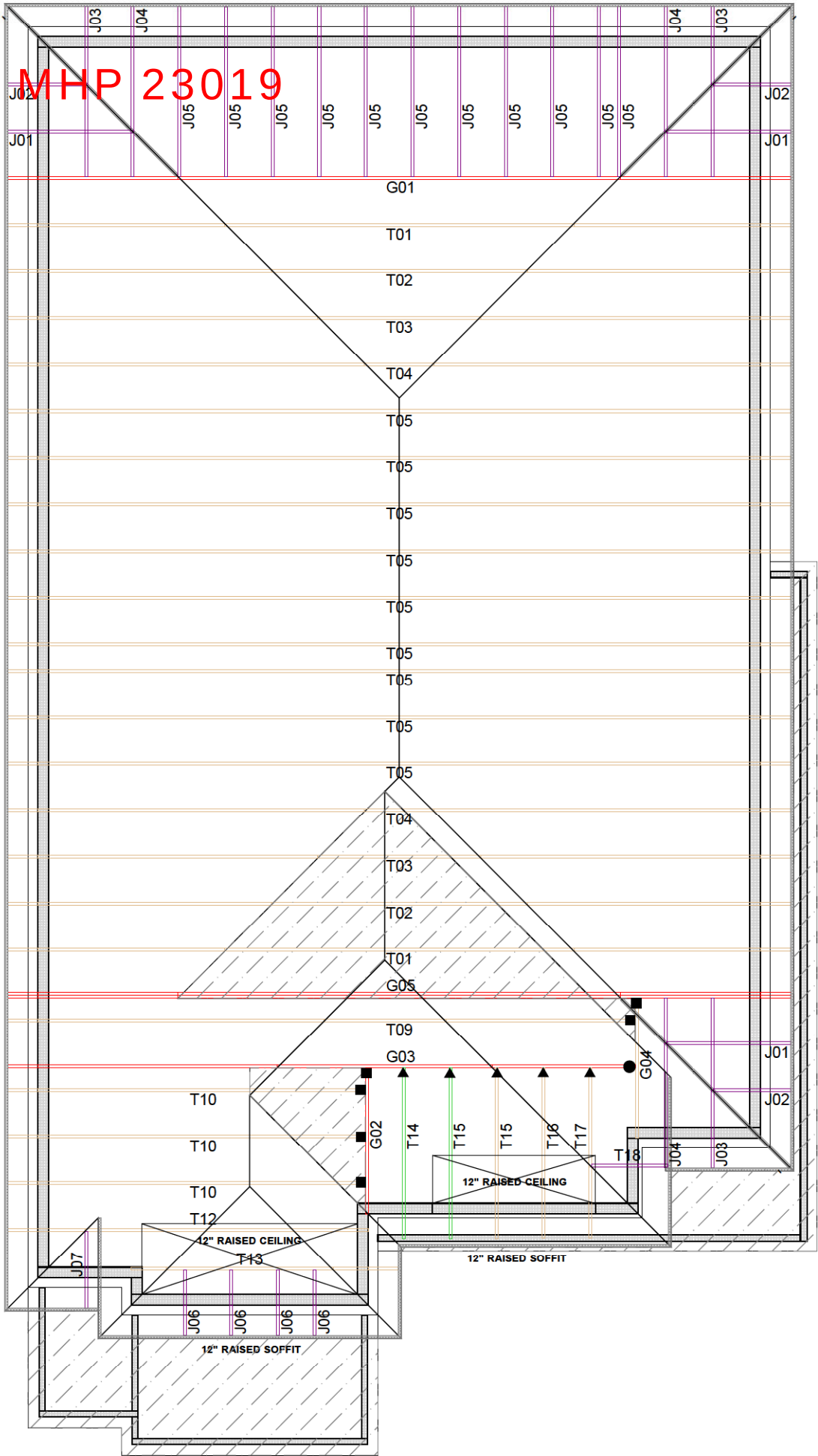




Hanger Name	Symbol	QTY
LUS24	▲	5
LJS26DS	■	6
HGUS26	●	1
	⬆	0
	△	0
	□	0
	⬆	0
	○	0

JOB INFORMATION

Customer	GREENPARK HOMES
Job #	22-00200R0
Address	ZADORRA PENROSE 2-3 OSHAWA,ON
Model	PENROSE 2-3
Sales Rep	RALPH MIRIGELLO
Designer	BB
Date	6/1/2022
Path	S:\DESIGN\KLU\CUSTOMERS\GREENPARK\ZADORRA ESTATES\MODELS\PENROSE 2\PENROSE 2-3\T

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	3.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





ENGINEERING NOTE PAGE (ENP-1)

PLEASE READ PRIOR TO INSTALLATION

MHP 23019

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

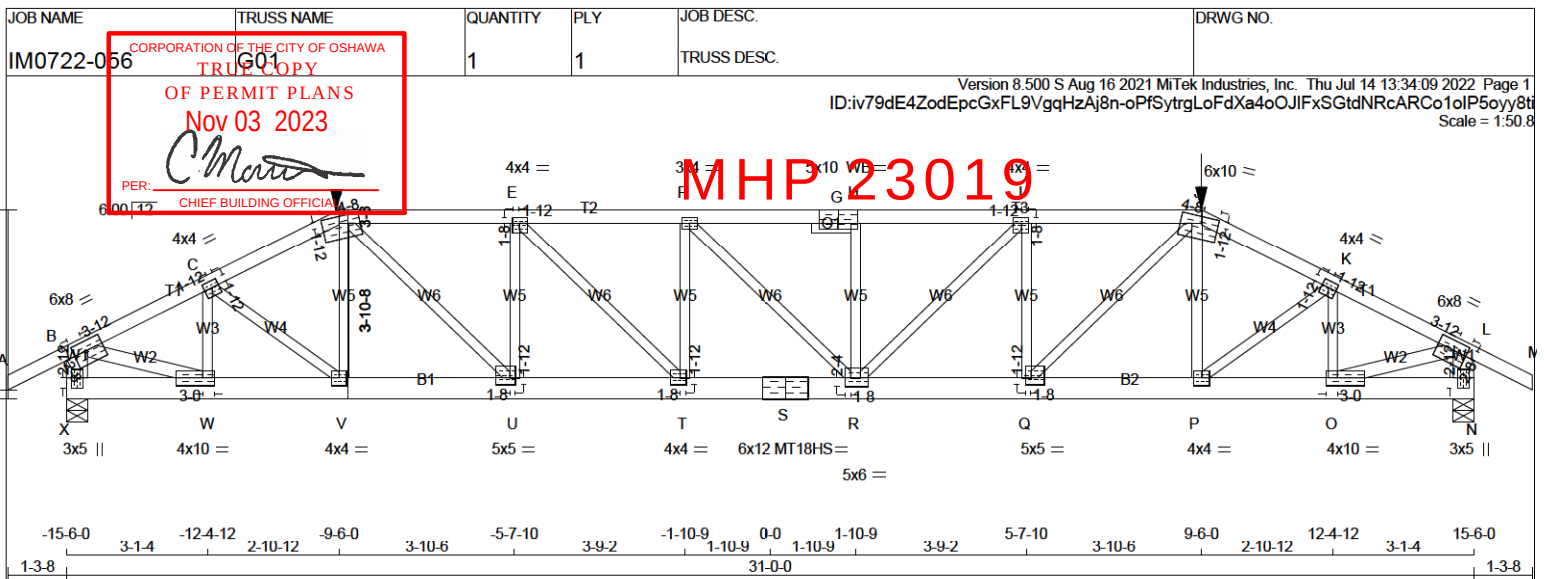
SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	G01	1	1	TRUSS DESC.	

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OF PERMIT PLANS

Nov 03 2023

WB - INDICATES BLC
PER: 
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:09 2022 Page 2
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MHP 23019



July 14, 2022

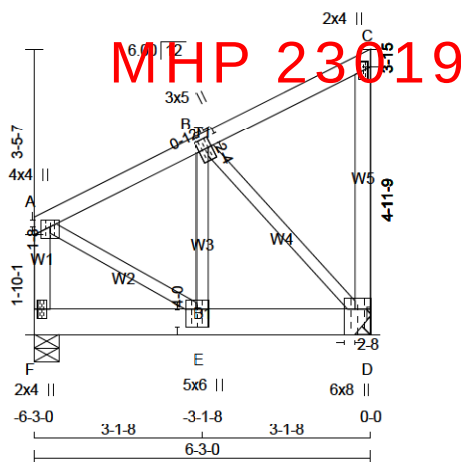
READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS ADV	1	1	TRUSS DESC.	

66 CORPORATION OF THE CITY OF OSHAWA
G02
TRUE COPY
OF PERMIT PLANS
Nov 03 2023
C. Morris
PER: _____
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:10 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-HbDq9Drl66NU9kf_y0GUUgP9Snpov?rxGS1ydEyy8th
Scale = 1:42.8



TOTAL WEIGHT = 35 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			SPF

PLATES (table is in inches)		W	LEN	Y	X
JT	TYPE	PLATES			
A	TMVW+p	MT20	4.0	4.0	1.50 2.00
B	TMVW+t	MT20	3.0	5.0	2.25 0.75
C	TMV+p	MT20	2.0	4.0	
D	BMVW1+t	MT20	6.0	8.0	Edge 2.50
E	BMVW+t	MT20	5.0	6.0	4.0 2.50
F	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			MAXIMUM FACTORED			INPUT	REQRD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1584	0	1584	0	0	5-8	1-11
D	1584	0	1584	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1098	847 / 0	0 / 0	0 / 0	0 / 0	251 / 0	0 / 0
D	1098	847 / 0	0 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
F-A	-1080 / 0	0.0	0.0	0.13 (1)	7.57	A-E	0 / 1016		
A-B	-971 / 0	-111.9	-111.9	0.20 (1)	6.04	E-B	0 / 959		
B-C	-17 / 0	-111.9	-111.9	0.18 (1)	6.25	B-D	-1293 / 0		
D-C	-137 / 0	0.0	0.0	0.06 (1)	7.81		0.43 (1)		
F-E	0 / 0	-395.1	-395.1	0.26 (1)	10.00				
E-D	0 / 883	-395.1	-395.1	0.37 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP	CH.	LL =	34.8 PSF
		DL =	3.0 PSF
BOT	CH.	LL =	0.0 PSF
		DL =	7.3 PSF
TOTAL LOAD		=	45.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 14-0-8
END DISTANCE = 6-3-0
END SPAN CARRIED = 14-0-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.20/1.00 (A-B:1), BC=0.37/1.00 (D-E:1),

WB=0.43/1.00 (B-D:1) , SSI=0.48/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

