11	(1)	5.13	K-C	0 / 779	0.10	(1)		
F-G	0 / 47	-95.2	-95.2	0.07	(1)	10.00	B-K	0 / 2370	0.29	(1)		
L-B	-3405 / 0	0.0	0.0	0.13	(1)	7.55	I-F	0 / 2453	0.30	(1)		
H-F	-3502 / 0	0.0	0.0	0.14	(1)	7.48						
L-M	0 / 0	-18.5	-18.5	0.22	(1)	10.00						
M-K	0 / 0	-18.5	-18.5	0.22	(1)	10.00						
K-N	0 / 2167	-18.5	-18.5	0.29	(1)	10.00						
N-J	0 / 2167	-18.5	-18.5	0.29	(1)	10.00						
J-O	0 / 2242	-18.5	-18.5	0.33	(1)	10.00						
O-I	0 / 2242	-18.5	-18.5	0.33	(1)	10.00						
I-P	0 / 0	-18.5	-18.5	0.20	(1)	10.00						
P-Q	0 / 0	-18.5	-18.5	0.20	(1)	10.00						
Q-H	0 / 0	-18.5	-18.5	0.20	(1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	6-0-12	-765	-765	---	TOP	VERT	TOTAL	---	C1
M	2-0-12	-765	-765	---	TOP	VERT	TOTAL	---	C1
N	4-0-12	-765	-765	---	TOP	VERT	TOTAL	---	C1
O	8-0-12	-765	-765	---	TOP	VERT	TOTAL	---	C1
P	10-0-12	-765	-765	---	TOP	VERT	TOTAL	---	C1
Q	12-0-12	-768	-768	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (F-H:1), BC=0.33/1.00 (I-J:1), WB=0.40/1.00 (D-J:1), SSI=0.40/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (D) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080174

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427546	T7	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:15 2022 Page 2
ID:2Omi0yVrZwt1e1g0AL1nK7z24Xg-Kv3A402GbKZWcw1uqT?5XUx6vAP4C5PDJnyppdyrvt2

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	8.0		

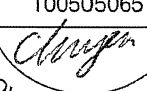
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER

8/02/22

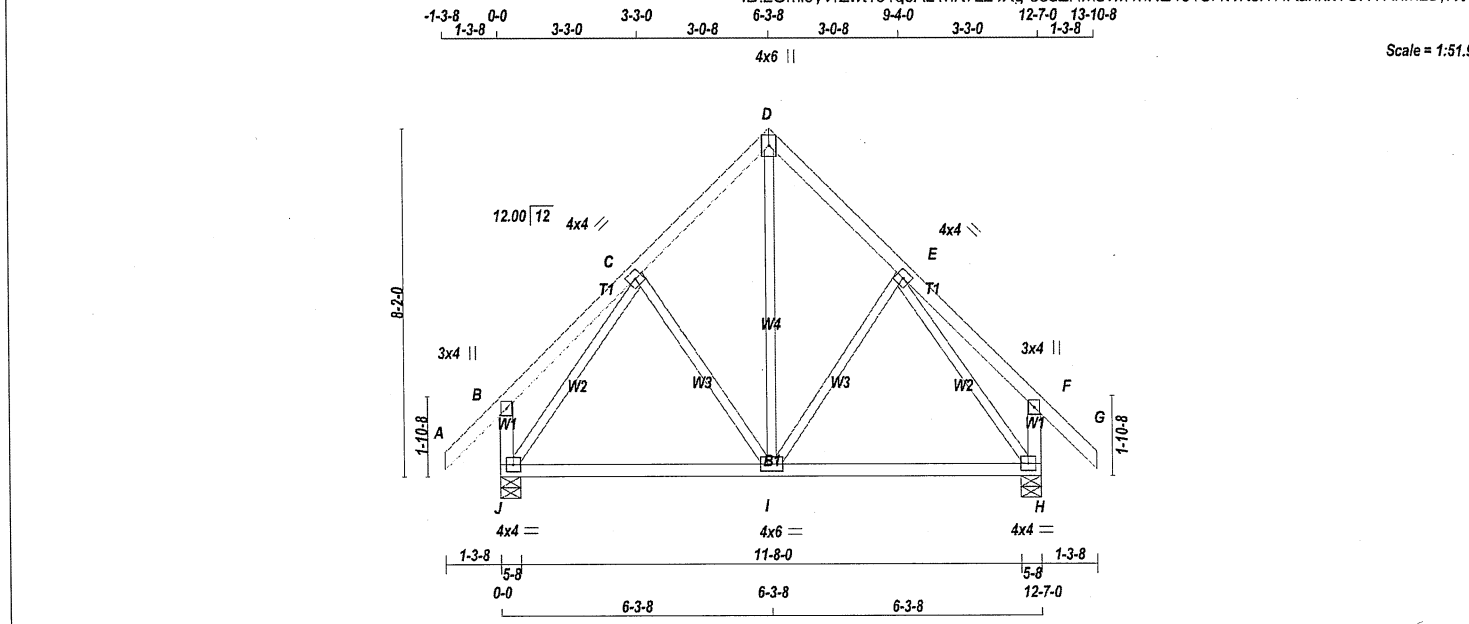
C. M. HEYENS

100505065



PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR22080174 PG 2/2



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS				DESIGN CRITERIA			
A - D	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:		
D - G	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	26.7	PSF
J - B	2x4	DRY	No.2	SPF	JT VERT	DOWN	IN-SX	IN-SX	DL =	6.0	PSF
H - F	2x4	DRY	No.2	SPF	J H	848 0	848 0	5-8 5-8	BOT CH. LL =	0.0	PSF
J - H	2x4	DRY	No.2	SPF	H	848 0	848 0	5-8 5-8	DL =	7.4	PSF
									TOTAL LOAD =	40.1	PSF
ALL WEBS	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS				SPACING = 24.0 IN./C		
EXCEPT					1ST LCASE	MAX./MIN. COMPONENT REACTIONS			THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015		
DRY: SEASONED LUMBER.					JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL			THIS DESIGN COMPLIES WITH:		
					J	596 411 / 0 0 / 0 0 / 0 0 / 0 185 / 0 0 / 0			- PART 9 OF CBC 2018 , ABC 2019		
					H	596 411 / 0 0 / 0 0 / 0 0 / 0 185 / 0 0 / 0			- PART 9 OF OBC 2012 (2019 AMENDMENT)		
									- CSA 086-14		
									- TPIC 2014		
									(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD		
									ALLOWABLE DEFL.(LL)= L/360 (0.42")		
									CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")		
									ALLOWABLE DEFL.(TL)= L/360 (0.42")		
									CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")		
									CSI: TC=0.15/1.00 (E-F:1) , BC=0.24/1.00 (I-J:4) , WB=0.35/1.00 (E-H:1) , SSI=0.10/1.00 (D-E:1)		
									DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10		
									COMPANION LIVE LOAD FACTOR = 1.00		
									TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .		
									NAIL VALUES		
									PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)		
									MAX MIN MAX MIN MAX MIN		
									MT20 650 371 1747 788 1987 1873		
									PLATE PLACEMENT TOL. = 0.250 inches		
									PLATE ROTATION TOL. = 5.0 Deg.		
									JSI GRIP= 0.87 (C) (INPUT = 0.90)		
									JSI METAL= 0.23 (C) (INPUT = 1.00)		

STRUCTURAL COMPONENT ONLY
DWG # TR22080175

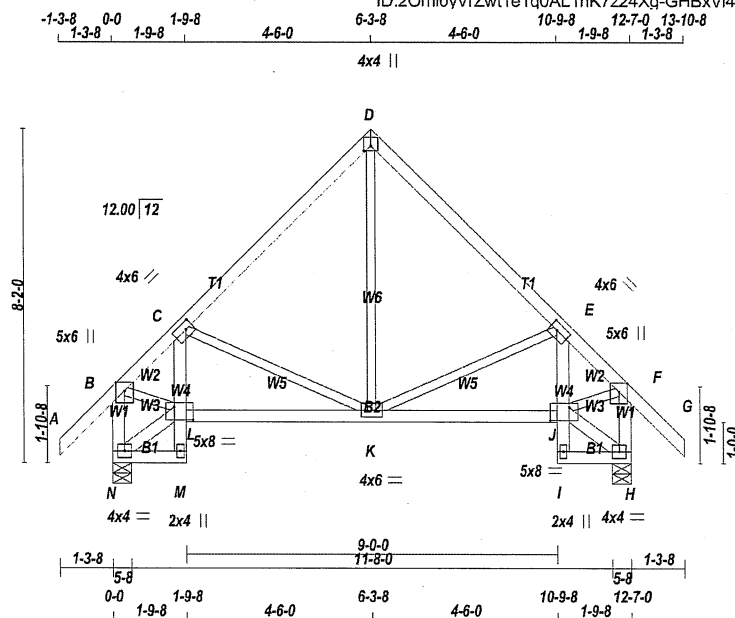
TOTAL WEIGHT = 66 lb

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427546	T8S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:17 2022 Page 1

ID:2Om0yVrZwt1e1q0AL1nK7z24Xg-GHBxVi4X7LpEsEAGyu1Zcv0Rq 8Fg39Wn5RvuVyrvt0



Scale = 1:54.1

TOTAL WEIGHT = 2 X 68 = 137 lb [M][F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - D	2x3	DRY	No.2	SPF
K - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TMVW-t	MT20	4.0	6.0	2.00	2.25
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMVW-t	MT20	4.0	6.0	2.00	2.25
F	TMVW+p	MT20	5.0	6.0	Edge	2.50
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW+w	MT20	2.0	4.0		
J	BMVWVW-t	MT20	5.0	8.0	3.75	5.50
K	BMVWVW-t	MT20	4.0	6.0		
L	BMVWVW-t	MT20	5.0	8.0	3.75	5.50
M	BMVW+w	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	848	0	848	0	5-8	5-8
H	848	0	848	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOOSE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
N	596	411 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	596	411 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 47	-95.2	-95.2 0.13 (1)	M-L	0 / 17	0.03 (1)	
B-C	-725 / 0	-95.2	-95.2 0.19 (1)	L-C	-69 / 16	0.02 (1)	
C-D	-484 / 0	-95.2	-95.2 0.25 (1)	I-J	0 / 17	0.03 (1)	
D-E	-484 / 0	-95.2	-95.2 0.25 (1)	J-E	-69 / 16	0.02 (1)	
E-F	-725 / 0	-95.2	-95.2 0.19 (1)	K-D	0 / 280	0.06 (1)	
F-G	0 / 47	-95.2	-95.2 0.13 (1)	K-E	-246 / 0	0.11 (1)	
N-B	-823 / 0	0.0	0.0 0.09 (1)	C-K	-246 / 0	0.11 (1)	
H-F	-823 / 0	0.0	0.0 0.09 (1)	N-L	-18 / 0	0.00 (1)	
				B-L	0 / 552	0.12 (1)	
N-M	0 / 15	-18.5	-18.5 0.02 (4)	J-H	-18 / 0	0.00 (1)	
L-K	0 / 546	-18.5	-18.5 0.16 (4)	J-F	0 / 552	0.12 (1)	
K-J	0 / 546	-18.5	-18.5 0.16 (4)				
I-H	0 / 15	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.25/1.00 (D-E:1), BC=0.16/1.00 (K-L:4), WB=0.12/1.00 (F-J:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)

JSI METAL= 0.26 (B) (INPUT = 1.00)

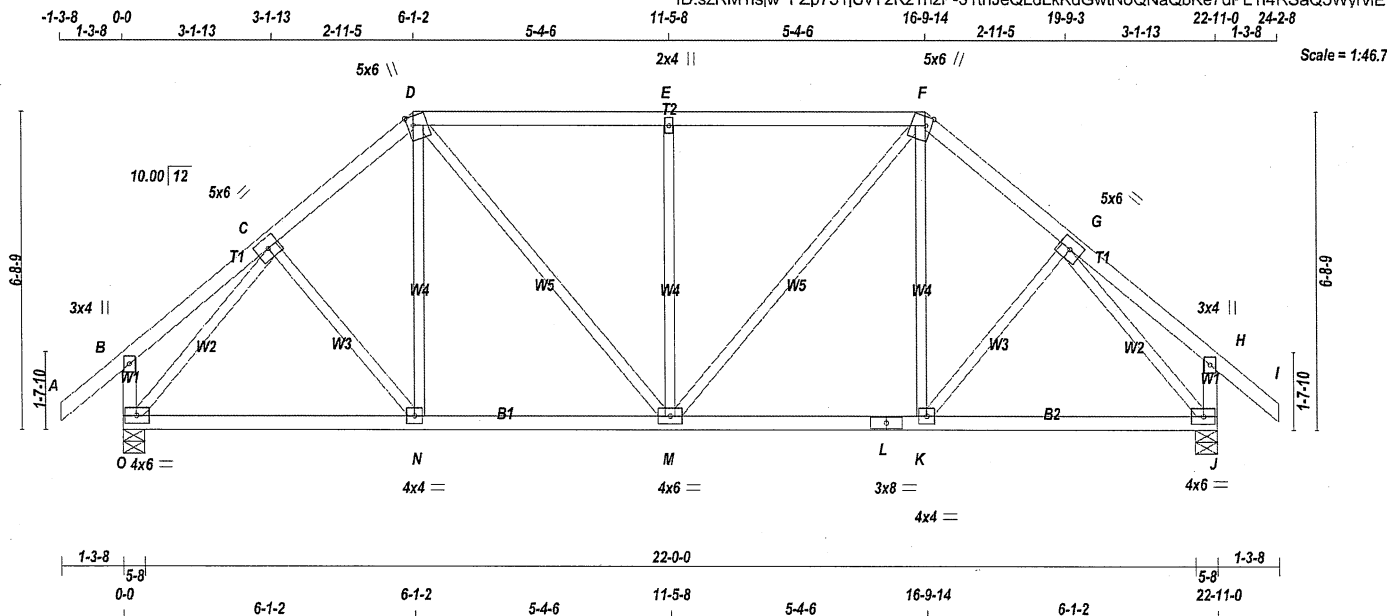


STRUCTURAL COMPONENT ONLY
DWG # TR22080176

JOB NAME 427556	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	-------------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:47 2022 Page 1
ID: szRM1Isjw FZp751jUvY2Kz1nzP-31tnJeQLdKdGwtNoQNaQbKe7dPL1I4RSaQ5WyrviE



TOTAL WEIGHT = 106 lb
(M/F)

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	TOP CH.	LL = 26.7 PSF
D - F	2x4	DRY	No.2	SPF	O	1435	0	1435	0	DL =	6.0 PSF
F - I	2x4	DRY	No.2	SPF	J	1435	0	1435	0	BOT CH.	LL = 0.0 PSF
O - B	2x4	DRY	No.2	SPF						DL =	7.4 PSF
J - H	2x4	DRY	No.2	SPF						TOTAL LOAD	= 40.1 PSF
O - L	2x4	DRY	No.2	SPF							
L - J	2x4	DRY	No.2	SPF							

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMVWW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
O	1010	687 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0		
J	1010	687 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2 -95.2 0.13 (1)	10.00	C-N	0 / 46	0.02 (4)	
B-C	0 / 20	-95.2 -95.2 0.13 (1)	10.00	N-D	0 / 106	0.04 (4)	
C-D	-1221 / 0	-95.2 -95.2 0.11 (1)	5.69	D-M	0 / 460	0.10 (1)	
D-E	-1217 / 0	-95.2 -95.2 0.37 (1)	5.36	M-E	-624 / 0	0.47 (1)	
E-F	-1217 / 0	-95.2 -95.2 0.37 (1)	5.36	M-F	0 / 460	0.10 (1)	
F-G	-1221 / 0	-95.2 -95.2 0.11 (1)	5.69	K-F	0 / 106	0.04 (4)	
G-H	0 / 20	-95.2 -95.2 0.13 (1)	10.00	K-G	0 / 46	0.02 (4)	
H-I	0 / 42	-95.2 -95.2 0.13 (1)	10.00	O-C	-1457 / 0	0.55 (1)	
O-B	-245 / 0	0.0 0.0 0.03 (1)	7.81	G-J	-1457 / 0	0.55 (1)	
J-H	-245 / 0	0.0 0.0 0.03 (1)	7.81				
O-N	0 / 904	-18.5 -18.5 0.23 (1)	10.00				
N-M	0 / 922	-18.5 -18.5 0.24 (4)	10.00				
M-L	0 / 922	-18.5 -18.5 0.24 (4)	10.00				
L-K	0 / 922	-18.5 -18.5 0.24 (4)	10.00				
K-J	0 / 904	-18.5 -18.5 0.23 (1)	10.00				

