

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPECIFIED LOADS:			
E - B	2x4	DRY	No.2					TOP CH. LL	=	34.8	PSF
A - C	2x4	DRY	No.2					DL	=	6.0	PSF
E - D	2x4	DRY	No.2					BOT CH. LL	=	0.0	PSF
								DL	=	7.3	PSF
								TOTAL LOAD	=	48.1	PSF

PLATES (table is in inches)				BEARING REACTIONS				SPACING = 240 IN. G/C			
JT	TYPE	PLATES	W	LEN	Y	X		THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
B	TMV+p	MT20	2.0	4.0				THIS DESIGN COMPLIES WITH:			
E	BMV+p	MT20	2.0	4.0				- PART 9 OF BCBC 2018, NBC-2019AE			
								- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 086-14			
								- TPIC 2014			

UNFACTORED REACTIONS				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C				DESIGN ASSUMPTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
E	241	177 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0	(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD			
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	ALLOWABLE DEFL.(LL)= L/360 (0.19")			
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	CALCULATED VERT. DEFL.(LL) = L/999 (0.00")			

BRACING				ALLOWABLE DEFL.(TL)= L/360 (0.19")				CALCULATED VERT. DEFL.(TL) = L/999 (0.01")			
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				CSI TC=0.16/1.00 (A-B 1), BC=0.06/1.00 (D-E 4),				WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)			
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10				COMP=1.10 SHEAR=1.10 TENS=1.10			
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				COMPANION LIVE LOAD FACTOR = 1.00				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			

LOADING				NAIL VALUES				PLATE PLACEMENT TOL. = 0.250 inches			
TOTAL LOAD CASES: (4)				PLATE GRIP(DRY) SHEAR SECTION				PLATE ROTATION TOL. = 5.0 Deg.			
CHORDS				(PSI) (PLI) (PLI)				JSI GRIP= 0.17 (B) (INPUT = 0.90)			
MEMB. FORCE (LBS)				MAX MIN MAX MIN MAX MIN				JSI METAL= 0.13 (B) (INPUT = 1.00)			

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS				1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.		
F	2-0-12	1	1	1	BACK	VERT	TOTAL	---	C1		

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS				1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.			
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.		
F	2-0-12	1	1	1	BACK	VERT	TOTAL	---	C1		

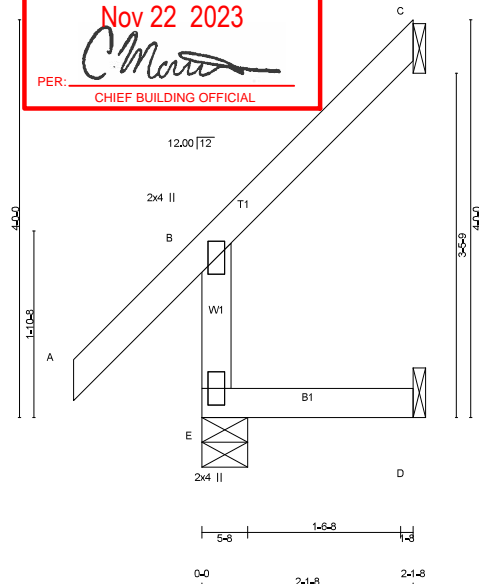
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL



MHP 23026



TOTAL WEIGHT = 2 X 10 = 20 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF
 E - B 2x4 DRY No.2 SPF
 A - C 2x4 DRY No.2 SPF
 E - D 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	345	0	345	0	5-8	1-8
C	96	0	96	0	1-8	1-8
D	18	0	20	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	238	189 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	66	56 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		W E B S	MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	MAX. CSI (LC)		MEMB. FORCE (LBS)	MAX. CSI (LC)
FR-TO							
E-B	-324 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 59	-119.4	-119.4	0.17 (1)	10.00		
B-C	-22 / 0	-119.4	-119.4	0.09 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 48.1 PSF

SPACING = 240 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, NBC-2019AE
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI TC=0.17/1.00 (A-B 1) BC=0.02/1.00 (D-E 4) ,
 WB=0.00/1.00 (n/a 0) , SSI=0.09/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PU)
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
 JSI METAL= 0.17 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

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 VOIDS THE ENGINEER'S SEAL

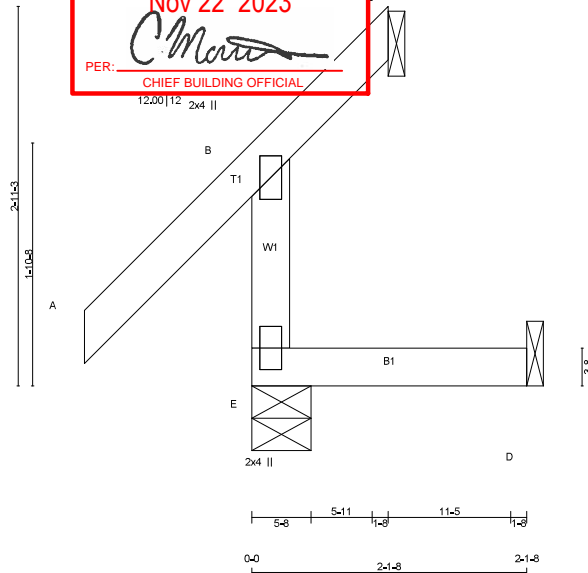
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
 TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com





MHP 23026

Scale = 1:14.0



TOTAL WEIGHT = 2 X 8 = 16 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	UPLIFT	BRG	UPLIFT
E	348	0	348	0	0	5-8	1-8	1-8	1-8
C	-26	0	0	0	-82	1-8	1-8	1-8	1-8
D	9	0	19	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
E	240	193 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	-18	0 / -52	0 / 0	0 / 0	0 / -1	0 / 0
D	8	0 / -6	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT, MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		MEMB.		W E B S	
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FROM	TO	UNBRACED LENGTH	FR-TO	TO
E-B	-319 / 0	0.0	0.0	0.03 (5)	7.81
A-B	0 / 59	-119.4	-119.4	0.17 (1)	10.00
B-C	-63 / 0	-119.4	-119.4	0.13 (1)	6.25
E-D	0 / 0	-18.2	-18.2	0.03 (5)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI TC=0.17/1.00 (A-B 1), BC=0.03/1.00 (D-E 5), WB=0.00/1.00 (n/a 0), SSI=0.09/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

