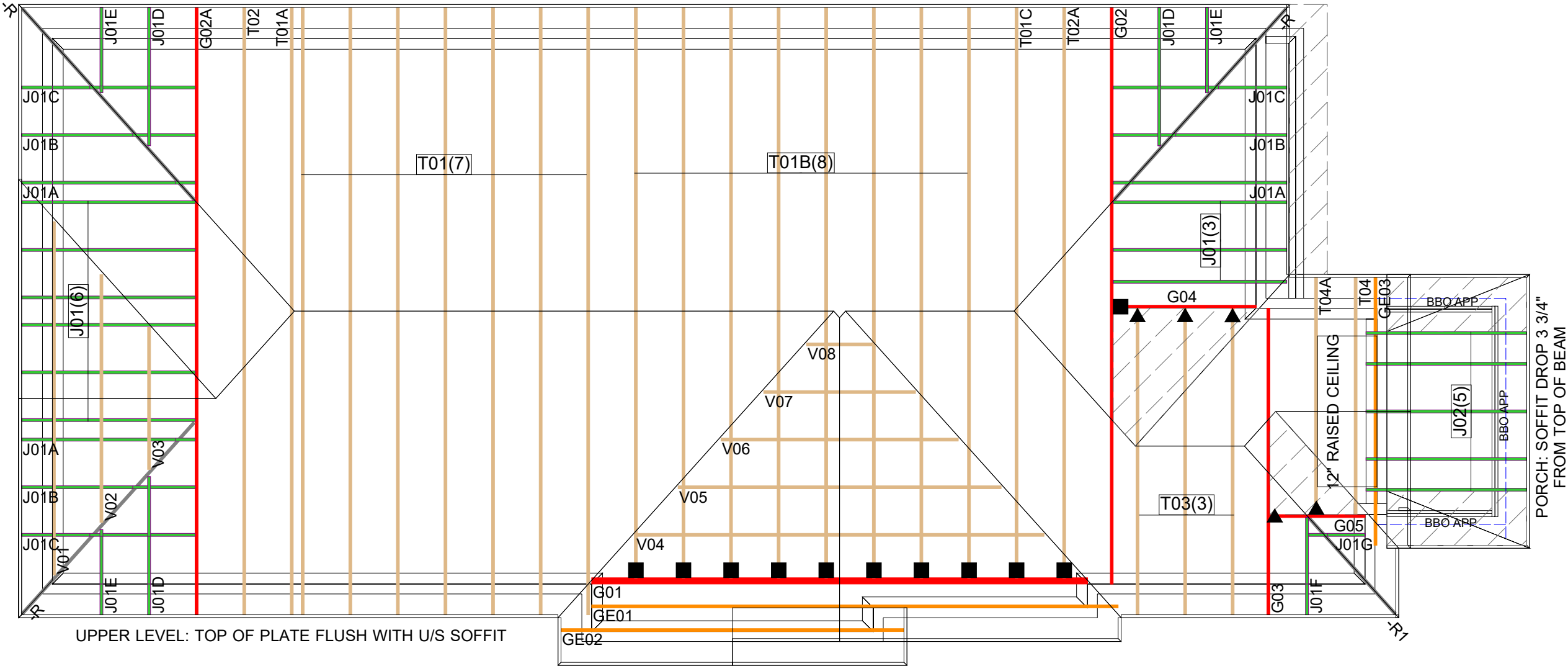


CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 22 2023
PER: 
CHIEF BUILDING OFFICIAL

MHP 23026



Hanger Name	Symbol	QTY
LUS24	▲	5
LJS26DS	■	11



CONVENTIONAL
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK GROUP
Job #	23-00084R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC OSHAWA, ON
Model	RIVER 9-1
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-04-27
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 9\RIVER 9-1\T-RIVE

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	6.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

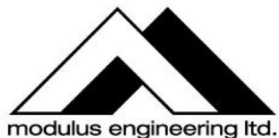
Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



**Read Carefully Prior to
Manufacture and Installation**

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)

2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.

3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses

4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all interested parties (e.g. Building designer where required)

5. Cut members to bear tightly against one another

6. Place plates on each face of truss at each joint and use proper roller or hydraulic press. Knots and warps at locations are regulated by TPIC Appendix G

7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC

8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture

9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment

10. Connections not shown are the responsibility of others

11. Do not cut or alter truss members or plates without prior approval of an engineer

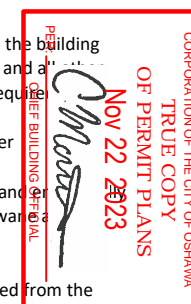
12. Install and load vertically unless otherwise noted

13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient

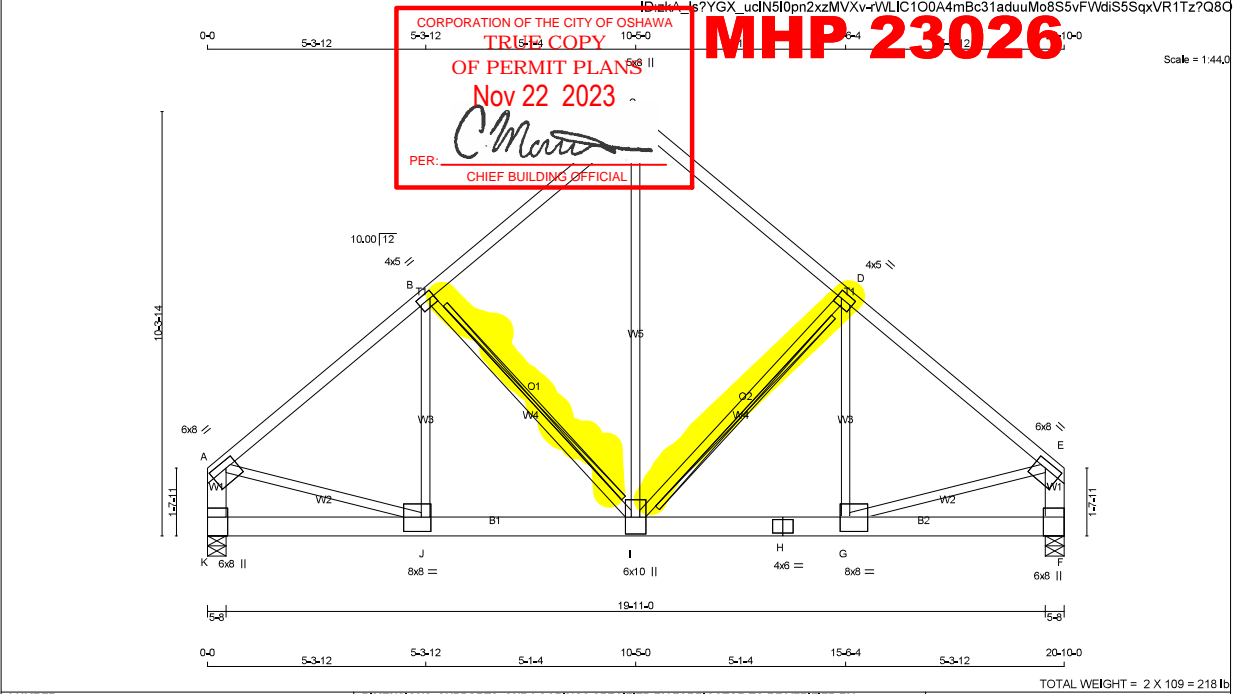
14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G