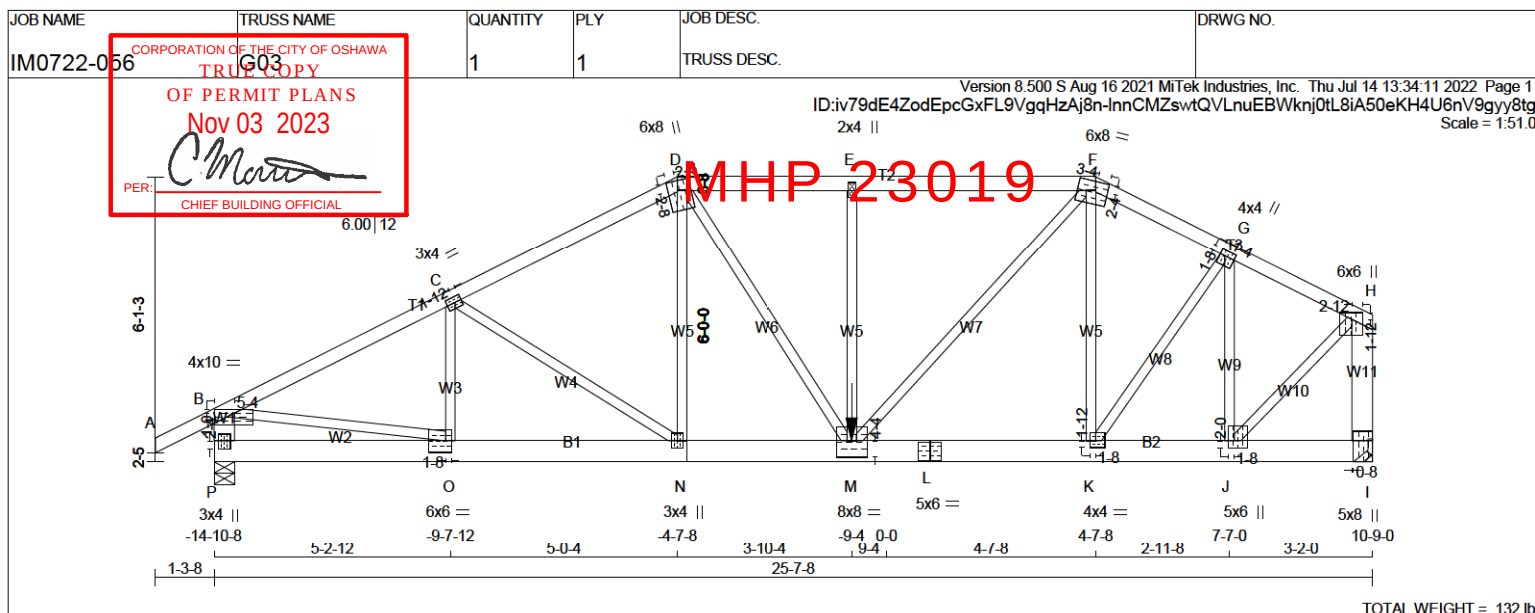
JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



July 14, 2022

**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
P - B	2x6	DRY	No.2
I - H	2x6	DRY	No.2
P - L	2x6	DRY	No.2
L - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	4.0	10.0	1.00	5.25
C	TMWW-t	MT20	3.0	4.0	1.50	1.75
D	TTWW+m	MT20	6.0	8.0	2.50	2.50
E	TMWW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	6.0	8.0	2.25	3.25
G	TMWW+t	MT20	4.0	4.0	1.50	1.25
H	TMWW+p	MT20	6.0	6.0	1.75	2.75
I	BMV1+t	MT20	5.0	8.0	Edge	0.50
J	BMWW+t	MT20	5.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	4.0	1.75	1.50
L	BS-t	MT20	5.0	6.0		
M	BMWWWW-t	MT20	8.0	8.0	4.25	4.00
N	BMWW+t	MT20	3.0	4.0		
O	BMWW-t	MT20	6.0	6.0	3.00	1.50
P	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
P	2851	0	2851	0
I	3641	0	3641	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 5-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1973	1538 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
I	2523	1947 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED LC1 (LC)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 34	-111.9	-111.9 0.16 (1)	10.00	O-C	-506 / 0	0.12 (1)
B-C	-4114 / 0	-111.9	-111.9 1.00 (1)	2.50	C-N	-208 / 0	0.14 (1)
C-D	-3979 / 0	-111.9	-111.9 0.94 (1)	2.58	N-D	0 / 226	0.06 (1)
D-E	-4303 / 0	-111.9	-111.9 0.74 (1)	2.56	D-M	0 / 1406	0.35 (1)
E-F	-4303 / 0	-111.9	-111.9 0.88 (1)	2.56	M-E	-626 / 0	0.38 (1)
F-G	-3412 / 0	-111.9	-111.9 0.33 (1)	3.48	M-F	0 / 1889	0.47 (1)
G-H	-2622 / 0	-111.9	-111.9 0.27 (1)	4.00	K-F	-254 / 8	0.15 (1)
P-B	-2782 / 0	0.0	0.0 0.20 (1)	6.23	K-G	0 / 1185	0.29 (1)
I-H	-3435 / 0	0.0	0.0 0.39 (1)	5.69	J-G	-1735 / 0	0.58 (1)
P-O	0 / 0	-18.2	-18.2 0.09 (1)	10.00	B-O	0 / 3745	0.93 (1)
O-N	0 / 3704	-18.2	-18.2 0.57 (1)	10.00	J-H	0 / 3167	0.78 (1)
N-M	0 / 3535	-18.2	-18.2 0.50 (1)	10.00			
M-L	0 / 3029	-141.6	-141.6 0.63 (1)	10.00			
L-K	0 / 3029	-141.6	-141.6 0.63 (1)	10.00			
K-J	0 / 2359	-141.6	-141.6 0.53 (1)	10.00			
J-I	0 / 0	-141.6	-141.6 0.11 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR	TYPE	HEEL	CONN.
M	14-14	-1584	-1584	MAX+	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 14-14
START SPAN CARRIED = 6-3-0
END DISTANCE = 25-7-8
END SPAN CARRIED = 6-3-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.85")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.16")
ALLOWABLE DEFL. (TL) = $L/360$ (0.85")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.27")

CSI: TC=1.00/1.00 (B-C-1), BC=0.63/1.00 (K-M-1), WB=0.93/1.00 (B-O-1), SSI=0.30/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	G03	1	1	TRUSS DESC.	

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MHP 23019

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.90 (B) (INPUT = 0.90)
 JSI METAL= 0.84 (O) (INPUT = 1.00)



July 14, 2022

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA G04 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: [Signature] CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	
Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:11 2022 Page 1 ID:iv79dE4ZodEpcGxFL9VggHzAj8n-InnCMZswtQVLnuEBWknj0tLGrA4leLB4U6nV9gyy8tg Scale = 1:17.9					
TOTAL WEIGHT = 31 lb					
LUMBER					
N.L.G.A. RULES	SIZE	LUMBER	DESCR.	SPF	
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x8	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
E - B	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A TMVW-t	MT20	4.0	10.0	1.75	3.00
B TMW+w	MT20	2.0	4.0		
C TMVW-t	MT20	4.0	10.0	1.75	2.75
D BMV1+t	MT20	4.0	10.0	Edge	0.50
E BMWWW-t	MT20	8.0	16.0	5.50	8.00
F BMV1+p	MT20	3.0	4.0	2.00	0.50
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.					
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
JT	FACTORED GROSS REACTION VERT	HORIZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORIZ UPLIFT	INPUT BRG IN-SX
F	2715	0	2715	0	5-8
D	3920	0	3920	0	MECHANICAL
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.					
UNFACTORED REACTIONS					
JT	1ST CASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND DEAD SOIL
F	1881	1452 / 0	0 / 0	0 / 0	430 / 0 0 / 0
D	2717	2096 / 0	0 / 0	0 / 0	621 / 0 0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F					
BRACING					
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.					
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.					
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.					
LOADING					
TOTAL LOAD CASES: (4)					
CHORDS					
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRACED LENGTH	WEBS
FR-TO					MEMB.
F-A	-2378 / 0	0.0	0.0 0.27 (1)	5.49	A-E
A-B	-4342 / 0	-111.9	-111.9 0.47 (1)	2.92	E-B
B-C	-4342 / 0	-111.9	-111.9 0.45 (1)	2.95	E-C
D-C	-2465 / 0	0.0	0.0 0.28 (1)	5.39	
F-E	0 / 0	-18.2	-18.2 0.47 (1)	10.00	
E-D	0 / 0	-771.9	-771.9 0.68 (1)	10.00	
FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1 MAX-	MAX+	FACE DIR	TYPE HEEL CONN.
E	3-0-12	-3640	-3640	— FRONT VERT	TOTAL — C1
CONNECTION REQUIREMENTS					
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.					
DESIGN CRITERIA					
*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE					
SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 45.1 PSF					
SPACING = 24.0 IN. C/C					
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM					
GIRDER TYPE: CSStd Girder START DISTANCE = 3-0-12 START SPAN CARRIED = 25-7-8 END DISTANCE = 6-0-0 END SPAN CARRIED = 25-7-8 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDTL LOADS BASED ON 55% OF GSL.					
*** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.					
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015					
THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014					
(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD					
ALLOWABLE DEFL (LL) = L/360 (0.20") CALCULATED VERT. DEFL (LL) = L/999 (0.06") ALLOWABLE DEFL (TL) = L/360 (0.20") CALCULATED VERT. DEFL (TL) = L/757 (0.10")					
CSI: TC=0.47/1.00 (A-B-1), BC=0.68/1.00 (D-E-1), WB=0.87/1.00 (C-E-1), SSI=0.85/1.00 (D-E-1)					
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00					
COMPANION LIVE LOAD FACTOR = 1.00					
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.					
CONTINUED ON PAGE 2					
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.					
KOTT					

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	G04	1	1	TRUSS DESC.	

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Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:11 2022 Page 2
ID:iv79dE4ZodEpcGxFL9VggHzAj8n-InnCMZswtQVLnuEBWknj0tLGrA4leLB4U6nV9ggy8tg

MHP 23019

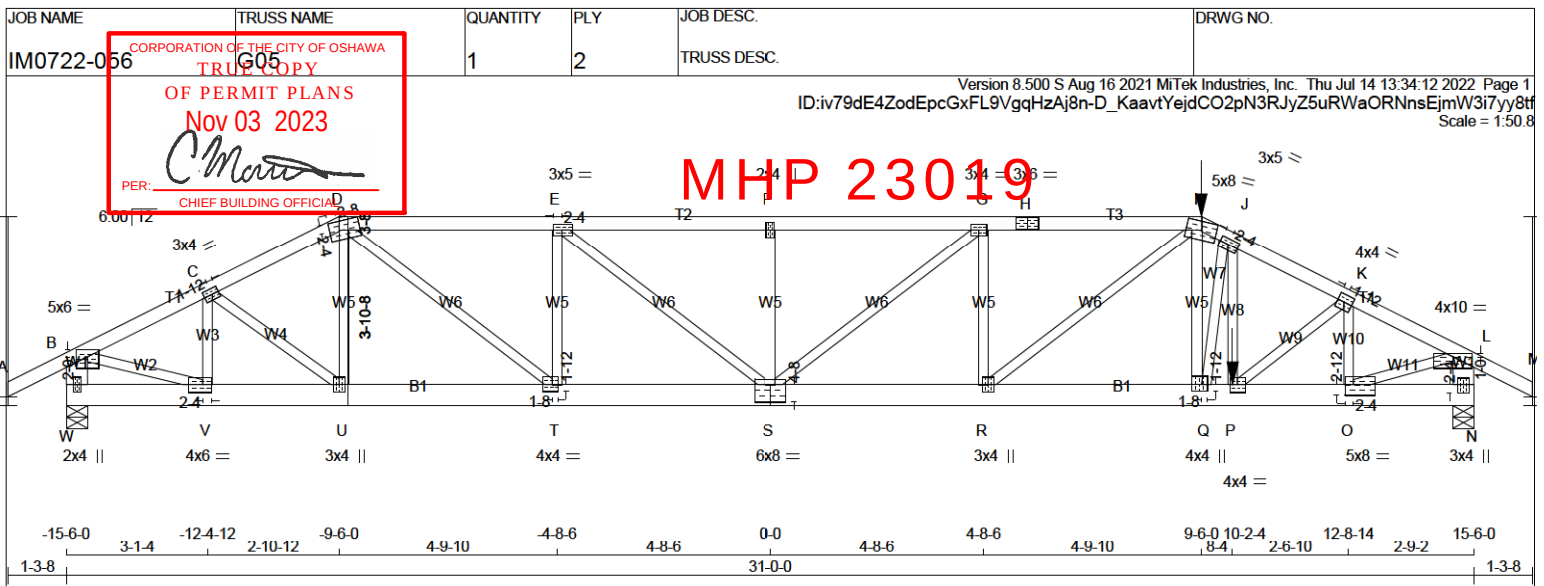
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.90 (C) (INPUT = 0.90)
 JSI METAL= 0.63 (E) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE
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**LUMBER**

