

PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILTIES

THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS ONLY LIMITED TO THE CALCULATION OF THIS BUILDING COMPONENT FOR THE LOADS AND CONDITIONS SHOWN ON THIS DRAWING.

THE RESPONSIBILITY OF THE UNDERSIGNED IS LIMITED TO THE VERIFICATION OF THE STRUCTURAL CAPACITY OF THE FLOOR JOISTS AND LVL BEAMS BASED ON PLACEMENT AS SHOWN ON THE LAYOUT. THE LOADS APPLIED ARE LIMITED TO THE GRAVITY EFFECTS OF THE SPECIFIED LOADS. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE EFFECT OF WIND, UPLIFT, SEISMIC, LATERAL OR OTHER FORCES, CALCULATION OF ADEQUATE SUPPORT AND ANCHORAGE OF COMPONENTS, AS WELL AS THE DIMENSIONS AND DESIGN LOADS USED TO CALCULATE COMPONENTS ARE THE RESPONSIBILITY OF THE OVERALL BUILDING DESIGNER. FLOOR JOISTS AND OSB RIM BOARD ARE DESIGNED TO CARRY UNIFORMLY DISTRIBUTED LOADS ONLY. POINT LOADS SHOULD BE TRANSFERRED THROUGH THE FLOOR CAVITY WITH TRANSFER BLOCKS. STRUCTURAL ELEMENTS SUCH AS WALLS, POSTS, CONNECTORS, AND TRANSFER BLOCKS ARE THE RESPONSIBILITY OF THE OVERALL BUILDING DESIGNER.

THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF BEING FURNISHED FAULTY OR INCORRECT INFORMATION, SPECIFICATIONS AND/OR DESIGNS.

COMPONENT DESIGN INFORMATION

- 1. THIS BUILDING COMPONENT IS CERTIFIED AS AN INDIVIDUAL COMPONENT FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE BASED ON INFORMATION PROVIDED BY KOTT DESIGN.
- 2. THE BUILDING COMPONENT USED IN CONSTRUCTION MUST BE THE SAME AS INDICATED ON THE DRAWINGS.
- 3. UNLESS NOTED OTHERWISE ON THE LAYOUT OR BEAM CALCULATION SHEET, MEMBERS CONSISTING OF MULTIPLE PLIES MUST BE CONNECTED AS PER THE DOCUMENT "MULTIPLE MEMBER CONNECTION DETAILS" SHOWN ON PAGE 2 OF THIS DOCUMENT.
- 4. PASS-THRU TRANSFER BLOCK FRAMING IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.
- 5. IT IS ASSUMED THAT EACH LVL BEAM WHERE NOT SEATED IN A HANGER IS ATTACHED USING (4) FOUR 3-1/4" COMMON SPIRAL NAILS FOR UP TO 5.5" LONG BEARINGS AND USING (6) SIX 3-1/4" COMMON SPIRAL NAILS FOR BEARINGS EQUAL TO OR LONGER THAN 5.5", UNLESS INDICATED OTHERWISE.

CODE

THIS BUILDING COMPONENT IS DESIGNED IN ACCORDANCE WITH THE NATIONAL BUILDING CODE OF CANADA, THE ONTARIO BUILDING CODE, CCMC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

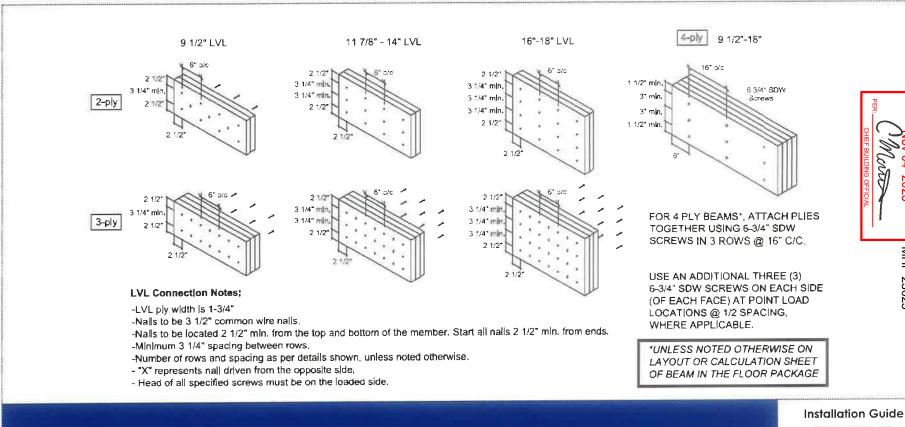
HANDLING AND INSTALLATION

- 1. DO NOT DRILL ANY HOLE, CUT OR NOTCH A CERTIFIED BUILDING COMPONENT WITHOUT A WRITTEN PRE-AUTHORIZATION.
- 2. INSTALLATION AND ASSEMBLY OF FLOOR JOISTS AND LVL BEAMS IS TO BE CARRIED OUT IN ACCORDANCE WITH THE CURRENT EDITION OF THE MANUFACTURER'S LITERATURE.

