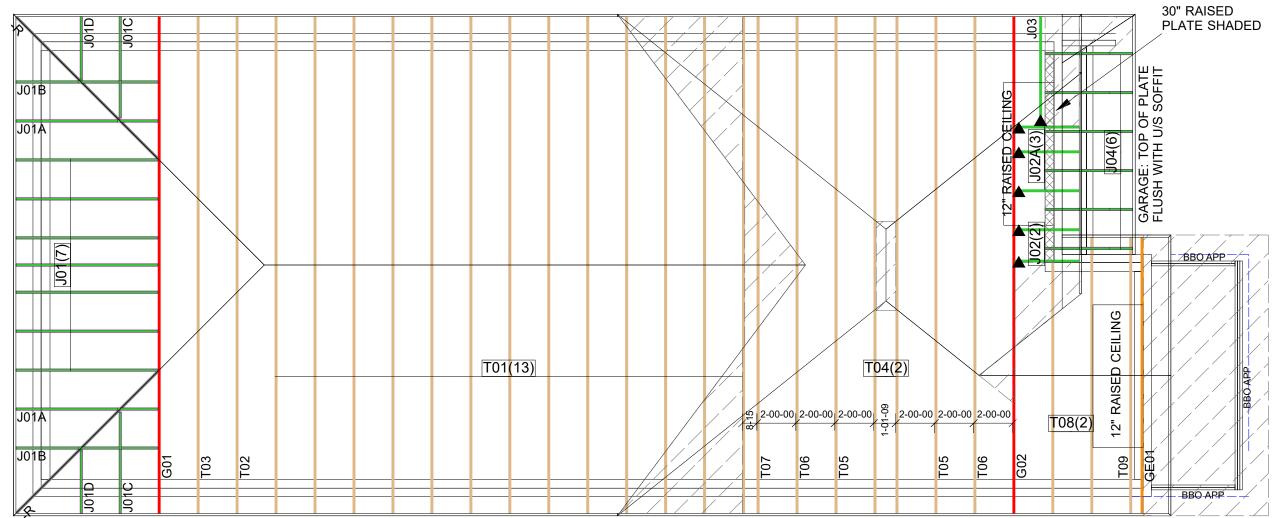


MHP 23023



UPPER LEVEL: TOP OF PLATE FLUSH WITH U/S SOFFIT

Hanger Name	Symbol	QTY
LUS24	A	6
		0



ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION						
Customer	GREENPARK GROUP					
Job #	23-00078R0					
	ZADORRA ESTATES					
Address	ZADORRA ESTATES INC					
	OSHAWA,ON					
Model	RIVER 3-1					
Sales Rep	RALPH MIRIGELLO					
Designer	LI					
Date	2023-04-26					
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 3\RIVER 3-1\T-RIVE					

DESIGN INFORMATION						
Code	NBCC 2015					
Bldg	Residential - HSB (NBCC Part 9)					
TC LL	34.8 lb/ft²					
TC DL	6.0 lb/ft²					
BC LL	0.0 lb/ft²					
BC DL	7.3 lb/ft²					
Deflection	LL=L/360 TL=L/360					
Spacing	24" O/C unless otherwise					
Spacing	noted					
Complies With	OBC 2012 (2019 Amendment) CSA 086-14 and TPIC 2014					

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400





General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



Read Carefully Prior to Manufacture and Installation

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-trangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

- 1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult_BCSI-CANADA for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)
- 2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.
- 3. Never exceed the design loading shown and never stack buil materials on inadequately braced trusses
- 4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)
- 5. Cut members to bear tightly against one another
- 6. Place plates on each face of truss at each joint and embed fully using proper roller or hydraulic press. Knots and wane at joint locations are regulated by TPIC Appendix G
- 7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC
- 8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture
- 9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in corrosive environment
- 10. Connections not shown are the responsibility of others
- 11. Do not cut or alter truss members or plates without prior approva of an engineer
- 12. Install and load vertically unless otherwise noted
- 13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient
- 14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G
- 16. Building designer must review individual component drawings to ensure they are suitable for the structure
- 15. Not designed for solar panels unless specifically noted

ME-TCD01 (VER. 06/2017)







