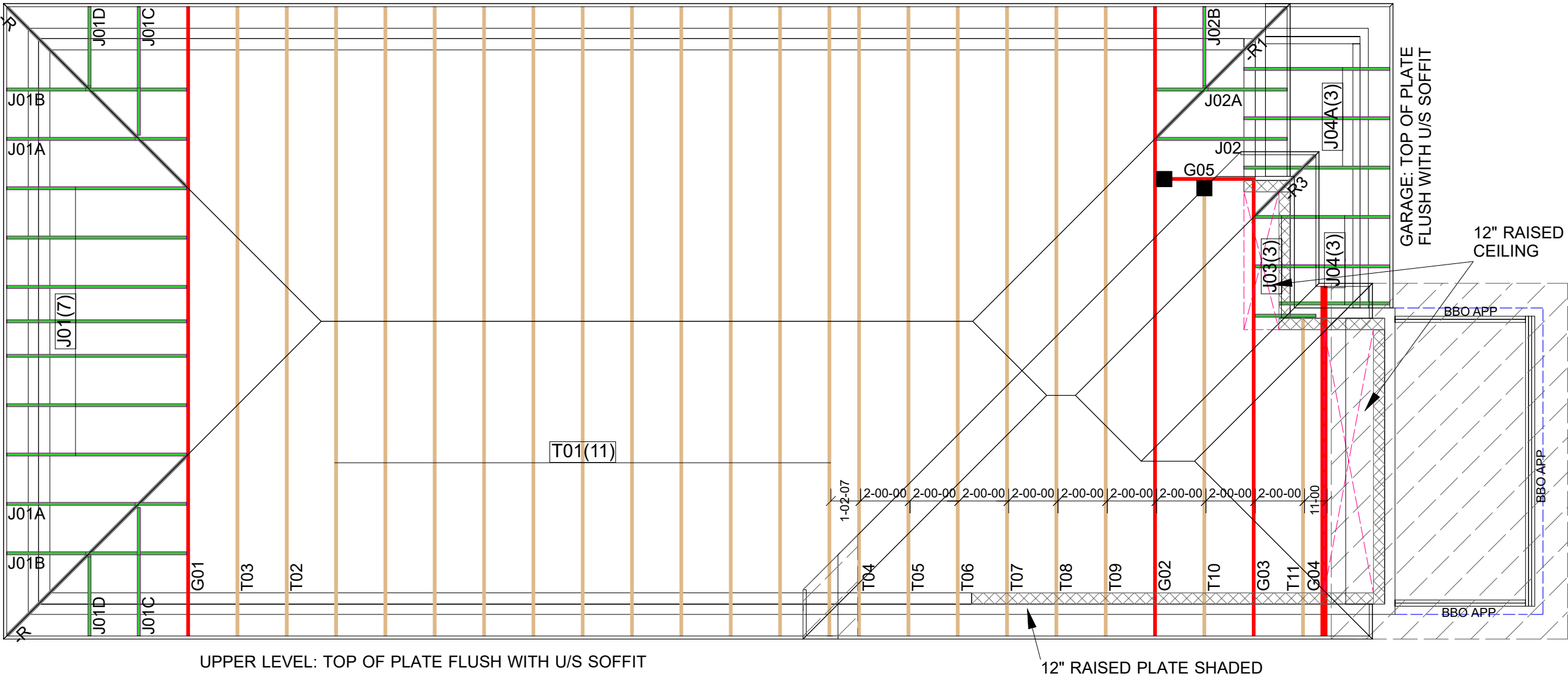


MHP 23022



Hanger Name	Symbol	QTY
	▲	0
LJS26DS	■	2



CONVENTIONAL  
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK GROUP
Job #	23-00075R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC OSHAWA,ON
Model	RIVER 2-3
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-04-24
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 2\RIVER 2-3\T-RIVE

DESIGN INFORMATION

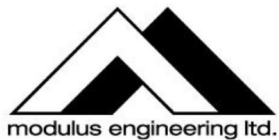
Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft²
TC DL	6.0 lb/ft²
BC LL	0.0 lb/ft²
BC DL	7.3 lb/ft²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

- Hangers and Fasteners to be installed as per manufacturer
- Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes
- For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines
- Read all notes on this page in addition to those shown on the KOTT Truss Engineering package
- Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering
- Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.
- Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.  
14 Anderson Blvd.  
Uxbridge, ON  
905.642.4400





## General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



**Read Carefully Prior  
Manufacture and Installation**

**Note:** It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

### Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

### Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

### Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

### Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

### Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

### Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

### Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

### Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

### Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

### Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

### Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

### Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

### Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

**Failure to follow these guidelines could cause property damage and personal injury**

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from [www.sbcindustry.com](http://www.sbcindustry.com))
2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.
3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses
4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)
5. Cut members to bear tightly against one another
6. Place plates on each face of truss at each joint and embed fully using proper roller or hydraulic press. Knots and wane at joint locations are regulated by TPIC Appendix G
7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC
8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture
9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment
10. Connections not shown are the responsibility of others
11. Do not cut or alter truss members or plates without prior approval of an engineer
12. Install and load vertically unless otherwise noted
13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient
14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G
16. Building designer must review individual component drawings to ensure they are suitable for the structure
15. Not designed for solar panels unless specifically noted

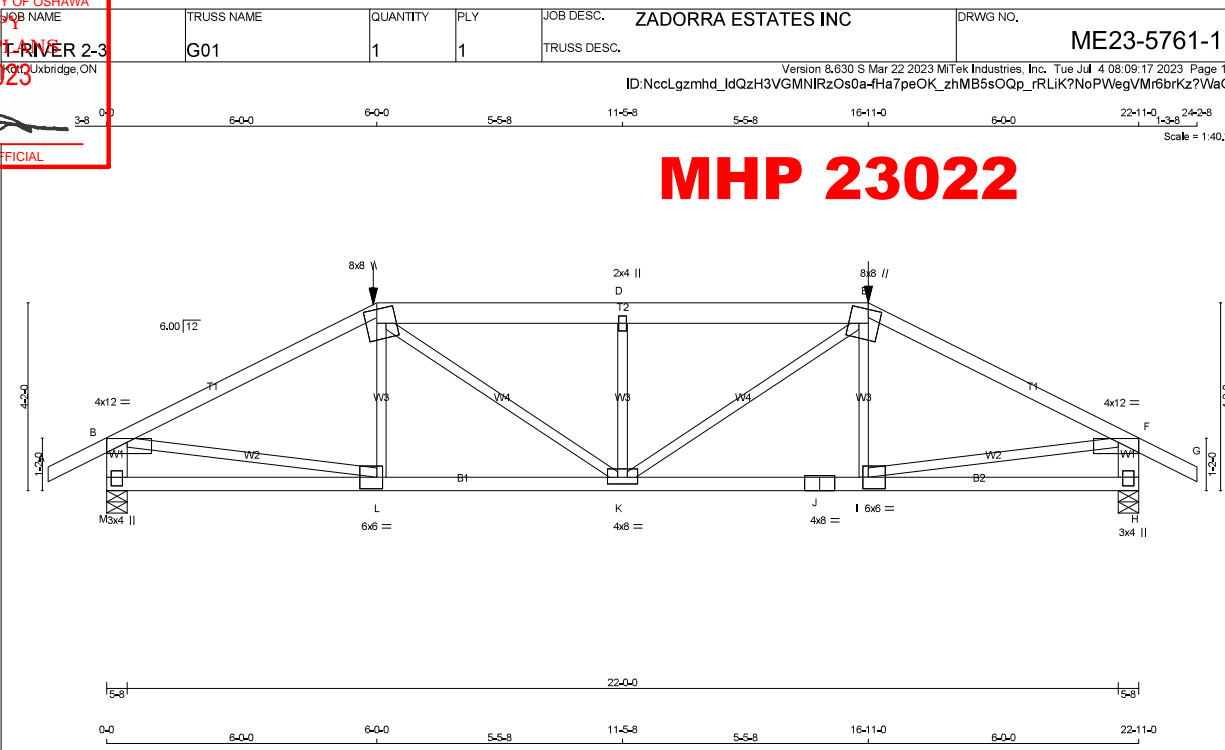
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Nov 23 2023

