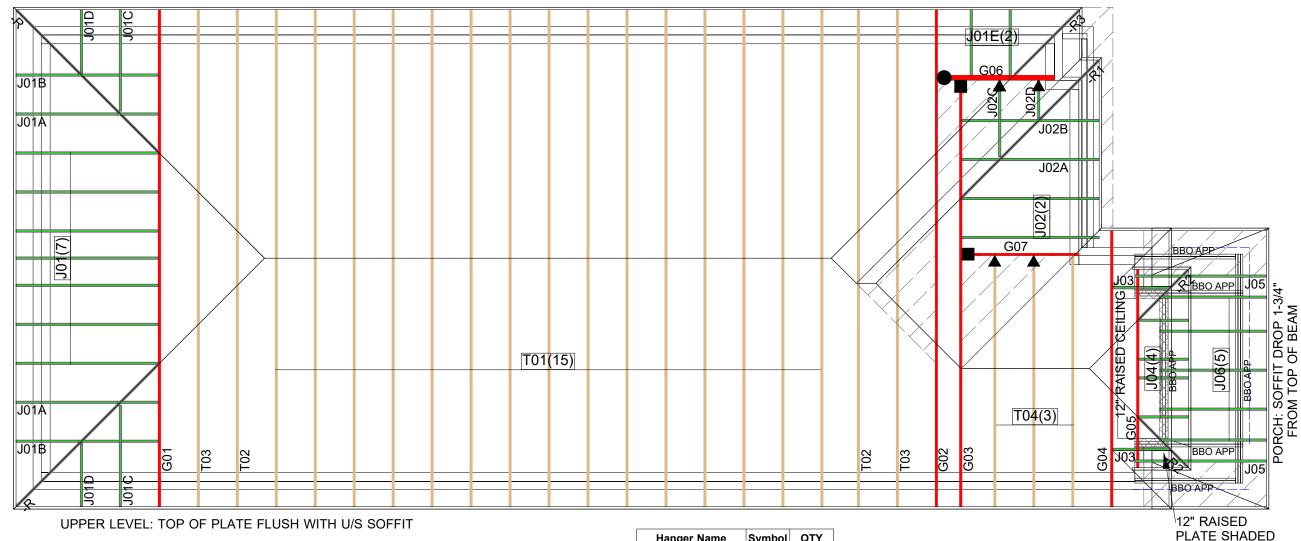


MHP 23023



Hanger Name	Symbol	QTY
LUS24	A	6
LJS26DS		2
HHUS26-2		1



ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB IN	FORMATION
Customer	GREENPARK GROUP
Job #	23-00080R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC
	OSHAWA,ON
Model	RIVER 3-3
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-04-26
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 3\RIVER 3-3\T-RIVE

DECICAL INICODA AATIONI

DESIGN	I INFORMATION
Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft²
TC DL	6.0 lb/ft ²
BC LL	0.0 lb/ft²
BC DL	7.3 lb/ft²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise
Spacing	noted
Complies With	OBC 2012 (2019 Amendment) CSA 086-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400





General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



Read Carefully Prior to Manufacture and Installation

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-trangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

- 1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult_BCSI-CANADA for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)
- 2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.
- 3. Never exceed the design loading shown and never stack buil materials on inadequately braced trusses
- 4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)
- 5. Cut members to bear tightly against one another
- 6. Place plates on each face of truss at each joint and embed fully using proper roller or hydraulic press. Knots and wane at joint locations are regulated by TPIC Appendix G
- 7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC
- 8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture
- 9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in corrosive environment
- 10. Connections not shown are the responsibility of others
- 11. Do not cut or alter truss members or plates without prior approva of an engineer
- 12. Install and load vertically unless otherwise noted
- 13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient
- 14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G
- 16. Building designer must review individual component drawings to ensure they are suitable for the structure
- 15. Not designed for solar panels unless specifically noted

ME-TCD01 (VER. 06/2017)