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2 SPF
D - H	2x4	DRY	No.2 SPF
H - I	2x4	DRY	No.2 SPF
I - M	2x4	DRY	No.2 SPF
W - B	2x6	DRY	No.2 SPF
N - L	2x6	DRY	No.2 SPF
W - S	2x6	DRY	No.2 SPF
S - N	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-H 1	12	TOP
H-I 1	12	SIDE(0.0)
I-M 1	12	SIDE(0.0)
W-B 2	12	TOP
N-L 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
W-S 2	12	TOP
S-N 2	12	SIDE(9.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY**BUILDING DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
W	2947	0	2947	0
N	5910	0	5910	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2039	1593 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0
N	4095	3163 / 0	0 / 0	0 / 0	0 / 0	933 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX. UNBRACED LENGTH	FR-TO	WEBS	MAX. FACTORED	FORCE	MAX
MEMB.	FORCE (LBS)	(PLF)	FROM	TO	CSI (LC)	FR-TO	MEMB.	FORCE (LBS)	CSI (LC)		
A-B	0 / 34	-111.9	-111.9	0.08 (1)	10.00	V-C	-947 / 0	0.08 (1)			
B-C	-3808 / 0	-111.9	-111.9	0.13 (1)	4.74	C-U	0 / 543	0.07 (1)			
C-D	-4316 / 0	-111.9	-111.9	0.12 (1)	4.51	U-D	-131 / 27	0.02 (1)			
D-E	-6001 / 0	-111.9	-111.9	0.36 (1)	3.70	D-T	0 / 2750	0.34 (1)			
E-F	-7341 / 0	-111.9	-111.9	0.44 (1)	3.31	T-E	-1675 / 0	0.22 (1)			
F-G	-7341 / 0	-111.9	-111.9	0.43 (1)	3.32	E-S	0 / 1722	0.21 (1)			
G-H	-7915 / 0	-111.9	-111.9	0.48 (1)	3.17	S-F	-483 / 0	0.06 (1)			
H-I	-7915 / 0	-111.9	-111.9	0.48 (1)	3.17	S-G	-737 / 0	0.23 (1)			
I-J	-8693 / 0	-111.9	-111.9	0.36 (1)	3.03	R-G	-123 / 61	0.02 (1)			
J-K	-9017 / 0	-111.9	-111.9	0.42 (1)	2.94	R-I	0 / 170	0.02 (1)			
K-L	-7871 / 0	-111.9	-111.9	0.28 (1)	3.31	Q-I	0 / 2881	0.36 (1)			
L-M	0 / 34	-111.9	-111.9	0.08 (1)	10.00	Q-J	-1100 / 0	0.13 (1)			
W-B	-2860 / 0	0.0	0.0	0.10 (1)	7.81	P-J	0 / 1237	0.15 (1)			
N-L	-5795 / 0	0.0	0.0	0.20 (1)	6.12	P-K	0 / 1295	0.16 (1)			
						O-K	-1717 / 0	0.15 (1)			
W-V	0 / 0	-18.2	-18.2	0.07 (1)	10.00	B-V	0 / 3521	0.44 (1)			
V-U	0 / 3416	-18.2	-18.2	0.29 (1)	10.00	O-L	0 / 7316	0.91 (1)			
U-T	0 / 3841	-18.2	-18.2	0.28 (1)	10.00						
T-S	0 / 6001	-18.2	-18.2	0.43 (1)	10.00						
S-R	0 / 7915	-18.2	-18.2	0.57 (1)	10.00						
R-Q	0 / 7782	-18.2	-18.2	0.59 (1)	10.00						
Q-P	0 / 8033	-34.4	-34.4	0.81 (1)	10.00						
P-O	0 / 7044	-34.4	-34.4	0.75 (1)	10.00						
O-N	0 / 0	-34.4	-34.4	0.07 (1)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR	TYPE	HEEL	CONN.
I	25-0.0	-503	-503	—	FRONT	VERT	TOTAL	—	C1
P	25-8.4	-3920	-3920	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 6-0-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL
LOADS APPLIED TO FIRST 6-2-0 OF SPAN
MEASURED FROM THE RIGHT.

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.03")
CALCULATED VERT. DEFL (LL) = L/999 (0.21")
ALLOWABLE DEFL (TL) = L/360 (1.03")
CALCULATED VERT. DEFL (TL) = L/999 (0.34")

CSI: TC=0.48/1.00 (G-I-1), BC=0.81/1.00 (P-Q-1), WB=0.91/1.00 (L-O-1), SSI=0.56/1.00 (P-Q-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	G05	1	2	TRUSS DESC.	

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Nov 03 2023

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:13 2022 Page 2
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PLATES (table is in it)

JT	TYPE	PLA	PERM	GRIP (DRY)	SHEAR	SECTION
				(PSI)	(PLI)	(PLI)
				MAX MIN	MAX MIN	MAX MIN
B	TMVW-p	MT20	5.0	8.0	2.25	2.50
C	TMVW-t	MT20	3.0	5.0	1.50	2.25
D	TTWW-m	MT20	2.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	TMVW-w	MT20	3.0	4.0		
G	TMVW-t	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TTWW-m	MT20	5.0	8.0	Edge	
J	TMVW-t	MT20	3.0	5.0	1.50	2.25
K	TMVW-t	MT20	4.0	4.0	2.00	1.75
L	TMVW-p	MT20	4.0	10.0	1.00	5.00
N	BMV1+p	MT20	3.0	4.0	2.25	1.50
O	BMVW-t	MT20	5.0	8.0	2.75	2.25
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	4.0	4.0	1.75	1.50
R	BMVW-t	MT20	3.0	4.0		
S	BSWWW-I	MT20	6.0	8.0	4.50	4.00
T	BMVW-t	MT20	4.0	4.0	1.75	1.50
U	BMVW-t	MT20	3.0	4.0		
V	BMVW-t	MT20	4.0	6.0	2.00	2.25
W	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

MHP 23019

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)

JSI METAL= 0.71 (I) (INPUT = 1.00)



July 14, 2022

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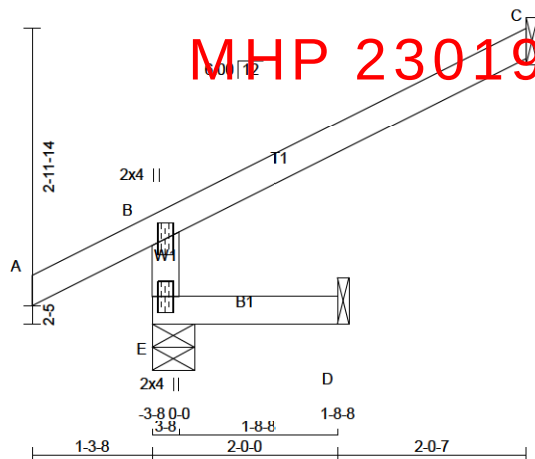


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS COPY	3	1	TRUSS DESC.	

OF PERMIT PLANS
Nov 03 2023

PER: *C. Morin*
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:13 2022 Page 1
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Scale = 1:24.8



TOTAL WEIGHT = 3 X 10 = 30 lb

<u>LUMBER</u>					
N. L. G. A. RULES					
CHORDS		SIZE		LUMBER	DESCR.
E - B	2x4	DRY		No.2	SPF
A - C	2x4	DRY		No.2	SPF
E - D	2x4	DRY		No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	454	0	454	0	5-8	1-8
C	169	0	169	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX / MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
E	309	270 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
C	114	105 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	MAX. LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED (PLF)							MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO										
E-B	-434 / 0	0.0	0.0	0.01 (4)	7.81					
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00					
B-C	-25 / 0	-111.9	-111.9	0.31 (1)	6.25					
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.31/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

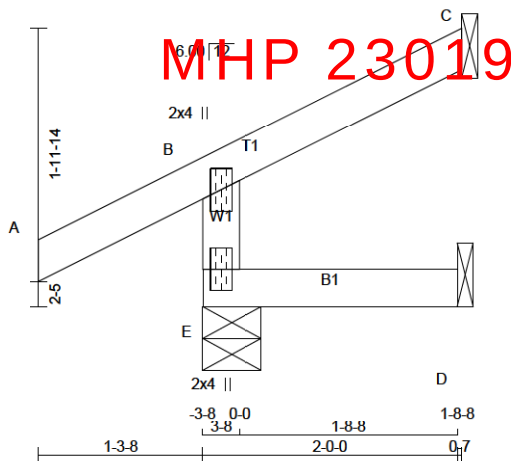


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS COPY	3	1	TRUSS DESC.	

