
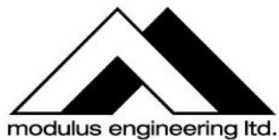


<h1>IMPORTANT INFORMATION</h1>	
<p>Hangers and Fasteners to be installed as per manufacturer</p>	
<p>Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes</p>	
<p>For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines</p>	
<p>Read all notes on this page in addition to those shown on the KOTT Truss Engineering package</p>	
<p>Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering</p>	
<p>Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.</p>	
<p>Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.</p>	
<p>KOTT Inc. 14 Anderson Blvd. Uxbridge, ON 905.642.4400</p>	



General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



**Read Carefully Prior
Manufacture and Installation**

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CHIEF BUILDING OFFICIAL
Nov 23 2023
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CORPORATION OF THE CITY OF OSHAWA

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

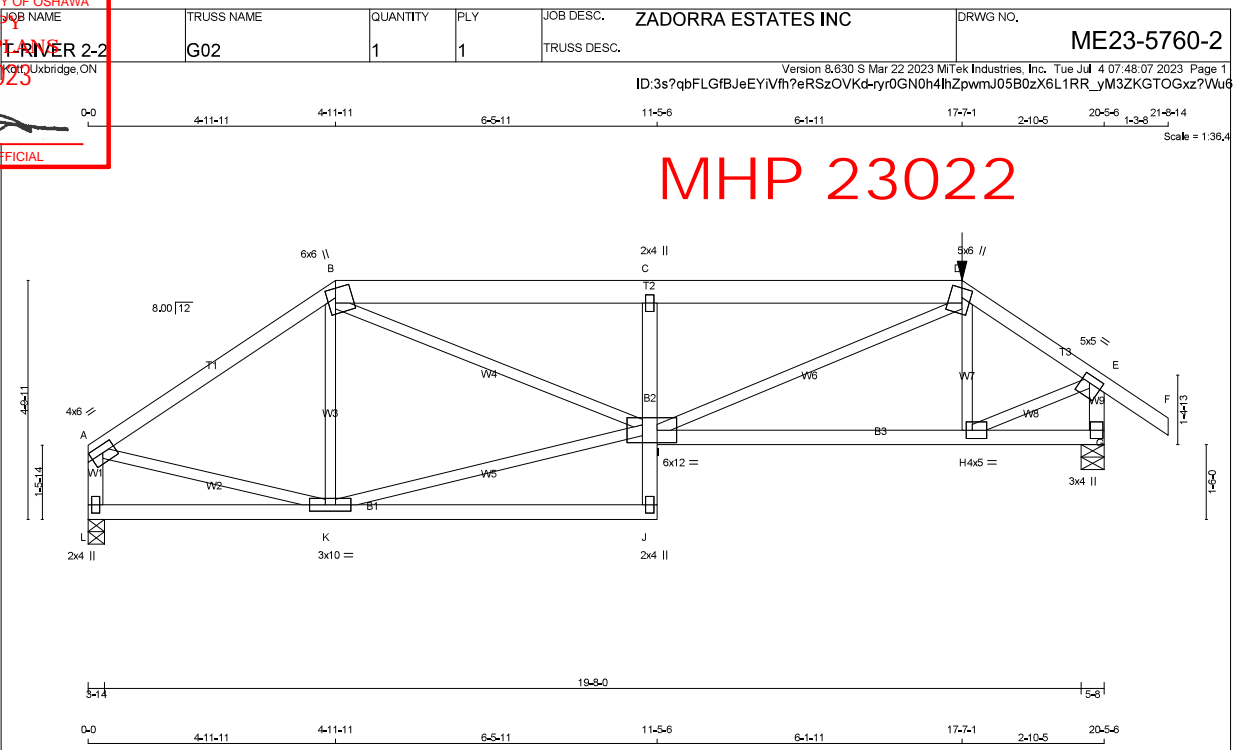
Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)
2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.
3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses
4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)
5. Cut members to bear tightly against one another
6. Place plates on each face of truss at each joint and embed fully using proper roller or hydraulic press. Knots and wane at joint locations are regulated by TPIC Appendix G
7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC
8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture
9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment
10. Connections not shown are the responsibility of others
11. Do not cut or alter truss members or plates without prior approval of an engineer
12. Install and load vertically unless otherwise noted
13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient
14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G
16. Building designer must review individual component drawings to ensure they are suitable for the structure
15. Not designed for solar panels unless specifically noted

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OF PERMIT PLANS
Nov 23 2023
PER: *C. Morris*
CHIEF BUILDING OFFICIAL



MHP 23022

LUMBER
N, L, G, A, RULES
CHORDS SIZE LUMBER DESCR.

A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW4	MT20	4.0	6.0	1.75	Edge
B TTVW+m	MT20	6.0	6.0	2.25	2.00
C TMV+p	MT20	2.0	4.0		
D TTVW+m	MT20	5.0	6.0	2.25	2.00
E TMVW4	MT20	5.0	5.0	1.75	2.00
G BMV1+p	MT20	3.0	4.0		
H BMVW4	MT20	4.0	5.0	2.00	1.50
I BMVW4	MT20	6.0	12.0	3.00	4.75
J BMV+p	MT20	2.0	4.0		
K BMVW4	MT20	3.0	10.0		
L BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	DOWN	HORZ	UPLIFT
L	1597	0	1597	0
G	2157	0	2157	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	1116	809 / 0 0 / 0 0 / 0 307 / 0 0 / 0
G	1505	1101 / 0 0 / 0 0 / 0 404 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.98 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. CSI (LC)	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
A-B	-1690 / 0	-119.4	-119.4, 0.68 (1)	4.16	K-B	-525 / 0	0.18 (1)
B-C	-3334 / 0	-119.4	-119.4, 0.56 (1)	3.99	K-I	0 / 1390	0.34 (1)
C-D	-3369 / 0	-194.5	-194.5, 0.60 (1)	3.88	B-I	0 / 2111	0.52 (1)
D-E	-2076 / 0	-119.4	-119.4, 0.25 (1)	4.43	I-D	0 / 1787	0.44 (1)
E-F	0 / 45	-119.4	-119.4, 0.18 (1)	10.00	H-D	-489 / 24	0.10 (1)
L-A	-1568 / 0	0.0	0.0, 0.18 (1)	6.55	A-K	0 / 1450	0.36 (1)
G-E	-2155 / 0	0.0	0.0, 0.24 (1)	5.73	H-E	0 / 1863	0.46 (1)
L-K	0 / 0	-18.2	-18.2, 0.19 (4)	10.00			
K-J	0 / 68	-18.2	-18.2, 0.20 (4)	10.00			
J-I	0 / 53	0.0	0.0, 0.16 (1)	10.00			
I-C	-1214 / 0	0.0	0.0, 0.22 (1)	6.98			
I-H	0 / 1752	-29.7	-29.7, 0.47 (1)	10.00			
H-G	0 / 0	-29.7	-29.7, 0.29 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	17-7-1	-198	-198	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-11-11
RIGHT SETBACK = 2-10-5
END SETBACK = 4-11-11
END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL,
LOADS APPLIED TO FIRST 9'-0" OF SPAN
MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 - NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.68/1.00 (A-B 1), BC=0.47/1.00 (H-I 1),
WB=0.52/1.00 (B-I 1), SSI=0.47/1.00 (C-D 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.62 (E) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com





JOB NAME
T-RIVER 2-2
Kirkcubridge, ON

TRUSS NAME

G04

QUANTITY

1

PLY

1

JOB DESC.

