

Client:

Project: Address: **ZADORRA ESTATES**

GREENPARK

OSHAWA,ON

W C Input by:

Job Name:

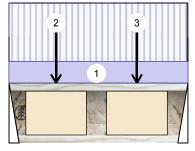
7/17/2023

F2-B **AJS 140**

11.875" - PASSET PN OF THE CITY OF OSHAWA

OF PERMIT PLANS lov 14 2023

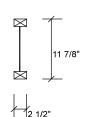
Level: Ground Floor



1 Hanger (LF2511) 0-2-0

2 Hanger (LF2511) 0-2-0 2'11 7/16'

2'11 7/16'



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015
Deflection LL:	360		OBC 2012(2020 Update)
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

lRLG	Direction	Live	Dead	Snow	vvina
1	Vertical	298	112	0	0
2	Vertical	290	108	0	0

Analysis Results

Ana l ysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	404 ft-lb	1'8 11/16"	5305 ft-lb	0.076 (8%)	1.25D+1.5L	L
Unbraced	404 ft-lb	1'8 11/16"	5305 ft-lb	0.076 (8%)	1.25D+1.5L	L
Shear	580 lb	1 1/4"	2350 lb	0.247 (25%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/22356)	1'6 9/16"	0.091 (L/360)	0.016 (2%)	D	Uniform
LL Defl inch	0.004 (L/8367)	1'6 5/8"	0.091 (L/360)	0.043 (4%)	L	L
TL Defl inch	0.005 (L/6088)	1'6 5/8"	0.137 (L/240)	0.039 (4%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap. R	teact D/L I b	Total	Ld. Case	Ld. Comb.
1 -	2.000"	Vert	37%	140 / 447	587	L	1.25D+1.5L
Hanger							
2 -	2.000"	Vert	35%	135 / 435	570	L	1.25D+1.5L
Hanger							

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: SPF, Thickness: 2 1/2"
- 4 Right Header: SPF, Thickness: 2 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum
- 2' o.c.
- 7 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-11-7	0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-5		Near Face	92 l b	245 lb	0 lb	0 lb	J4
3	Point	2-1-5		Near Face	94 lb	252 lb	0 lb	0 lb	J4

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation

 1. Loist flanges must not be cut or drilled

 2. Refer to latest copy of the Libist product information details for framing details, stiffener tables, web hole chart, bridging details, militury fastening details and handling/erection details

 3. Damaged bloists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







Client: **GREENPARK**

Project:

ZADORRA ESTATES

OSHAWA,ON

7/17/2023 Date: W C Input by:

Job Name:

F3 **AJS 140**

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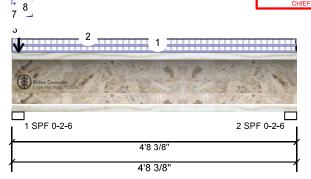
11.875" - PASSE

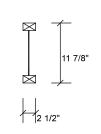
Address:

RATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS

Level: Ground Floor

lov 14 2023





Member Information Type: Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal - II General Load Floor Live: 40 PSF

Dead:

Application: Floor (Residential) Design Method: LSD Building Code: **NBCC 2015** OBC 2012(2020 Update) Load Sharing: Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift) Wind Brg Direction Live Dead Snow 489 383 Vertical 190 1 0 2 Vertical 127 48 n 0

Cap. React D/L lb

611 / 764

60 / 191

82%

17%

Total Ld. Case

1375 L

251 L

Ld. Comb.

1.25D+1.5S +L

1 25D+1 5L

Bearings and Factored Reactions

Dir.

Vert

Vert

Bearing Length

1 - SPF 2.375"

2 - SPF 2.375"

Analysis Results

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	262 ft-lb	2'4 1/16"	4721 ft-lb	0.056 (6%)	1.25D+1.5L	L
Unbraced	262 ft-lb	2'4 1/16"	4721 ft-lb	0.056 (6%)	1.25D+1.5L	L
Shear	277 l b	1 5/8"	2092 l b	0.132 (13%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/40028)	2'3 7/8"	0.147 (L/360)	0.009 (1%)	D	Uniform
LL Defl inch	0.003 (L/15454)	2'4 1/8"	0.147 (L/360)	0.023 (2%)	L+0.5S	L
TL Defl inch	0.005 (L/11150)	2'4 1/16"	0.221 (L/240)	0.022 (2%)	D+L+0.5S	L

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Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at bearings

- Botton	made be laterally brace	a at boarnigor							
I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-8-6	0-7-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 4-8-6	0-8-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-0 to 0-1-2		Тор	15 PLF	0 PLF	40 PLF	0 PLF	
4	Part. Uniform	0-0-0 to 0-1-2		Тор	20 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Tapered Start	0-0-0		Тор	2 PLF	6 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be cut or drilled

 2. Refer to latest copy of the Lioist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-qly fastening details and handling/erection details

 3. Damaged Lioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787





Page 23 of 36



Client: **GREENPARK**

Project:

Date: 7/17/2023 Input by:

W C

Job Name: VILLA 2-1 STD

F3 **AJS 140**

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11.875" - PASSED RATION OF THE CITY OF OSHAWA

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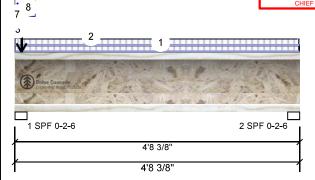
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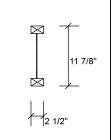
Level: Ground Floor

OF PERMIT PLANS lov 14 2023

ZADORRA ESTATES

OSHAWA,ON





Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	End	0-1-2			2 PLF	6 PLF	0 PLF	0 PLF	
6	Part. Uniform	0-0-0 to 0-1-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
7	Part. Uniform	0-0-0 to 0-1-2		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
8	Part. Uniform	0-0-0 to 0-4-0		Тор	30 PLF	0 PLF	80 PLF	0 PLF	
9	Part. Uniform	0-0-0 to 0-4-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Tapered Start	0-0-0		Тор	5 PLF	13 PLF	0 PLF	0 PLF	
	End	0-4-0			5 PLF	13 PLF	0 PLF	0 PLF	
11	Part. Uniform	0-0-0 to 0-4-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
12	Part. Uniform	0-0-0 to 0-4-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
13	Point	0-1-5		Тор	382 lb	58 lb	353 lb	0 lb	В3
	Bearing Length	0-1-8							



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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-pty fastening details and handling/erection details

 3. Damaged Lioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches 7. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

