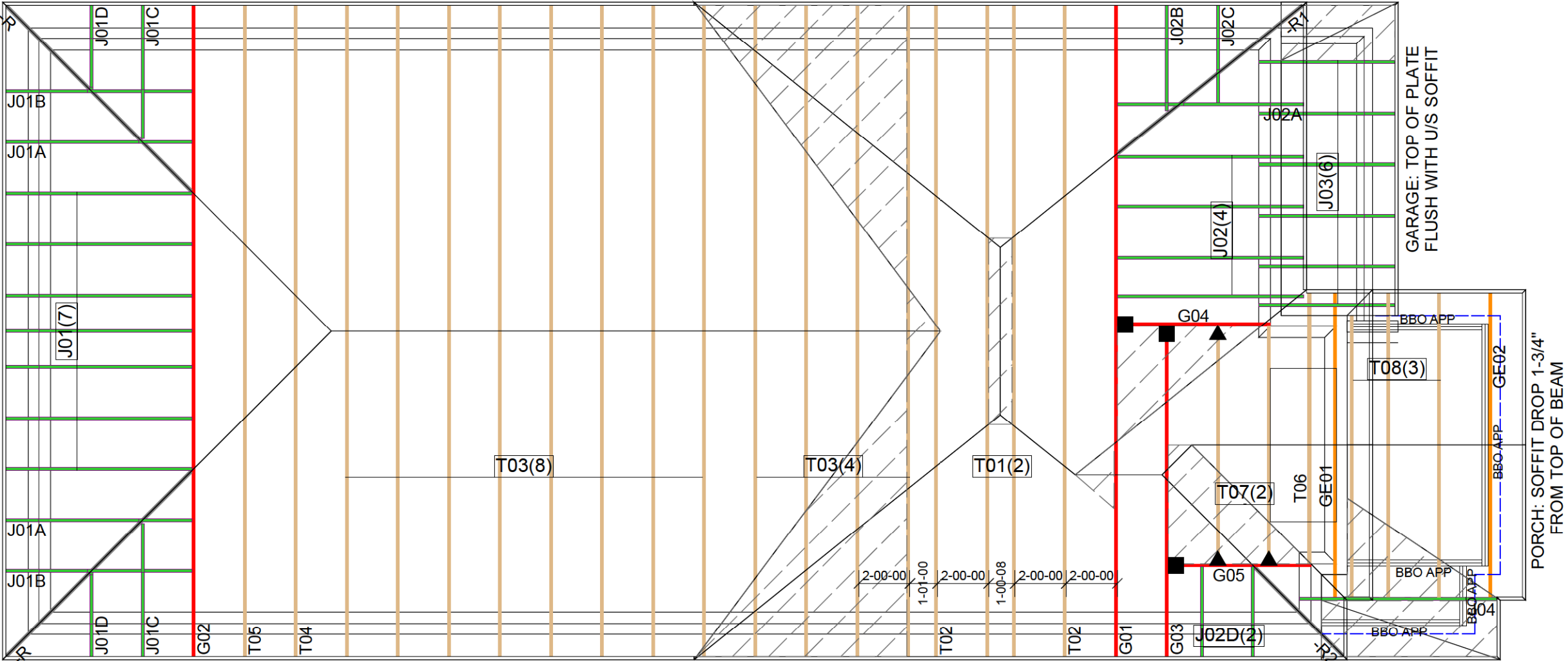




MHP 23021



UPPER LEVEL: TOP OF PLATE FLUSH WITH U/S SOFFIT

Hanger Name	Symbol	QTY
LUS24	▲	3
LJS26DS	■	3



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK GROUP
Job #	23-00070R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC OSHAWA,ON
Model	RIVER 1-1
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-04-20
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 1\RIVER 1-1\T-RIVE

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	6.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

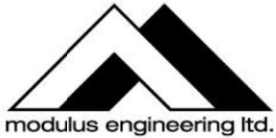
Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



**Read Carefully Prior to
Manufacture and Installation**

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)

2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.

3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses

4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all interested parties (e.g. Building designer where required)

5. Cut members to bear tightly against one another

6. Place plates on each face of truss at each joint and use proper roller or hydraulic press. Knots and wane at locations are regulated by TPIC Appendix G

7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC

8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture

9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment

10. Connections not shown are the responsibility of others

11. Do not cut or alter truss members or plates without prior approval of an engineer

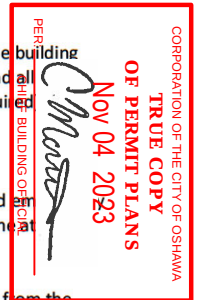
12. Install and load vertically unless otherwise noted

13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient

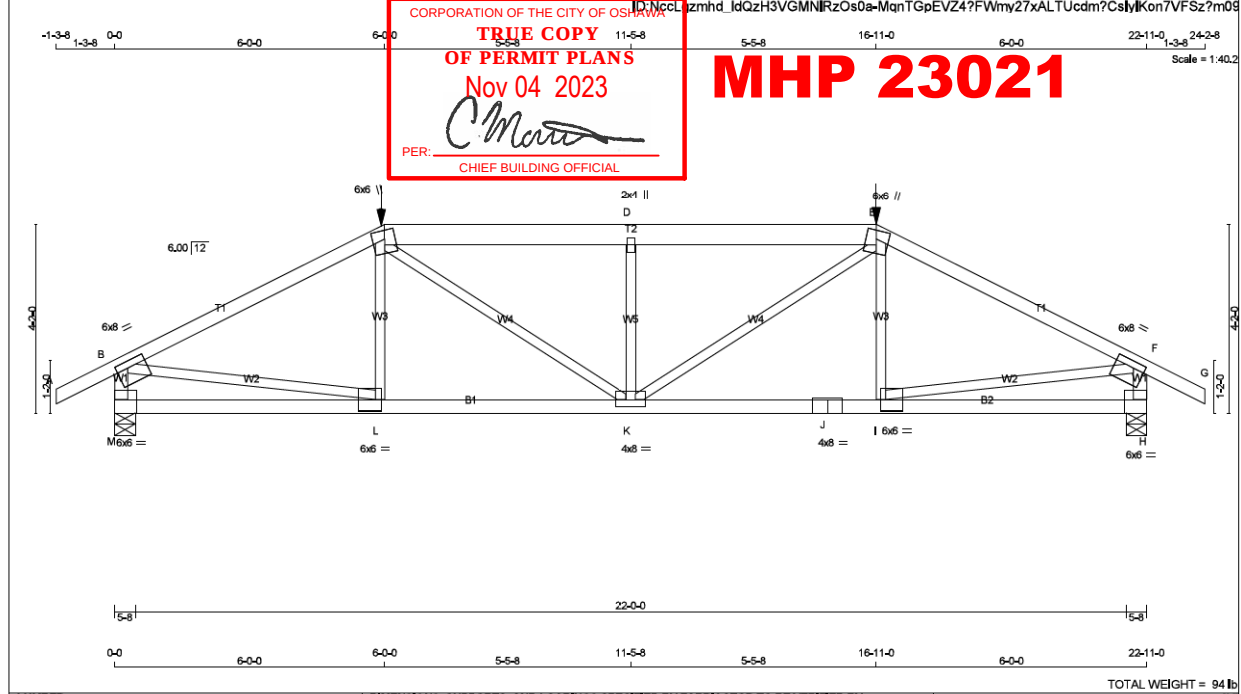
14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G

16. Building designer must review individual component drawings to ensure they are suitable for the structure

15. Not designed for solar panels unless specifically noted



MHP 23021



LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA				
N, L, G, A, RULES					CHORDS					BEARINGS					SPECIFIED LOADS:				
			SIZE	LUMBER	DESCR.														
A - C	-	2x4	DRY	2100F 1.8E	SPF														
C - E	-	2x6	DRY	No.2	SPF														
E - G	-	2x4	DRY	2100F 1.8E	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX						
M - B	-	2x4	DRY	No.2	SPF	M	3039	0	3039	0	0	5-8	5-8						
H - F	-	2x4	DRY	No.2	SPF	H	3039	0	3039	0	0	5-8	5-8						
M - J	-	2x4	DRY	No.2	SPF														
J - H	-	2x4	DRY	No.2	SPF														
ALL WEBS EXCEPT					No.2	SPF	UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C		
							1ST LCASE MAX./MIN. COMPONENT REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM		
							JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL					
							M	2123	1542 / 0	0 / 0	0 / 0	0 / 0	581 / 0	0 / 0					
							H	2123	1542 / 0	0 / 0	0 / 0	0 / 0	581 / 0	0 / 0					
DRY: SEASONED LUMBER,																			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) M, H (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