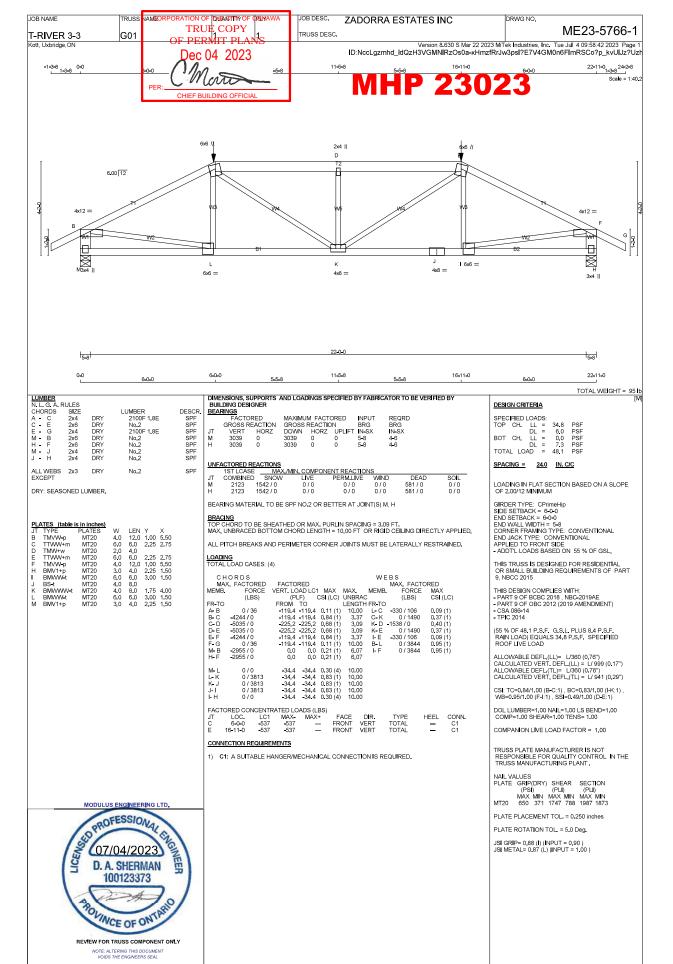












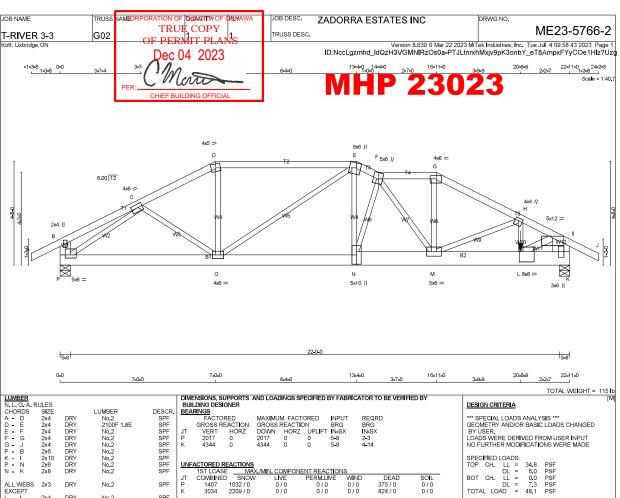
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not furst sedesjner. Breading shown is for lateral support of individual web members only. Additional reportance to stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com





LUMBER				
N.L.G.A.R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	2100F 1.8E	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
K - I	2x10	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x8	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN	Υ	Х					
В	TMV+p	MT20	2.0	4.0							
С	TMWW-t	MT20	4.0	6.0	1.75	2.50					
D	TTW-m	MT20	4.0	5.0							
Е	TTWW+m	MT20	6.0	6.0	2,50	2,25					
F	TTVVV+m	MT20	5.0	6.0	3.25	2.00					
G	TTW-I	MT20	4.0	6.0	1.75	3.25					
Н	TMWW+t	MT20	4.0	4.0	1.50	1.75					
1	TMVW-p	MT20	5.0	12.0	Edge						
K	BMV1+p	MT20	3.0	6.0							
L	BMWW-t	MT20	8.0	8.0	5.50	2.00					
M	BMWWW-t	MT20	5.0	8.0							
Ν	BSWW+I	MT20	5.0	10.0							
0	BMWWW-t	MT20	4.0	6.0							
Ρ	BMVW1-t	MT20	5.0	6.0	1.75	2.75					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1407	1032 / 0	0/0	0/0	0/0	375 / 0	0/0
K	3034	2209 / 0	0/0	0/0	0/0	824 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	CHORDS							
CHORDS				WEBS				
	X. FACTORED					MAX. FACTO		
MEMB.		VERT. LOAD LC					MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 36	-119.4 -119.4	0.17(1)	10.00	C- O	-8 / 60	0.02(4)	
B-C	0 / 18	-119.4 -119.4	0.22(1)	10.00	0- D	0 / 588	0.15(1)	
C-D	-2542 / 0	-119.4 -119.4	0.32(1)	3.99	0- E	-848 / 0	0.93 (1)	
D-E	-2269 / 0	119.4 119.4	0.70(1)	4.41	N-E	0 / 1518	0.38 (1)	
E-F	-3298 / 0	119.4 119.4				-1482 / 0	0.38(1)	
F-G	-3177 / 0	-119.4 -119.4	0.28(1)	3.64	F- M	-316 / 0	0.10(1)	
G-H	-3527 / 0	-119.4 -119.4	0.48(1)	3.27	M-G	0 / 1220	0,30 (1)	
H-I	-5040 / 0	119.4 119.4	0.45(1)	2.72	M-H	-1521 / 0	0.48(1)	
I-J	0 / 36	119.4 119.4			L- H	0 / 903	0.22 (1)	
P-B	-331 / 0	0.0 0.0	0.02(1)	7.81	P-C	-2786 / 0	0.83(1)	
K-I	-3958 / 0	0.0 0.0	0.17 (1)	6.67	L-1	0 / 4747	0.84 (1)	
P-0	0 / 2261	-18.2 -18.2	0.36(1)	10.00				
0- N	0 / 2968	18.3 -18.3	0.43(1)	10.00				
N-M	0 / 3353		0.40(1)					
M-L	0 / 4523	18.2 18.2						
177.12	0 / 4020		0.00 (1)					

FACTORED CONCENTRATED LOADS (LBS) LOC. LC1 MAX- MAX+ FACE DIR. 20-8-9 -2882 -2882 --- FRONT VERT HEEL CONN. C1

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2,00/12 MINIMUM

*** NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.88/1.00 (L-M:1) , WB=0.93/1.00 (E-O:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90) JSI METAL= 0.79 (L) (INPUT = 1.00)