OF PERMIT PLANS
Nov 03 2023

PER: *C. Morin*
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:14 2022 Page 1
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Scale = 1:18.1



TOTAL WEIGHT = 3 X 7 = 22 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	314	0	314	0
C	86	0	86	0
D	16	0	16	0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	215	183/0	0/0	0/0	0/0	32/0	0/0
C	58	54/0	0/0	0/0	0/0	5/0	0/0
D	13	0/0	0/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
E-B	-294 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00		
B-C	-12 / 0	-111.9	-111.9	0.08 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

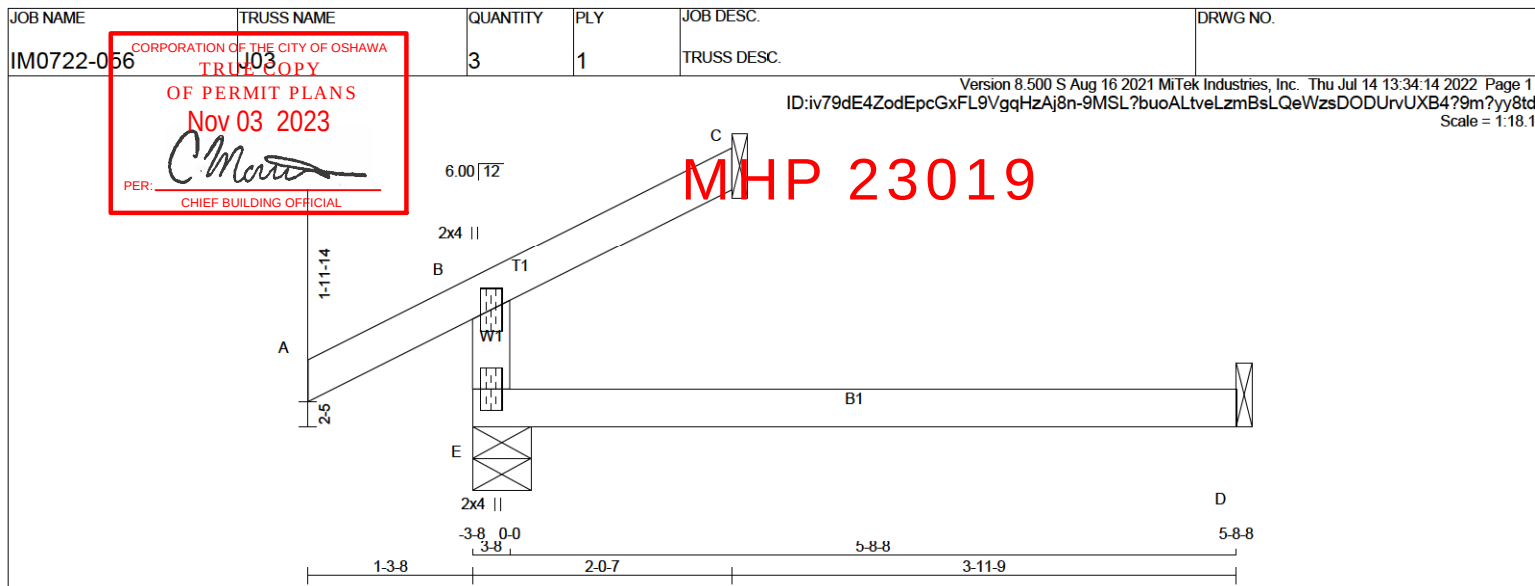
JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 12 = 36 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	DOWN	IN-SX	IN-SX
E	358	0	5-8	1-8
C	86	0	1-8	1-8
D	45	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	183 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	58	54 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. CS1 (LC)	UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO										
E-B		-294 / 0	0.0	0.0	0.13 (4)	7.81				
A-B		0 / 34	-111.9	-111.9	0.15 (1)	10.00				
B-C		-12 / 0	-111.9	-111.9	0.08 (1)	6.25				
E-D		0 / 0	-18.2	-18.2	0.13 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
 ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.15/1.00 (A-B:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
 JSI METAL= 0.12 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

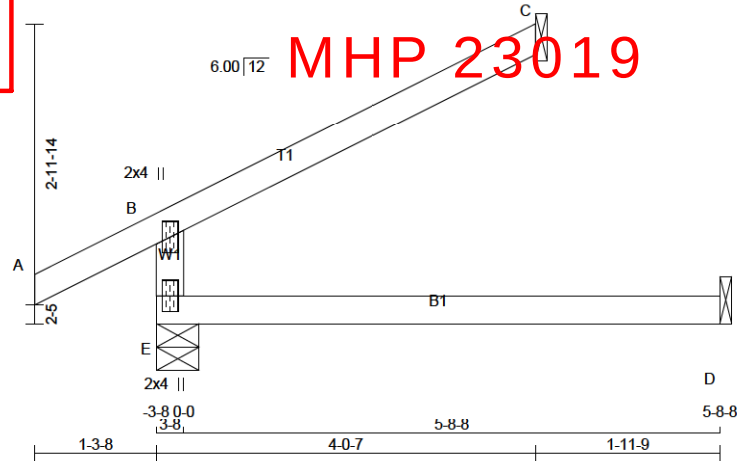


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS COPY	3	1	TRUSS DESC.	

OF PERMIT PLANS
Nov 03 2023

PER: *C. Morin*
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:15 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-dZ0jCvxQxe?mFVYylasfBjW?PoZjaMkgPkIjIRyy8tc
Scale = 1:24.5



TOTAL WEIGHT = 3 X 15 = 44 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	498	0	498	0	5-8	1-8
C	169	0	169	0	1-8	1-8
D	45	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	345	270 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	114	105 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. CSI (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)					MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
E-B	-434 / 0	0.0	0.0	0.13 (4)	7.81					
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00					
B-C	-25 / 0	-111.9	-111.9	0.31 (1)	6.25					
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.31/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



July 14, 2022

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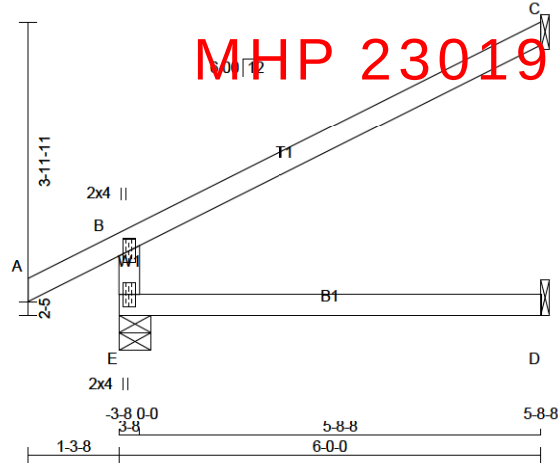


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS COPY	11	1	TRUSS DESC.	

OF PERMIT PLANS
Nov 03 2023

PER: *C. Morin*
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:16 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-5la5QGw3iy7dtf78IHNUjx34GBvyJp_peOUGruffy8tb
Scale = 1:32.8



TOTAL WEIGHT = 11 X 17 = 188 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
E	636	0	636	0
C	252	0	252	0
D	45	0	51	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	437	355 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	170	157 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
E-B		-571 / 0	0.0	0.0	0.13 (4)	7.81			
A-B		0 / 34	-111.9	-111.9	0.15 (1)	10.00			
B-C		-38 / 0	-111.9	-111.9	0.69 (1)	6.25			
E-D		0 / 0	-18.2	-18.2	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.69/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

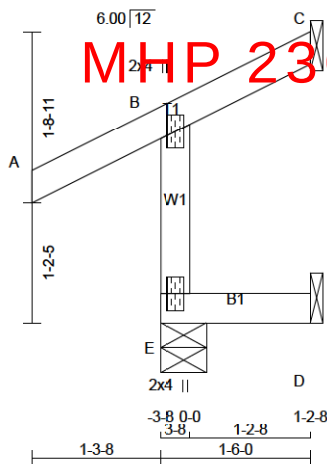


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS COPY	4	1	TRUSS DESC.	

OF PERMIT PLANS
Nov 03 2023

PER: *C. Morin*
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:17 2022 Page 1
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Scale = 1:23.1



TOTAL WEIGHT = 4 X 7 = 29 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	318	0	318	0	5-8	1-8
C	26	0	26	0	-51	1-8
D	3	0	14	0	-4	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	217	189 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
C	17	16 / -34	0 / 0	0 / 0	0 / 0	2 / 0	0 / 0
D	4	0 / -8	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	LC2 MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-294 / 0	0.0	0.0	0.03 (5)	7.81		
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00		
B-C	-26 / 0	-111.9	-111.9	0.12 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.03 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.03/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

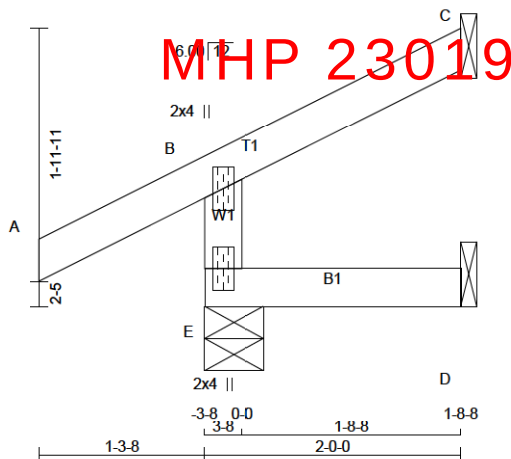


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS COPY	1	1	TRUSS DESC.	

OF PERMIT PLANS
Nov 03 2023

PER: *C. Morin*
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:17 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-Zx8TdcxhTGFUVpiKs_u7G8bNTbH12GEzt2EqNKyy8ta
Scale = 1:18.0



TOTAL WEIGHT = 7 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	311	0	311	0	5-8	1-8
C	85	0	85	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E	213	181 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0	
C	57	53 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0	
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 (LC)	MAX. CS1 (LC)	MAX. UNBRAC	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CS1 (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
E-B	-291 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00				
B-C	-12 / 0	-111.9	-111.9	0.07 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME TRUSS NAME QUANTITY PLY JOB DESC. DRWG NO.

IM0722-056 CORPORATION OF THE CITY OF OSHAWA TRUE COPY

OF PERMIT PLANS Nov 03 2023

PER: [Signature] CHIEF BUILDING OFFICIAL

MHP 23019

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:18 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-28irgyyJEZNL6zGXQiQmPM8TO?VpnaP66izNvmyy8tZ
Scale = 1:50.8

LUMBER N L G A RULES CHORDS SIZE LUMBER DESCR. ALL WEBS EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.75
D	TTWW+m	MT20	6.0	6.0	2.50	2.00
E	TMW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	3.0	4.0		
G	TTWW+m	MT20	6.0	6.0	2.50	2.00
H	TMWW-t	MT20	3.0	4.0	1.50	1.75
I	TMWW-t	MT20	5.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	1.75	1.50
M	BMWW-t	MT20	3.0	4.0		
N	BMWW-t	MT20	4.0	4.0	2.00	1.75
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0	1.75	2.00
Q	BMWW-t	MT20	3.0	4.0		
R	BMWW-t	MT20	4.0	6.0	1.75	1.50
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX		
S	2169	0	2169	0	5-8	3-9
K	2169	0	2169	0	5-8	3-9

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1500	1173 / 0	0 / 0	0 / 0	0 / 0	327 / 0	0 / 0
K	1500	1173 / 0	0 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS			WEBS			
		VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00	R-C	-468 / 0	0.09 (1)
B-C	-2809 / 0	-111.9	-111.9	0.31 (1)	3.90	C-Q	-81 / 0	0.03 (1)
C-D	-2781 / 0	-111.9	-111.9	0.30 (1)	3.92	Q-D	0 / 143	0.03 (4)
D-E	-3182 / 0	-111.9	-111.9	0.45 (1)	3.53	D-P	0 / 993	0.22 (1)
E-F	-3182 / 0	-111.9	-111.9	0.42 (1)	3.58	P-E	-602 / 0	0.24 (1)
F-G	-3184 / 0	-111.9	-111.9	0.45 (1)	3.53	P-F	-3 / 0	0.00 (1)
G-H	-2780 / 0	-111.9	-111.9	0.30 (1)	3.92	N-F	-603 / 0	0.24 (1)
H-I	-2809 / 0	-111.9	-111.9	0.31 (1)	3.90	N-G	0 / 996	0.22 (1)
I-J	0 / 34	-111.9	-111.9	0.15 (1)	10.00	M-G	0 / 142	0.03 (4)
S-B	-2130 / 0	0.0	0.0	0.22 (1)	5.81	M-H	-82 / 0	0.03 (1)
K-I	-2130 / 0	0.0	0.0	0.22 (1)	5.81	L-H	-467 / 0	0.09 (1)
S-R	0 / 0	-18.2	-18.2	0.07 (4)	10.00	B-R	0 / 2586	0.58 (1)
R-Q	0 / 2531	-18.2	-18.2	0.46 (1)	10.00	L-I	0 / 2586	0.58 (1)
Q-P	0 / 2469	-18.2	-18.2	0.45 (1)	10.00			
P-O	0 / 3184	-18.2	-18.2	0.56 (1)	10.00			
O-N	0 / 3184	-18.2	-18.2	0.56 (1)	10.00			
N-M	0 / 2468	-18.2	-18.2	0.44 (1)	10.00			
M-L	0 / 2531	-18.2	-18.2	0.45 (1)	10.00			
L-K	0 / 0	-18.2	-18.2	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03") CALCULATED VERT. DEFL.(LL) = L/999 (0.18") ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.45/1.00 (D-E-1), BC

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB NO.