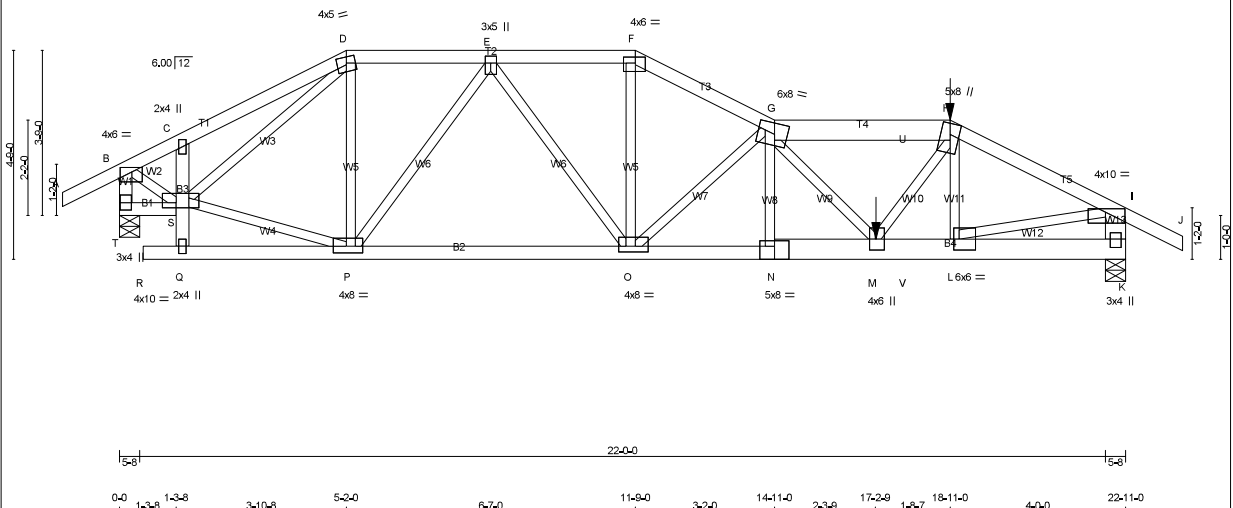
PER: 

CHIEF BUILDING OFFICIAL



<b>LUMBER</b> N, L, G, A, RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY 2100F 1,8E SPF C - E 2x6 DRY No.2 SPF E - G 2x4 DRY 2100F 1,8E SPF M - B 2x6 DRY No.2 SPF H - F 2x6 DRY No.2 SPF M - J 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF  ALL WEBS 2x3 DRY No.2 SPF EXCEPT  DRY, SEASONED LUMBER.		<b>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</b> <b>BEARINGS</b> FACTORED GROSS REACTION DOWN VERT 3039 0 3039 0 0 5-8 4-6 MAXIMUM FACTORED GROSS REACTION UP LIFT 0 0 0 5-8 4-6 INPUT BRG IN-SX 5-8 4-6 REQRD BRG IN-SX 5-8 4-6  <b>UNFACTORED REACTIONS</b> 1ST LOASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL M 2123 1542 / 0 0 / 0 0 / 0 581 / 0 0 / 0 H 2123 1542 / 0 0 / 0 0 / 0 581 / 0 0 / 0  BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H  <b>BRACING</b> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT. MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.  <b>LOADING</b> TOTAL LOAD CASES: (4) <table><tr><th colspan="2">C H O R D S</th><th colspan="2">W E B S</th></tr><tr><th>MEMB.</th><th>FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRACED LENGTH (FT)</th></tr><tr><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 36</td><td>-119.4 -119.4</td><td>0.11 (1)</td></tr><tr><td>B-C</td><td>-4271 / 0</td><td>-119.4 -119.4</td><td>0.84 (1)</td></tr><tr><td>C-D</td><td>-5031 / 0</td><td>-225.2 -225.2</td><td>0.68 (1)</td></tr><tr><td>D-E</td><td>-5031 / 0</td><td>-225.2 -225.2</td><td>0.68 (1)</td></tr><tr><td>E-F</td><td>-4271 / 0</td><td>-119.4 -119.4</td><td>0.84 (1)</td></tr><tr><td>F-G</td><td>0 / 36</td><td>-119.4 -119.4</td><td>0.11 (1)</td></tr><tr><td>M-B</td><td>-2952 / 0</td><td>0 0</td><td>0.21 (1)</td></tr><tr><td>H-F</td><td>-2952 / 0</td><td>0 0</td><td>0.21 (1)</td></tr><tr><td>M-L</td><td>0 / 0</td><td>-34.4 -34.4</td><td>0.32 (4)</td></tr><tr><td>L-K</td><td>0 / 3820</td><td>-34.4 -34.4</td><td>0.83 (1)</td></tr><tr><td>K-J</td><td>0 / 3820</td><td>-34.4 -34.4</td><td>0.83 (1)</td></tr><tr><td>J-I</td><td>0 / 3820</td><td>-34.4 -34.4</td><td>0.83 (1)</td></tr><tr><td>I-H</td><td>0 / 0</td><td>-34.4 -34.4</td><td>0.32 (4)</td></tr></table> FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. C 6-0-0 -537 -537 - FRONT VERT TOTAL - C1 E 16-11-0 -537 -537 - FRONT VERT TOTAL - C1		C H O R D S		W E B S		MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	FR-TO				A-B	0 / 36	-119.4 -119.4	0.11 (1)	B-C	-4271 / 0	-119.4 -119.4	0.84 (1)	C-D	-5031 / 0	-225.2 -225.2	0.68 (1)	D-E	-5031 / 0	-225.2 -225.2	0.68 (1)	E-F	-4271 / 0	-119.4 -119.4	0.84 (1)	F-G	0 / 36	-119.4 -119.4	0.11 (1)	M-B	-2952 / 0	0 0	0.21 (1)	H-F	-2952 / 0	0 0	0.21 (1)	M-L	0 / 0	-34.4 -34.4	0.32 (4)	L-K	0 / 3820	-34.4 -34.4	0.83 (1)	K-J	0 / 3820	-34.4 -34.4	0.83 (1)	J-I	0 / 3820	-34.4 -34.4	0.83 (1)	I-H	0 / 0	-34.4 -34.4	0.32 (4)
C H O R D S		W E B S																																																																	
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<b>PLATES (table is in inches)</b> JT TYPE PLATES W LEN Y X B TMVW-p MT20 4.0 12.0 1.00 5.50 C TTVW+m MT20 8.0 8.0 4.25 2.75 D TMW+w MT20 2.0 4.0 E TTVW+m MT20 8.0 8.0 4.25 2.75 F TMVW-p MT20 4.0 12.0 1.00 5.50 H BMV1+p MT20 3.0 4.0 2.25 1.50 I BMWVW4 MT20 6.0 6.0 3.00 1.50 J BS4 MT20 4.0 8.0 K BMWVW4 MT20 4.0 8.0 1.75 4.00 L BMWVW4 MT20 6.0 6.0 3.00 1.50 M BMV1+p MT20 3.0 4.0 2.25 1.50		<b>DESIGN CRITERIA</b> <b>SPECIFIED LOADS:</b> TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF  <b>SPACING = 24.0 IN. GIG</b>  LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM  GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE -ADDTL LOADS BASED ON 55 % OF GSL.  THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015  THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014  (55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD  ALLOWABLE DEFL.(LL)= L/360 (0.76") CALCULATED VERT. DEFL.(LL) = L/999 (0.17") ALLOWABLE DEFL.(TL)= L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/938 (0.29") CSI TC=0.84/1.00 (B-C:1), BC=0.83/1.00 (I-K:1), WB=0.96/1.00 (F-1), SSI=0.49/1.00 (D-E:1)  DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00  COMPANION LIVE LOAD FACTOR = 1.00  TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.  NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (FUI) (FUI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873  PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.  JSI GRIP= 0.89 (L) (INPUT = 0.90 ) JSI METAL= 0.87 (L) (INPUT = 1.00 )																																																																	
<b>CONNECTION REQUIREMENTS</b> 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.																																																																			