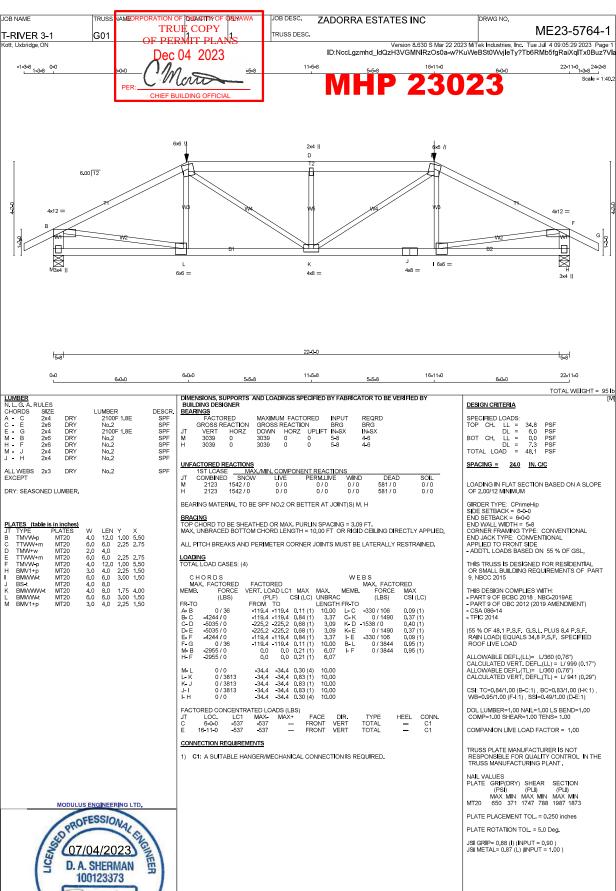


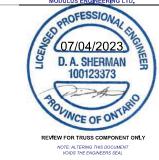












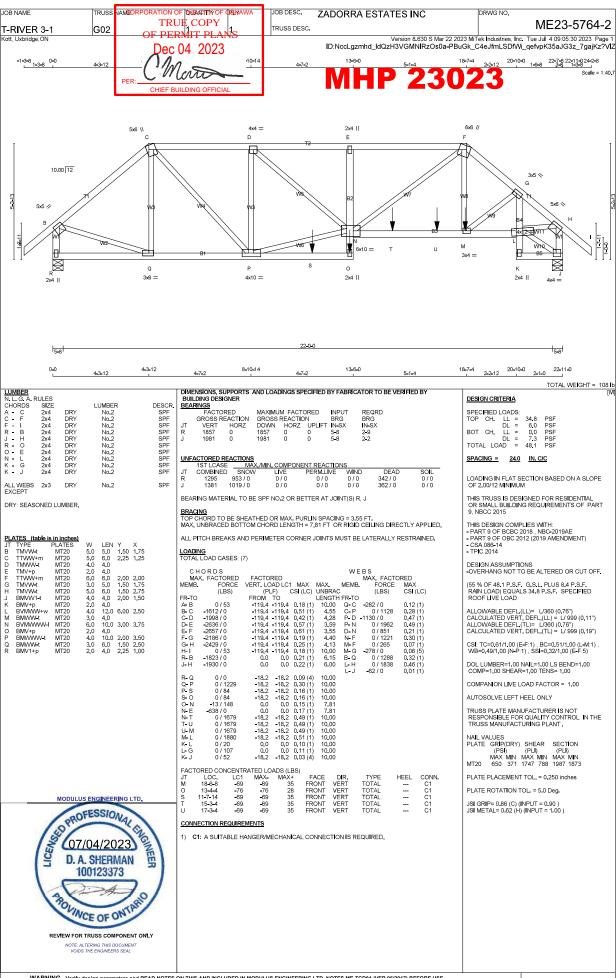
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