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.37/1.00 (D-E:1), BC=0.24/1.00 (M-N:4), WB=0.55/1.00 (C-O:1), SI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.33 (L) (INPUT = 1.00)



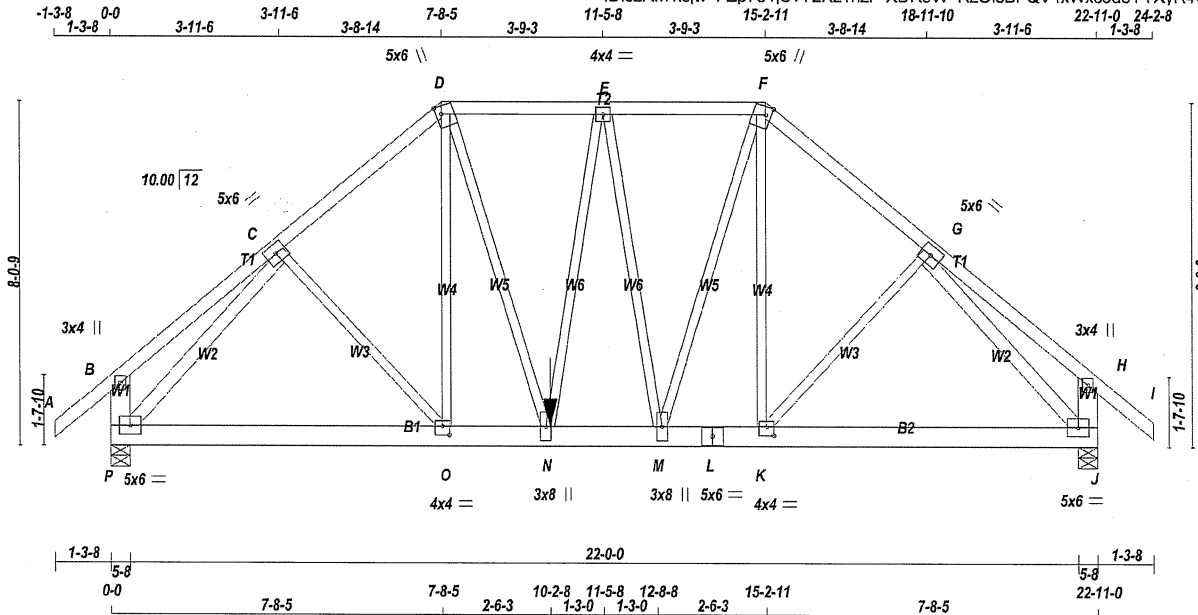
STRUCTURAL COMPONENT ONLY
DWG # TR22080181

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T11	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:48 2022 Page 1

ID:szRM1lsjw FZp751jUvY2Kz1nzP-XDR9W RzOfsBFQV4xWxc6d8YXYR4QdDg6J eyyrvID



TOTAL WEIGHT = 2 X 139 = 277 lb
[M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
P - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-F 1	12	TOP
F-I 1	12	TOP
P-B 2	12	TOP
J-H 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-L 2	12	SIDE(183.1)
L-J 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
N-E 1	6	SIDE(553.2)
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
P	3004	0	3004	0
J	2687	0	2687	0

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
P	2112	1457 / 0	0 / 0	0 / 0	0 / 0	655 / 0	0 / 0	
J	1890	1301 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0 / 42	-95.2	-95.2	0.07 (1)	10.00	C-O	0 / 217
B-C	0 / 25	-95.2	-95.2	0.11 (1)	10.00	O-D	0 / 164
C-D	-3248 / 0	-95.2	-95.2	0.17 (1)	5.02	K-F	0 / 93
D-E	-2989 / 0	-95.2	-95.2	0.13 (1)	5.21	K-G	0 / 149
E-F	-2628 / 0	-95.2	-95.2	0.12 (1)	5.49	P-C	-3544 / 0
F-G	-2825 / 0	-95.2	-95.2	0.16 (1)	5.31	G-J	-3119 / 0
G-H	0 / 26	-95.2	-95.2	0.12 (1)	10.00	N-E	0 / 904
H-I	0 / 42	-95.2	-95.2	0.07 (1)	10.00	E-M	-1300 / 0
P-B	-274 / 0	0.0	0.0	0.01 (1)	7.81	D-N	0 / 1677
J-H	-273 / 0	0.0	0.0	0.01 (1)	7.81	M-F	0 / 1568
P-O	0 / 2323	-18.5	-18.5	0.20 (1)	10.00		
O-N	0 / 2471	-18.5	-18.5	0.29 (1)	10.00		
N-M	0 / 2830	-18.5	-18.5	0.32 (1)	10.00		
M-L	0 / 2145	-18.5	-18.5	0.18 (1)	10.00		
L-K	0 / 2145	-18.5	-18.5	0.18 (1)	10.00		
K-J	0 / 2045	-18.5	-18.5	0.17 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	10-2-8	-1981	-1981	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.17/1.00 (C-D-1), BC=0.32/1.00 (M-N-1), WB=0.80/1.00 (E-M-1), SSI=0.09/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (F) (INPUT = 0.90)
JSI METAL= 0.40 (G) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080182 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427556	T11	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:48 2022 Page 2
ID:szRM1lsiw FZp751jUvY2Kz1nzP-XDR9W RzOfsBFQV4xWxc6d8YYXyR4QdDg6J eyrvrviD

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.50	2.00
L	BS-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	8.0		
N	BMWW+t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	4.0	2.50	2.00
P	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



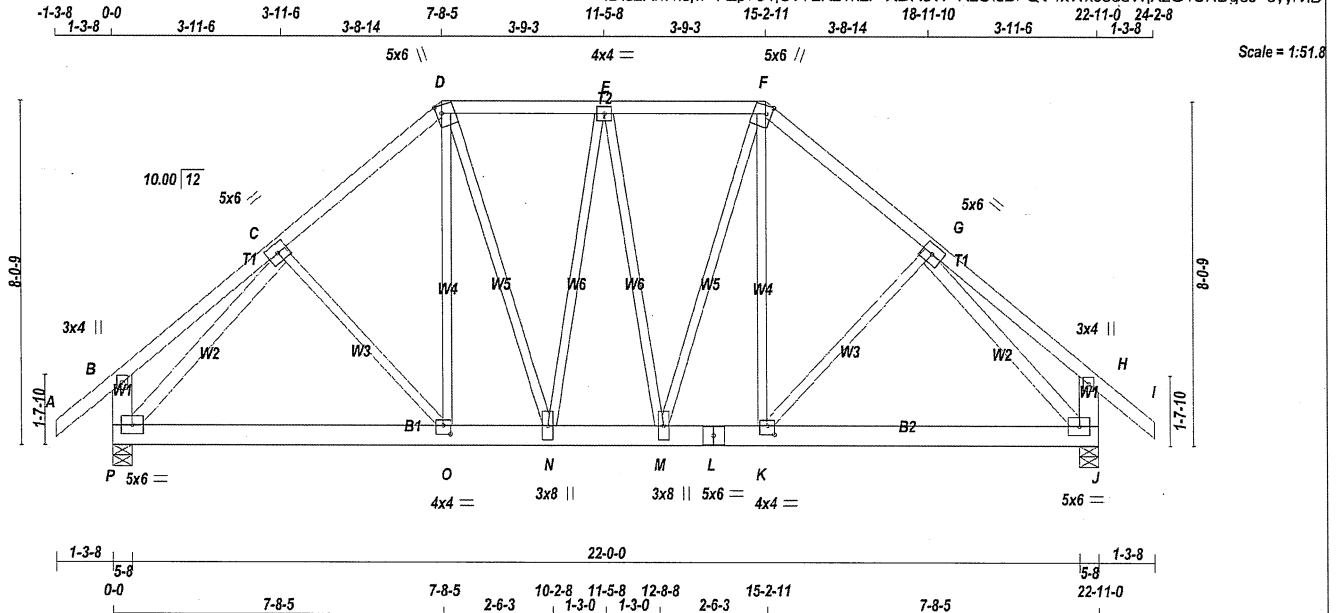
STRUCTURAL COMPONENT ONLY
DWG # TR22080182 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T11Z	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Tue Aug 2 11:44:48 2022 Page 1

ID:szRM1lsjw FZp751lUvY2Kz1nzP-XDR9W RZOfsBFQV4xWxc6d8WjXzG4UKDg6J eyrvrviD



TOTAL WEIGHT = 139 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
P - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
P - L	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
P - C	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	4.0	2.50	2.00
L	BS-t	MT20	5.0	6.0		
M	BMVW+t	MT20	3.0	8.0		
N	BMVW+t	MT20	3.0	8.0		
O	BMVW-t	MT20	4.0	4.0	2.50	2.00
P	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ
JT	1435	0	1435	0
J	1435	0	1435	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	
P	1010	687 / 0	0 / 0	0 / 0	324 / 0
J	1010	687 / 0	0 / 0	0 / 0	324 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH
FR-TO						FR-TO		
A-B	0 / 42	-95.2	-95.2	0.14 (1)	10.00	C-O	-116 / 10	0.07 (1)
B-C	0 / 27	-95.2	-95.2	0.25 (1)	10.00	O-D	0 / 233	0.06 (4)
C-D	-1175 / 0	-95.2	-95.2	0.28 (1)	5.51	K-F	0 / 233	0.06 (4)
D-E	-952 / 0	-95.2	-95.2	0.26 (1)	5.99	K-G	-116 / 10	0.07 (1)
E-F	-955 / 0	-95.2	-95.2	0.26 (1)	5.99	P-C	-1460 / 0	0.57 (1)
F-G	-1175 / 0	-95.2	-95.2	0.28 (1)	5.51	G-J	-1460 / 0	0.57 (1)
G-H	0 / 27	-95.2	-95.2	0.25 (1)	10.00	N-E	-215 / 0	0.26 (1)
H-I	0 / 42	-95.2	-95.2	0.14 (1)	10.00	E-M	-223 / 0	0.28 (1)
P-B	-271 / 0	0.0	0.0	0.02 (1)	7.81	D-N	0 / 226	0.06 (1)
J-H	-271 / 0	0.0	0.0	0.02 (1)	7.81	M-F	0 / 236	0.06 (1)
P-O	0 / 957	-18.5	-18.5	0.20 (1)	10.00			
O-N	0 / 883	-18.5	-18.5	0.19 (4)	10.00			
N-M	0 / 990	-18.5	-18.5	0.15 (1)	10.00			
M-L	0 / 883	-18.5	-18.5	0.19 (4)	10.00			
L-K	0 / 883	-18.5	-18.5	0.19 (4)	10.00			
K-J	0 / 957	-18.5	-18.5	0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.28/1.00 (F-G:1), BC=0.20/1.00 (O-P:1), WB=0.57/1.00 (G-J:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (K) (INPUT = 0.90)

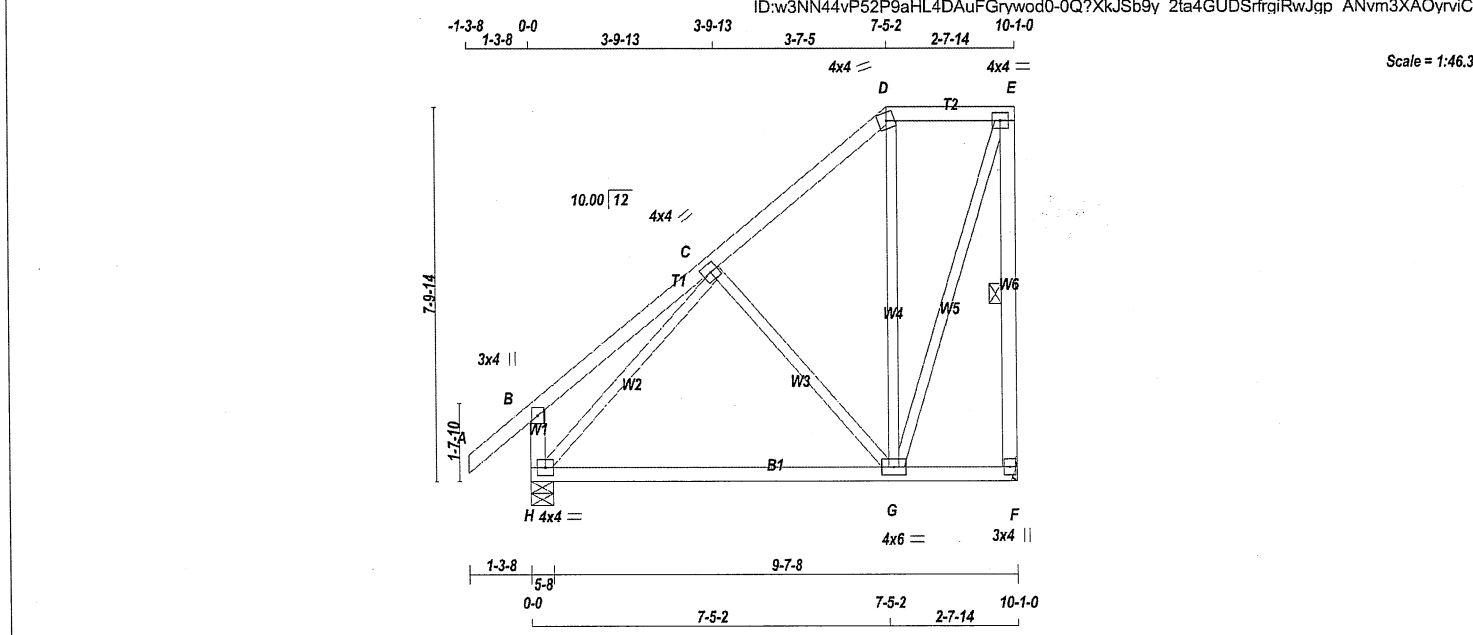
JSI METAL= 0.33 (G) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080183

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T12	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 58 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
F	573	0	573	0	0	MECHANICAL			
H	705	0	705	0	0	5-8			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	405	270 / 0	0 / 0
H	495	344 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX. FACTORED	MEMB.	FORCE (LBS)	MAX. FACTORED
FR-TO		FROM	TO	FR-TO		LENGTH	FR-TO		LENGTH
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00		C-G	-250 / 0	0.15 (1)
B-C	0 / 28	-95.2	-95.2	0.22 (1)	10.00		G-D	-131 / 5	0.15 (1)
C-D	-233 / 0	-95.2	-95.2	0.17 (1)	6.25		G-E	0 / 486	0.11 (1)
D-E	-156 / 0	-95.2	-95.2	0.09 (1)	6.25		H-C	-501 / 0	0.27 (1)
F-E	-585 / 0	0.0	0.0	0.16 (1)	6.25				
H-B	-264 / 0	0.0	0.0	0.03 (1)	7.81				
H-G	0 / 323	-18.5	-18.5	0.25 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	26.7	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.22/1.00 (B-C:1), BC=0.25/1.00 (G-H:4), WB=0.27/1.00 (C-H:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

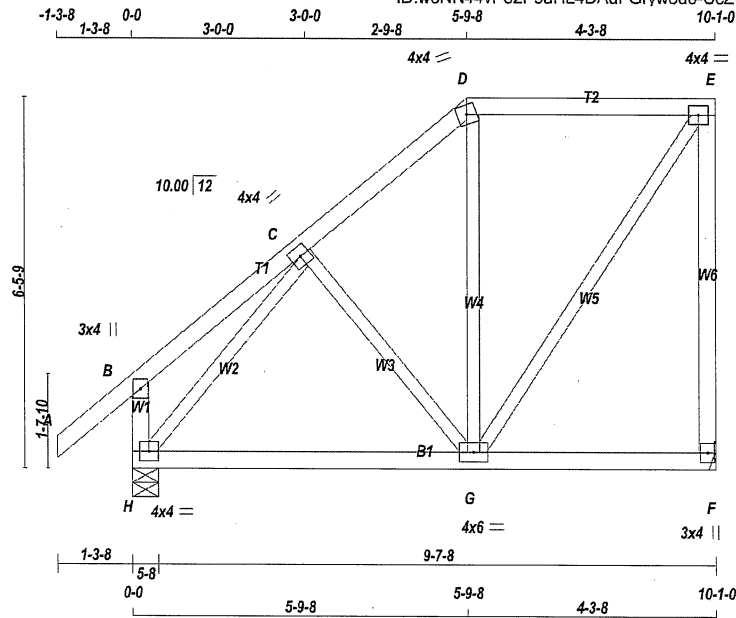
JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080184

JOB NAME 427556	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:50 2022 Page 1
ID:w3NN44vP52P9aHL4DAuFGrywod0-UcZwxFTDwG7vUkfS2xz4C2DpLKfDYTkW7Po5iqyrvIB



Scale = 1:38.5

TOTAL WEIGHT = 53 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:		
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	26.7	PSF
D - E	2x4	DRY	No.2	SPF	VERT	DOWN	UPLIFT	IN-SX	DL =	6.0	PSF
F - E	2x4	DRY	No.2	SPF	F	573	0	5-8	BOT CH. LL =	0.0	PSF
H - B	2x4	DRY	No.2	SPF	H	705	0	5-8	DL =	7.4	PSF
H - F	2x4	DRY	No.2	SPF					TOTAL LOAD =	40.1	PSF
ALL WEBS	2x3	DRY	No.2	SPF	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.						
EXCEPT											
DRY: SEASONED LUMBER.											