# MHP 23022



<b>LUMBER</b> N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF B - F 2x4 DRY No.2 SPF F - G 2x4 DRY No.2 SPF G - H 2x6 DRY No.2 SPF H - J 2x4 DRY No.2 SPF T - B 2x4 DRY No.2 SPF K - I 2x6 DRY No.2 SPF T - S 2x4 DRY No.2 SPF Q - C 2x4 DRY No.2 SPF R - N 2x4 DRY No.2 SPF N - K 2x6 DRY No.2 SPF  ALL WEBS 2x3 DRY No.2 SPF EXCEPT  DRY: SEASONED LUMBER.	<b>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</b> <b>BEARINGS</b> FACTORED GROSS REACTION DOWN UP JT 2128 0 2128 0 0 5-8 3-7 K 2936 0 2936 0 0 5-8 4-1  <b>UNFACTORED REACTIONS</b> 1ST LOASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL T 1486 1083 / 0 0 / 0 0 / 0 403 / 0 0 / 0 K 2049 1496 / 0 0 / 0 0 / 0 554 / 0 0 / 0  BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K  <b>BRACING</b> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.  ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.  <b>LOADING</b> TOTAL LOAD CASES: (4)  C H O R D S W E B S MEMB. MAX. FACTORED VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FACTORED FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 36 -119.4 -119.4 0.17 (1) 10.00 S-P 0 / 1804 0.45 (1) B-C -1809 / 0 -119.4 -119.4 0.22 (1) 4.61 S-D -306 / 0 0.13 (1) C-D -1835 / 0 -119.4 -119.4 0.31 (1) 4.59 P-D 0 / 668 0.17 (1) D-E -1867 / 0 -119.4 -119.4 0.25 (1) 4.61 P-E -1248 / 0 0.65 (1) E-F -2690 / 0 -119.4 -119.4 0.33 (1) 3.69 E-O 0 / 681 0.17 (1) F-G -3330 / 0 -119.4 -119.4 0.39 (1) 3.44 O-F 0 / 1167 0.29 (1) G-U -4616 / 0 -119.4 -119.4 0.30 (1) 3.77 O-G -2578 / 0 0.82 (1) U-H -4616 / 0 -165.5 -165.5 0.30 (1) 3.77 N-G 0 / 87 0.02 (1) H-I -3981 / 0 -119.4 -119.4 0.66 (1) 2.86 G-M -464 / 0 0.11 (1) I-J 0 / 36 -119.4 -119.4 0.17 (1) 10.00 M-H 0 / 1933 0.48 (1) T-B -2115 / 0 0.0 0.0 0.23 (1) 5.78 L-H -603 / 0 0.11 (1) K-I -2868 / 0 0.0 0.0 0.20 (1) 6.15 B-S 0 / 1990 0.49 (1) L-I 0 / 3642 0.90 (1)  T-S 0 / 0 -18.2 -18.2 0.01 (4) 10.00 Q-S 0 / 42 0.0 0.0 0.17 (1) 10.00 S-C -308 / 0 0.0 0.0 0.16 (1) 7.81 R-Q 0 / 0 -18.2 -18.2 0.02 (4) 10.00 Q-P 0 / 106 -18.2 -18.2 0.14 (4) 10.00 P-O 0 / 2594 -18.2 -18.2 0.53 (1) 10.00 O-N 0 / 4914 -18.2 -18.2 0.93 (1) 10.00 N-M 0 / 4913 -18.2 -18.2 0.77 (1) 10.00 M-V 0 / 3595 -18.2 -18.2 0.59 (1) 10.00 V-L 0 / 3595 -25.3 -25.3 0.59 (1) 10.00 L-K 0 / 0 -25.3 -25.3 0.07 (1) 10.00  FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. H 18-11-0 -199 -199 - FRONT VERT TOTAL - C1 M 17-2-12 -1288 -1288 - FRONT VERT TOTAL - C1	<b>DESIGN CRITERIA</b> *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE  SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF  <b>SPACING = 24.0 IN. G/C</b>  LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM  GIRDER TYPE: CPrimeHip LEFT SETBACK = 5-2-0 RIGHT SETBACK = 4-0-0 END SETBACK = 4-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF G.S.L. LOADS APPLIED TO FIRST 5-0-0 OF SPAN MEASURED FROM THE RIGHT.  *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.  THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015  THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014  DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF.  (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD  ALLOWABLE DEFL.(LL)= L/360 (0.76") CALCULATED VERT. DEFL.(LL) = U/999 (0.16") ALLOWABLE DEFL.(TL)= L/360 (0.76") CALCULATED VERT. DEFL.(TL) = U/999 (0.27")  CSI: TC=0.66/1.00 (H+1), BC=0.93/1.00 (N+0.1), WB=0.90/1.00 (L-1), SS=0.20/1.00 (D+E-1)  DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00  COMPANION LIVE LOAD FACTOR = 1.00  AUTOSOLVE RIGHT HEEL ONLY  TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.  NAIL VALUES PLATE GRP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873  PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.
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MHP 23022



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PER: 

