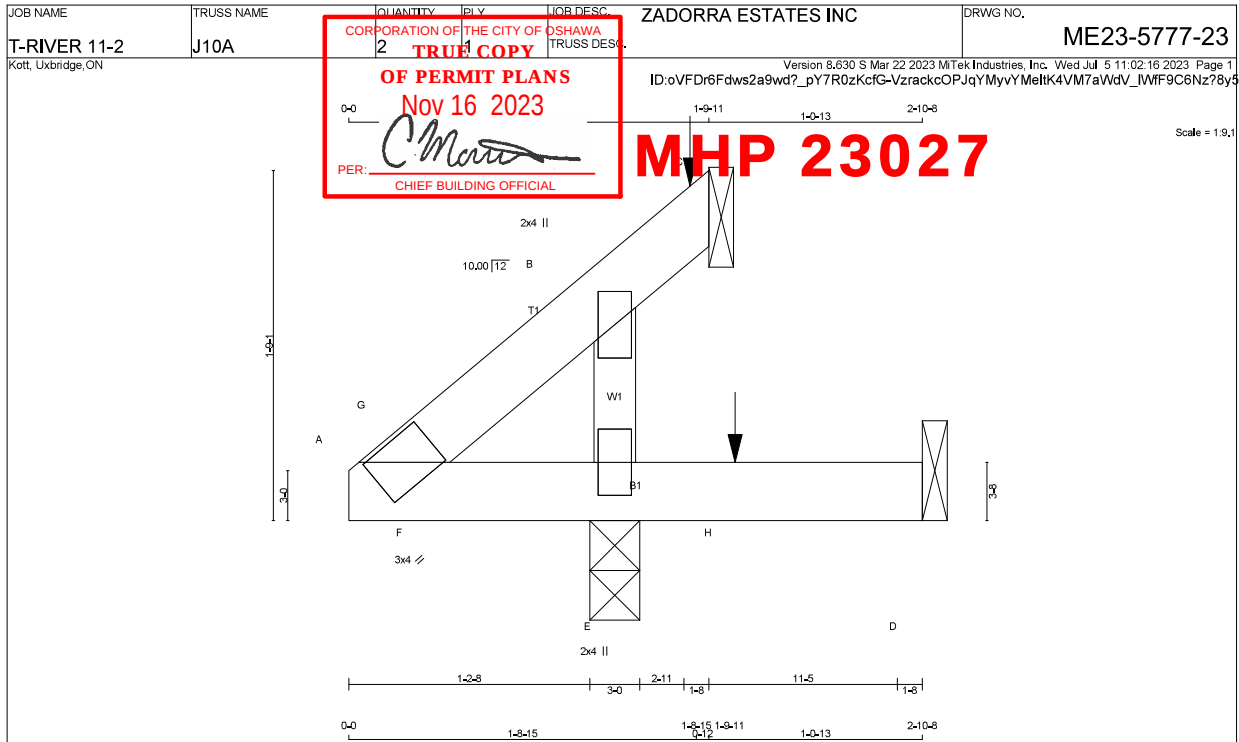
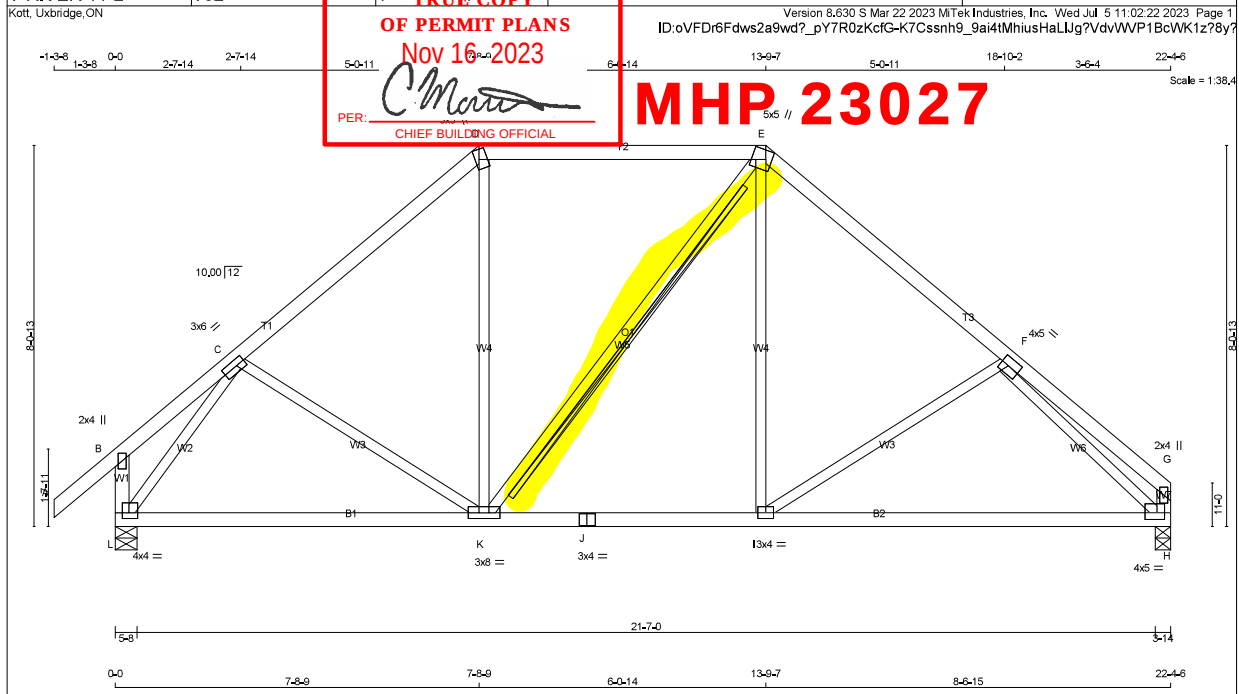




LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER A - C 2x4 DRY No.2 A - D 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER.		DESCR. SPF SPF
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM-h MT20 3.0 4.0 B TMW+w MT20 2.0 4.0 E BMW1+w MT20 2.0 4.0		
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX C 15 0 15 0 -97 1-8 1-8 D -19 0 6 0 -31 1-8 1-8 E 400 0 400 0 0 3-0 1-8 SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT UNFACTORED REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL C 10 12 / -60 0 / 0 0 / 0 1 / -2 0 / 0 D -12 0 / -22 0 / 0 0 / 0 5 / 0 0 / 0 E 280 199 / 0 0 / 0 0 / 0 83 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (5) CHORDS MAX. FACTORED FACTORED WEBS MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO A-G -8 / 0 -119.4 -119.4 0.02 (1) 10.00 E-B -236 / 0 0.03 (1) G-B 0 / 12 -119.4 -119.4 0.10 (5) 10.00 F-G -50 / 0 0.00 (1) B-C -73 / 0 -119.4 -119.4 0.09 (5) 6.25 A-F 0 / 25 -18.3 -18.3 0.01 (5) 10.00 F-E 0 / 23 -18.2 -18.2 0.10 (5) 10.00 E-H 0 / 0 -18.2 -18.2 0.10 (5) 10.00 H-D 0 / 0 -18.2 -18.2 0.10 (5) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. C 1-9-11 -92 -92 --- FRONT VERT TOTAL --- C1 H 1-11-4 -35 -35 --- FRONT VERT TOTAL --- C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED. CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN PATTERN LOADING CHECK APPLIED TO THIS TRUSS.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.00") CANTILEVER DEFLECTION: ALLOWABLE DEFL.(LL) = L/120 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.01") ALLOWABLE DEFL.(TL) = L/120 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.02") CSI TC=0.10/1.00 (B-G-5), BC=0.10/1.00 (E-F-5), WB=0.03/1.00 (B-E-1), SSI=0.09/1.00 (B-C-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