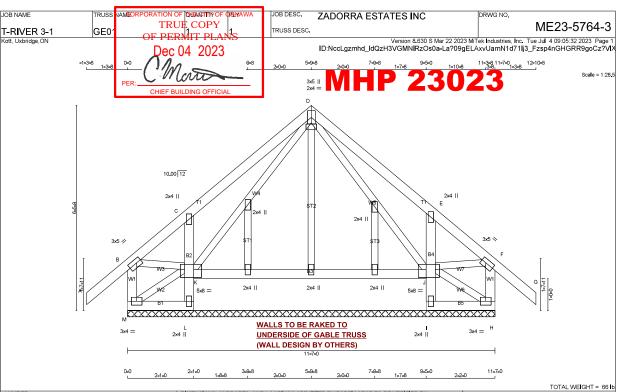




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
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LUMBER N. L. G. A. R CHORDS A - D D - G M - B H - F M - L L - C	ULES SIZE 2x4 2x4 2x4 2x4 2x4 2x4 2x4	DRY DRY DRY DRY DRY DRY	LUMBER No.2 No.2 No.2 No.2 No.2 No.2	DESCR. SPF SPF SPF SPF SPF SPF
L - C K - J I - E I - H	2x4 2x4 2x4 2x4	DRY DRY DRY DRY	No.2 No.2 No.2 No.2	SPF SPF SPF SPF
ALL WEBS ALL GABLE DRY: SEASO	2x3	DRY	No.2 No.2	SPF SPF
DIVI. SEMO	JINED E	DIVIDEN.		

GABLE STUDS SPACED AT 2-0-0 OC.

PL	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Х				
В	TMVW-t	MT20	3.0	5.0	1.50	1.75				
С	TMV+p	MT20	2.0	4.0						
D	TTWW+p	MT20	3.0	5.0						
D	NP-p	MT20	2.0	4.0	0.50	2.00				
Е	TMV+p	MT20	2.0	4.0						
F	TMVW-t	MT20	3.0	5.0	1.50	1.75				
Н	BMVW1-t	MT20	3.0	4.0						
1	BMV1+p	MT20	2.0	4.0						
J	BVMWWW-I	MT20	5.0	8.0	3.00	3.00				
K	BVMWWW-I	MT20	5.0	8.0	3,00	3.00				
L	BMV1+p	MT20	2.0	4.0						
M	BMVW1-t	MT20	3.0	4.0						
Ν,	O, P, Q, R									
N	NP+w	MT20	2.0	4.0						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS BEACHURED MAYIMUM FACTORED NIGHT BEODD

<u>ΞΑ</u>	RINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS RI	EACTION	GROSS	REACTIO	N	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	595	0	595	0	0	11-7-0 (2-2-0	1-8
	595	0	595	0	0	11-7-0 (2-2-0	1-8
	364	0	364	0	0	11-7-0 (2-2-0	1-8
	372	0	372	0	0	11-7-0 (2-2-0	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNF	ACTURED RE	ACTIONS					
	1ST LCASE	MAX./	MIN. COMPO	VENT REACTION	VS.		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	416	297 / 0	0/0	0/0	0/0	119/0	0/0
1	416	297 / 0	0/0	0/0	0/0	120 / 0	0/0
M	251	200 / 0	0/0	0/0	0/0	51 / 0	0/0
Н	257	205 / 0	0/0	0/0	0/0	52 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, I, M, H

JT L I

BRACINS
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6,25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CHUNG DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	ORDS					WE	BS	
MAX.	FACTORED	FACTOR	RED				MAX, FACTO	RED
MEMB.	FORCE	VERT. LO	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLI	F) (CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A-B	0 / 53	119.4	119.4	0.16(1)	10.00	K-D	-164 / 0	0.12(1)
B-C	-107 / 0	119.4	119.4	0.21(1)	6,25	D-J	-167 / 0	0.12(1)
C-D	-122 / 0	119.4	119.4	0.22(1)	6.25	B-K	0/94	0.02(1)
D-E	-120 / 0	119.4	-119.4	0.21(1)	6.25	M-K	-2/0	0.00(1)
E-F	-108 / 0	119.4	119.4	0.20(1)	6,25	J - F	0/92	0.02(1)
F-G	0 / 53	119.4	119.4	0.16(1)	10.00	J- H	-2/0	0.00(1)
M-B	-345 / 0	0.0	0.0	0.04(1)	7.81			
H-F	-353 / 0	0.0	0.0	0.04(1)	7.81			
M-L	0/2			0.02(4)				
L-K	- 577 / 0	0.0						
K-C	-393 / 0	0.0		0.00(1)				
	0 / 172	-18.2	-18.2	0.32(4)	10.00			
I-J	- 576 / 0	0.0	0.0	0.01(1)	7.81			
J-E	-387 / 0	0.0		0.00(1)				
I- H	0/2	-18.2	-18.2	0.02(4)	10.00			

DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-D:1) , BC=0.32/1.00 (J-K:4) , WB=0.12/1.00 (D-J:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

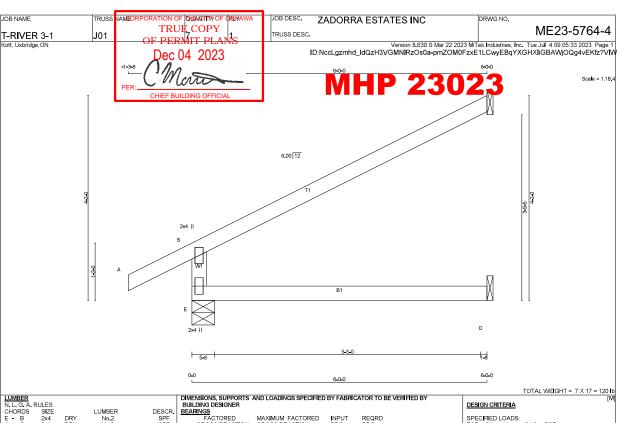
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (H) (INPUT = 0.90) JSI METAL= 0.21 (C) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mites connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of the property parameters and proper
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X 2.0 4.0 2.0 4.0

MT20 MT20

B TMV+p E BMV1+p

FACTO		MAXIMU	M FACT	ORED	INPUT	REQRI
GROSS R	EACTION	GROSS	REACTION	N	BRG	BRG
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
674	0	674	0	0	5-8	1-8
269	0	269	0	0	1-8	1-8
45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

 UNFACTORED REACTIONS

 1ST LCASE
 MAX./MIN. COMPONENT REACTIONS

 JT
 COMBINED
 SNOW
 LIVE
 PERM.LIVE
 WIND
 DEAD 113 / 0 27 / 0 36 / 0 355 / 0 157 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS	WEBS					
MA>	. FACTORED	FACTORED			MA	X. FACTO	RED
MEMB.	FORCE	VERT. LOAD L	C1 MAX	MAX. M	EMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH FF	R-TO		
E-B	-610/0		0 0.13 (4)				
A-B	0 / 36	119.4 -119.	4 0.16 (1	10.00			
B-C	-40 / 0	119.4 -119	4 0.73 (1)	6.25			

SPACING = 24.0 IN. C/C

SOIL 0/0 0/0 0/0

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

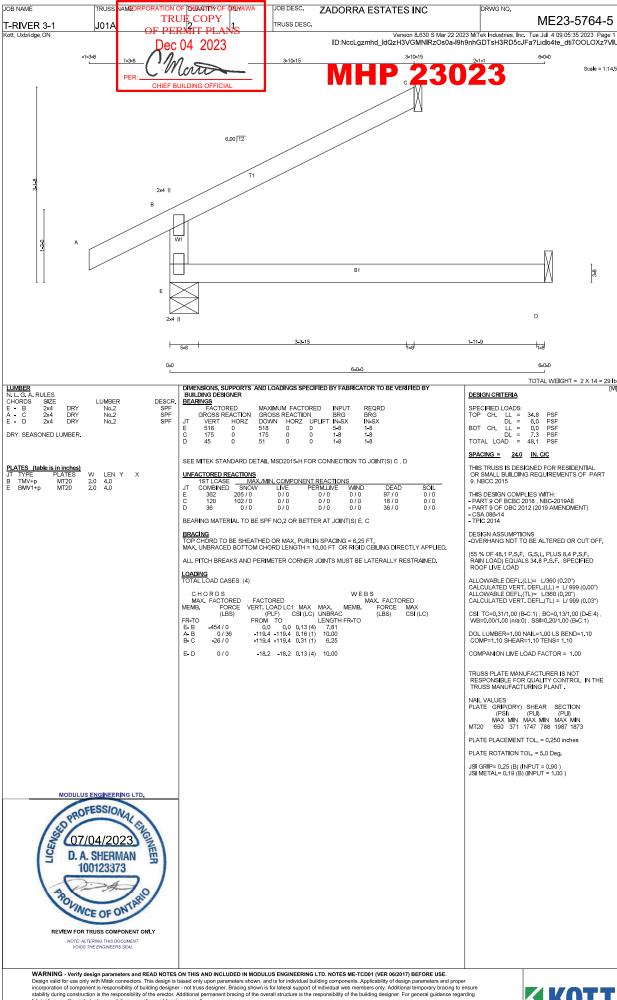
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90) JSI METAL= 0.25 (B) (INPUT = 1.00)