Manufacturer Info

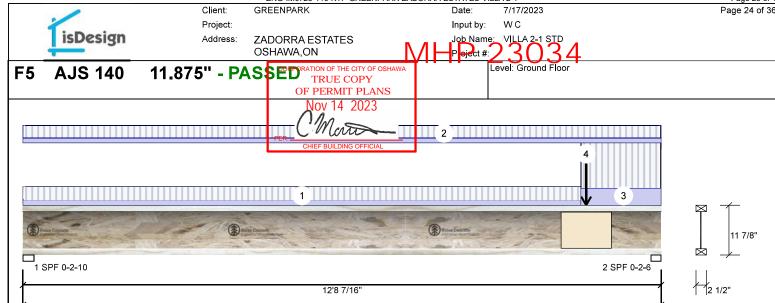
Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







Member Information **Unfactored Reactions UNPATTERNED lb (Uplift)** Application: Floor (Residential) Wind Type: Brg Direction Live Dead Snow Plies: Design Method: LSD Vertical 269 101 0 1 0 Moisture Condition: Dry Building Code: **NBCC 2015** 2 Vertical 557 208 n 0 OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 126 / 403 1 - SPF 2.625" Vert 30% 529 L 1.25D+1.5L 260 / 837 1.25D+1.5L 2 - SPF 2.375" Vert 65% 1097

12'8 7/16'

Analysis Results

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	1855 ft-lb	7'3 15/16"	5305 ft-lb	0.350 (35%)	1.25D+1.5L	L
Unbraced	1855 ft-lb	7'3 15/16"	5305 ft-lb	0.350 (35%)	1.25D+1.5L	L
Shear	1075 l b	12'6 13/16"	2350 lb	0.457 (46%)	1.25D+1.5L	L
Perm Defl in.	0.037 (L/3987)	6'8"	0.414 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.100 (L/1493)	6'8"	0.414 (L/360)	0.241 (24%)	L	L
TL Defl inch	0.137 (L/1086)	6'8"	0.620 (L/240)	0.221 (22%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 4 Bottom flange must be laterally braced at a maximum of 11'2 9/16" o.c.



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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-1-5	0-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-8-7	0-5-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	11-1-5 to 12-8-7	1-6-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	11-2-9		Far Face	108 l b	290 lb	0 lb	0 l b	F2

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Handling & Installation

- Handling & Installation

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 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

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Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

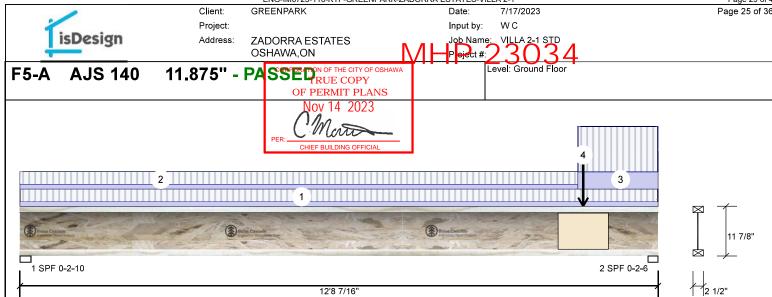
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Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787







Member Information **Unfactored Reactions UNPATTERNED lb (Uplift)** Application: Floor (Residential) Wind Type: Brg Direction Live Dead Snow Plies: Design Method: LSD 259 Vertical 97 0 1 0 Moisture Condition: Dry Building Code: **NBCC 2015** 2 Vertical 556 209 n 0 OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF 15 PSF Dead: Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.625" Vert 29% 121 / 389 510 L 1.25D+1.5L 2 - SPF 2.375" Vert 65% 261 / 834 1095 L 1.25D+1.5L

12'8 7/16'

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1809 ft-lb	7'4 13/16"	5305 ft-lb	0.341 (34%)	1.25D+1.5L	L
Unbraced	1809 ft-lb	7'4 13/16"	5305 ft-lb	0.341 (34%)	1.25D+1.5L	L
Shear	1074 l b	12'6 13/16"	2350 lb	0.457 (46%)	1.25D+1.5L	L
Perm Defl in.	0.036 (L/4082)	6'8 3/16"	0.414 (L/360)	0.088 (9%)	D	Uniform
LL Defl inch	0.097 (L/1532)	6'8 3/16"	0.414 (L/360)	0.235 (24%)	L	L
TL Defl inch	0.134 (L/1114)	6'8 3/16"	0.620 (L/240)	0.215 (22%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o c
- 4 Bottom flange must be laterally braced at a maximum of 11'2 9/16" o.c.



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	·									
	I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 12-8-7	0-5-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 11-1-5	0-5-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Tie-In	11-1-5 to 12-8-7	1-6-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	4	Point	11-2-9		Near Face	112 l b	298 lb	0 lb	0 l b	F2
ı										

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Handling & Installation

- Handling & Installation

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 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

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Manufacturer Info

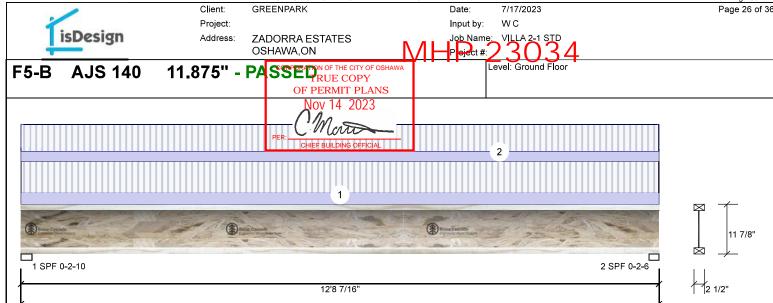
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Kott Inc.







Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Application: Floor (Residential) Wind Type: Brg Direction Live Dead Snow Plies: Design Method: LSD 365 Vertical 137 0 1 0 Moisture Condition: Dry Building Code: **NBCC 2015** 2 Vertical 364 136 0 0 OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF 15 PSF Dead: Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.625" Vert 41% 171 / 548 719 L 1.25D+1.5L 2 - SPF 2.375" Vert 43% 171 / 545 716 L 1.25D+1.5L

12'8 7/16'

Analysis Results

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	2175 ft-lb	6'4 5/16"	5305 ft-lb	0.410 (41%)	1.25D+1.5L	L
Unbraced	2175 ft-lb	6'4 5/16"	5305 ft-lb	0.410 (41%)	1.25D+1.5L	L
Shear	701 lb	12'6 13/16"	2350 lb	0.298 (30%)	1.25D+1.5L	L
Perm Defl in.	0.043 (L/3467)	6'4 3/8"	0.414 (L/360)	0.104 (10%)	D	Uniform
LL Defl inch	0.115 (L/1300)	6'4 3/8"	0.414 (L/360)	0.277 (28%)	L	L
TL Defl inch	0.158 (L/945)	6'4 3/8"	0.620 (L/240)	0.254 (25%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

2' O.C.	4 Bottom flange must be laterally braced at bearings.						USED IN THE DESIGN OF THIS COMPONENT.			
4 Bollom Hange	illust be laterally brac	eu at beatiligs.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 12-8-7	0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 12-8-7	0-8-0	Ton	15 PSF	40 PSF	0 PSF	0 PSF		

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I.MATIJEVIC 100528832

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JULY 19, 2023

AS IT CONTAINS SPECIFICATIONS AND CRITERIA

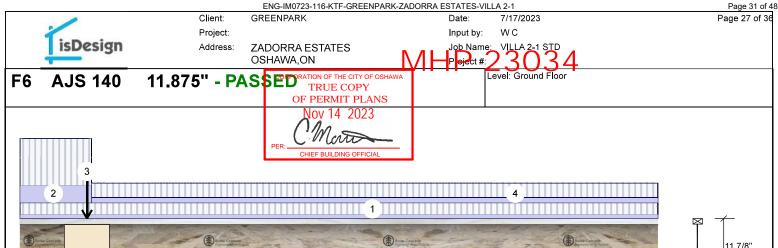
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14'2 1/4' 14'2 1/4'

Analysis Results

1 SPF 0-2-6

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2172 ft-lb	6'1 1/8"	5305 ft-lb	0.409 (41%)	1.25D+1.5L	L
Unbraced	2172 ft-lb	6'1 1/8"	5305 ft-lb	0.409 (41%)	1.25D+1.5L	L
Shear	1170 l b	1 5/8"	2350 lb	0.498 (50%)	1.25D+1.5L	L
Perm Defl in	0.053 (L/3118)	6'9 5/16"	0.463 (L/360)	0.115 (12%)	D	Uniform
LL Defl inch	0.142 (L/1171)	6'9 3/8"	0.463 (L/360)	0.308 (31%)	L	L
TL Defl inch	0.196 (L/851)	6'9 3/8"	0.695 (L/240)	0.282 (28%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o c
- 4 Bottom flange must be laterally braced at a maximum of 12'8 3/8" o.c.



2 SPF 0-2-10

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

	· ,								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-2-4	0-4-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-7-2	1-7-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-14		Near Face	121 l b	321 l b	0 lb	0 lb	F2
4	Tie-In	1-7-2 to 14-2-4	0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

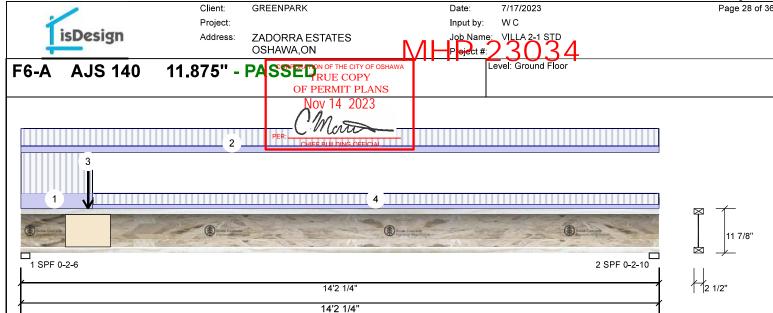
Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Application: Floor (Residential) Wind Type: Brg Direction Live Dead Snow Plies: Design Method: LSD 260 Vertical 693 0 1 0 Moisture Condition: Dry Building Code: **NBCC 2015** 2 Vertical 356 134 0 0 OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF 15 PSF Dead: Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" Vert 81% 326 / 1039 1365 L 1.25D+1.5L 2 - SPF 2.625" Vert 40% 167 / 534 701 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2666 ft-lb	6'3 5/16"	5305 ft-lb	0.503 (50%)	1.25D+1.5L	L
Unbraced	2666 ft-lb	6'3 5/16"	5305 ft-lb	0.503 (50%)	1.25D+1.5L	L
Shear	1341 lb	1 5/8"	2350 lb	0.571 (57%)	1.25D+1.5L	L
Perm Defl in	0.066 (L/2543)	6'9 15/16"	0.463 (L/360)	0.142 (14%)	D	Uniform
LL Defl inch	0.175 (L/955)	6'9 15/16"	0.463 (L/360)	0.377 (38%)	L	L
TL Defl inch	0.240 (L/694)	6'9 15/16"	0.695 (L/240)	0.346 (35%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 4 Bottom flange must be laterally braced at a maximum of 12'8 3/8" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-2	1-7-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-2-4	0-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-14		Far Face	127 l b	337 lb	0 lb	0 l b	F2
4	Tie-In	1-7-2 to 14-2-4	0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-rily fastening details and
 handling/erection details

 3. Damaged IJoists must not be used
 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







Member Infor	mation		Unfactored Reactions UNPATTERNED lb (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	L	ive	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	Vertical	4	401	150	0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2015	2	Vertical	4	402	151	0	0
Deflection LL:	360		OBC 2012(2020 Update)							
Deflection TL:	240	Load Sharing:	No							
Importance:	Normal - II	Deck:	Not Checked							
General Load		Vibration:	Not Checked							
Floor Live:	40 PSF			Bea	rings and Fa	actored	Rea	ctions		
Dead:	15 PSF			Bea	aring Length	Dir.	Сар.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 -	SPF 2.375"	Vert	47%	188 / 602	790 L	1.25D+1.5L
				2 -	SPF 2.625"	Vert	46%	188 / 604	792 L	1.25D+1.5L
Analycic Pocul	+c									

Analysis Results

Ana l ysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	2671 ft-lb	7'1"	5305 ft-lb	0.503 (50%)	1.25D+1.5L	L
Unbraced	2671 ft-lb	7'1"	5305 ft-lb	0.503 (50%)	1.25D+1.5L	L
Shear	774 l b	1 5/8"	2350 lb	0.329 (33%)	1.25D+1.5L	L
Perm Defl in.	0.064 (L/2593)	7'1 1/16"	0.463 (L/360)	0.139 (14%)	D	Uniform
LL Defl inch	0.171 (L/972)	7'1 1/16"	0.463 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.236 (L/707)	7'1 1/16"	0.695 (L/240)	0.339 (34%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at bearings.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-9-1	0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-2-4	0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	4-10-3 to 9-6-8	0-8-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	9-7-10 to 14-2-4	0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