IM0722-056

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 03 2023

PER:
CHIEF BUILDING OFFICIAL

T02

2

1

TRUSS DESC.

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:19 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-WKGE2lyx?IVCk7rj_PxBLZhdpPr5W1PGKMJwRDyy8tY
Scale = 1:53.5

MHP 23019

LUMBER

N L G A RULES CHORDS SIZE LUMBER DESCR.

A - D 2x4 DRY No.2 SPF

D - F 2x4 DRY No.2 SPF

F - I 2x4 DRY No.2 SPF

P - B 2x4 DRY No.2 SPF

J - H 2x4 DRY No.2 SPF

P - M 2x4 DRY No.2 SPF

M - J 2x4 DRY No.2 SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMVV-t MT20 4.0 8.0 1.50 3.00

C TMMWW-t MT20 3.0 4.0 1.50 1.75

D TTWW-m MT20 5.0 6.0 2.50 2.00

E TMMW-w MT20 2.0 4.0

F TTWW-m MT20 5.0 6.0 2.50 2.00

G TMMWW-t MT20 3.0 4.0 1.50 1.75

H TMVV-t MT20 4.0 8.0 1.50 3.00

J BMV1+p MT20 3.0 4.0

K BMWW-t MT20 4.0 6.0 1.75 1.75

L BMWW-t MT20 3.0 4.0

M BSWWW-I MT20 5.0 6.0 3.00 3.00

N BMWW-t MT20 3.0 4.0

O BMWW-t MT20 4.0 6.0 1.75 1.75

P BMV1+p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION DOWN HORZ BRG IN-SX BRG IN-SX

JT 2169 0 2169 0 0 5-8 3-9

J 2169 0 2169 0 0 5-8 3-9

UNFACTORED REACTIONS

JT 1ST LOASE MAX/MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

J 1500 1173 / 0 0 / 0 0 / 0 0 / 0 327 / 0 0 / 0

J 1500 1173 / 0 0 / 0 0 / 0 0 / 0 327 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX FROM TO CSI (LC) UNBRAC LENGTH FR-TO

A-B 0 / 34 -111.9 -111.9 0.15 (1) 10.00

B-C -2900 / 0 -111.9 -111.9 0.47 (1) 3.67

C-D -2640 / 0 -111.9 -111.9 0.44 (1) 3.85

D-E -2729 / 0 -111.9 -111.9 0.54 (1) 3.66

E-F -2729 / 0 -111.9 -111.9 0.54 (1) 3.66

F-G -2640 / 0 -111.9 -111.9 0.44 (1) 3.85

G-H -2900 / 0 -111.9 -111.9 0.47 (1) 3.67

H-I 0 / 34 -111.9 -111.9 0.15 (1) 10.00

P-B -2125 / 0 0.0 0.0 0.21 (1) 5.81

J-H -2125 / 0 0.0 0.0 0.21 (1) 5.81

WEBS

MEMB. MAX. FACTORED FORCE (LBS) MAX CSI (LC) FR-TO

O-C -353 / 12 0.08 (1)

C-N -346 / 0 0.22 (1)

N-D 0 / 307 0.07 (1)

D-M 0 / 571 0.13 (1)

M-E -753 / 0 0.45 (1)

M-F 0 / 571 0.13 (1)

L-F 0 / 307 0.07 (1)

L-G -346 / 0 0.22 (1)

K-G -353 / 12 0.08 (1)

B-O 0 / 2656 0.60 (1)

K-H 0 / 2656 0.60 (1)

P-O 0 / 0 -18.2 -18.2 0.10 (4) 10.00

O-N 0 / 2619 -18.2 -18.2 0.49 (1) 10.00

N-M 0 / 2339 -18.2 -18.2 0.45 (1) 10.00

M-L 0 / 2339 -18.2 -18.2 0.45 (1) 10.00

L-K 0 / 2619 -18.2 -18.2 0.49 (1) 10.00

K-J 0 / 0 -18.2 -18.2 0.10 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF DL = 3.0 PSF

BOT CH. LL = 0.0 PSF DL = 7.3 PSF

TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03") CALCULATED VERT. DEFL.(LL) = L/999 (0.16") ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.54/1.00 (D-E-1), BC=0.49/1.00 (K-L-1), WB=0.60/1.00 (H-K-1), SSI=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90) JSI METAL= 0.62 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER

I.MATUEVIC

100528832

PROVINCE OF ONTARIO

July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

[illegible]

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: CHIEF BUILDING OFFICIAL	9	1	TRUSS DESC.	
Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:21 2022 Page 1 ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-SjN_T__BXUlwzQ?65qz3Q_mwUCv8_xoZogC1W5yy8tW Scale = 170.5					
TOTAL WEIGHT = 9 X 121 = 1085 lb [M][F]					
LUMBER	N L G A RULES	CHORDS SIZE	LUMBER	DESCR.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
A - C	2x4	DRY	2100F 1.8E	SPF	BEARINGS
C - E	2x4	DRY	2100F 1.8E	SPF	FACTORED GROSS REACTION
E - G	2x4	DRY	2100F 1.8E	SPF	DOWN UP
G - I	2x4	DRY	2100F 1.8E	SPF	BRG IN-SX
N - B	2x4	DRY	No.2	SPF	REACT
J - H	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS
DRY: SEASONED LUMBER.					1ST CASE MAX/MIN COMPONENT REACTIONS
					JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
					N 1500 1173 / 0 0 / 0 0 / 0 327 / 0 0 / 0
					J 1500 1173 / 0 0 / 0 0 / 0 327 / 0 0 / 0
					BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J
					BRACING
					TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
					MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
					ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
					1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-L, D-L DBS = 20-0-0 : CBF = 125 LBS.
					DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.
					END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW
					LOADING
					TOTAL LOAD CASES: (4)
					CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) LC1 MAX. UNBRACED LENGTH FR-TO
					A-B 0 / 34 -111.9 -111.9 0.09 (1) 10.00
					B-C -2933 / 0 -111.9 -111.9 0.72 (1) 4.10
					C-D -2933 / 0 -111.9 -111.9 0.72 (1) 4.10
					D-E -2078 / 0 -111.9 -111.9 0.64 (1) 4.79
					E-F -2078 / 0 -111.9 -111.9 0.64 (1) 4.79
					F-G -2933 / 0 -111.9 -111.9 0.72 (1) 4.10
					G-H -2933 / 0 -111.9 -111.9 0.72 (1) 4.10
					H-I 0 / 34 -111.9 -111.9 0.09 (1) 10.00
					N-B -2111 / 0 0.0 0.0 0.21 (1) 5.83
					J-H -2111 / 0 0.0 0.0 0.21 (1) 5.83
					WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH FR-TO
					L-E 0 / 1172 0.26 (1) 4.10
					K-F -121 / 132 0.05 (1) 4.10
					D-L -1001 / 0 0.57 (1) 4.79
					M-D -121 / 132 0.05 (1) 4.79
					B-M 0 / 2680 0.60 (1) 4.10
					K-H 0 / 2680 0.60 (1) 4.10
					N-M 0 / 0 -18.2 -18.2 0.31 (4) 10.00
					M-L 0 / 2664 -18.2 -18.2 0.58 (1) 10.00

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

DRWG NO.

IM0722-056

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 03 2023

PER: CHIEF BUILDING OFFICIAL

1

1

TRUSS DESC.

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-vvxMgk?pHotnbaalfYUizBI6Dcu2jLyI0Kxb2Yyy8tV

Scale = 1:57.7

TOTAL WEIGHT = 111 lb [M][F]

LUMBER

N L G A	RULES	CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTWW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	4.0	5.0	1.50	2.25
H	BMV1+t	MT20	4.0	5.0	Edge 0.50	
I	BMVW-t	MT20	4.0	4.0	1.50	1.50
J	BMVWWW-t	MT20	3.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	3.0	5.0		
M	BMVW-t	MT20	4.0	5.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1819	0	0	5-8
H	1667	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1258	986 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
H	1155	892 / 0	0 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	
A-B	0 / 34	-111.9	10.00	M-C	-170 / 79	0.05 (1)	
B-C	-2332 / 0	-111.9	3.75	C-L	-707 / 0	0.74 (1)	
C-D	-1743 / 0	-111.9	4.32	L-D	0 / 489	0.11 (1)	
D-E	-1332 / 0	-111.9	5.10	D-J	-338 / 0	0.19 (1)	
E-F	-1512 / 0	-111.9	5.08	J-E	0 / 207	0.05 (1)	
F-G	-1330 / 0	-111.9	5.34	J-F	0 / 187	0.04 (1)	
N-B	-1770 / 0	0.0	6.27	I-F	-769 / 0	0.33 (1)	
H-G	-1635 / 0	0.0	6.48	B-M	0 / 2138	0.48 (1)	
N-M	0 / 0	-18.2	10.00	I-G	0 / 1478	0.33 (1)	
M-L	0 / 2118	-18.2	10.00				
L-K	0 / 1532	-18.2	10.00				
K-J	0 / 1532	-18.2	10.00				
J-I	0 / 1211	-18.2	10.00				
I-H	0 / 0	-18.2	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.66/1.00 (B-C-1) , BC=0.41/1.00 (L-M-1) , WB=0.74/1.00 (C-L-1) , SS=0.30/1.00 (B-C-1)