D TWW+M				MT20	2.0	4.0	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED,										- ADD'L LOADS based on 55 % of GSL.										
E TWW+M				MT20	6.0	6.0	2.25	2.75	LOADING										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015								
F TWW+M				MT20	6.0	6.0	1.75	3.00	TOTAL LOAD CASES: (4)										THIS DESIGN COMPLIES WITH:-								
H BMV+L				MT20	6.0	6.0	Edge	2.50											- PART 9 OF BCBC 2018 - NBC-2019AE								
I BMW+L				MT20	6.0	6.0	3.00	1.50											- PART 9 OF OBC 2012 (2019 AMENDMENT)								
J BS-L				MT20	4.0	8.0											- CSA 086-14										
K BMW+M+L				MT20	4.0	8.0	1.75	4.00											- TPIC 2014								
L BMW+L				MT20	6.0	6.0	3.00	1.50											(55 % of 48.1 P.S.F, G.S.L, PLUS 8.4 P.S.F, RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD								
M BMV+L				MT20	6.0	6.0	3.50											ALLOWABLE DEF(L)= L/360 (0.78")									
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																								CALCULATED VERT. DEF(L)= L/ 999 (0.17")			
																								ALLOWABLE DEF(TL)= L/360 (0.78")			
																								CALCULATED VERT. DEF(TL)= L/ 937 (0.29")			
																								CSI TO=0.84/1.00 (B-C) , BC=0.82/1.00 (K-L) , WB=0.95/1.00 (B-L) , SS=0.49/1.00 (C-D) ,			

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS			
JT	LOC.	LC1	MAX- MAX+	FACE	DIR.	TYPE	HEEL CONN.
C	6-0-0	-537	-537	FRONT	VERT	TOTAL	C1
E	16-11-0	-537	-537	FRONT	VERT	TOTAL	C1

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PSI) (PSI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

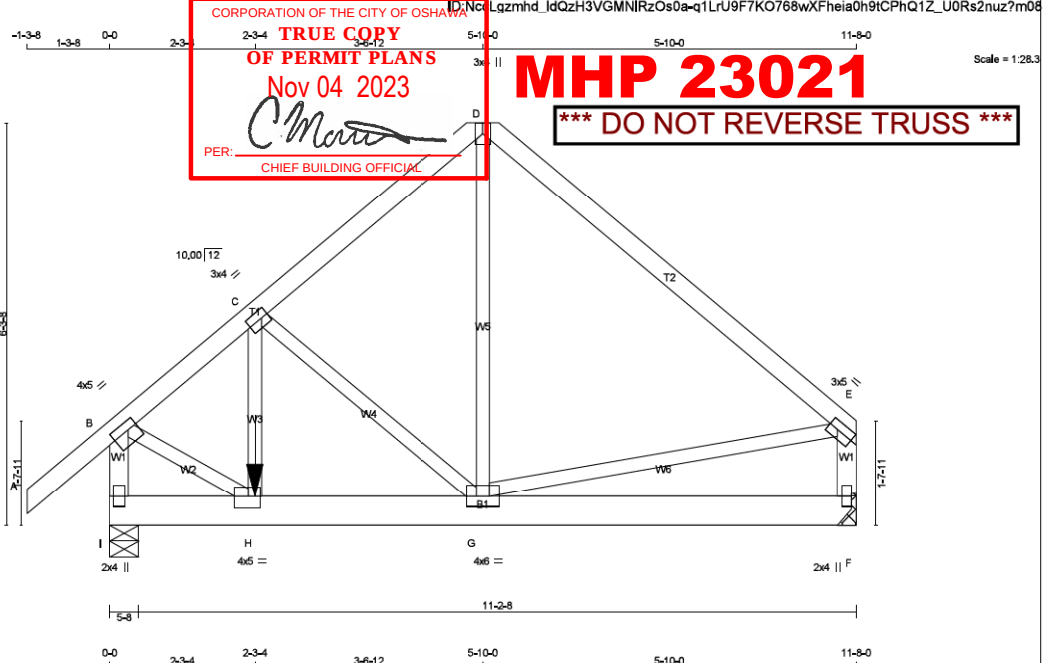
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.87 (L) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

07/04/2023
D. A. SHERMAN
100123373
PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - D 2x4 DRY No.2 SPF
B - E 2x4 DRY 2100F 1.8E SPF
I - B 2x4 DRY No.2 SPF
F - E 2x4 DRY No.2 SPF
I - F 2x6 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	5.0	1.50	1.75
D	TMVW4	MT20	3.0	4.0	1.50	1.25
C	TMTMW+p	MT20	3.0	4.0		
E	TMVW4	MT20	3.0	5.0	1.50	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BMVW4	MT20	4.0	6.0		
H	BMVW4	MT20	4.0	5.0	2.25	2.25
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORZ	UP/LIFT
I	1839	0	1839	0
F	1013	0	1013	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
I	1282	942 / 0	0 / 0	0 / 0	0 / 0	340 / 0	0 / 0	0 / 0
F	708	512 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0 / 53	-119.4 -119.4	0.18 (1)	10.00	H-C	0 / 435	0.11 (1)
B-C	-1410 / 0	-119.4 -119.4	0.22 (1)	5.21	C-G	-680 / 0	0.25 (1)
C-D	-809 / 0	-119.4 -119.4	0.21 (1)	6.25	G-D	0 / 476	0.12 (1)
D-E	-778 / 0	-119.4 -119.4	0.54 (1)	6.25	B-H	0 / 1261	0.31 (1)
I-B	-1768 / 0	0.0	0.0	0.20 (1)	G-E	0 / 610	0.15 (1)
F-E	-870 / 0	0.0	0.0	0.11 (1)			
I-H	0 / 0	-18.2	-18.2	0.08 (1)	10.00		
H-G	0 / 1111	-18.2	-18.2	0.23 (1)	10.00		
G-F	0 / 0	-18.2	-18.2	0.07 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX+	MAX-	FACE	DIR.	TYPE	HEEL	CONN.
H	2-3-4	-1080	-1080		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.54/1.00 (D-E-1), BC=0.23/1.00 (G-H-1), WB=0.31/1.00 (B-H-1), SS=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

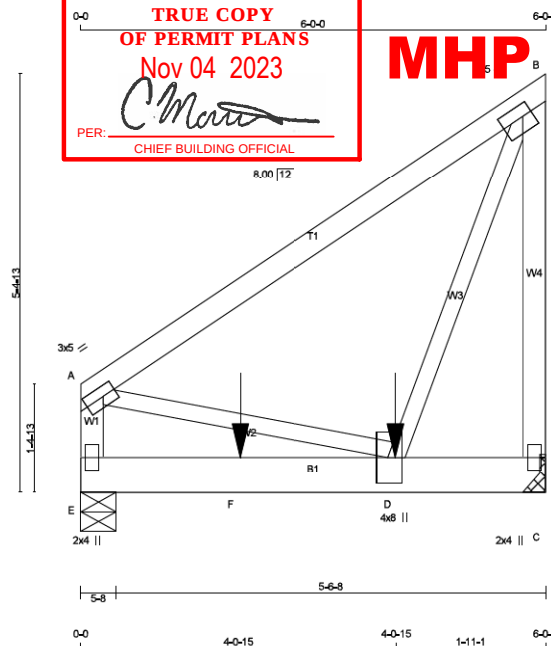
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)