MULTIPLE MEMBER CONNECTIONS FOR BEAMS SHOWN ON KOTT LAYOUTS



MULTIPLE MEMBER CONNECTIONS FOR UNIFORMLY DISTRIBUTED TOP & SIDE LOADED LVL BEAMS SHOWN ON KOTT LAYOUTS

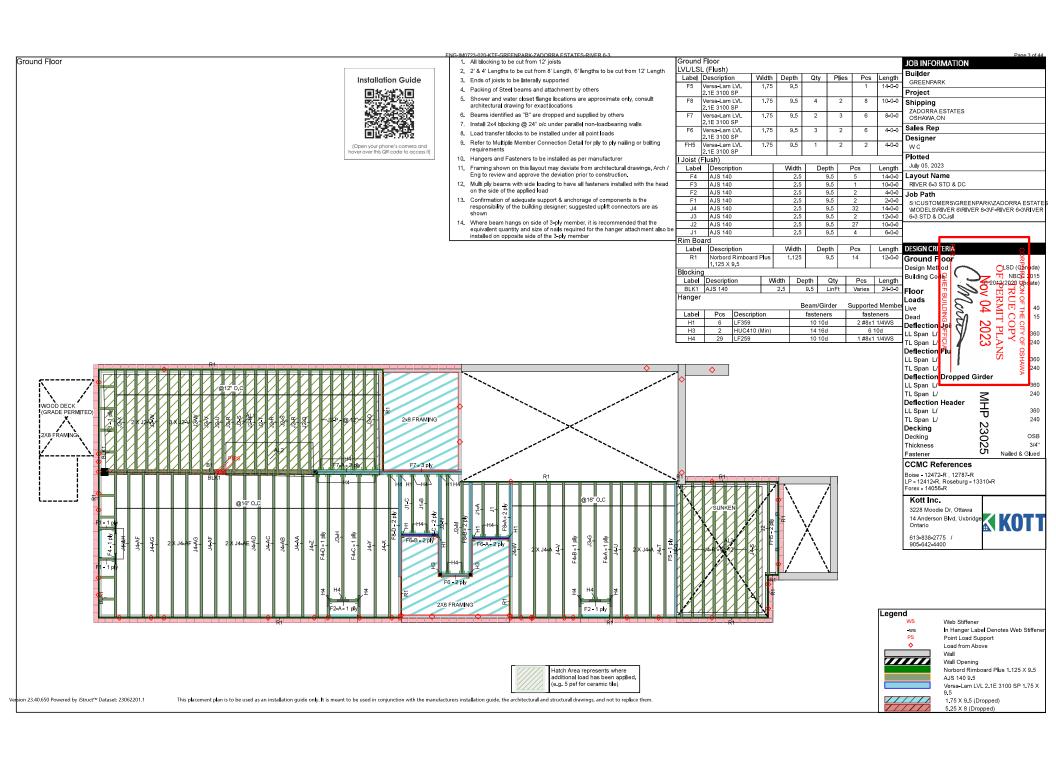


FOR MULTIPLE MEMBER CONNECTION OF BOISE ALLJOISTS REFER TO THE BOISE CASCADE INSTALLATION GUIDE

MHP 23025



(Open your phone's camera and hover over this QR code to access it!





AJS 140

F1

Client: Project: Address:

9.500" - PA

GREENPARKE THE CITY OF OSHAWA ZADORRANESTATES ZADORRAPSTATESNS OSHANOV 04 2023

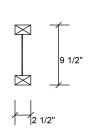
7/3/2023 Date: W C Input by:

Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Level: Ground Floor

1 1 SPF 0-2-6 (LF259) 0



0

n

Wind

0

0

Member Information

1'4 5/8 1'4 5/8'

Type: Plies: 1 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal - II

General Load Floor Live: 40 PSF 15 PSF Dead:

Application: Floor (Residential) Design Method: LSD

> **Building Code: NBCC 2015** OBC 2012(2020 Update)

Load Sharing: Not Checked Deck: Vibration: Not Checked **Unfactored Reactions UNPATTERNED lb (Uplift)** Brg Direction Live Dead Snow Vertical 40 15 1

38

Bearings and Factored Reactions

2

Vertical

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 79 L 1 - SPF 2.375" Vert 5% 19 / 60 1.25D+1.5L 2 -2.000" Vert 5% 18 / 57 75 L 1.25D+1.5L Hanger

14

Analysis Results

Ana l ysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	18 ft-lb	8 1/2"	4095 ft-lb	0.004 (0%)	1.25D+1.5L	L
Unbraced	18 ft-lb	8 1/2"	4095 ft-lb	0.004 (0%)	1.25D+1.5L	L
Shear	64 lb	1'3 3/8"	1830 l b	0.035 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/199583)	8 1/2"	0.038 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/74844)	8 1/2"	0.038 (L/360)	0.005 (0%)	L	L
TL Defl inch	0.000 (L/54432)	8 1/2"	0.057 (L/240)	0.004 (0%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Right Header: SPF, Thickness: 2 1/2"
- 4 Girders are designed to be supported on the bottom edge only.
- 5 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 6 If attached to the hottom flange, hottom flange must be laterally braced at

ır sneatr maximui	ning is not attached to the m 2' o.c.	e bottom flange, botto	m flange must	pe laterali	y braced at				
D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	Tie-In	0-0-0 to 1-4-10	1-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

ID

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads,

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc. 3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400







AJS 140

Client: Project: Address:

GREENPARKE THE CITY OF OSHAWA ZADORRARESTATES ZADORRAPSTATESNS OSHANOV 04 2023 9.500" - PA

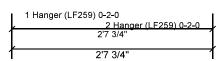
Date: 7/3/2023 Input by: W C

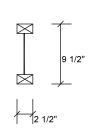
Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Level: Ground Floor

2 1





Wind

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition: Deflection LL:	Dry 360	Building Code:	NBCC 2015 OBC 2012(2020 Update)
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Direction Live Dead

1	Vertical	199	75	0	0
2	Vertical	190	71	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	414 ft-lb	1'3 7/16"	4095 ft-lb	0.101 (10%)	1.25D+1.5L	L
Unbraced	414 ft-lb	1'3 7/16"	4095 ft-lb	0.101 (10%)	1.25D+1.5L	L
Shear	386 lb	1 1/4"	1830 l b	0.211 (21%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/15990)	1'3 7/16"	0.081 (L/360)	0.023 (2%)	D	Uniform
LL Defl inch	0.005 (L/5973)	1'3 7/16"	0.081 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.007 (L/4349)	1'3 7/16"	0.122 (L/240)	0.055 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap. R	eact D/L I b	Total	Ld. Case	Ld. Comb.
1 -	2.000"	Vert	25%	93 / 299	392	L	1.25D+1.5L
Hanger							
2 -	2.000"	Vert	24%	89 / 285	374	L	1.25D+1.5L
Hanger							



- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: SPF, Thickness: 2 1/2"
- 4 Right Header: SPF, Thickness: 2 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 7 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-7-12	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-3-7		Far Face	115 b	308 lb	0 lb	0 l b	J3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

 3. Damaged Jioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent ponding.

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.