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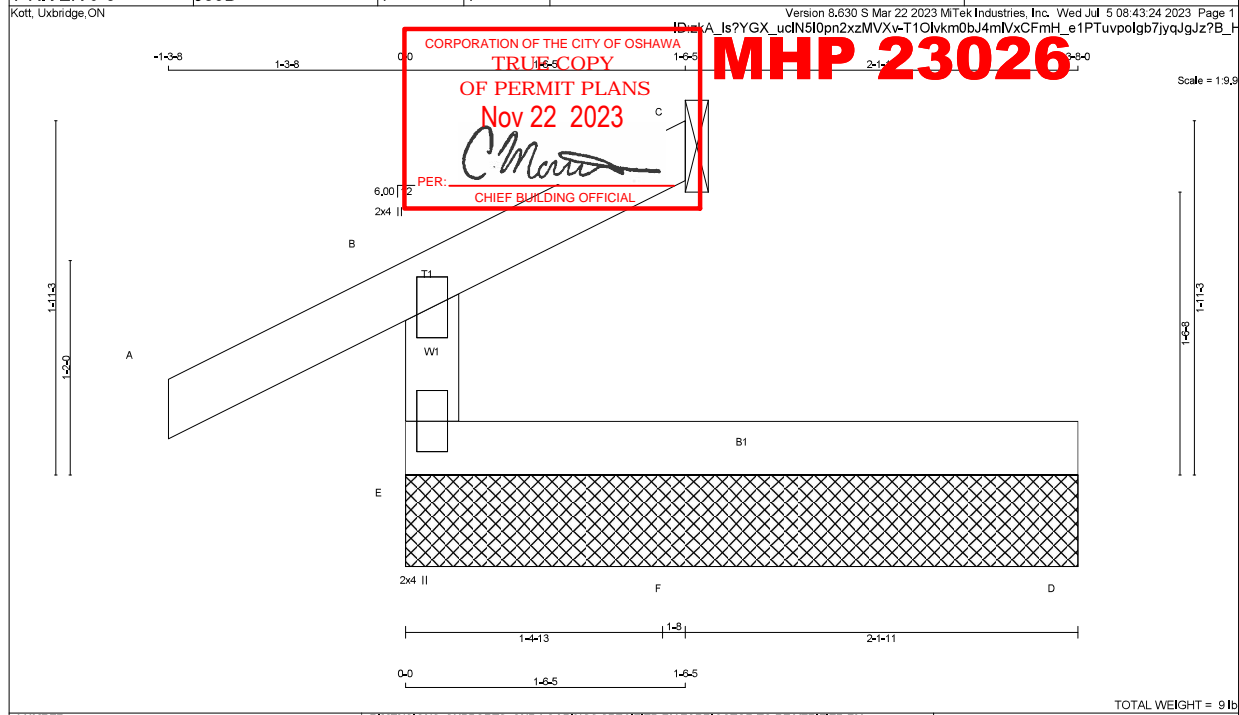


REVIEW FOR TRUSS COMPONENT ONLY

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	DRY	SPF	DESCR.	SPF	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
E - B	2x4	DRY	No.2	SPF				GROSS REACTION	GROSS REACTION	BRG	BRG
A - C	2x4	DRY	No.2	SPF				VERT	DOWN	HORZ	UPLIFT
E - D	2x4	DRY	No.2	SPF				E 359	0	359	0
DRY: SEASONED LUMBER.								C 19	0	82	0
								D 33	0	37	0

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C
 PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS				DESIGN CRITERIA			
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS		THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	249	190/0	0/0	0/0	0/0	59/0	0/0
C	13	53/-42	0/0	0/0	0/0	2/0	0/0
D	27	0/0	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, D
 BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				DESIGN CRITERIA			
TOTAL LOAD CASES: (13)				DESIGN ASSUMPTIONS			
CHORDS				-OVERHANG NOT TO BE ALTERED OR CUT OFF.			
MEMB.	FACTORED	VERT. LOAD	LC1 MAX	MAX. FACTORED			
FR-TO	FORCE (LBS)	FROM	TO	LENGTH FR-TO			
E-B	-325/0	0.0	0.0	0.03 (1)			
A-B	0/36	-119.4	-119.4	0.16 (1)			
B-C	-32/0	-119.4	-119.4	0.15 (1)			
E-F	0/0	-18.2	-18.2	0.07 (4)			
F-D	0/0	-18.2	-18.2	0.07 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN
 PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

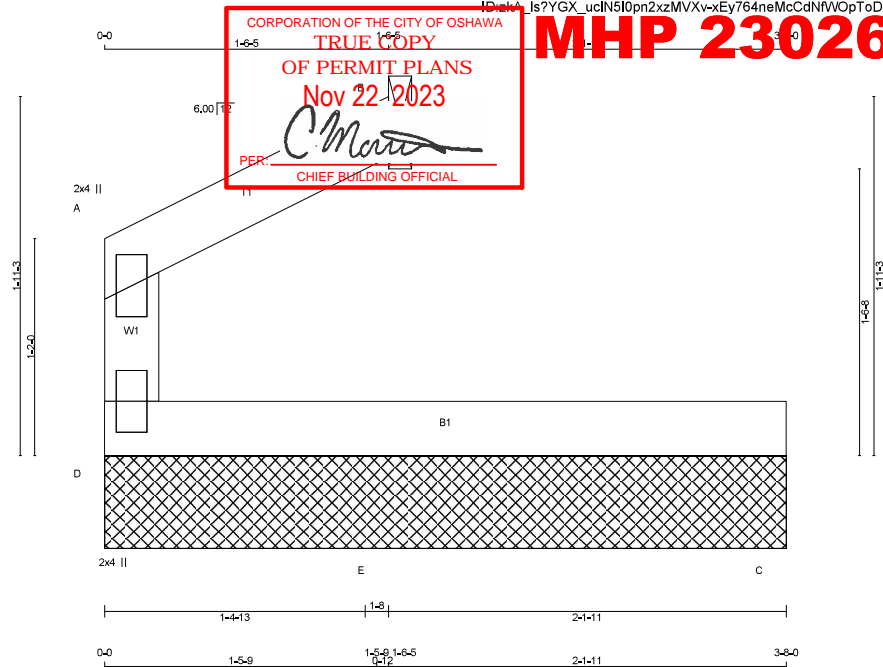
NAIL VALUES				DESIGN CRITERIA			
PLATE	GRIP (DRY)	SHEAR	SECTION	ALLOWABLE DEFL.(TL)=	L/960 (0.19")		
(PSI)	(PLI)	(PLI)	(PLI)	CALCULATED VERT. DEFL.(TL) =	L/999 (0.01")		
MT20	650	371	1747	788	1987	1873	

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP = 0.18 (B) (INPUT = 0.90)
 JSI METAL = 0.14 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPECIFIED LOADS:	TOP CH.	LL	PSF
D - A	2x4	DRY	No.2	SPF	SPF	SPF	SPF	TOP CH. LL	34.8	PSF	
A - B	2x4	DRY	No.2	SPF	SPF	SPF	SPF	DL	6.0	PSF	
D - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF	BOT CH. LL	0.0	PSF	
DRY: SEASONED LUMBER.								DL	7.3	PSF	
								TOTAL LOAD	48.1	PSF	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
A	TMV+p	MT20	2.0	4.0	
D	BMV1+p	MT20	2.0	4.0	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	87	55 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	66	50 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
C	25	1 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (7)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
D-A	-87 / 0	0.0	0.03 (4)	7.81			
A-B	0 / 4	-119.4	-119.4	0.05 (1)	10.00		
D-E	0 / 0	-18.2	-18.2	0.06 (4)	10.00		
E-C	0 / 0	-18.2	-18.2	0.06 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.05/1.00 (A-B:1), BC=0.06/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SS=0.07/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