TOTAL WEIGHT = 97 TONS		[M][F]																																																																																																										
LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - E 2x4 DRY 2100F 1,8E SPF E - G 2x4 DRY No.2 SPF L - B 2x4 DRY No.2 SPF H - G 2x4 DRY No.2 SPF L - J 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX L 1705 0 1705 0 0 5-8 1-14 H 1540 0 1540 0 0 3-14 1-11 UNFACTORED REACTIONS 1ST CASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL L 1189 875 / 0 0 / 0 0 / 0 314 / 0 0 / 0 H 1076 778 / 0 0 / 0 0 / 0 297 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 2x4 DRY SPF No.2 T-BRACE AT E-K FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="3">CHORDS</th><th colspan="3">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. CS1 (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. CS1 (LC)</th></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 53</td><td>-119.4 -119.4</td><td>0.16 (1)</td><td>10.00</td><td>C-K</td><td>-64 / 35 0.05 (1)</td></tr><tr><td>B-C</td><td>0 / 71</td><td>-119.4 -119.4</td><td>0.39 (1)</td><td>10.00</td><td>K-D</td><td>0 / 230 0.05 (4)</td></tr><tr><td>C-D</td><td>-1356 / 0</td><td>-119.4 -119.4</td><td>0.41 (1)</td><td>5.09</td><td>K-E</td><td>-80 / 0 0.06 (1)</td></tr><tr><td>D-E</td><td>-1016 / 0</td><td>-119.4 -119.4</td><td>0.38 (1)</td><td>6.25</td><td>E-F</td><td>0 / 335 0.08 (1)</td></tr><tr><td>E-F</td><td>-1418 / 0</td><td>-119.4 -119.4</td><td>0.41 (1)</td><td>5.01</td><td>F-G</td><td>-343 / 0 0.24 (1)</td></tr><tr><td>F-G</td><td>0 / 54</td><td>-119.4 -119.4</td><td>0.40 (1)</td><td>10.00</td><td>L-C</td><td>-1792 / 0 0.52 (1)</td></tr><tr><td>L-B</td><td>-205 / 0</td><td>0.0 0.0</td><td>0.02 (1)</td><td>7.81</td><td>F-H</td><td>-1910 / 0 0.72 (1)</td></tr><tr><td>H-G</td><td>-119 / 0</td><td>0.0 0.0</td><td>0.01 (1)</td><td>7.81</td><td></td><td></td></tr><tr><td>L-K</td><td>0 / 1066</td><td>-18.2 -18.2</td><td>0.31 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>K-J</td><td>0 / 1065</td><td>-18.2 -18.2</td><td>0.34 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>J-I</td><td>0 / 1065</td><td>-18.2 -18.2</td><td>0.34 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>I-H</td><td>0 / 1348</td><td>-18.2 -18.2</td><td>0.38 (1)</td><td>10.00</td><td></td><td></td></tr></table>		CHORDS			WEBS			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	FR-TO		FROM TO		FR-TO			A-B	0 / 53	-119.4 -119.4	0.16 (1)	10.00	C-K	-64 / 35 0.05 (1)	B-C	0 / 71	-119.4 -119.4	0.39 (1)	10.00	K-D	0 / 230 0.05 (4)	C-D	-1356 / 0	-119.4 -119.4	0.41 (1)	5.09	K-E	-80 / 0 0.06 (1)	D-E	-1016 / 0	-119.4 -119.4	0.38 (1)	6.25	E-F	0 / 335 0.08 (1)	E-F	-1418 / 0	-119.4 -119.4	0.41 (1)	5.01	F-G	-343 / 0 0.24 (1)	F-G	0 / 54	-119.4 -119.4	0.40 (1)	10.00	L-C	-1792 / 0 0.52 (1)	L-B	-205 / 0	0.0 0.0	0.02 (1)	7.81	F-H	-1910 / 0 0.72 (1)	H-G	-119 / 0	0.0 0.0	0.01 (1)	7.81			L-K	0 / 1066	-18.2 -18.2	0.31 (1)	10.00			K-J	0 / 1065	-18.2 -18.2	0.34 (4)	10.00			J-I	0 / 1065	-18.2 -18.2	0.34 (4)	10.00			I-H	0 / 1348	-18.2 -18.2	0.38 (1)	10.00			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL)= L/999 (0.04") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL)= L/999 (0.19") CSI: TC=0.41/1.00 (C-D:1), BC=0.38/1.00 (H-I:1), WB=0.72/1.00 (F-H:1), SSI=0.28/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (C) (INPUT = 0.90) JSI METAL= 0.49 (F) (INPUT = 1.00)
CHORDS			WEBS																																																																																																									
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MODULUS ENGINEERING LTD.																																																																																																												
																																																																																																												
REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL																																																																																																												

Version 8,630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 5 11:02:23 2023 Page 1
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TOTAL WEIGHT = 106 lb

<u>DESIGN CRITERIA</u>		
SPECIFIED LOADS:		
TOP CH.	LL =	34.8 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.3 PSF
TOTAL LOAD	=	48.1 PSF
<u>SPACING =</u>	<u>240</u>	<u>IN. C/C</u>
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM		

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
2x4 DRY SPF No.2 T-BRACE AT E-K
FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90%
OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW
LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS

WEBS

CHORDS				WEBS			
MEMB.	MAX. FACTORED (LBS)	FACTORED VERT. LOAD (PLF)	LO1 CSI (LC)	LO2 MAXBR CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0/53	-119.4	-119.4	0.16 (1)	10.00	C-K	-237 / 0
C-B	0/75	-119.4	-119.4	0.54 (1)	10.00	K-D	0 / 316
D-C	-1258 / 0	-119.4	-119.4	0.55 (1)	5.00	K-E	-74 / 0
E-F	-337 / 0	-119.4	-119.4	0.14 (1)	6.25	I-E	0 / 408
F-G	-1287 / 0	-119.4	-119.4	0.55 (1)	4.97	I-F	-472 / 0
E-F	0/62	-119.4	-119.4	0.56 (1)	10.00	L-C	-1789 / 0
L-B	-247 / 0	0.0	0.0	0.03 (1)	7.81	F-H	-1877 / 0
H-G	-154 / 0	0.0	0.0	0.02 (1)	7.81		
K-K	0 / 1130	-18.2	-18.2	0.40 (4)	10.00		
L-K	0 / 959	-18.2	-18.2	0.48 (4)	10.00		
J-I	0 / 959	-18.2	-18.2	0.48 (4)	10.00		
I-H	0 / 1346	-18.2	-18.2	0.49 (4)	10.00		

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48,1 P.S.F. G.S.I. PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF_L(L_L)= L/360 (0,75")
CALCULATED VERT. DEF_L(L_L)= L/ 999 (0,04")
ALLOWABLE DEF_L(T_L)= L/360 (0,75")
CALCULATED VERT. DEF_L(T_L)= L/ 819 (0,33")

CSI TC=0,56/1,0 (F-G-1), BC=0,49/1,0 (H-I-4), WB=80,0/1,0 (C-L-1), SSI=0,25/1,0 (E-F-1)

DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,10 COMP=1,10 SHEAR=1,10 TENS=1,10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0,250 inches

PLATE ROTATION TOL. = 5,0 Deg.