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW-t	MT20	4.0	6.0	
H	BMVW1-t	MT20	4.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	405	270 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	495	344 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 42	-95.2	-95.2 0.13 (1)	10.00	C-G	-148 / 0	0.06 (1)
B-C	0 / 21	-95.2	-95.2 0.13 (1)	10.00	G-D	-114 / 16	0.08 (1)
C-D	-329 / 0	-95.2	-95.2 0.10 (1)	6.25	G-E	0 / 417	0.09 (1)
D-E	-234 / 0	-95.2	-95.2 0.22 (1)	6.25	H-C	-538 / 0	0.19 (1)
F-E	-548 / 0	0.0	0.0 0.47 (1)	7.81			
H-B	-236 / 0	0.0	0.0 0.03 (1)	7.81			
H-G	0 / 330	-18.5	-18.5 0.17 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.47/1.00 (E-F:1), BC=0.17/1.00 (G-H:4)
WB=0.19/1.00 (C-H:1), SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (C) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)



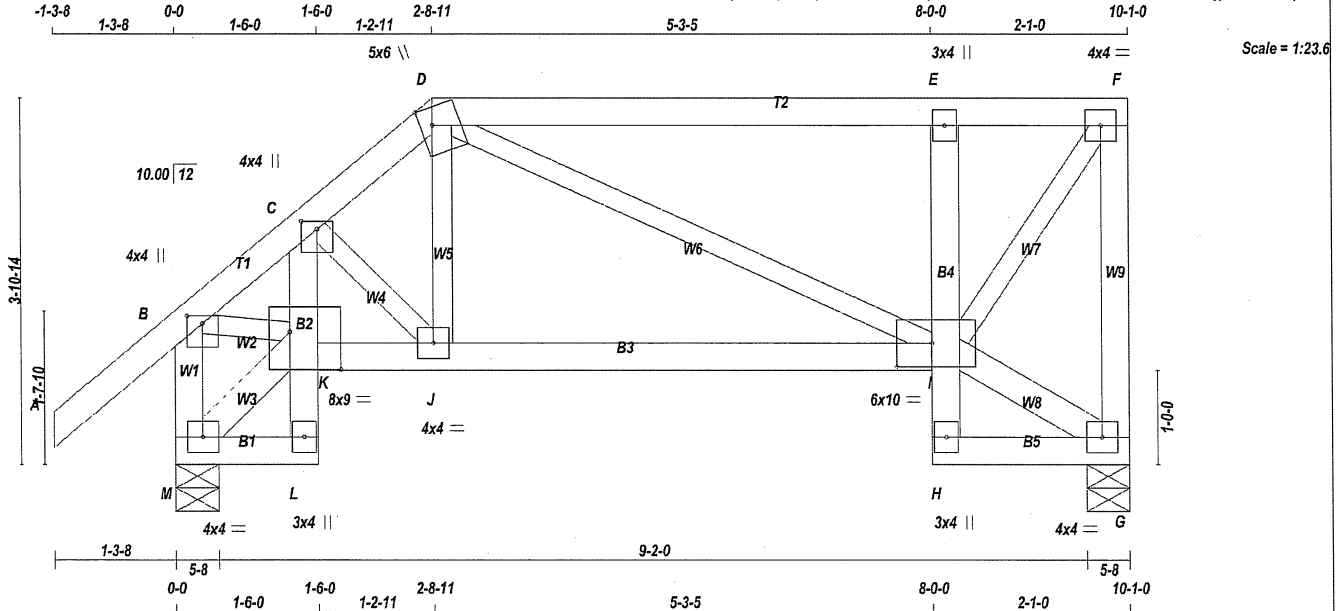
STRUCTURAL COMPONENT ONLY
DWG # TR22080185

STRUCTURAL COMPONENT ONLY
DWG # TR22080186

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:51 2022 Page 1
ID:szRM1lsjw FZp751jUvY2Kz1nzP-yo719?ThaFm6tEfoeUJkGI0kk?cHwOgM3YeEHyrviA



TOTAL WEIGHT = 52 lb [M]

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - L	2x4	DRY	No.2	SPF	
L - C	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
H - E	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWV+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMV+p	MT20	3.0	4.0		
I	BMVWWW-t	MT20	6.0	10.0	3.00	4.50
J	BMVW-t	MT20	4.0	4.0		
K	BMVWWW-m	MT20	8.0	9.0	4.75	6.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	564	0	564	0	5-8	5-8
M	714	0	714	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
G	398	264 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
M	502	349 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2 -95.2	0.13 (1)	10.00	C-J	-55 / 0	0.01 (1)
B-C	-641 / 0	-95.2 -95.2	0.13 (1)	6.25	J-D	0 / 99	0.03 (4)
C-D	-558 / 0	-95.2 -95.2	0.05 (1)	6.25	D-I	-40 / 0	0.03 (4)
D-E	-435 / 0	-95.2 -95.2	0.36 (1)	6.25	I-G	-22 / 0	0.00 (1)
E-F	-427 / 0	-95.2 -95.2	0.35 (1)	6.25	I-F	0 / 718	0.16 (1)
G-F	-536 / 0	0.0 0.0	0.13 (1)	7.81	M-K	-24 / 0	0.00 (1)
M-B	-685 / 0	0.0 0.0	0.07 (1)	7.81	B-K	0 / 454	0.10 (1)
M-L	0 / 18	-18.5 -18.5	0.01 (4)	10.00			
L-K	0 / 14	0.0 0.0	0.03 (1)	10.00			
K-C	-2 / 12	0.0 0.0	0.03 (1)	10.00			
K-J	0 / 471	-18.5 -18.5	0.16 (4)	10.00			
J-I	0 / 435	-18.5 -18.5	0.16 (4)	10.00			
H-I	0 / 20	0.0 0.0	0.03 (1)	10.00			
I-E	-529 / 0	0.0 0.0	0.03 (1)	7.81			
H-G	0 / 20	-18.5 -18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.36/1.00 (D-E:1), BC=0.16/1.00 (I-J:4), WB=0.16/1.00 (F-I:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (F) (INPUT= 0.90)
JSI METAL= 0.20 (F) (INPUT= 1.00)



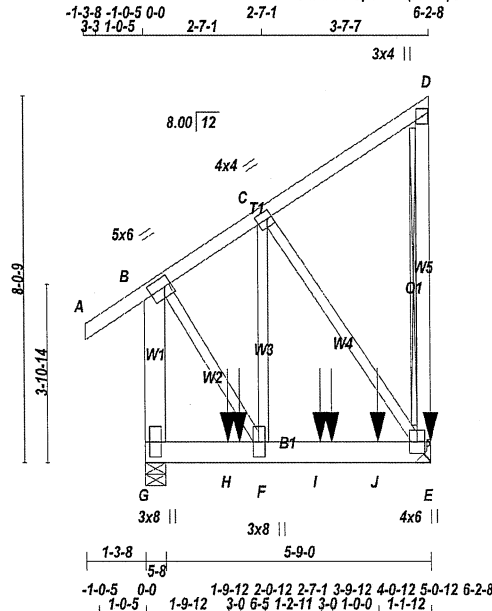
STRUCTURAL COMPONENT ONLY
DWG # TR22080187

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T16	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:52 2022 Page 1

ID:szRM1lsjw FZp751jUvY2Kz1nzP-Q?hgMLUTSStNck1prAL?YHTIEX8H00HapbjHbnjyvi9



Scale = 1:48.6

TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	E - D	2x4	DRY	No.2	SPF
	G - B	2x6	DRY	No.2	SPF
	G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2		SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
E - D 1	12	TOP
G - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - E 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50 1.75



STRUCTURAL COMPONENT ONLY
DWG # TR22080188 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	2841	0	2841	0	0	MECHANICAL
G	2143	0	2143	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1996	1385 / 0	0 / 0	0 / 0	0 / 0	611 / 0	0 / 0
G	1504	1055 / 0	0 / 0	0 / 0	0 / 0	448 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO		
A-B	0 / 36	-95.2 -95.2	0.07 (1)	10.00	F-C	0 / 1551 0.19 (1)
B-C	-1069 / 0	-95.2 -95.2	0.09 (1)	6.25	C-E	-1597 / 0 0.55 (1)
C-D	-17 / 0	-95.2 -95.2	0.08 (1)	6.25	B-F	0 / 1528 0.19 (1)
E-D	-142 / 0	0.0 0.0	0.03 (1)	7.81		
G-B	-2046 / 0	0.0 0.0	0.17 (1)	7.81		
G-H	0 / 0	-18.5 -18.5	0.32 (1)	10.00		
H-F	0 / 0	-18.5 -18.5	0.32 (1)	10.00		
F-I	0 / 909	-18.5 -18.5	0.40 (1)	10.00		
I-J	0 / 909	-18.5 -18.5	0.40 (1)	10.00		
J-E	0 / 909	-18.5 -18.5	0.40 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	6-2-8	-395	-395	---	FRONT	VERT	TOTAL	---	C1
H	1-9-12	-581	-581	---	BACK	VERT	TOTAL	---	C1
H	2-0-12	-383	-383	---	FRONT	VERT	TOTAL	---	C1
I	3-9-12	-581	-581	---	BACK	VERT	TOTAL	---	C1
I	4-0-12	-390	-390	---	FRONT	VERT	TOTAL	---	C1
J	5-0-12	-581	-581	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.17/1.00 (B-G:1), BC=0.40/1.00 (E-F:1), WB=0.55/1.00 (C-E:1), SS=0.48/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T16	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:52 2022 Page 2
ID:szRM1lsiw FZp751jUvY2Kz1nzP-Q?hgMLUTStNck1prAL?YHTIEX8H00HapbiHBnlyrvi9

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0		
E	BMVW1+p	MT20	4.0	6.0		
F	BMWW+t	MT20	3.0	8.0		
G	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

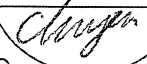
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER

8/02/22

C. M. HEYENS

100505065



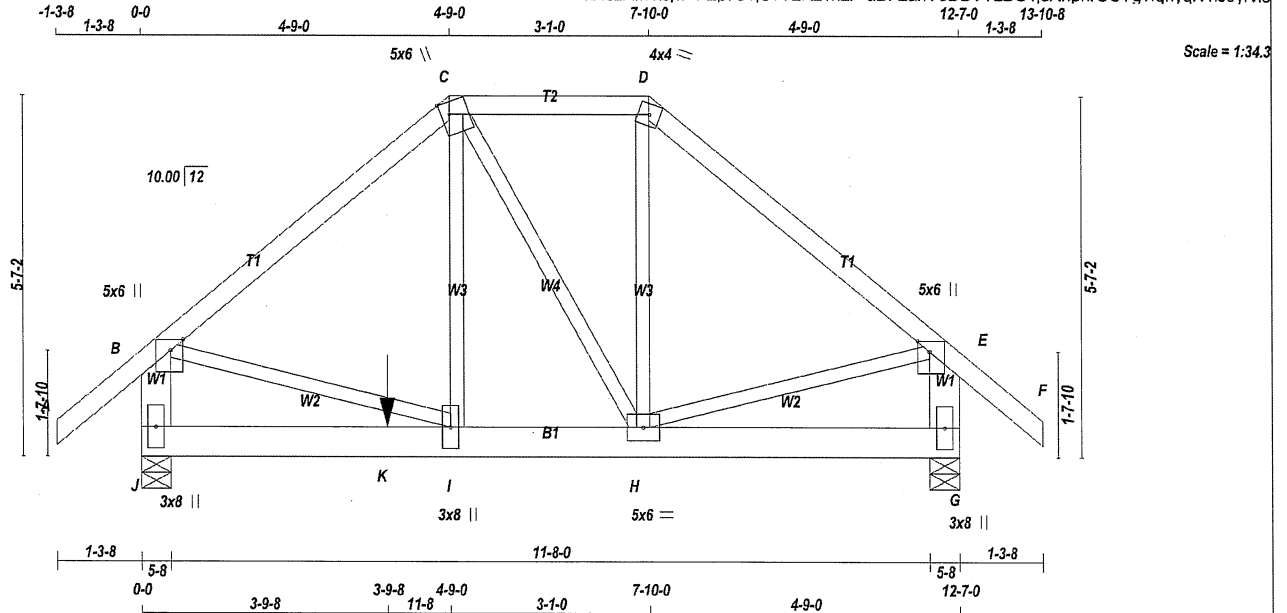
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

DWG # TR22080188 PG 2/2

JOB NAME 427556	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:53 2022 Page 1
ID:szRM1lsjw FZp751jUvY2Kz1nzP-uBF2ahV6DBVTLBO1j3XnpHrOSYg?lqnyqN1j9yrv8



TOTAL WEIGHT = 2 X 68 = 137 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - D	1	12
D - F	1	12
J - B	2	12
G - E	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - G	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1586	0	1586	0
G	1166	0	1166	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
J	1112
G	819

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
MEMB.									
FR-TO									
A-B	0 / 42	-95.2	-95.2	0.07 (1)	10.00	I-C	0 / 915	0.11 (1)	
B-C	-1250 / 0	-95.2	-95.2	0.22 (1)	6.25	C-H	-556 / 0	0.17 (1)	
C-D	-704 / 0	-95.2	-95.2	0.09 (1)	6.25	H-D	0 / 212	0.03 (1)	
D-E	-910 / 0	-95.2	-95.2	0.21 (1)	6.25	B-I	0 / 988	0.12 (1)	
E-F	0 / 42	-95.2	-95.2	0.07 (1)	10.00	H-E	0 / 719	0.09 (1)	
J-B	-1401 / 0	0.0	0.0	0.05 (1)	7.81				
G-E	-1115 / 0	0.0	0.0	0.04 (1)	7.81				
J-K	0 / 0	-18.5	-18.5	0.21 (1)	10.00				
K-I	0 / 0	-18.5	-18.5	0.21 (1)	10.00				
I-H	0 / 977	-18.5	-18.5	0.23 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.03 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	3-9-8	-739	-739	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.22/1.00 (B-C:1), BC=0.23/1.00 (H-I:1), WB=0.17/1.00 (C-H:1), SSI=0.32/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (I) (INPUT = 0.90)
JSI METAL= 0.29 (I) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080190 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427556	T18	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Tue Aug 2 11:44:53 2022 Page 2
ID:szRM1lsiw FZp751jUvY2Kz1nzP-uBF2ahV6DBVTLBO1j3XnphrOSYg?lqnyqN1J9yrv8