TRUSS DESC.

ZADORRA ESTATES INC

DRWG NO.

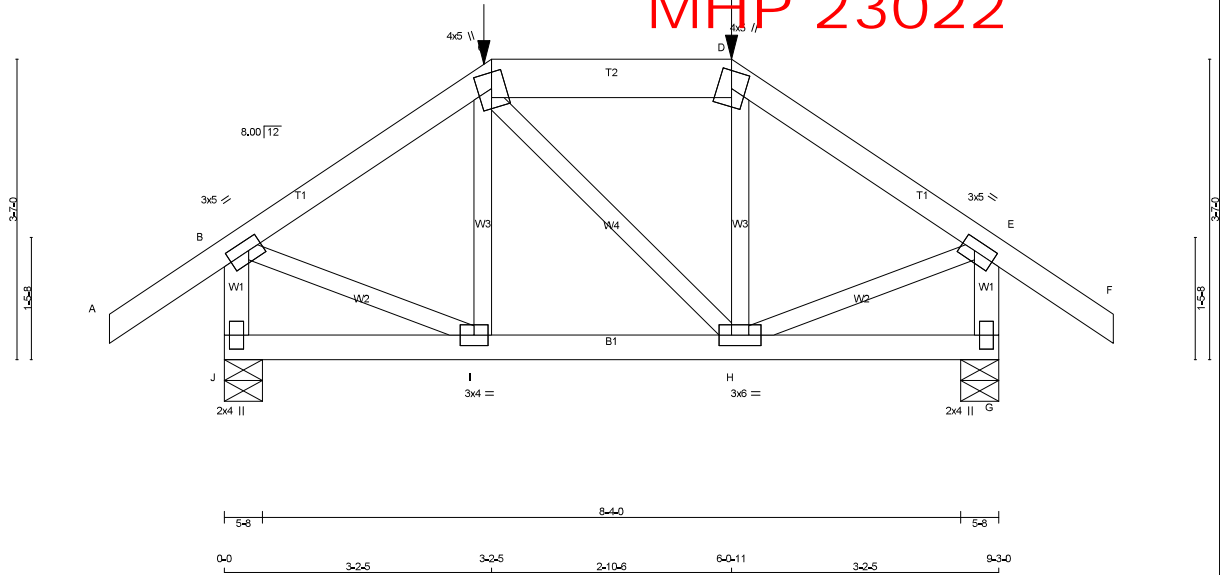
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Version 8,630 S Mar 22 2023 MTek Industries, Inc. Tue Jul 4 07:48:09 2023 Page 1

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1-4-7 1-4-7 0-0 3-2-5 3-2-5 2-10-6 6-0-11 3-2-5 9-3-0 1-4-7 10-7-7
Scale = 1:21.6

MHP 23022



TOTAL WEIGHT = 441b

LUMBER

N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
C - D 2x6 DRY No.2
D - F 2x4 DRY No.2
J - B 2x4 DRY No.2
G - E 2x4 DRY No.2
J - G 2x4 DRY No.2

LUMBER

DESCR.

SPF
SPF
SPF
SPF
SPF
SPFALL WEBS
EXCEPT

DRY: SEASONED LUMBER,

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW4	MT20	3.0	5.0	1.50	2.00
C TTWW+m	MT20	4.0	5.0	2.25	2.00
D TTW+m	MT20	4.0	5.0		
E TMVW4	MT20	3.0	5.0	1.50	2.00
G BMV1+p	MT20	2.0	4.0		
H BMVWW4	MT20	3.0	6.0		
I BMVWW4	MT20	3.0	4.0		
J BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
J	855	0	855	0
G	855	0	855	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	594	451 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0	0 / 0
G	594	451 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 48	-119.4	-119.4	0.20 (1)	10.00	I-C	-109 / 22
B-C	-546 / 0	-119.4	-119.4	0.24 (1)	6.25	C-H	0 / 0
C-D	-459 / 0	-115.7	-115.7	0.09 (1)	6.25	H-D	-110 / 22
D-E	-545 / 0	-119.4	-119.4	0.24 (1)	6.25	B-I	0 / 486
E-F	0 / 48	-119.4	-119.4	0.20 (1)	10.00	H-E	0 / 485
J-B	-831 / 0	0.0	0.0	0.09 (1)	7.81		
G-E	-831 / 0	0.0	0.0	0.09 (1)	7.81		
J-I	0 / 0	-17.7	-17.7	0.04 (4)	10.00		
I-H	0 / 459	-17.7	-17.7	0.10 (1)	10.00		
H-G	0 / 0	-17.7	-17.7	0.04 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-2-5	-53	-53	—	FRONT	VERT	TOTAL	—	C1
D	6-0-11	-53	-53	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-2-5
END SETBACK = 2-4-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")CSI TC=0.24/1.00 (B-C:1), BC=0.10/1.00 (H-I:1),
WB=0.12/1.00 (B-I:1), SSI=0.14/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (FUI) (FUI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



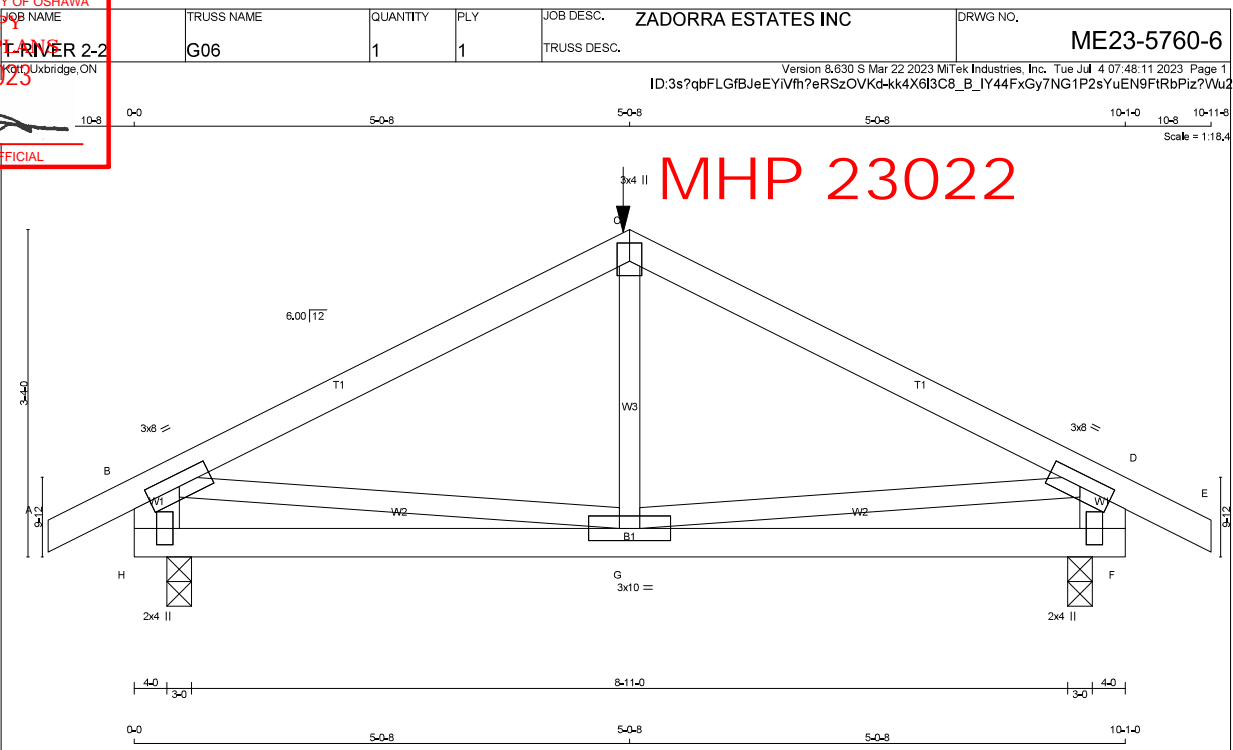
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com