JSI GRIP= 0,90 (D) (INPUT = 0,90)

MODULUS ENGINEERING LTD.

07/05/2023

D. A. SHERMAN

100123373

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

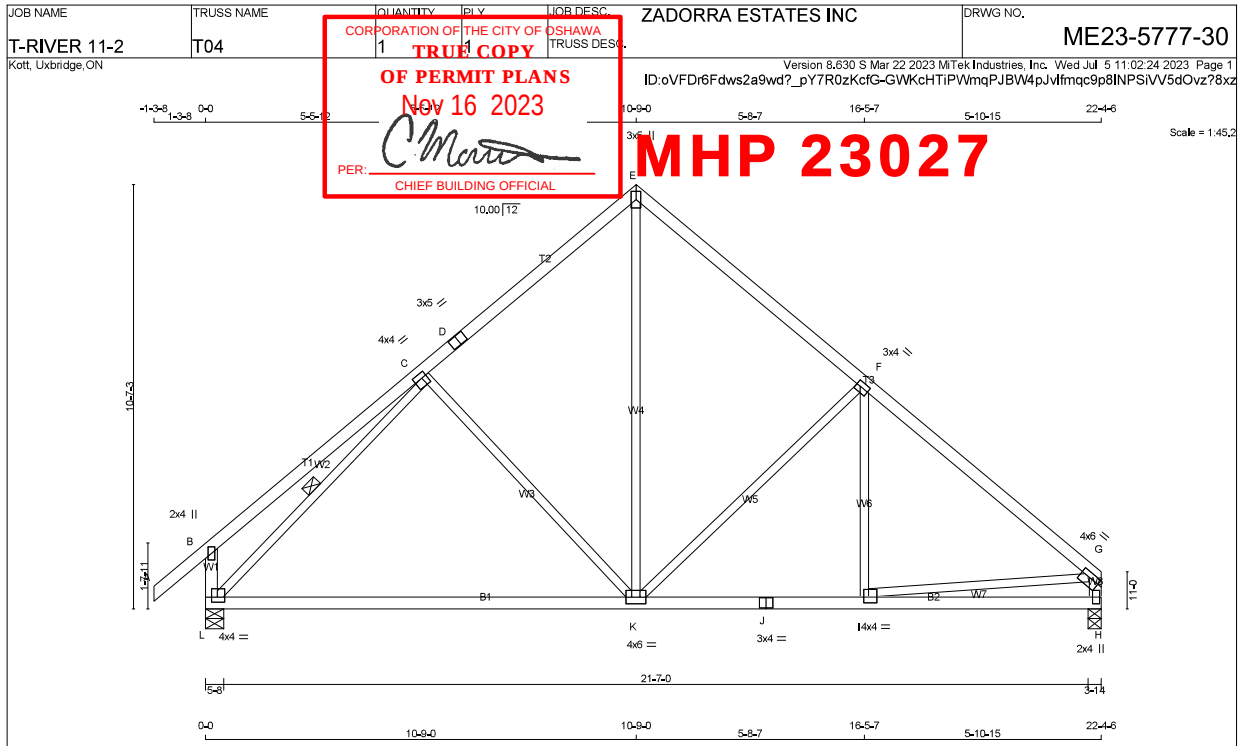
*NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL*