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
G	BMV1+p	MT20	3.0	8.0		
H	BMVWW-t	MT20	5.0	6.0		
I	BMVWW+t	MT20	3.0	8.0		
J	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

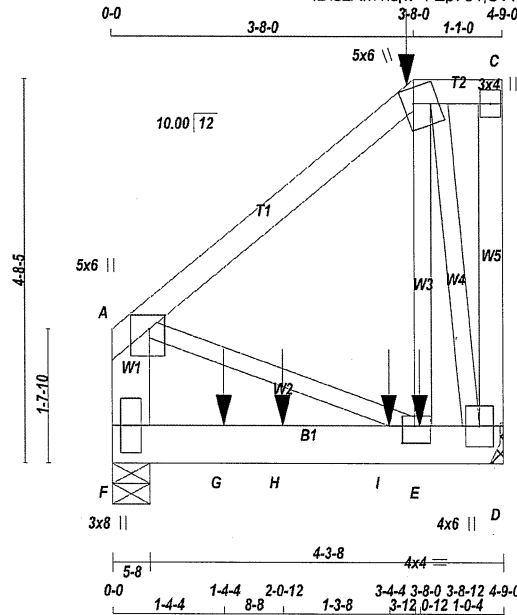


STRUCTURAL COMPONENT ONLY
DWG # TR22080190 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	T19	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:54 2022 Page 1
ID:szRM1lsiw FZp751jUvY2Kz1nzP-MNpQn1Wk VdKzLzDhm20MuNbx0SUGV621mlrcyrv7



Scale = 1:27.0

TOTAL WEIGHT = 2 X 31 = 63 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	SIDE(61.0)
B - C	12	SIDE(61.0)
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1076	0	1076	0
F	944	0	944	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
D	754	534 / 0	0 / 0
F	662	464 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO							FR-TO			
A-B	-370 / 0	-95.2	-95.2	0.12 (1)	6.25	E-B	0 / 977	0.12 (1)		
B-C	0 / 0	-95.2	-95.2	0.01 (1)	10.00	B-D	-1255 / 0	0.20 (1)		
D-C	-52 / 0	0.0	0.0	0.01 (1)	7.81	A-E	0 / 298	0.04 (1)		
F-A	-504 / 0	0.0	0.0	0.02 (1)	7.81					
F-G	0 / 0	-18.5	-18.5	0.21 (1)	10.00					
G-H	0 / 0	-18.5	-18.5	0.21 (1)	10.00					
H-I	0 / 0	-18.5	-18.5	0.21 (1)	10.00					
I-E	0 / 0	-18.5	-18.5	0.21 (1)	10.00					
E-D	0 / 307	-18.5	-18.5	0.09 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-8-0	-174	-174	---	BACK	VERT	TOTAL	---	C1
E	3-8-12	-9	-9	---	BACK	VERT	TOTAL	---	C1
G	1-4-4	-422	-422	---	FRONT	VERT	TOTAL	---	C1
H	2-0-12	-6	-6	---	BACK	VERT	TOTAL	---	C1
I	3-4-4	-424	-424	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	26.7	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.4	PSF
TOTAL LOAD	40.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.21/1.00 (E-F:1), WB=0.20/1.00 (B-D:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MIN	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)

JSI METAL= 0.14 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22080191 PG 1/2

DRWG NO.

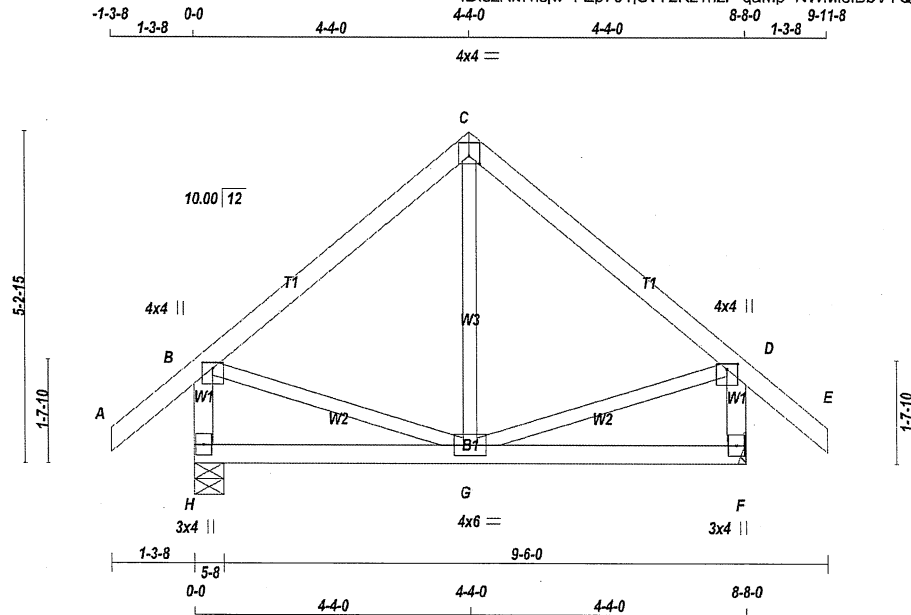
Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:54 2022 Page 2
ID:szRM1siw FZp751iUvY2Kz1nzP-MNpQn1Wk VdKzLzDHm20MuNpIx0SUGV621mlrcrvv7

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



JOB NAME 427556	TRUSS NAME T20	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:55 2022 Page 1
ID: szRM1lsjw FZp751jUvY2Kz1nzP-qaMp NWMloBbVYQrUZfV6wjinLOUDm4FHhWrN2yrv6
Scale = 1:35.0



TOTAL WEIGHT = 40 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-l	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
H	625	0	5-8	5-8
F	625	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	438	306 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
F	438	306 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO							FR-TO			
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00		G-C	-37 / 63	0.02 (4)	
B-C	-293 / 0	-95.2	-95.2	0.23 (1)	6.25		B-G	0 / 235	0.05 (1)	
C-D	-293 / 0	-95.2	-95.2	0.23 (1)	6.25		G-D	0 / 235	0.05 (1)	
D-E	0 / 42	-95.2	-95.2	0.13 (1)	10.00					
H-B	-593 / 0	0.0	0.0	0.06 (1)	7.81					
F-D	-593 / 0	0.0	0.0	0.06 (1)	7.81					
H-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00					
G-F	0 / 0	-18.5	-18.5	0.10 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (C-D:1), BC=0.10/1.00 (G-H:4), WB=0.05/1.00 (B-G:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

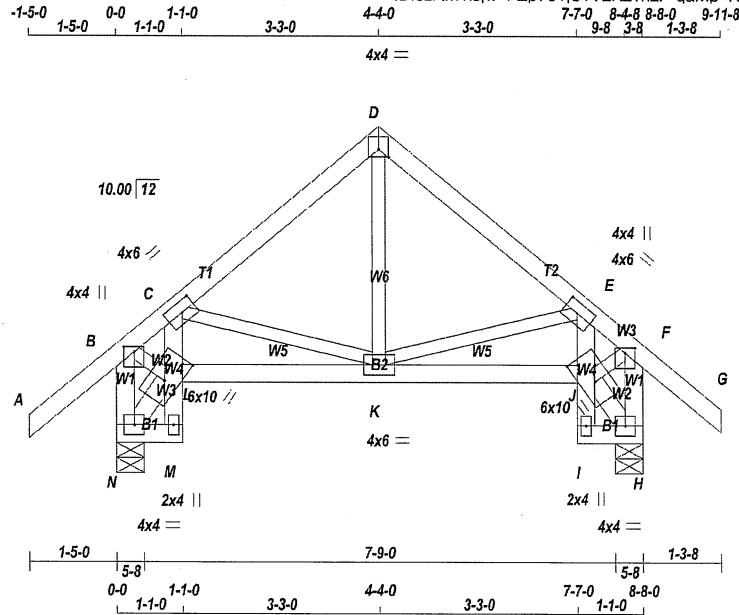


STRUCTURAL COMPONENT ONLY
DWG # TR22080192

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427556	T20S	2	1	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:55 2022 Page 1
ID:szRM1lsiw FZp751jUvY2Kz1nzP-qaMp NWMloBbVYQrUZfV6wKxLOfDleFHhWrN2yrv6



Scale = 1:36.7

TOTAL WEIGHT = 2 X 46 = 91 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:		
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	26.7	PSF
D - G	2x4	DRY	No.2	SPF	VERT	DOWN	UPLIFT	IN-SX	DL =	6.0	PSF
N - B	2x4	DRY	No.2	SPF	H	623	0	5-8	BOT CH. LL =	0.0	PSF
H - F	2x4	DRY	No.2	SPF	N	638	0	5-8	DL =	7.4	PSF
N - M	2x4	DRY	No.2	SPF	H	623	0	5-8	TOTAL LOAD =	40.1	PSF
L - J	2x4	DRY	No.2	SPF							
I - H	2x4	DRY	No.2	SPF							

ALL WEBS	SIZE	LUMBER	DESCR.
EXCEPT			
K - D	2x3	DRY	No.2
K - E	2x3	DRY	No.2
C - K	2x3	DRY	No.2
B - L	2x3	DRY	No.2
J - F	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	6.0	2.00	2.75
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMVW-t	MT20	4.0	6.0	2.00	2.75
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW+w	MT20	2.0	4.0		
J	BMVWW*-w	MT20	6.0	10.0	2.75	5.75
K	BMVWW-t	MT20	4.0	6.0		
L	BMVWW*-w	MT20	6.0	10.0	2.75	5.75
M	BMVW+w	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
N	638	638	0	5-8
H	623	623	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	448	314 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	437	305 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	MEMB.	FORCE (LBS)
FR-TO		FROM	TO			FR-TO	
A-B	0 / 46	-95.2	-95.2	0.16 (1)	10.00	M-L	0 / 10
B-C	-428 / 0	-95.2	-95.2	0.12 (1)	6.25	L-C	-132 / 0
C-D	-346 / 0	-95.2	-95.2	0.13 (1)	6.25	I-J	0 / 10
D-E	-347 / 0	-95.2	-95.2	0.12 (1)	6.25	J-E	-137 / 0
E-F	-431 / 0	-95.2	-95.2	0.13 (1)	6.25	K-D	0 / 151
F-G	0 / 42	-95.2	-95.2	0.11 (1)	10.00	K-E	-96 / 0
N-B	-611 / 0	0.0	0.0	0.06 (1)	7.81	C-K	-83 / 0
H-F	-595 / 0	0.0	0.0	0.06 (1)	7.81	B-L	0 / 346
						N-L	-27 / 0
N-M	0 / 19	-18.5	-18.5	0.01 (4)	10.00	J-F	0 / 360
L-K	0 / 333	-18.5	-18.5	0.09 (4)	10.00	J-H	-27 / 0
K-J	0 / 345	-18.5	-18.5	0.09 (4)	10.00		
I-H	0 / 19	-18.5	-18.5	0.01 (4)	10.00		

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (A-B-1), BC=0.09/1.00 (J-K-4), WB=0.08/1.00 (F-J-1), SSI=0.11/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (L) (INPUT = 0.90)

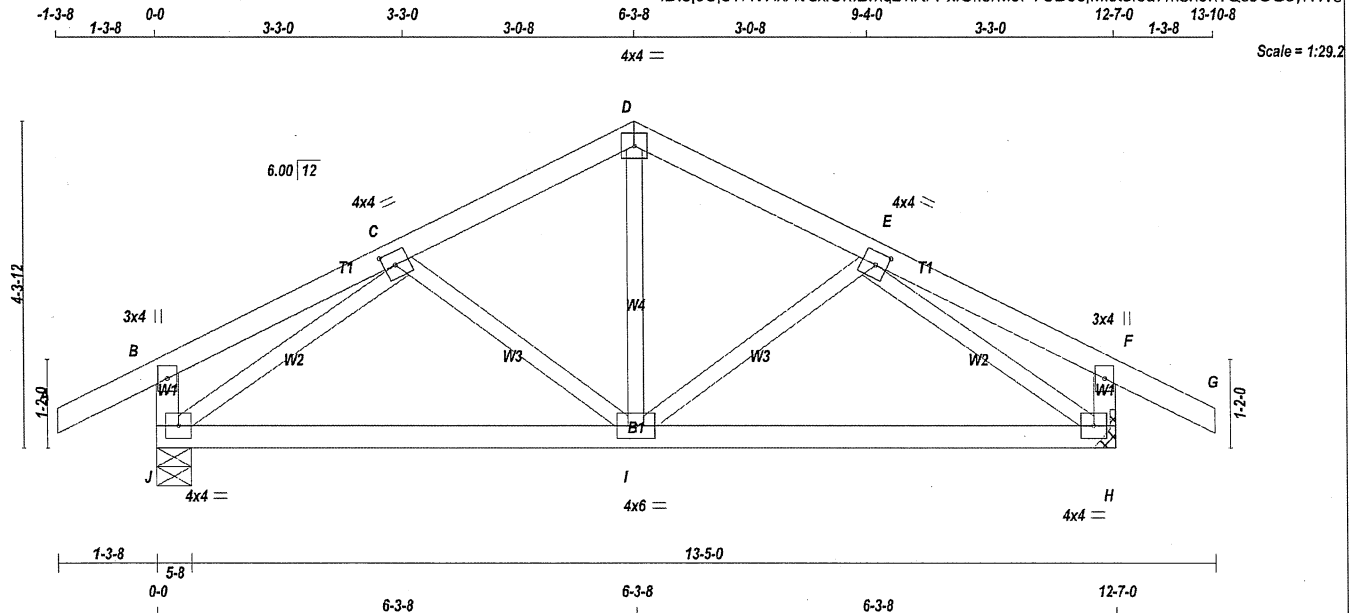
JSI METAL= 0.14 (F) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080193

JOB NAME 427574	TRUSS NAME T21	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:41 2022 Page 1
ID:ej0CjeYAVHxAVskiOhfBmqz1IKf-PxfOllorM9FVUD5ejMltxfefu7m8h9n?Qs3OGeyrvW8



TOTAL WEIGHT = 2 X 51 = 101 lb
[M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
J	845	0	845	0
H	845	0	845	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
J	594	409 / 0 0 / 0 0 / 0 0 / 0 185 / 0 0 / 0
H	594	409 / 0 0 / 0 0 / 0 0 / 0 185 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 29	-95.2 -95.2	0.12 (1)	10.00	I-D	0 / 348	0.08 (1)
B-C	0 / 14	-95.2 -95.2	0.14 (1)	10.00	I-E	-168 / 0	0.05 (1)
C-D	-665 / 0	-95.2 -95.2	0.11 (1)	6.25	C-I	-168 / 0	0.05 (1)
D-E	-665 / 0	-95.2 -95.2	0.11 (1)	6.25	J-C	-903 / 0	0.24 (1)
E-F	0 / 14	-95.2 -95.2	0.14 (1)	10.00	E-H	-903 / 0	0.24 (1)
F-G	0 / 29	-95.2 -95.2	0.12 (1)	10.00			
J-B	-248 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-248 / 0	0.0 0.0	0.03 (1)	7.81			
J-I	0 / 715	-18.5 -18.5	0.26 (4)	10.00			
I-H	0 / 715	-18.5 -18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (E-F:1) . BC=0.26/1.00 (I-J:4) , WB=0.24/1.00 (E-H:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