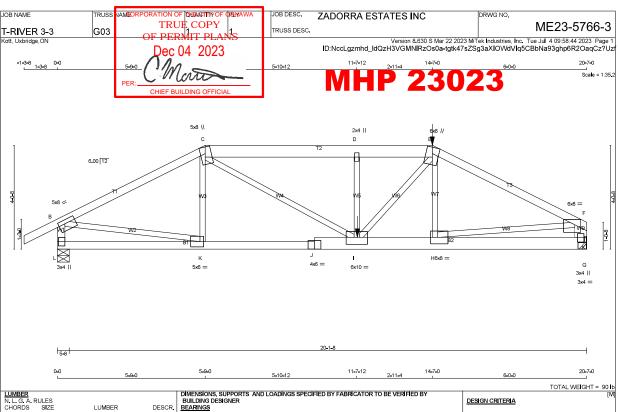
PROFESSIONAL HILL OT/04/2023 100123373 ROVINCE OF ONTARIO

> REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x6	DRY	No.2	SPF
E - F	2x4	DRY	2100F 1.8E	SPF
L - B	2x4	DRY	No.2	SPF
G-F	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

BMWWV-t

PLATES (table is in inches) TYPE TMVW+t TTWW+m TMW+w TTWW+m TMVW-p BMV1+p TP-t BMWWV-t BMWWW-t BS-t

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

MT20 MT20

	FACTOR	ED	MAXIMUN	/ FACTO	DRED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	N-SX
	2470	0	2470	0	0	5-8	4-8
3	2878	0	2878	0	0	MECHANIC	:AL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS 1STICASE MAX/MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1723	1266 / 0	0/0	0/0	0/0	456 / 0	0/0
G	2014	1443 / 0	0/0	0/0	0/0	571 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 3,27 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS				WEBS						
	MAX	, FACTORED	FACTOR	RED		MAX, FACTORED				
	MEMB.	FORCE	VERT. LO	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
		(LBS)	(PL	.F) (CSI (LC)	UNBRAG		(LBS)	CSI (LC	2)
	FR-TO		FROM	TO		LENGTH	FR-TO			
	A-B	0 / 36	119.4	-119.4	0.11(1)	10.00	K-C	-350 / 6	0.09 (1)
	B-C	-3322 / 0	-119.4	-119.4	0.67(1)	3.92	C-I	0 / 2280	0.56 (1	į.
	C-D	- 4932 / 0	-119.4	-119.4	0.48(1)	3,47	I- D	-881 / 0	0.21 (1	
	D-E	-4933 / 0						0 / 1469		
	E-F	-4461 / 0	-119.4	-119.4	0.86(1)	3.27	H-E	-257 / 88	0.06 (1)
	L-B	-2430 / 0						0 / 3012		í
	G-F	-2776 / 0	0.0	0.0	0,19 (1)	6,23	H-F	0 / 4023	1,00 (1)
	L-K	0/0	-18.2	-18.2	0.16(4)	10.00				
	K-J	0 / 2988	-18.3	-18.3	0.62(1)	10.00				
	J- I	0 / 2988	-18.2	-18.2	0,62 (1)	10,00				
	I-H	0 / 4006	-34.4	34.4	0.60(1)	10.00				
	H-G	0/0	-34.4	34.4	0.15(4)	10.00				
	FACTO	RED CONCENT	RATED LO	ADS (L	BS)					
	iT.	100 104	144 V	MANY	=	AOE 5	OID.	TVDE	LIEEL A	201

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER,
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2,00/12 MINIMUM

GIRDER TYPE: CPrimeHip LEFT SETBACK = 5-9-0 RIGHT SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-9 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE -ADDTL LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 8-11-0 OF SPAN MFASI IRED FROM THE RIGHT. MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL)= L/360 (0.69")
CALCULATED VERT. DEFL (LL)= L/999 (0.14")
ALLOWABLE DEFL (TL)= L/360 (0.69")
CALCULATED VERT. DEFL (TL)= L/995 (0.25")

CSI: TC=0.86/1.00 (E-F:1) , BC=0.62/1.00 (I-K:1) , WB=1.00/1.00 (F-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

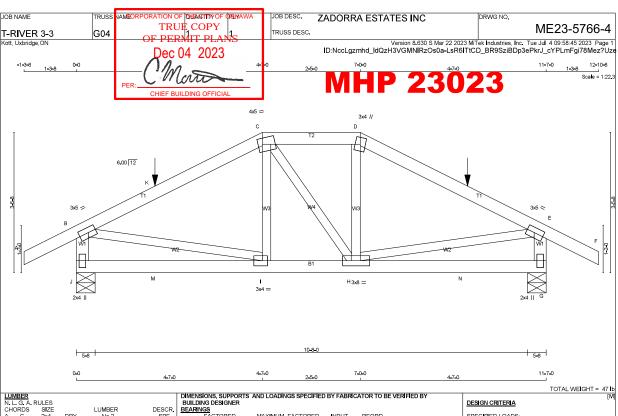
JSI GRIP= 0.88 (L) (INPUT = 0.90) JSI METAL= 0.73 (J) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

PL	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	Х					
В	TMVW-t	MT20	3.0	5.0	1,50	2,25					
С	TTVVV-m	MT20	4.0	5.0	1.75	1,25					
D	TTW+m	MT20	3.0	4.0							
Е	TMVW-t	MT20	3.0	5.0	1,50	2,25					
G	BMV1+p	MT20	2.0	4.0							
Н	BMWWW-t	MT20	3.0	8.0							
1	BMWW-t	MT20	3.0	4.0	1.50	1.50					
J	BMV1+p	MT20	2.0	4.0							