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com

 **KOTT**





TOTAL WEIGHT = 98 lbs																																																																																																																																																																																																																																									
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LUMBER N, L, G, A, RULES <table><tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr><tr><td>A - D</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>D - E</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>E - G</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>L - B</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>H - G</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>L - J</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr><tr><td>J - H</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr></table> ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER.		CHORDS	SIZE	LUMBER	DESCR.	A - D	2x4	DRY No.2	SPF	D - E	2x4	DRY No.2	SPF	E - G	2x4	DRY No.2	SPF	L - B	2x4	DRY No.2	SPF	H - G	2x4	DRY No.2	SPF	L - J	2x4	DRY No.2	SPF	J - H	2x4	DRY No.2	SPF	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr><tr><td>JT</td><td>VERT</td><td>DOWN</td><td>UP</td><td>IN-SX</td></tr><tr><td>L</td><td>1705</td><td>0</td><td>1705</td><td>0</td></tr><tr><td>H</td><td>1540</td><td>0</td><td>1540</td><td>0</td></tr></table> UNFACTORED REACTIONS <table><tr><th>JT</th><th>1ST CASE</th><th>SNOW</th><th>MAX./MIN. COMPONENT REACTIONS</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>L</td><td>1189</td><td>875 / 0</td><td>0 / 0</td><td>0 / 0</td><td>314 / 0</td><td>0 / 0</td></tr><tr><td>H</td><td>1076</td><td>778 / 0</td><td>0 / 0</td><td>0 / 0</td><td>297 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT. MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF C4L. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX, UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>VERT. LOAD (PLF)</th><th>MAX. CSI (LC)</th><th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>MAX. CSI (LC)</th><th></th></tr><tr><td>FR-TO</td><td>FROM</td><td>TO</td><td>LENGTH</td><td>FR-TO</td><td>FROM</td><td>TO</td><td>LENGTH</td></tr><tr><td>A-B</td><td>0 / 53</td><td>-119.4</td><td>-119.4</td><td>0.16 (1)</td><td>C-K</td><td>-400 / 0</td><td>0.52 (1)</td></tr><tr><td>B-C</td><td>0 / 50</td><td>-119.4</td><td>-119.4</td><td>0.57 (1)</td><td>K-E</td><td>0 / 928</td><td>0.21 (1)</td></tr><tr><td>C-D</td><td>-1163 / 0</td><td>-119.4</td><td>-119.4</td><td>0.47 (1)</td><td>K-F</td><td>-628 / 0</td><td>0.82 (1)</td></tr><tr><td>D-E</td><td>-1163 / 0</td><td>-119.4</td><td>-119.4</td><td>0.47 (1)</td><td>F-F</td><td>-50 / 57</td><td>0.03 (1)</td></tr><tr><td>E-F</td><td>-1169 / 0</td><td>-119.4</td><td>-119.4</td><td>0.55 (1)</td><td>L-C</td><td>-1660 / 0</td><td>0.68 (1)</td></tr><tr><td>F-G</td><td>-1649 / 0</td><td>-119.4</td><td>-119.4</td><td>0.60 (1)</td><td>I-G</td><td>0 / 1314</td><td>0.30 (1)</td></tr><tr><td>L-B</td><td>-407 / 0</td><td>0.0</td><td>0.0</td><td>0.04 (1)</td><td></td><td></td><td></td></tr><tr><td>H-G</td><td>-1488 / 0</td><td>0.0</td><td>0.0</td><td>0.15 (1)</td><td></td><td></td><td></td></tr><tr><td>L-K</td><td>0 / 1130</td><td>-18.2</td><td>-18.2</td><td>0.52 (4)</td><td></td><td></td><td></td></tr><tr><td>K-J</td><td>0 / 1307</td><td>-18.2</td><td>-18.2</td><td>0.53 (4)</td><td></td><td></td><td></td></tr><tr><td>J-I</td><td>0 / 1307</td><td>-18.2</td><td>-18.2</td><td>0.53 (4)</td><td></td><td></td><td></td></tr><tr><td>I-H</td><td>0 / 0</td><td>-18.2</td><td>-18.2</td><td>0.16 (4)</td><td></td><td></td><td></td></tr></table>		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	DOWN	UP	IN-SX	L	1705	0	1705	0	H	1540	0	1540	0	JT	1ST CASE	SNOW	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL	L	1189	875 / 0	0 / 0	0 / 0	314 / 0	0 / 0	H	1076	778 / 0	0 / 0	0 / 0	297 / 0	0 / 0	CHORDS				WEBS				MEMB.	FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FACTORED FORCE (LBS)	MAX. CSI (LC)		FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH	A-B	0 / 53	-119.4	-119.4	0.16 (1)	C-K	-400 / 0	0.52 (1)	B-C	0 / 50	-119.4	-119.4	0.57 (1)	K-E	0 / 928	0.21 (1)	C-D	-1163 / 0	-119.4	-119.4	0.47 (1)	K-F	-628 / 0	0.82 (1)	D-E	-1163 / 0	-119.4	-119.4	0.47 (1)	F-F	-50 / 57	0.03 (1)	E-F	-1169 / 0	-119.4	-119.4	0.55 (1)	L-C	-1660 / 0	0.68 (1)	F-G	-1649 / 0	-119.4	-119.4	0.60 (1)	I-G	0 / 1314	0.30 (1)	L-B	-407 / 0	0.0	0.0	0.04 (1)				H-G	-1488 / 0	0.0	0.0	0.15 (1)				L-K	0 / 1130	-18.2	-18.2	0.52 (4)				K-J	0 / 1307	-18.2	-18.2	0.53 (4)				J-I	0 / 1307	-18.2	-18.2	0.53 (4)				I-H	0 / 0	-18.2	-18.2	0.16 (4)				DESIGN CRITERIA SPECIFIED LOADS: <table><tr><td>TOP CH.</td><td>LL</td><td>=</td><td>34.8</td><td>PSF</td></tr><tr><td></td><td>DL</td><td>=</td><td>6.0</td><td>PSF</td></tr><tr><td>BOT CH.</td><td>LL</td><td>=</td><td>0.0</td><td>PSF</td></tr><tr><td></td><td>DL</td><td>=</td><td>7.3</td><td>PSF</td></tr><tr><td>TOTAL LOAD</td><td>=</td><td>48.1</td><td>PSF</td><td></td></tr></table> SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL) = L/999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL) = L/694 (0.39") CSI: TC=0.60/1.00 (F-G:1), BC=0.53/1.00 (I-K:4), WB=0.82/1.00 (F-K:1), SS=0.26/1.00 (F-G:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES <table><tr><th>PLATE</th><th>GRIP (DRY)</th><th>SHEAR (PSI)</th><th>SECTION (PL)</th></tr><tr><td>MT20</td><td>650</td><td>371</td><td>1747</td></tr><tr><td></td><td>788</td><td>1987</td><td>1873</td></tr></table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.85 (H) (INPUT = 0.90) JSI METAL= 0.55 (C) (INPUT = 1.00)	TOP CH.	LL	=	34.8	PSF		DL	=	6.0	PSF	BOT CH.	LL	=	0.0	PSF		DL	=	7.3	PSF	TOTAL LOAD	=	48.1	PSF		PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)	MT20	650	371	1747		788	1987	1873
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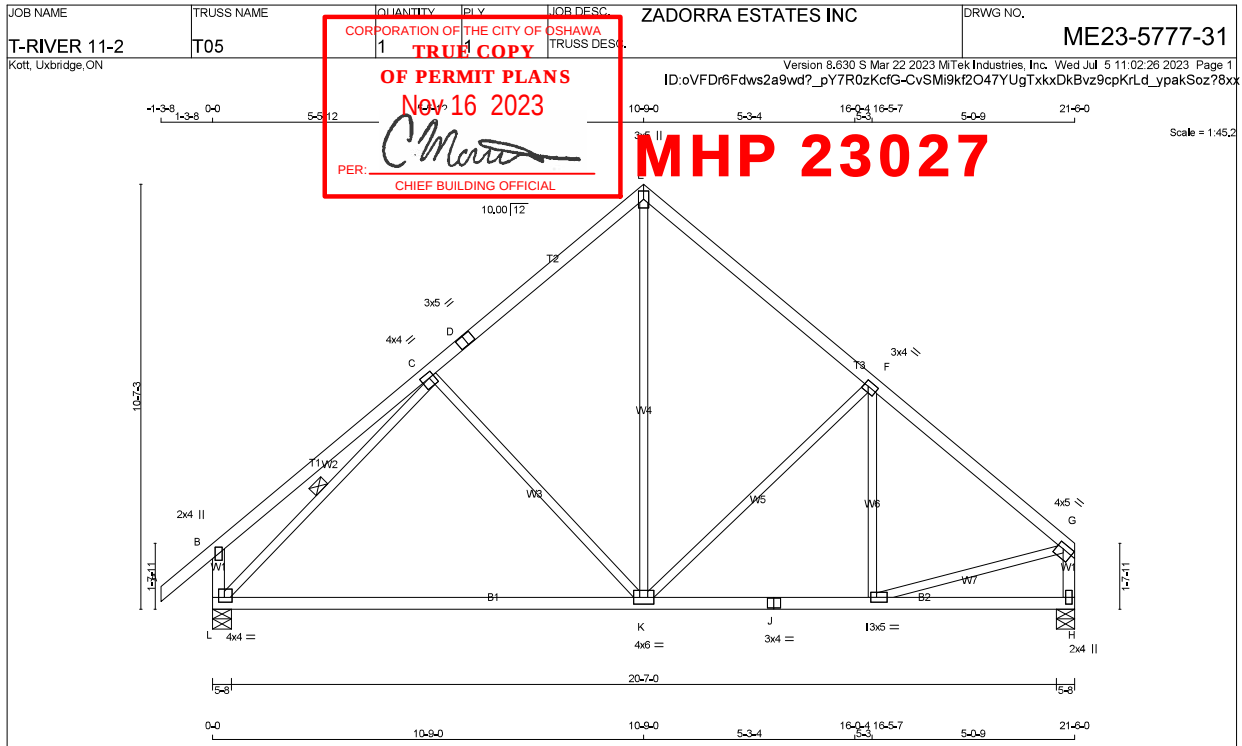
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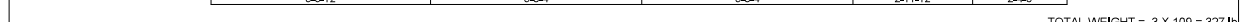
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REVIEW FOR TRUSS COMPONENT ONLY