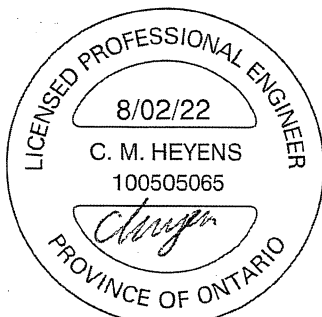
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (J) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

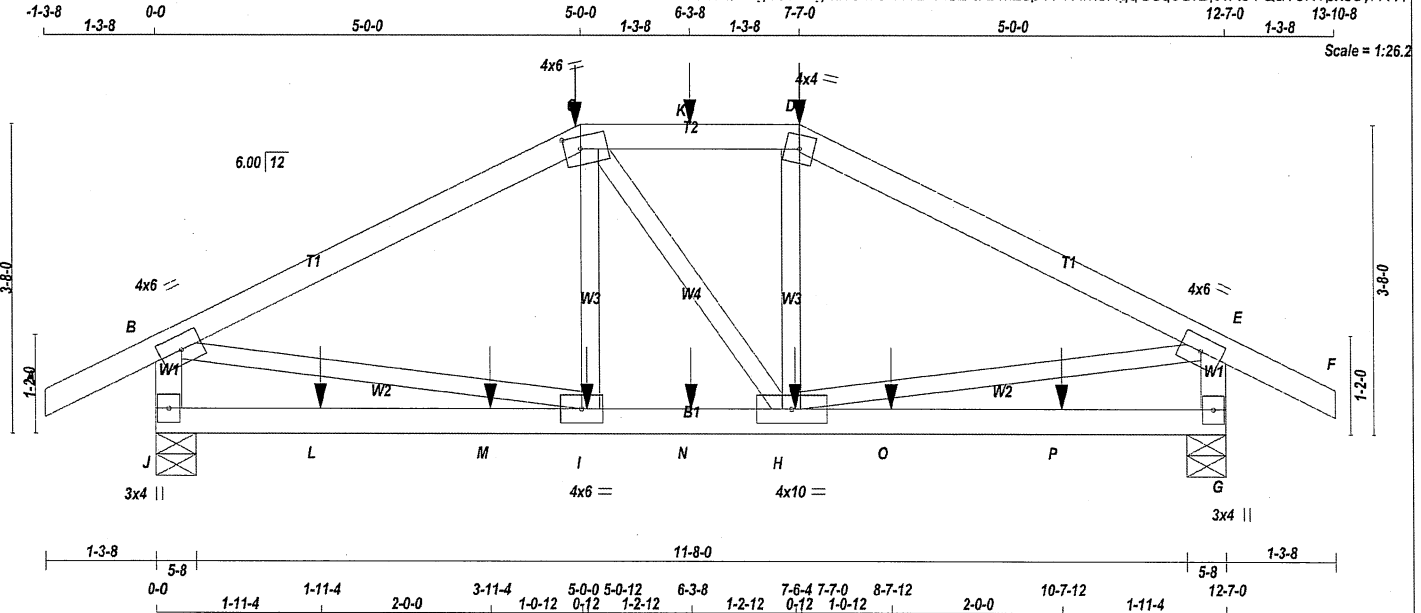


STRUCTURAL COMPONENT ONLY
DWG # TR22080201

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427574	T22	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:42 2022 Page 1

ID:A14z6QwBg06dkBgmL0wCwWz RSz-t7DmzepT7TNM6NqG3q6UrBjWX6YQaT9fVpxo5yrvV7



TOTAL WEIGHT = 51 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
-----------------	-----	-----	------	-----

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	10.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
JT	933	0	0	1323	0	0	0	5-8	5-8
G	1327	0	0	1327	0	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	627 / 0	0 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	935	631 / 0	0 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		MEMB.		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1	MAX.	UNBRAC	MEMB.	FORCE (LBS)	MAX.	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 29	-95.2	-95.2	0.14 (1)	10.00	I-C	-58 / 79	0.03 (4)	
B-C	-1505 / 0	-95.2	-95.2	0.53 (1)	4.63	C-H	0 / 13	0.00 (1)	
C-K	-1351 / 0	-95.2	-95.2	0.25 (1)	5.26	H-D	-65 / 77	0.03 (4)	
K-D	-1351 / 0	-95.2	-95.2	0.25 (1)	5.26	B-I	0 / 1364	0.34 (1)	
D-E	-1515 / 0	-95.2	-95.2	0.53 (1)	4.62	H-E	0 / 1372	0.34 (1)	
E-F	0 / 29	-95.2	-95.2	0.14 (1)	10.00				
J-B	-1268 / 0	0.0	0.0	0.14 (1)	7.11				
G-E	-1271 / 0	0.0	0.0	0.14 (1)	7.11				
J-L	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
L-M	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
M-I	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
I-N	0 / 1343	-18.5	-18.5	0.31 (1)	10.00				
N-H	0 / 1343	-18.5	-18.5	0.31 (1)	10.00				
H-O	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
O-P	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
P-G	0 / 0	-18.5	-18.5	0.18 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-0-0	-215	-215	---	FRONT	VERT	TOTAL	---	C1
D	7-7-0	-230	-230	---	FRONT	VERT	TOTAL	---	C1
H	7-6-4	-30	-30	---	FRONT	VERT	TOTAL	---	C1
I	5-0-12	-30	-30	---	FRONT	VERT	TOTAL	---	C1
K	6-3-8	-72	-72	---	FRONT	VERT	TOTAL	---	C1
L	1-11-4	-15	-15	---	FRONT	VERT	TOTAL	---	C1
M	3-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
N	6-3-8	-30	-30	---	FRONT	VERT	TOTAL	---	C1
O	8-7-12	-22	-22	---	FRONT	VERT	TOTAL	---	C1
P	10-7-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	26.7	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.53/1.00 (D-E:1), BC=0.31/1.00 (H-I:1), WB=0.34/1.00 (E-H:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
MT20	650	371	1747
	MAX	MIN	MAX
	MIN	MAX	MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080202

DRWG NO.	
----------	--

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:43 2022 Page 1
ID:ej0CieYAVHxAVskiOhfBmaz1IKf-LKm8Azo5umVDkWF0anLI13k?SxSV94BjuAYVI_XynW6



TOTAL WEIGHT = 2 X 29 = 58 lb

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00

C H O R D S				W E B S				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	MAX. FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	-1328 / 0	-95.2	-95.2	0.07 (1)	6.25	E-B	0 / 1007	0.12 (1)
B-C	-111 / 0	-95.2	-95.2	0.07 (1)	6.25	B-D	-1410 / 0	0.18 (1)
D-C	-134 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 1254	0.16 (1)
F-A	-1054 / 0	0.0	0.0	0.04 (1)	7.81			
F-G	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
G-E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E-H	0 / 1200	-18.5	-18.5	0.27 (1)	10.00			
H-D	0 / 1200	-18.5	-18.5	0.27 (1)	10.00			

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-579	-579	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	-579	-579	---	BACK	VERT	TOTAL	---	C1

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED



JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL = 0.36 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427574	T23	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:43 2022 Page 2
ID:ej0CjeYAVHxAVskiOnfBmqz1lKf-LKm8Azp5umVDkWF0qnLL13k?SxSV94BluAYVLXyrvW6

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	3.0	8.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22080160

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:23 2022 Page 1

ID:2Omi0yVrZwt1e1q0AL1nK7z24Xg-f6RdlhSCxgWw6UmCNctWoMLFR5 eTdrq3JyyFyvT

Scale = 1:14.1

The drawing shows a roof truss with a main span of 24.0 feet. The top chord is labeled 'A' and the bottom chord is labeled 'B'. The truss is supported by a central vertical post labeled 'C'. The truss is divided into sections by vertical bracing labeled 'E' and 'F'. The truss is shown with various dimensions and loadings. The dimensions are given in feet and inches. The loadings are given in pounds per square foot (PSF). The truss is shown with various components and connections. The components are labeled with letters and numbers. The connections are labeled with letters and numbers. The truss is shown with various details and specifications. The details are given in the tables below.

LUMBER		CHORDS		LUMBER		DESCR.	
N. L. G. A. RULES	SIZE	DRY	No.2	SPF	SPF	SPF	SPF
E - B	2x4	DRY	No.2	SPF	SPF	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)		W		LEN		Y		X	
JT	TYPE	PLATES	W	LEN	Y	X	Y	X	X
B	TMV+p	MT20	3.0	4.0					
E	BMV1+p	MT20	3.0	4.0					

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG					
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
E	293	0	293	0	0	5-8	5-8		
C	64	0	64	0	0	1-8	1-8		
D	43	0	52	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	206	144 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	47	22 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-237 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 29	-95.2	-95.2 0.12 (1)	10.00			
B-C	-10 / 9	-95.2	-95.2 0.08 (4)	6.25			
E-F	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
F-G	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
G-D	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	1-11-4	5	1	8	BACK	VERT	TOTAL
G	3-11-4	1	1	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

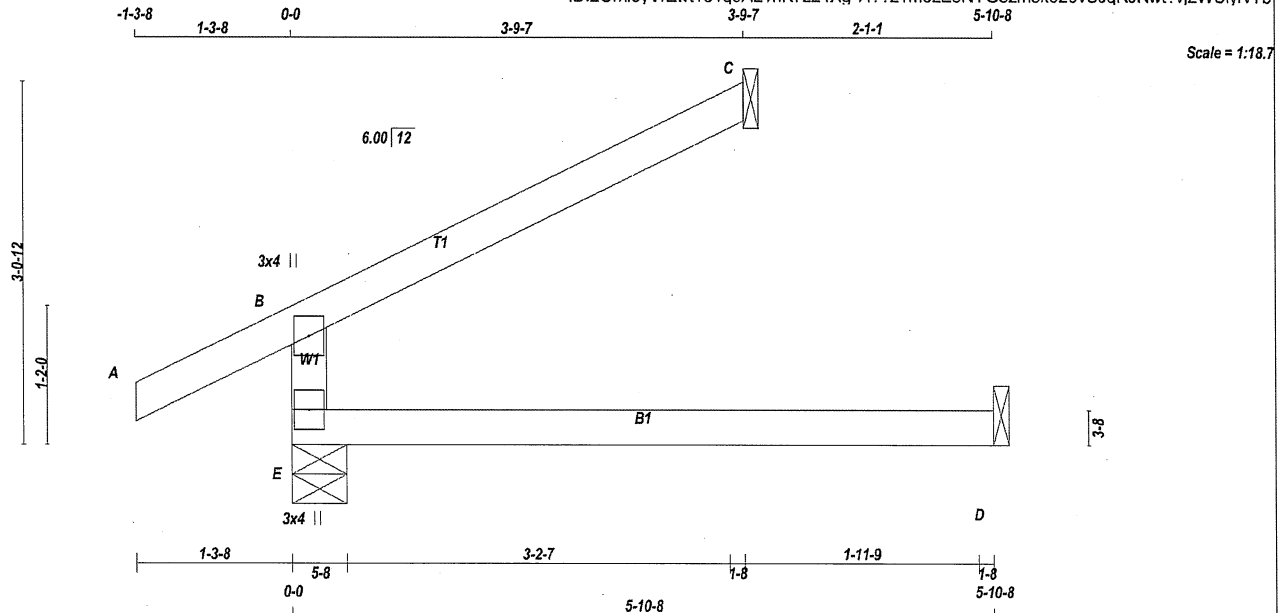
NAIL VALUES		PLATE GRIP(DRY)		SHEAR SECTION	
PLATE	(PSI)	(PLI)	(PLI)	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY
DWG # TR22080161

JOB NAME 427546	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:24 2022 Page 1
ID:2OmI0yVrZwt1e1q0AL1nK7z24Xq-7l??z1m5zEoNYG3zm5k620vUJqRJNwt?vj2WUiyrvTb



TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
E	418	0	418	0
C	135	0	135	0
D	45	0	50	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	295	199 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	93	76 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
E-B	-354 / 0	0.0	0.0	0.13 (4)	7.81					
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00					
B-C	-20 / 0	-95.2	-95.2	0.23 (1)	6.25					
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.23/1.00 (B-C-1), BC=0.13/1.00 (D-E-4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)

JSI METAL= 0.10 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080162

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:25 2022 [M

ID:2OmiQyVrZwt1e1q0AL1nK7z24Xg-cUZOAnnjYwEAQe9KoFLbDRhkEp?6N788No308yvrvTa

Scale = 1:13.6

The diagram shows a roof truss with a peak at joint C. The left side has a vertical height of 24'-12" and a horizontal span of 1-3-8. The right side has a horizontal span of 1-9-7. The bottom chord has a total length of 1-10-8. The truss is supported by bearings at joints B, D, and E. The top chord is labeled with dimensions 1-3-8, 0-0, 1-9-7, and 1-10-8. The bottom chord is labeled with dimensions 1-3-8, 0-0, 5-8, 1-2-7, 0-7, 1-1-1, and 1-10-8. The truss is labeled with dimensions 6.00' and 12'. The truss is labeled with dimensions 3x4 and 3x4. The truss is labeled with dimensions 1-2-0 and 3-8. The truss is labeled with dimensions 1-3-8, 0-0, 1-9-7, and 1-10-8. The truss is labeled with dimensions 1-3-8, 0-0, 5-8, 1-2-7, 0-7, 1-1-1, and 1-10-8. The truss is labeled with dimensions 6.00' and 12'. The truss is labeled with dimensions 3x4 and 3x4. The truss is labeled with dimensions 1-2-0 and 3-8. The truss is labeled with dimensions 1-3-8, 0-0, 1-9-7, and 1-10-8. The truss is labeled with dimensions 1-3-8, 0-0, 5-8, 1-2-7, 0-7, 1-1-1, and 1-10-8.

TOTAL WEIGHT = 2 X 7 = 14 lb

[M

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT UPLIFT IN-SX	REQD BRG IN-SX
E	280	0	280	0	5-8	5-8
C	46	0	46	-24	1-8	1-8
D	8	0	17	-2	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	195	148 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	32	25 / -19	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-253 / 0	0.0	0.0	0.04 (5)	7.81				
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00				
B-C	-17 / 0	-95.2	-95.2	0.09 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	26.7 PSF
DL	=	6.0 PSF
BOT CH. LL	=	0.0 PSF
DL	=	7.4 PSF
TOTAL LOAD	=	40.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5)
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)

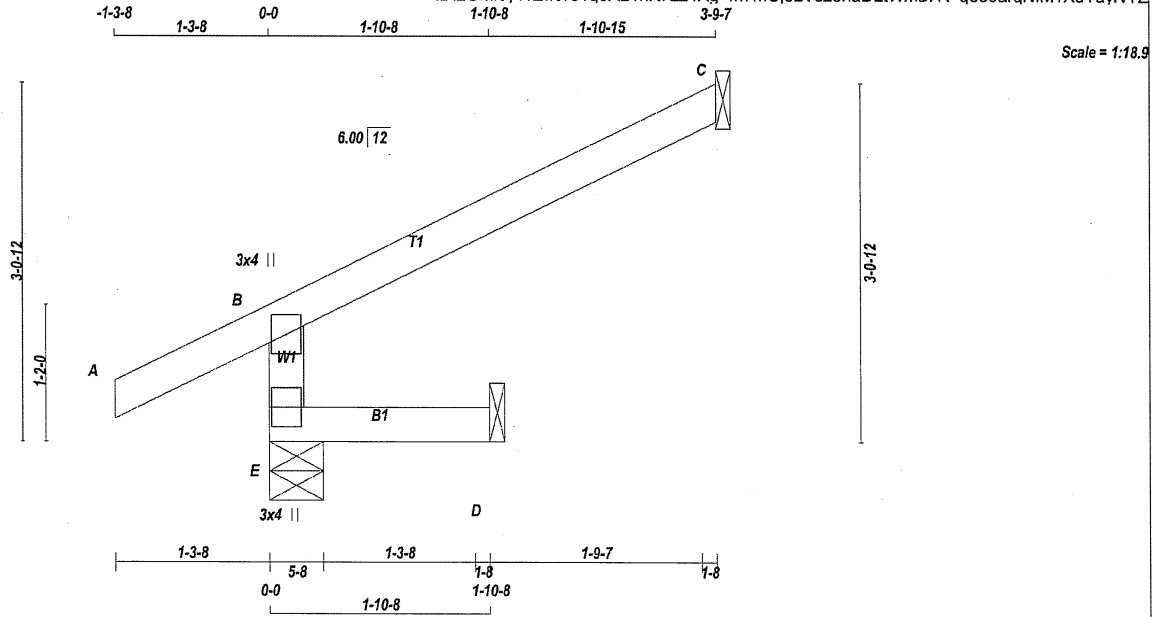
JSI METAL= 0.07 (B) (INPUT = 1.00)