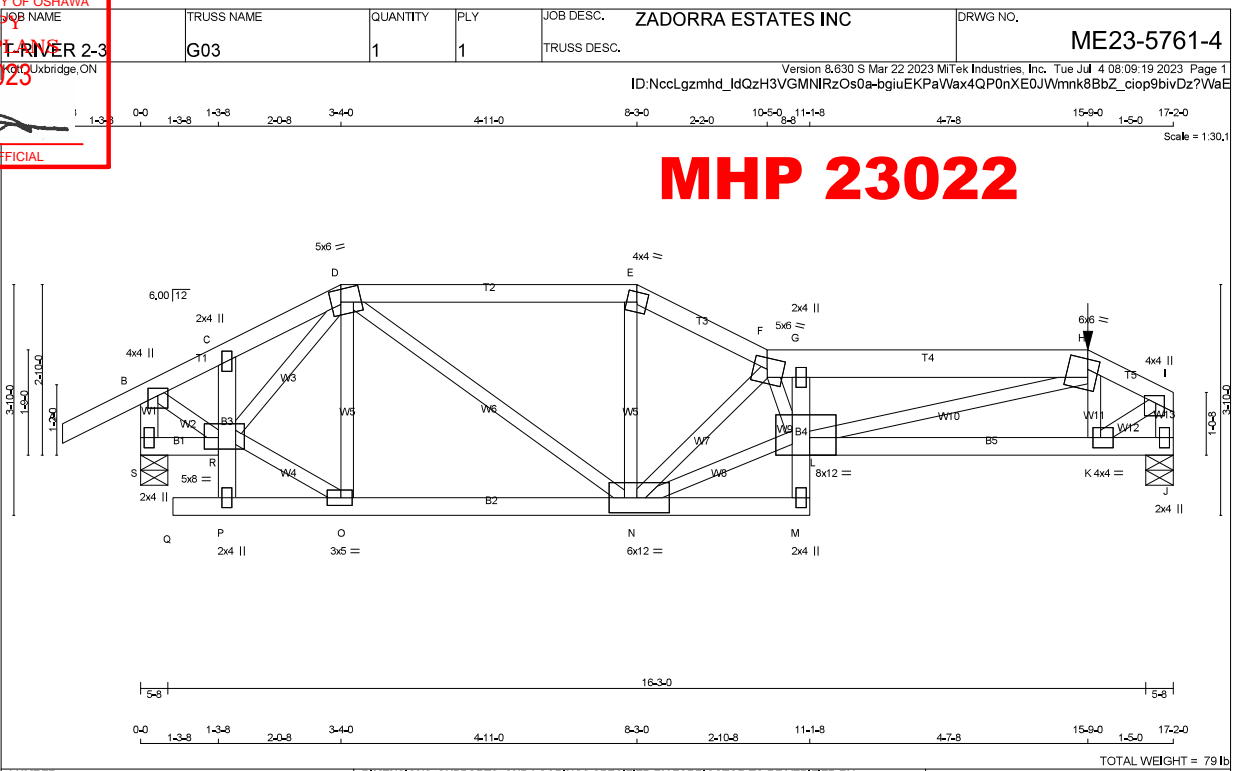
CHIEF BUILDING OFFICIAL

PROJECT NAME

RIVER 2-3

LOCATION

Uxbridge, ON



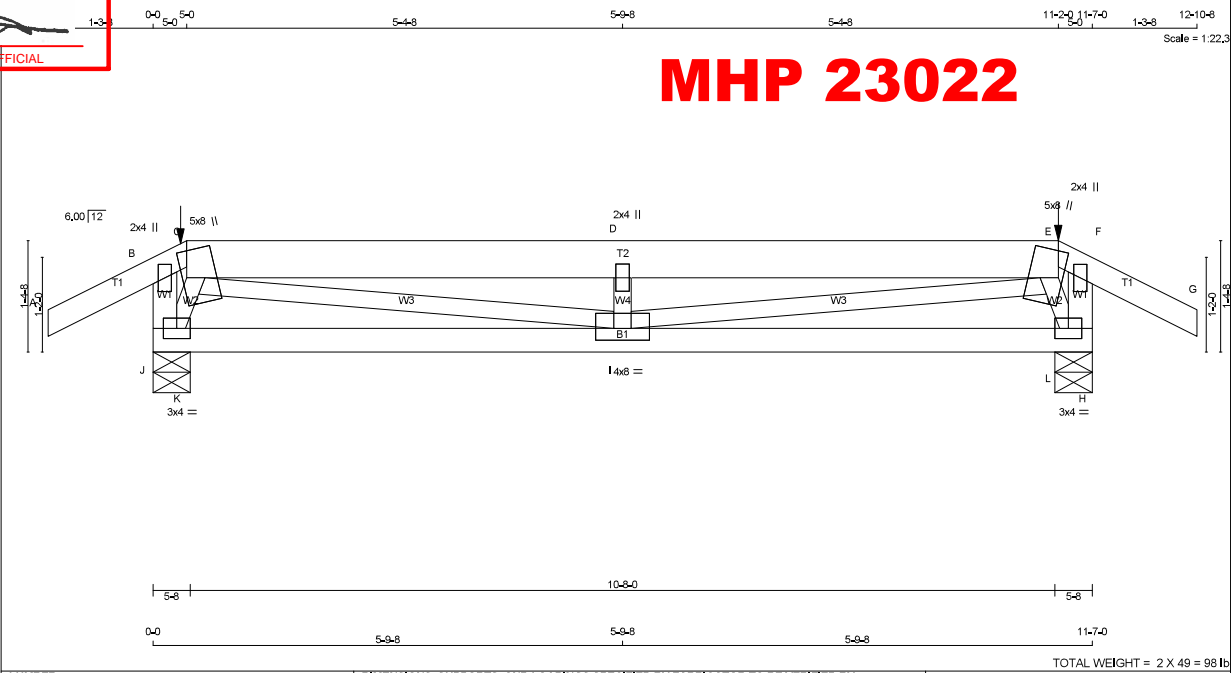
<b>LUMBER</b> N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.			<b>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</b>			<b>DESIGN CRITERIA</b>		
A - D 2x4 DRY No.2 SPF			<b>BEARINGS</b>			SPECIFIED LOADS:		
D - E 2x4 DRY No.2 SPF			FACTORED GROSS REACTION			TOP CH. LL = 34.8 PSF		
E - F 2x4 DRY No.2 SPF			DOWN HORZ UP LIFT			DL = 6.0 PSF		
F - H 2x6 DRY No.2 SPF			S 1350 0 1350 0 0			BOT CH. LL = 0.0 PSF		
H - I 2x4 DRY No.2 SPF			J 1128 0 1128 0 0			DL = 7.3 PSF		
S - B 2x4 DRY No.2 SPF			<b>UNFACTORED REACTIONS</b>			TOTAL LOAD = 48.1 PSF		
S - R 2x4 DRY No.2 SPF			1ST CASE MAX./MIN. COMPONENT REACTIONS			<b>SPACING = 24.0 IN. GIG</b>		
P - C 2x4 DRY No.2 SPF			JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL			LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM		
Q - M 2x4 DRY No.2 SPF			S 942 688 / 0 0 / 0 0 / 0 254 / 0 0 / 0			GIRDER TYPE: CPrimeHip		
M - G 2x4 DRY No.2 SPF			J 788 572 / 0 0 / 0 0 / 0 216 / 0 0 / 0			LEFT SETBACK = 3'-4"		
L - J 2x4 DRY No.2 SPF			BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J			RIGHT SETBACK = 1'-5"		
ALL WEBS EXCEPT			<b>BRACING</b>			END SETBACK = 2'-0"		
DRY: SEASONED LUMBER.			TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.			END WALL WIDTH = 5'-8"		
			MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.			CORNER FRAMING TYPE: CONVENTIONAL		
			ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.			END JACK TYPE: CONVENTIONAL		
			<b>LOADING</b>			APPLIED TO FRONT SIDE		
			TOTAL LOAD CASES: (4)			- ADDTL LOADS BASED ON 55 % OF GSL.		
			CHORDS			LOADS APPLIED TO FIRST 6'-0" OF SPAN MEASURED FROM THE RIGHT.		
			MEMB. FORCE (LBS)			THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015		
			FR-TO FROM TO LENGTH FR-TO			THIS DESIGN COMPLIES WITH:		
			A-B 0 / 36 -119.4 -119.4 0.17 (1) 10.00 R-O 0 / 937 0.23 (1)			- PART 9 OF CBC 2018 - NBC-2019AE		
			B-C -1066 / 0 -119.4 -119.4 0.18 (1) 5.85 R-D -1 / 7 0.00 (4)			- PART 9 OF CBC 2012 (2019 AMENDMENT)		
			C-D -990 / 0 -119.4 -119.4 0.10 (1) 6.13 O-D -334 / 0 0.08 (1)			- CSA 086-14		
			D-E -1390 / 0 -119.4 -119.4 0.61 (1) 4.58 D-N 0 / 633 0.16 (1)			- TPIC 2014		
			E-F -1568 / 0 -119.4 -119.4 0.14 (1) 5.10 N-E 0 / 300 0.07 (1)			DESIGN ASSUMPTIONS		
			F-G -3017 / 0 -119.4 -119.4 0.11 (1) 4.74 N-F -2052 / 0 0.47 (1)			- OVERHANG NOT TO BE ALTERED OR CUT OFF.		
			G-H -3186 / 0 -105.7 -105.7 0.32 (1) 4.39 N-L 0 / 2698 0.67 (1)			(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD		
			H-I -1148 / 0 -119.4 -119.4 0.08 (1) 5.85 F-L 0 / 710 0.18 (1)			ALLOWABLE DEFL.(LL) = L/360 (0.57")		
			I-J -1157 / 0 0.0 0.0 0.13 (1) 7.37 K-H -480 / 0 0.08 (1)			CALCULATED VERT. DEFL.(LL) = L/999 (0.12")		
			S-R 0 / 0 -18.2 -18.2 0.01 (4) 10.00 K-I 0 / 1187 0.29 (1)			ALLOWABLE DEFL.(TL) = L/360 (0.57")		
			P-R 0 / 22 0.0 0.0 0.07 (1) 10.00			CALCULATED VERT. DEFL.(TL) = L/999 (0.21")		
			R-C -101 / 0 0.0 0.0 0.07 (1) 7.81			CSI: TC=0.61/1.00 (D-E:1), BC=0.34/1.00 (L-M:1), WB=0.67/1.00 (L-N:1), SH=0.25/1.00 (D-E:1)		
			Q-P 0 / 0 -18.2 -18.2 0.02 (4) 10.00			DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00		
			O-Q 0 / 47 -18.2 -18.2 0.08 (4) 10.00			COMPANION LIVE LOAD FACTOR = 1.00		
			O-N 0 / 876 -18.2 -18.2 0.21 (1) 10.00			AUTOSOLVE RIGHT HEEL ONLY		
			N-M 0 / 227 -18.2 -18.2 0.09 (4) 10.00			TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.		
			M-L 0 / 19 0.0 0.0 0.34 (1) 10.00			NAIL VALUES		
			L-G -302 / 7 0.0 0.0 0.34 (1) 7.81			PLATE GRIP(DRY) SHEAR SECTION		
			L-K 0 / 1072 -16.2 -16.2 0.27 (1) 10.00			(PSI) (PLI) (PU)		
			K-J 0 / 0 -16.2 -16.2 0.10 (4) 10.00			MAX MIN MAX MIN MAX MIN		
			FACTORED CONCENTRATED LOADS (LBS)			MT20 650 371 1747 788 1987 1873		
			JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN.			PLATE PLACEMENT TOL. = 0.250 inches		
			H 15-9-0 -14 -14 - FRONT VERT TOTAL - C1			PLATE ROTATION TOL. = 5.0 Deg.		
			<b>CONNECTION REQUIREMENTS</b>			JSI GRIP= 0.89 (H) (INPUT = 0.90 )		
			1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			JSI METAL= 0.38 (L) (INPUT = 1.00 )		