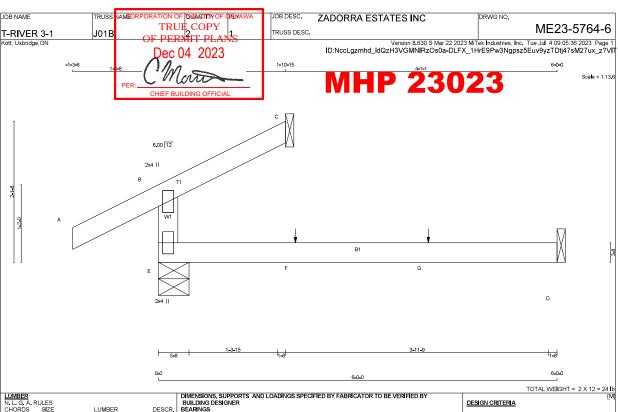


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding









LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
DRY: SEAS	ONED L	JMBER.		

 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES
 W
 LEN Y
 X

 B
 TMV+p
 MT20
 2.0
 4.0

 GRAV/4+n
 MT20
 2.0
 4.0

DEA	RINGS						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	368	0	368	0	0	5-8	1-8
С	86	0	86	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	ACTORED RE	<u>ACTIONS</u>				
	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	VS.	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	
E	259	177 / 0	0/0	0/0	0/0	8
l c	59	50 / 0	0/0	0/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHC	ORDS				WE	BS		
MAX.	FACTORED	FACTORED				MAX, FACT	ORED	
мемв.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO			
E-B	-304 / 0	0.0 0.0	0.13(4)	7.81				
A-B	0 / 36	-119.4 -119.4	0.16(1)	10.00				
B-C	-12 / 0	-119.4 -119.4	0.07 (1)	6.25				
E-F	0/0	18.2 -18.2	0.13 (4)	10.00				
F-G	0/0	-18.2 -18.2	0.13(4)	10.00				
G-D	0/0	-18.2 -18.2	0.13 (4)	10,00				
FACTOR	ED CONCENT	TRATED LOADS (L	BS)					
	LOC LC			ACE [OIR.	TYPE	HEEL	CONN
	0.12	1 1				TOTAL		C1
						TOTAL		01

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

SOIL 0/0 0/0 0/0

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

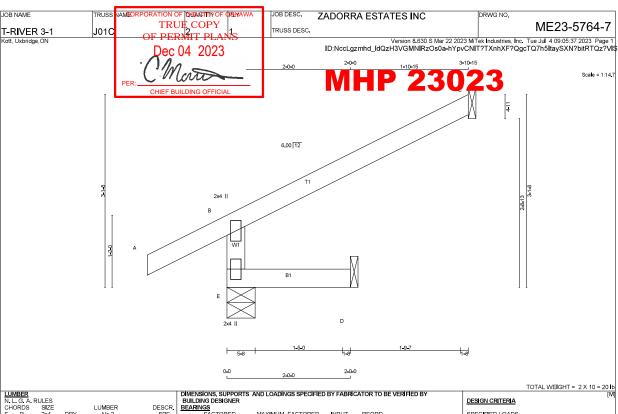
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DRY DRY DRY DRY: SEASONED LUMBER.