DEA	RINGS						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	975	0	975	0	0	5-8	1-8
G	975	0	975	0	0	5-8	1-8

UNFACTORED REACTIONS

	IST LUASE	IVIAA./I	VIIIN, COIVIFOI				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	678	509 / 0	0/0	0/0	0/0	169 / 0	0/0
G	678	509 / 0	0/0	0/0	0/0	169 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J. G

DRAUNO TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

ı	CHC	RDS				WE	BS		
ı	MAX.	FACTORED	FACTORED				MAX. FACT	ORED	
ı	MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
ı		(LBS)	(PLF) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
ı	FR-TO		FROM TO		LENGTH	FR-TO			
ı	A-B	0 / 36	119.4 -119.4	0.17(1)	10.00	I-C	-66 / 38	0.01(4)	
ı	B-K	-807 / 0	119.4 -119.4	0.53(1)	5.83	C-H	0/2	0.00(4)	
ı	K-C	-807 / 0	119.4 -119.4	0,53(1)	5,83	H-D	-64 / 40	0.02(4)	
ı	C-D	-719 / 0	119.4 -119.4	0.14(1)	6.25	B-I	0 / 732	0.18(1)	
ı	D-L	-809 / 0	119.4 -119.4	0.53 (1)	5.83	H-E	0 / 734	0.18 (1)	
ı	L-E	-809 / 0	-119.4 -119.4	0.53(1)	5.83				
ı	E-F	0 / 36	119.4 119.4	0,17(1)	10,00				
ı	J-B	-940 / 0	0.0 0.0	0.10(1)	7,81				
ı	G-E	-940 / 0	0.0 0.0	0.10(1)	7.81				
ı									
ı	J-M	0/0	-16.2 -16.2	0.09(4)	10,00				
ı	M- I	0/0	-18.2 -18.2	0.09(4)	10.00				
ı	I- H	0/718	-18.2 -18.2	0.16(1)	10.00				
ı	H-N	0/0	18.2 -18.2	0.09(4)	10.00				
ı	N-G	0/0	16.2 -16.2	0.09(4)	10,00				
ı									
ı	FACTOR	ED CONCEN	TRATED LOADS (L	BS)					
ı				+ F/		DIR.	TYPE		NNC.
ı			0 -20 -				TOTAL		21
ı	L	9-7-0 -2	0 -20 -	 FR⁰ 	ONT VE	RT	TOTAL	_ (21

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

SPECIFIED LOADS:										
TOP	CH.	LL	=	34.8	PSF					
		DL	=	6.0	PSF					
BOT	CH.	LL	=	0.0	PSF					
		DL	=	7.3	PSF					
TOTA	L LO	AD	=	48,1	PSF					

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-0-0
END SETBACK = 2-0-0
END SWALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE:
- ADDTL LOADS BASED ON 55 OF GSL.
LOADS APPLIED TO FRST 2-1-0 OF SPAN
MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-0-0
END SETBACK = 2-0-0
END WALL WIDTH = 5-8
CORNER FRANING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 OF GSL
LOADS APPLIED TO F

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL)= L/360 (0.39")
CALCULATED VERT. DEFL (LL) = L/999 (0.01")
ALLOWABLE DEFL (TL)= L/360 (0.39")
CALCULATED VERT. DEFL (TL) = L/999 (0.03")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.16/1.00 (H-I:1) , WB=0.18/1.00 (E-H:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY) (PSI)
 SHEAR (PLI)
 SECTION (PLI)

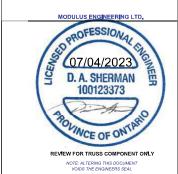
 MAX
 MIN
 MAX
 MIN
 MAX
 MIN

 MT20
 650
 371
 1747
 788
 1873

PLATE PLACEMENT TOL. = 0.250 inches

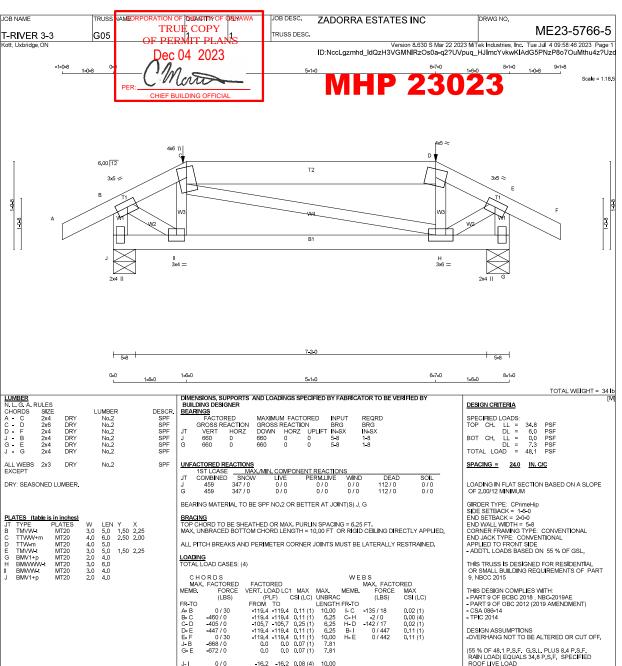
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90) JSI METAL= 0.29 (E) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





J- I I- H H- G 0/0 0/406 0/0 -16.2 -16.2 0.08 (4) -16.2 -16.2 0.12 (4) -16.2 -16.2 0.09 (4) FACTORED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX- MAX+