16. Building designer must review individual component drawings to ensure they are suitable for the structure

15. Not designed for solar panels unless specifically noted



MHP 23026



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:		
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	DOWN	BRG	IN-SX	TOP CH. LL = 34.8 PSF		
C - E	2x4	DRY	No.2	SPF	JT VERT	HORZ	UP/LIFT	IN-SX	DL = 6.0 PSF		
K - A	2x6	DRY	No.2	SPF	K	8676	0	8676	BOT CH. LL = 0.0 PSF		
F - E	2x6	DRY	No.2	SPF	F	8676	0	8676	DL = 7.3 PSF		
K - H	2x6	DRY	2100F 1.8E	SPF					TOTAL LOAD = 48.1 PSF		
H - F	2x6	DRY	2100F 1.8E	SPF							

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0,122"x3") SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
K-A	12	TOP
F-E	12	TOP
BOTTOM CHORDS : (0,122"x3") SPIRAL NAILS		
K-H	8	SIDE(347.6)
H-F	8	SIDE(347.6)
WEBS : (0,122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				LOADING			
JT TYPE	PLATES	W	LEN	Y	X	TOTAL LOAD CASES: (4)	
A	TMWV4	MT20	6.0	8.0	2.00	3.25	
B	TMWV4	MT20	4.0	5.0	2.00	1.25	
C	TTW+p	MT20	5.0	8.0	Edge		
D	TMWV4	MT20	4.0	5.0	2.00	1.25	
E	TMWV4	MT20	6.0	8.0	2.00	3.25	
F	BMV1+1	MT20	6.0	8.0	Edge	0.50	
G	BMWV4	MT20	8.0	8.0	4.25	2.75	
H	BS4	MT20	4.0	6.0			
I	BMWVW+1	MT20	6.0	10.0			
J	BMWV4	MT20	8.0	8.0	4.25	2.75	
K	BMV1+1	MT20	6.0	8.0	5.50		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

MODULUS ENGINEERING LTD.

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REVIEW FOR TRUSS COMPONENT ONLY

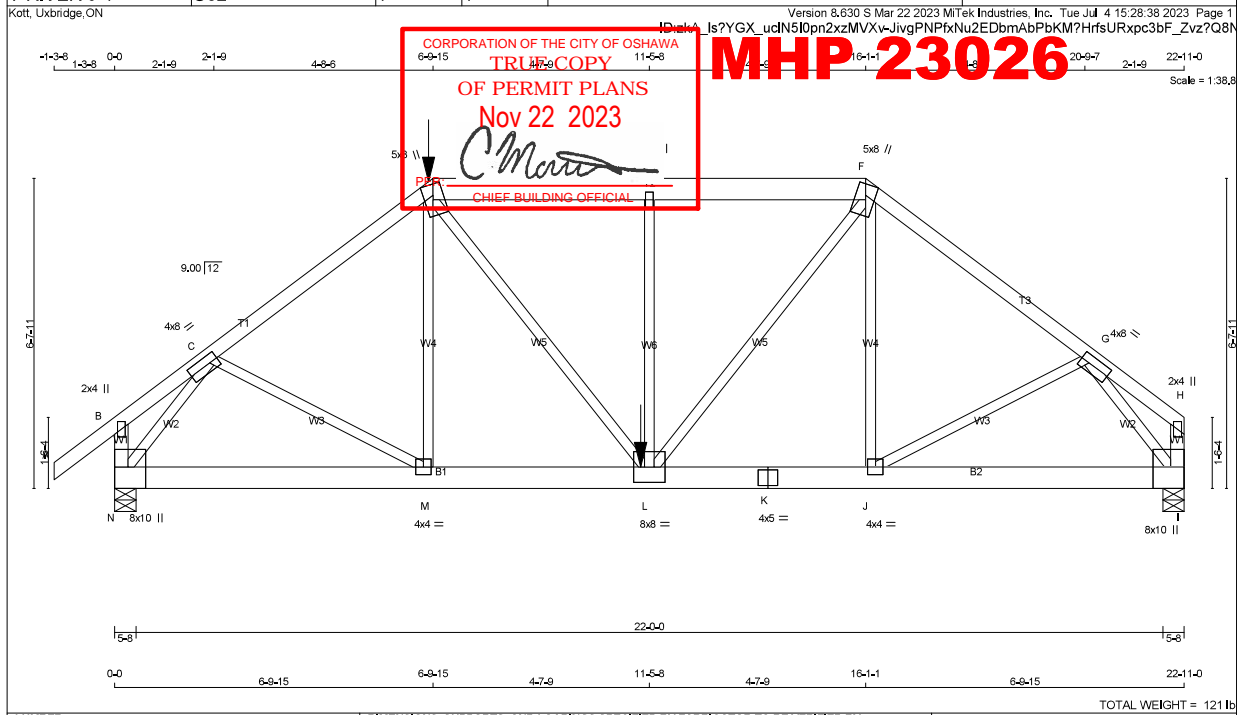
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