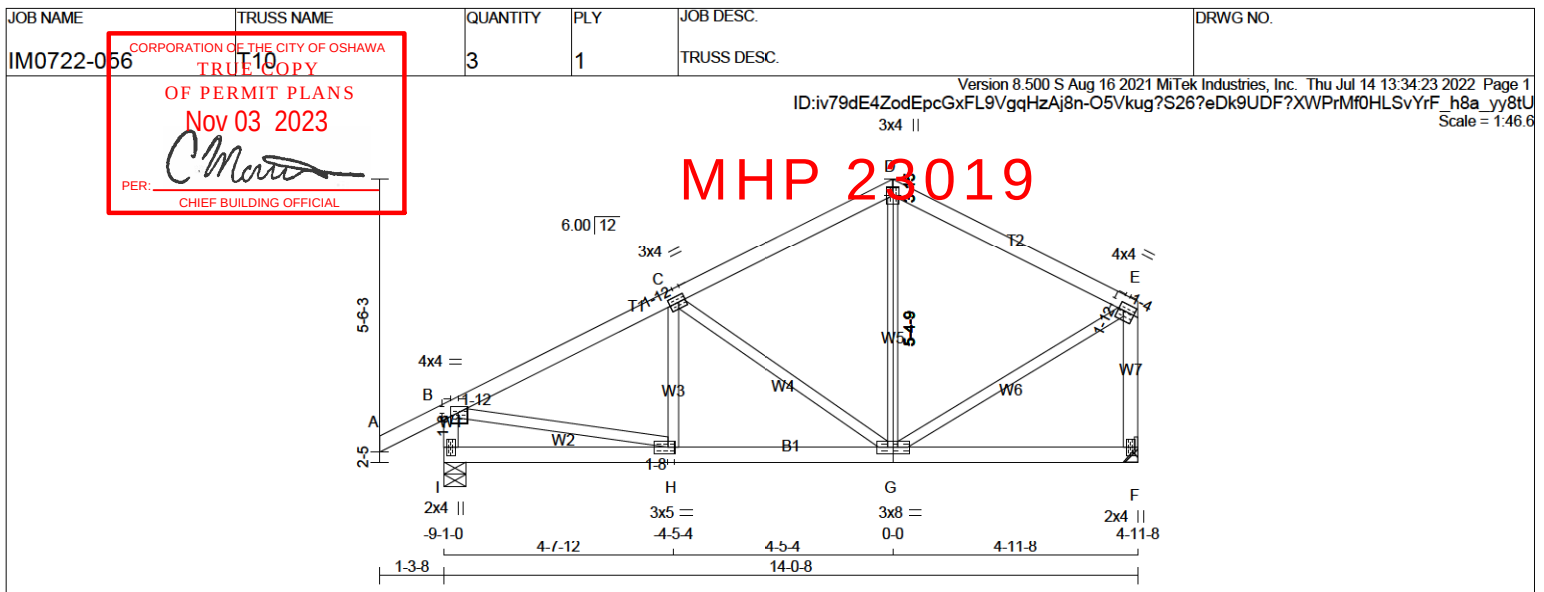
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747



TOTAL WEIGHT = 3 X 58 = 175 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.50	1.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	4.0	1.75	1.25
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	3.0	8.0		
H	BMVWW-t	MT20	3.0	5.0	1.50	1.50
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1065	0	1065	0
F	914	0	914	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
I	736	583 / 0	0 / 0
F	633	489 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 34	-111.9 -111.9	0.15 (1)	10.00	H-C	-99 / 59	0.02 (4)
B-C	-1076 / 0	-111.9 -111.9	0.30 (1)	5.71	C-G	-536 / 0	0.27 (1)
C-D	-643 / 0	-111.9 -111.9	0.29 (1)	6.25	G-D	0 / 94	0.03 (4)
D-E	-615 / 0	-111.9 -111.9	0.36 (1)	6.25	B-H	0 / 1002	0.23 (1)
I-B	-1027 / 0	0.0 0.0	0.10 (1)	7.74	G-E	0 / 639	0.14 (1)
F-E	-877 / 0	0.0 0.0	0.16 (1)	7.81			
I-H	0 / 0	-18.2 -18.2	0.08 (4)	10.00			
H-G	0 / 986	-18.2 -18.2	0.21 (1)	10.00			
G-F	0 / 0	-18.2 -18.2	0.10 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	45.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.36/1.00 (D-E-1), BC=0.21/1.00 (G-H-1), WB=0.27/1.00 (C-G-1), SSI=0.22/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)

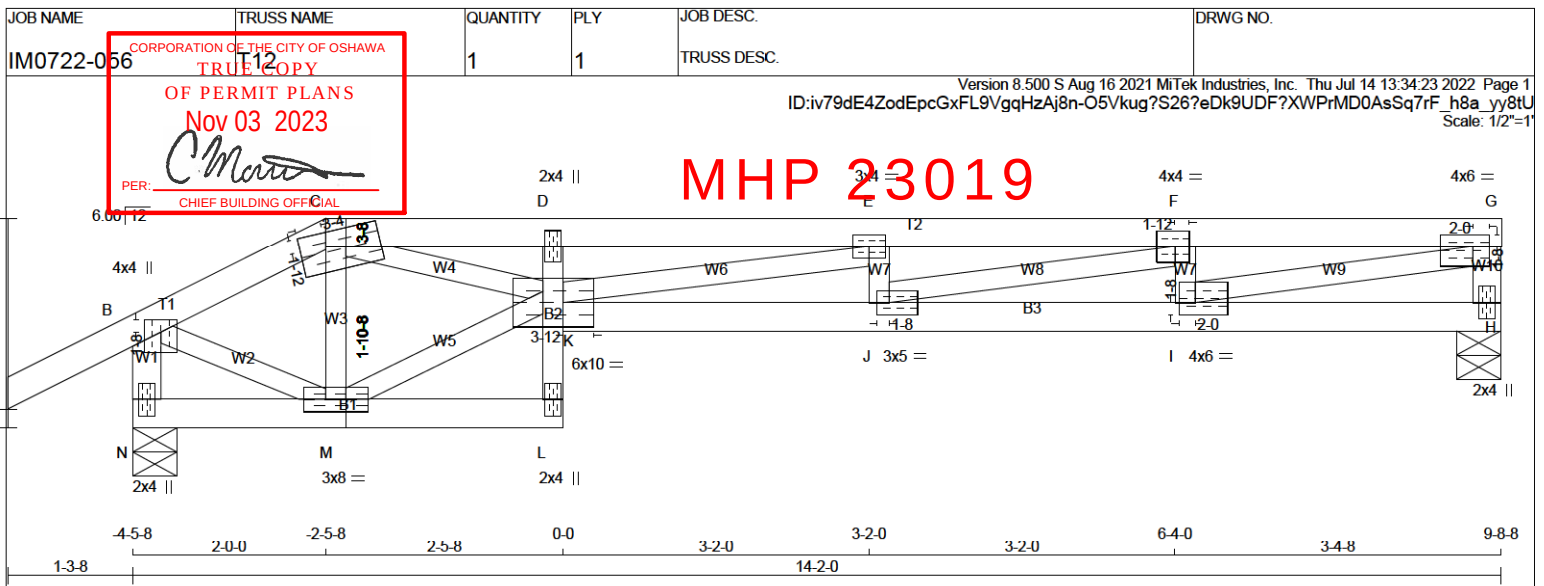
JSI METAL= 0.35 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER****N. L. G. A. RULES**

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - G	2x4	DRY	No.2 SPF
H - G	2x4	DRY	No.2 SPF
N - B	2x4	DRY	No.2 SPF
N - L	2x4	DRY	No.2 SPF
L - D	2x3	DRY	No.2 SPF
K - H	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	10.0	1.75	3.25
D	TMV+p	MT20	2.0	4.0		
E	TMWW-t	MT20	3.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-t	MT20	4.0	6.0	1.50	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-t	MT20	4.0	6.0	1.50	2.00
J	BMVW-t	MT20	3.0	5.0	1.50	1.50
K	BVMWWW-t	MT20	6.0	10.0	3.00	3.75
L	BMV+p	MT20	2.0	4.0		
M	BMVWW-t	MT20	3.0	8.0		
N	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY**BUILDING DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	922	0	922	0
N	1074	0	1074	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	639	493 / 0	0 / 0
N	741	587 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 34	-111.9	-111.9	0.15 (1)	10.00	M-C	-637 / 0
B-C	-979 / 0	-111.9	-111.9	0.09 (1)	6.20	M-K	0 / 817
C-D	-3087 / 0	-111.9	-111.9	0.20 (1)	3.83	C-K	0 / 2396
D-E	-3193 / 0	-111.9	-111.9	0.32 (1)	3.64	K-E	-456 / 0
E-F	-3634 / 0	-111.9	-111.9	0.30 (1)	3.45	J-E	-247 / 3
F-G	-2639 / 0	-111.9	-111.9	0.26 (1)	4.02	J-F	0 / 1033
H-G	-876 / 0	0.0	0.0	0.09 (1)	7.81	I-F	-655 / 0
N-B	-1061 / 0	0.0	0.0	0.11 (1)	7.64	I-G	0 / 2731
						B-M	0 / 944
N-M	0 / 0	-18.2	-18.2	0.03 (4)	10.00		
M-L	0 / 92	-18.2	-18.2	0.03 (1)	10.00		
L-K	0 / 18	0.0	0.0	0.25 (1)	10.00		
K-D	-313 / 0	0.0	0.0	0.25 (1)	7.81		
K-J	0 / 3634	-18.2	-18.2	0.69 (1)	10.00		
J-I	0 / 2639	-18.2	-18.2	0.55 (1)	10.00		
I-H	0 / 0	-18.2	-18.2	0.08 (1)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (LL) = $L/818$ (0.21")
ALLOWABLE DEFL. (TL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (TL) = $L/502$ (0.34")

CSI: TC=0.32/1.00 (D-E-1), BC=0.69/1.00 (J-K-1), WB=0.61/1.00 (G-I-1), SSI=0.15/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

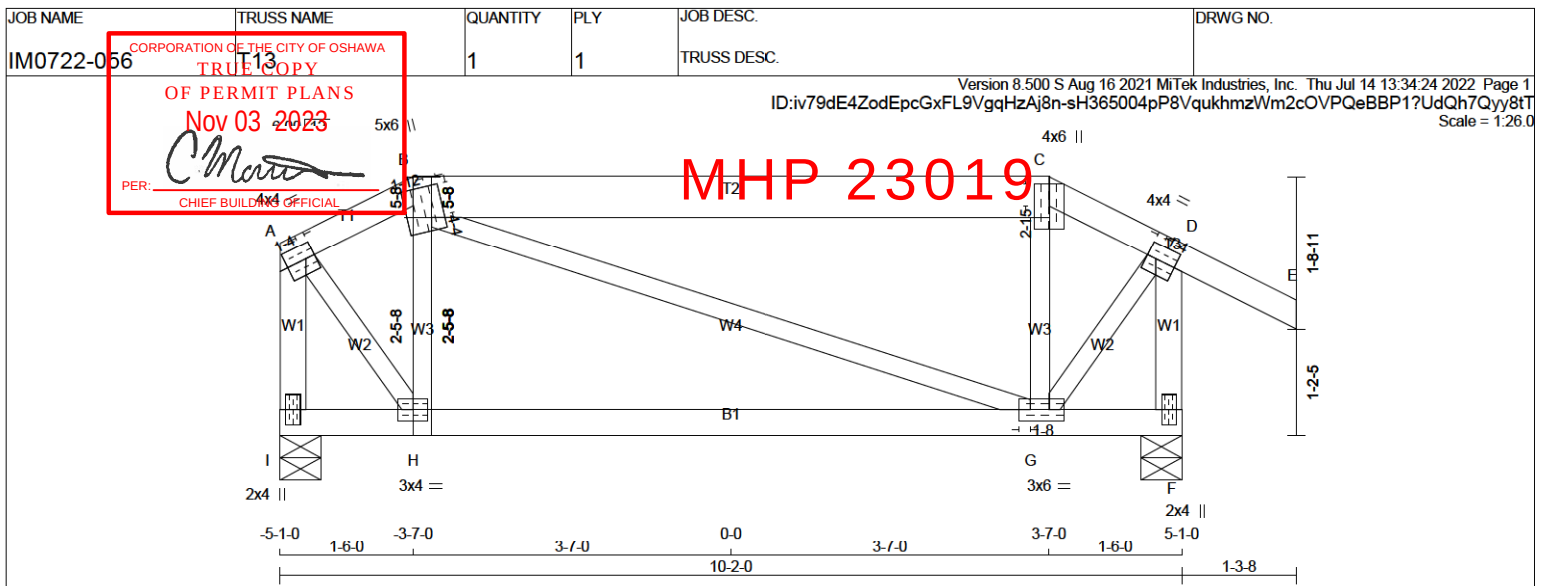
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.63 (I) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 47 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - B 2x4 DRY No.2