C 1-8-0 -15 -15 -
D 6-7-0 -15 -15 --

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.12/1.00 (H-I:4) , WB=0.11/1.00 (B-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (H) (INPUT = 0.90) JSI METAL= 0.16 (B) (INPUT = 1.00)

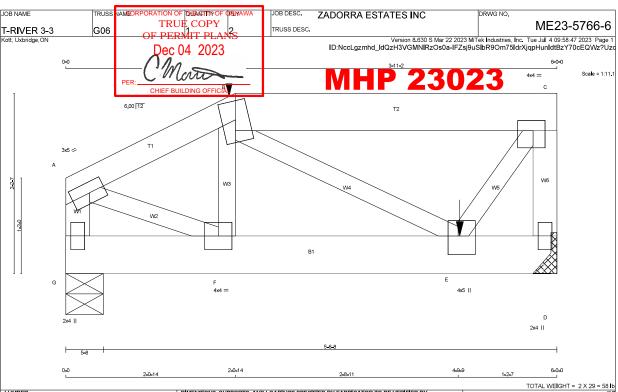


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not furst sedesjner. Breading shown is for lateral support of individual web members only. Additional reportance to stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding







LUMBER											
N. L. G. A. R	N. L. G. A. RULES										
CHORDS	SIZE		LUMBER	DESCR.							
G - A	2x4	DRY	No.2	SPF							
A - B	2x4	DRY	No.2	SPF							
B - C	2x6	DRY	No.2	SPF							
D - C	2x4	DRY	No.2	SPF							
G - D	2x6	DRY	No.2	SPF							
ALL WEBS	2x3	No.2	SPF								
DRY: SEASO	NED L	UMBER.									

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORE	S #ROWS	SURFACE	LOAD(PLF)							
		SPACING (IN)								
TOP CH	TOP CHORDS: (0.122"X3") SPIRAL NAILS									
G-A	1	12	TOP							
A-B	1	12	SIDE(0.0)							
C- D	1	12	TOP							
B-C	2	12	SIDE(5.8)							
BOTTO	M CHORDS	: (0,122"X3") SPIRAL NAILS								
G-D	2	12	SIDE(1.0)							
WEBS: (0.122"X3") SPIRAL NAILS										
2v3	1	6								

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING, REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JI.	TTPE	PLATES	vv	LEN	Y	Α.
Α	TMVW-t	MT20	3.0	5.0	1.50	2,25
В	TTVVV+m	MT20	4.0	6.0	2.75	2.00
C	TMVW-t	MT20	4.0	4.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E F	BMVVVV+t	MT20	4.0	5.0	2.75	1.50
F G	BMV/W-t	MT20	4.0	4.0		
G	BMV1+p	MT20	2.0	4.0		

PROFESSIONAL CHERMAN

100123373

ROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

QRD
G
SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX_/I	MIN. COMPO	VS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	690	502 / 0	0/0	0/0	0/0	188 / 0	0/0
D	1879	1360 / 0	0/0	0/0	0/0	518 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	ORDS	FACTO	DED		WEBS					
						MAX. FACTORED				
MEMB.	FORCE	VERT. LC	DAD LC1	MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(P	LF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
G-A	-1001 / 0	0.0	0.0	0.06(1)	7.81	A-F	0 / 1074	0.13(1)		
A-B	-1110/0	-119.4	-119.4	0.05(1)	6.25	F-B	-189 / 0	0.02(1)		
B-C	-1509 / 0	-107.9	-107.9	0.08(1)	6.25	B-E	0 / 595	0.07(1)		
D-C	-2408 / 0	0.0	0.0	0.14(1)	7,26	E-C	0 / 2654	0.33 (1)		
G-F	0/0	-16.5	-16.5	0.02(1)	10.00					
F-E	0 / 1011	-16.5	-16.5	0.19(1)	10,00					
E-D	0/0	-16.5	-16.5	0.12(1)	10.00					

FACT	FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
В	2-0-14	-24	-24		FRONT	VERT	TOTAL		C1	
l F	4-9-12	-2882	-2882		FRONT	VERT	TOTAL	_	C1	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER,
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2,00/12 MINIMUM

GIRDER TYPE: CPrimeHip LEFT SETBACK = 2-0-14 RIGHT SETBACK = 0-0 END SETBACK = 2-0-14 END WALL WIDTH = 5-3 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE: -ADDTL LOADS BASED ON 55 % OF GSL

GIRDER TYPE: CStdGirder
START DISTANCE = 0.0
START SPAN CARRIED = 2-0-0
END DISTANCE = 4-9-0
END SPAN CARRIED = 2-0-0
END WALL WIDTH = 0-0
APPLIED TO BACK SIDE OF BOTTOM CHORD.
-ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.14/1.00 (C-D:1) , BC=0.19/1.00 (E-F:1) , WB=0.33/1.00 (C-E:1) , SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL TRUSS MANUFACTURING PLANT. ONTROL IN THE

 NAIL VALUES

 PLATE
 GRIP(DRY) (PSI)
 SHEAR
 SECTION (PL)

 MAX
 MIN
 MAX
 MIN
 MAX
 MIN

 MT20
 650
 371
 1747
 788
 1987
 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

CONTINUED ON PAGE 2

TRUE COPY
G06
OF PERMIT PLANS JOB NAME T-RIVER 3-3 Kott, Uxbridge,ON Dec-04 2023

Morto

TRUSS DESC.