MHP 23021



TOTAL WEIGHT = 32 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF

E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	2100F 1.8E	SPF
C - B	2x4	DRY	No.2	SPF
E - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	3.0	5.0	1.50	2.00
B	TMVW4	MT20	4.0	5.0	1.75	1.50
C	BMV1+p	MT20	2.0	4.0		
D	BMVW+t	MT20	4.0	8.0	4.00	1.75
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT
E	1159	0	1159	0
C	1322	0	1322	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	810	586 / 0	0 / 0	0 / 0	224 / 0	0 / 0	0 / 0
C	924	669 / 0	0 / 0	0 / 0	255 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	MEMB.
FR-TO		FROM TO		FR-TO			
E-A	-846 / 0	0.0 0.0	0.09 (1)	A-D	0 / 551	0.14 (1)	
A-B	-643 / 0	-119.4 +119.4	0.56 (1)	D-B	0 / 1490	0.37 (1)	
C-B	-1392 / 0	0.0 0.0	0.65 (1)				
E-F	0 / 0	-18.2 -18.2	0.45 (1)				
F-D	0 / 0	-18.2 -18.2	0.45 (1)				
D-C	0 / 0	-18.2 -18.2	0.12 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	4-0-12	-1013	-1013		BACK	VERT	TOTAL		C1
F	2-0-12	-642	-642		BACK	VERT	TOTAL		

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.65/1.00 (B-C:1), BC=0.45/1.00 (D-E:1), WB=0.37/1.00 (B-D:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.32 (D) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



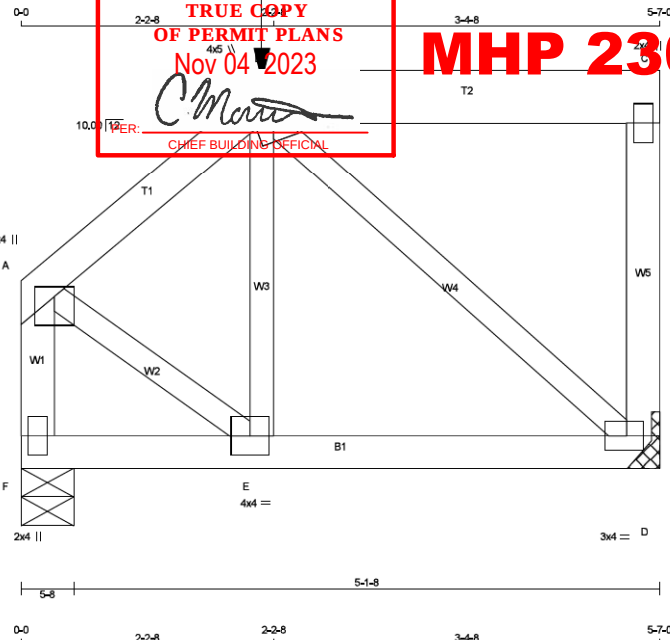
REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSCI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbciindustry.com



CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 04 2023
CHIEF BUILDING OFFICIAL

MHP 23021



TOTAL WEIGHT = 28 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE LUMBER
F - A 2x4 DRY No.2
A - B 2x4 DRY No.2
B - C 2x6 DRY No.2
D - C 2x4 DRY No.2
F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
F	1097	0	1097	0
D	1081	0	1081	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	766	557 / 0	0 / 0
D	755	548 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (L/C)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (L/C)
FR-TO	F-A	-949 / 0	0.0	0.0	0.11 (1)	7.81	A-E	0 / 666	0.16 (1)
	A-B	-733 / 0	-119.4	-119.4	0.12 (1)	6.25	E-B	0 / 620	0.15 (1)
	B-C	0 / 0	-112.0	-112.0	0.12 (1)	10.00	B-D	-743 / 0	0.25 (1)
	D-C	-189 / 0	0.0	0.0	0.03 (1)	7.81			
	F-E	0 / 0	-269.5	-269.5	0.43 (1)	10.00			
	E-D	0 / 545	-269.5	-269.5	0.52 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-2-8	-31	-	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-8
RIGHT SETBACK = 0-0
END SETBACK = 2-2-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-4-0
END DISTANCE = 5-7-0
END SPAN CARRIED = 9-4-0
END WALL WIDTH = 0-0
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.12/1.00 (B-C:1), BC=0.52/1.00 (D-E:1), WB=0.25/1.00 (B-D:1), SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

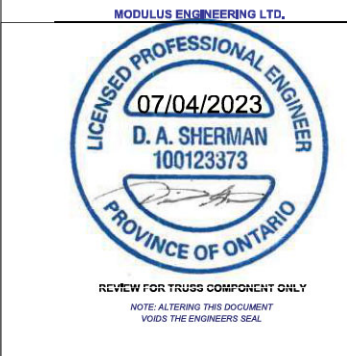
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (D) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSC-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPI Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C. D.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C	- CSA 086-14 - TPIC 2014
--	-----------------------------

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F.

TOTAL LOAD CASES: (4)	ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
-----------------------	---

MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)	CSI: TC=0.16/1.00 (A-B:1) , BC=0.13/1.00 (D-E:4) ,
-------	----------------	---------------------	---------------------	----------------	-------	----------------	-----------------	--

FACTORED CONCENTRATED LOADS (LBS) TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

CONNECTION REQUIREMENTS	PLATE (PSI)	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
-------------------------	----------------	--------------------	----------------	------------------

PLATE PLACEMENT TOL. = 0.250 inches

JSI GRIP= 0.17 (B) (INPUT = 0.90)

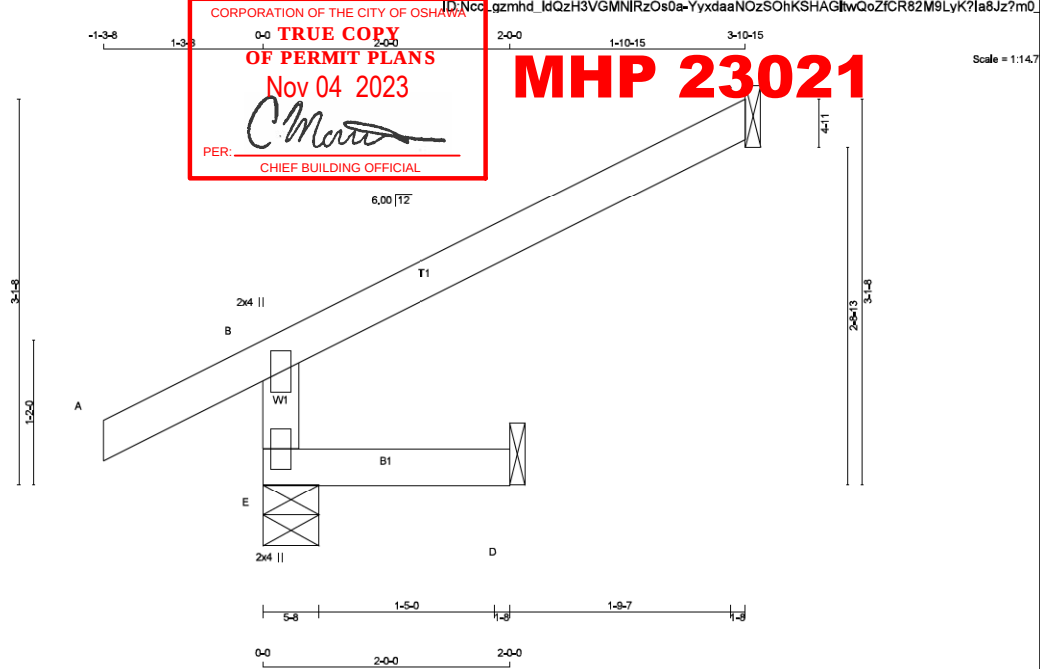
[illegible][illegible]

	Date _____
--	------------

[illegible][illegible][illegible]

based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper use - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure

available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee



TOTAL WEIGHT = 2 X 10 = 20 lb

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4 DRY	No.2
A - C	2x4 DRY	No.2
E - D	2x4 DRY	No.2

DRY: SEASONED LUMBER

JT	TYPE	PLATES
2	TABLE	15722

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

BEARINGS

FA
GRO

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	474	0	474	0	0	5-8	1-8
C	175	0	175	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