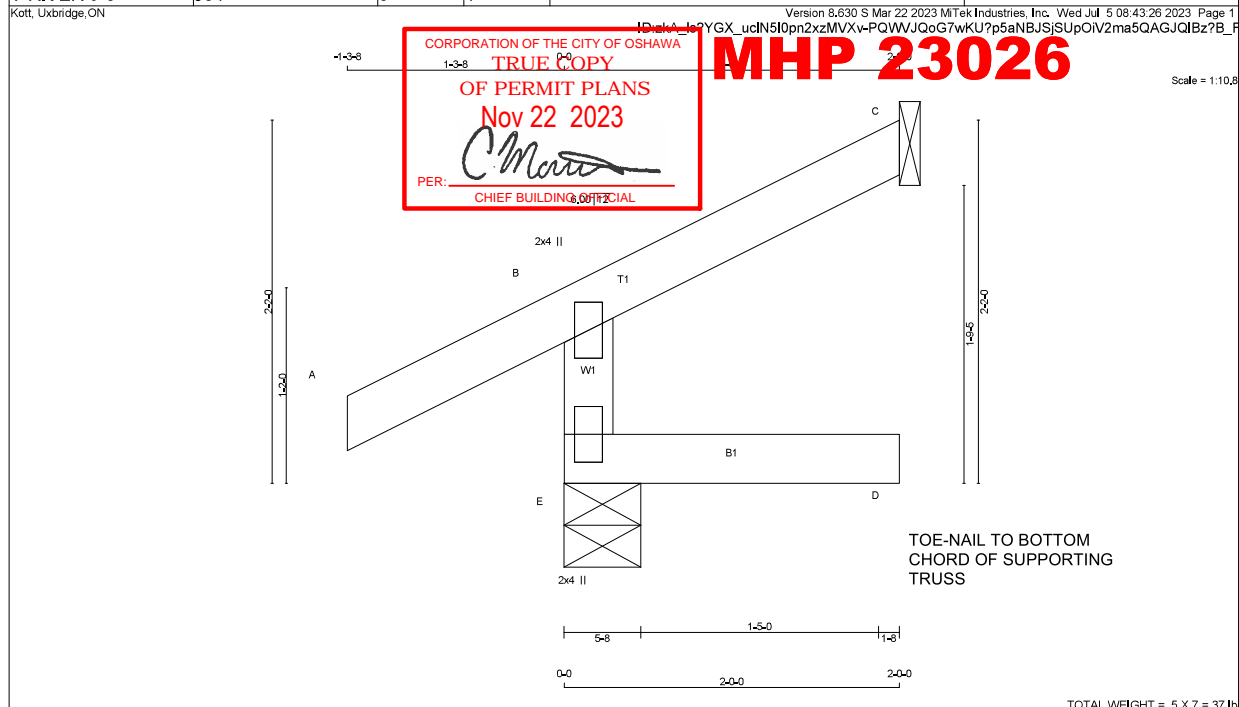
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG IN-SX</th><th>REQRD BRG IN-SX</th></tr><tr><td>JT</td><td>VERT DOWN</td><td>HORIZ</td><td>UPLIFT</td><td></td></tr><tr><td>E</td><td>347</td><td>0</td><td>347</td><td>0</td></tr><tr><td>C</td><td>90</td><td>0</td><td>90</td><td>0</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C UNFACTORED REACTIONS <table><tr><th>1ST LCASE</th><th>MAX./MIN.</th><th>COMPONENT REACTIONS</th></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW LIVE PERM.LIVE WIND DEAD SOIL</td></tr><tr><td>E</td><td>242</td><td>181 / 0 0 / 0 0 / 0 0 / 0 60 / 0 0 / 0</td></tr><tr><td>C</td><td>62</td><td>53 / 0 0 / 0 0 / 0 0 / 0 9 / 0 0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (5) <table><tr><th>CHORDS</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. FACTORED HORIZ. LOAD (LC)</th><th>MEMB. LENGTH FR-TO</th></tr><tr><td>E-B</td><td>-311 / 0</td><td>0.0</td><td>0.0</td><td>0.03 (1)</td></tr><tr><td>A-B</td><td>0 / 36</td><td>-119.4</td><td>-119.4</td><td>0.16 (1)</td></tr><tr><td>B-C</td><td>-13 / 0</td><td>-119.4</td><td>-119.4</td><td>0.08 (1)</td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.2</td><td>-18.2</td><td>0.09 (4)</td></tr></table> CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN PATTERN LOADING CHECK APPLIED TO THIS TRUSS.						FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX	JT	VERT DOWN	HORIZ	UPLIFT		E	347	0	347	0	C	90	0	90	0	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL	E	242	181 / 0 0 / 0 0 / 0 0 / 0 60 / 0 0 / 0	C	62	53 / 0 0 / 0 0 / 0 0 / 0 9 / 0 0 / 0	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC)	MEMB. LENGTH FR-TO	E-B	-311 / 0	0.0	0.0	0.03 (1)	A-B	0 / 36	-119.4	-119.4	0.16 (1)	B-C	-13 / 0	-119.4	-119.4	0.08 (1)	E-D	0 / 0	-18.2	-18.2	0.09 (4)	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(TL)= L/960 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.01") CSI TC=0.16/1.00 (A-B-1), BC=0.09/1.00 (D-E-4), WB=0.00/1.00 (n/a/0), SSI=0.11/1.00 (A-B-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)				
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MODULUS ENGINEERING LTD.