B - C 2x6 DRY No.2

C - E 2x4 DRY No.2

I - A 2x4 DRY No.2

F - D 2x4 DRY No.2

I - F 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

FACTORED MAXIMUM FACTORED INPUT REQD

GROSS REACTION GROSS REACTION BRG BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

I 651 0 651 0 0 5-8 1-8

F 823 0 823 0 0 5-8 1-8

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

A TMVW-t MT20 4.0 4.0 2.00 1.25

B TTWW+m MT20 5.0 6.0 4.25 1.75

C TTW+p MT20 4.0 6.0 3.00 2.00

D TMVW-t MT20 4.0 4.0 2.00 1.25

F BMV1+p MT20 2.0 4.0

G BMVWW-t MT20 3.0 6.0 1.50 1.50

H BMVW-t MT20 3.0 4.0

I BMV1+p MT20 2.0 4.0



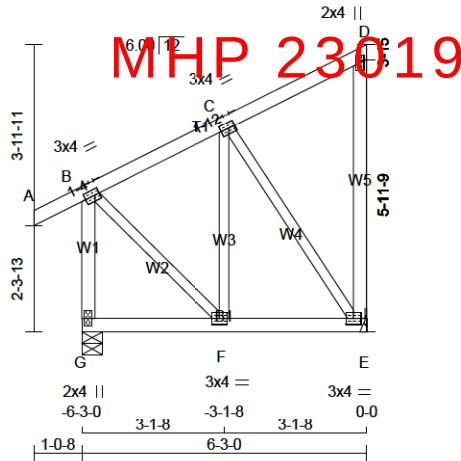
July 14, 2022

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ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	T-14 CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:25 2022 Page 1
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Scale = 1:50.5



TOTAL WEIGHT = 37 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	3.0	4.0	1.50	1.25
C	TMW-t	MT20	3.0	4.0	1.50	1.75
D	TMV+p	MT20	2.0	4.0		
E	BMWV1-t	MT20	3.0	4.0		
F	BMWV1-t	MT20	3.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT		DOWN	IN-SX	IN-SX
G	530	0	5-8	1-8
E	407	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
G	365	294 / 0	0 / 0	0 / 0	71 / 0	0 / 0	
E	282	217 / 0	0 / 0	0 / 0	64 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
G-B	-507 / 0	0.0	0.0 0.09 (1)	7.81	B-F	0 / 240	0.05 (1)
A-B	0 / 28	-111.9	-111.9 0.10 (1)	10.00	F-C	-95 / 43	0.03 (1)
B-C	-179 / 0	-111.9	-111.9 0.14 (1)	6.25	C-E	-307 / 0	0.14 (1)
C-D	-19 / 0	-111.9	-111.9 0.14 (1)	6.25			
E-D	-132 / 0	0.0	0.0 0.10 (1)	7.81			
G-F	0 / 0	-18.2	-18.2 0.05 (4)	10.00			
F-E	0 / 177	-18.2	-18.2 0.06 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL =	34.8	PSF
DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.3	PSF	
TOTAL LOAD =	45.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.21")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.21")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.14/1.00 (B-C:1), BC=0.06/1.00 (E-F:4), WB=0.14/1.00 (C-E:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



July 14, 2022

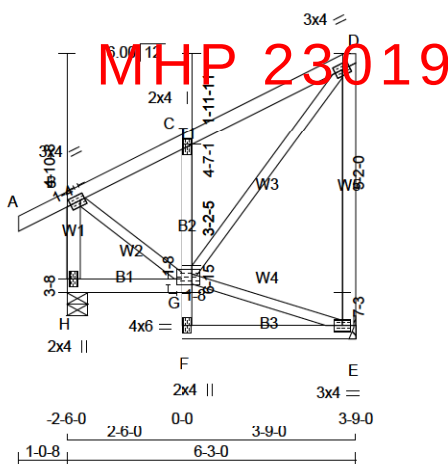
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS BODY	2	1	TRUSS DESC.	

CORPORATION OF THE CITY OF OSHAWA
56
715
TRUE COPY
OF PERMIT PLANS
Nov 03 2023
C. Morris
PER: _____
CHIEF BUILDING OFFICIAL

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-KUDvIL1iajGMS2JtKg2?bqwiDq?1wsr8jHAFfsyy8tS
Scale = 1.49



TOTAL WEIGHT = $2 \times 38 = 75 \text{ lb}$

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	4.0	1.50	1.25
C	TMV+p	MT20	2.0	4.0		
D	TMVW-t	MT20	3.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMV+p	MT20	2.0	4.0		
G	BVMMWW-I	MT20	4.0	6.0	1.50	1.50
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	407	0	407	0	0	MECHANICAL	
H	530	0	530	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	282	217/0	0/0	0/0	0/0	64/0	0/0
H	365	294/0	0/0	0/0	0/0	71/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 28	-111.9 -111.9	0.10 (1)	10.00	G-E	-6 / 0	0.00 (1)		
B-C	-248 / 0	-111.9 -111.9	0.15 (1)	6.25	G-D	0 / 394	0.09 (1)		
C-D	-276 / 0	-111.9 -111.9	0.16 (1)	6.25	B-G	0 / 299	0.07 (1)		
E-D	-372 / 0	0.0 0.0	0.29 (1)	7.81					
H-B	-507 / 0	0.0 0.0	0.06 (1)	7.81					
H-G	0 / 0	-18.2 -18.2	0.04 (4)	10.00					
F-G	0 / 37	0.0 0.0	0.02 (1)	10.00					
G-C	-436 / 0	0.0 0.0	0.03 (1)	7.81					
F-E	0 / 6	-18.2 -18.2	0.07 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	34.8	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	45.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.29/1.00 (D-E:1), BC=0.07/1.00 (E-F:4), WB=0.09/1.00 (D-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)



July 14, 2022

**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**

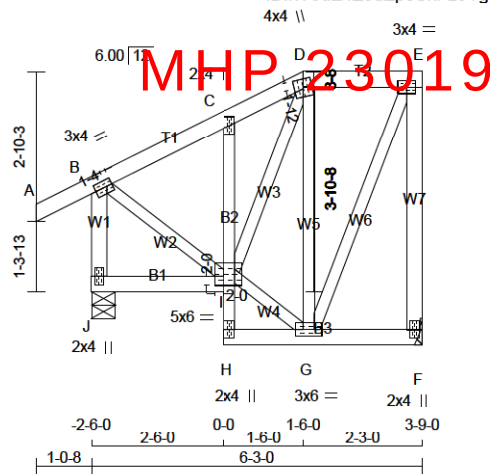


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUE COPY T16 OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-pgBtWh2KL1OD4Bu3uNZE71TvhDLpfJdlxxvoBJyy8tR

Scale = 1:43.5



TOTAL WEIGHT = 41 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
H - C	2x3	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.25
C	TMV+p	MT20	2.0	4.0		
D	TTWW+m	MT20	4.0	4.0	1.75	2.00
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	3.0	6.0		
H	BMV+p	MT20	2.0	4.0		
I	BMVWW-t	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	407	0	407	0
J	530	0	530	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	282	217 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
J	365	294 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-111.9	-111.9	0.10 (1)	I-G	0 / 140	0.03 (1)
B-C	-232 / 0	-111.9	-111.9	0.08 (1)	I-D	0 / 249	0.06 (1)
C-D	-249 / 0	-111.9	-111.9	0.05 (1)	G-D	-301 / 0	0.12 (1)
D-E	-117 / 0	-111.9	-111.9	0.07 (1)	G-E	0 / 290	0.07 (1)
F-E	-390 / 0	0.0	0.0	0.18 (1)	B-I	0 / 264	0.06 (1)
J-B	-507 / 0	0.0	0.0	0.06 (1)			
J-I	0 / 0	-18.2	-18.2	0.04 (4)			
H-I	0 / 10	0.0	0.0	0.02 (1)			
I-C	-277 / 0	0.0	0.0	0.02 (1)			
H-G	0 / 6	-18.2	-18.2	0.02 (4)			
G-F	0 / 0	-18.2	-18.2	0.02 (4)			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.21")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.21")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (E-F:1), BC=0.04/1.00 (I-J:4), WB=0.12/1.00 (D-G:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

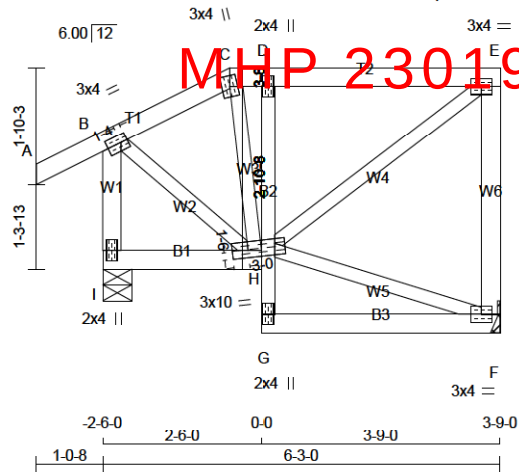
July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:26 2022 Page 1
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Scale = 1:36.3



TOTAL WEIGHT = 35 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
G - D	2x3	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.25
C	TTW+m	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMV+p	MT20	2.0	4.0		
H	BMVWWWW*	MT20	3.0	10.0	1.50	3.00
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	407	0	407	0
I	530	0	530	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. / MIN.	COMPONENT REACTIONS				
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	282	217 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
I	365	294 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-111.9	-111.9	0.10 (1)	C-H	0 / 180	0.04 (1)
B-C	-249 / 0	-111.9	-111.9	0.08 (1)	H-F	-3 / 0	0.00 (1)
C-D	-257 / 0	-111.9	-111.9	0.18 (1)	H-E	0 / 329	0.07 (1)
D-E	-258 / 0	-111.9	-111.9	0.18 (1)	B-H	0 / 273	0.06 (1)
F-E	-372 / 0	0.0	0.0	0.10 (1)			
I-B	-507 / 0	0.0	0.0	0.06 (1)			
I-H	0 / 0	-18.2	-18.2	0.04 (4)			
G-H	0 / 37	0.0	0.0	0.02 (1)			
H-D	-482 / 0	0.0	0.0	0.02 (1)			
G-F	0 / 3	-18.2	-18.2	0.07 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.21")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.21")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.18/1.00 (D-E:1), BC=0.07/1.00 (F-G:4), WB=0.07/1.00 (E-H:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.20 (H) (INPUT = 1.00)



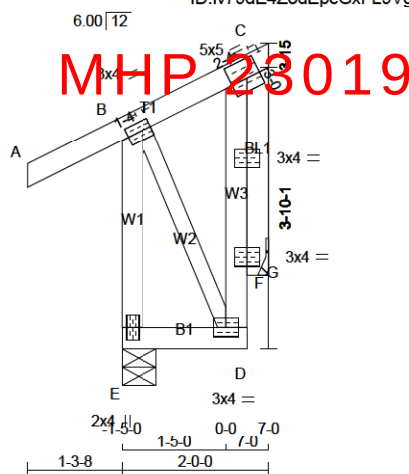
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-056	CORPORATION OF THE CITY OF OSHAWA TRUSS ADV	1	1	TRUSS DESC.	