Page 3 of 40



Client: Project: Address:

GREENPARKE THE CITY OF OSHAWA ZADORRATESTATES ZADORRAPSTATESNS OSHANOV-04 202

7/3/2023 Date: W C Input by:

Job Name: RIVER 6-3 STD & DC

Level: Ground Floor

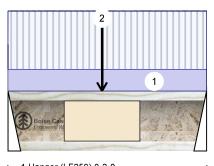
MHP 23025pject #

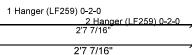
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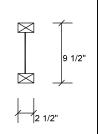
Member Information

9.500" -









Wind

0

0

Application: Floor (Residential) Type: Plies Design Method: LSD Moisture Condition: Dry **Building Code: NBCC 2015** OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF Dead:

	Unfactored Reactions UNPATTERNED lb (Uplift)											
	Brg	Direction	Live	Dead	Snow							
	1	Vertical	213	80	0							
:)	2	Vertical	194	73	0							

Analysis Results Comb. Case Location Allowed Analysis Actual Capacity 1'2 7/8" 4095 ft-lb Moment 429 ft-lb 0.105 (10%) 1.25D+1.5L L Unbraced 429 ft-lb 1'2 7/8" 4095 ft-lb 0.105 (10%) 1.25D+1.5L L 0.225 (23%) 1.25D+1.5L L 412 lb 1 1/4" 1830 lb Shear Perm Defl in 0.002 1'2 7/8" 0.080 (L/360) 0.024 (2%) D Uniform (L/15298) LL Defl inch 0.005 (L/5726) 1'2 7/8" 0.080 (L/360) 0.063 (6%) L L TL Defl inch 0.007 (L/4167) 1'2 7/8" 0.121 (L/240) 0.058 (6%) D+L

Bearings and Factored Reactions Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 _ 2.000" Vert 26% 100 / 319 419 L 1.25D+1.5L Hanger 2 -2.000" Vert 24% 91 / 291 382 L 1.25D+1.5L Hanger



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Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: SPF, Thickness: 2 1/2"
- 4 Right Header: SPF, Thickness: 2 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 7 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-7-7	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-14		Far Face	122 b	326 lb	0 l b	0 lb	J3

L

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.





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F3

Client: Project: Address:

9.500" - PA

GREENPARKE THE CITY OF OSHAWA ZADORRARESTATES ZADORRAPSTATES OSHANOV-04 2023

Date: 7/3/2023 Input by: W C

Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Level: Ground Floor

4 3 9 1/2" 1 SPF 0-2-10 2 SPF 0-2-6 9'4 11/16'

9'4 11/16'

Member Inform	ation			Unfa	actored Rea	ctions UNP	ATTERNED I	b (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	Vertical	229	114	0	0
Moisture Condition:	Dry	Building Code:	NBCC 2015	2	Vertical	228	111	0	0
Deflection LL:	360		OBC 2012(2020 Update)						
Deflection TL:	240	Load Sharing:	No						
Importance:	Normal - II	Deck:	Not Checked						
General Load		Vibration:	Not Checked						
Floor Live:	40 PSF			Bear	rings and Fa	actored Rea	ctions		
Dead:	15 PSF			Bea	aring Length	Dir. Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 -	SPF 2.625"	Vert 29%	143 / 343	486 L	1.25D+1.5L
Analysis Results				2 -	SPF 2.375"	Vert 29%	139 / 342	481 L	1.25D+1.5L

Analysis Results

Ana l ysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1068 ft-lb	4'8 7/16"	4095 ft-lb	0.261 (26%)	1.25D+1.5L	L
Unbraced	1068 ft-lb	4'8 7/16"	4095 ft-lb	0.261 (26%)	1.25D+1.5L	L
Shear	470 lb	1 7/8"	1830 l b	0.257 (26%)	1.25D+1.5L	L
Perm Defl in	0.025 (L/4340)	4'8 1/2"	0.303 (L/360)	0.083 (8%)	D	Uniform
LL Defl inch	0.051 (L/2162)	4'8 1/2"	0.303 (L/360)	0.167 (17%)	L	L
TL Defl inch	0.076 (L/1443)	4'8 1/2"	0.455 (L/240)	0.166 (17%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o c
- 4 Bottom flange must be laterally braced at bearings.



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I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 9-4-11	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 9-4-11	0-5-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-0 to 8-11-15		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-0-0 to 8-11-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

Manufacturer Info

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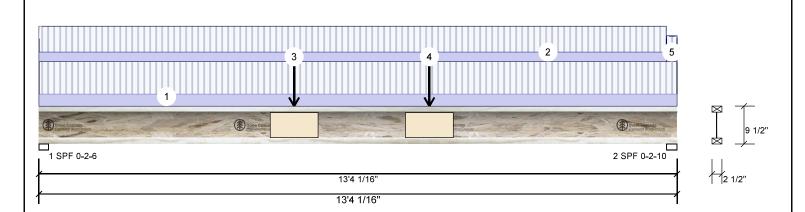
GREENPARKE THE CITY OF OSHAWA ZADORRANESTATES ZADORRAPSTATESNS OSHANOV-04 2023 9.500" - PA

Date: 7/3/2023 W C Input by:

Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Application: Floor (Residential) Wind Type: Brg Direction Live Dead Snow Plies: Design Method: LSD Vertical 406 152 0 1 0 Moisture Condition: Dry **Building Code: NBCC 2015** 2 Vertical 407 152 0 0 OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF 15 PSF Dead: Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" Vert 48% 190 / 609 799 L 1.25D+1.5L 2 - SPF 2.625" Vert 47% 190 / 610 800 I 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2699 ft-lb	6'7 13/16"	4095 ft-lb	0.659 (66%)	1.25D+1.5L	L
Unbraced	2699 ft-lb	6'7 13/16"	4095 ft-lb	0.659 (66%)	1.25D+1.5L	L
Shear	785 l b	13'2 3/16"	1830 lb	0.429 (43%)	1.25D+1.5L	L
Perm Defl in.	0.095 (L/1643)	6'7 15/16"	0.435 (L/360)	0.219 (22%)	D	Uniform
LL Defl inch	0.255 (L/614)	6'7 15/16"	0.435 (L/360)	0.586 (59%)	L	L
TL Defl inch	0.350 (L/447)	6'7 15/16"	0.652 (L/240)	0.537 (54%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o c
- 4 Bottom flange must be laterally braced at a maximum of 5'4" o.c.