07/05/2023

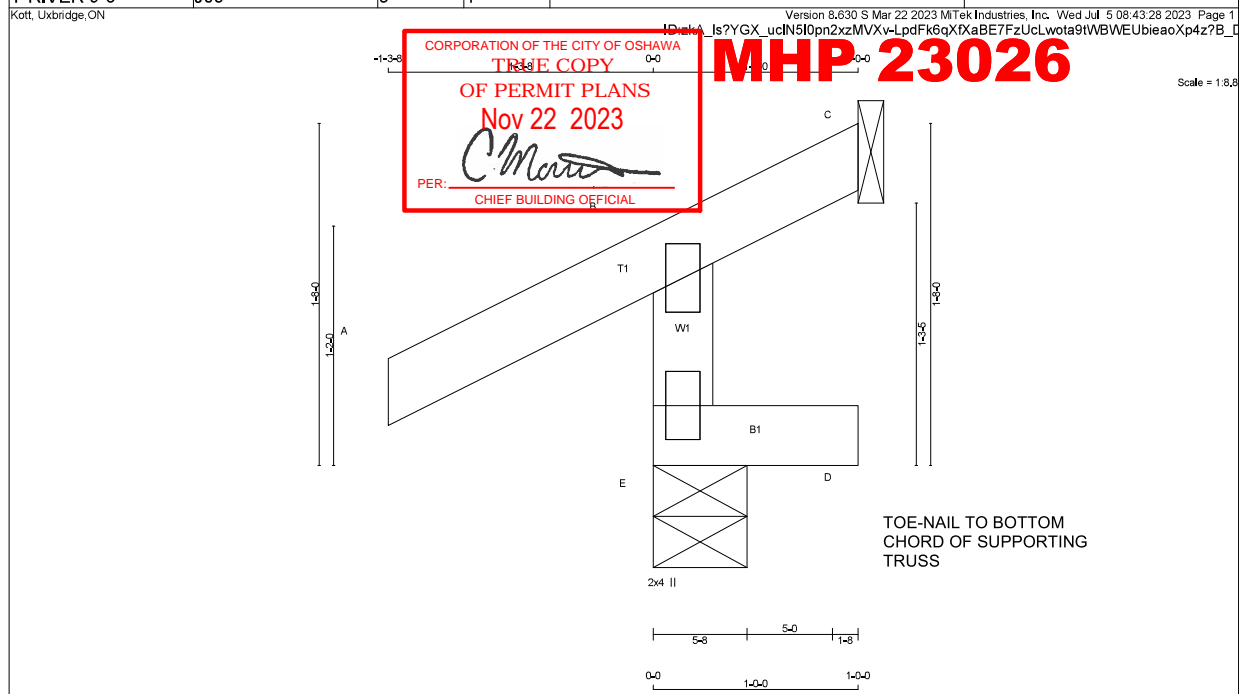
D. A. SHERMAN

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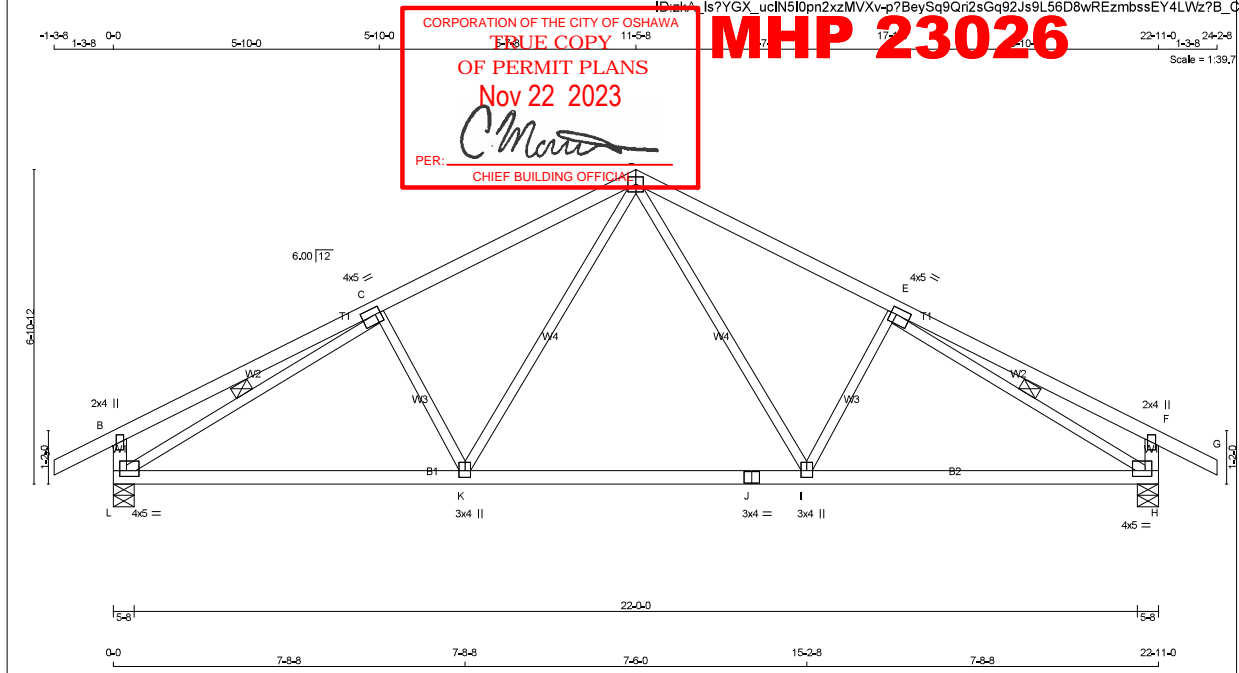
PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL



<div>LUMBER</div> <div>N. L. G. A. RULES</div> <table><tr><td>CHORDS</td><td>SIZE</td><td>LUMBER</td><td>DESCR.</td></tr><tr><td>E - B</td><td>2x4</td><td>DRY</td><td>No.2</td></tr><tr><td>A - C</td><td>2x4</td><td>DRY</td><td>No.2</td></tr><tr><td>E - D</td><td>2x4</td><td>DRY</td><td>No.2</td></tr></table> <div>SPF</div> <div>SPF</div> <div>SPF</div> <div>DRY: SEASONED LUMBER.</div>				CHORDS	SIZE	LUMBER	DESCR.	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COMPONENT REACTIONS						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	221	178 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0	C	-14	0 / -44	0 / 0	0 / 0	0 / 0	0 / -2	0 / 0	CHORDS		FACTORED		WEBS		MAX. FACTORED		MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE		(LBS)	(PLF)	CS1 (LC)	UNBRAC			(LBS)	FR-TO		FROM	TO	LENGTH	FR-TO			E-B	-301 / 0	0.0	0.0	0.05 (5)	7.81			A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00			B-C	-36 / 0	-119.4	-119.4	0.11 (1)	6.25			E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			<div>DESIGN CRITERIA</div> <div>SPECIFIED LOADS:</div> <table><tr><td>TOP CH.</td><td>LL</td><td>=</td><td>34.8</td><td>PSF</td></tr><tr><td></td><td>DL</td><td>=</td><td>6.0</td><td>PSF</td></tr><tr><td>BOT CH.</td><td>LL</td><td>=</td><td>0.0</td><td>PSF</td></tr><tr><td></td><td>DL</td><td>=</td><td>7.3</td><td>PSF</td></tr><tr><td>TOTAL LOAD</td><td>=</td><td>48.1</td><td>PSF</td><td></td></tr></table> <div>SPACING = 24.0 IN. 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TOTAL WEIGHT = 7 X 89 = 626 lb

LUMBER				N, L, G, A, RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2	D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2	L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2	H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2	L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2	J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2	ALL WEBS	2x3	DRY	No.2
EXCEPT				EXCEPT			
DRY: SEASONED LUMBER,							

PLATES (table is in inches)				JT TYPE			
PLATES	SIZE	W	LEN	Y	X	PLATES	SIZE
B	TMV+p	MT20	2.0	4.0		B	TMV+p
C	TMWW+4	MT20	4.0	5.0	1.75	C	TMWW+4
D	TMWW+p	MT20	4.0	4.0		D	TMWW+p
E	TMWW+4	MT20	4.0	5.0	1.75	E	TMWW+4
F	TMV+p	MT20	2.0	4.0		F	TMV+p
H	BMVW+1	MT20	4.0	5.0	1.50	H	BMVW+1
I	BMVW+1	MT20	3.0	4.0	1.75	I	BMVW+1
J	BS4	MT20	3.0	4.0		J	BS4
K	BMVW+1	MT20	3.0	4.0	1.75	K	BMVW+1
L	BMVW+1	MT20	4.0	5.0	1.75	L	BMVW+1

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQRD				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX		
L	1740	0	1740	0	0	5-8	1-14		
H	1740	0	1740	0	0	5-8	1-14		