1ST LCASE MA
 1ST LCASE MA

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

TOP CHC

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

TOTAL LC

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS1 (LC)	MAX. UNBRAC LENG	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)		
FR-TO		FROM TO		FR-TO					
E-B	-454 / 0	0.0	0.0	0.01 (4)		7.81			
A-B	0 / 36	-119.4	-119.4	0.16 (1)		10.00			
B-C	-26 / 0	-119.4	-119.4	0.31 (1)		6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

ORDERED: 10/1/01

TOP CH. LL =

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

Write the number in standard form.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

-OVERHANG NOT TO BE

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.31/1.00 (B-C:1), BC=0.02/1.00 (D-E:4),
WR=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



NOTE: ALTERING THIS DOCUMENT

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

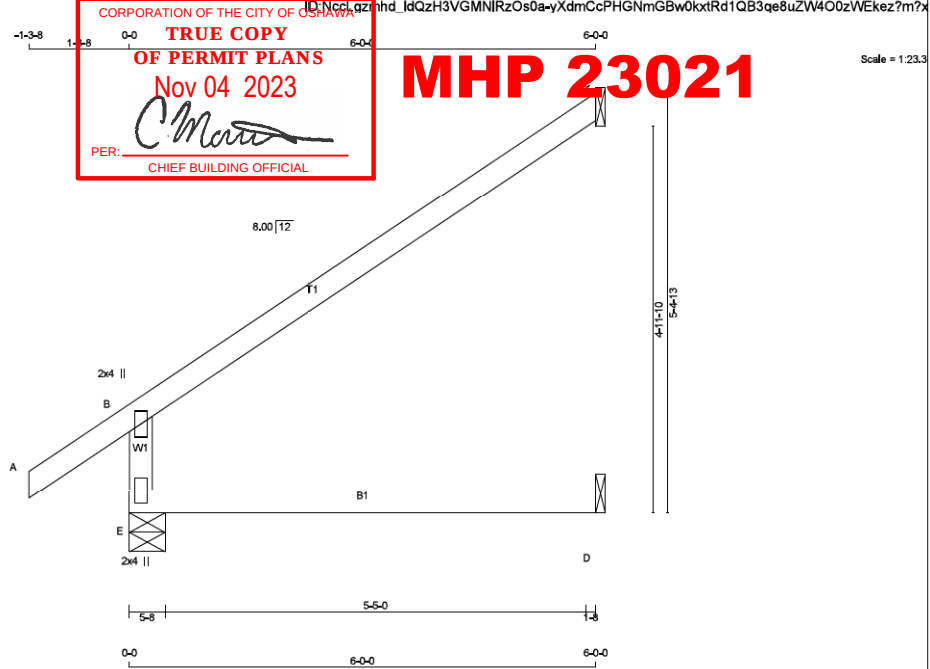
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.com and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.bcsincanada.com





TOTAL WEIGHT = 2 X 7 = 15 lb





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
E	675	0	5-8	1-8
C	269	0	1-8	1-8
D	46	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	469	357 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
C	184	157 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1 TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
E-B	-612 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 45	-119.4	-119.4	0.16 (1)	10.00			
B-C	-50 / 0	-119.4	-119.4	0.73 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.73/1.00 (B-C-1) BC=0.14/1.00 (D-E-4) ,
WB=0.00/1.00 (n/a.0) , SSJ=0.29/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.99)
JSI METAL= 0.31 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

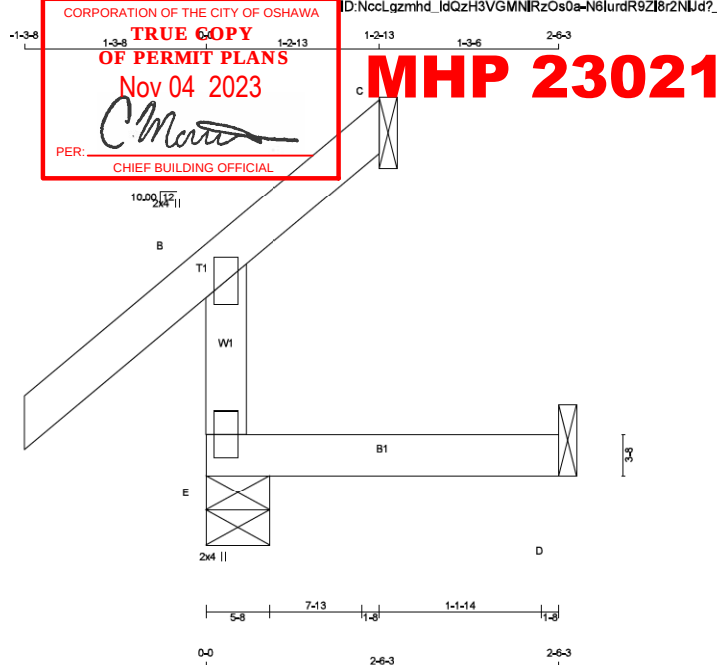
NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSC-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com









LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF
E - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						DESIGN CRITERIA	
BEARINGS						SPECIFIED LOADS:	
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		TOP CH. LL = 34.8 PSF	
JT	VERT	JT	VERT	JT	VERT	DL = 6.0 PSF	
E	345	0	345	0	5-8	BOT CH. LL = 0.0 PSF	
C	0	0	4	0	1-8	DL = 7.3 PSF	
D	13	0	23	0	1-8	TOTAL LOAD = 48.1 PSF	
SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D						SPACING = 24.0 IN. GIC	
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT						THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015	
UNFACTORED REACTIONS						THIS DESIGN COMPLIES WITH:	
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	- PART 9 OF BCBC 2018, NBC-2019AE	
E	238	189 / 0	0 / 0	0 / 0	49 / 0	- PART 9 OF OBC 2012 (2019 AMENDMENT)	
C	0	0 / -43	0 / 0	0 / 0	3 / 0	- CSA 086-14	
D	12	0 / -5	0 / 0	0 / 0	16 / 0	- TPIC 2014	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING						DESIGN ASSUMPTIONS	
TOTAL LOAD CASES: (5)						- OVERHANG NOT TO BE ALTERED OR CUT OFF.	
CHORDS		FACTORED		WEBS		(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX. (PLF)	MEMB.	FORCE (LBS)	ALLOWABLE DEFL.(LL)= L/360 (0.19")	
FR-TO	FROM TO	FR-TO	UNBRACED LENGTH	FR-TO	FROM TO	CALCULATED VERT. DEFL.(LL) = L/999 (0.00")	
E-B	-313 / 0	0.0	0.0	0.04 (5)	7.81	ALLOWABLE DEFL.(TL)= L/360 (0.19")	
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00	CALCULATED VERT. DEFL.(TL) = L/999 (0.00")	
B-C	-47 / 0	-119.4	-119.4	0.12 (1)	6.25	CSJ: TC=0.16/1.00 (A-B 1) BC=0.04/1.00 (D-E 5) ,	
E-D	0 / 0	-18.2	-18.2	0.04 (5)	10.00	WB=0.00/1.00 (n/a 0) , SSJ=0.10/1.00 (A-B 1)	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN
PATTERN LOADING CHECK APPLIED TO THIS TRUSS.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.21 (B) (INPUT = 0.99)
JSI METAL= 0.17 (B) (INPUT = 1.00)

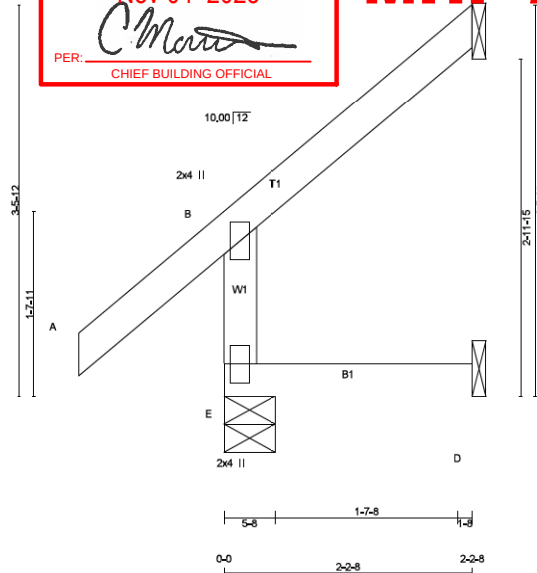
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL