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T Dolloin Iding	ic mast be laterally bi	acca at a maximum	010 + 0.0.						
I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-4-1	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-1-7	0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	5-4-0		Far Face	14 l b	38 l b	0 l b	0 l b	F1
4	Point	8-1-15		Far Face	14 lb	38 lb	0 l b	0 l b	F1
5	Tie-In	13-1-7 to 13-4-1	0-5-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc. 3228 Moodie Dr, Ottawa, Ontario

613-838-2775 / 905-642-4400







GREENPARKE THE CITY OF OSHAWA ZADORRANESTATES ZADORRAPSTATESNS OSHANOV-04 2023

Date: 7/3/2023 W C Input by:

Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Brg

1

2

Direction

Vertical

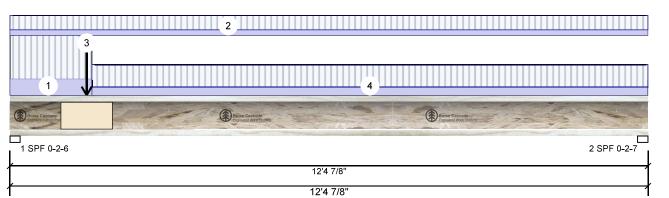
Vertical

Level: Ground Floor

AJS 140

9.500" -





Unfactored Reactions UNPATTERNED Ib (Uplift) Wind Dead Snow 191 0 0 122 0 0

Application: Floor (Residential) Type: Plies: Design Method: LSD Moisture Condition: Dry **Building Code: NBCC 2015** OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF Dead:

Bearings and Factored Reactions Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" Vert 61% 239 / 767 1006 L 1.25D+1.5L 2 - SPF 2.438" Vert 38% 152 / 486 638 1.25D+1.5L

Live

511

324

Analysis Results

Member Information

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	2048 ft-lb	5'8 9/16"	4095 ft-lb	0.500 (50%)	1.25D+1.5L	L
Unbraced	2048 ft-lb	5'8 9/16"	4095 ft-lb	0.500 (50%)	1.25D+1.5L	L
Shear	985 l b	1 5/8"	1830 lb	0.538 (54%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/2238)	6' 7/16"	0.404 (L/360)	0.161 (16%)	D	Uniform
LL Defl inch	0.174 (L/839)	6' 7/16"	0.404 (L/360)	0.429 (43%)	L	L
TL Defl inch	0.239 (L/610)	6' 7/16"	0.606 (L/240)	0.393 (39%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o c
- 4 Bottom flange must be laterally braced at a maximum of 10'11" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-2	1-5-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-4-14	0-5-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-14		Far Face	71 l b	190 lb	0 l b	0 lb	F2
4	Tie-In	1-7-2 to 12-4-14	0-8-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc. 3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400







GREENPARKE THE CITY OF OSHAWA ZADORRAPESTATES ZADORRAPSTATESNS OSHANOV-04 2023

Date: 7/3/2023 W C Input by:

Job Name: RIVER 6-3 STD & DC

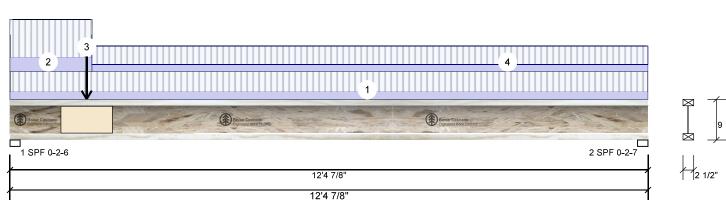
MHP 23025pject #

Level: Ground Floor

AJS 140

9.500" -





Floor (Residential)

OBC 2012(2020 Update)

NBCC 2015

Not Checked

Not Checked

LSD

Member Information Type:

Plies: Moisture Condition: Dry Deflection LL: 360 240

Deflection TL: Importance: Normal - II General Load

Floor Live: 40 PSF 15 PSF Dead:

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	582	219	0	0
2	Vertical	386	145	0	0

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap. F	React D/L I b	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	Vert	69%	273 / 873	1146	L	1.25D+1.5L
2 - SPF	2.438"	Vert	46%	181 / 579	760	L	1.25D+1.5L

Analysis Results

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	2415 ft-lb	5'9 5/16"	4095 ft-lb	0.590 (59%)	1.25D+1.5L	L
Unbraced	2415 ft-lb	5'9 5/16"	4095 ft-lb	0.590 (59%)	1.25D+1.5L	L
Shear	1123 lb	1 5/8"	1830 l b	0.614 (61%)	1.25D+1.5L	L
Perm Defl in.	0.077 (L/1898)	6' 11/16"	0.404 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.204 (L/712)	6' 11/16"	0.404 (L/360)	0.505 (51%)	L	L
TL Defl inch	0.281 (L/518)	6' 11/16"	0.606 (L/240)	0.463 (46%)	D+L	L

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Vibration:

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o c

4 Bottom flange must be laterally braced at a maximum of 10'11" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 12-4-14	0-9-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 1-7-2	1-5-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-5-14		Near Face	75 l b	199 l b	0 lb	0 l b	F2	
4	Tie-In	1-7-2 to 12-4-14	0-8-6	Top	15 PSF	40 PSF	0 PSF	0 PSF		

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be cut or drilled

 2. Refer to latest copy of the Lioist product information details for framing details. stifferer tables, web hole chart, bridging details. multi-qly fastening details and handling/erection details