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Nov 23 2023
PER: *C. Morris*
CHIEF BUILDING OFFICIAL



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x6	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	3.0	8.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW4	MT20	3.0	8.0		
F	BMV1+p	MT20	2.0	4.0	2.00	0.75
G	BMVW4	MT20	3.0	10.0		
H	BMV1+p	MT20	2.0	4.0	2.00	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	1226	0	1226	0
F	1226	0	1226	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
H	856	624 / 0	0 / 0
F	856	624 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 25	-119.4	-119.4	0.08 (1)	10.00	G-C	-64 / 149 0.06 (4)
B-C	-1396 / 0	-119.4	-119.4	0.65 (1)	4.50	B-G	0 / 1255 0.31 (1)
C-D	-1396 / 0	-119.4	-119.4	0.65 (1)	4.50	G-D	0 / 1255 0.31 (1)
D-E	0 / 25	-119.4	-119.4	0.08 (1)	10.00		
H-B	-1163 / 0	0.0	0.0	0.08 (1)	7.81		
F-D	-1163 / 0	0.0	0.0	0.08 (1)	7.81		
H-G	0 / 0	-31.0	-31.0	0.24 (4)	10.00		
G-F	0 / 0	-31.0	-31.0	0.24 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5'-0"	-7'11"	-7'11"	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	PSF
	DL	=	34.8
	LL	=	6.0

BOT

CH.	LL	PSF
	DL	= 0.0
	LL	= 7.3

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 5'-0"

END SETBACK = 5'-0"

END WALL WIDTH = 3'-0"

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL) = L/360 (0.34")

CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI TC=0.65/1.00 (B-C:1) BC=0.24/1.00 (F-G:4) WB=0.31/1.00 (D-G:1) SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)

JSI METAL= 0.28 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

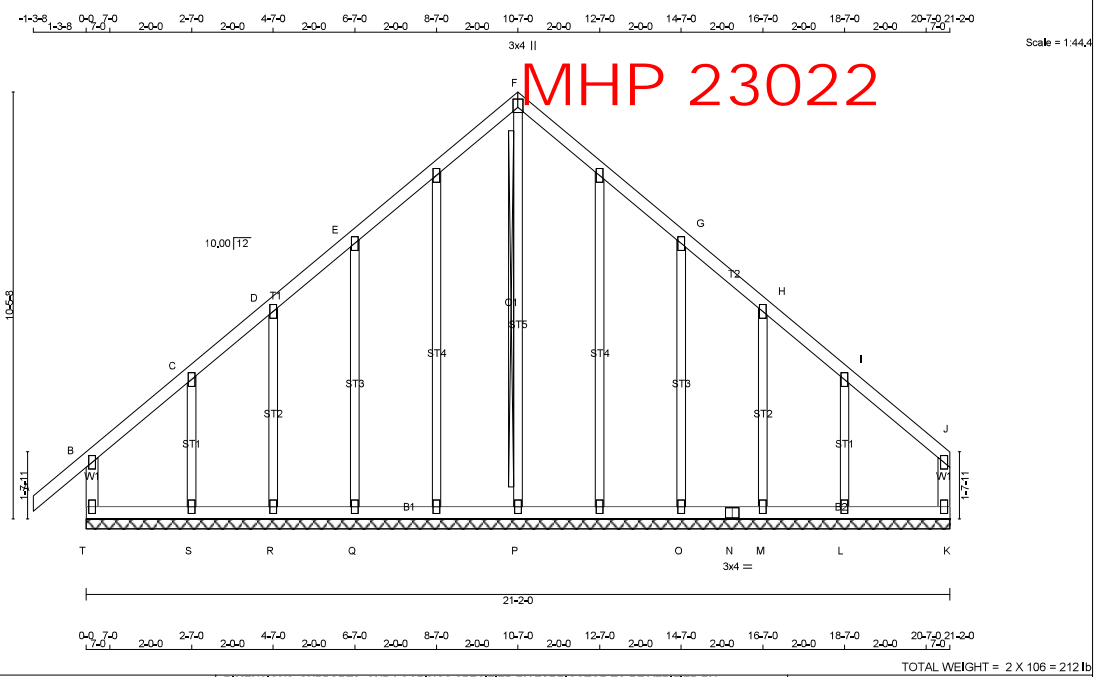
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



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PROJECT NAME T-RIVER 2-2	TRUSS NAME GE01	QUANTITY 2	PLY 1	JOB DESC. ZADORRA ESTATES INC	DRWG NO. ME23-5760-7
TRUSS DESC.					

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 4 07:48:12 2023 Page 1
ID:3s7qbFLGfBJeEYIVh7eRSzOVkd-CweVJ54qvJrwHfGpenBgblMSERdggITXA9xz?Wu1



LUMBER

N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
T - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER,				
GABLE STUDS SPACED AT	2'-0" O.C.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I					
C TMV+w	MT20	2.0	4.0		
F TTV+p	MT20	3.0	4.0	2.50	1.50
J TMV+p	MT20	2.0	4.0		
K BMV1+p	MT20	2.0	4.0		
L, M, O, P, Q, R, S					
L BMV1+w	MT20	2.0	4.0		
N BS+	MT20	3.0	4.0		
T BMV1+p	MT20	2.0	4.0		
U, V, W, X					
U NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
T-B	-378 / 0	0.0	0.08 (1)	7.81	P-F	-433 / 0	0.37 (1)
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00	Q-E	-391 / 0
B-C	-95 / 0	-119.4	-119.4	0.09 (1)	6.25	R-D	-172 / 0
C-D	-91 / 0	-119.4	-119.4	0.07 (1)	6.25	S-C	-271 / 0
D-E	-46 / 0	-119.4	-119.4	0.16 (1)	6.25	O-G	-391 / 0
E-F	-59 / 0	-119.4	-119.4	0.26 (1)	6.25	M-H	-171 / 0
F-G	-59 / 0	-119.4	-119.4	0.26 (1)	6.25	L-I	-275 / 0
G-H	-46 / 0	-119.4	-119.4	0.16 (1)	6.25		
H-I	-92 / 0	-119.4	-119.4	0.07 (1)	6.25		
I-J	-93 / 0	-119.4	-119.4	0.08 (1)	6.25		
K-J	-210 / 0	0.0	0.08 (1)	7.81			
T-S	0 / 70	-18.2	-18.2	0.06 (1)	10.00		
S-R	0 / 62	-18.2	-18.2	0.03 (4)	10.00		
R-Q	0 / 59	-18.2	-18.2	0.04 (4)	10.00		
Q-P	0 / 54	-18.2	-18.2	0.07 (4)	10.00		
P-O	0 / 54	-18.2	-18.2	0.07 (4)	10.00		
O-N	0 / 59	-18.2	-18.2	0.04 (4)	10.00		
N-M	0 / 59	-18.2	-18.2	0.04 (4)	10.00		
M-L	0 / 62	-18.2	-18.2	0.03 (4)	10.00		
L-K	0 / 70	-18.2	-18.2	0.05 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (F-G:1), BC=0.07/1.00 (O-P:1), WB=0.37/1.00 (F-P:1), SSH=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (F) (INPUT = 0.90)