UNFACTORED REACTIONS									
JT	1ST CASE	MAX./MIN.	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	1213	892 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0	0 / 0	
H	1213	892 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C₄, E₄.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX	LC2 MAX	
FR-TO		FROM	TO	LENGTH	FR-TO		FROM	TO	LENGTH
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	D-I	0 / 679	0.15 (1)	
B-C	0 / 36	-119.4	-119.4	0.63 (1)	10.00	I-E	-497 / 0	0.15 (1)	
C-D	-1939 / 0	-119.4	-119.4	0.56 (1)	4.20	K-D	0 / 679	0.15 (1)	
D-E	-1939 / 0	-119.4	-119.4	0.56 (1)	4.20	C-K	-497 / 0	0.15 (1)	
E-F	0 / 36	-119.4	-119.4	0.63 (1)	10.00	L-C	-2308 / 0	0.72 (1)	
F-G	0 / 36	-119.4	-119.4	0.16 (1)	10.00	E-H	-2308 / 0	0.72 (1)	
L-B	-424 / 0	0.0	0.0	0.04 (1)	7.81				
H-F	-424 / 0	0.0	0.0	0.04 (1)	7.81				
L-K	0 / 1936	-18.2	-18.2	0.44 (1)	10.00				
K-J	0 / 1367	-18.2	-18.2	0.34 (1)	10.00				
J-I	0 / 1367	-18.2	-18.2	0.34 (1)	10.00				
I-H	0 / 1936	-18.2	-18.2	0.44 (1)	10.00				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	34.8 PSF
DL	=	6.0	PSF
BOT CH.	LL	=	0.0 PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. GIG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")

CALCULATED VERT. DEFL.(LL) = L/999 (0.09")

ALLOWABLE DEFL.(TL) = L/360 (0.76")

CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.63/1.00 (E-F:1), BC=0.44/1.00 (H-I:1), WB=0.72/1.00 (E-H:1), SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PL)	(PL)	(PL)		
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)

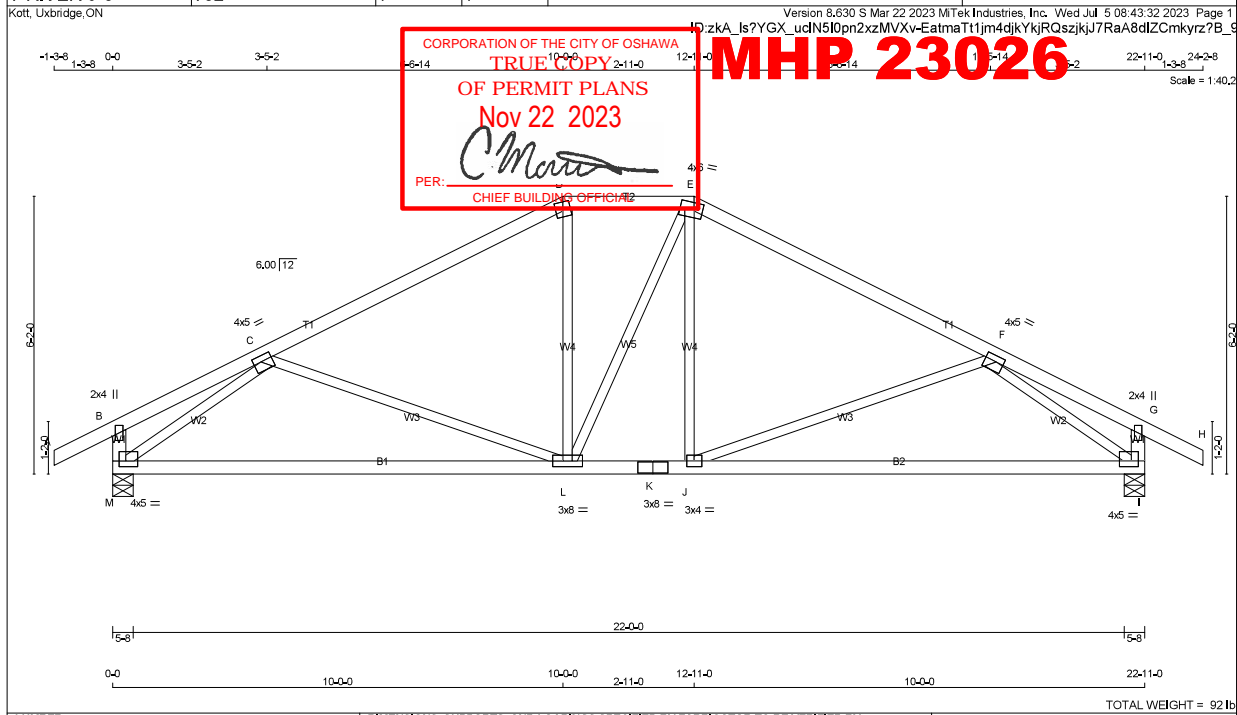
JSI METAL= 0.62 (C) (INPUT = 1.00)

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REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	GROSS REACTION	DOWN	UP	BRG	TOP CH. LL = 34.8 PSF	
D - E	2x4	DRY	No.2	SPF	M	VERT	0	0	5-8	DL = 6.0 PSF	
E - H	2x4	DRY	No.2	SPF	I	HORZ	1740	0	5-8	BOT CH. LL = 0.0 PSF	
M - B	2x4	DRY	No.2	SPF						DL = 7.3 PSF	
I - G	2x4	DRY	No.2	SPF						TOTAL LOAD = 48.1 PSF	
M - K	2x4	DRY	No.2	SPF							
K - I	2x4	DRY	No.2	SPF							
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF							
DRY, SEASONED LUMBER.											

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1213	892 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
I	1213	892 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS	MEMB.	MAX. FACTORED	VERT. LOAD	FROM	TO	WEBS	MEMB.
A-B	0 / 36	-119.4	-119.4	0.16	(1)	10.00	C-L
B-C	0 / 62	-119.4	-119.4	0.64	(1)	10.00	L-D
C-D	-1752 / 0	-119.4	-119.4	0.70	(1)	4.10	L-E
D-E	-1543 / 0	-119.4	-119.4	0.16	(1)	5.16	J-F
E-F	-1752 / 0	-119.4	-119.4	0.70	(1)	4.10	J-F
F-G	0 / 62	-119.4	-119.4	0.64	(1)	10.00	M-C
G-H	0 / 36	-119.4	-119.4	0.16	(1)	10.00	F-I
H-I	-223 / 0	0.0	0.0	0.02	(1)	7.81	
I-G	-223 / 0	0.0	0.0	0.02	(1)	7.81	
M-L	0 / 1903	-18.2	-18.2	0.52	(1)	10.00	
L-K	0 / 1543	-18.2	-18.2	0.50	(4)	10.00	
K-J	0 / 1543	-18.2	-18.2	0.50	(4)	10.00	
J-I	0 / 1903	-18.2	-18.2	0.53	(1)	10.00	

ALLOWABLE DEFL.(LL)= L/360 (0.76")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL)= L/360 (0.76")

CALCULATED VERT. DEFL.(TL) = L/813 (0.34")

CSI: TC=0.70/1.00 (C-D-1), BC=0.53/1.00 (I-J-1), WB=0.70/1.00 (C-M-1), SS=0.33/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	MAX MIN	MAX MIN	MAX MIN
	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)

JSI METAL= 0.89 (K) (INPUT = 1.00)