 3. Damaged Lioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.





Wind

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

0

0

Snow 0

0

Ld. Case

L

Page 8 of 40



Client: Project: Address:

GREENPARKE THE CITY OF OSHAWA ZADORRARESTATES ZADORRAPSTATESNS OSHANOV-04 2023

Date: 7/3/2023 Input by: W C

Job Name: RIVER 6-3 STD & DC

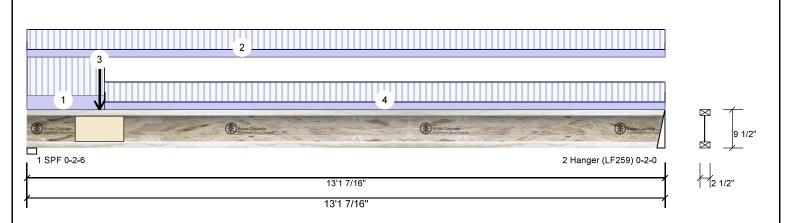
Unfactored Reactions UNPATTERNED Ib (Uplift)

MHP 23025pject #

Level: Ground Floor

AJS 140 9.500" -





Type:	Girder	Application:	Floor (Residential)	Brg I	Direction		Live	Dead		5
Plies:	1	Design Method:	LSD	1 1	/ertical		604	227		
Moisture Condit Deflection LL: Deflection TL: Importance:	tion: Dry 360 240 Normal - II	Building Code: Load Sharing: Deck:	NBCC 2015 OBC 2012(2020 Update) No Not Checked	2	/ertical		411	154		
General Load		Vibration:	Not Checked							_
Floor Live:	40 PSF			Bearii	ngs and Fa	actore	d React	tions		
Dead:	15 PSF			Beari	ng Length	Dir.	Cap. F	React D/L I b	Total	ī
				1 - SF	PF 2.375"	Vert	72%	283 / 907	1190	L
Analysis Res	u it s			2 - Hang	2.000" er	Vert	51%	192 / 616	808	L

Member Information

Ana l ysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	2716 ft-lb	6'2 5/16"	4095 ft-lb	0.663 (66%)	1.25D+1.5L	L
Unbraced	2716 ft-lb	6'2 5/16"	4095 ft-lb	0.663 (66%)	1.25D+1.5L	L
Shear	1167 lb	1 5/8"	1830 lb	0.637 (64%)	1.25D+1.5L	L
Perm Defl in.	0.096 (L/1612)	6'5 3/8"	0.429 (L/360)	0.223 (22%)	D	Uniform
LL Defl inch	0.256 (L/605)	6'5 3/8"	0.429 (L/360)	0.595 (60%)	L	L
TL Defl inch	0.351 (L/440)	6'5 3/8"	0.644 (L/240)	0.546 (55%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Right Header: DF, Thickness: 5 1/4"
- 4 Girders are designed to be supported on the bottom edge only.
- 5 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

6 Bottom flange must be laterally braced at a maximum of 11'7 9/16" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-2	1-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-1-7	0-8-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-14		Far Face	73 lb	194 l b	0 l b	0 l b	F2
4	Tie-In	1-7-2 to 13-1-7	0-8-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Joist flanges must not be cut or drilled

 2. Refer to latest copy of the Juoist product information details for framing details. stifferer tables, web hole chart, bridging details, multi-ray fastening details and handling/erection details

 3. Damaged Juoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent ponding.

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.





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Client: Project: Address:

GREENPARKE THE CITY OF OSHAWA ZADORRARESTATES ZADORRAPSTATESNS OSHANOV-04 2023

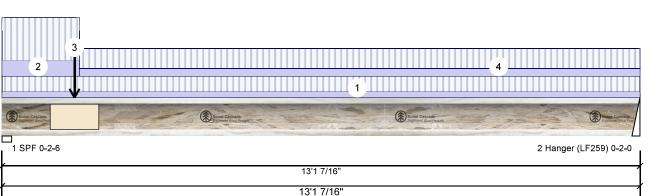
Date: 7/3/2023 Input by: W C

Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Level: Ground Floor

AJS 140 9.500" -



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015
Deflection LL:	360		OBC 2012(2020 Update)
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	547	205	0	0
2	Vertical	335	126	0	0

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2263 ft-lb	6' 5/8"	4095 ft-lb	0.553 (55%)	1.25D+1.5L	L
Unbraced	2263 ft-lb	6' 5/8"	4095 ft-lb	0.553 (55%)	1.25D+1.5L	L
Shear	1057 l b	1 5/8"	1830 lb	0.577 (58%)	1.25D+1.5L	L
Perm Defl in.	0.080 (L/1929)	6'4 13/16"	0.429 (L/360)	0.187 (19%)	D	Uniform
LL Defl inch	0.214 (L/723)	6'4 13/16"	0.429 (L/360)	0.498 (50%)	L	L
TL Defl inch	0.294 (L/526)	6'4 13/16"	0.644 (L/240)	0.456 (46%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap.	React D/L I b	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	Vert	65%	257 / 820	1077	L	1.25D+1.5L
2 - Hanger	2.000"	Vert	42%	157 / 503	660	L	1.25D+1.5L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Right Header: DF, Thickness: 5 1/4"

15 PSF

- 4 Girders are designed to be supported on the bottom edge only.
- 5 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

6 Bottom flange must be laterally braced at a maximum of 11'7 9/16" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 13-1-7	0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 1-7-2	1-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Point	1-5-14		Near Face	80 lb	213 lb	0 l b	0 l b	F2
	4	Tie-In	1-7-2 to 13-1-7	0-8-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation

 1. Lioist flanges must not be cut or drilled

 2. Refer to latest copy of the Lioist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ray fastening details and handling/erection details

 3. Damaged Lioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

 7. 230622011

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.