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LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER DESCR, SPF A - D 2x4 DRY No.2 D - E 2x4 DRY No.2 E - G 2x4 DRY No.2 L - B 2x4 DRY No.2 H - G 2x4 DRY No.2 L - J 2x4 DRY No.2 J - H 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY, SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX H 1480 0 1480 0 0 5-8 1-10 L 1645 0 1645 0 0 5-8 1-13 UNFACTORED REACTIONS 1ST CASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL H 1034 748 / 0 0 / 0 0 / 0 286 / 0 0 / 0 L 1147 845 / 0 0 / 0 0 / 0 303 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT. MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX, UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB, FACTORED VERT. LOAD LC1 MAX MAX. FACTORED FORCE (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO MEMB, FORCE MAX (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 53 -119.4 -119.4 0.16 (1) 10.00 C-K -411 / 0 0.53 (1) B-C 0 / 50 -119.4 -119.4 0.57 (1) 10.00 K-E 0 / 811 0.18 (1) C-D -1082 / 0 -119.4 -119.4 0.46 (1) 5.44 K-F -448 / 0 0.58 (1) D-E -1082 / 0 -119.4 -119.4 0.46 (1) 5.44 L-G 0 / 1152 0.26 (1) E-F -1081 / 0 -119.4 -119.4 0.50 (1) 5.40 I-F -221 / 1 0.12 (1) F-G -1398 / 0 -119.4 -119.4 0.48 (1) 4.91 L-C -1580 / 0 0.65 (1) L-B -407 / 0 0.0 0.04 (1) 7.81 H-G -1435 / 0 0.0 0.0 0.15 (1) 6.82 L-K 0 / 1075 -18.2 -18.2 0.52 (4) 10.00 K-J 0 / 1115 -18.2 -18.2 0.53 (4) 10.00 J-I 0 / 1115 -18.2 -18.2 0.53 (4) 10.00 I-H 0 / 0 -18.2 -18.2 0.12 (4) 10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.72") CALCULATED VERT. DEFL.(LL) = L/999 (0.04") ALLOWABLE DEFL.(TL) = L/360 (0.72") CALCULATED VERT. DEFL.(TL) = L/684 (0.38") CSI: TC=0.57/1.00 (B-C:1), BC=0.53/1.00 (I-K:4), WB=0.65/1.00 (C-L:1), SSH=0.25/1.00 (E-F:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.85 (I) (INPUT = 0.90) JSI METAL= 0.53 (C) (INPUT = 1.00)
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					MAX./MIN. COMPONENT REACTIONS							THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 0. N.B.A.C. 1015.
					1ST CASE							
REINFORCING MEMBERS				JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
HW2	2x6	DRY	No.2	SPF	I	1056	744 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
					O	1542	948 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.	- CSA 086-14 - TPIC 2014
ALL RIGID BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.	DESIGN ASSUMPTIONS

D	TS4	MT20	3.0	6.0	CHORDS		WEBS		ROOF LIVE LOAD
E	TTW+p	MT20	3.0	5.0	MAX. FACTORED		MAX. FACTORED		
F	MMWWH	MT20	3.0	8.0	MEMB. FORCE VERT. LOAD LC1		MAX. MAX. MEMB. FORCE MAX		ALLOWABLE DEFL.(LL)= 1/360 (0.71")
G	TMW	MT20	3.0	8.0	1/360 (0.71")		1/360 (0.71")		ALLOWABLE DEFL.(LL)= 1/360 (0.71")

Q-B	1/39770	0.0	0.0	0.17 (1)	0.34	R-B	1/17070	0.06 (1)
Q-P	0/0	-18.2	-18.2	0.12 (4)	10.00	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE		
P-O	0/1102	-18.2	-18.2	0.24 (1)	10.00			

	R-7	0' 9.68'	-10,2	-10,2	0,27 (-)	10,00	PLATE PLACEMENT TOL. = 0,250 inches PLATE ROTATION TOL. = 5,0 Deg.
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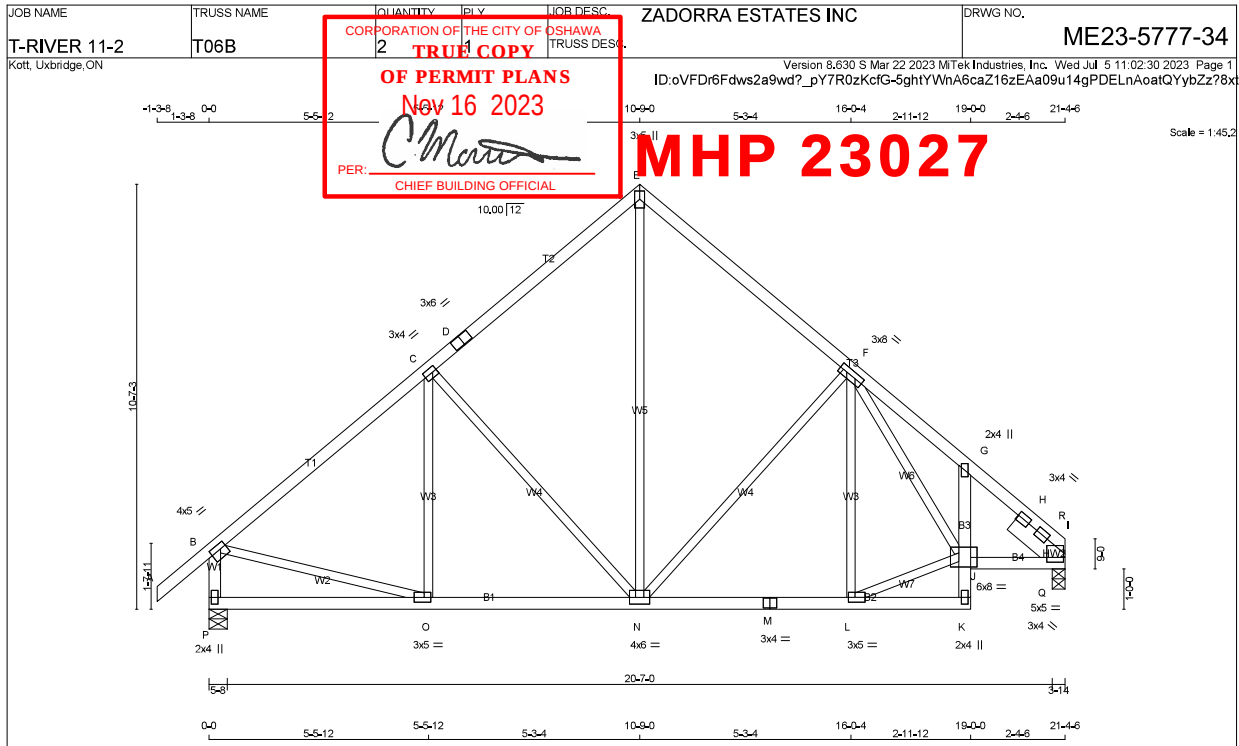
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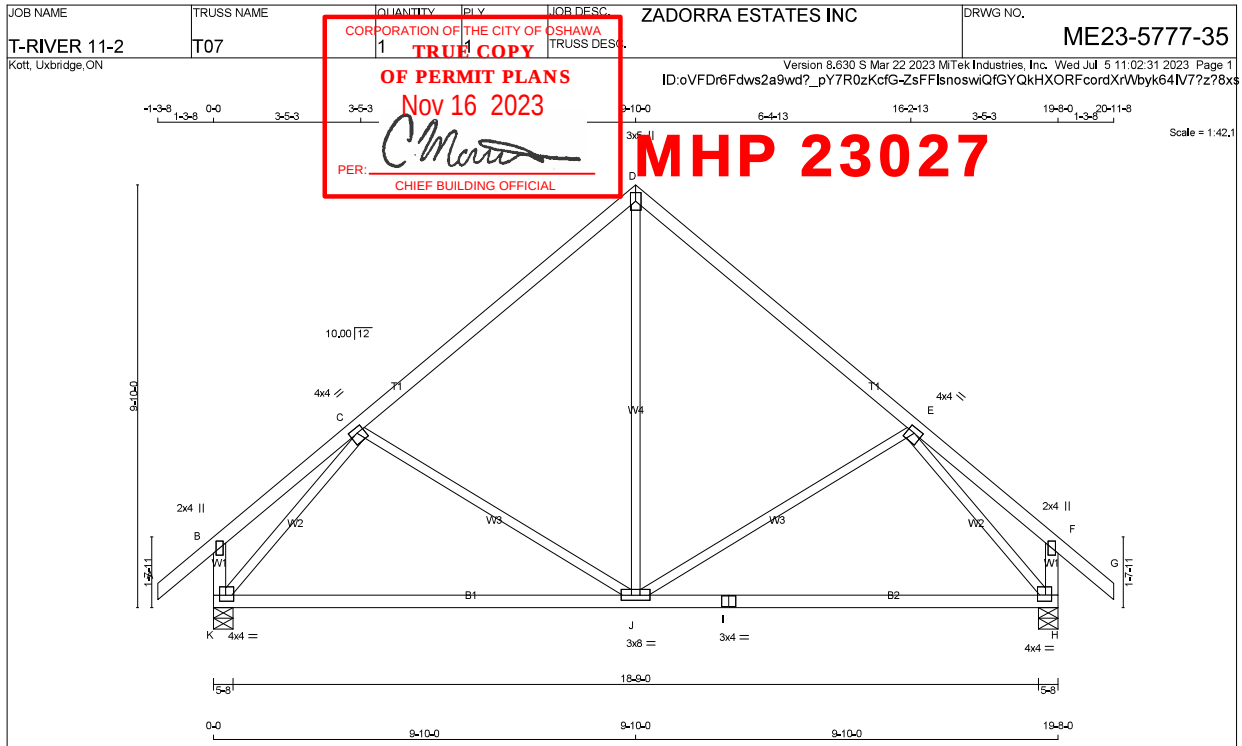


NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

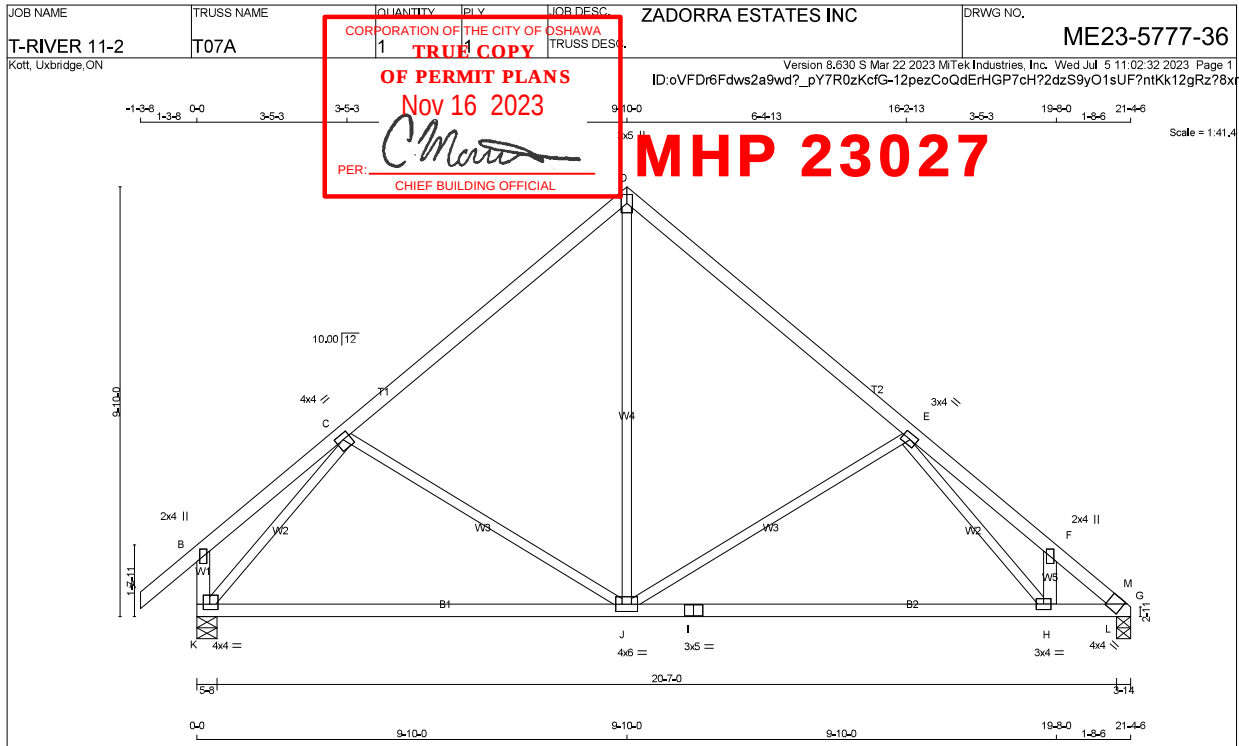
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI 781 N. Lee