JSI METAL= 0.35 (F) (INPUT = 1.00)

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REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
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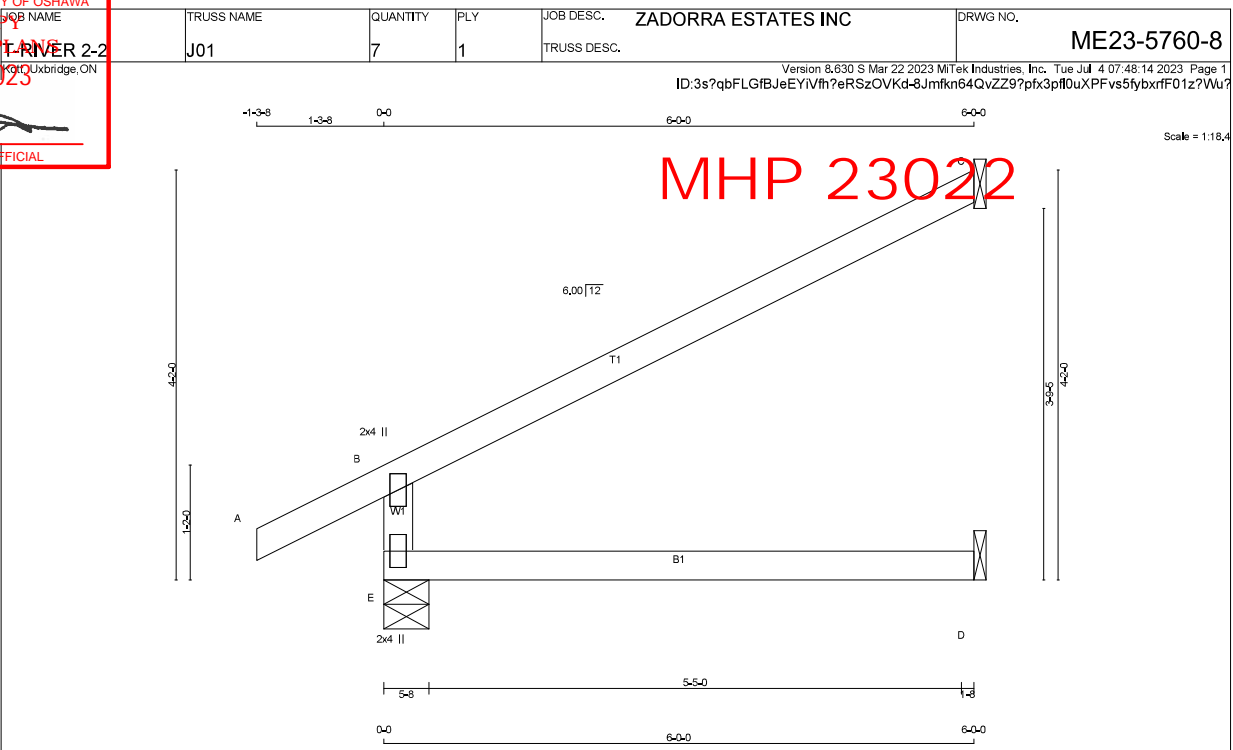
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF

E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG
E	674	0	674	0	0	5-8	1-8
C	269	0	269	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	468	355 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
C	184	157 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
E-B	-610 / 0	0.0	0.0 0.13 (4)	7.81			
A-B	0 / 36	-119.4	-119.4 0.16 (1)	10.00			
B-C	-40 / 0	-119.4	-119.4 0.73 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	(PU)
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	788 1987	1673