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F5

Client: Project: Address:

GREENPARKE THE CITY OF OSHAWA ZADORRAPESTATES Y ZADORRAPSTATES OSHANOV 04 2023

Date: 7/3/2023 Input by: WС

Job Name: RIVER 6-3 STD & DC

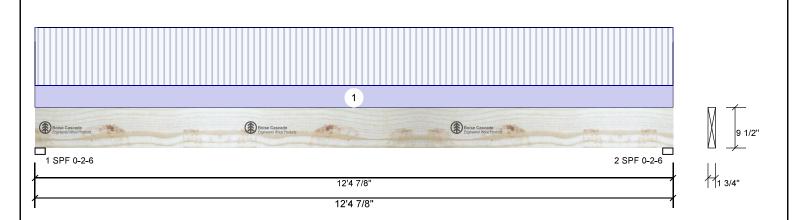
MHP 23025pject #

Versa-Lam LVL 2.1E 3100 SP



- PASSED

Level: Ground Floor



Member Inform	nation			Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Direction	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	Vertical	120	74	0	0
Moisture Condition:	: Dry	Building Code:	NBCC 2015	2	Vertical	120	74	0	0
Deflection LL:	360		OBC 2012(2020 Update)						
Deflection TL:	240	Load Sharing:	No						
Importance:	Normal - II	Deck:	Not Checked						
General Load		Vibration:	Not Checked						
Floor Live:	40 PSF			Bear	ings and Fa	actored Read	ctions		
Dead:	15 PSF			Bea	aring Length	Dir. Cap.	React D/L Ib	Total Ld. Case	Ld. Comb.
				1 - 3	SPF 2.375"	Vert 11%	93 / 180	273 L	1.25D+1.5L
A l				2 -	SPF 2.398"	Vert 11%	93 / 180	273 L	1.25D+1.5L

Analysis Results

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	809 ft-lb	6'2 7/16"	11610 ft-lb	0.070 (7%)	1.25D+1.5L	L
Unbraced	809 ft-lb	6'2 7/16"	11610 ft-lb	0.070 (7%)	1.25D+1.5L	L
Shear	234 lb	11 7/8"	5287 lb	0.044 (4%)	1.25D+1.5L	L
Perm Defl in.	0.022 (L/6502)	6'2 7/16"	0.404 (L/360)	0.055 (6%)	D	Uniform
LL Defl inch	0.036 (L/4030)	6'2 7/16"	0.404 (L/360)	0.089 (9%)	L	L
TL Defl inch	0.059 (L/2488)	6'2 7/16"	0.607 (L/240)	0.096 (10%)	D+L	L

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

Location

0-0-0 to 12-4-14 0-5-13

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top must be continuously laterally braced.

Tie-In

4 Bottom must be laterally braced at bearings Load Type



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Wind Comments Snow

0 PSF

0 PSF

Self Weight 5 PLF

Trib Width

Side

Тор

ID

1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and badings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Dead

15 PSF

Live

40 PSF

Manufacturer Info Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12472

Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400





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Client: Project: Address:

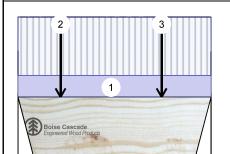
GREENPARKE THE CITY OF OSHAWA ZADORRAPESTATES Y ZADORRAPSTATESNS OSHANOV-04 2023

7/3/2023 Date: W C Input by:

Job Name: RIVER 6-3 STD & DC

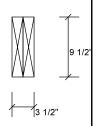
MHP 23025pject #:

Level: Ground Floor -P v - PASSED



Versa-Lam LVL 2.1E 3100 SP

1 Hanger (HUC410 (Min)) 0-2-8 2 Hanger (HUC410 (Min)) 0-2'6 9/16" 2'6 9/16"



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015
Deflection LL:	360		OBC 2012(2020 Update)
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Direction	Live	Dead	Snow	Wind
Vertical	288	120	0	0
Vertical	268	113	0	0
		Vertical 288	Vertical 288 120	Vertical 288 120 0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	267 ft-lb	1'4 1/8"	23220 ft-lb	0.011 (1%)	1.25D+1.5L	L
Unbraced	267 ft-lb	1'4 1/8"	23220 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	432 lb	1'	10574 lb	0.041 (4%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/221628)	1'3 3/8"	0.075 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/92648)	1'3 3/8"	0.075 (L/360)	0.004 (0%)	L	L
TL Defl inch	0.000 (L/65335)	1'3 3/8"	0.113 (L/240)	0.004 (0%)	D+L	L

Bearings and Factored Reactions

_							
Bearing	Length	Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.500"	Vert	6%	150 / 432	582	L	1.25D+1.5L
2 - Hanger	2.500"	Vert	6%	141 / 402	543	L	1.25D+1.5L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: DF, Thickness: 3 1/2"
- 4 Right Header: DF, Thickness: 3 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Multiple plies must be fastened together as per manufacturer's details.
- 7 Top loads must be supported equally by all plies.
- 8 Top must be continuously laterally braced.
- 9 Bottom must have sheathing attached or be continuously braced.
- 10 Lateral slenderness ratio based on full section width.



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Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. UVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788

www.bc.com CCMC: 12472

Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400





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Client: Project: Address:

GREENPARKE THE CITY OF OSHAWA ZADORRARESTATES ZADORRAPESTATESNS OSHANOVO4 2023 1.7 Maria

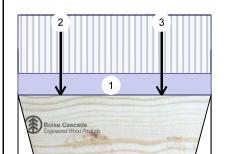
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Job Name: RIVER 6-3 STD & DC

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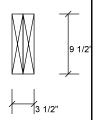
-Py - PASSED

Level: Ground Floor



Versa-Lam LVL 2.1E 3100 SP

1 Hanger (HUC410 (Min)) 0-2-8 2 Hanger (HUC410 (Min)) 0-2-8 2'6 9/16" 2'6 9/16"



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-6-9	1-10-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-6-12		Far Face	70 l b	187 l b	0 l b	0 l b	J2
3	Point	1-10-12		Far Face	67 l b	178 l b	0 lb	0 lb	J2
	Self Weight				9 PLF				



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Handling & Installation

Handling & Installation

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2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