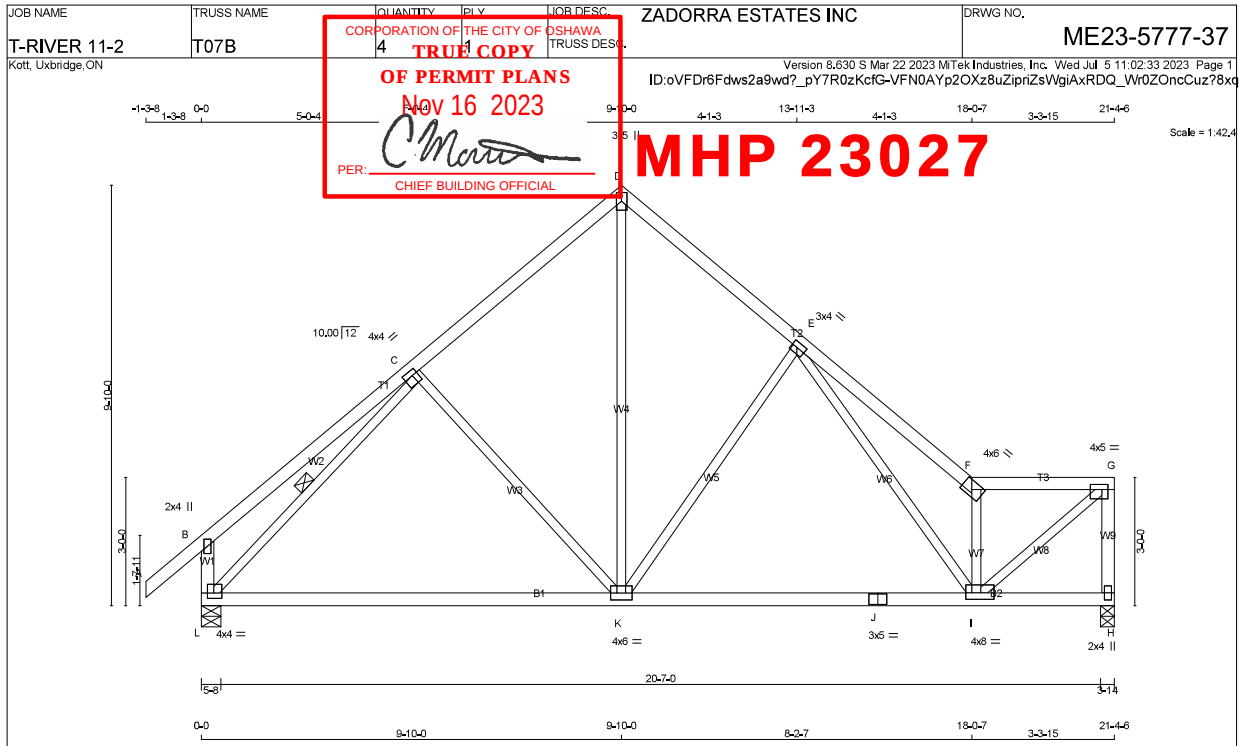
LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA			
N. L. G. A. RULES														[M][F]			
CHORDS				BEARINGS										SPECIFIED LOADS:			
A - D 2x4 DRY LUMBER				JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										TOP CH. LL = 34.8 PSF			
D - E 2x4 DRY No.2				I 1471 0 1471 0 0 3-14 1-9										BOT CH. LL = 0.0 PSF			
E - I 2x4 DRY No.2				P 1636 0 1636 0 0 5-8 1-13										DL = 7.3 PSF			
P - B 2x4 DRY No.2														TOTAL LOAD = 48.1 PSF			
P - M 2x4 DRY No.2																	
M - K 2x4 DRY No.2																	
K - G 2x4 DRY No.2																	
J - I 2x4 DRY No.2																	
REINFORCING MEMBERS				UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C			
HW2 2x6 DRY No.2				1ST CASE MAX./MIN. COMPONENT REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
ALL WEBS 2x3 DRY No.2				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										THIS DESIGN COMPLIES WITH:			
EXCEPT				I 1028 744 / 0 0 / 0 0 / 0 0 / 0 284 / 0 0 / 0										- PART 9 OF BCBC 2018, NBC-2019AE			
				P 1141 840 / 0 0 / 0 0 / 0 0 / 0 301 / 0 0 / 0										- PART 9 OF OBC 2012 (2019 AMENDMENT)			
														- CSA 086-14			
														- TPIC 2014			
DRY: SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P										(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD			
				BRACING										ALLOWABLE DEFL.(LL)= L/360 (0.71")			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.										CALCULATED VERT. DEFL.(LL) = L/999 (0.04")			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.										ALLOWABLE DEFL.(TL)= L/360 (0.71")			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.										CALCULATED VERT. DEFL.(TL) = L/999 (0.08")			
PLATES (table is in inches)				LOADING										CSI TC=0.49/1.00 (B-C:1), BC=0.33/1.00 (J-Q:1), WB=0.64/1.00 (C-N:1), SSH=0.25/1.00 (G-R:1)			
JT TYPE PLATES W LEN Y X				TOTAL LOAD CASES: (4)										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
B TMVW4 MT20 4.0 5.0 1.50 1.75				CHORDS										COMP=1.10 SHEAR=1.10 TENS=1.10			
C TMVW4 MT20 3.0 4.0 1.50 1.25				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										COMPANION LIVE LOAD FACTOR = 1.00			
D TS4 MT20 3.0 6.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
E TTV+p MT20 3.0 5.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										NAIL VALUES			
F TMVW4 MT20 3.0 8.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI) (PLI)			
G TMV+p MT20 2.0 4.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										MAX MIN MAX MIN MAX MIN			
I TMBMR1-I MT20 5.0 5.0 2.50 0.50				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										MT20 650 371 1747 788 1987 1873			
J RT4 MT20 3.0 4.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										PLATE PLACEMENT TOL. = 0.250 inches			
K BVMW4 MT20 6.0 8.0 4.25 5.50				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										PLATE ROTATION TOL. = 5.0 Deg.			
L BMV+p MT20 2.0 4.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										JSI GRIP= 0.89 (O) (INPUT = 0.90)			
M BMVW4 MT20 3.0 5.0 1.50 2.00				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO										JSI METAL= 0.48 (B) (INPUT = 1.00)			
N BS4 MT20 3.0 4.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO													
O BMVW4 MT20 4.0 6.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO													
P BMV1+p MT20 2.0 4.0				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO													
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				MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-													



TOTAL WEIGHT = 86 lb									
[M]F									
LUMBER									
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CHORDS SIZE LUMBER									
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF K - B 2x4 DRY No.2 SPF K - I 2x4 DRY No.2 SPF I - G 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT H - F 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED BRG INPUT REQD JT VERT DOWN HORZ UPLIFT IN-SX IN-SX G 1471 0 1471 0 0 3-14 1-9 K 1636 0 1636 0 0 5-8 1-12 UNFACTORED REACTIONS 1ST CASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL G 1028 744 / 0 0 / 0 0 / 0 284 / 0 0 / 0 K 1141 840 / 0 0 / 0 0 / 0 301 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. MAX. FACTORED VERT. LOAD (LBS) MAX. FACTORED (PLF) MAX. UNBRACED LENGTH (LC) MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED (LC) FR-TO FROM TO A-B 0 / 53 -119.4 -119.4 0.16 (1) 10.00 K-C -1729 / 0 0.77 (1) B-C 0 / 88 -119.4 -119.4 0.64 (1) 10.00 G-J -295 / 0 0.36 (1) C-D -1141 / 0 -119.4 -119.4 0.65 (1) 4.98 J-D 0 / 769 0.17 (1) D-E -1143 / 0 -119.4 -119.4 0.65 (1) 4.98 J-E -761 / 0 0.93 (1) E-F -2120 / 0 -119.4 -119.4 0.56 (1) 3.99 E-H 0 / 373 0.08 (1) F-M -2271 / 0 -119.4 -119.4 0.19 (1) 4.38 H-F -42 / 22 0.01 (4) M-G -2298 / 0 -119.4 -119.4 0.09 (1) 4.46 L-M -65 / 0 0.00 (1) K-B -225 / 0 0.0 0.0 0.02 (1) 7.81 K-J 0 / 1092 -18.2 -18.2 0.51 (4) 10.00 J-I 0 / 1488 -18.2 -18.2 0.54 (4) 10.00 I-H 0 / 1488 -18.2 -18.2 0.54 (4) 10.00 H-L 0 / 1720 -18.2 -18.2 0.49 (1) 10.00 L-G 0 / 1685 -18.2 -18.2 0.48 (1) 10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.71") CALCULATED VERT. DEFL.(LL) = L/999 (0.06") ALLOWABLE DEFL.(TL) = L/360 (0.71") CALCULATED VERT. DEFL.(TL) = L/999 (0.24") CSI TC=0.65/1.00 (C-D-1), BC=0.54/1.00 (H-J-4), WB=0.93/1.00 (E-J-1), SSI=0.28/1.00 (G-L-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (G) (INPUT = 0.90) JSI METAL= 0.57 (I) (INPUT = 1.00)
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[illegible]

						UNFACTORED REACTIONS							SPACING = 240 IN./C/C	
						1ST LCASE MAX./MIN. COMPONENT REACTIONS								
PLATES (table in inches)						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2018, NBC-2018/AE - PART 9 OF CBC 2012 (2018 AMENDMENT) - CSA 086-14
JT TYPE	PLATES	W	LEN	Y	X	A	227	165 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0	
A	TBM1+H	MT20	3.0	4.0		D	119	86 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0	
B	TMW+H	MT20	3.0	4.0		E	596	430 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0	
C	TMV+P	MT20	2.0	4.0		BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E								
D	BMV1+P	MT20	3.0	4.0										
E	BMW1+W	MT20	2.0	4.0										

	LOADING TOTAL LOAD CASES: (4)	CSI: TC=0.44/1.00 (B-G:1), BC=0.29/1.00 (E-F:1),
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CHORDS		WEBS						DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO						
A-G	-18 / 0	-119.4	-119.4	0.06 (4)	6.25	E-B	-672 / 0	0.16 (1)	
G-B	0 / 18	-119.4	-119.4	0.44 (1)	10.00	F-G	-276 / 3	0.00 (1)	COMPANION LIVE LOAD FACTOR = 1.00