MHP 23021



TOTAL WEIGHT = 2 X 9 = 19 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
E - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	352	0	352	0	5-8	1-8
C	100	0	100	0	1-8	1-8
D	18	0	21	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	243	192 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
C	68	58 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING						
TOTAL LOAD CASES: (5)						
CHORDS		FACTORED				
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO				
E-B	-330 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00	
B-C	-21 / 0	-119.4	-119.4	0.10 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSJ=0.10/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LBS DE=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.99)
JSI METAL= 0.18 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

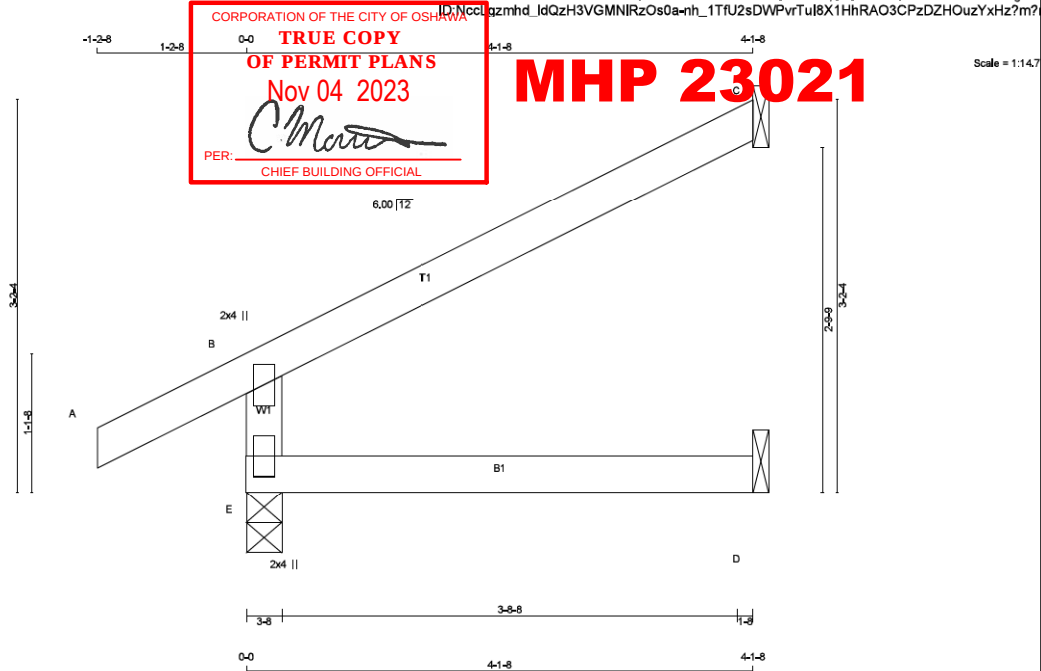


REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSC-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





TOTAL WEIGHT = 6 X 12 = 74 lb

LUMBER

N. L. G. A. RULES	
CHORDS	SIZE
E - B	2x4
A - C	2x4
E - D	2x4

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT							
E	503	0	503	0	0	3-8	1-8
C	185	0	185	0	0	1-8	1-8
D	32	0	36	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	349	268 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	126	108 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
E-B	-460 / 0	0.0	0.0	0.06 (4)		7.81			
A-B	0 / 34	-119.4	-119.4	0.14 (1)		10.00			
B-C	-27 / 0	-119.4	-119.4	0.35 (1)		6.25			
E-D	0 / 0	-18.2	-18.2	0.06 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSJ: TC=0.35/1.00 (B-C:1), BC=0.06/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



REVIEW FOR TRUSS COMPONENT ONLY

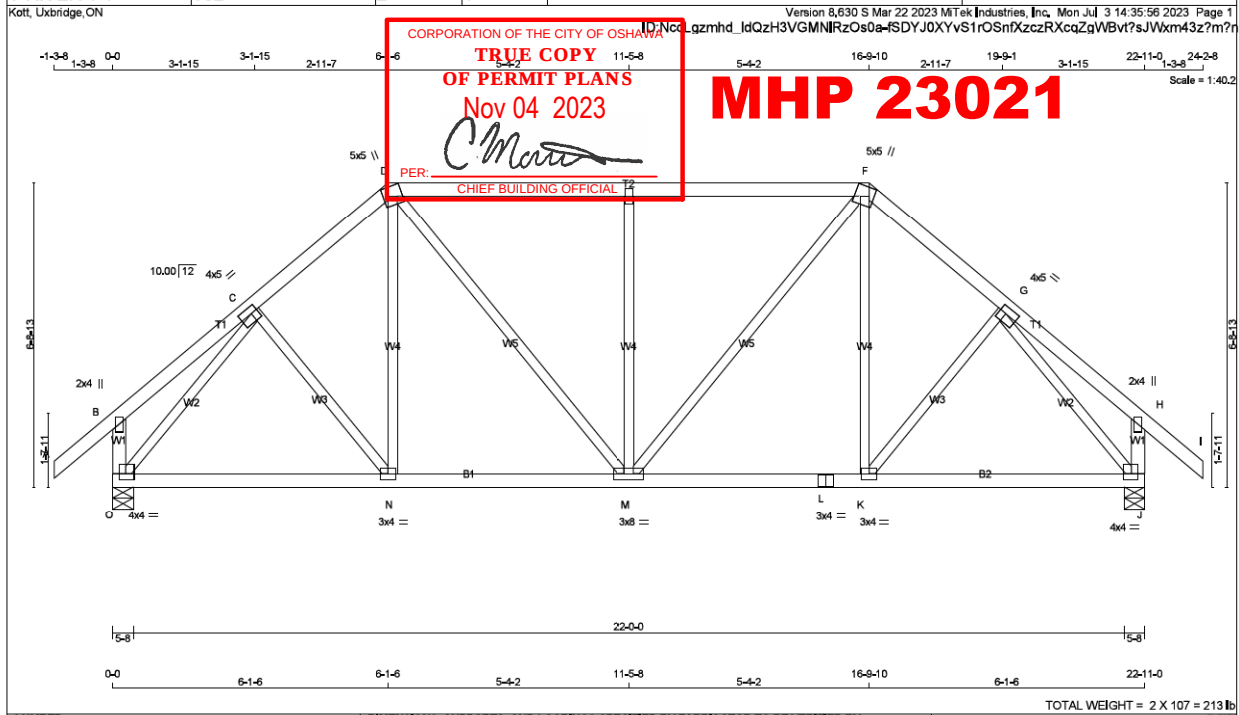
NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