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GREENPARKE THE CITY OF OSHAWA ZADORRARESTATES ZADORRAPSTATESNS OSHANOV-04 2023

7/3/2023 Date: W C Input by:

Job Name: RIVER 6-3 STD & DC

MHP 23025pject #

Vertical

Vertical

2

Hanger

Hanger

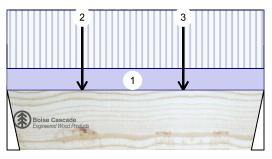
2.000"

Vert

2 -

2 Ply - PASSED

Level: Ground Floor



Versa-Lam LVL 2.1E 3100 SP

Wind

1.25D+1.5L

0

0

0

n

484 L

	1 Hanger (LF359) 0-2-0 2 H	langer (LF359) 0-2-0	
	3'4 11/16"	1	
,		L	
	3'4 11/16"	1	

Application: Floor (Residential) Type: Plies: 2 Design Method: LSD Moisture Condition: Dry **Building Code: NBCC 2015** OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF Dead:

Unfa	ctored I	Reactions	UNPAT	TERNED	lb (Uplift)	
Brg	Direction	l	_ive	Dead	Snov	٧

240

236

6%

Bearings and Factored Reactions								
Bearing	Length	Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.	
1-	2.000"	Vert	6%	132 / 361	493	L	1.25D+1.5L	

130 / 354

106

104

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	409 ft-lb	1'8 15/16"	23220 ft-lb	0.018 (2%)	1.25D+1.5L	L
Unbraced	409 ft-lb	1'8 15/16"	23220 ft-lb	0.018 (2%)	1.25D+1.5L	L
Shear	365 lb	11 1/2"	10574 l b	0.034 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/112651)	1'8 3/8"	0.106 (L/360)	0.003 (0%)	D	Uniform
LL Defl inch	0.001 (L/48588)	1'8 3/8"	0.106 (L/360)	0.007 (1%)	L	L
TL Defl inch	0.001 (L/33946)	1'8 3/8"	0.159 (L/240)	0.007 (1%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: DF, Thickness: 3 1/2"
- 4 Right Header: DF, Thickness: 3 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Multiple plies must be fastened together as per manufacturer's details.
- 7 Top loads must be supported equally by all plies.
- 8 Top must be continuously laterally braced.
- 9 Bottom must have sheathing attached or be continuously braced.
- 10 Lateral slenderness ratio based on full section width.



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Notes

Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-by fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

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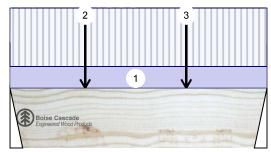
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Job Name: RIVER 6-3 STD & DC

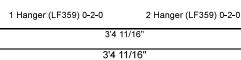
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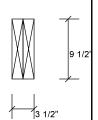
2 Ply - PASSED

Level: Ground Floor



Versa-Lam LVL 2.1E 3100 SP





I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-4-11		Тор	25 PLF	67 PLF	0 PLF	0 PLF	
2	Point	0-11-14		Far Face	46 lb	123 l b	0 l b	0 l b	J1
3	Point	2-3-14		Far Face	47 lb	126 l b	0 lb	0 lb	J1
	Self Weight				9 PLF				



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Handling & Installation

Handling & Installation

1. IVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

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Member Information

Client: Project: Address:

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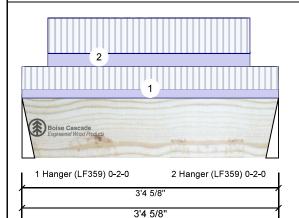
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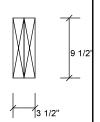
MHP 23025_{oject #}

2 Ply - PASSED

Level: Ground Floor



Versa-Lam LVL 2.1E 3100 SP



Application: Floor (Residential) Type Plies 2 Design Method: LSD Moisture Condition: Dry **Building Code: NBCC 2015** OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF Dead:

Brg Direction Live Dead Snow Wind Vertical 252 111 0 1 0 2 Vertical 249 110 n 0

Unfactored Reactions UNPATTERNED lb (Uplift)

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case Moment 431 ft-lb 1'8 5/16" 23220 ft-lb 0.019 (2%) 1.25D+1.5L L Unbraced 431 ft-lb 1'8 5/16" 23220 ft-lb 0.019 (2%) 1.25D+1.5L L 0.036 (4%) 1.25D+1.5L L 377 lb 2'5 1/8" 10574 lb Shear Perm Defl in 0.000 1'8 1/4" 0.106 (L/360) 0.003 (0%) D Uniform (L/110348) 0.001 1'8 1/4" 0.106 (L/360) 0.008 (1%) L LL Defl inch (L/47799) TL Defl inch 0.001 1'8 1/4" 0.159 (L/240) 0.007 (1%) D+L (L/33352)

Bearings and Factored Reactions Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 -2.000" Vert 7% 139 / 378 517 L 1.25D+1.5L Hanger 2 -2.000" Vert 7% 137 / 374 511 L 1.25D+1.5L



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Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: DF, Thickness: 3 1/2"
- 4 Right Header: DF, Thickness: 3 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Multiple plies must be fastened together as per manufacturer's details.
- 7 Top loads must be supported equally by all plies.
- 8 Top must be continuously laterally braced.
- 9 Bottom must have sheathing attached or be continuously braced.
- 10 Lateral slenderness ratio based on full section width.