 3. Damaged Jioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







Client: Project: Address: **GREENPARK**

W C Input by: Job Name: VILLA 2-1 STD & WOC

7/17/2023

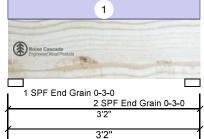
ZADORRA ESTATES OSHAWA,ON

YPI 1875 OF 0s 2 VPIV - PASSED Level: Ground Floor

Versa-Lam LVL 2.1E 3100 SP



OF PERMIT PLANS lov 14 2023



15 PSF

Member Info	rmation
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Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015
Deflection LL:	360		OBC 2012(2020 Update)
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	48	182	0	0
2	Vertical	48	182	0	0
1					

Analysis Results

Dead:

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	184 ft-lb	1'7"	25128 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	184 ft-lb	1'7"	25128 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	202 lb	1'11 1/8"	9384 lb	0.022 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/183529)	1'7"	0.093 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/702510)	1'7"	0.093 (L/360)	0.001 (0%)	L	L
TL Defl inch	0.000 (L/145514)	1'7"	0.140 (L/240)	0.002 (0%)	D+L	L

Bearings and Factored Reactions

Bearing L	ength I	Dir. C	ap. Read	t D/L l b	Total	Ld. Case	Ld. Comb.
1 - SPF 3. End Grain	.000" \	Vert	4%	227 / 71	299	L	1.25D+1.5L
2 - SPF 3. End Grain	.000" \	Vert	4%	227 / 71	299	L	1.25D+1.5L



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.
- 6 Bottom must have sheathing attached or be continuously braced.
- 7 Lateral slenderness ratio based on full section width.

I D	Load Type	Location Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-2-0	Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-0-0 to 3-2-0	Near Face	8 PLF	0 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-0-0 to 3-2-0	Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Tapered Start	0-0-0	Near Face	11 PLF	30 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

Kott Inc. 3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400





Page 31 of 36



Client: Project: Address: **GREENPARK**

7/17/2023

Input by: W C

ZADORRA ESTATES OSHAWA,ON

Job Name: VILLA 2-1 STD & WOC

Versa-Lam LVL 2.1E 3100 SP

.750**AXM11875*05032*PIV - PASSED Level: Ground Floor OF PERMIT PLANS

lov 14 2023

5 3 2 1 1 SPF End Grain 0-3-0 2 SPF End Grain 0-3-0 3'2' 3'2'

11 7/8'

.Continued from page 1

ID Location Trib Width Load Type Side Dead Live Snow Wind Comments 3-2-0 11 PLF 30 PLF 0 PLF 0 PLF

5 Part. Uniform 0-0-0 to 3-2-0 Near Face 4 PLF 0 PLF 0 PLF 0 PLF Rim Board Self Weight

> Self Weight 12 PLF



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

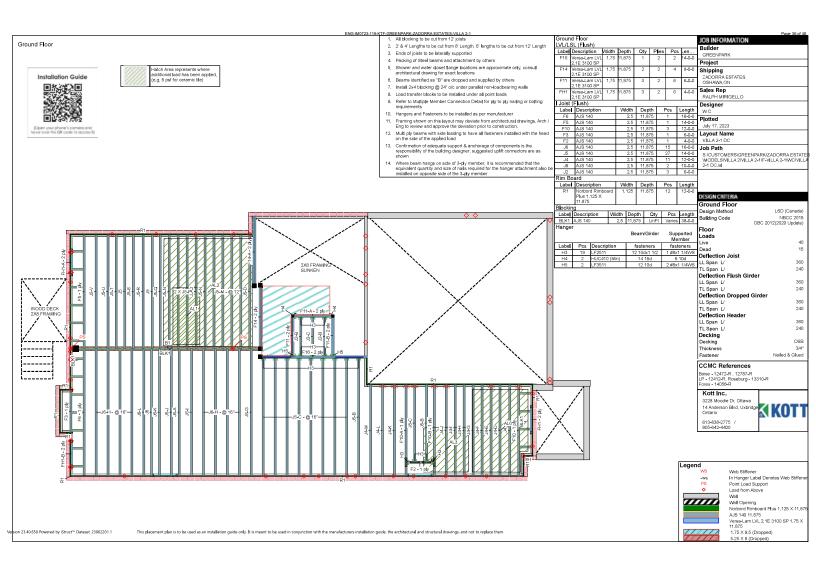
(800) 232-0788 www.bc.com CCMC: 12472

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400









Page 1 of 6



Client: Project: Address:

GREENPARK

ZADORRA ESTATES

W C Input by:

OSHAWA,ON

YPI 1875'0F 0S 2 YPI - PASSED Level: Ground Floor RUE COPY OF PERMIT PLANS

Job Name:

7/17/2023

Versa-Lam LVL 2.1E 3100 SP

5 3 2 1

1 SPF End Grain 0-3-0 2 SPF End Grain 0-3-0 3'2' 3'2'

lov 14 2023

Member Information	on
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	Туре:	Girder	Application:	Floor (Residential)
	Plies:	2	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2015
	Deflection LL:	360		OBC 2012(2020 Update)
	Deflection TL:	240	Load Sharing:	No
	Importance:	Normal - II	Deck:	Not Checked
	General Load		Vibration:	Not Checked
	Floor Live:	40 PSF		
	Dead:	15 PSF		
ı				

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	48	182	0	0
2	Vertical	48	182	0	0
l					

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	184 ft-lb	1'7"	25128 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	184 ft-lb	1'7"	25128 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	202 lb	1'11 1/8"	9384 lb	0.022 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/183529)	1'7"	0.093 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/702510)	1'7"	0.093 (L/360)	0.001 (0%)	L	L
TL Defl inch	0.000 (L/145514)	1'7"	0.140 (L/240)	0.002 (0%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap. R	eact D/L I b	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	4%	227 / 71	299	L	1.25D+1.5L
2 - SPF End Grain	3.000"	Vert	4%	227 / 71	299	L	1.25D+1.5L



- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.
- 6 Bottom must have sheathing attached or be continuously braced.