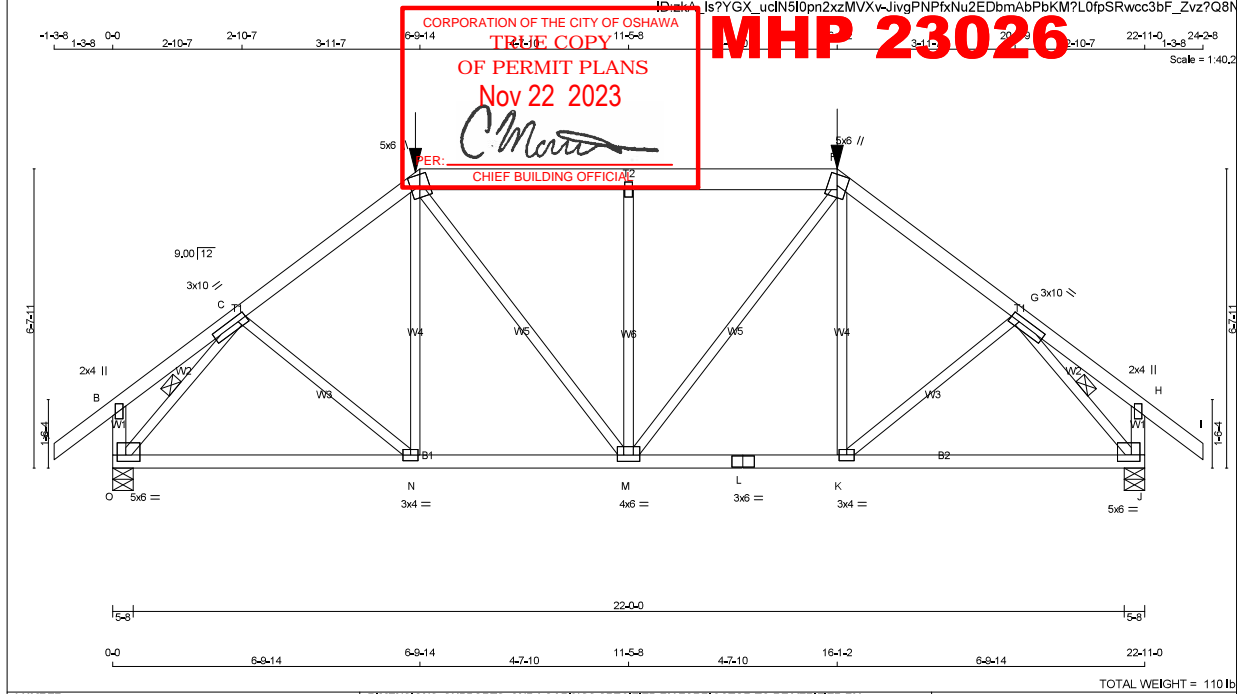
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x6 DRY No.2 SPF F - H 2x4 DRY No.2 SPF N - B 2x4 DRY No.2 SPF I - H 2x4 DRY No.2 SPF N - K 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF ALL WEBS EXCEPT 2x3 DRY No.2 SPF DRY, SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION DOWN MAXIMUM FACTORED GROSS REACTION UPLIFT INPUT BRG REQD BRG JT 3285 0 3285 0 0 5-8 5-8 I 2681 0 2681 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LOASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL N 2296 1659 / 0 0 / 0 0 / 0 636 / 0 0 / 0 I 1873 1361 / 0 0 / 0 0 / 0 511 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I BEARING SIZE FACTOR = 1.15 AT JNT(S) N (BASED ON SUPPORT DEPTH = 1-8) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. MAX. UNBRACED W E B S FR-TO FROM TO LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC) A-B 0 / 49 -119.4 -119.4 0.18 (1) 10.00 C-M 0 / 644 0.16 (1) B-C 0 / 68 -119.4 -119.4 0.35 (1) 10.00 M-D -53 / 170 0.06 (4) C-D -3608 / 0 -119.4 -119.4 0.75 (1) 3.05 D-L 0 / 1128 0.28 (1) D-E -3547 / 0 -225.2 -225.2 0.41 (1) 4.07 L-E -973 / 0 0.63 (1) E-F -3547 / 0 -119.4 -119.4 0.31 (1) 4.21 L-F 0 / 1844 0.46 (1) F-G -3073 / 0 -119.4 -119.4 0.65 (1) 3.38 J-F -89 / 90 0.06 (1) G-H 0 / 70 -119.4 -119.4 0.36 (1) 10.00 J-G 0 / 494 0.12 (1) N-B -169 / 0 0.0 0.0 0.02 (1) 7.81 N-C -3798 / 0 0.82 (1) I-H -2 / 0 0.0 0.0 0.00 (4) 7.81 G-I -3308 / 0 0.71 (1) N-M 0 / 2306 -34.4 -34.4 0.40 (1) 10.00 M-L 0 / 2872 -34.4 -34.4 0.46 (1) 10.00 L-K 0 / 2443 -18.2 -18.2 0.38 (1) 10.00 K-J 0 / 2443 -18.2 -18.2 0.38 (1) 10.00 J-I 0 / 2009 -18.2 -18.2 0.32 (1) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. D 6-9-15 -612 -612 - FRONT VERT TOTAL - C1 I 11-3-4 -1359 -1359 - FRONT VERT TOTAL - C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.	DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 34.8 PSF BOT CH. LL = 6.0 PSF DL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-15 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF G.S.L. LOADS APPLIED TO FIRST 11-3-4 OF SPAN MEASURED FROM THE LEFT. *** NON STANDARD GIRDER *** ADDTL USER DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F., G.S.L. PLUS 6.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.76") CALCULATED VERT. DEFL.(LL) = L/999 (0.09") ALLOWABLE DEFL.(TL) = L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/999 (0.15") CSI TC=0.75/1.00 (C-D-1), BC=0.46/1.00 (L-M-1), WB=0.82/1.00 (C-N-1), SSI=0.39/1.00 (D-E-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.88 (C) (INPUT = 0.90) JSI METAL= 0.71 (K) (INPUT = 1.00)
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA	
N, L, G, A, RULES				BEARINGS										SPECIFIED LOADS:	
CHORDS		SIZE	LUMBER	DESCR.	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	TOP CH. LL = 34.8 PSF				
					GROSS REACTION		GROSS REACTION		BRG	BRG	DL = 6.0 PSF				
A - D	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
F - I	2x4	DRY	No.2	SPF	O	3029	0	3029	0	5-8	4-11	LL = 0.0 PSF			
O - B	2x4	DRY	No.2	SPF	J	3029	0	3029	0	0	5-8	4-11			
J - H	2x4	DRY	No.2	SPF									DL = 7.3 PSF		
O - L	2x4	DRY	No.2	SPF									TOTAL LOAD = 48.1 PSF		
L - J	2x4	DRY	No.2	SPF											
				UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C	
				1ST LOADING											
				MAXIMUM COMPONENT REACTIONS											
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
						O	2116	1536 / 0	0 / 0	0 / 0	0 / 0	580 / 0	0 / 0		
						J	2116	1536 / 0	0 / 0	0 / 0	0 / 0	580 / 0	0 / 0		
DRY: SEASONED LUMBER.														LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM	