STRUCTURAL COMPONENT ONLY

DWG # TR22080163

JOB NAME 427546	TRUSS NAME J5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:00:26 2022 Page 1
ID:2Om0yVrZwt1e1q0AL1nK7z24Xg-4h7mOJoLVs25naDLtVma7R qoe9arqNIM1XdYayrvTZ



TOTAL WEIGHT = 2 X 10 = 19 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:		
E - B	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	26.7	PSF
A - C	2x4	DRY	No.2	SPF	VERT	DOWN	HORZ	IN-SX	DL =	6.0	PSF
E - D	2x4	DRY	No.2	SPF	E	374	0	5-8	BOT CH. LL =	0.0	PSF
					C	135	0	1-8	DL =	7.4	PSF
					D	16	0	1-8	TOTAL LOAD =	40.1	PSF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN	COMPONENT REACTIONS				
E	259	199 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	93	76 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	UNBRAC LENGTH FR-TO
FR-TO		FROM	TO	FR-TO			
E-B	-354 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 29	-95.2	-95.2	0.13 (5)	10.00		
B-C	-20 / 0	-95.2	-95.2	0.23 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



STRUCTURAL COMPONENT ONLY
DWG # TR22080164

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS: TC=0.23/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

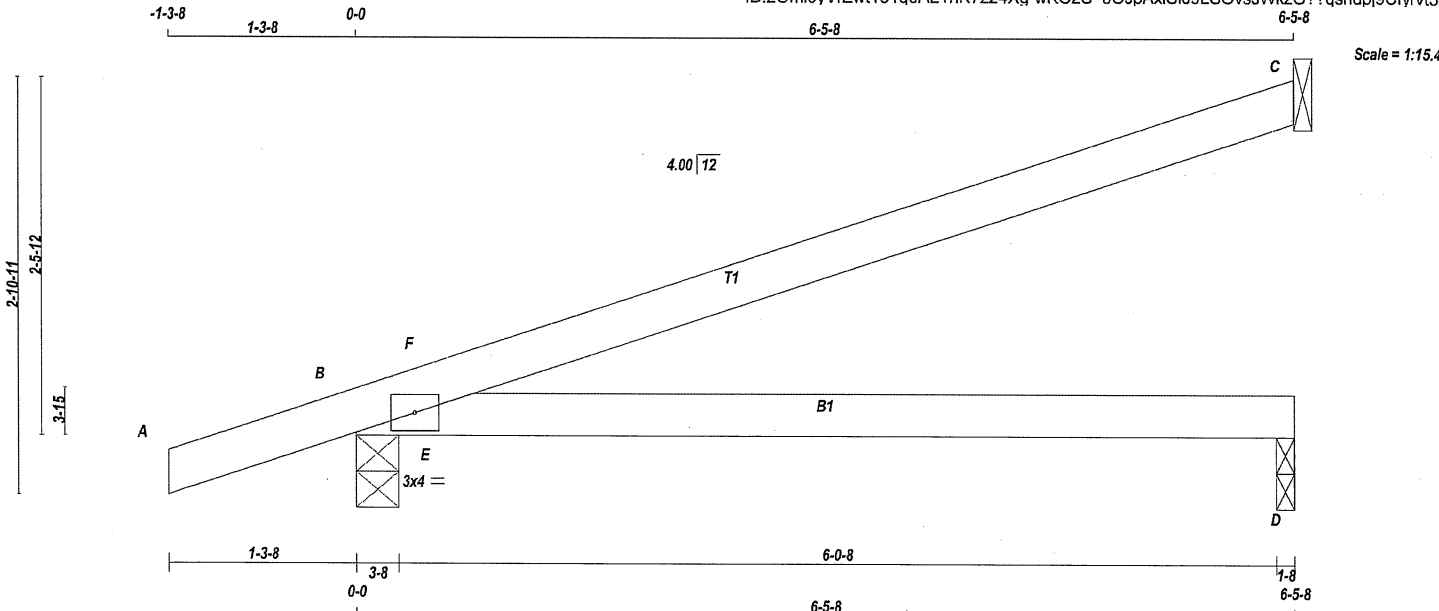
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

JOB NAME 427546	TRUSS NAME J6	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:33:12 2022 Page 1
ID:2Om0yYrZwt1e1q0AL1nK7z24Xg-wKO2S 0OJpAxISIJ9LSOvsJWkzO??qndpj9Clyrvt5



TOTAL WEIGHT = 5 X 17 = 84 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	TOP CH.	LL	= 26.7 PSF
CHORDS			SPF	GROSS REACTION	GROSS REACTION	BRG	BRG		DL	= 6.0 PSF	
A - C	2x4	DRY	No.2	VERT	HORZ	UPLIFT	IN-SX		BOT CH.	LL	= 0.0 PSF
B - D	2x4	DRY	No.2	DOWN	HORZ				DL	= 7.4 PSF	
DRY: SEASONED LUMBER.				C	269	0	1-8	1-8	TOTAL LOAD	= 40.1 PSF	
				B	495	0	3-8	3-8			
				D	98	0	1-8	1-8			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	186	146 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
B	347	244 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
D	73	27 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 19	-95.2	-95.2 0.12 (1)	E-F	-408 / 15	0.00 (1)	10.00
B-F	-22 / 32	-95.2	-95.2 0.08 (1)				6.25
F-C	-3 / 2	-95.2	-95.2 0.51 (1)				10.00
B-E	0 / 0	-18.5	-18.5 0.35 (1)				10.00
E-D	0 / 0	-18.5	-18.5 0.35 (1)				10.00

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL)= L/733 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL)= L/362 (0.21")

CSI: TC=0.51/1.00 (C-F:1), BC=0.35/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.33/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080170

DRWG NO.	
----------	--

ID:szRM1Isiw FZp751iUvY2Kz1nzP-fSCehcOSLQMmpCllqsgvnrzIveY8oAelUl-mVBvnyiH



TOTAL WEIGHT = 13 lb

DRY: SEASONED LUMBER.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

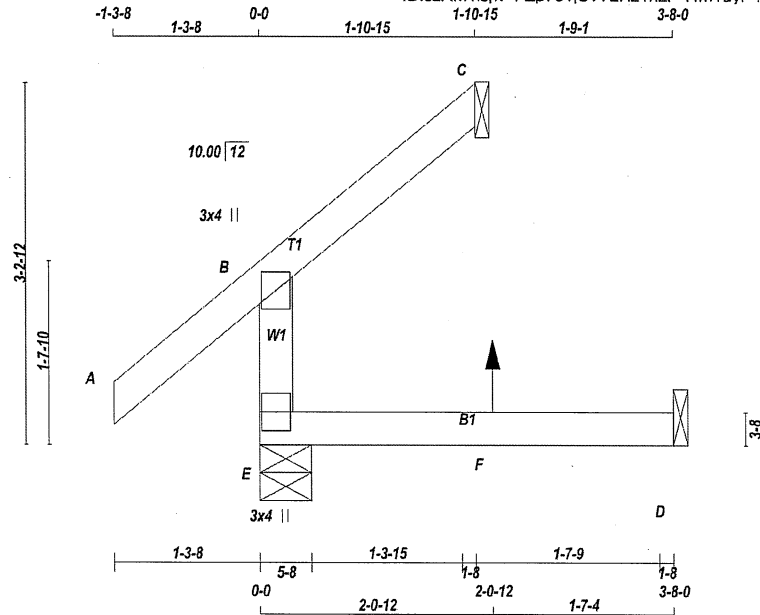
CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



JOB NAME 427556	TRUSS NAME J22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:44:45 2022 Page 1
ID:szRM1lsiw FZp751jUvY2Kz1nzP-7fm1uyP46kUcOynVFNNvV?W1dJ ItFQn 85K1dyrvig



Scale = 1:19.8

TOTAL WEIGHT = 11 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
E	296	0	296	0	5-8
C	55	0	55	-22	1-8
D	24	0	34	0	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	207	149 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0	0 / 0
C	38	27 / -20	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0
D	20	0 / -6	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FORCE	VERT. LOAD	FORCE	MAX
(LBS)	(PLF)	(LBS)	CSI (LC)
FR-TO	FROM	FR-TO	LENGTH
E-B	-260 / 0	0.0	0.05 (5)
A-B	0 / 42	-95.2	-95.2 0.15 (5)
B-C	-24 / 0	-95.2	-95.2 0.10 (6)
E-F	0 / 0	-18.5	-18.5 0.06 (4)
F-D	0 / 0	-18.5	-18.5 0.06 (4)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	4	1	8	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.15/1.00 (A-B:5), BC=0.06/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

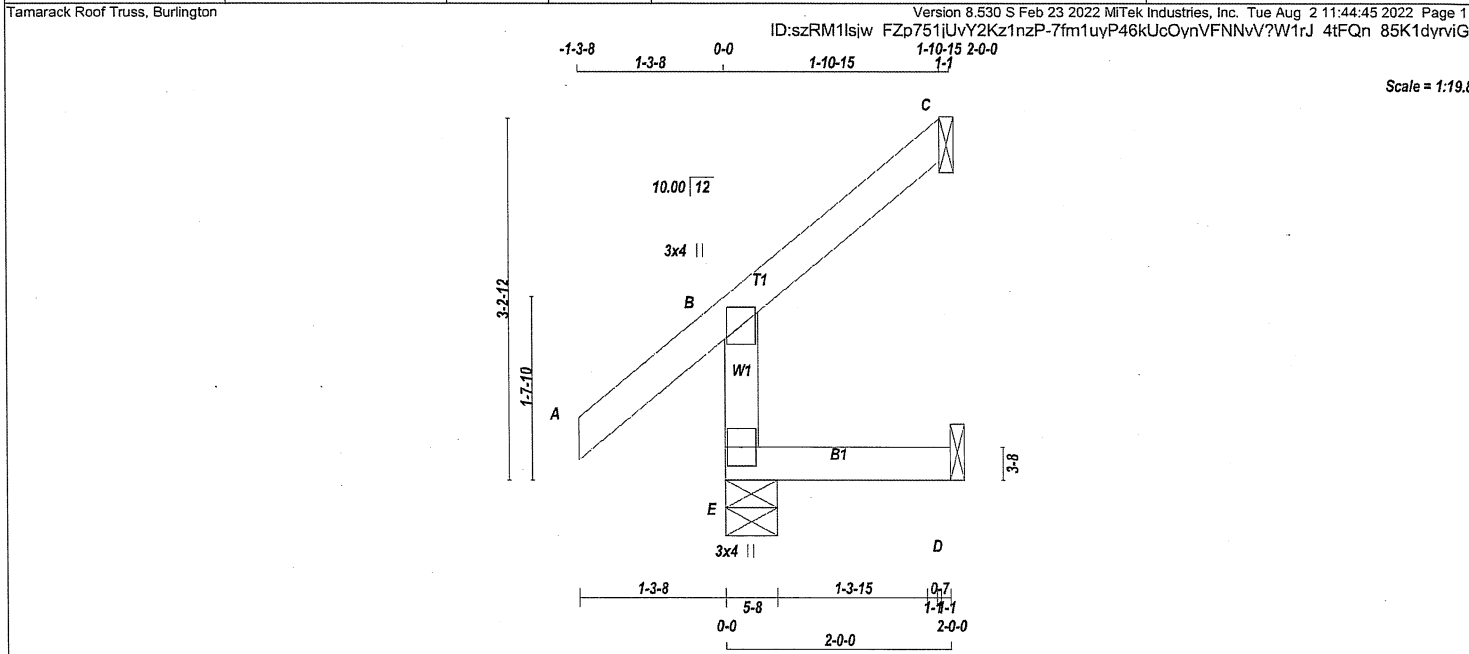
JSI GRIP= 0.12 (B) (INPUT = 0.90)

JSI METAL= 0.09 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427556	J23	1	1	TRUSS DESC.		



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES																													
CHORDS SIZE																													
E - B 2x4 DRY No.2																				SPECIFIED LOADS:									
A - C 2x4 DRY No.2																				TOP CH. LL = 26.7 PSF									
E - D 2x4 DRY No.2																				DL = 6.0 PSF									
																				BOT CH. LL = 0.0 PSF									
																				DL = 7.4 PSF									
																				TOTAL LOAD = 40.1 PSF									
DRY: SEASONED LUMBER.																													
																				SPACING = 24.0 IN./C									
																				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
																				THIS DESIGN COMPLIES WITH:									
																				- PART 9 OF BCBC 2018 , ABC 2019									
																				- PART 9 OF OBC 2012 (2019 AMENDMENT)									
																				- CSA 086-14									
																				- TPIC 2014									
																				DESIGN ASSUMPTIONS									
																				-OVERHANG NOT TO BE ALTERED OR CUT OFF.									
																				(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD									
																				ALLOWABLE DEFL.(LL)= L/360 (0.19")									
																				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")									
																				ALLOWABLE DEFL.(TL)= L/360 (0.19")									
																				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")									
																				CSI: TC=0.13/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)									
																				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
																				COMP=1.10 SHEAR=1.10 TENS= 1.10									
																				COMPANION LIVE LOAD FACTOR = 1.00									
																				AUTOSOLVE RIGHT HEEL ONLY									
																				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
																				NAIL VALUES									
																				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
																				MAX MIN MAX MIN MAX MIN									
																				MT20 650 371 1747 788 1987 1873									
																				PLATE PLACEMENT TOL. = 0.250 inches									
																				PLATE ROTATION TOL. = 5.0 Deg.									
																				JSI GRIP= 0.12 (B) (INPUT = 0.90)									
																				JSI METAL= 0.09 (B) (INPUT = 1.00)									

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Y	X			
B	TMV+p	MT20	3.0	4.0					
E	BMV1+p	MT20	3.0	4.0					

NOTES- (1)									
1) Lateral braces to be a minimum of 2X4 SPF #2.									