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



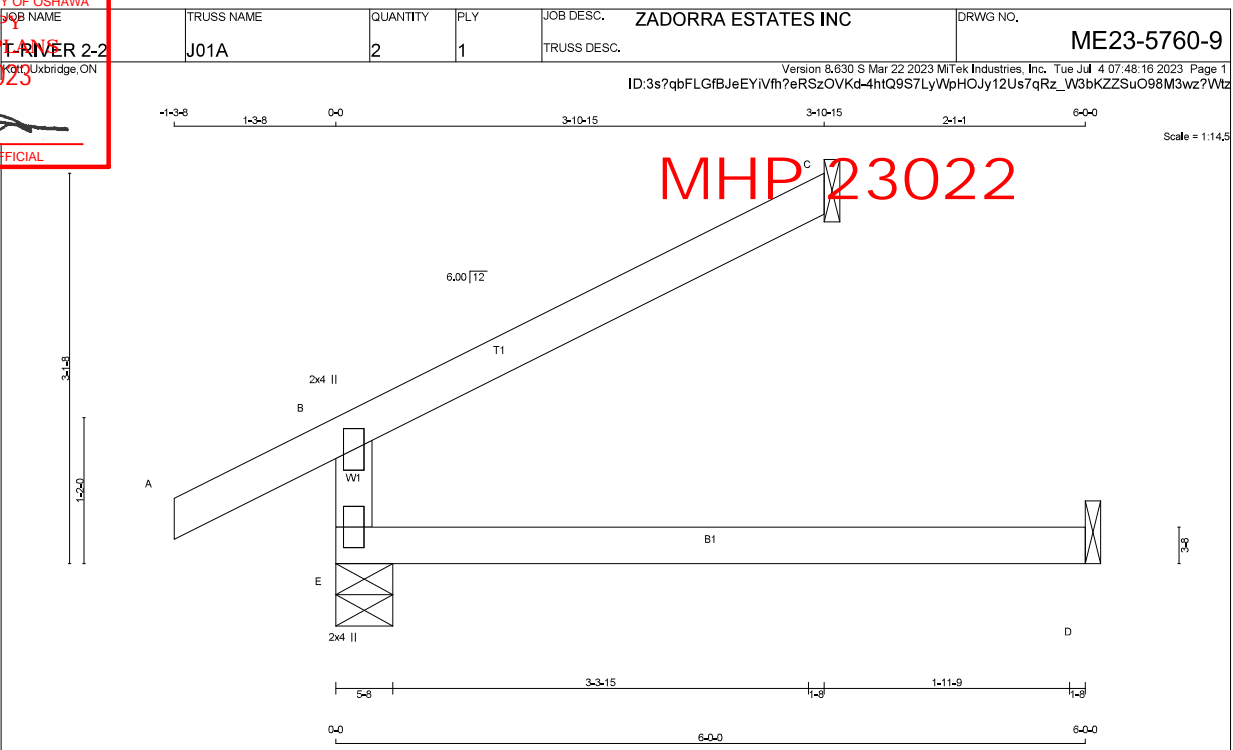
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
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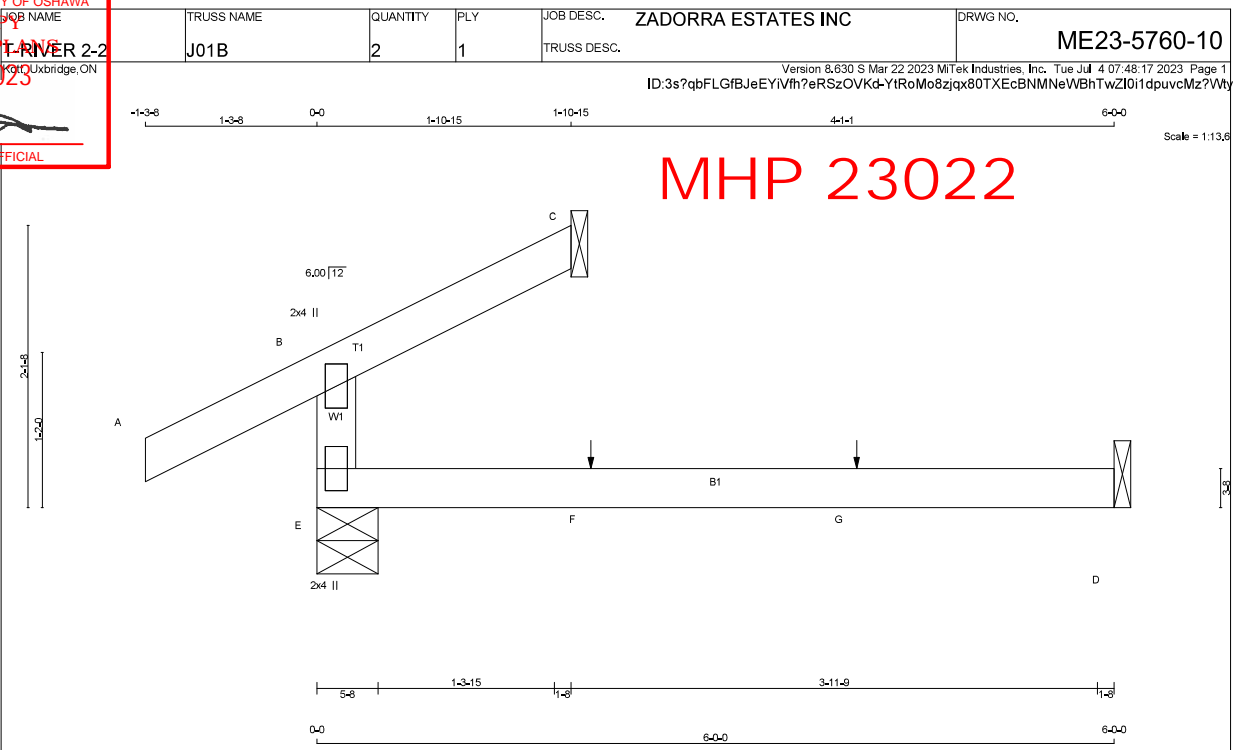
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.		DESCR. SPF SPF SPF	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 2.0 4.0 E BMV1+p MT20 2.0 4.0		BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX E 518 0 518 0 0 5-8 1-8 C 175 0 175 0 0 1-8 1-8 D 45 0 51 0 0 1-8 1-8	
UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 362 265 / 0 0 / 0 0 / 0 97 / 0 0 / 0 C 120 102 / 0 0 / 0 0 / 0 0 / 0 18 / 0 0 / 0 D 36 0 / 0 0 / 0 0 / 0 0 / 0 36 / 0 0 / 0		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/999 (0.03") CSI TC=0.31/1.00 (B-C:1) BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.25 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)	
BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.		LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. (PLF) MAX. (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. (LC) FR-TO E-B -454 / 0 0.0 0.0 0.13 (4) 7.81 A-B 0 / 36 -119.4 -119.4 0.16 (1) 10.00 B-C -26 / 0 -119.4 -119.4 0.31 (1) 6.25 E-D 0 / 0 -18.2 -18.2 0.13 (4) 10.00	



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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.		DESCR. SPF SPF SPF	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 2.0 4.0 E BMV1+p MT20 2.0 4.0		BEARINGS FACTORED GROSS REACTION JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX E 368 0 368 0 0 5-8 1-8 C 86 0 86 0 0 1-8 1-8 D 45 0 51 0 0 1-8 1-8	
BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/999 (0.03") CSI TC=0.16/1.00 (A-B 1), BC=0.13/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)	
LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. UNBRACED LENGTH (FT) FR-TO E-B -304 / 0 0.0 0.0 0.13 (4) 7.81 A-B 0 / 36 -119.4 -119.4 0.16 (1) 10.00 B-C -12 / 0 -119.4 -119.4 0.07 (1) 6.25 E-F 0 / 0 -18.2 -18.2 0.13 (4) 10.00 F-G 0 / 0 -18.2 -18.2 0.13 (4) 10.00 G-D 0 / 0 -18.2 -18.2 0.13 (4) 10.00 WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH (FT) FR-TO E-B 0.0 0.0 0.13 (4) 7.81 A-B 0 / 36 -119.4 -119.4 0.16 (1) 10.00 B-C -12 / 0 -119.4 -119.4 0.07 (1) 6.25 E-F 0 / 0 -18.2 -18.2 0.13 (4) 10.00 F-G 0 / 0 -18.2 -18.2 0.13 (4) 10.00 G-D 0 / 0 -18.2 -18.2 0.13 (4) 10.00		FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX+ MAX- FACE DIR. TYPE HEEL CONN. F 2-0-12 1 1 - FRONT VERT TOTAL - C1 G 4-0-12 1 1 - FRONT VERT TOTAL - C1	
CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.		REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL	