				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J				ORDER TYPE: CPrimeHip SIDE SETBACK = 6-0-14 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55% OF GSL.			
PLATES (table is in inches)				BRACING							
JT TYPE	PLATES	W	LEN Y X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED, ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
B TMV+p	MT20	2.0	4.0	1 Lateral brace(S) AT 1/2 LENGTH OF C-o, G-J,				- PART 9 OF BCBC 2018, NBC-2019AE			
C TMWw+4	MT20	3.0	10.0 1.50 2.50	END VERTICAL BRACE(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW							
D TMWw+m	MT20	5.0	6.0 Edge	LOADING							
E TMWw+w	MT20	2.0	4.0	TOTAL LOAD CASES: (4)							
F TMWw+m	MT20	5.0	6.0 Edge								
G TMWw+4	MT20	3.0	10.0 1.50 2.50								
H TMV+p	MT20	2.0	4.0								
J BMWw+4	MT20	5.0	6.0 1.75 2.25								
K BMWw+4	MT20	3.0	4.0								
L BS+4	MT20	3.0	6.0								

FACTORED CONCENTRATED LOADS (LBS)				CONNECTION REQUIREMENTS			
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6-9-14	-611	-611	---	FRONT	VERT	TOTAL
F	16-1-2	-611	-611	---	FRONT	VERT	TOTAL
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.				NAIL VALUES			
				PLATE GRIP(DRY) SHEAR SECTION			
				(PSI) (FUI) (FUI)			
				MAX MIN MAX MIN MAX MIN			
				MT20 650 371 1747 788 1987 1873			

MODULUS ENGINEERING LTD.

07/04/2023

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REVIEW FOR TRUSS COMPONENT ONLY

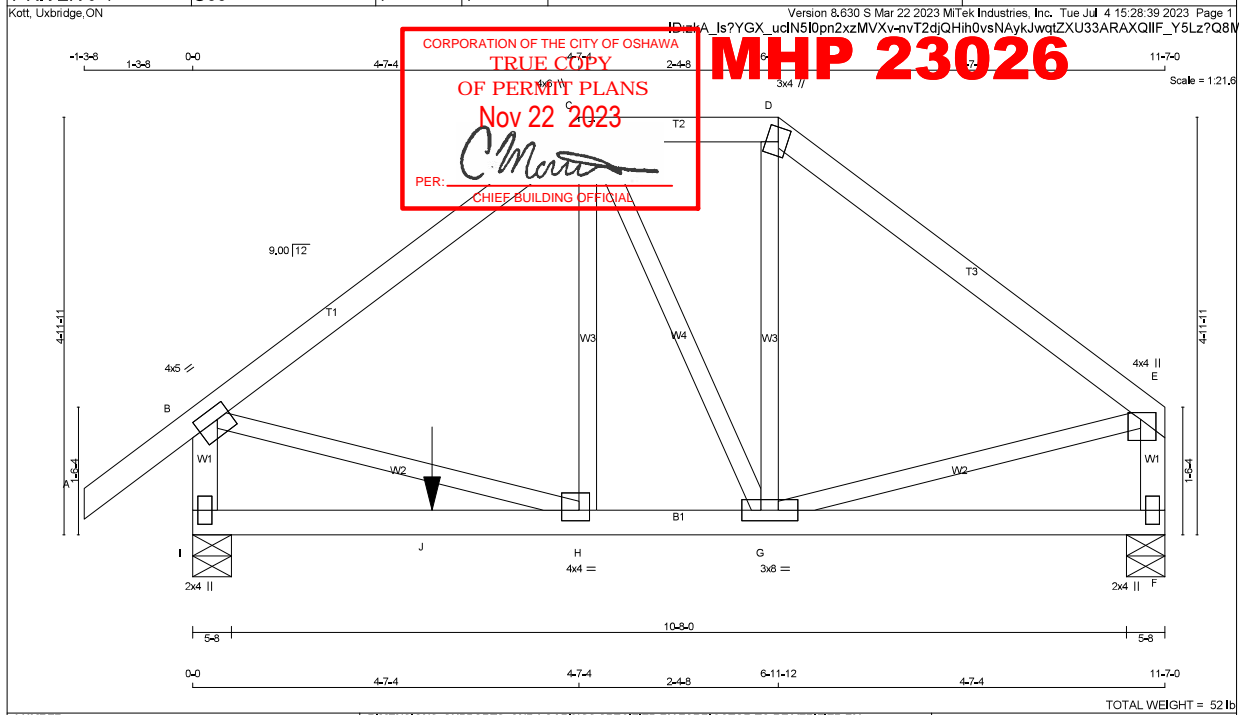
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSCI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com