TRUE COPY  
OF PERMIT PLANS  
Nov 23 2023

PER: *C. Monte*  
CHIEF BUILDING OFFICIAL

PROJECT NAME T-RIVER 2-3	TRUSS NAME G04	QUANTITY 1	PLY 2	JOB DESC. ZADORRA ESTATES INC	DRWG NO. ME23-5761-5
TRUSS DESC.					

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 4 08:09:20 2023 Page 1  
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LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N, L, G, A, RULES	A - C	2x4	DRY	No.2
	C - E	2x6	DRY	No.2
	E - G	2x4	DRY	No.2
	J - B	2x4	DRY	No.2
	H - F	2x4	DRY	No.2
	J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF  
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0,122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(0,0)
E-G 1	12	SIDE(0,0)
J-B 1	12	TOP
H-F 1	12	TOP
C-E 2	12	SIDE(13,1)
BOTTOM CHORDS : (0,122"x3") SPIRAL NAILS		
J-H 1	12	SIDE(2,2)
WEBS : (0,122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/4 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

#### PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
C TTVW+m	MT20	5.0	8.0	4.50	1.75
D TMV+w	MT20	2.0	4.0		
E TTVW+m	MT20	5.0	8.0	4.50	1.75
F TMV+p	MT20	2.0	4.0		
H BMVW+L	MT20	3.0	4.0		
I BMVW+L	MT20	4.0	8.0	1.75	4.00
J BMVW+L	MT20	3.0	4.0		

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT  
VOIDS THE ENGINEERS SEAL

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
J	1403	0	1403	0
H	1403	0	1403	0

#### UNFACTORED REACTIONS

1ST LOASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW
J	1005	587 / 0
H	1005	587 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

#### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

#### LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO		
A-B	0 / 48	-156.9	-156.9	0.11 (1)	10.00	C-I	0 / 3130
B-C	-134 / 0	-156.9	-156.9	0.11 (1)	6.25	I-D	-1064 / 0
C-D	-3372 / 0	-183.1	-183.1	0.25 (1)	5.77	E-F	0 / 3130
D-E	-3372 / 0	-183.1	-183.1	0.25 (1)	5.77	J-C	-799 / 0
E-F	-134 / 0	-156.9	-156.9	0.11 (1)	6.25	E-H	-799 / 0
F-G	0 / 48	-156.9	-156.9	0.11 (1)	10.00		
J-B	-592 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-592 / 0	0.0	0.0	0.03 (1)	7.81		
J-K	0 / 299	-22.2	-22.2	0.11 (4)	10.00		
K-I	0 / 299	-22.2	-22.2	0.11 (4)	10.00		
I-L	0 / 299	-22.2	-22.2	0.11 (4)	10.00		
L-H	0 / 299	-22.2	-22.2	0.11 (4)	10.00		

#### FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-0	-12	-12	---	FRONT	VERT	TOTAL	---	C1
E	11-2-0	-12	-12	---	FRONT	VERT	TOTAL	---	C1

#### CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

#### DESIGN CRITERIA

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

#### SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
BOT CH.	LL	=	6.0	PSF
	DL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip  
SIDE SETBACK = 5-0  
END SETBACK = 3-0-0  
END WALL WIDTH = 1-8  
CORNER FRAMING TYPE: CONVENTIONAL  
END JACK TYPE: CONVENTIONAL  
APPLIED TO FRONT SIDE  
-ADDTL LOADS BASED ON 55 % OF GSL.

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2015 - NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

#### DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")  
ALLOWABLE DEFL.(TL) = L/360 (0.39")  
CALCULATED VERT. DEFL.(TL) = L/978 (0.14")

CSI: TC=0.25/1.00 (D-E-I) BC=0.11/1.00 (I-J-4) ,  
WB=0.39/1.00 (E-4-I) , SSI=0.17/1.00 (C-D-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

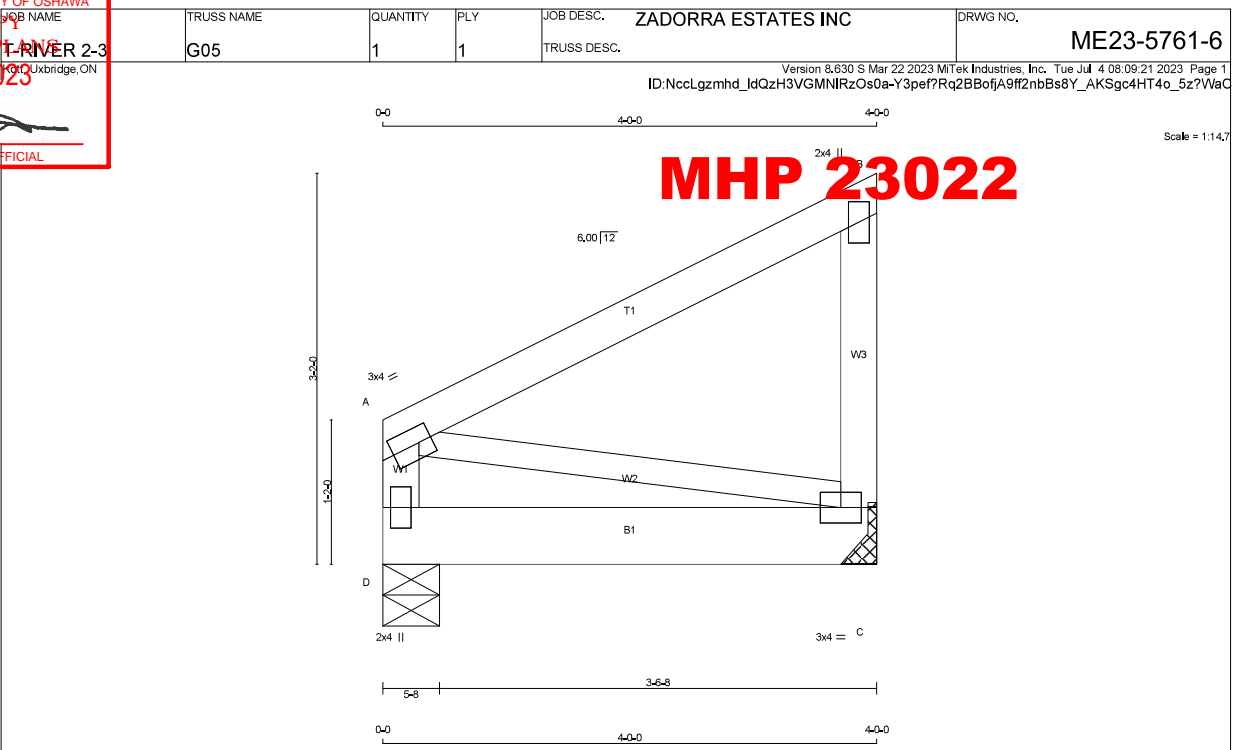
JSI GRIP= 0.86 (E) (INPUT = 0.90 )  
JSI METAL= 0.47 (C) (INPUT = 1.00 )

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult  
TPIC Appendix G - Minimum quality Manufacturing Criteria available from [www.tpica.ca](http://www.tpica.ca) and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or [www.sbindustry.com](http://www.sbindustry.com)



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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR.  
D - A 2x4 DRY No.2 SPF  
A - B 2x4 DRY No.2 SPF  
C - B 2x4 DRY No.2 SPF  
D - C 2x6 DRY No.2 SPF  
ALL WEBS 2x3 DRY No.2 SPF  
DRY: SEASONED LUMBER.