7 Lateral slenderness ratio based on full section width.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-2-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-0-0 to 3-2-0		Near Face	8 PLF	0 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-0-0 to 3-2-0		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Tapered Start	0-0-0		Near Face	11 PLF	30 PLF	0 PLF	0 PLF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- 1. UVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Boise Cascade Wood Products

1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

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Page 2 of 6



Client: Project: Address: **GREENPARK**

7/17/2023

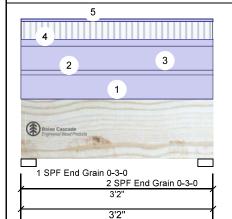
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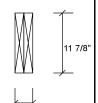
ZADORRA ESTATES OSHAWA,ON

Job Name: VILLA 2-1 DC

Versa-Lam LVL 2.1E 3100 SP

YPI 1875'0F 0S 2 YPI - PASSED Level: Ground Floor RUE COPY OF PERMIT PLANS lov 14 2023





.Continued from page 1

ID Location Trib Width Load Type Side Dead Live Snow Wind Comments 3-2-0 11 PLF 30 PLF 0 PLF 0 PLF

5 Part. Uniform 0-0-0 to 3-2-0 Near Face 4 PLF 0 PLF 0 PLF 0 PLF Rim Board Self Weight Self Weight

12 PLF



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

3228 Moodie Dr, Ottawa, Ontario

613-838-2775 / 905-642-4400





Page 3 of 6



Client: Project: Address:

GREENPARK

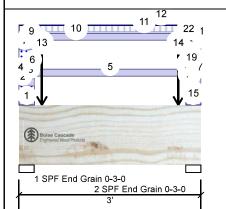
ZADORRA ESTATES

7/17/2023 W C Input by:

Job Name: P eject #:

Level: Ground Floor

Versa-Lam LVL 2.1E 3100 SP



OSHAWA,ON **I .750^{1k}^X○1⊈ .875**Ч०F **2**4**Ply** TRUE COPY OF PERMIT PLANS

- PASSED Jov 14 2023

11 7/8'

Member Information

ı	Type:	Girder	Application:	Floor (Residential)
ı	Plies:	2	Design Method:	LSD
I	Moisture Condition:	Dry	Building Code:	NBCC 2015
ı	Deflection LL:	360		OBC 2012(2020 Update)
ı	Deflection TL:	240	Load Sharing:	No
I	Importance:	Normal - II	Deck:	Not Checked
I	General Load		Vibration:	Not Checked
ı	Floor Live:	40 PSF		
ı	Dead:	15 PSF		
ı				

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	76	410	239	0
2	Vertical	75	410	239	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	213 ft-lb	1'5 15/16"	23005 ft-lb	0.009 (1%)	1.25D+1.5L	L
Unbraced	213 ft-lb	1'5 15/16"	23005 ft-lb	0.009 (1%)	1.25D+1.5L	L
Shear	222 l b	1'9 1/8"	8591 lb	0.026 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/154042)	1'6"	0.088 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/359167)	1'6"	0.088 (L/360)	0.001 (0%)	S+0.5L	L
TL Defl inch	0.000 (L/107806)	1'6"	0.131 (L/240)	0.002 (0%)	D+S+0.5L	L

Bearings and Factored Reactions

Bearing	Length	Dir.	Сар.	React D/L I b	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	9%	512 / 434	946	L	1.25D+1.5S +L
2 - SPF End Grain	3.000"	Vert	9%	512 / 434	946	L	1.25D+1.5S +L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 3.
- 2 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 3.
- 3 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Multiple plies must be fastened together as per manufacturer's details.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be continuously laterally braced.
- 8 Bottom must have sheathing attached or be continuously braced.
- 9 Lateral slenderness ratio based on full section width.



JULY 19, 2023

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Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. UVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

This design is valid until 4/17/2026

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12472



Page 4 of 6



1 SPF End Grain 0-3-0

Client: Project: Address:

GREENPARK

ZADORRA ESTATES

OSHAWA,ON

Date: 7/17/2023 Input by: W C

Job Name: VILLA 2-1 DC P eject #: Level: Ground Floor - PASSED

Versa-Lam LVL 2.1E 3100 SP

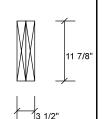
10 22 1 Γ9 14 -13 ... 6 **r** 19 4 5 2 15

2 SPF End Grain 0-3-0

I .750³^k^**X○¶⊈ 875**Ч०೯**○2±₽Iy** TRUE COPY OF PERMIT PLANS Jov 14 2023

PROFESSION I.MATIJEVIC 100528832 VCE OF JULY 19, 2023

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-3-1		Тор	30 PLF	0 PLF	80 PLF	0 PLF	
2	Part. Uniform	0-0-0 to 0-3-1		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Tapered Start	0-0-0		Тор	4 PLF	10 PLF	0 PLF	0 PLF	
	End	0-3-1			4 PLF	10 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-0-0 to 0-3-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
5	Part. Uniform	0-0-0 to 3-0-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	0-0-0 to 0-3-1		Near Face	30 PLF	0 PLF	80 PLF	0 PLF	
7	Part. Uniform	0-0-0 to 0-3-1		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
8	Tapered Start	0-0-0		Near Face	4 PLF	10 PLF	0 PLF	0 PLF	
	End	0-3-1			4 PLF	10 PLF	0 PLF	0 PLF	
9	Part. Uniform	0-0-0 to 0-3-1		Near Face	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
10	Part. Uniform	0-0-0 to 3-0-0		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
11	Tapered Start	0-0-0		Near Face	12 PLF	31 PLF	0 PLF	0 PLF	
	End	3-0-0			12 PLF	31 PLF	0 PLF	0 PLF	
12	Part. Uniform	0-0-0 to 3-0-0		Near Face	4 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
13	Point	0-4-9		Тор	210 l b	24 lb	199 lb	0 lb	Header Column Header Column
	Bearing Length	0-3-8							
14	Point	2-7-9		Тор	210 lb	24 lb	199 b	0 lb	Header Column Header Column
	Bearing Length	0-3-8							
15	Part. Uniform	2-9-1 to 3-0-0		Тор	30 PLF	0 PLF	80 PLF	0 PLF	
16	Part. Uniform	2-9-1 to 3-0-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
17	Tapered Start	2-9-1		Тор	4 PLF	10 PLF	0 PLF	0 PLF	
	End	3-0-0			4 PLF	10 PLF	0 PLF	0 PLF	
18	Part. Uniform	2-9-1 to 3-0-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
19	Part. Uniform	2-9-1 to 3-0-0		Near Face	30 PLF	0 PLF	80 PLF	0 PLF	
20	Part. Uniform	2-9-1 to 3-0-0		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
21	Tapered Start	2-9-1		Near Face	4 PLF	10 PLF	0 PLF	0 PLF	
	End	3-0-0			4 PLF	10 PLF	0 PLF	0 PLF	
22	Part. Uniform	2-9-1 to 3-0-0		Near Face	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
	Self Weight				12 PLF				