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07/05/2023

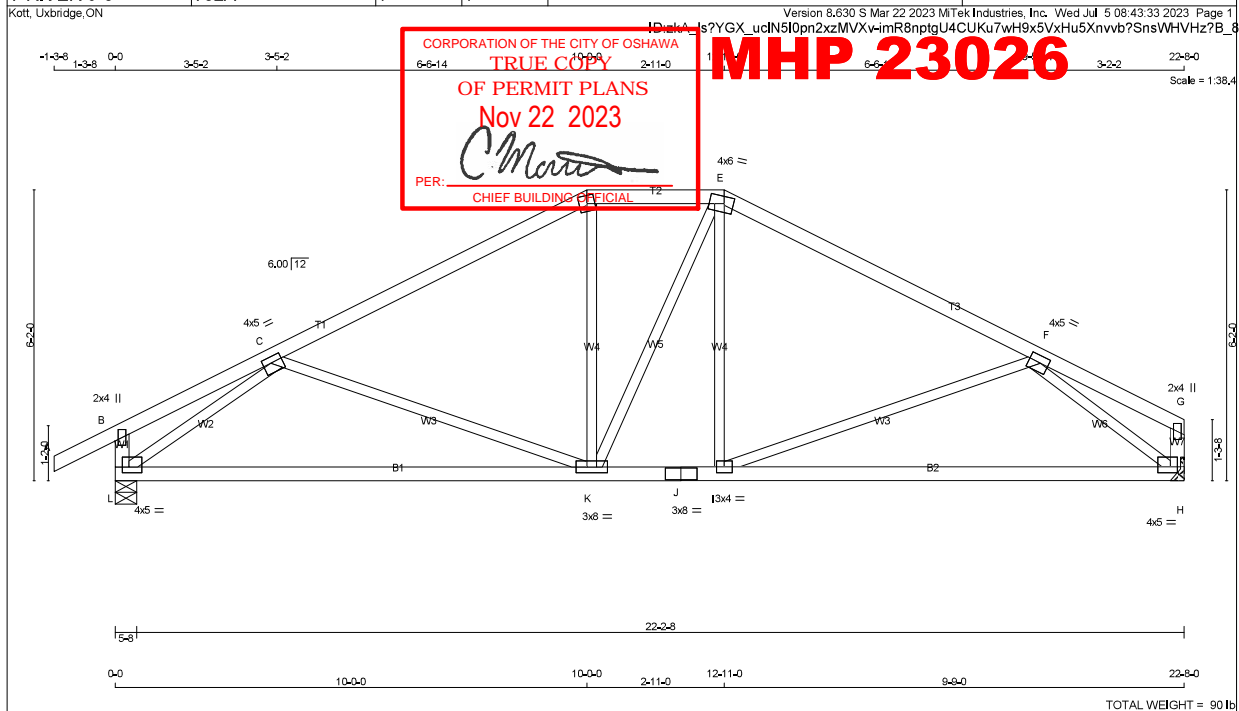
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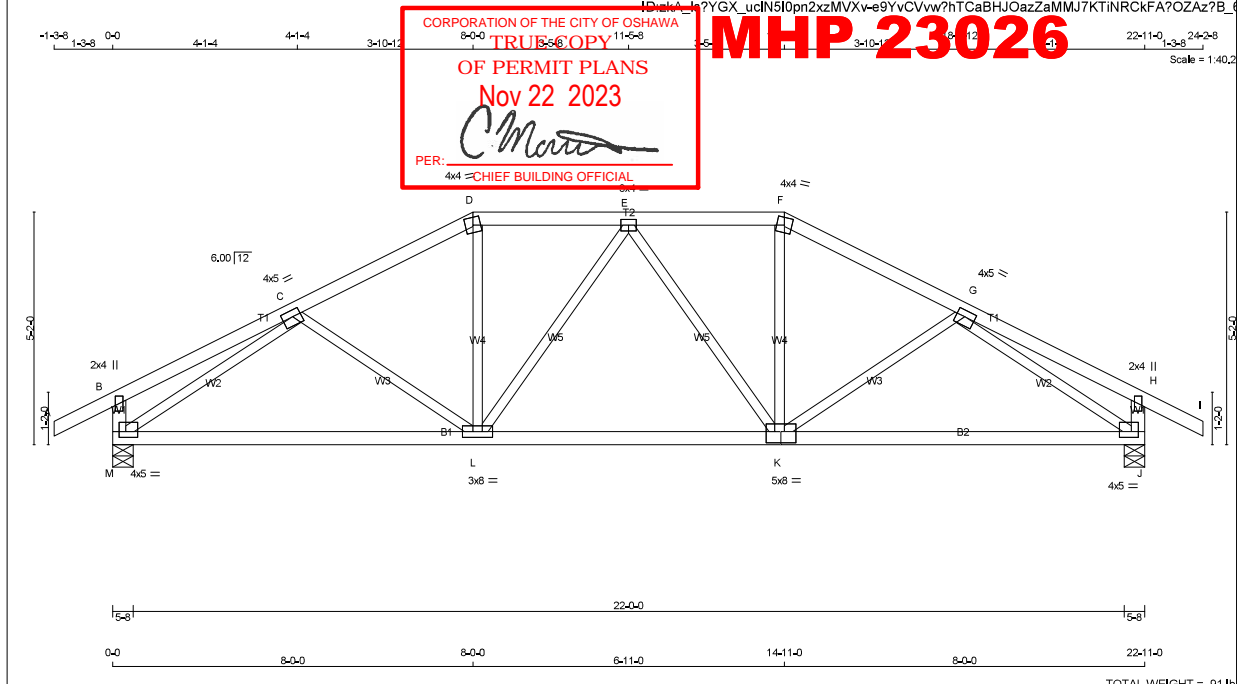
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REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						DESIGN CRITERIA		[M/F]		
				BEARINGS										
				FACTORED		MAXIMUM FACTORED		INPUT REQD		SPECIFIED LOADS:				
				GROSS REACTION		GROSS REACTION		BRG BRG		TOP CH. LL = 34.8 PSF				
				JT VERT HORZ	DOWN HORZ UPLIFT		IN-SX IN-SX		DL = 6.0 PSF					
				L 1722 0	1722 0 5-8		1-14		BOT CH. LL = 0.0 PSF					
				H 1560 0	1560 0 0 MECHANICAL				DL = 7.3 PSF					
													TOTAL LOAD = 48.1 PSF	
													SPACING = 24.0 IN. C/C	
													LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM	
ALL WEBS 2x3 DRY No.2 SPF				A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, MINIMUM BEARING LENGTH AT JOINT H = 1-11.										
EXCEPT				UNFACTORED REACTIONS										
DRY: SEASONED LUMBER,														



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 34.8 PSF	
D - F	2x4	DRY	No.2	SPF	M	VERT	DOWN	HORZ	UPLIFT	DL = 6.0 PSF	
F - I	2x4	DRY	No.2	SPF	J	1740	0	1740	0	BOT CH. LL = 0.0 PSF	
M - B	2x4	DRY	No.2	SPF	J	1740	0	1740	0	DL = 7.3 PSF	
J - H	2x4	DRY	No.2	SPF						TOTAL LOAD = 48.1 PSF	
M - K	2x4	DRY	No.2	SPF							
K - J	2x4	DRY	No.2	SPF							
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF							
DRY, SEASONED LUMBER.											