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C									
--	--	--	--	--	--	--	--	--	--

BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
--	--	--	--	--	--	--	--	--	--

LOADING									
TOTAL LOAD CASES: (5)									
CHORDS									
MAX. FACTORED									
FACTORED									
VERT. LOAD LC1									
MAX									
CS									
MEMB.									
FORCE									
(LBS)									
FR-TO									
E- B									
-262 / 0									
A- B									
0 / 42									
B- C									
-25 / 0									
E- D									
0 / 0									
FROM									
TO									
0.0									
0.0									
0.04 (5)									
7.81									
-95.2									
-95.2									
0.13 (1)									
10.00									
-95.2									
-95.2									
0.10 (1)									
6.25									
-18.5									
-18.5									
0.04 (5)									
10.00									

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN									
--	--	--	--	--	--	--	--	--	--

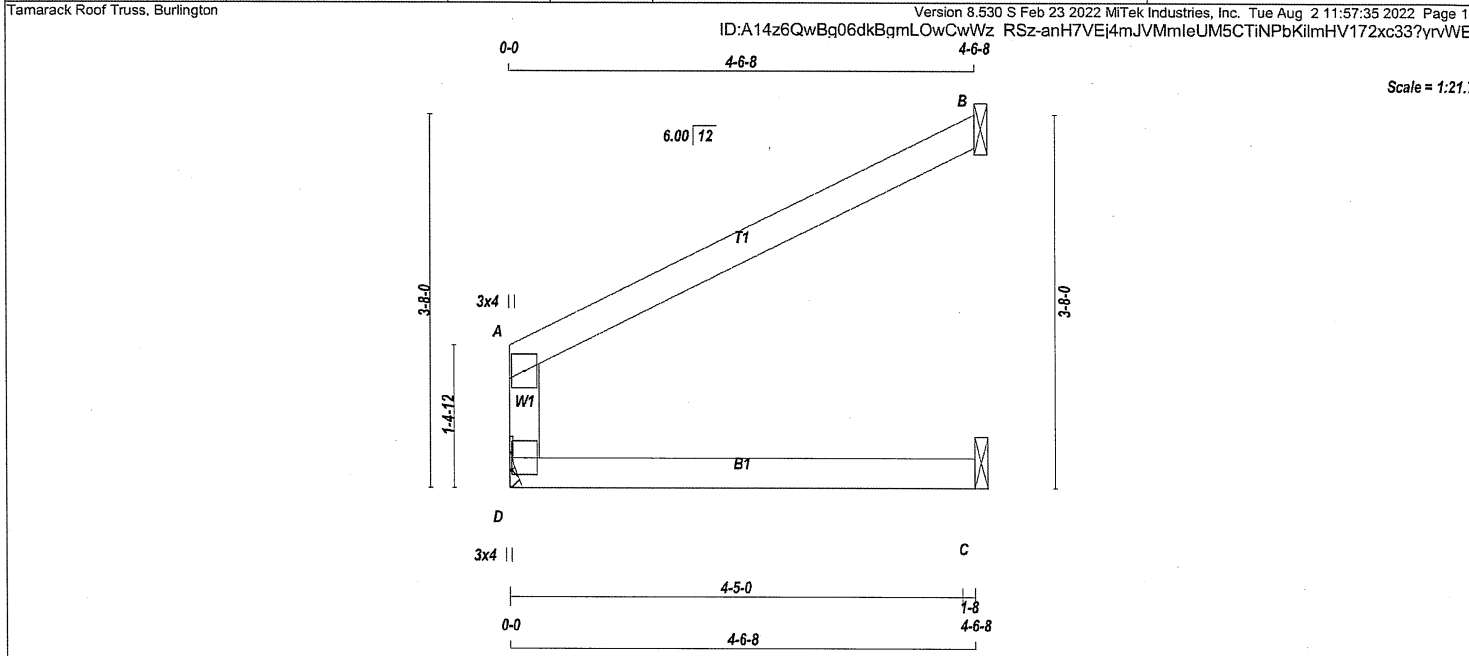
LICENSED PROFESSIONAL ENGINEER									
8/02/22									
C. M. HEYENS									
100505065									
Province of Ontario									

STRUCTURAL COMPONENT ONLY

DWG # TR22080179



STRUCTURAL COMPONENT ONLY
DWG # TR22080179

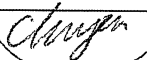


TOTAL WEIGHT = 3 X 12 = 37 I									
[M]									
LUMBER									
N. L. G. A. RULES									
CHORDS SIZE LUMBER DESCR.									
D - A 2x4 DRY No.2 SPF									
A - B 2x4 DRY No.2 SPF									
D - C 2x4 DRY No.2 SPF									
DRY: SEASONED LUMBER.									
PLATES (table is in inches)									
JT TYPE PLATES W LEN Y X									
A TMV+p MT20 3.0 4.0									
D BMV1+p MT20 3.0 4.0									
NOTES- (1)									
1) Lateral braces to be a minimum of 2X4 SPF #2.									
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED MAXIMUM FACTORED INPUT REQD									
GROSS REACTION GROSS REACTION BRG BRG									
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX									
D 258 0 258 0 0 MECHANICAL									
B 199 0 199 0 0 1-8 1-8									
C 60 0 60 0 0 1-8 1-8									
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.									
SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C									
UNFACTORED REACTIONS									
1ST LCASE MAX /MIN. COMPONENT REACTIONS									
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL									
D 182 121 / 0 0 / 0 0 / 0 0 / 0 61 / 0 0 / 0									
B 137 109 / 0 0 / 0 0 / 0 0 / 0 28 / 0 0 / 0									
C 45 12 / 0 0 / 0 0 / 0 0 / 0 33 / 0 0 / 0									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS WEBS									
MEMB. MAX. FACTORED VERT. LOAD LC1 MAX MAX. MEMB. MAX. FACTORED									
FORCE (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO FORCE MAX									
(LBS) CSI (LC)									
FR-TO FROM TO									
D- A -234 / 0 0.0 0.0 0.09 (1) 7.81									
A- B -8 / 0 -95.2 -95.2 0.27 (1) 10.00									
D- C 0 / 0 -18.5 -18.5 0.13 (1) 10.00									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 26.7 PSF									
DL = 6.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.4 PSF									
TOTAL LOAD = 40.1 PSF									
SPACING = 24.0 IN. C/C									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF CBC 2018 , ABC 2019									
- PART 9 OF OBC 2012 (2019 AMENDMENT)									
- CSA 086-14									
- TPIC 2014									
(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/360 (0.19")									
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")									
ALLOWABLE DEFL.(TL)= L/360 (0.19")									
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")									
CSI: TC=0.27/1.00 (A-B:1) , BC=0.13/1.00 (C-D:1) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (A-B:1)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
COMPANION LIVE LOAD FACTOR = 1.00									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION									
(PSI) (PLI) (PLI)									
MAX MIN MAX MIN MAX MIN									
MT20 650 371 1747 788 1987 1873									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.09 (D) (INPUT = 0.90)									
JSI METAL= 0.06 (A) (INPUT = 1.00)									

8/02/22

C. M. HEYENS

100505065



LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

DWG # TR22080194

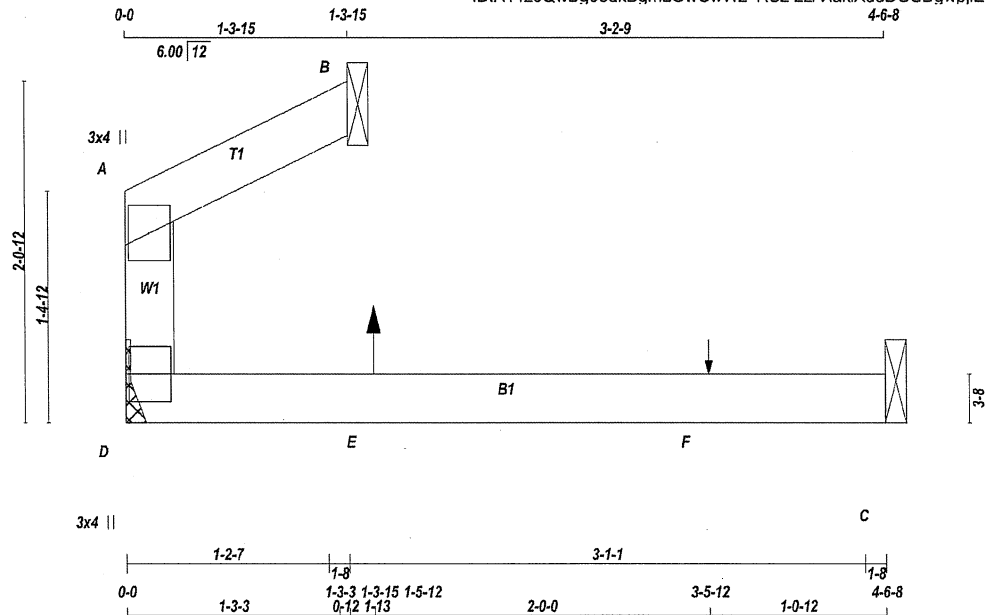


STRUCTURAL COMPONENT ONLY
DWG # TR22080194

STRUCTURAL COMPONENT ONLY
DWG # TR22080195

JOB NAME 427574	TRUSS NAME J32A	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:36 2022 Page 1
ID:A14z6QwBg06dkBgmlOWCwWz RSz-2zrViakXddDOSDgwpjIExaqS66h0xHGHbMdbRyrWWD



Scale = 1:13.4

TOTAL WEIGHT = 8 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	91	0	91	0
B	76	0	76	0
C	36	0	41	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	66	34/0	0/0	0/0	0/0	32/0	0/0
B	54	33/0	0/0	0/0	0/0	22/0	0/0
C	29	0/-2	0/0	0/0	0/0	30/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. FORCE (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. MEMB. FORCE (LBS)
FR-TO			FROM TO	CSI (LC)	UNBRAC LENGTH	FR-TO		
D-A	-52/8	0.0	0.0	0.06 (4)	7.81			
A-B	0/9	-95.2	-95.2	0.06 (4)	10.00			
D-E	0/0	-18.5	-18.5	0.09 (4)	10.00			
E-F	0/0	-18.5	-18.5	0.09 (4)	10.00			
F-C	0/0	-18.5	-18.5	0.09 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-5-12	5	1	8	FRONT	VERT	TOTAL	---	C1
F	3-5-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.06/1.00 (A-B:4), BC=0.09/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (C-D:4)

DOL LUMBER=0.88 NAIL=0.88 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

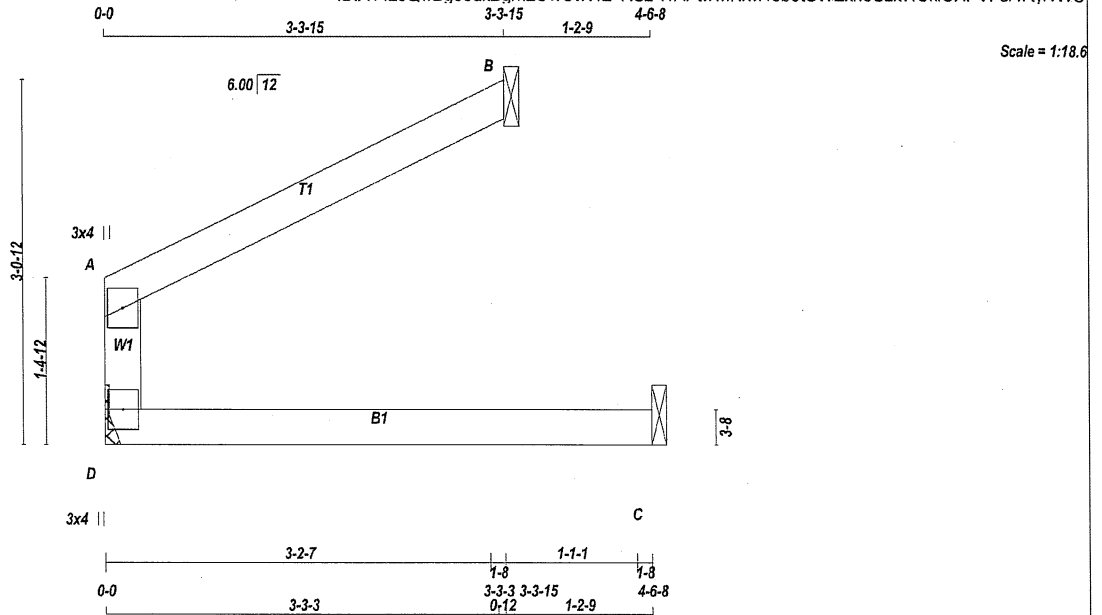
JSI GRIP= 0.02 (D) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080196

JOB NAME 427574	TRUSS NAME J33A	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:37 2022 Page 1
ID:A14z6QwBg06dkBgmLowCwWz RSz-WAPtwwlKlwl40botUWExnoUzkWSkIOXPVF5A7tyrWVC



TOTAL WEIGHT = 2 X 11 = 21 lb [M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	202	202	MECHANICAL	
B	151	151	1-8	1-8
C	47	47	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	144	91/0	0/0	0/0	0/0	53/0	0/0
B	105	82/0	0/0	0/0	0/0	23/0	0/0
C	37	5/0	0/0	0/0	0/0	31/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
D-A	-166/0	0.0	0.0	0.03 (1)	7.81	
A-B	-3/2	-95.2	-95.2	0.15 (1)	10.00	
D-C	0/0	-18.5	-18.5	0.10 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.15/1.00 (A-B:1), BC=0.10/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (D) (INPUT= 0.90)
JSI METAL= 0.05 (A) (INPUT= 1.00)

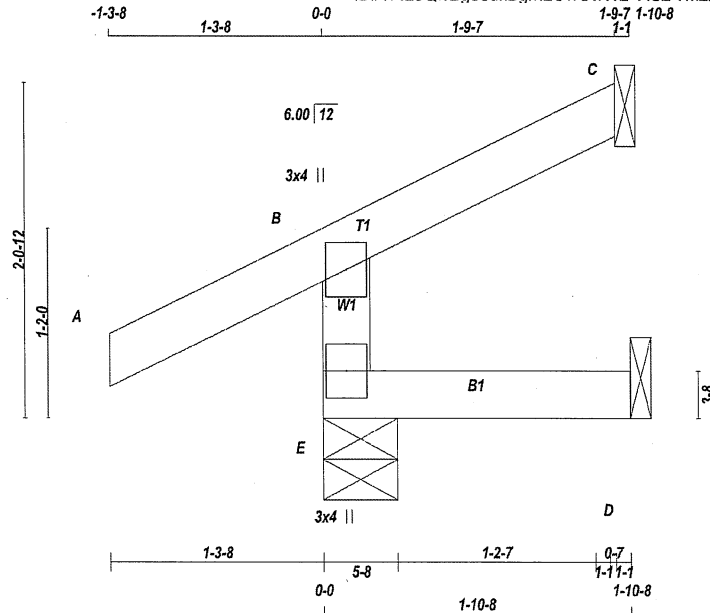


STRUCTURAL COMPONENT ONLY
DWG # TR22080197

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427574	J34	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 11:57:38 2022 Page 1
ID:A14z6QwBg06dkBgmLowCwWz RSz-7MzF7Glz3EtdIN32DIAK?19uwpwUrnZkvrkfKyrWb



Scale = 1:13.6

TOTAL WEIGHT = 2 X 7 = 14 lb [M]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	280	0	280	0	5-8	5-8
C	46	0	46	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
E	195	148 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	32	25 / -19	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-253 / 0	0.0	0.0 0.04 (5)	7.81			
A-B	0 / 29	-95.2	-95.2 0.12 (1)	10.00			
B-C	-17 / 0	-95.2	-95.2 0.09 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5)
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