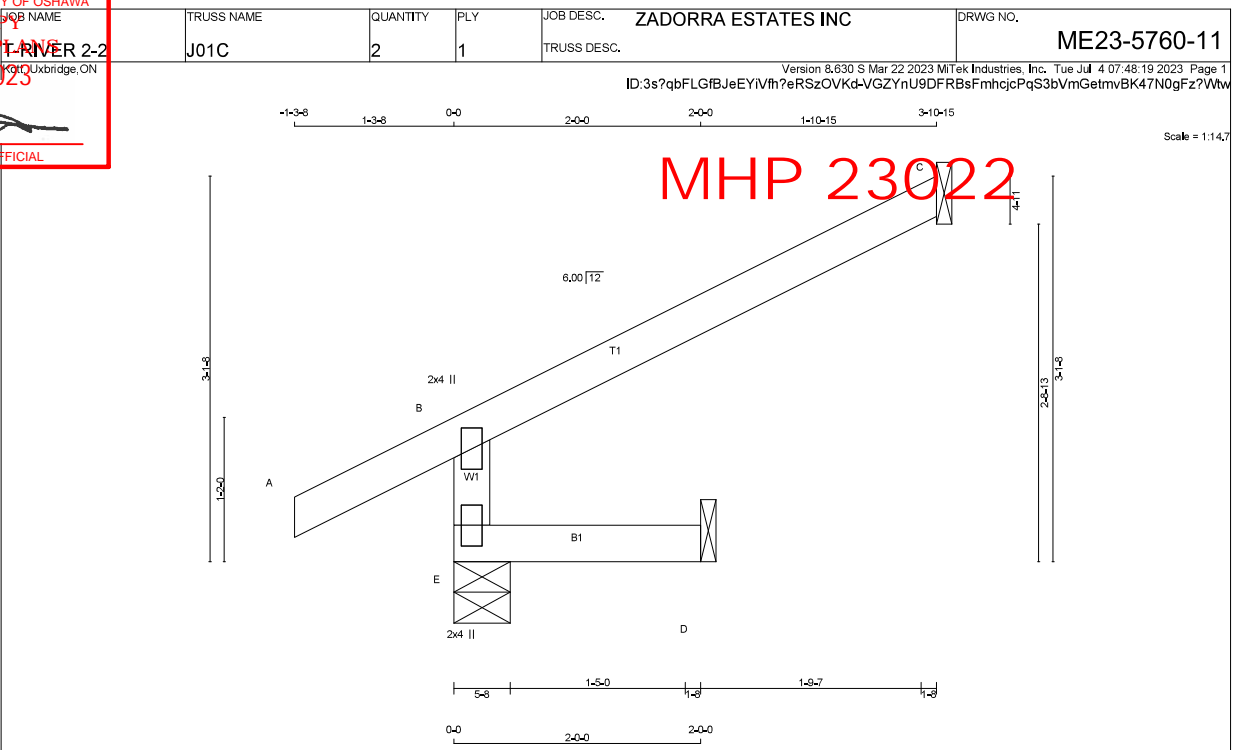
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Nov 23 2023

PER: 

CHIEF BUILDING OFFICIAL



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr><tr><th>JT</th><th>VERT</th><th>DOWN</th><th>UP/LIFT</th><th></th></tr><tr><td>E</td><td>474</td><td>0</td><td>0</td><td>5-8</td></tr><tr><td>C</td><td>175</td><td>0</td><td>0</td><td>1-8</td></tr><tr><td>D</td><td>16</td><td>0</td><td>0</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><th>JT</th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>326</td><td>265 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>62 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>120</td><td>102 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>18 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>13</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>13 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (5) <table><tr><th colspan="2">CHORDS</th><th colspan="2">W E B S</th></tr><tr><th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRACED LENGTH FR-TO</th></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td></tr><tr><td>E-B</td><td>-454 / 0</td><td>0.0</td><td>0.0 (0.01 (4) 7.81</td></tr><tr><td>A-B</td><td>0 / 36</td><td>-119.4</td><td>-119.4 0.16 (1) 10.00</td></tr><tr><td>B-C</td><td>-26 / 0</td><td>-119.4</td><td>-119.4 0.31 (1) 6.25</td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.02 (4) 10.00</td></tr></table> CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.											FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	DOWN	UP/LIFT		E	474	0	0	5-8	C	175	0	0	1-8	D	16	0	0	1-8	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0	C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	CHORDS		W E B S		MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	FR-TO		FROM	TO	E-B	-454 / 0	0.0	0.0 (0.01 (4) 7.81	A-B	0 / 36	-119.4	-119.4 0.16 (1) 10.00	B-C	-26 / 0	-119.4	-119.4 0.31 (1) 6.25	E-D	0 / 0	-18.2	-18.2 0.02 (4) 10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00") CSI TC=0.31/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.25 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)									
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG																																																																																																														
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E	474	0	0	5-8																																																																																																														
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JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																																																											
E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0																																																																																																											
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0																																																																																																											
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0																																																																																																											
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MODULUS ENGINEERING LTD.

07/04/2023

D. A. SHERMAN

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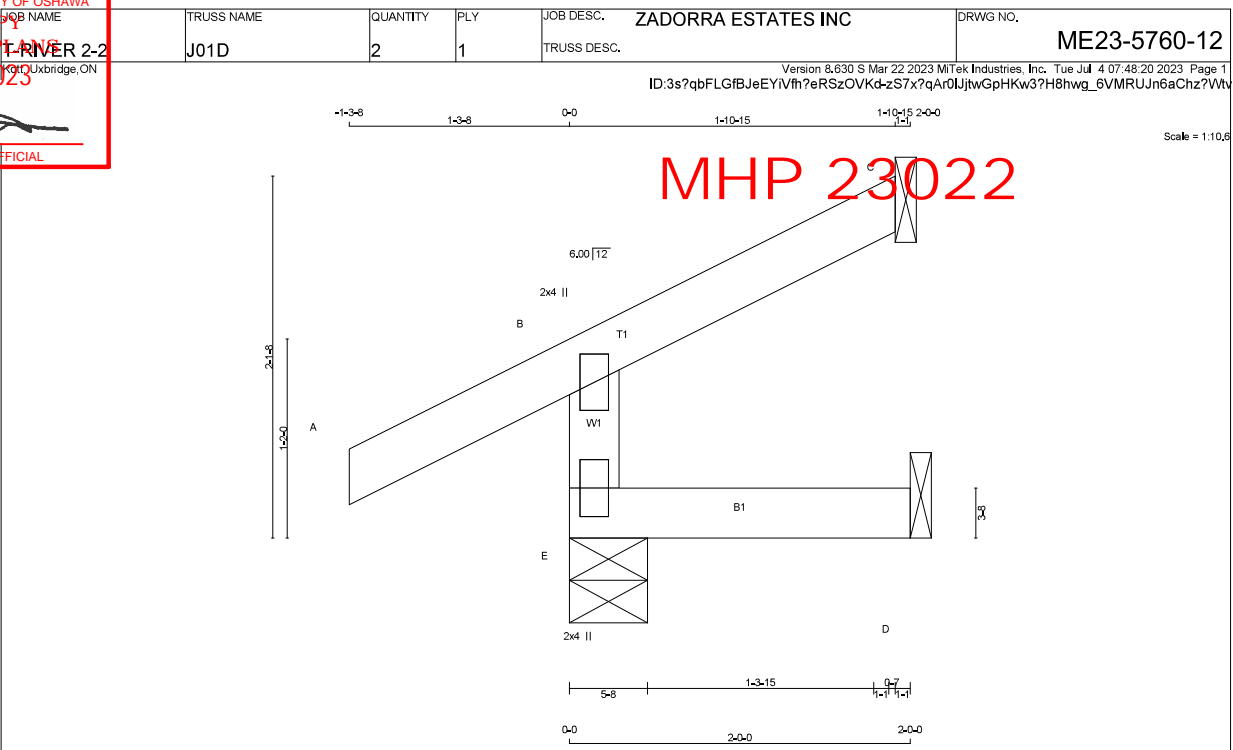
PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
E - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	FACTORED GROSS REACTION UP	INPUT BRG IN-SX	REQD BRG IN-SX
E	324	0	324	0	5-8
C	86	0	86	0	1-8
D	16	0	18	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	224	177 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM	TO	FR-TO			
E-B	-304 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.


NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

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REVIEW FOR TRUSS COMPONENT ONLY

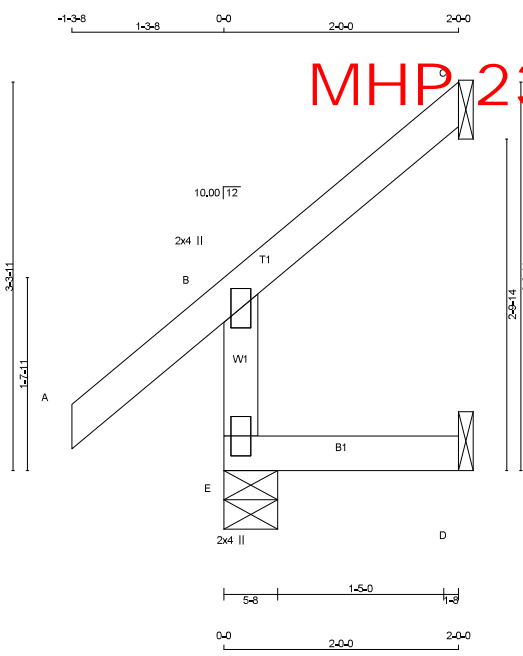
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



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JOB NAME T-RIVER 2-2	TRUSS NAME J02	QUANTITY 4	PLY 1	JOB DESC. ZADORRA ESTATES INC	TRUSS DESC.	DRWG NO. ME23-5760-13
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MHP 23022

LUMBER				DESCR.	
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	SPF	SPF
E - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQRD BRG IN-SX	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	334	0	334	0	0	5-8	1-8	1-8	1-8
C	90	0	90	0	0	1-8	1-8	1-8	1-8
D	17	0	19	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	230	183 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	62	53 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (5)									
C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. FROM	LOAD LC1 TO	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
E-B	-314 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00				
B-C	-19 / 0	-119.4	-119.4	0.08 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

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- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.10/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

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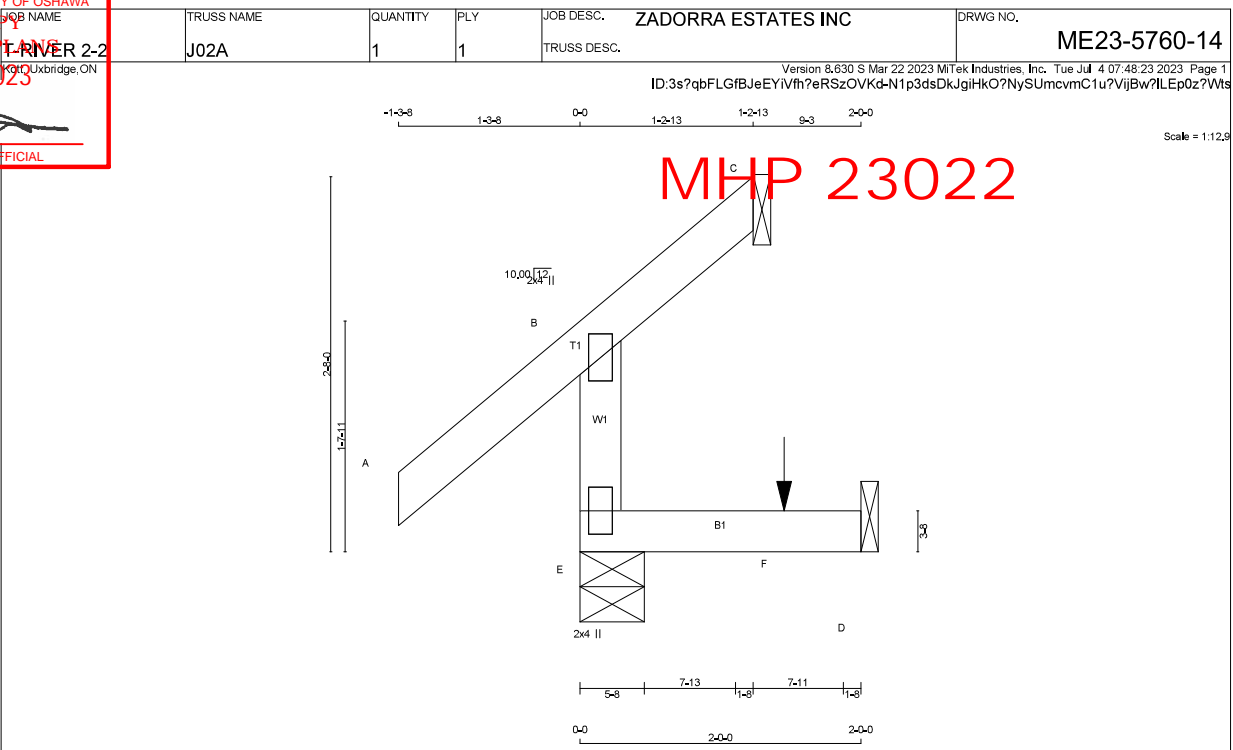
REVIEW FOR TRUSS COMPONENT ONLY

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TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	2.0	4.0	X
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	IN-SX	IN-SX	BRG	BRG
E	344	0	344	0	0	5-8	1-8		
C	0	0	2	0	-65	1-8	1-8		
D	8	0	20	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
E	COMBINED	SNOW	LIVE	PERM. LIVE	0/0	47/0
C	0	0/-42	0/0	0/0	0/0	1/0
D	8	0/-7	0/0	0/0	0/0	14/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX. MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO				
E-B	-313/0	0.0	0.0	0.04 (5)	7.81		
A-B	0/53	-119.4	-119.4	0.16 (1)	10.00		
B-C	-47/0	-119.4	-119.4	0.12 (1)	6.25		
E-F	0/0	-18.2	-18.2	0.04 (5)	10.00		
F-D	0/0	-18.2	-18.2	0.04 (5)	10.00		

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX-	MAX+	FRONT	VERT	TOTAL			C1	
F	1-5-7	-2									

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI TC=0.16/1.00 (A-B 1), BC=0.04/1.00 (D-E 5), WB=0.00/1.00 (n/a 0), SSI=0.10/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PU)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



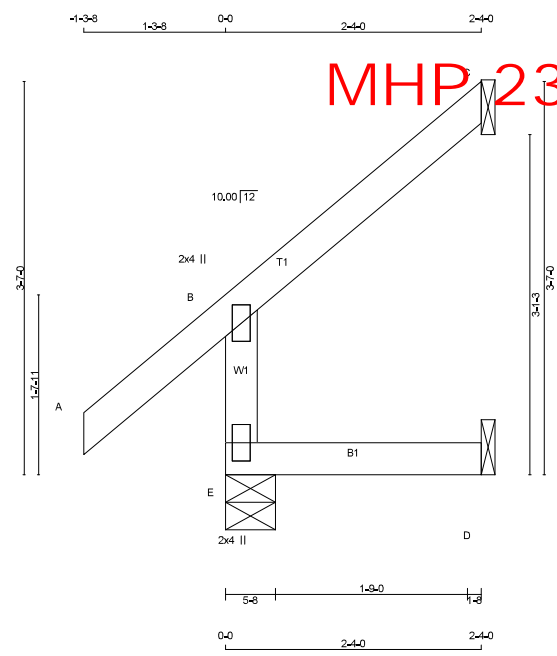
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