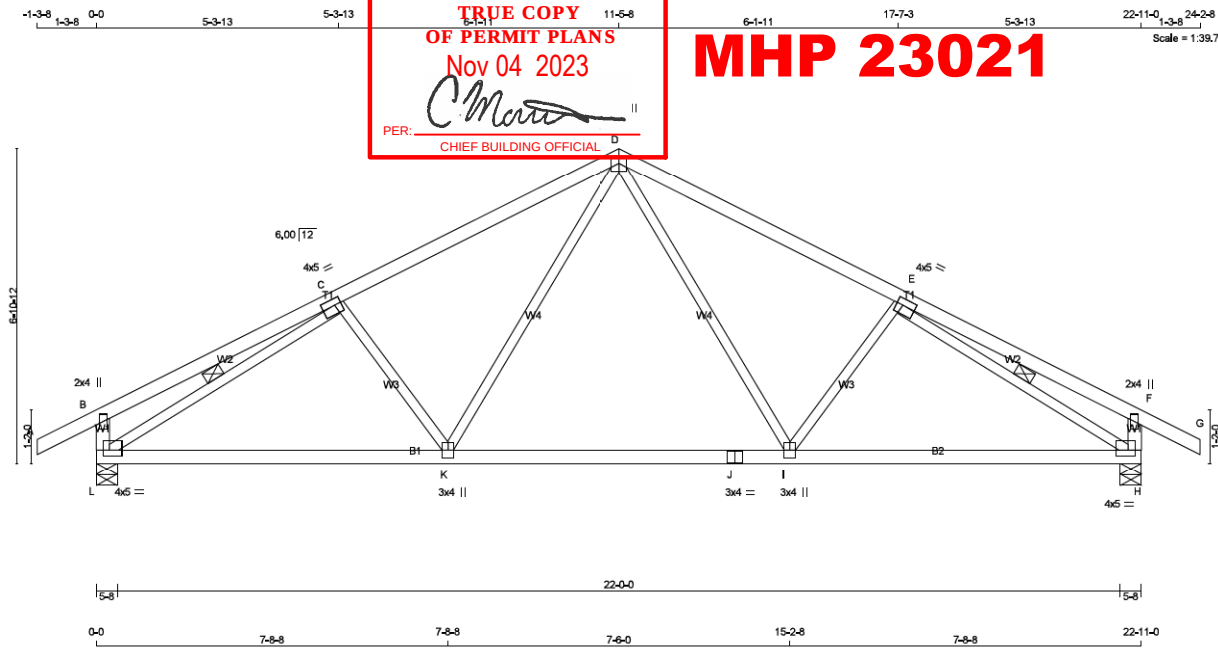
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.bscindustry.com









TOTAL WEIGHT = 12 X 89 = 1063 lb

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	No.2	SPF
D - G	2x4	No.2	SPF
L - B	2x4	No.2	SPF
H - F	2x4	No.2	SPF
L - J	2x4	No.2	SPF
J - H	2x4	No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT

DRY: SEASONED LUMBER.

JT	TYPE	PLATES
5	5	5

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWV+t	MT20	4.0	5.0	1.75	1.75
D	TTWV+p	MT20	4.0	4.0		
E	TMWV+t	MT20	4.0	5.0	1.75	1.75
F	TMV+p	MT20	2.0	4.0		
H	BMWV+t	MT20	4.0	5.0	1.50	1.75
I	BMWV+t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMWV+t	MT20	3.0	4.0		
L	BMWV+t	MT20	4.0	5.0	1.50	1.75

BUILDING DESIGNER

GRO

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQUIRED BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1740	0	1740	0	0	5-8	1-14
H	1740	0	1740	0	0	5-8	1-14

1ST LCASE MA

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1213	892/0	0/0	0/0	0/0	321/0	0/0
H	1213	892/0	0/0	0/0	0/0	321/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

TOP CHO

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H,

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LO

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 36	-119.4 -119.4	0.16 (1)	10.00	D-I	0 / 622	0.14 (1)
C-D	-119.4 / 0	-119.4 -119.4	0.65 (1)	10.00	E-E	-480 / 0	0.15 (1)
C-D	-1906 / 0	-119.4 -119.4	0.62 (1)	4.13	K-C	0 / 622	0.14 (1)
D-E	-1906 / 0	-119.4 -119.4	0.62 (1)	10.00	K-C	-480 / 0	0.15 (1)
E-F	0 / 41	-119.4 -119.4	0.65 (1)	10.00	L-C	-2346 / 0	0.64 (1)
F-G	0 / 36	-119.4 -119.4	0.16 (1)	10.00	E-H	-2346 / 0	0.64 (1)
L-B	-383 / 0	0.0 0.0	0.04 (1)	7.81			
H-F	-383 / 0	0.0 0.0	0.04 (1)	7.81			
L-K	0 / 1954	-18.2 -18.2	0.44 (1)	10.00			
K-J	0 / 1367	-18.2 -18.2	0.34 (1)	10.00			
J-I	0 / 1367	-18.2 -18.2	0.34 (1)	10.00			
I-H	0 / 1954	-18.2 -18.2	0.44 (1)	10.00			

ORDERED: 10/1/98

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

- PART 9 OF BCBC 2018 , NBC-2

- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.65/1.00 (B-C:1), BC=0.44/1.00 (K-L:1),
WB=0.64/1.00 (C-L:1), SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.61 (E) (INPUT = 1.00)



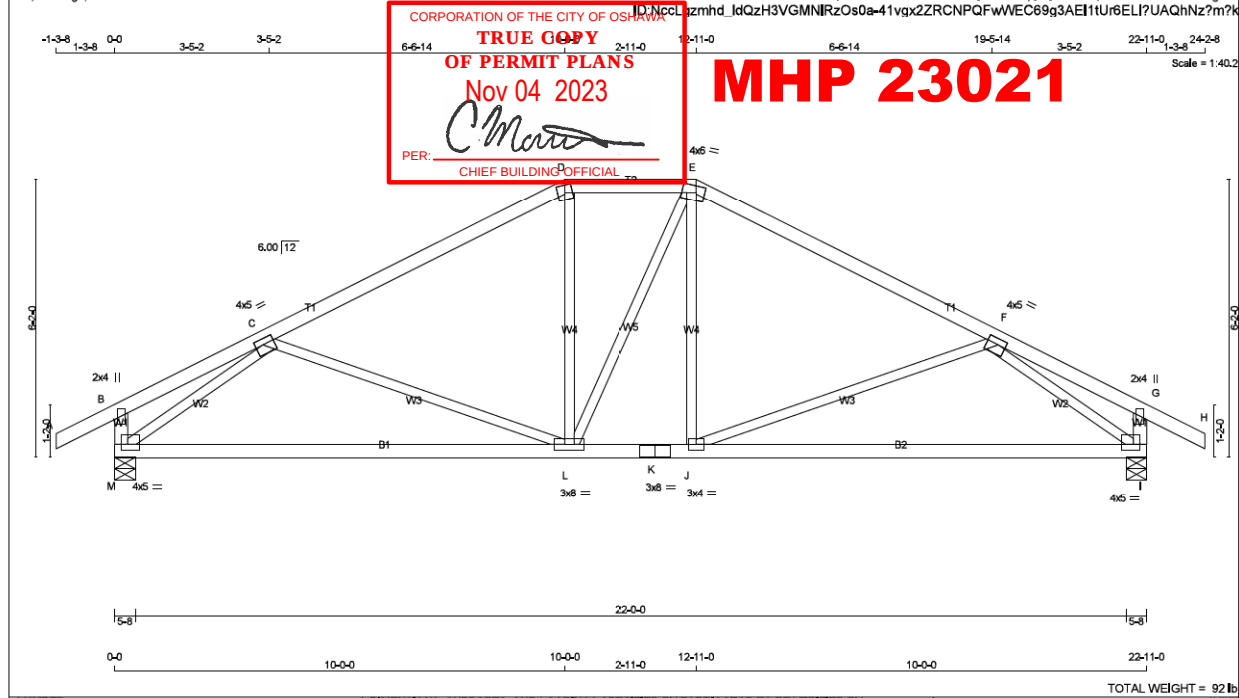
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPI Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	GROSS REACTION	DOWN	UP	BRG	TOP CH. LL = 34.8 PSF	
D - E	2x4	DRY	No.2	SPF	M	VERT	0	0	0	DL = 6.0 PSF	
E - H	2x4	DRY	No.2	SPF	I	HORIZ	1740	0	0	BOT CH. LL = 0.0 PSF	
M - B	2x4	DRY	No.2	SPF			1740	0	0	DL = 7.3 PSF	
I - G	2x4	DRY	No.2	SPF						TOTAL LOAD = 48.1 PSF	
M - K	2x4	DRY	No.2	SPF							
K - I	2x4	DRY	No.2	SPF							
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF							
DRY, SEASONED LUMBER,											

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW+H	MT20	4.0	5.0	1.50	2.00
D	TTW-m	MT20	4.0	4.0		
E	TTWV-m	MT20	4.0	6.0	1.75	2.25
F	TMVW+H	MT20	4.0	5.0	1.50	2.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW+H	MT20	4.0	5.0	1.50	1.75
J	BMVW+H	MT20	3.0	4.0		
L	BMVW+H	MT20	3.0	8.0		
M	BMVW+H	MT20	4.0	5.0	1.50	1.75