B-C	-44 / 0	-119, 0	-119, 4	0, 44 (1)	6, 25	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .
D-C	-165 / 0	0, 0	0, 0	0, 14 (1)	7, 81	
A-F	-32 / 0	-18, 3	-18, 3	0, 29 (1)	6, 25	
F-F	-0 / 15	-18, 2	-18, 2	0, 29 (1)	10, 00	NAIL VALUES

E-D	0/0	-18.2	-18.2	0.17 (1)	10.00	PLATE	GRIP(DRY)	SHEAR	SECTION
							(PSI)	(PLI)	(PLI)
						MAX	MIN	MAX	MIN
						650	371	1747	788
						MT20		1987	1873

	PLATE PLACEMENT TOL. = 0,250 inches
	PLATE ROTATION TOL. = 5,0 Deg.
	(PLATE = 0.11 (0.11 INCHES) 0.00)

		JSI CRIMP= 0.44 (B) (INPUT = 0.90) JSI METAL= 0.34 (B) (INPUT = 1.00)
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MODULUS ENGINEERING LTD. 		
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07/05/2023


D. A. SHERMAN
 100123373


 PROVINCIA DI ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NO. _____ (PRINT NAME) _____ (PRINT ADDRESS) _____ (PRINT CITY) _____ (PRINT STATE) _____ (PRINT ZIP) _____ (PRINT PHONE)	NO. _____ (PRINT NAME) _____ (PRINT ADDRESS) _____ (PRINT CITY) _____ (PRINT STATE) _____ (PRINT ZIP) _____ (PRINT PHONE)	NO. _____ (PRINT NAME) _____ (PRINT ADDRESS) _____ (PRINT CITY) _____ (PRINT STATE) _____ (PRINT ZIP) _____ (PRINT PHONE)	NO. _____ (PRINT NAME) _____ (PRINT ADDRESS) _____ (PRINT CITY) _____ (PRINT STATE) _____ (PRINT ZIP) _____ (PRINT PHONE)
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper interpretation of design of building designer, not Mitek responsibility. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com



REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPI Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com



Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 5 11:02:40 2023 Page 1
ID:oVfDr6Fdws2a9wd?_pY7R0zKcfG-oblflexuRlhr8Eek9IqBVl8UQJFih7p?2A_zTy_z?8xi

Scale = 1:21.0

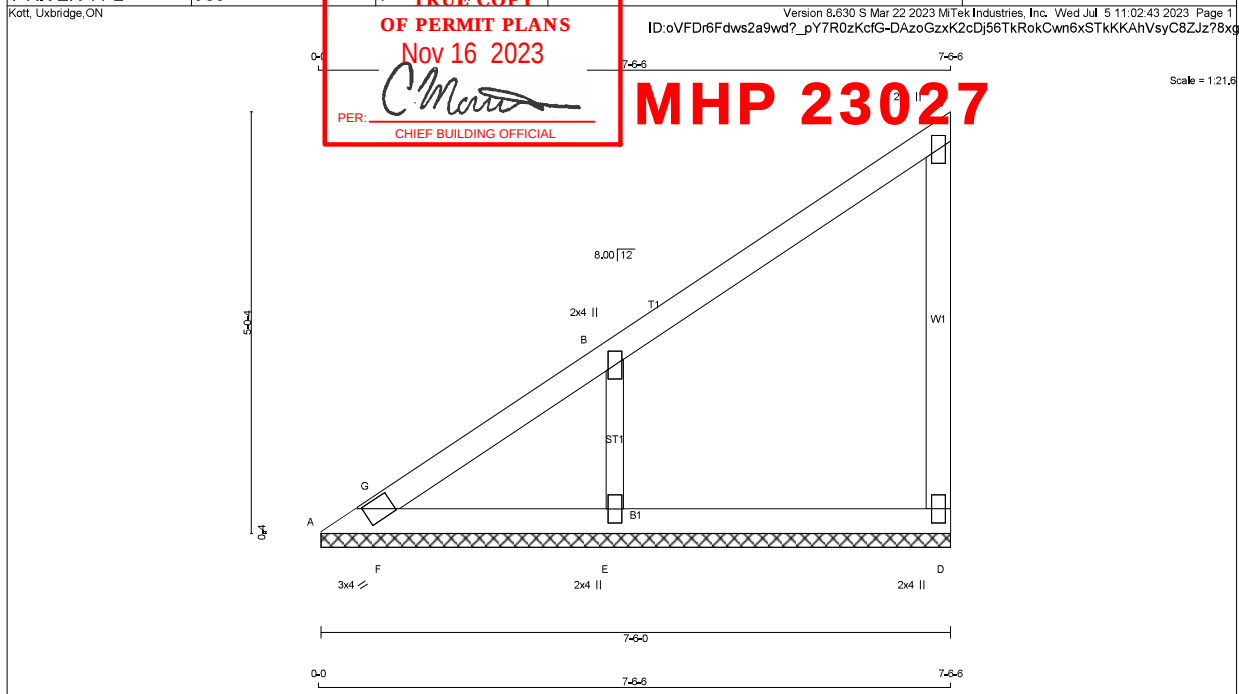


JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.bcsincanada.com





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER A - C 2x4 DRY No.2 D - C 2x4 DRY No.2 A - D 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY, SEASONED LUMBER.		DESCR. SPF SPF SPF	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1+h MT20 3,0 4,0 B TMV+w MT20 2,0 4,0 C TMV+p MT20 2,0 4,0 D BMV1+p MT20 2,0 4,0 E BMV1+w MT20 2,0 4,0		BEARINGS FACTORED GROSS REACTION JT VERT HORZ A 177 0 D 219 0 E 636 0 MAXIMUM FACTORED GROSS REACTION DOWN HORZ UPLIFT 177 0 0 219 0 0 636 0 0 INPUT BRG IN-SX 7'-6" 1-8 7'-6" 1-8 7'-6" 1-8 REQD BRG IN-SX 1-8 1-8 1-8	
UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL A 123 90 / 0 0 / 0 0 / 0 34 / 0 0 / 0 D 153 111 / 0 0 / 0 0 / 0 42 / 0 0 / 0 E 445 321 / 0 0 / 0 0 / 0 124 / 0 0 / 0		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34,8 PSF DL = 6,0 PSF BOT CH. LL = 0,0 PSF DL = 7,3 PSF TOTAL LOAD = 48,1 PSF SPACING = 240 IN. GIG THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48,1 P.S.F. G.S.L. PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0,28/1,00 (B-G:1) , BC=0,08/1,00 (E-F:1) , WB=0,08/1,00 (B-E:1) , SS=0,19/1,00 (B-C:1) DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,10 COMP=1,10 SHEAR=1,10 TENS=1,10 COMPANION LIVE LOAD FACTOR = 1,00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0,250 inches PLATE ROTATION TOL. = 5,0 Deg. JSI GRIP= 0,34 (B) (INPUT = 0,90) JSI METAL= 0,27 (B) (INPUT = 1,00)	
LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) MEMB. FR-TO A-G -44 / 0 G-B 0 / 25 B-C -27 / 0 D-C -194 / 0 FACTORED VERT. LOAD LC1 MAX. MAX. UNBRAC LENGTH FR-TO 6,25 FROM TO -119,4 -119,4 0,05 (1) -119,4 -119,4 0,28 (1) -119,4 -119,4 0,28 (1) 0,0 0,0 0,08 (1) 7,81 WEBS MAX. FACTORED FORCE (LBS) MEMB. FR-TO A-B -527 / 0 B-G -59 / 2 0,08 (1) 0,00 (1) 10,00 10,00 10,00 10,00		WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com	