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING			
JT	TYPE	PLATES	W LEN Y X	1ST CASE	MAX./MIN.	COMPONENT REACTIONS		1ST CASE	MAX./MIN.	COMPONENT REACTIONS	
B	TMV+p	MT20	2.0 4.0	JT	COMBINED	SNOW	LIVE	JT	COMBINED	SNOW	LIVE
C	TMVW4	MT20	4.0 5.0 1.75 2.25	M	1213	892 / 0	0 / 0	M	1213	892 / 0	0 / 0
D	TTW-m	MT20	4.0 4.0	J	1213	892 / 0	0 / 0	J	1213	892 / 0	0 / 0
E	TMVW4	MT20	3.0 4.0								
F	TTW-m	MT20	4.0 4.0								
G	TMVW4	MT20	4.0 5.0 1.75 2.25								
H	TMV+p	MT20	2.0 4.0								
J	BMVW4	MT20	4.0 5.0 1.50 1.75								
K	BSVW4	MT20	5.0 8.0 3.00 4.00								
L	BMVW4	MT20	3.0 8.0								
M	BMVW4	MT20	4.0 5.0 1.50 1.75								

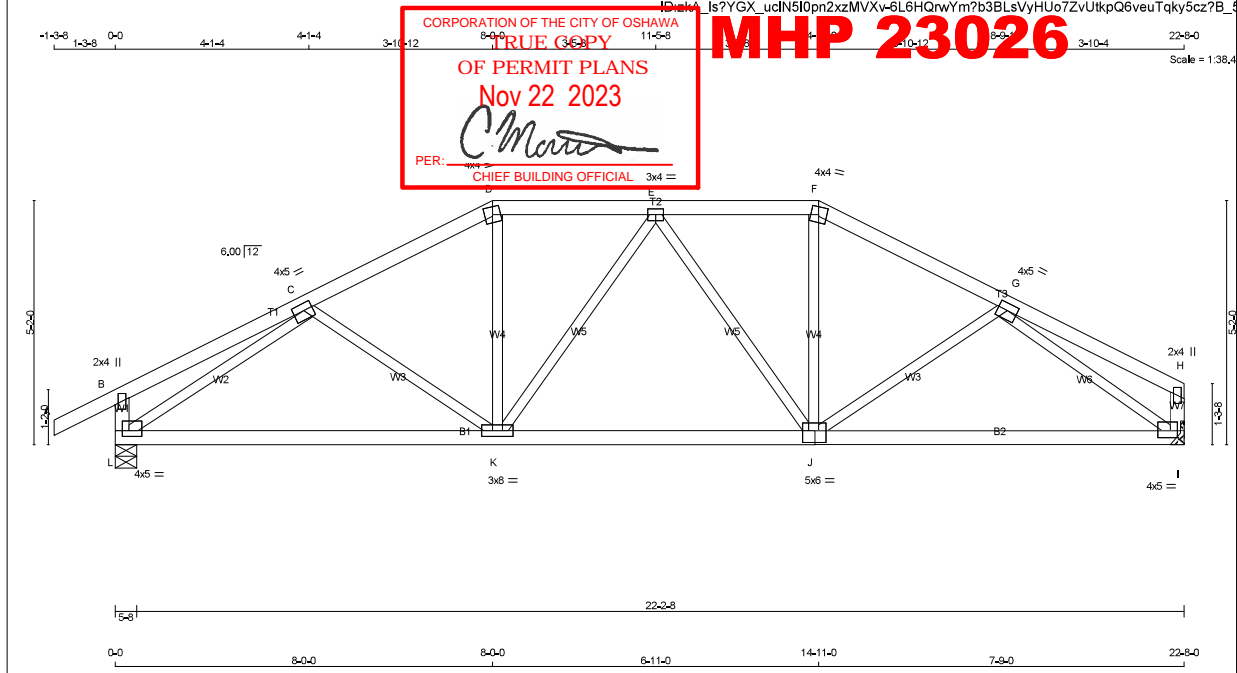
CHORDS				WEBS			
MEMB.	FORCE	VERT. LOAD	MAX. FACTORED	MEMB.	FORCE	MAX. FACTORED	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
A-B	0 / 36	-119.4	-119.4 0.16 (1)	C-L	-189 / 19	0.08 (1)	
B-C	0 / 23	-119.4	-119.4 0.28 (1)	L-D	0 / 506	0.11 (1)	
C-D	-1933 / 0	-119.4	-119.4 0.28 (1)	L-E	-281 / 0	0.17 (1)	
D-E	-1718 / 0	-119.4	-119.4 0.20 (1)	E-K	-323 / 0	0.20 (1)	
E-F	-1693 / 0	-119.4	-119.4 0.20 (1)	K-F	0 / 495	0.11 (1)	
F-G	-1909 / 0	-119.4	-119.4 0.26 (1)	K-G	-193 / 14	0.08 (1)	
G-H	0 / 23	-119.4	-119.4 0.28 (1)	M-C	-2288 / 0	0.90 (1)	
H-I	0 / 36	-119.4	-119.4 0.16 (1)	G-J	-2264 / 0	0.89 (1)	
M-B	-351 / 0	0.0	0.0 0.04 (1)				
J-H	-351 / 0	0.0	0.0 0.04 (1)				
M-L	0 / 1862	-18.2	-18.2 0.45 (1)				
L-K	0 / 1877	-18.2	-18.2 0.46 (1)				
K-J	0 / 1843	-18.2	-18.2 0.50 (1)				

NAIL VALUES				PLATE PLACEMENT TOL. = 0.250 inches			
PLATE	GRIP(DRY)	SHEAR	SECTION	PLATE ROTATION TOL. = 5.0 Deg.			
	(PSI)	(PLI)	(PLI)				
	MAX	MIN	MAX				
MT20	650	371	1747				

NAIL VALUES				PLATE PLACEMENT TOL. = 0.250 inches			
PLATE	GRIP(DRY)	SHEAR	SECTION	PLATE ROTATION TOL. = 5.0 Deg.			
	(PSI)	(PLI)	(PLI)				
	MAX	MIN	MAX				
MT20	650	371	1747				

NAIL VALUES				PLATE PLACEMENT TOL. = 0.250 inches			
PLATE	GRIP(DRY)	SHEAR	SECTION	PLATE ROTATION TOL. = 5.0 Deg.			
	(PSI)	(PLI)	(PLI)				
	MAX	MIN	MAX				
MT20	650	371	1747				

NAIL VALUES				PLATE PLACEMENT TOL. = 0.250 inches			
PLATE	GRIP(DRY)	SHEAR	SECTION	PLATE ROTATION TOL. = 5.0 Deg.			
	(PSI)	(PLI)	(PLI)				
	MAX	MIN	MAX				
MT20	650	371	1747				





OR SMALL BUILDING REQUIREMENTS OF PART

D	TMVW-4	MT20	3.0	5.0	1.50	2.25
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ALL DITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

TOTAL LOAD CASES: (4)		CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")
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NAIL VALUES	
NAIL NO.	DEPTH (mm)
1	10
2	10
3	10
4	10
5	10
6	10
7	10
8	10
9	10
10	10
11	10
12	10
13	10
14	10
15	10
16	10
17	10
18	10
19	10
20	10
21	10
22	10
23	10
24	10
25	10
26	10
27	10
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29	10
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31	10
32	10
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82	10
83	10
84	10
85	10
86	10
87	10
88	10
89	10
90	10
91	10
92	10
93	10
94	10
95	10
96	10
97	10
98	10
99	10
100	10

PLATE PLACEMENT TOL. = 0.250 inches

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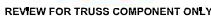
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NOTE: ALTERING THIS DOCUMENT