**PLATES (table is in inches)**  
JT TYPE PLATES W LEN Y X  
A TMVW4 MT20 3.0 4.0 1.50 1.25  
B TMV+p MT20 2.0 4.0  
C BMVW1+ MT20 3.0 4.0  
D BMV1+p MT20 2.0 4.0

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT
D	1288	0	1288	0
C	1288	0	1288	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C, MINIMUM BEARING LENGTH AT JOINT C = 1'-8".

**UNFACTORED REACTIONS**

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED							
D	900	651 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0	0 / 0
C	900	651 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX. (PLF)	LC2	MAX. (PLF)	UNBRACED LENGTH	FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. (PLF)
D-A	-239 / 0	0.0	0.0	0.03	(1)	7.81	A-C	0 / 0	0.00	(1)			
A-B	0 / 0	-119.4	-119.4	0.36	(1)	10.00							
C-B	-239 / 0	0.0	0.0	0.04	(1)	7.81							
D-C	0 / 0	-524.5	-524.5	0.77	(1)	10.00							

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G.C**

GIRDER TYPE: CStdGirder  
START DISTANCE = 0-0  
START SPAN CARRIED = 17'-2-0  
END DISTANCE = 4'-0-0  
END SPAN CARRIED = 17'-2-0  
END WALL WIDTH = 5'-8  
APPLIED TO FRONT SIDE OF BOTTOM CHORD.  
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 085-14  
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/933 (0.05")  
ALLOWABLE DEFL.(TL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/526 (0.09")

CSI TC=0.36/1.00 (A-B:1) , BC=0.77/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.70/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90 )  
JSI METAL= 0.10 (B) (INPUT = 1.00 )

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REVIEW FOR TRUSS COMPONENT ONLY

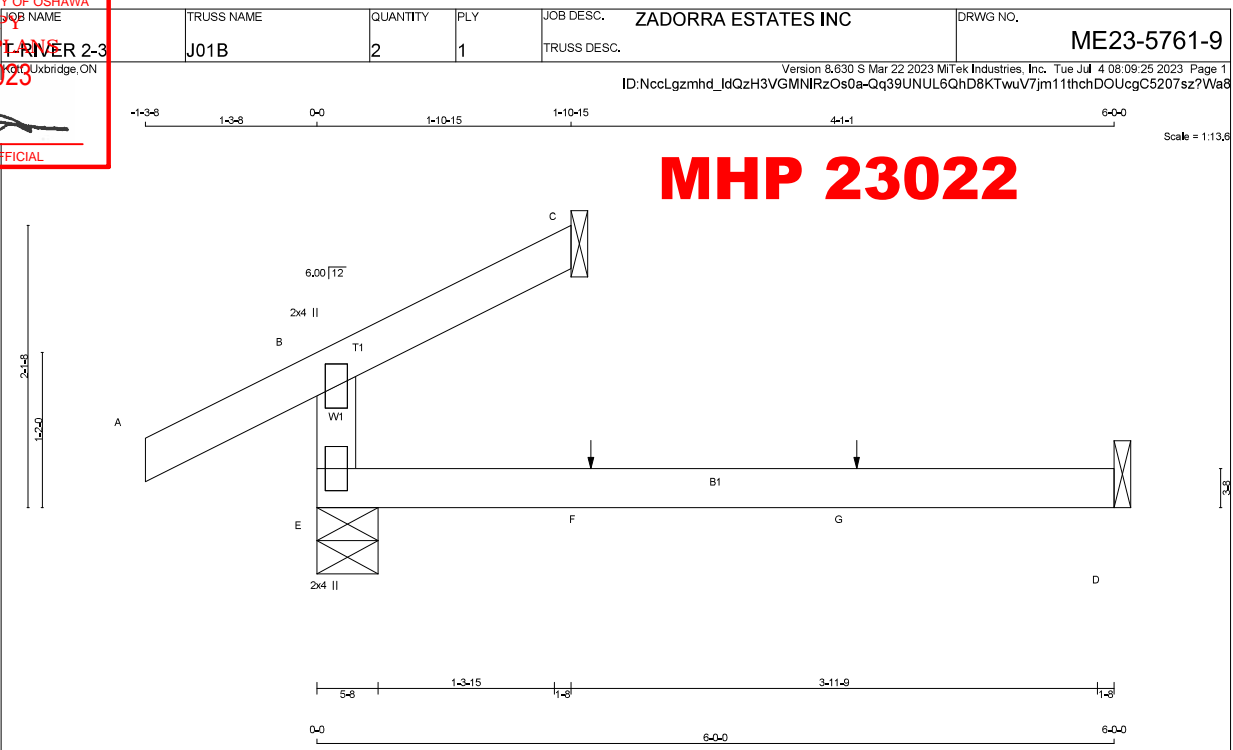
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Nov 23 2023  
PER:   
CHIEF BUILDING OFFICIAL



**LUMBER**  
N, L, G, A, RULES  
CHORDS SIZE LUMBER  
E - B 2x4 DRY No.2  
A - C 2x4 DRY No.2  
E - D 2x4 DRY No.2  
DRY, SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	FACTORED GROSS REACTION UP	INPUT BRG IN-SX	REQRD BRG IN-SX
E	368	0	0	5-8	1-8
C	86	0	0	1-8	1-8
D	45	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
E-B	-304 / 0	0.0 0.0	0.13 (4)	7.81			
A-B	0 / 36	-119.4 -119.4	0.16 (1)	10.00			
B-C	-12 / 0	-119.4 -119.4	0.07 (1)	6.25			
E-F	0 / 0	-18.2 -18.2	0.13 (4)	10.00			
F-G	0 / 0	-18.2 -18.2	0.13 (4)	10.00			
G-D	0 / 0	-18.2 -18.2	0.13 (4)	10.00			

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1
G	4-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1

**CONNECTION REQUIREMENTS**  
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**  
SPECIFIED LOADS:  
TOP CH. LL = 34.8 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**  
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.  
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI TC=0.16/1.00 (A-B 1), BC=0.13/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**


PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90 )  
JSI METAL= 0.13 (B) (INPUT = 1.00 )