KOTT

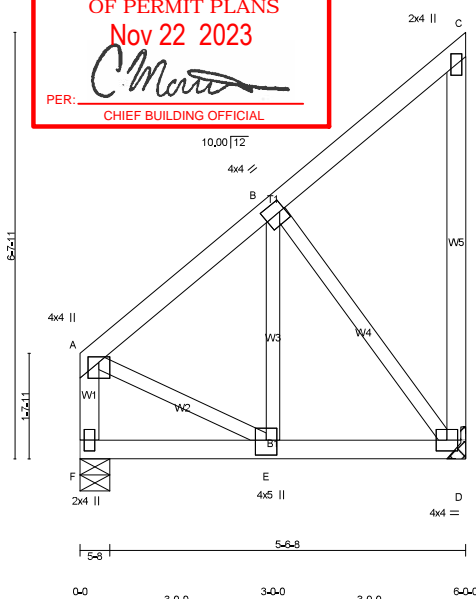


LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA	
N, L, G, A, RULES				BEARINGS										*** SPECIAL LOADS ANALYSIS ***	
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
A - C	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	LOADS WERE DERIVED FROM USER INPUT			
C - D	2x4	DRY	No.2	SPF	I	1539	0	1539	0	5-8	1-8	NO FURTHER MODIFICATIONS WERE MADE			
D - E	2x4	DRY	No.2	SPF	F	986	0	986	0	5-8	1-8				
E - F	2x4	DRY	No.2	SPF											
F - G	2x4	DRY	No.2	SPF											
G - H	2x4	DRY	No.2	SPF											
H - I	2x4	DRY	No.2	SPF											
I - J	2x4	DRY	No.2	SPF											
J - K	2x4	DRY	No.2	SPF											
K - L	2x4	DRY	No.2	SPF											
L - M	2x4	DRY	No.2	SPF											
M - N	2x4	DRY	No.2	SPF											
N - O	2x4	DRY	No.2	SPF											
O - P	2x4	DRY	No.2	SPF											
P - Q	2x4	DRY	No.2	SPF											
Q - R	2x4	DRY	No.2	SPF											
R - S	2x4	DRY	No.2	SPF											
S - T	2x4	DRY	No.2	SPF											
T - U	2x4	DRY	No.2	SPF											
U - V	2x4	DRY	No.2	SPF											
V - W	2x4	DRY	No.2	SPF											
W - X	2x4	DRY	No.2	SPF											
X - Y	2x4	DRY	No.2	SPF											
Y - Z	2x4	DRY	No.2	SPF											
Z - AA	2x4	DRY	No.2	SPF											
AA - AB	2x4	DRY	No.2	SPF											
AB - AC	2x4	DRY	No.2	SPF											
AC - AD	2x4	DRY	No.2	SPF											
AD - AE	2x4	DRY	No.2	SPF											
AE - AF	2x4	DRY	No.2	SPF											
AF - AG	2x4	DRY	No.2	SPF											
AG - AH	2x4	DRY	No.2	SPF											
AH - AI	2x4	DRY	No.2	SPF											
AI - AJ	2x4	DRY	No.2	SPF											
AJ - AK	2x4	DRY	No.2	SPF											
AK - AL	2x4	DRY	No.2	SPF											
AL - AM	2x4	DRY	No.2	SPF											
AM - AN	2x4	DRY	No.2	SPF											
AN - AO	2x4	DRY	No.2	SPF											
AO - AP	2x4	DRY	No.2	SPF											
AP - AQ	2x4	DRY	No.2	SPF											
AQ - AR	2x4	DRY	No.2	SPF											
AR - AS	2x4	DRY	No.2	SPF											
AS - AT	2x4	DRY	No.2	SPF											
AT - AU	2x4	DRY	No.2	SPF											
AU - AV	2x4	DRY	No.2	SPF											
AV - AW	2x4	DRY	No.2	SPF											
AW - AX	2x4	DRY	No.2	SPF											
AX - AY	2x4	DRY	No.2	SPF											
AY - AZ	2x4	DRY	No.2	SPF											
AZ - BA	2x4	DRY	No.2	SPF											
BA - BB	2x4	DRY	No.2	SPF											
BB - BC	2x4	DRY	No.2	SPF											
BC - BD	2x4	DRY	No.2	SPF											
BD - BE	2x4	DRY	No.2	SPF											
BE - BF	2x4	DRY	No.2	SPF											
BF - BG	2x4	DRY	No.2	SPF											
BG - BH	2x4	DRY	No.2	SPF											
BH - BI	2x4	DRY	No.2	SPF											
BI - BJ	2x4	DRY	No.2	SPF											
BJ - BK	2x4	DRY	No.2	SPF											
BK - BL	2x4	DRY	No.2	SPF											
BL - BM	2x4	DRY	No.2	SPF											
BM - BN	2x4	DRY	No.2	SPF											
BN - BO	2x4	DRY	No.2	SPF											
BO - BP	2x4	DRY	No.2	SPF											
BP - BQ	2x4	DRY	No.2	SPF											
BQ - BR	2x4	DRY	No.2	SPF											
BR - BS	2x4	DRY	No.2	SPF											
BS - BT	2x4	DRY	No.2	SPF											
BT - BU	2x4	DRY	No.2	SPF											
BU - BV	2x4	DRY	No.2	SPF											
BV - BW	2x4	DRY	No.2	SPF											
BW - BX	2x4	DRY	No.2	SPF											
BX - BY	2x4	DRY	No.2	SPF											
BY - BZ	2x4	DRY	No.2	SPF											
BZ - CA	2x4	DRY	No.2	SPF											
CA - CB	2x4	DRY	No.2	SPF											
CB - CC	2x4	DRY	No.2	SPF											
CC - CD	2x4	DRY	No.2	SPF											
CD - CE	2x4	DRY	No.2	SPF											
CE - CF	2x4	DRY	No.2	SPF											
CF - CG	2x4	DRY	No.2	SPF											
CG - CH	2x4	DRY	No.2	SPF											
CH - CI	2x4	DRY	No.2	SPF											
CI - CJ	2x4	DRY	No.2	SPF											
CJ - CK	2x4	DRY	No.2	SPF											
CK - CL	2x4	DRY	No.2	SPF											
CL - CM	2x4	DRY	No.2	SPF											
CM - CN	2x4	DRY	No.2	SPF											
CN - CO	2x4	DRY	No.2	SPF											
CO - CP	2x4	DRY	No.2	SPF											
CP - CQ	2x4	DRY	No.2	SPF											
CQ - CR	2x4	DRY	No.2	SPF											
CR - CS	2x4	DRY	No.2	SPF											
CS - CT	2x4	DRY	No.2	SPF											
CT - CU	2x4	DRY	No.2	SPF											
CU - CV	2x4	DRY	No.2	SPF											
CV - CW	2x4	DRY	No.2	SPF											
CW - CX	2x4	DRY	No.2	SPF											
CX - CY	2x4	DRY	No.2	SPF											
CY - CZ	2x4	DRY	No.2	SPF											
CZ - DA	2x4	DRY	No.2	SPF											
DA - DB	2x4	DRY	No.2	SPF											



MHP 23026

Scale = 1:28.2



TOTAL WEIGHT = 331b

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		SPF	
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMVW+4	MT20	4.0	4.0	2.00 1.00
C	TMV+p	MT20	2.0	4.0	
D	BMVW1+4	MT20	4.0	4.0	
E	BMVWV+4	MT20	4.0	5.0	2.75 2.00
F	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
F	1355	0	1355	0	5-8
D	1355	0	1355	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERLIVE	WIND	DEAD	SOIL
F	947	685 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
D	947	685 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)	
FR-TO		FROM	TO	FR-TO			
F-A	-971 / 0	0.0	0.0 0.11 (1)	A-E	0 / 716	0.18 (1)	
A-B	-829 / 0	-119.4	-119.4 0.20 (1)	E-B	0 / 940	0.23 (1)	
B-C	-26 / 0	-119.4	-119.4 0.18 (1)	B-D	-1061 / 0	0.40 (1)	
D-C	-138 / 0	0.0	0.0 0.12 (1)				
F-E	0 / 0	-332.3	-332.3 0.51 (1)				
E-D	0 / 657	-332.3	-332.3 0.63 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-7-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 11-7-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI TC=0.20/1.00 (A-B 1), BC=0.63/1.00 (D-E 1),

WB=0.40/1.00 (B-D 1), SSI=0.53/1.00 (E-F 1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