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Nov 23 2023
PER: 
CHIEF BUILDING OFFICIAL

PROJECT NAME T-RIVER 2-2 Kitchener, ON	TRUSS NAME J03	QUANTITY 3	PLY 1	JOB DESC. ZADORRA ESTATES INC	TRUSS DESC.	DRWG NO. ME23-5760-16
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Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 4 07:48:26 2023 Page 1
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MHP 23022

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX			
	VERT	HORZ	DOWN	HORZ		UP	DOWN		
E	362	0	362	0	5-8	1-8	1-8		
C	105	0	105	0	1-8	1-8	1-8		
D	19	0	22	0	1-8	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	198 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	72	81 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (7)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH FR-TO	
FR-TO									
E-B	-339 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00				
B-C	-22 / 0	-119.4	-119.4	0.11 (6)	6.25				
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 34.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.3 PSF
TOTAL LOAD	= 48.1 PSF

SPACING = 24.0 IN. G/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4),
WB=0.00/1.00 (n/a 0), SSI=0.10/1.00 (B-C 6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI) (PU)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

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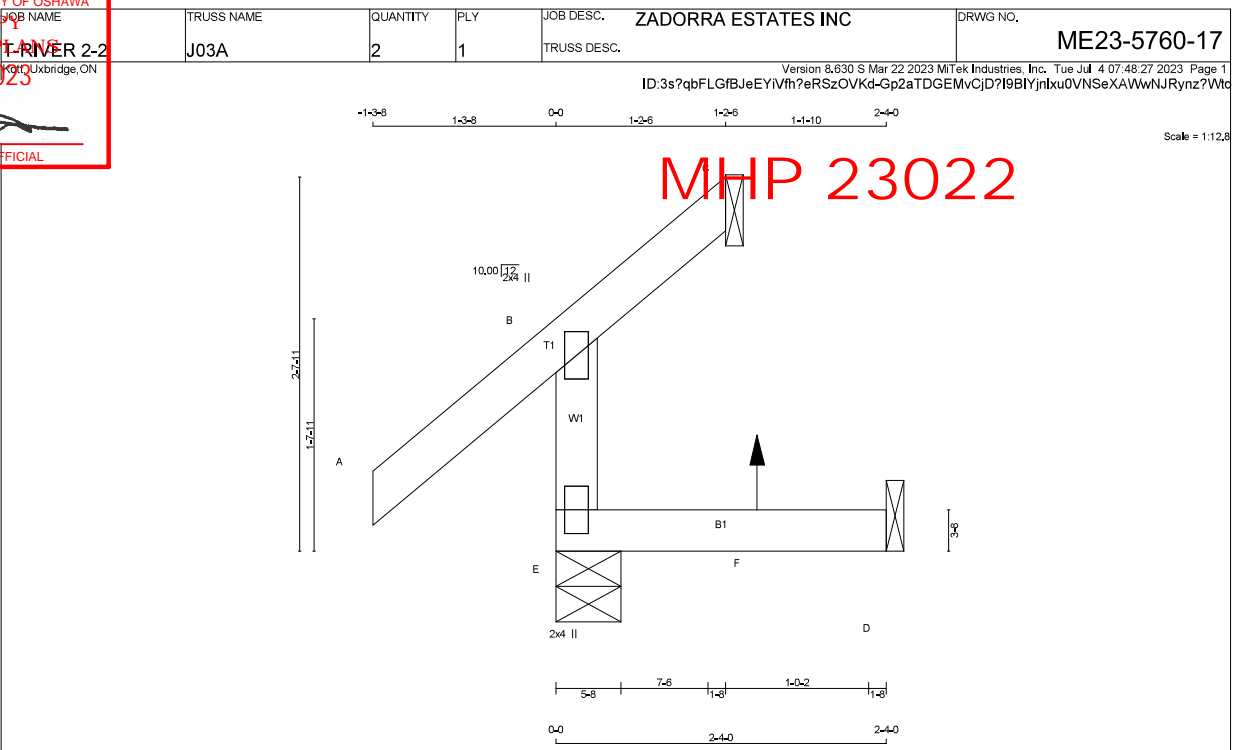


REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
E - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
E	341	0	341	0
C	-5	0	2	0
D	5	0	21	0

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
E	235	187 / 0	0 / 0	0 / 0	0 / 0
C	-3	0 / -44	0 / 0	0 / 0	2 / 0
D	6	0 / -12	0 / 0	0 / 0	15 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (9)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	W E B S	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
E-B	-315 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00		
B-C	-50 / 0	-119.4	-119.4	0.13 (6)	6.25		
E-F	0 / 0	-18.2	-18.2	0.04 (5)	10.00		
F-D	0 / 0	-18.2	-18.2	0.04 (5)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-5-1	11		18	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")
CSI: TC=0.16/1.00 (A-B 1), BC=0.04/1.00 (D-E 5), WB=0.00/1.00 (n/a 0), SSI=0.10/1.00 (A-B 1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PU)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

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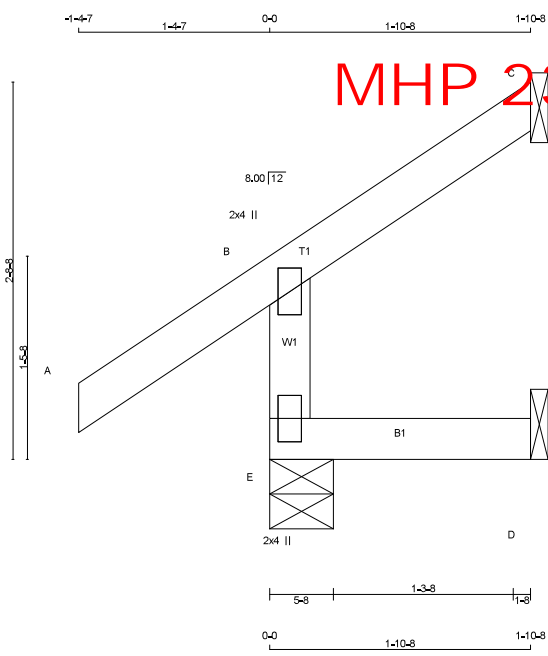
REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



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PROJECT NAME RIVER 2-2	TRUSS NAME J03B	QUANTITY 2	PLY 1	JOB DESC. ZADORRA ESTATES INC	DRWG NO. ME23-5760-18
Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 4 07:48:28 2023 Page 1 ID:3s?qbFLGfBJeEYIVh?eRSzOVkd-k?cygZGs7CKar9tL73yJzT3YvjRN_Qf912?VDz?Wtr					



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF

E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	369	0	369	0
C	58	0	58	0
D	4	0	16	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX.	MIN.	PERM.	LIVE	WIND	DEAD	SOIL
E	254	205 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0	
C	40	33 / -27	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0	
D	5	0 / -12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-339 / 0	0.0	0.0	0.05 (5)	7.81		
A-B	0 / 48	-119.4	-119.4	0.18 (1)	10.00		
B-C	-30 / 0	-119.4	-119.4	0.14 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.05 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.18/1.00 (A-B 1), BC=0.05/1.00 (D-E 5), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

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VOIDS THE ENGINEER'S SEAL