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSJ (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSJ (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 36	-119.4 -119.4	0.16 (1)	C-L	-390 / 0	0.42 (1)	
B-C	0 / 62	-119.4 -119.4	0.64 (1)	L-D	0 / 269	0.06 (1)	
C-D	-1752 / 0	-119.4 -119.4	0.70 (1)	L-E	0 / 1	0.00 (1)	
D-E	-1543 / 0	-119.4 -119.4	0.16 (1)	J-E	0 / 268	0.06 (1)	
E-F	-1752 / 0	-119.4 -119.4	0.70 (1)	J-F	-390 / 0	0.42 (1)	
F-G	0 / 62	-119.4 -119.4	0.64 (1)	M-C	-2386 / 0	0.70 (1)	
G-H	0 / 36	-119.4 -119.4	0.16 (1)	F-I	-2386 / 0	0.70 (1)	
M-B	-223 / 0	0.0 0.0	0.02 (1)				
I-G	-223 / 0	0.0 0.0	0.02 (1)				
M-L	0 / 1903	-18.2 -18.2	0.52 (1)				
L-K	0 / 1543	-18.2 -18.2	0.50 (4)				
K-J	0 / 1543	-18.2 -18.2	0.50 (4)				
J-I	0 / 1903	-18.2 -18.2	0.53 (1)				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE

- PART 9 OF CBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL) = L/360 (0.76")

CALCULATED VERT. DEFL.(TL) = L/813 (0.34")

CSJ: TC=0.70/1.00 (C-D-1), BC=0.53/1.00 (I-J-1), WB=0.70/1.00 (C-M-1), SS=0.33/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	MAX 650	MIN 371	MAX 1747
	MIN 500	MIN 371	MIN 1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)

JSI METAL= 0.89 (K) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

LICENSED PROFESSIONAL ENGINEER

07/04/2023

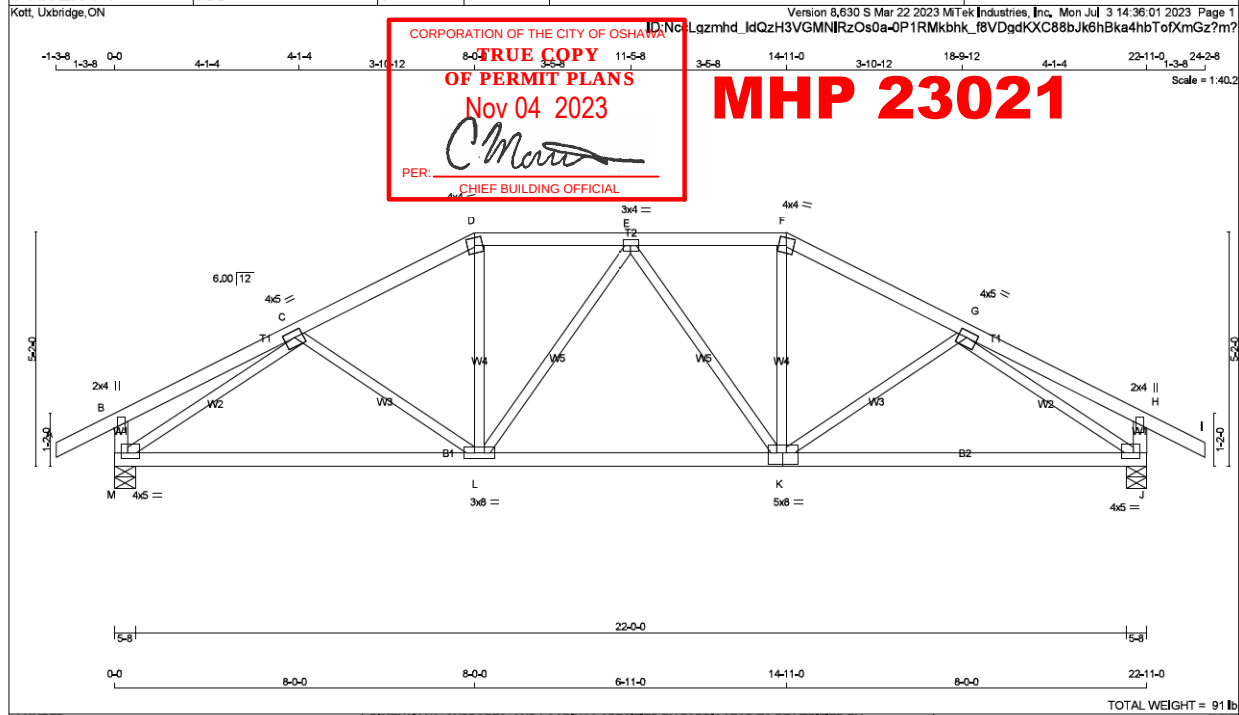
D. A. SHERMAN

100123373

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

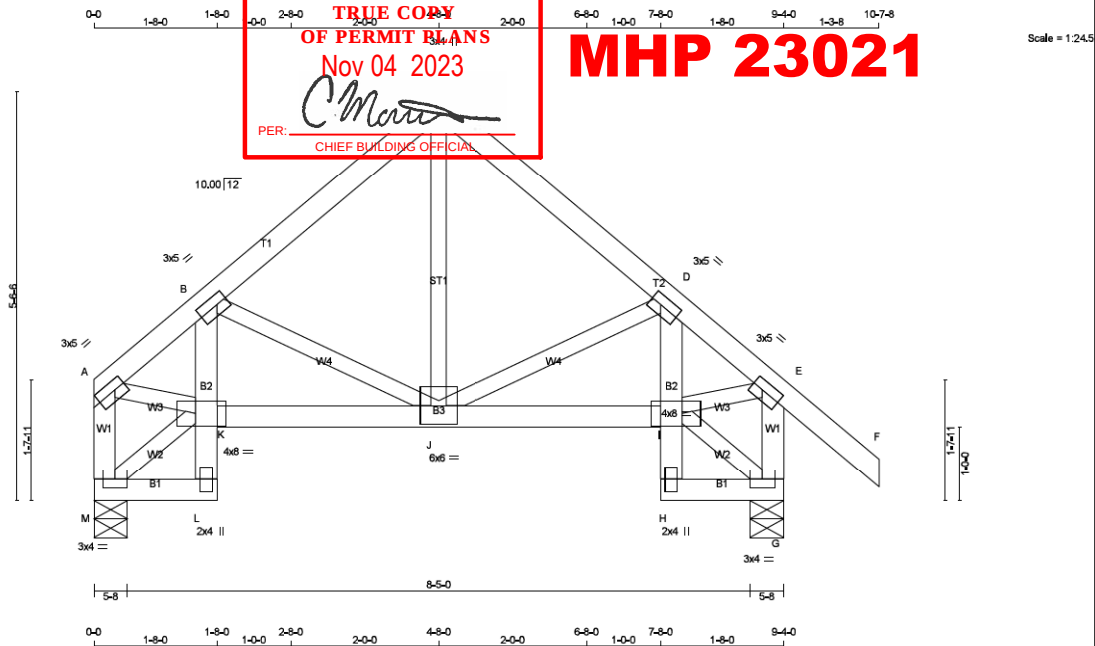
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - I 2x4 DRY No.2 SPF M - B 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF M - K 2x4 DRY No.2 SPF K - J 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER,			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQD JT VERT DOWN HORZ UPLIFT IN-SX IN-SX M 1740 0 1740 0 0 5-8 1-14 J 1740 0 1740 0 0 5-8 1-14 UNFACTORED REACTIONS 1ST CASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL M 1213 892 / 0 0 / 0 0 / 0 321 / 0 0 / 0 J 1213 892 / 0 0 / 0 0 / 0 321 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. MAX. FACTORED FORCE VERT. LOAD (LBS) (PLF) CSJ (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE VERT. LOAD (LBS) (PLF) CSJ (LC) FR-TO FROM TO A-B 0 / 36 -119.4 -119.4 0.16 (1) 10.00 C-L -189 / 19 0.08 (1) B-C 0 / 23 -119.4 -119.4 0.28 (1) 10.00 L-D 0 / 506 0.11 (1) C-D -1933 / 0 -119.4 -119.4 0.28 (1) 4.60 L-E -281 / 0 0.17 (1) D-E -1718 / 0 -119.4 -119.4 0.20 (1) 4.90 E-K -323 / 0 0.20 (1) E-F -1693 / 0 -119.4 -119.4 0.20 (1) 4.93 K-F 0 / 495 0.11 (1) F-G -1909 / 0 -119.4 -119.4 0.26 (1) 4.63 K-G -193 / 14 0.08 (1) G-H 0 / 23 -119.4 -119.4 0.28 (1) 10.00 M-C -2288 / 0 0.90 (1) H-I 0 / 36 -119.4 -119.4 0.16 (1) 10.00 G-J -2264 / 0 0.89 (1) M-B -351 / 0 0.0 0.0 0.04 (1) 7.81 J-H -351 / 0 0.0 0.0 0.04 (1) 7.81 M-L 0 / 1862 -18.2 -18.2 0.45 (1) 10.00 L-K 0 / 1877 -18.2 -18.2 0.46 (1) 10.00 K-J 0 / 1843 -18.2 -18.2 0.50 (1) 10.00						DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIC LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.76") CALCULATED VERT. DEFL.(LL) = L/999 (0.08") ALLOWABLE DEFL.(TL) = L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/999 (0.26") CSJ: TC=0.28/1.00 (G+H-1), BC=0.50/1.00 (J+K-1), WB=0.90/1.00 (C-M-1), SS=0.20/1.00 (G+H-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.88 (K) (INPUT = 0.90) JSI METAL= 0.59 (C) (INPUT = 1.00)		
---	--	--	--	--	--	--	--	--	---	--	--