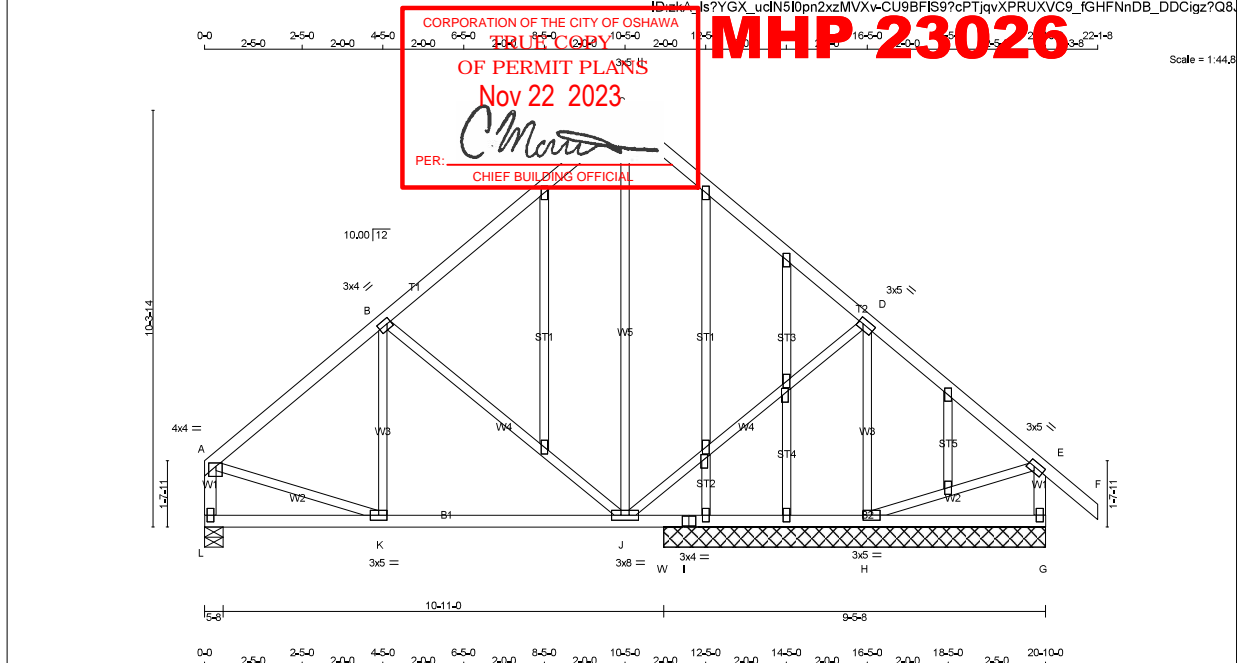
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com



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 **KOTT**



TOTAL WEIGHT = 113 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:	
A - C	2x4	DRY	No.2	SPF		GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	34.8 PSF
C - F	2x4	DRY	No.2	SPF		VERT	DOWN	HORZ	UPLIFT	DL =	8.0 PSF
L - A	2x4	DRY	No.2	SPF		L	1086	0	0	BOT CH. LL =	0.0 PSF
G - E	2x4	DRY	No.2	SPF		H	1218	0	0	DL =	7.3 PSF
L - I	2x4	DRY	No.2	SPF		G	531	0	0	TOTAL LOAD =	48.1 PSF
I - G	2x4	DRY	No.2	SPF		W	198	0	0		

ALL WEBS EXCEPT				UNFACTORED REACTIONS				SPACING = 24.0 IN. G/C			
ALL GABLE WEBS	2x3	DRY	No.2	SPF	1ST CASE	MAX. MIN. COMPONENT REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015	
DRY: SEASONED LUMBER.					JT	COMBINED	SNOW	LIVE	PERM. LIVE	THIS DESIGN COMPLIES WITH:	
GABLE STUDS SPACED AT 2'-0" OC.					L	757	557 / 0	0 / 0	0 / 0	- PART 9 OF BCBC 2018, NBC-2019AE	
					H	853	608 / 0	0 / 0	0 / 0	- PART 9 OF OBC 2021 (2019 AMENDMENT)	
					G	366	295 / 0	0 / 0	0 / 0	- CSA 086-14	
					W	142	96 / 0	0 / 0	0 / 0	- TPIC 2014	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H, G, W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5,26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	-967 / 0	-119.4	-119.4, 0.63 (1)	5,26	J-C	0 / 74	0,03 (4)
B-C	-544 / 0	-119.4	-119.4, 0.63 (1)	6,25	J-D	0 / 308	0,07 (1)
C-D	-545 / 0	-119.4	-119.4, 0.63 (1)	6,25	H-D	-1186 / 0	0,50 (1)
D-E	-127 / 0	-119.4	-119.4, 0.59 (1)	6,25	B-J	-529 / 0	0,66 (1)
E-F	0 / 53	-119.4	-119.4, 0.16 (1)	10,00	K-B	-112 / 66	0,05 (1)
L-A	-1059 / 0	0,0	0,0 0,11 (1)	7,65	A-K	0 / 824	0,19 (1)
G-E	-496 / 0	0,0	0,0 0,05 (1)	7,81	H-E	0 / 152	0,03 (1)

L-K				NAIL VALUES			
J-K	0 / 790	-18,2	-18,2 0,13 (4)	10,00	PLATE GRIP (DRY)	SHEAR SECTION	
J-W	0 / 146	-18,2	-18,2 0,17 (1)	10,00	(PSI)	(PLI)	(PLI)
W-I	0 / 146	-18,2	-18,2 0,17 (1)	10,00	MAX MIN	MAX MIN	MAX MIN
I-H	0 / 146	-18,2	-18,2 0,17 (1)	10,00	MT20	650	371
H-G	0 / 0	-18,2	-18,2 0,09 (4)	10,00		1747	788

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0,250 inches

PLATE ROTATION TOL. = 5,0 Deg.

JSI GRIP= 0,89 (D) (INPUT = 0,90)
JSI METAL= 0,27 (A) (INPUT = 1,00)

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

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MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