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

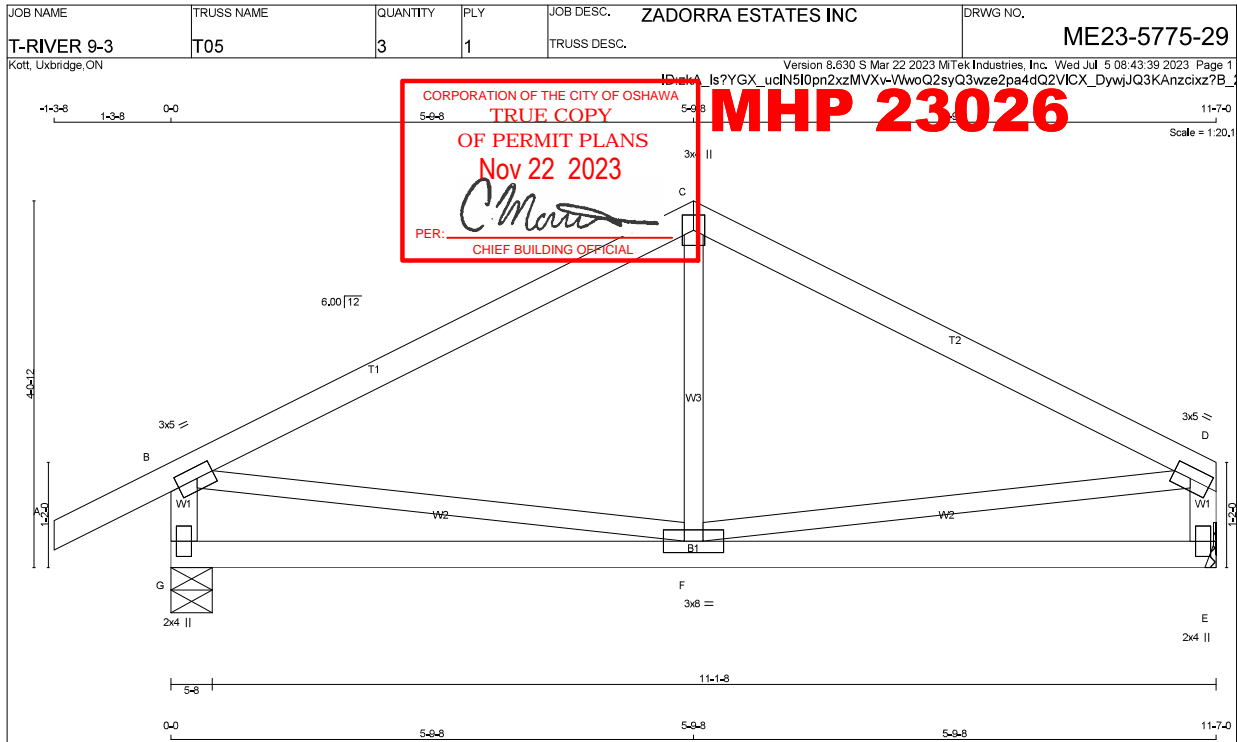


NOTE: ALTERING THIS DOCUMENT
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					DESIGN CRITERIA				
A - C 2x4 DRY 2100F 1.8E SPF					BEARINGS					SPECIFIED LOADS:				
C - D 2x4 DRY 2100F 1.8E SPF					FACTORED MAXIMUM FACTORED INPUT REQD					TOP CH. LL = 34.8 PSF				
G - B 2x4 DRY No.2 SPF					GROSS REACTION GROSS REACTION BRG BRG					DL = 8.0 PSF				
E - D 2x4 DRY No.2 SPF					JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX					BOT CH. LL = 0.0 PSF				
G - E 2x4 DRY No.2 SPF					G 959 0 959 0 0 5-8 1-8					DL = 7.3 PSF				
ALL WEBS 2x3 DRY No.2 SPF					E 797 0 797 0 0 5-8 MECHANICAL					TOTAL LOAD = 48.1 PSF				
EXCEPT					A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, MINIMUM BEARING LENGTH AT JOINT E = 1-8.					SPACING = 24.0 IN.CIC				
DRY: SEASONED LUMBER.					UNFACTORED REACTIONS					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015				
PLATES (table is in inches)					1ST LCASE MAX./MIN. COMPONENT REACTIONS					THIS DESIGN COMPLIES WITH:				
JT TYPE PLATES W LEN Y X					JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL					- PART 9 OF BCBC 2018, NBC-2019AE				
B TMVW4 MT20 3.0 5.0 1.50 2.25					G 668 498 / 0 0 / 0 0 / 0 170 / 0 0 / 0					- PART 9 OF OBC 2012 (2019 AMENDMENT)				
C TTW+p MT20 3.0 4.0					E 557 403 / 0 0 / 0 0 / 0 154 / 0 0 / 0					- CSA 086-14				
D TMVW4 MT20 3.0 5.0 1.50 2.25					BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G					- TPIC 2014				
E BMV1+p MT20 2.0 4.0					BRACING					(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD				
F BMVW4 MT20 3.0 8.0					TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.					ALLOWABLE DEFL.(LL)= L/360 (0.39")				
G BMV1+p MT20 2.0 4.0					MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.					CALCULATED VERT. DEFL.(LL) = L/999 (0.01")				
					ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.					ALLOWABLE DEFL.(TL)= L/360 (0.39")				
					LOADING					CALCULATED VERT. DEFL.(TL) = L/999 (0.03")				
					TOTAL LOAD CASES: (4)					CSI TC=0.34/1.00 (C-D-1), BC=0.17/1.00 (E-F-4), VWB=0.14/1.00 (B-F-1), SSI=0.24/1.00 (C-D-1)				
					CHORDS					DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10				
					MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) MAX. UNBRACED LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. (LC)					COMPANION LIVE LOAD FACTOR = 1.00				
					FR-TO FROM TO CSI (LC) UNBRACED LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. (LC)					TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				
					A-B 0 / 36 -119.4 -119.4 0.10 (1) 10.00 F-C -59 / 91 0.03 (4)					NAIL VALUES				
					B-C -708 / 0 -119.4 -119.4 0.34 (1) 6.25 B-F 0 / 640 0.14 (1)					PLATE GRIP (DRY) SHEAR SECTION				
					C-D -708 / 0 -119.4 -119.4 0.34 (1) 6.25 F-D 0 / 640 0.14 (1)					(PSI) (PLI) (PLI)				
					G-B -918 / 0 0.0 0.0 0.09 (1) 7.81					MAX MIN MAX MIN MAX MIN				
					E-D -756 / 0 0.0 0.0 0.08 (1) 7.81					MT20 650 371 1747 788 1987 1873				
					G-F 0 / 0 -18.2 -18.2 0.17 (4) 10.00					PLATE PLACEMENT TOL. = 0.250 inches				
					F-E 0 / 0 -18.2 -18.2 0.17 (4) 10.00					PLATE ROTATION TOL. = 5.0 Deg.				
										JSI GRIP= 0.81 (D) (INPUT = 0.90)				
										JSI METAL= 0.27 (D) (INPUT = 1.00)				
MODULUS ENGINEERING LTD.														
REVIEW FOR TRUSS COMPONENT ONLY														
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL														

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com