JΤ	TYPE	PLATES	W	LEN Y	Х
В	TMV+p	MT20	2.0	4.0	
Е	BMV1+p	MT20	2.0	4.0	

<u> </u>	NINOS						
	FACTO			M FACT		INPUT	REQRD
	GROSS RI	EACTION	GROSS	REACTIC	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	474	0	474	0	0	5-8	1-8
2	175	0	175	0	0	1-8	1-8
)	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	151 LUASE	IVIAA./	MIN. COMPO	VENT REACTION	VS.		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	326	265 / 0	0/0	0/0	0/0	62 / 0	0/0
C	120	102 / 0	0/0	0/0	0/0	18 / 0	0/0
D	13	0/0	0/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

СНС	RDS			w	EBS		
MAX.	FACTORED	FACTORED			MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX. MEME	B. FORCE	MAX	
	(LBS)		CSI (LC)	UNBRAC	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO)		
E-B	-454 / 0		0.01 (4)				
A-B	0 / 36	-119.4 -119.4					
B-C	-26 / 0	119.4 -119.4	0.31 (1)	6.25			
E-D	0/0	-18.2 -18.2	0.02(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA								
SPEC	IFIED	LOA	os:					
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTA	L LO	AD	=	48.1	PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

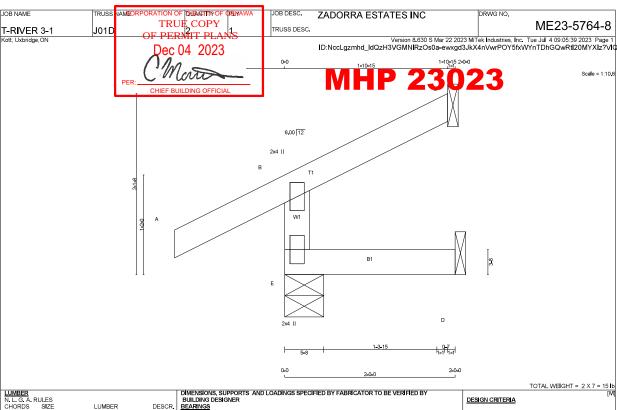
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)



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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DRY DRY DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches) JT TYPE PLATES W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p

<u> </u>	NINGS						
	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	324	0	324	0	0	5-8	1-8
С	86	0	86	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPO	VENT REACTION	4S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	224	177 / 0	0/0	0/0	0/0	47 / 0	0/0
l c	59	50/0	0/0	0/0	0/0	9/0	0/0
D	13	0/0	0/0	0/0	0/0	13 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

СН	ORDS				WE	BS	
MAX	. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	FORCE	VERT, LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	:	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-304 / 0	0.0 0.0	0.01(4)	7.81			
A-B	0 / 36	119.4 -119.4	0.16(1)	10.00			
B-C	-12 / 0	119.4 -119.4	0.07(1)	6.25			
E-D	0/0	-18.2 -18.2	0.02(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA									
SPEC	IFIED	LOA	os:						
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48,1	PSF				

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

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- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

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(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

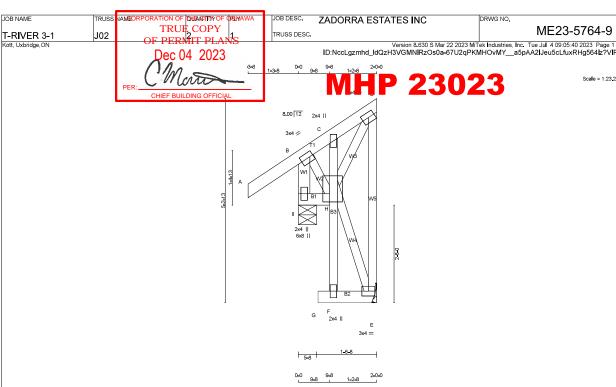
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





LUMBER				
N. L. G. A. R				
CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEAS	ONED L	JMBER.		

JT	TYPE	PLATES	W	LEN	Υ	X
В	TMVW-t	MT20	3.0	4.0	1.50	1.00
С	TMV+p	MT20	2.0	4.0		
D	TMVW-t	MT20	3.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMV+p	MT20	2.0	4.0		
Н	BVMWWW+I	MT20	6.0	8.0	2,50	2.00
1	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEAL	RINGS						
	FACTOR	MAXIMUM FACTORED			INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
ı	362	0	362	0	0	5-8	1-8
E	85	0	85	0	-30	MECHANIC	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS								
	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	VS.			
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL	
1	250	198 / 0	0/0	0/0	0/0	52 / 0	0/0	
E	60	37 / -33	0/0	0/0	0/0	24/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

<u>BRACING</u>
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT,
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