JOB DESC. ZADORRA ESTATES INC

DRWG NO.

ME23-5766-7

Version 8.630 S Mar 22 2023 MTek Industries. Inc. Tue Jul. 4 09:58.47 2023 Page 2 ID:NccLgzmhd_ldQzH3VGMNIRzOs0a-IFZsj9uSlbR9Om75ldrXjqpHunldtBzY70cEQWz?Uzc

MHP 230 (E) (INPUT = 0.90) (INPUT = 1.00)

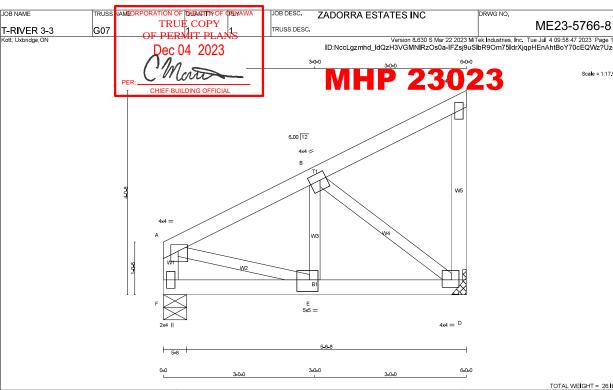


REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 08/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G. Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee
Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com





	DIM
	BU
DESCR.	BEA
SPF	
SPF	
SPF	JT
CDE	_

| N. L. G. A. RULES | CHORDS | SIZE | F - A | 2x4 | A - C | 2x4 | D - C | 2x4 | F - D | 2x4 | LUMBER No.2 No.2 No.2 No.2 ALL WEBS 2x3 DRY DRY: SEASONED LUMBER. No.2

PL	ATES	(table	is in	inches)
JT	TYP		PL	ATES

01	1111	FLAILS	V V	LLIA		^
Α	TMVW-p	MT20	4.0	4.0	1.50	1.75
В	TMWW-t	MT20	4.0	4.0	1.75	1,50
С	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		1.75
Е	BMWW-t	MT20	5.0	5.0	2,75	2,00
F	BMV1+p	MT20	2.0	4.0		

ENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY ILDING DESIGNER

	NINOS						
	FACTORED		MAXIMUN	/ FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1355	0	1355	0	0	5-8	1-8
D	1355	0	1355	0	0	MECHANIC	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS 1ST LOASE MAY MINL COMPONENT REACTIONS

	1ST LCASE	MAX_/I	MIN. COMPO	NENT REACTION	VS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	947	685 / 0	0/0	0/0	0/0	262 / 0	0/0
D	947	685 / 0	0/0	0/0	0/0	262 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 5.56 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	IORDS				W E	BS	
MA.	X. FACTORED	FACTORED	MAX, FACTORED				
MEMB.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	. FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
F-A	- 968 / 0	0.0 0.0	0.11 (1)	7.81	A-E	0 / 1140	0.28(1)
A-B	-1222 / 0	-119.4 -119.4	0.18 (1)	5.56	E-B	0 / 948	0.23(1)
B-C	-16 / 0	119.4 -119.4	0.16 (1)	6.25	B-D	-1380 / 0	0,34(1)
D-C	-144 / 0	0.0 0.0	0.04 (1)	7,81			
F-E	0/0	332.3 -332.3	0.50 (1)	10.00			
E-D	0 / 1107	332.3 332.3	0.69 (1)	10,00			

DESIGN CRITERIA

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
зот	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48,1	PSF				

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11.7-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 11.7-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADDT'L LOADS BASED ON 55 % OF GSL.

TOTAL WEIGHT = 26 lb

ME23-5766-8

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL)= L/360 (0.20")
CALCULATED VERT. DEFL (LL) = L/999 (0.03")
ALLOWABLE DEFL (TL)= L/360 (0.20")
CALCULATED VERT. DEFL (TL) = L/999 (0.05")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.69/1.00 (D-E:1) , WB=0.34/1.00 (B-D:1) , SSI=0.52/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

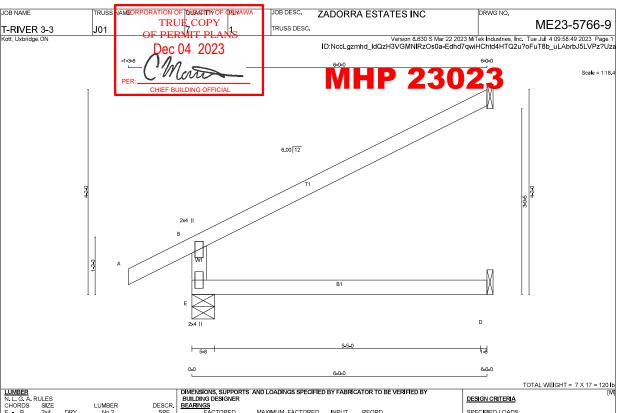
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90) JSI METAL= 0.38 (D) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DRY DRY DRY DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X 2.0 4.0 2.0 4.0 MT20 MT20

B TMV+p E BMV1+p

FACTORED MAXIMUM FACTORED INPU GROSS REACTION GROSS REACTION BRG IT VERT HORZ DOWN HORZ UPLIFT IN-5: 674 0 674 0 5-8	
T VERT HORZ DOWN HORZ UPLIFT IN-S	
	BRG
. 674 0 674 0 0 69	X IN-SX
	1-8
C 269 0 269 0 0 1-8	1-8
) 45 0 51 0 0 1 - 8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX_/I	MIN. COMPO	NENT REACTION	4S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	468	355 / 0	0/0	0/0	0/0	113 / 0	0/0
С	184	157 / 0	0/0	0/0	0/0	27 / 0	0/0
D	36	0/0	0/0	0/0	0/0	36 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