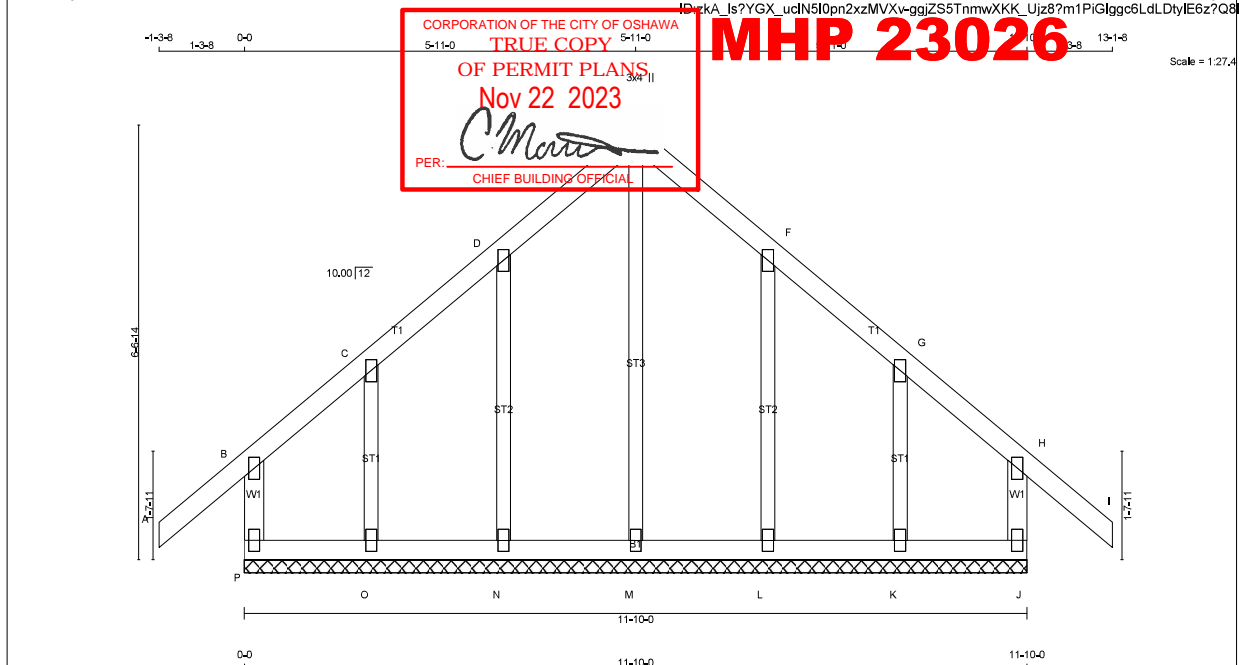
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TOTAL WEIGHT = 541b

LUMBER
N, L, G, A, RULES
CHORDS SIZE LUMBER DESCR.
P - B 2x4 DRY No.2 SPF
A - E 2x4 DRY No.2 SPF
E - I 2x4 DRY No.2 SPF
J - H 2x4 DRY No.2 SPF
P - J 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0.0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	UNBRACED LENGTH
FR-TO					FR-TO				
P-B	-285 / 0	0.0	0.05 (1)	7.81	M-E	-297 / 0	0.20 (1)		
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00	N-D	-255 / 0	0.09 (1)	
B-C	-1 / 10	-119.4	-119.4	0.11 (1)	10.00	O-C	-183 / 0	0.03 (1)	
C-D	0 / 44	-119.4	-119.4	0.07 (1)	10.00	L-F	-255 / 0	0.09 (1)	
D-E	0 / 41	-119.4	-119.4	0.07 (1)	10.00	K-G	-183 / 0	0.03 (1)	
E-F	0 / 41	-119.4	-119.4	0.07 (1)	10.00				
F-G	0 / 44	-119.4	-119.4	0.07 (1)	10.00				
G-H	-1 / 10	-119.4	-119.4	0.11 (1)	10.00				
H-I	0 / 53	-119.4	-119.4	0.16 (1)	10.00				
J-H	-285 / 0	0.0	0.05 (1)	7.81					
P-O	-22 / 0	-18.2	-18.2	0.02 (4)	6.25				
O-N	-28 / 0	-18.2	-18.2	0.02 (4)	6.25				
N-M	-34 / 0	-18.2	-18.2	0.01 (4)	6.25				
M-L	-34 / 0	-18.2	-18.2	0.01 (4)	6.25				
L-K	-28 / 0	-18.2	-18.2	0.02 (4)	6.25				
K-J	-22 / 0	-18.2	-18.2	0.02 (4)	6.25				

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF
SPACING = 24.0 IN. G.C.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.16/1.00 (A-B:1), BC=0.02/1.00 (N-O:4), WB=0.20/1.00 (E-M:1), SSI=0.10/1.00 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

07/04/2023

P. R. HEAL

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

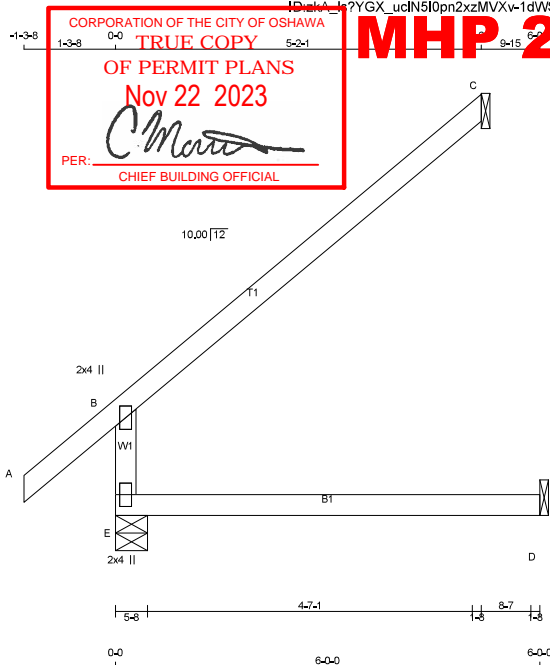
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC061 (VER 06/2017) BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
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Scale = 1/256

TOTAL WEIGHT = 3 X 18 = 54 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	615	0	615	0	0	5-8	1-8	1-8	1-8
C	232	0	232	0	0	1-8	1-8	1-8	1-8
D	46	0	52	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	427	321 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
C	158	135 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM	TO	CS (LC)	UNBRACED LENGTH	FR-TO			
E-B	-552 / 0	0.0	0.0	0.12 (4)	7.81				
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00				
B-C	-49 / 0	-119.4	-119.4	0.55 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 34.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.3 PSF
TOTAL LOAD	= 48.1 PSF

SPACING = 24.0 IN. G/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI TC=0.55/1.00 (B-C:1) BC=0.14/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.23/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	(PU)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