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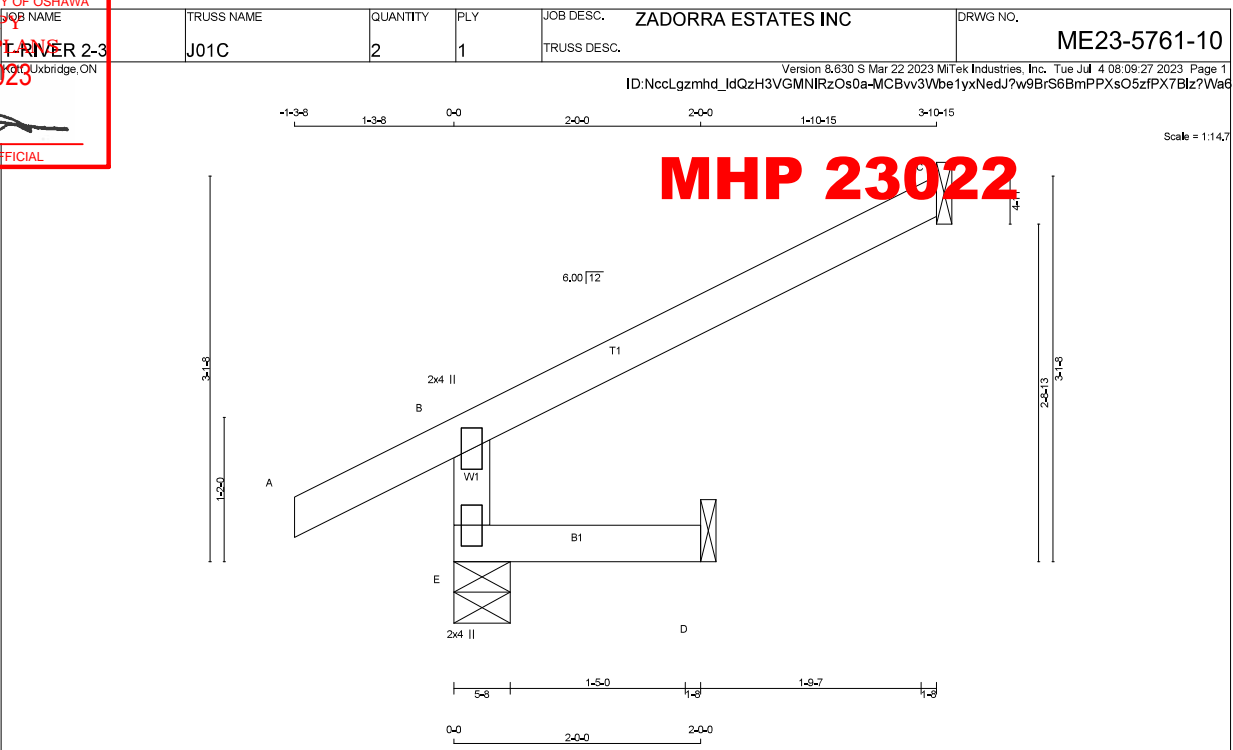


REVIEW FOR TRUSS COMPONENT ONLY  
NOTE: ALTERING THIS DOCUMENT  
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**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.  
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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR. SPF  
E - B 2x4 DRY No.2 SPF  
A - C 2x4 DRY No.2 SPF  
E - D 2x4 DRY No.2 SPF  
DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	FACTORED GROSS REACTION UP	INPUT BRG IN-SX	REQRD BRG IN-SX
E	474	0	0	5-8	1-8
C	175	0	0	1-8	1-8
D	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	W E B S	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO									
E-B		-454 / 0	0.0	0.0	0.01 (4)	7.81			
A-B		0 / 36	-119.4	-119.4	0.16 (1)	10.00			
B-C		-26 / 0	-119.4	-119.4	0.31 (1)	6.25			
E-D		0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.  
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/999 (0.00")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL) = L/999 (0.00")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI TC=0.31/1.00 (B-C:1) BC=0.02/1.00 (D-E:4) ,  
WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90 )  
JSI METAL= 0.19 (B) (INPUT = 1.00 )

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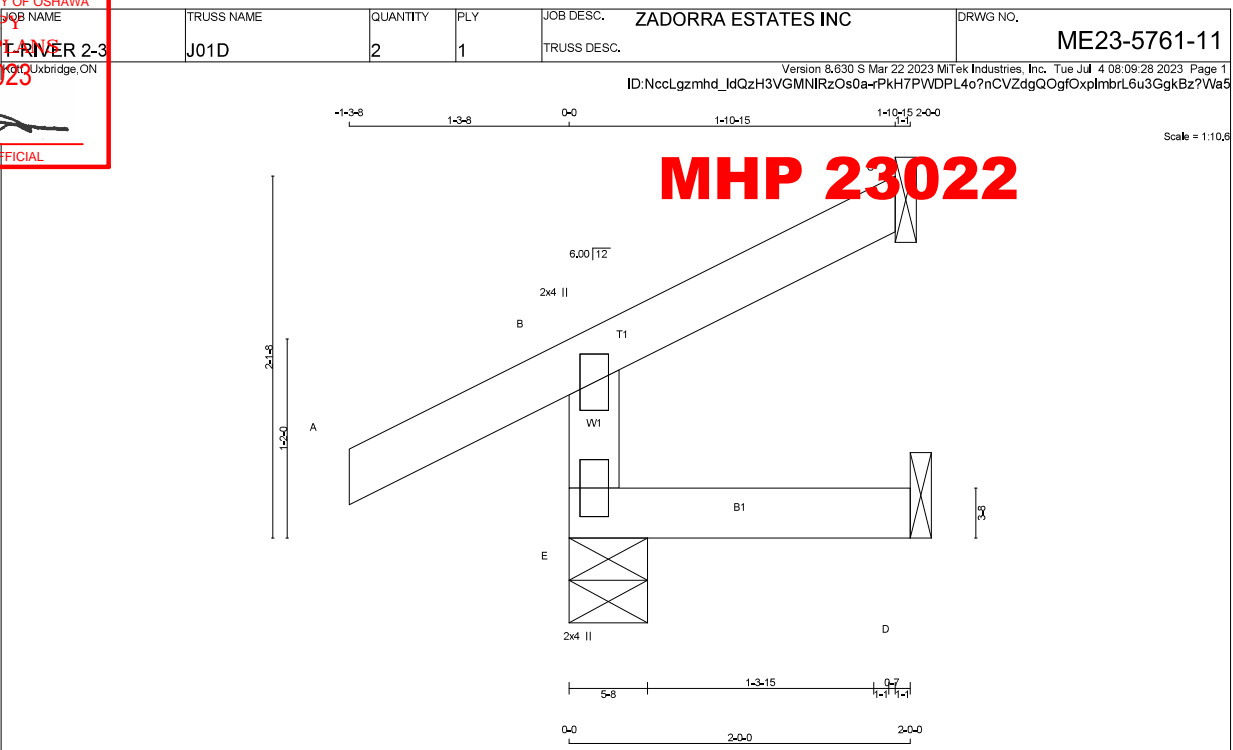
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT  
VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
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**LUMBER**

N, L, G, A, RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF
E - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	DOWN		
E	324	0	5-8	1-8
C	86	0	1-8	1-8
D	16	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	224	177 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-304 / 0	0.0	0.0 (4)	7.81			
A-B	0 / 36	-119.4	-119.4 (1)	10.00			
B-C	-12 / 0	-119.4	-119.4 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

**SPACING = 24.0 IN. G.C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

**DESIGN ASSUMPTIONS**

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4), WB=0.00/1.00 (n/a.0), SSI=0.11/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90 )

JSI METAL= 0.13 (B) (INPUT = 1.00 )

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REVIEW FOR TRUSS COMPONENT ONLY