E-D

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

0/0

		R D S FACTORED	FACTORED		W	/EBS MAX. FACTO	ORED
ı	MEMB.	FORCE	VERT. LOAD LC		MAX. MEM		MAX
ı		(LBS)		CSI (LC)	UNBRAC	(LBS)	CSI (LC)
ı	FR-TO		FROM TO		LENGTH FR-T	0	
ı	E-B	-610 / 0	0.0 0.0	0.13 (4)	7.81		
ı	A-B	0 / 36	-119.4 -119.4	0.16(1)	10.00		
ı	B-C	-40 / 0	-119.4 -119.4	0.73 (1)	6.25		
ı							

-18.2 -18.2 0.13 (4) 10.00

DESIGN CRITERIA

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY)
 SHEAR
 SECTION

 (PSI)
 (PLI)
 (PLI)

 MAX
 MIN
 MAX
 MIN

 MT2
 788
 1987
 1873

PLATE PLACEMENT TOL. = 0.250 inches

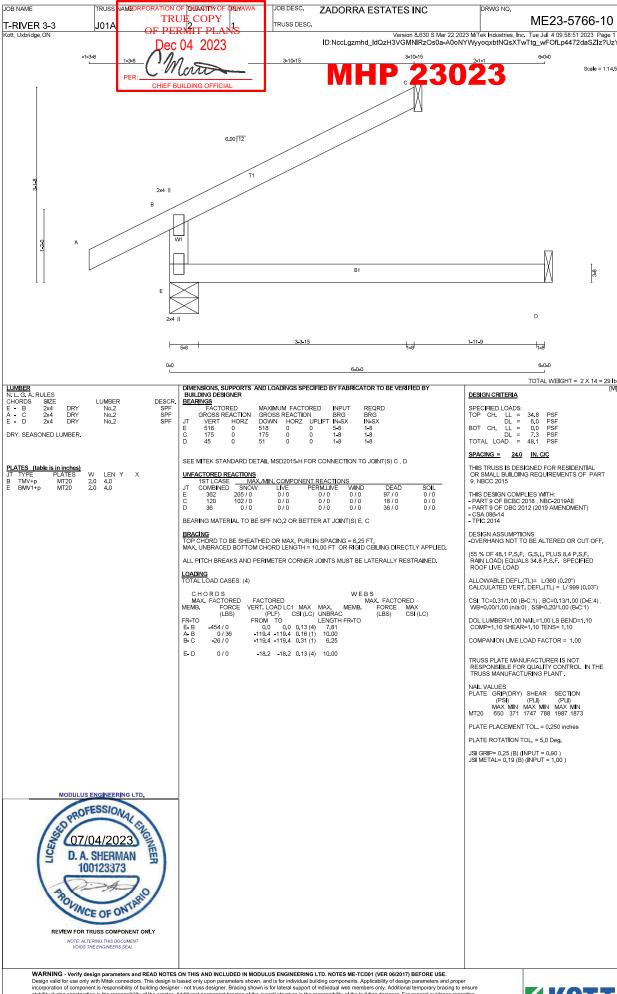
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90) JSI METAL= 0.25 (B) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding



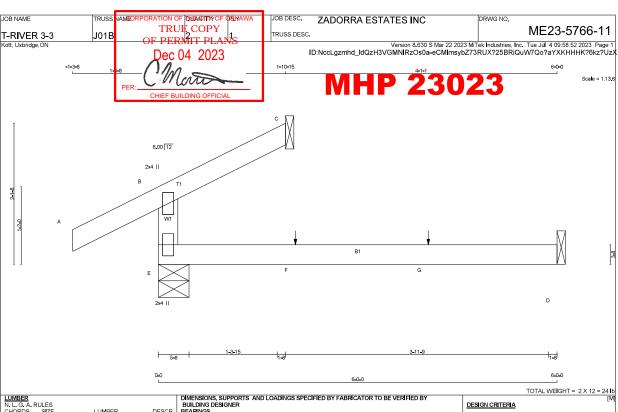


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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY DRY: SEASONED LUMBER.

PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p

<u>BEA</u>	RINGS						
	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	GROSS REACTION			BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	N-SX	IN-SX
E	368	0	368	0	0	5-8	1-8
С	86	0	86	0	0	1-8	1-8
n	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	ACTORED RE	ACTIONS					
	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	4S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	
E	259	177 / 0	0/0	0/0	0/0	82 / 0	
c	59	50 / 0	0/0	0/0	0/0	9/0	
l n	36	0 / 0	0/0	0.70	0.70	36 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