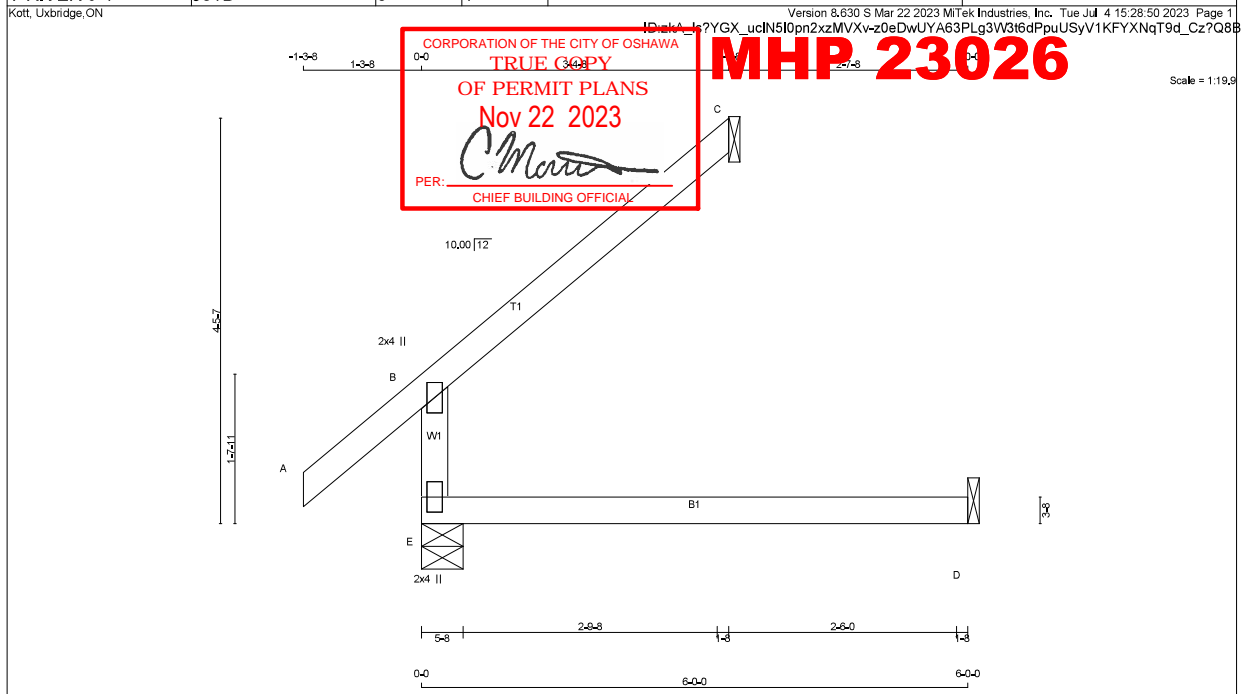


REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQD BRG</th></tr><tr><th>JT</th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th><th>IN-SX</th><th>IN-SX</th></tr><tr><td>E</td><td>480</td><td>0</td><td>480</td><td>0</td><td>0</td><td>5-8</td><td>1-8</td></tr><tr><td>C</td><td>151</td><td>0</td><td>151</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>46</td><td>0</td><td>52</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><th>JT</th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM. LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>335</td><td>243 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>92 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>103</td><td>88 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>15 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>37</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>37 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>FACTORED LC1 MAX (LC)</th><th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>FACTORED MAX. (LBS)</th><th>FACTORED UNBRACED LENGTH (FT)</th></tr><tr><td>FR-TO</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>E-B</td><td>-417 / 0</td><td>0.0</td><td>0.0 0.12 (4)</td><td></td><td></td><td></td><td>7.81</td></tr><tr><td>A-B</td><td>0 / 53</td><td>-119.4</td><td>-119.4 0.16 (1)</td><td></td><td></td><td></td><td>10.00</td></tr><tr><td>B-C</td><td>-32 / 0</td><td>-119.4</td><td>-119.4 0.23 (1)</td><td></td><td></td><td></td><td>6.25</td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.14 (4)</td><td></td><td></td><td></td><td>10.00</td></tr></table>			FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	E	480	0	480	0	0	5-8	1-8	C	151	0	151	0	0	1-8	1-8	D	46	0	52	0	0	1-8	1-8	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	E	335	243 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0	C	103	88 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0	D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	CHORDS				WEBS				MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MEMB.	FACTORED FORCE (LBS)	FACTORED MAX. (LBS)	FACTORED UNBRACED LENGTH (FT)	FR-TO								E-B	-417 / 0	0.0	0.0 0.12 (4)				7.81	A-B	0 / 53	-119.4	-119.4 0.16 (1)				10.00	B-C	-32 / 0	-119.4	-119.4 0.23 (1)				6.25	E-D	0 / 0	-18.2	-18.2 0.14 (4)				10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04") CSI TC=0.23/1.00 (B-C-1) , BC=0.14/1.00 (D-E-4) , WB=0.00/1.00 (n/a.0) , SSI=0.15/1.00 (B-C-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.29 (B) (INPUT = 0.90) JSI METAL= 0.22 (B) (INPUT = 1.00)	
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG																																																																																																																														
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MODULUS ENGINEERING LTD.

07/04/2023

LICENSED PROFESSIONAL ENGINEER

P. R. HEAL

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL



TOTAL WEIGHT = 3 X 12 = 36 lb

MODULUS ENGINEERING LTD.

07/04/2023

P. R. HEAL

PROVINCE OF ONTARIO

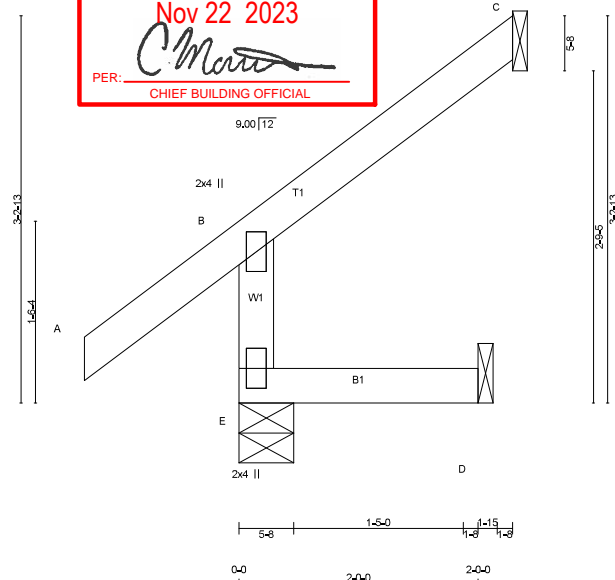
REVIEW FOR TRUSS COMPONENT ONLY

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VOIDS THE ENGINEERS SEAL





MHP 23026



Scale = 1:15,1

TOTAL WEIGHT = 3 X 9 = 27 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
E - B 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
E - D 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
E	355	0	355	0	0	5-8	1-8	1-8	1-8
C	103	0	103	0	0	1-8	1-8	1-8	1-8
D	17	0	19	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	245	195 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
C	70	60 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. CSI (LC)
		FROM	TO		FR-TO		
E-B	-335 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00		
B-C	-20 / 0	-119.4	-119.4	0.10 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
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DESIGN ASSUMPTIONS
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WB=0.00/1.00 (n/a.0), SSI=0.11/1.00 (B-C-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

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TOTAL WEIGHT = 10 lb

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