MHP 23021



TOTAL WEIGHT = 48 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - E	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
H - D	2x4	DRY	No.2
H - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2'-0" OC.		

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A, B, D, E					
A - TMVW-H	MT20	3.0	5.0	1.50	1.75
C - TTW+p	MT20	3.0	4.0	2.50	1.50
G - BMVW+1	MT20	3.0	4.0		
H - BMV+p	MT20	2.0	4.0		
I - BVMW+1	MT20	4.0	8.0	2.00	5.00
J - BVMW+1	MT20	6.0	8.0	2.00	5.00
K - BVMW+1	MT20	4.0	8.0		
L - BMV+p	MT20	2.0	4.0		
M - BMVW+1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	HORZ	UP/LIFT	BRG	IN-SX	BRG	IN-SX
M	630	0	630	0	0	5-8	1-8		
G	820	0	820	0	0	5-8	1-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	441	318 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
G	570	428 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	-633 / 0	-119.4 -119.4	0.20 (4)	J-D	-123 / 0	0.03 (1)	
B-C	-338 / 0	-119.4 -119.4	0.31 (1)	B-J	-195 / 0	0.04 (1)	
C-D	-334 / 0	-119.4 -119.4	0.32 (1)	A-K	0 / 445	0.10 (1)	
D-E	-804 / 0	-119.4 -119.4	0.21 (1)	I-E	0 / 380	0.09 (1)	
E-F	0 / 53	-119.4 -119.4	0.16 (1)	M-K	-21 / 0	0.00 (1)	
M-A	-604 / 0	0.0 0.0	0.06 (1)	I-G	-21 / 0	0.00 (1)	
G-E	-794 / 0	0.0 0.0	0.08 (1)				
M-L	0 / 17	-18.2 -18.2	0.01 (4)				
L-K	0 / 16	0.0 0.0	0.03 (1)				
K-B	0 / 76	0.0 0.0	0.04 (1)				
K-J	0 / 459	-18.2 -18.2	0.43 (1)				
J-I	0 / 384	-18.2 -18.2	0.42 (1)				
H-I	0 / 16	0.0 0.0	0.03 (1)				
I-D	0 / 82	0.0 0.0	0.04 (1)				
H-G	0 / 17	-18.2 -18.2	0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.31")
CALCULATED VERT. DEFL.(LL) = $L/840$ (0.13")
ALLOWABLE DEFL.(TL) = $L/360$ (0.31")
CALCULATED VERT. DEFL.(TL) = $L/388$ (0.29")
CSI: TC=0.32/1.00 (C-D-1), BC=0.43/1.00 (J-K-1), WB=0.10/1.00 (A-K-1), SSI=0.16/1.00 (D-E-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
650	371	1747	788

PLATE PLACEMENT TOL. = 0.250 inches

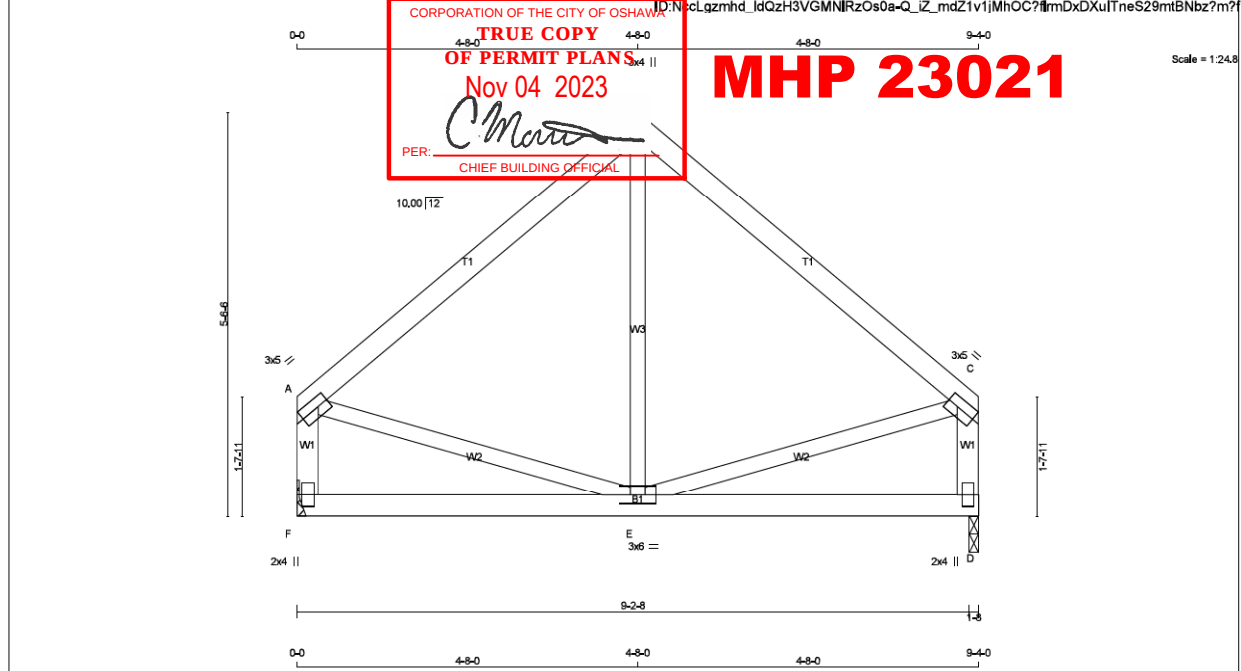
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (M) (INPUT = 0.90)
JSI METAL= 0.22 (A) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSC-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





TOTAL WEIGHT = 2 X 39 = 78 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW4	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMVW4	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
F	642	0	642	0
D	642	0	642	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	449	325 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
D	449	325 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	MEMB.
FR-TO				FR-TO			
A-B	-387 / 0	-119.4	0.33 (1)	E-B	-62 / 69	0.03 (1)	
B-C	-387 / 0	-119.4	0.33 (1)	A-E	0 / 309	0.07 (1)	
F-A	-609 / 0	0.0	0.06 (1)	E-C	0 / 309	0.07 (1)	
D-C	-609 / 0	0.0	0.06 (1)				
F-E	0 / 0	-18.2	-18.2 0.11 (4)				
E-D	0 / 0	-18.2	-18.2 0.11 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. G.C.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.33/1.00 (B-C:1), BC=0.11/1.00 (E-F:4), WB=0.07/1.00 (A-E:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1973

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (E) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 1.00)