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 LVL not to be treated with fire retardant or corrosive

Handling & Installation

CIVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 6. For flat roofs provide proper drainage to prevent ponding

Hanger

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Manufacturer Info

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F6-B

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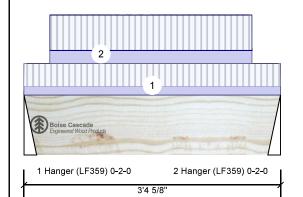
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Date: 7/3/2023 Input by: W C

Job Name: RIVER 6-3 STD & DC

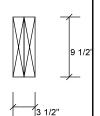
MHP 23025pject #:

2-Ply - PASSED Level: Ground Floor



3'4 5/8'

Versa-Lam LVL 2.1E 3100 SP



1 Part. Uniform 0-0-0 to 3-4-10 Top 25 PLF 67 PLF 0 PLF 0 PLF	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Part. Uniform	0-0-0 to 3-4-10		Тор	25 PLF	67 PLF	0 PLF	0 PLF	
2 Part. Uniform 0-4-1 to 3-0-1 Far Face 39 PLF 103 PLF 0 PLF 0 PLF	2	Part. Uniform	0-4-1 to 3-0-1		Far Face	39 PLF	103 PLF	0 PLF	0 PLF	
Self Weight 9 PLF		Self Weight				9 PLF				



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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. IVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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7/5/2023 Date: W C Input by:

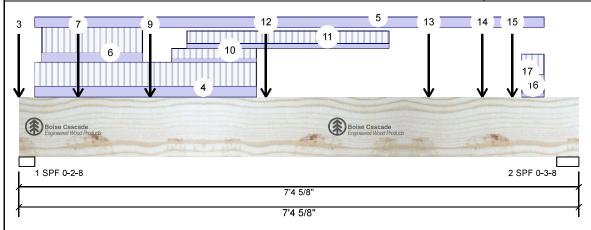
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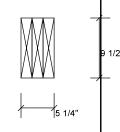
MHP 23025_{oject #}

Versa-Lam LVL 2.1E 3100 SP

-Plv - PASSED

Level: Ground Floor





Member Information

Type:	Girder
Plies:	3
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal - II
General Load	

40 PSF 15 PSF Application: Floor (Residential) LSD

Building Code: NBCC 2015 OBC 2012(2020 Update)

Load Sharing:

Design Method:

Not Checked Deck: Vibration: Not Checked **Unfactored Reactions UNPATTERNED lb (Uplift)**

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	2257	1403	0	0
2	Vertical	1620	1074	0	0

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF	2.500"	Vert	64%	1754 / 3385	5139	L	1.25D+1.5L
2 - SPF	3.500"	Vert	33%	1343 / 2431	3774	L	1.25D+1.5L

Analysis Results

Floor Live:

Dead:

-						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7551 ft-lb	3'3 1/8"	36222 ft-lb	0.208 (21%)	1.25D+1.5L	L
Unbraced	7551 ft-lb	3'3 1/8"	36222 ft-lb	0.208 (21%)	1.25D+1.5L	L
Shear	4418 l b	1'	15860 lb	0.279 (28%)	1.25D+1.5L	L
Perm Defl in.	0.024 (L/3526)	3'6 5/16"	0.234 (L/360)	0.102 (10%)	D	Uniform
LL Defl inch	0.037 (L/2301)	3'5 3/4"	0.234 (L/360)	0.156 (16%)	L	L
TL Defl inch	0.060 (L/1392)	3'6"	0.351 (L/240)	0.172 (17%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 3.5.
- 2 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
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ID	Load Type	Location ⁻	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-0-0		Тор	12 lb	0 lb	0 lb	0 l b	Wall Self Weight
	Bearing Length	0-5-8							
2	Point	0-0-0		Тор	9 lb	0 lb	0 lb	0 l b	Wall Self Weight
	Bearing Length	0-5-8							

Continued on page 2...

Notes

Notes

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- Handling & Installation
- 1. UVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used

- Dariga Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

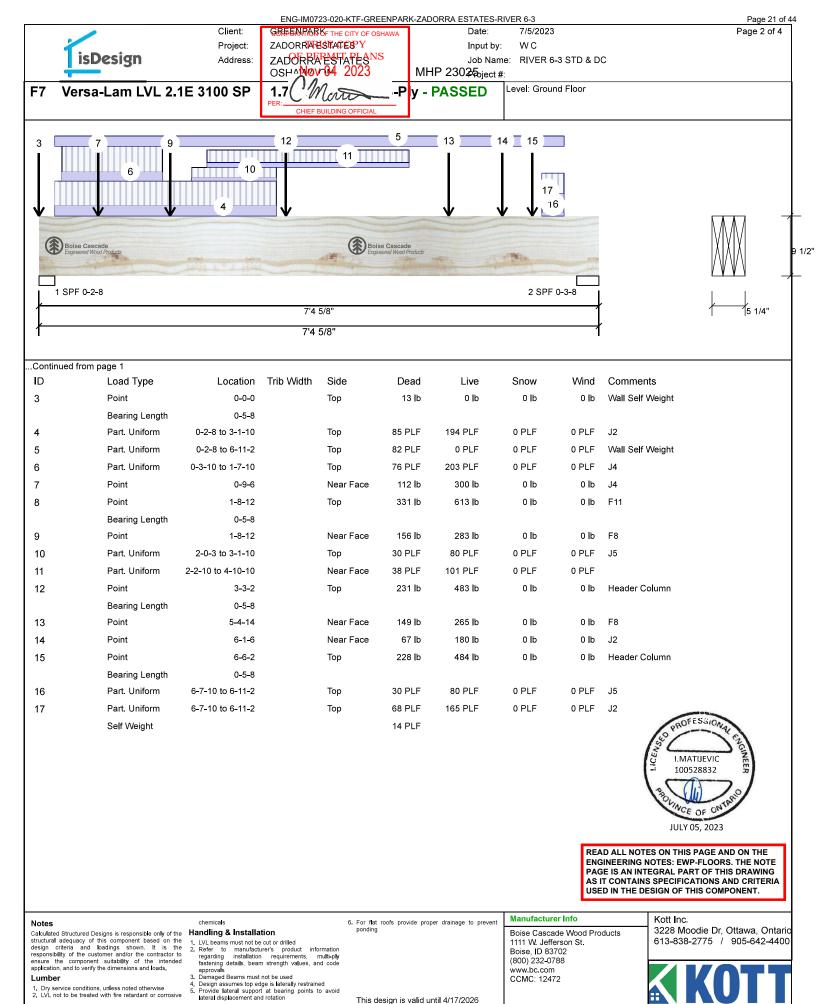
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CSD DESIGN