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. IVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Boise Cascade Wood Products

1111 W. Jefferson St. Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400





Page 5 of 6



Client: Project:

GREENPARK

W C Input by: Job Name:

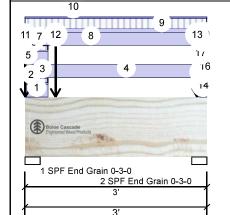
7/17/2023

Address: **ZADORRA ESTATES** OSHAWA,ON

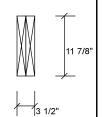
P eject #: - PASSED

VILLA 2-1 DC Level: Ground Floor





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Member Information

ı	Type:	Girder	Application:	Floor (Residential)
	Plies:	2	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2015
	Deflection LL:	360		OBC 2012(2020 Update)
	Deflection TL:	240	Load Sharing:	No
	Importance:	Normal - II	Deck:	Not Checked
	General Load		Vibration:	Not Checked
	Floor Live:	40 PSF		
	Dead:	15 PSF		
ı				

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	84	439	252	0
2	Vertical	86	449	252	0

Analysis Results

Analysis	Actua l	Location	Allowed	Capacity	Comb.	Case
Moment	194 ft-lb	1'4 3/16"	23005 ft-lb	0.008 (1%)	1.25D+1.5L	L
Unbraced	194 ft-lb	1'4 3/16"	23005 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	220 lb	1'9 1/8"	8591 lb	0.026 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/179787)	1'5 5/16"	0.088 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/651480)	1'5 7/16"	0.088 (L/360)	0.001 (0%)	L+0.5S	L
TL Defl inch	0.000 (L/140904)	1'5 3/8"	0.131 (L/240)	0.002 (0%)	D+L+0.5S	L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	194 ft-lb	1'4 3/16"	23005 ft-lb	0.008 (1%)	1.25D+1.5L	L
Unbraced	194 ft-lb	1'4 3/16"	23005 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	220 lb	1'9 1/8"	8591 l b	0.026 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/179787)	1'5 5/16"	0.088 (L/360)	0.002 (0%)	D	Uniform
LL Defl inch	0.000 (L/651480)	1'5 7/16"	0.088 (L/360)	0.001 (0%)	L+0.5S	L
TL Defl inch	0.000 (L/140904)	1'5 3/8"	0.131 (L/240)	0.002 (0%)	D+L+0.5S	L

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap. R	eact D/L I b	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	10%	549 / 462	1011	L	1.25D+1.5S +L
2 - SPF End Grain	3.000"	Vert	10%	562 / 464	1025	L	1.25D+1.5S +L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 3.
- 2 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 3.
- 3 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
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- 5 Multiple plies must be fastened together as per manufacturer's details.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be continuously laterally braced.
- 8 Bottom must have sheathing attached or be continuously braced.
- 9 Lateral slenderness ratio based on full section width.



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Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. UVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

Manufacturer Info

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400





Page 6 of 6



Client: Project: Address:

GREENPARK

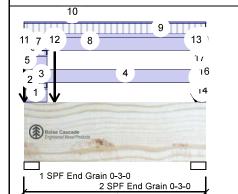
ZADORRA ESTATES

Date: 7/17/2023 Input by:

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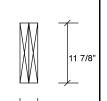
Job Name: VILLA 2-1 DC P eject #: Level: Ground Floor

Versa-Lam LVL 2.1E 3100 SP



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> PROFESSIONA I.MATIJEVIC 100528832 JULY 19, 2023



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AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

I D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-4-9		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Tapered Start	0-0-0		Тор	5 PLF	13 PLF	0 PLF	0 PLF	
	End	0-4-9			5 PLF	13 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-0-0 to 0-4-9		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
4	Part. Uniform	0-0-0 to 3-0-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Part. Uniform	0-0-0 to 0-4-9		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Tapered Start	0-0-0		Near Face	5 PLF	13 PLF	0 PLF	0 PLF	
	End	0-4-9			5 PLF	13 PLF	0 PLF	0 PLF	
7	Part. Uniform	0-0-0 to 0-4-9		Near Face	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
8	Part. Uniform	0-0-0 to 3-0-0		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
9	Tapered Start	0-0-0		Near Face	12 PLF	31 PLF	0 PLF	0 PLF	
	End	3-0-0			12 PLF	31 PLF	0 PLF	0 PLF	
10	Part. Uniform	0-0-0 to 3-0-0		Near Face	4 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
11	Point	0-0-1		Тор	122 lb	0 lb	252 lb	0 lb	Header Column Header Column
	Bearing Length	0-3-8							
12	Point	0-6-1		Тор	137 lb	32 lb	0 lb	0 lb	Header Column Header Column
	Bearing Length	0-3-8							
13	Point	2-10-1		Тор	259 b	32 lb	252 lb	0 l b	Header Column Header Column Header Column Header Column
	Bearing Length	0-3-8							
14	Part. Uniform	2-10-9 to 3-0-0		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
15	Tapered Start	2-10-9		Тор	5 PLF	13 PLF	0 PLF	0 PLF	
	End	3-0-0			5 PLF	13 PLF	0 PLF	0 PLF	
16	Part. Uniform	2-10-9 to 3-0-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
17	Part. Uniform	2-10-9 to 3-0-0		Near Face	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
18	Tapered Start	2-10-9		Near Face	5 PLF	13 PLF	0 PLF	0 PLF	
	End	3-0-0			5 PLF	13 PLF	0 PLF	0 PLF	
19	Part. Uniform	2-10-9 to 3-0-0		Near Face	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 4/17/2026