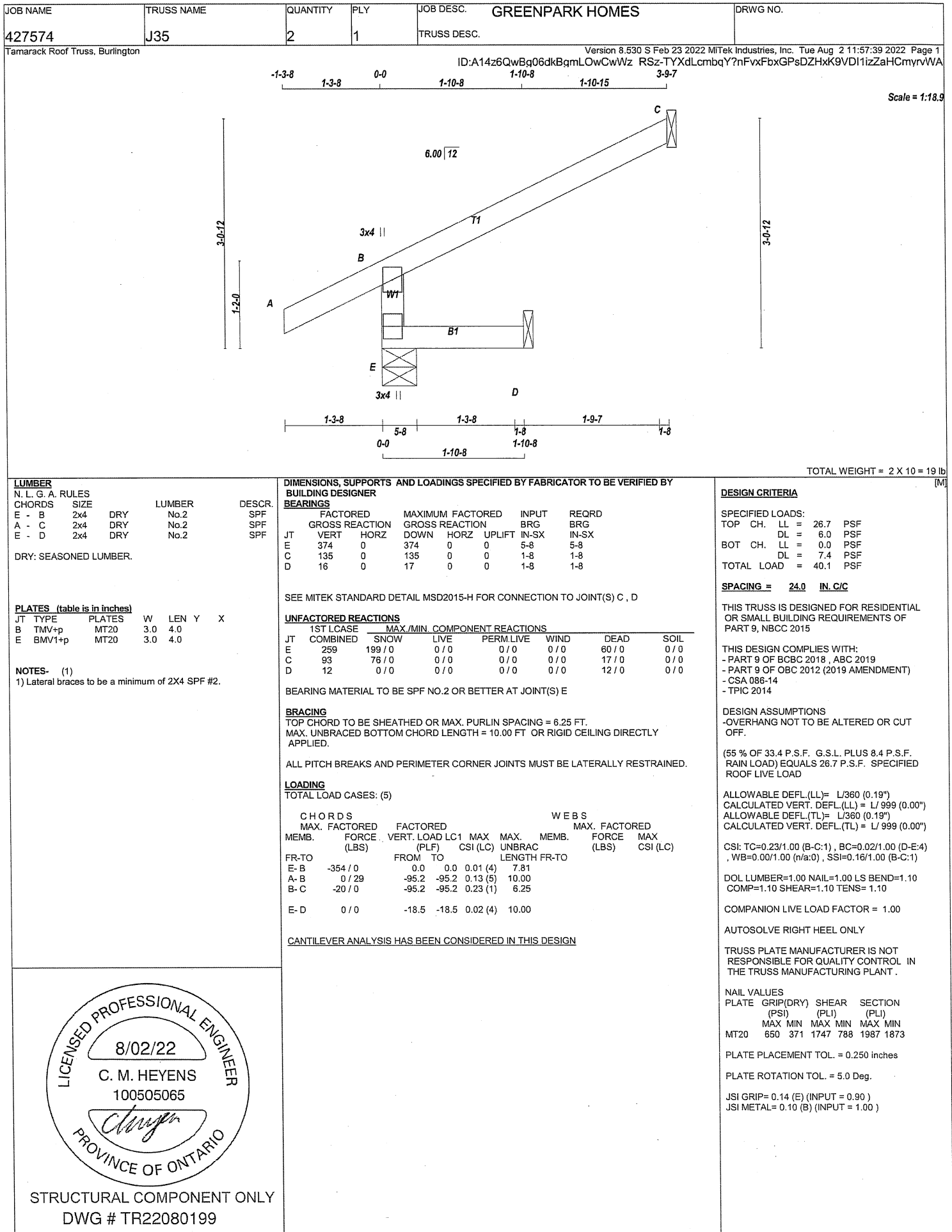
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080198



EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

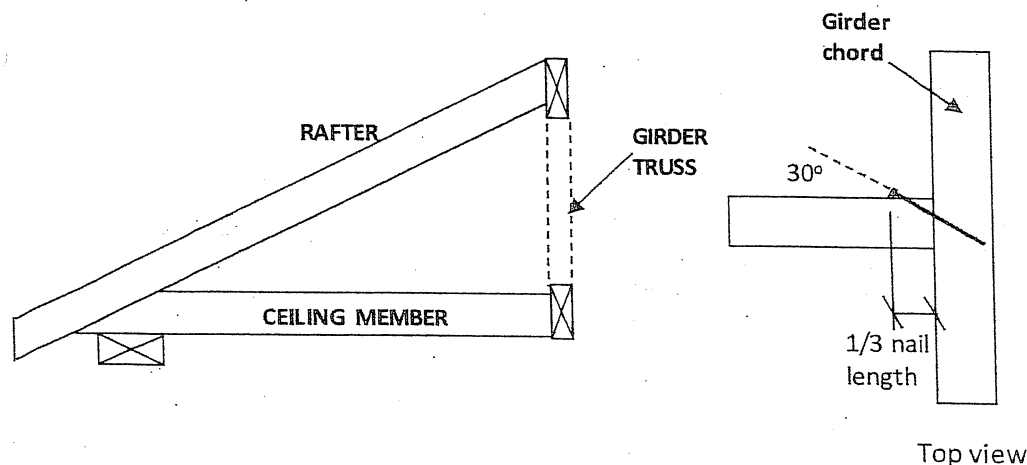
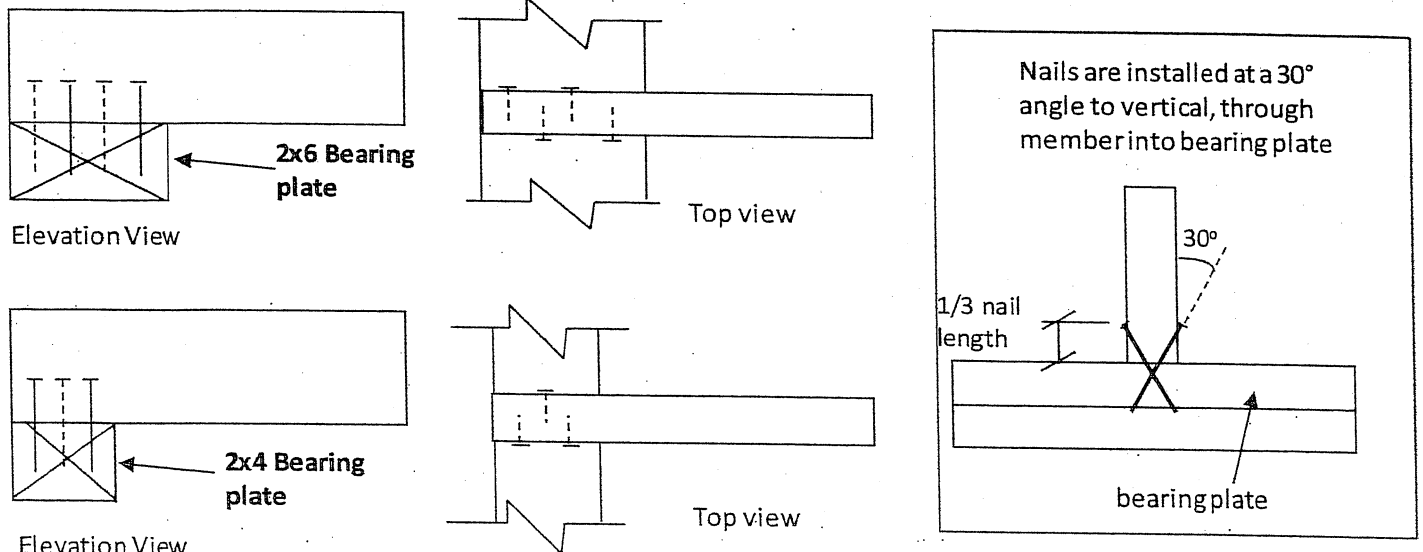


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

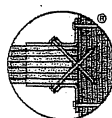
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

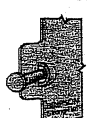
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

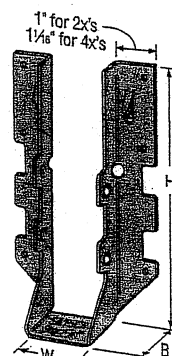
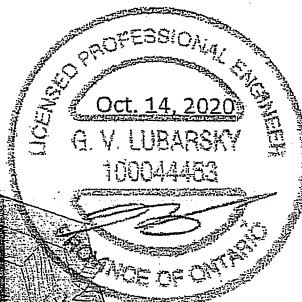
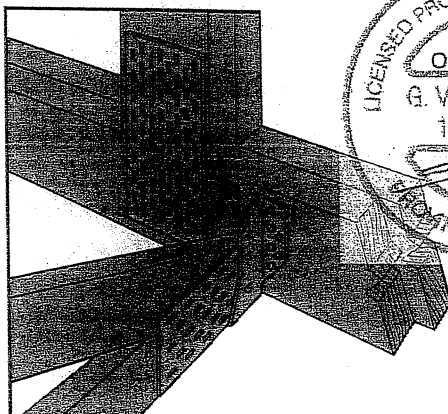


Double-Shear Nailing Side View; Do not bend tab

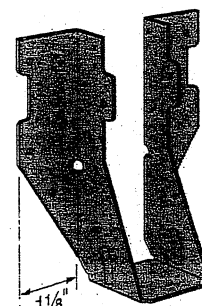


Dome Double-Shear Nailing Side View (available on some models)

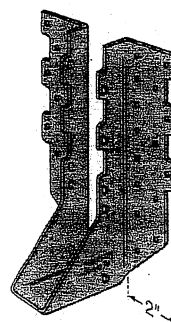
Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



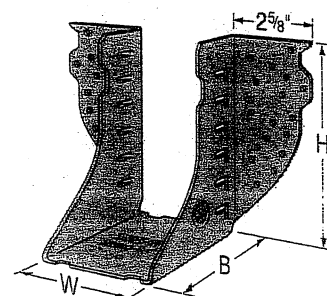
✓ LUS28



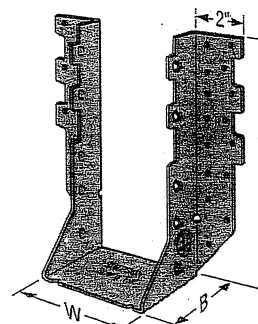
LU26L



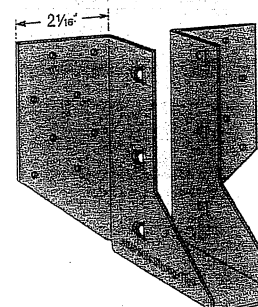
✓ HUS210
(HUS26, HUS28, and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

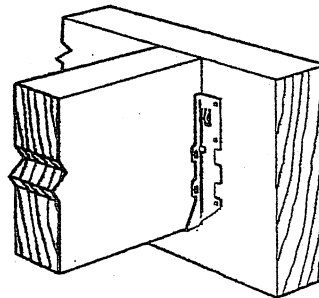
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

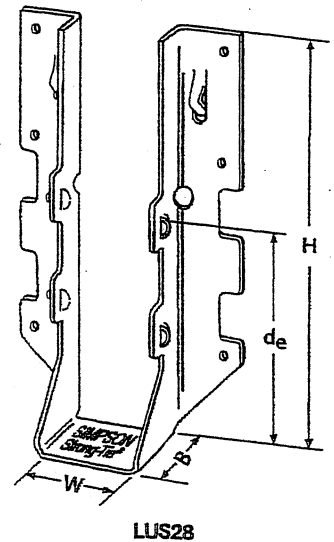
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

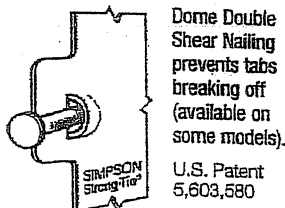


Typical LUS
Installation



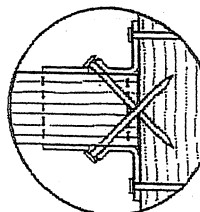
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/16	3 1/8	1 1/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/4	1 1/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 3/8	1 1/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 13/16	1 1/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

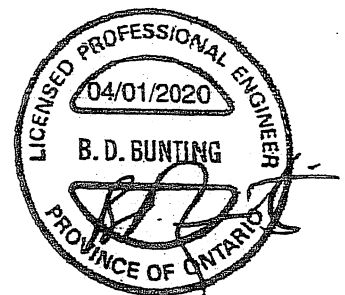


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

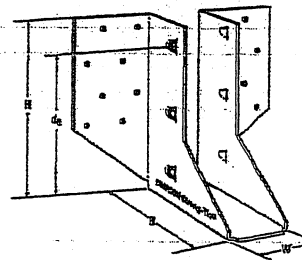
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

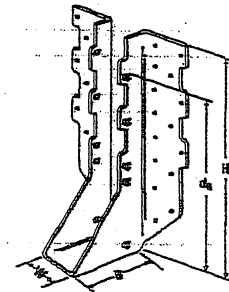
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

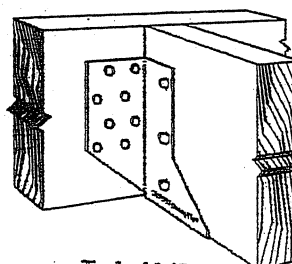
- See current catalogue for options



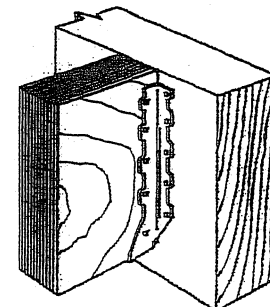
LJS26DS



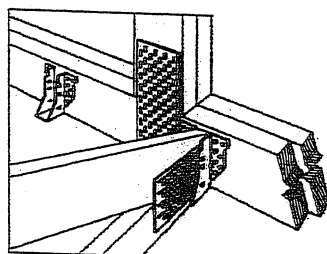
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



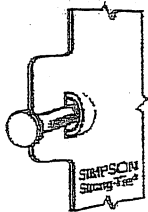
Typical HUS
Installation



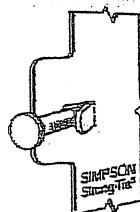
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D-Fr-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 5/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 1/2	3	6 7/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 1/2	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 7/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

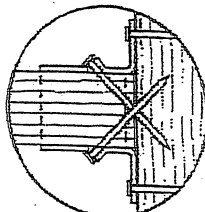
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



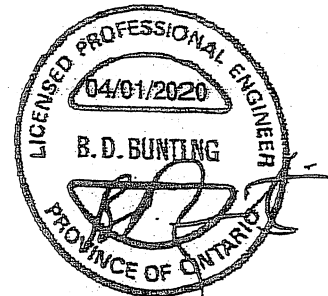
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

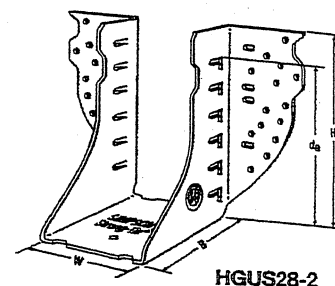
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

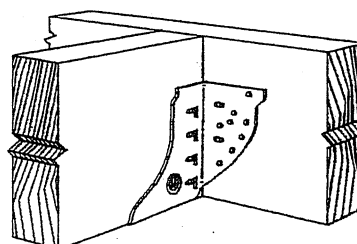
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

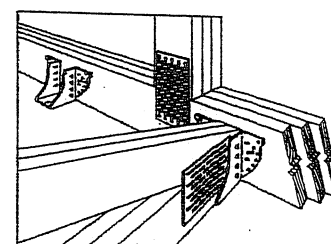
- See current catalogue for options



HGUS28-2



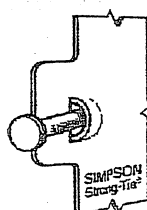
Typical HGUS Installation



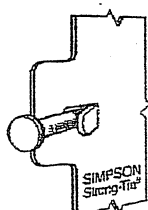
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁		Face	Joist	D.Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
HGUS26	12	1⅞	5⅞	5	4⅝		(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞		(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞		(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞		(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞		(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞		(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞		(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞		(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅞	9⅞	5	7⅞		(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅞	9⅞	4	8⅞		(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4⅞	9⅞	4	8⅞		(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞		(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞		(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞		(66) 16d	(22) 16d	10130	16400	7195	11645

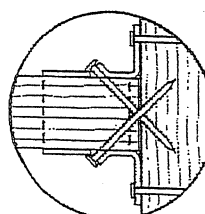
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



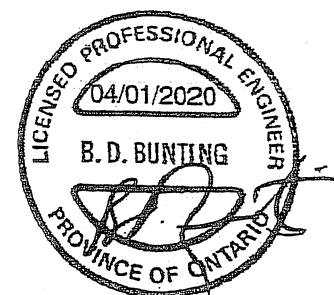
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

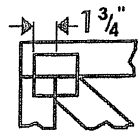
© 2020 Simpson Strong-Tie Company Inc.

T-SPECHGUS20 3/20 exp. 6/22

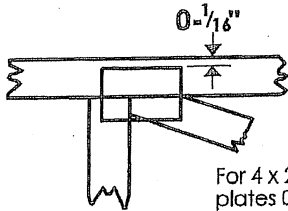
(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



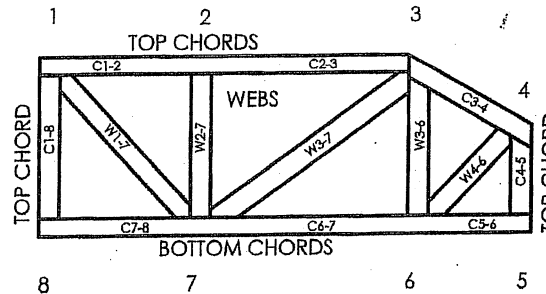
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.