WEBS

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4) CHORDS

MAX.	FACTORED	FACTORED				MAX. FACT	ORED	
MEMB.	FORCE	VERT, LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF) (CSI (LC)	UNBRAC	;	(LBS)	CSI (L	.C)
FR-TO		FROM TO		LENGTH	FR-TO			
E-B	-304 / 0	0.0 0.0	0.13(4)	7.81				
A-B	0 / 36	-119.4 -119.4	0.16(1)	10.00				
B-C	- 12 / 0	-119.4 -119.4	0.07 (1)	6.25				
E-F	0/0	-18.2 -18.2	0.13(4)	10.00				
F-G	0/0	18.2 18.2						
G-D	0/0	-18.2 -18.2	0.13 (4)	10.00				
FACTOR	ED CONCENT	DATED LOADS (L	DO)					
		RATED LOADS (L				m.m.		
JT	LOC. LC1	MAX- MAX			DIR.	TYPE	HEEL	CONN.
	-0-12 1	1 -	 BA 			TOTAL		C1
G 4	-0-12 1	1 -	 BA 	CK VE	RT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

SOIL 0/0 0/0 0/0

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY)
 SHEAR
 SECTION

 (PSI)
 (PLI)
 (PLI)

 MAX
 MIN
 MAX
 MIN

 MT2
 788
 1987
 1873

PLATE PLACEMENT TOL. = 0.250 inches

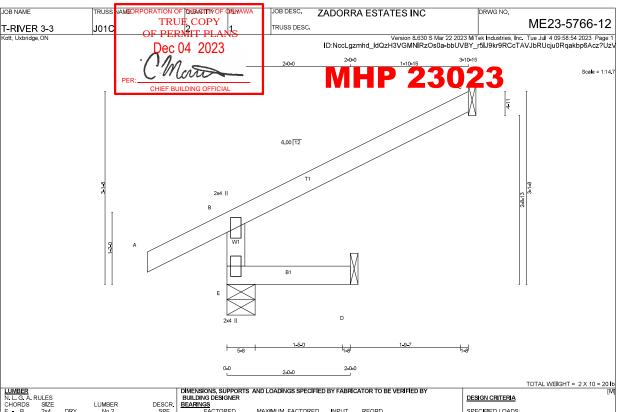
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



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incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DRY DRY DRY DRY: SEASONED LUMBER.

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN Y	X
В	TMV+p	MT20	2.0	4.0	
Ε	BMV1+p	MT20	2.0	4.0	

	VINOS						
	FACTOR		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	474	0	474	0	0	5-8	1-8
С	175	0	175	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	VENT REACTION	4S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	326	265 / 0	0/0	0/0	0/0	62 / 0	0/0
C	120	102 / 0	0/0	0/0	0/0	18 / 0	0/0
D	13	0/0	0/0	0/0	0/0	13 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

СНС	RDS			WEBS				
MAX.	FACTORED	FACTORED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC	:	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
E-B	-454 / 0	0.0 0.0	0.01(4)	7.81				
A-B	0 / 36	-119.4 -119.4	0.16(1)	10.00				
B-C	-26 / 0	119.4 -119.4	0.31(1)	6.25				
E-D	0/0	-18.2 -18.2	0.02(4)	10.00				
l .								

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA								
SPEC	IFIED	LOA	os:					
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTA	L LO	AD	=	48,1	PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
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- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY)
 SHEAR
 SECTION

 (PSI)
 (PLI)
 (PLI)

 MAX
 MIN
 MAX
 MIN

 MT2
 788
 1987
 1873

PLATE PLACEMENT TOL. = 0.250 inches

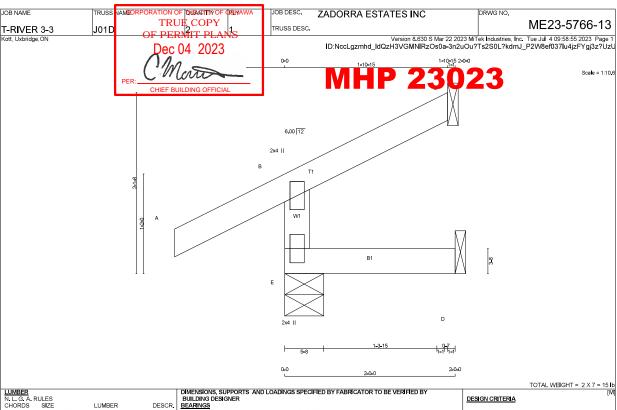
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)



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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER									
N. L. G. A. RULES									
CHORDS	SIZE		LUMBER	DESCR.					
E - B	2x4	DRY	No.2	SPF					
A - C	2x4	DRY	No.2	SPF					
E - D	2x4	DRY	No.2	SPF					

PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 2.0 4.0 GRAV/4+n MT20 2.0 4.0

DEAR	UNOS						
	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIC	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	324	0	324	0	0	5-8	1-8
С	86	0	86	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	UNFACTORED REACTIONS									
	1ST LCASE	MAX.	MIN. COMPO	VENT REACTION	VS.					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WI					
E	224	177 / 0	0/0	0/0	0 /					

SOIL 0/0 0/0 0/0 DEAD 47 / 0 9 / 0 13 / 0 C

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

СН	ORDS			WEBS			
MAX	. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	FORCE	VERT, LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	:	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-304 / 0	0.0 0.0	0.01(4)	7.81			
A-B	0 / 36	119.4 -119.4	0.16(1)	10.00			
B-C	-12 / 0	119.4 -119.4	0.07(1)	6.25			
E-D	0/0	-18.2 -18.2	0.02(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA								
SPEC	IFIED	LOA	os:					
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTA	L LO	AD	=	48.1	PSF			

SPACING = 24.0 IN. C/C

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(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY)
 SHEAR
 SECTION

 (PSI)
 (PLI)
 (PLI)

 MAX
 MIN
 MAX
 MIN

 MT2
 788
 1987
 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



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