	RDS					WE		
	FACTORED	FACTO					MAX. FACTO	
MEMB.	FORCE	VERT, LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PI	_F) (CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TO		FROM			LENGTH	FR-TO		
I- B	-354 / 0	0.0	0.0	0.04(1)	7.81	B- H	- 65 / 10	0.01 (5)
A-B	0 / 45	119.4	-119.4	0.16(1)	10,00	H-E	0/2	0.00 (5)
B-C	- 78 / 0	119.4	-119.4	0.15(1)	6,25	H-D	-81 / 16	0.01 (5)
C-D	- 7 / 45	-119.4	-119.4	0.03(1)	10.00			
E- D	- 74 / 36	0.0	0.0	0.05 (1)	7.81			
I- H	0/0	-18.2	10 7	0.00 (4)	10.00			
F-H	0 / 19	0.0		0.00 (4)				
H-C	0 / 143	0.0		0.04 (5)				
G-F	0/0	-18.2	-18.2	0.00(4)	10,00			
F-E	0/0	-18.2	-18.2	0.01(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

TOTAL WEIGHT = 2 X 20 = 40 lb

DESIGN CRITERIA								
SPECIFI	ED	LOA	os:					
TOP C	Н.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT C	Η.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTAL	LO.	AD	=	48.1	PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.04/1.00 (C-H:5) , WB=0.01/1.00 (D-H:5) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

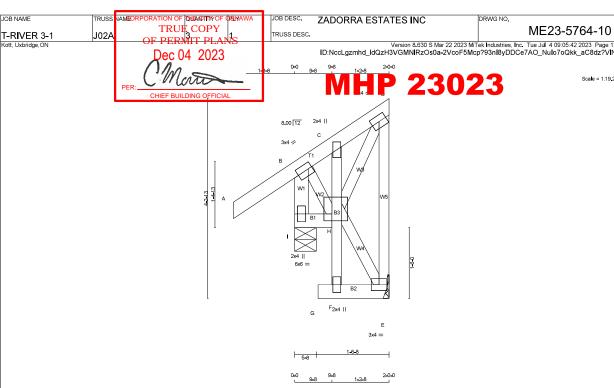
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90) JSI METAL= 0.07 (I) (INPUT = 1.00)



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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASO	DNED L	UMBER.		

LEN Y X 4.0 1.50 1.00 4.0 4.0 4.0 4.0 6.0 1.75 2.00 4.0 W 3.0 2.0 3.0 3.0 2.0 6.0 2.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	divos						
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION .	GROSS REACTION			BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	362	0	362	0	0	5-8	1-8
Ε.	85	0	85	0	-30	MECHANIC	AL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS MAY MAIN COMPONENT REACTIONS

	151 LCASE	IVIAX./I	VIIN. COMPO	NEINT REACTION	V5		
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
1	250	198 / 0	0/0	0/0	0/0	52 / 0	0/0
E	60	37 / -33	0/0	0/0	0/0	24/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

<u>BRACING</u>
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT,
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

	ORDS					WE		
MAX	. FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT, LO	DAD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pi	LF)	CSI (LC)	UNBRAG		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		, ,
I- B	-354 / 0	0.0	0.0	0.04(1)	7.81	B- H	-65 / 10	0.01(5)
A-B	0 / 45	119.4	-119.4	0.16(1)	10,00	H-E	0/2	0.00(5)
B-C	-78 / 0	119.4	-119.4	0.15(1)	6,25	H-D	-80 / 16	0.01(5)
C-D	- 7 / 45	-119.4	-119.4	0.03(1)	10.00			
E-D	-74 / 36	0.0	0.0	0.03(1)	7.81			
I- H	0/0	-18.2		0.00(4)				
F-H	0 / 19	0.0	0.0	0.01(4)	10.00			
H- C	0 / 143	0.0	0.0	0.04 (5)	10.00			
G-F	0/0	-18.2	-18.2	0.00(4)	10,00			
F-E	- 1/0	-18.2	-18.2	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

TOTAL WEIGHT = 3 X 18 = 53 lb

DESIGN CRITERIA								
SPEC	IFIED	LOA	os:					
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTA	L LO	AD	=	48.1	PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

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ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.04/1.00 (C-H:5) , WB=0.01/1.00 (D-H:5) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90) JSI METAL= 0.07 (I) (INPUT = 1.00)



NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

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