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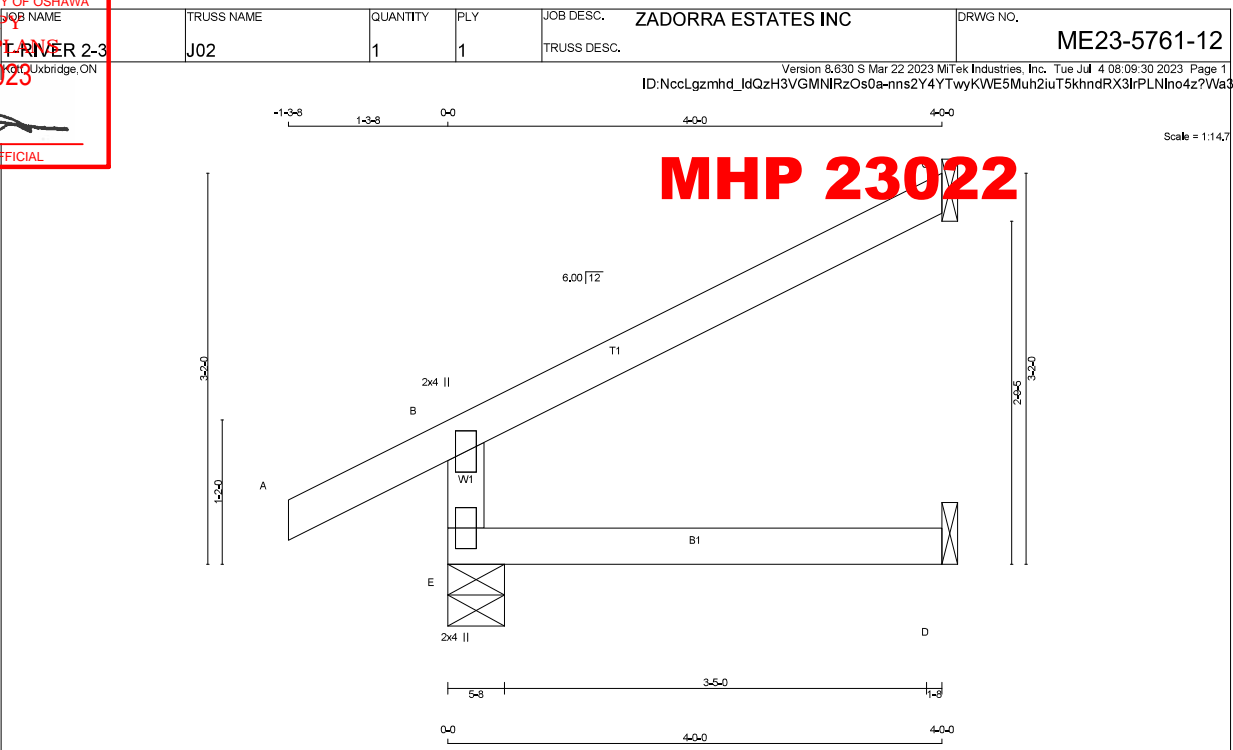
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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR. SPF

E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
E	503	503	0	1-8
C	179	179	0	1-8
D	31	35	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	348	268 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	123	105 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
		FROM	TO			
FR-TO						
E-B	-460 / 0	0.0	0.0	0.05 (4)	7.81	
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	
B-C	-27 / 0	-119.4	-119.4	0.32 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00	

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

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ALLOWABLE DEFL.(LL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) ,  
WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PU)
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90 )  
JSI METAL= 0.19 (B) (INPUT = 1.00 )

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

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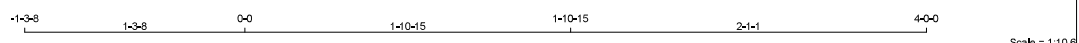
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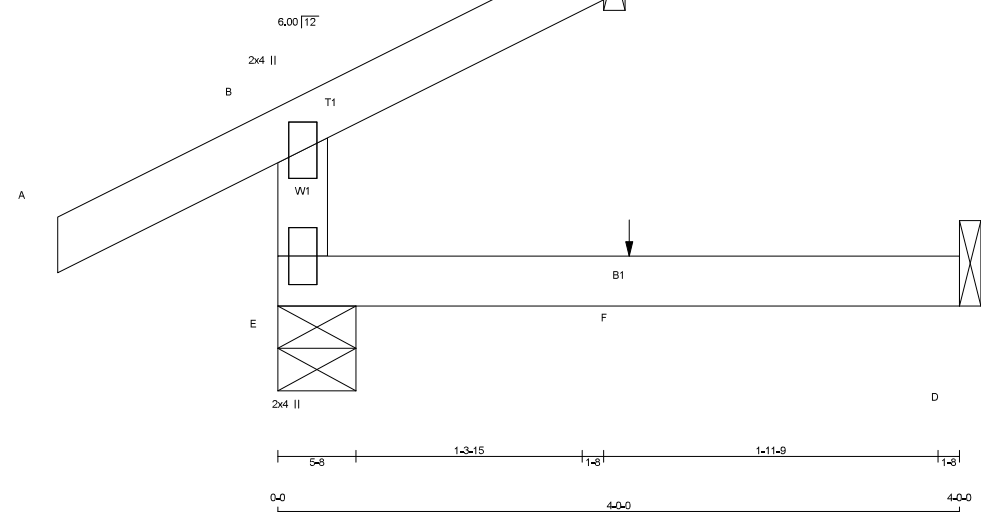


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PROJECT NAME RIVER 2-3	TRUSS NAME J02A	QUANTITY 1	PLY 1	JOB DESC. ZADORRA ESTATES INC	DRWG NO. ME23-5761-13
Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 4 08:09:31 2023 Page 1 ID:NccLgzmdhd_IdQzH3VGMNIRzOs0a-F_QQIQZ6hGSNsFw4EIE7?IHvA0n0C5Za1VKKWz?Wa2					



MHP 23022



TOTAL WEIGHT = 10 lb

<b>LUMBER</b> N, L, G, A, RULES CHORDS SIZE LUMBER DESCR. SPF E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF  DRY: SEASONED LUMBER.	<b>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</b> <b>BEARINGS</b> <table><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQD BRG</th></tr><tr><td>JT</td><td>VERT</td><td>DOWN</td><td>UPLIFT</td><td>IN-SX</td></tr><tr><td>E</td><td>345</td><td>0</td><td>0</td><td>5-8</td></tr><tr><td>C</td><td>86</td><td>0</td><td>0</td><td>1-8</td></tr><tr><td>D</td><td>31</td><td>0</td><td>0</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D <b>UNFACTORED REACTIONS</b> <table><tr><th>JT</th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>241</td><td>177 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>64 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>59</td><td>50 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>9 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>25</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>25 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C <b>BRACING</b> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. <b>LOADING</b> TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRACED LENGTH (FT)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRACED LENGTH (FT)</th><th></th></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>E-B</td><td>-304 / 0</td><td>0.0</td><td>0.05 (4)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 36</td><td>-119.4</td><td>-119.4</td><td>0.16 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>B-C</td><td>-12 / 0</td><td>-119.4</td><td>-119.4</td><td>0.07 (1)</td><td>6.25</td><td></td><td></td></tr><tr><td>E-F</td><td>0 / 0</td><td>-18.2</td><td>-18.2</td><td>0.06 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>F-D</td><td>0 / 0</td><td>-18.2</td><td>-18.2</td><td>0.06 (4)</td><td>10.00</td><td></td><td></td></tr></table> FACTORED CONCENTRATED LOADS (LBS) <table><tr><th>JT</th><th>LOC.</th><th>LC1</th><th>MAX-</th><th>MAX+</th><th>FACE</th><th>DIR.</th><th>TYPE</th><th>HEEL</th><th>CONN.</th></tr><tr><td>F</td><td>2-0-12</td><td>1</td><td>1</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr></table> <b>CONNECTION REQUIREMENTS</b> 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	JT	VERT	DOWN	UPLIFT	IN-SX	E	345	0	0	5-8	C	86	0	0	1-8	D	31	0	0	1-8	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	241	177 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0	C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)		FR-TO		FROM	TO	FR-TO				E-B	-304 / 0	0.0	0.05 (4)	7.81				A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00			B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25			E-F	0 / 0	-18.2	-18.2	0.06 (4)	10.00			F-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00			JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	F	2-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1	<b>DESIGN CRITERIA</b> <b>SPECIFIED LOADS:</b> TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF  <b>SPACING = 240 IN. G/C</b>  THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015  THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014  <b>DESIGN ASSUMPTIONS</b> - OVERHANG NOT TO BE ALTERED OR CUT OFF.  (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD  ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.01")  CSI: TC=0.16/1.00 (A-B 1), BC=0.06/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)  DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10  COMPANION LIVE LOAD FACTOR = 1.00  TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.  NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873  PLATE PLACEMENT TOL. = 0.250 inches  PLATE ROTATION TOL. = 5.0 Deg.  JSI GRIP= 0.17 (B) (INPUT = 0.90 ) JSI METAL= 0.13 (B) (INPUT = 1.00 )
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