6. For flat roofs provide proper drainage to prevent ponding

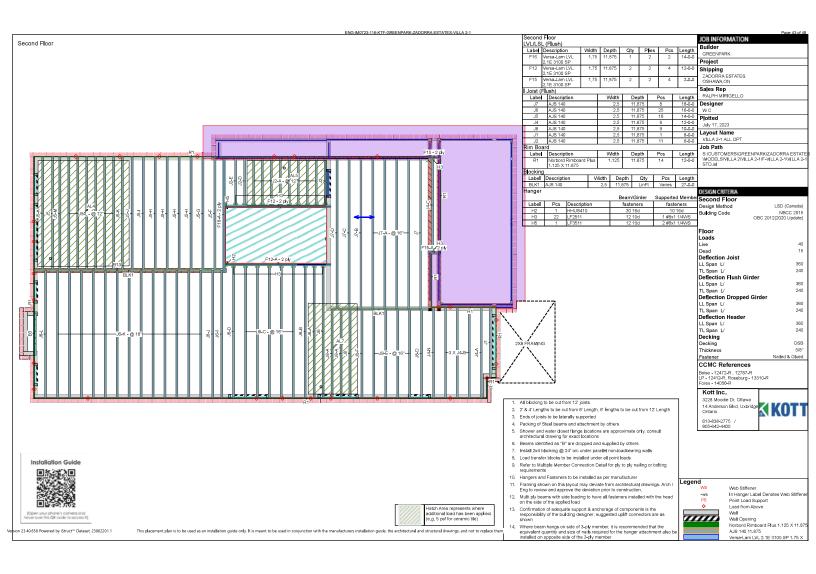
Manufacturer Info

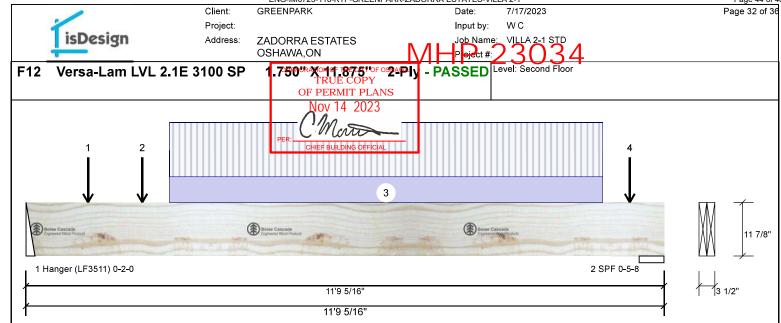
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/lember Info	mation			Unfa	ctored Rea	ctions	UNP	ATTERNED I I	o (Upli	ft)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction		Live	Dead		Snow	Wind
Plies:	2	Design Method:	LSD	1	Vertical		545	308		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2015	2	Vertical		631	360		0	0
Deflection LL:	360		OBC 2012(2020 Update)								
Deflection TL:	240	Load Sharing:	No								
Importance:	Normal - II	Deck:	Not Checked								
General Load		Vibration:	Not Checked								
Floor Live:	40 PSF			Beari	ings and Fa	actored	d Read	tions			
Dead:	15 PSF			Bea	ring Length	Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.
				1 - Han	2.000" ger	Vert	16%	385 / 818	1203	L	1.25D+1.5L
Analysis Resu	lts			2 - 5	SPF 5.500"	Vert	12%	450 / 946	1397	L	1.25D+1.5L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3671 ft-lb	5'9 1/16"	35392 ft-lb	0.104 (10%)	1.25D+1.5L	L
Unbraced	3671 ft-lb	5'9 1/16"	35392 ft-lb	0.104 (10%)	1.25D+1.5L	L
Shear	1321 l b	10'3 15/16"	13217 l b	0.100 (10%)	1.25D+1.5L	L
Perm Defl in.	0.021 (L/6304)	5'9 1/16"	0.376 (L/360)	0.057 (6%)	D	Uniform
LL Defl inch	0.037 (L/3622)	5'8 7/8"	0.376 (L/360)	0.099 (10%)	L	L
TL Defl inch	0.059 (L/2301)	5'9"	0.564 (L/240)	0.104 (10%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: DF, Thickness: 3 1/2"
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Multiple plies must be fastened together as per manufacturer's details.
- 6 Top must be continuously laterally braced.
- 7 Bottom must have sheathing attached or be continuously braced.
- 8 Lateral slenderness ratio based on full section width.



JULY 19, 2023

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	1-1-12		Far Face	45 l b	119 lb	0 lb	0 l b	J2
2	Point	2-1-12		Far Face	41 l b	104 lb	0 lb	0 lb	J2
3	Part. Uniform	2-7-12 to 10-7-12		Far Face	49 PLF	104 PLF	0 PLF	0 PLF	
4	Point	11-1-12		Far Face	51 l b	121 lb	0 lb	0 lb	J2
	Self Weight				12 PLF				

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Handling & Installation

Handling & Installation

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3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

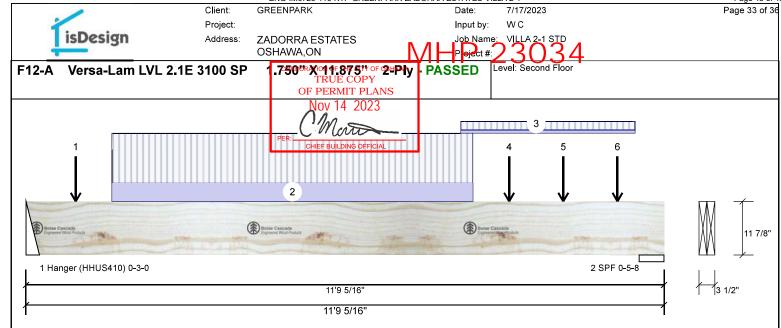
Manufacturer Info

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www.bc.com CCMC: 12472





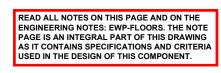


Member Inform	nation				Unfa	actored Re	eaction	s UNP	ATTERNED II	o (Uplift)	
Type:	Girder		Application:	Floor (Residential)	Brg	Direction		Live	Dead	Snow	Wind
Plies:	2		Design Method:	LSD	1	Vertical		1631	683	0	0
Moisture Condition: Deflection LL:	Dry 360		Building Code:	NBCC 2015 OBC 2012(2020 Update)	2	Vertical		1820	784	0	0
Deflection TL: Importance: General Load	240 Normal - II		Load Sharing: Deck: Vibration:	No Not Checked Not Checked							
Floor Live:	40 PSF				Bear	ings and	Factor	ed Read	ctions		
Dead:	15 PSF				Bea	aring Lengtl	n Dir.	Сар.	React D/L Ib	Total Ld. Case	Ld. Comb.
					1 - Har	3.000" nger	Vert	29%	854 / 2446	3300 L	1.25D+1.5L
Analysis Results	5				2 -	SPF 5.500"	Vert	31%	980 / 2730	3710 L	1.25D+1.5L
Analysis Actu	ual	Location A	llowed Capac	ity Comb. Case							

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9495 ft-lb	5'10 1/2"	35392 ft-lb	0.268 (27%)	1.25D+1.5L	L
Unbraced	9495 ft-lb	5'10 1/2"	35392 ft-lb	0.268 (27%)	1.25D+1.5L	L
Shear	3613 lb	10'3 15/16"	13217 l b	0.273 (27%)	1.25D+1.5L	L
Perm Defl in.	0.044 (L/3056)	5'9 13/16"	0.373 (L/360)	0.118 (12%)	D	Uniform
LL Defl inch	0.105 (L/1280)	5'9 3/4"	0.373 (L/360)	0.281 (28%)	L	L
TL Defl inch	0.149 (L/902)	5'9 3/4"	0.560 (L/240)	0.266 (27%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: DF, Thickness: 3 1/2"
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Multiple plies must be fastened together as per manufacturer's details.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be continuously laterally braced.
- 8 Bottom must have sheathing attached or be continuously braced.
- 9 Lateral slenderness ratio based on full section width.