CORPORATION OF THE CITY OF OSHAWA
56
718
TRUE COPY
OF PERMIT PLANS
Nov 03 2023
C. Morris
PER: _____
CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 13:34:27 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-HskFj13y6KW3hLSFS54TgF04xdhNOnhRABfJlJyy8tQ
Scale = 1:31.5



TOTAL WEIGHT = 19 lb

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY- SEASONED LUMBER				

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)		W	LEN	Y	X
JT	TYPE	PLATES			
B	TMVW-t	MT20	3.0	4.0	1.50 1.25
C	TMVK1-t	MT20	5.0	5.0	3.00 2.25
D	BMVW-t	MT20	3.0	4.0	
E	BMV1+p	MT20	2.0	4.0	
F	KP-p	MT20	3.0	4.0	
F	KP-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G(C)	45	0	45	0	-39	MECHANICAL	
(** SEE "BEARING NOTE" **)							
E	329	0	329	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G(C). MINIMUM BEARING LENGTH AT JOINT G(C) = 1-12.

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G(C)	32	19 / -33	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
E	224	194 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/34	-111.9 -111.9	0.15 (1)	10.00	B-D	-5/5	0.00 (1)
B-C	-30/0	-111.9 -111.9	0.14 (1)	6.25	C-G	-45/40	0.01 (1)
D-F	0/16	0.0 0.0	0.01 (4)	10.00	F-G	-3/3	0.00 (1)
F-C	0/16	0.0 0.0	0.01 (4)	10.00			
E-B	-313/0	0.0 0.0	0.05 (1)	7.81			
E-D	0/0	-18.2 -18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.



July 14, 2022

**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP	CH.	LL =	34.8 PSF
		DL =	3.0 PSF
BOT	CH.	LL =	0.0 PSF
		DL =	7.3 PSF
TOTAL LOAD		=	45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.01/1.00 (C-G:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)





STANDARD DETAIL MSD2015-H

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

MHP 23019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

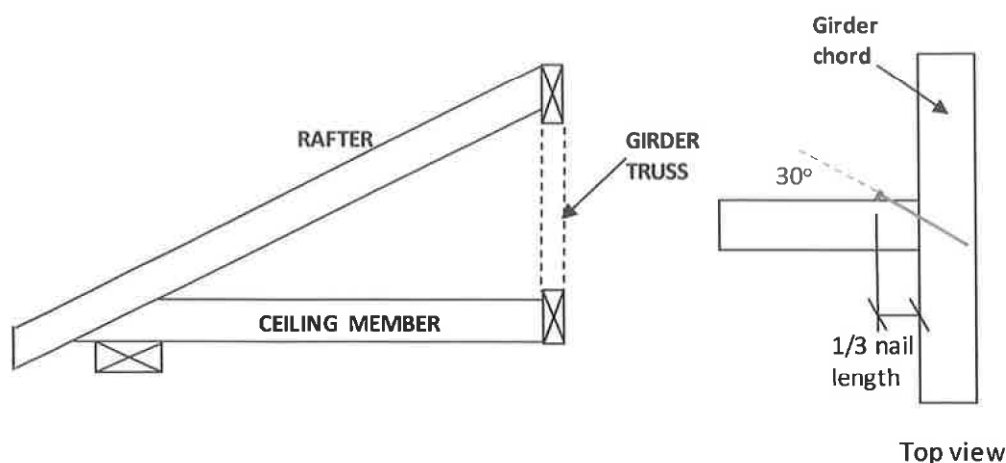


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss





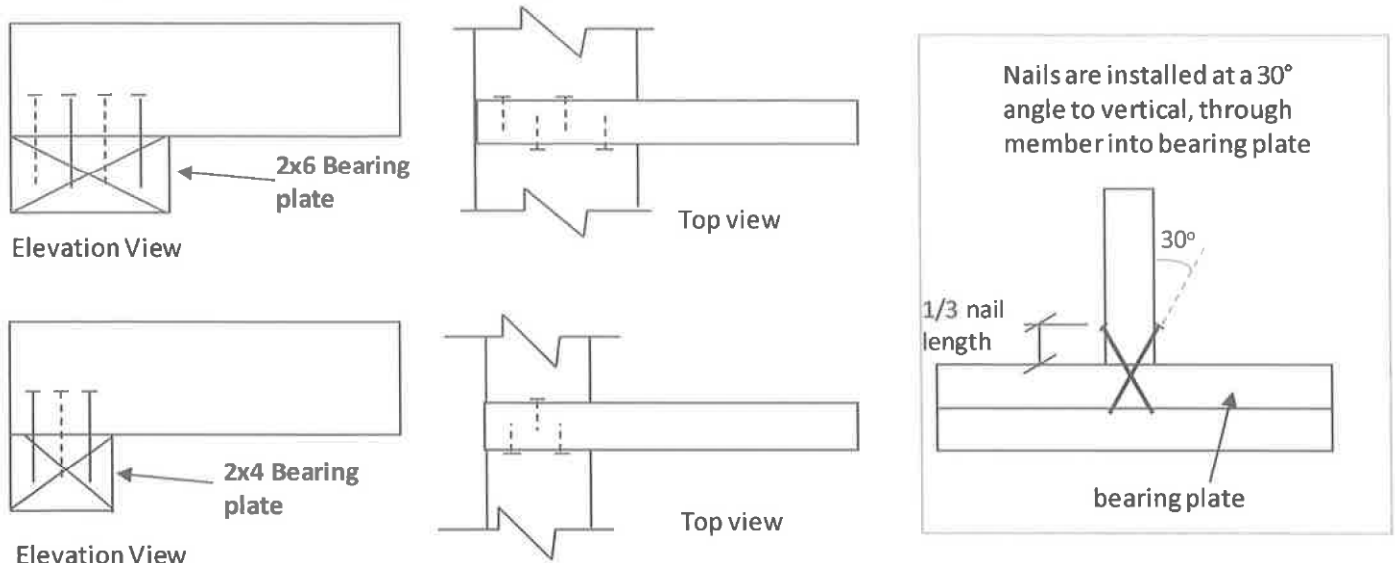
STANDARD DETAIL MSD2015-H

Issued: MARCH 1, 2022
Expiry: APRIL 30, 2024

MHP 23019

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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STANDARD DETAIL MSD2015-J

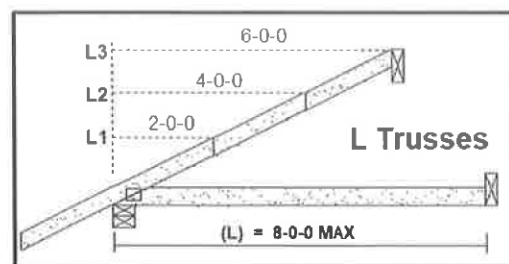
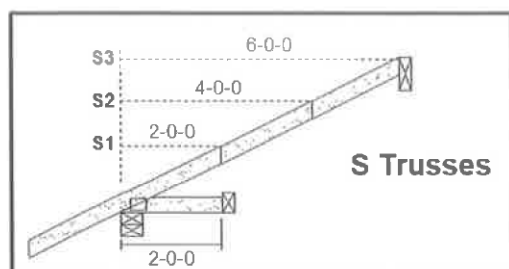
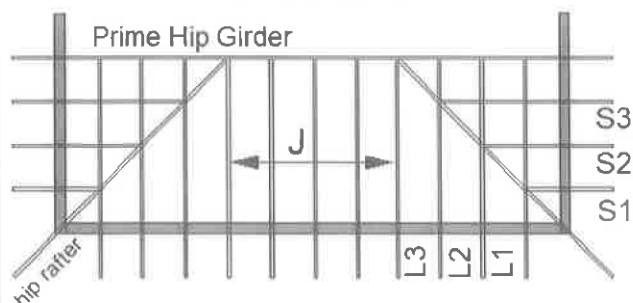
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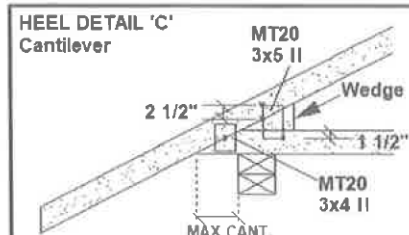
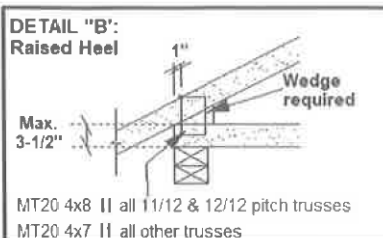
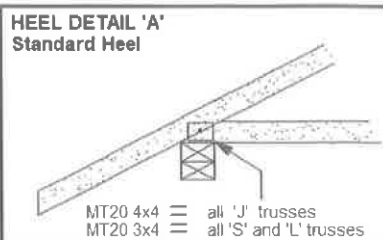
STANDARD HIP END FRAMING

PLAN VIEW



Specified Load Rating:

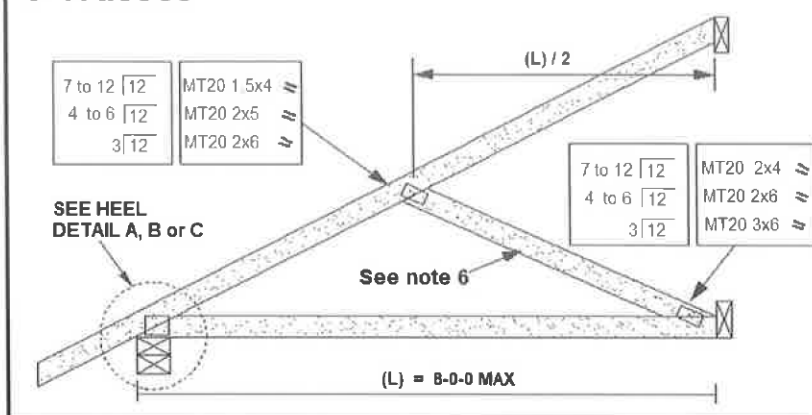
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL "C"

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 X 5	2 X 3
4/12	14"	3 X 5	2 X 3
5/12	12"	3 X 5	2 X 4
6/12	10"	3 X 5	2 X 4
7/12	9"	3 X 5	2 X 6
8/12	8 5/8"	3 X 5	2 X 6
9/12	8"	3 X 5	2 X 6
10/12	7 5/8"	3 X 5	2 X 6

J Trusses



PEO

Certificate No. 10889485

NOTES:

1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
2. Overhang length shall not exceed 24 inches.
3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
6. For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'