JULY 19, 2023

PROFESSION

I.MATIJEVIC 100528832

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-11-0		Near Face	144 l b	384 lb	0 lb	0 lb	J6
2	Part. Uniform	1-7-0 to 8-3-0		Near Face	109 PLF	291 PLF	0 PLF	0 PLF	
3	Part. Uniform	8-0-5 to 11-2-15		Тор	19 PLF	50 PLF	0 PLF	0 PLF	
4	Point	8-11-0		Near Face	131 lb	339 lb	0 lb	0 lb	J6

Continued on page 2...

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12472

Manufacturer Info







READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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www.bc.com CCMC: 12472

Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400







Client: **GREENPARK**

Project:

7/17/2023 Date:

W C Input by:

Address:

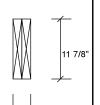
ZADORRA ESTATES OSHAWA,ON

Job Name: P <mark>eje</mark>ct #: Level: Second Floor **PASSED**

Versa-Lam LVL 2.1E 3100 SP

.750thXi011:875th OF 2HPly TRUE COPY OF PERMIT PLANS

lov 14 2023



1 SPF 0-5-4 2 SPF 0-4-2 1'4 1/8' 1'4 1/8

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal - II
General Load	

15 PSF

Application: Floor (Residential) Design Method: LSD Building Code: **NBCC 2015** OBC 2012(2020 Update) Load Sharing:

Not Checked Deck: Vibration: Not Checked 40 PSF

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	176	74	0	0
2	Vertical	41	23	0	0
1					

Bearings and Factored Reactions

Bearing	Length	Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	Vert	3%	93 / 264	357	L	1.25D+1.5L
2 - SPF	4.125"	Vert	1%	28 / 61	89	L	1.25D+1.5L

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	45 ft-lb	6 1/16"	35392 ft-lb	0.001 (0%)	1.25D+1.5L	L
Unbraced	45 ft-lb	6 1/16"	35392 ft-lb	0.001 (0%)	1.25D+1.5L	L
Shear	351 lb	1'5 1/8"	13217 lb	0.027 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/3030066)	6 1/16"	0.023 (L/360)	0.000 (0%)	D	Uniform
LL Defl inch	0.000 (L/1195211)	6 1/16"	0.023 (L/360)	0.000 (0%)	L	L
TL Defl inch	0.000 (L/857120)	6 1/16"	0.034 (L/240)	0.000 (0%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top must be continuously laterally braced.

Self Weight

- 5 Bottom must have sheathing attached or be continuously braced.
- 6 Lateral slenderness ratio based on full section width.



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Trib Width ID Load Type Location Side Dead Live Snow Wind Comments Point 0-6-1 Far Face 81 lb 217 lb 0 lb 0 lb

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. UVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

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Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
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6. For flat roofs provide proper drainage to prevent ponding

12 PLF

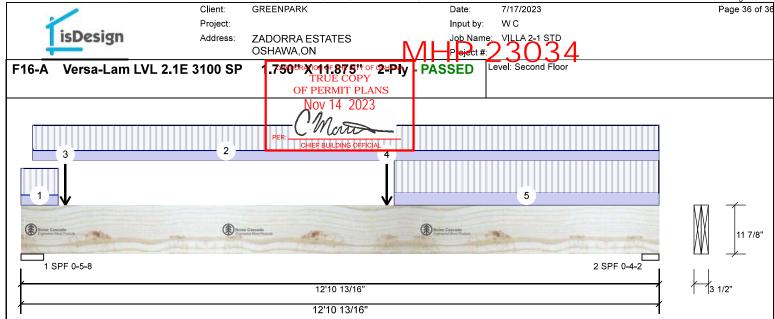
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Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400





Member Information					Unfactored Reactions UNPATTERNED lb (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Live	Dead	Snow	Wind			
Plies:	2	Design Method:	LSD	1	Vertical	1968	928	0	0			
Moisture Condition	n: Dry	Building Code:	NBCC 2015	2	Vertical	621	371	0	0			
Deflection LL:	360		OBC 2012(2020 Update)									
Deflection TL:	240	Load Sharing:	No									
Importance:	Normal - II	Deck:	Not Checked									
General Load		Vibration:	Not Checked									
Floor Live:	40 PSF			Bear	ings and Fa	actored Rea	ctions					
Dead:	15 PSF			Bea	aring Length	Dir. Cap.	React D/L lb	Total Ld. Case	Ld. Comb.			
				1 - 3	SPF 5.500"	Vert 35%	1160 / 2952	4111 L	1.25D+1.5L			
				2 - :	SPF 4.125"	Vert 16%	463 / 931	1395 L	1.25D+1.5L			

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5685 ft-lb	7'4 13/16"	35392 ft-lb	0.161 (16%)	1.25D+1.5L	L
Unbraced	5685 ft-lb	7'4 13/16"	35392 ft-lb	0.161 (16%)	1.25D+1.5L	L
Shear	4024 lb	1'5 3/8"	13217 l b	0.304 (30%)	1.25D+1.5L	L
Perm Defl in.	0.036 (L/4052)	6'7 1/16"	0.408 (L/360)	0.089 (9%)	D	Uniform
LL Defl inch	0.062 (L/2365)	6'6 11/16"	0.408 (L/360)	0.152 (15%)	L	L
TL Defl inch	0.098 (L/1493)	6'6 13/16"	0.611 (L/240)	0.161 (16%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.
- 6 Bottom must be laterally braced at a maximum of 6'6" o.c.
- 7 Lateral slenderness ratio based on full section width.



JULY 19, 2023

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 0-9-0	0-6-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-2-13 to 12-10-13	0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	0-10-12		Near Face	683 lb	1631 lb	0 lb	0 l b	F12	
4	Point	7-4-13		Near Face	308 lb	545 lb	0 lb	0 l b	F12	
5	Tie-In	7-6-9 to 12-10-13	0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
	Self Weight				12 PLF					

Notes

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