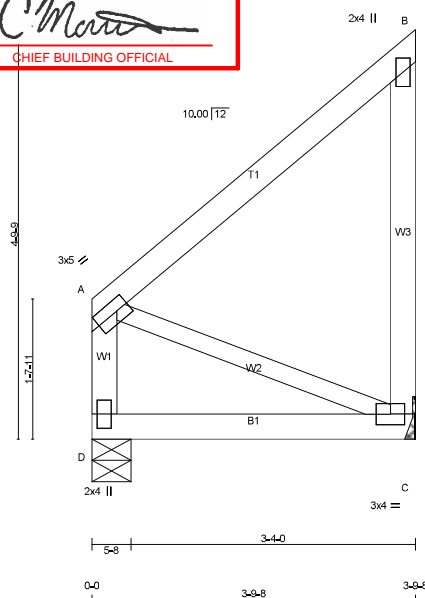


JOB NAME T-RIVER 6-2	TRUSS NAME G02	CORPORATION OF THE CITY OF TORONTO TRUE COPY OF PERMIT PLANS Nov 04 2023 PER: [Signature] CHIEF BUILDING OFFICIAL	JOB DESC. ZADORRA ESTATES INC.	Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Tue Jul 4 14:33:08 2023 Page 1 ID:RtjVCInd4HbpXNgtnXTCfzM992-p4ioVWk6s9ELKt1m6Egin74DXJlDwKSkwbnhwz7QyP																																																																																																																																																																																																				
Scale = 1:40.7																																																																																																																																																																																																								
TOTAL WEIGHT = 117 lb																																																																																																																																																																																																								
LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER DESCR. SPF A - C 2x4 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - I 2x4 DRY No.2 SPF R - B 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF R - O 2x4 DRY 2100F 1.8E SPF O - E 2x4 DRY No.2 SPF N - L 2x4 DRY No.2 SPF K - G 2x4 DRY No.2 SPF K - J 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.																																																																																																																																																																																																								
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQ'D BRG IN-SX JT VERT HORZ DOWN HORZ UPLIFT IN-SX R 2181 0 2181 0 0 5-8 2-5 J 2370 0 2370 0 0 5-8 2-8 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL R 1522 1115 / 0 0 / 0 0 / 0 407 / 0 0 / 0 J 1653 1211 / 0 0 / 0 0 / 0 442 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J BEARING SIZE FACTOR = 1.15 AT JNT(S) R (BASED ON SUPPORT DEPTH = 1-8) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><thead><tr><th colspan="3">CHORDS</th><th colspan="3">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD LC1 (PLF)</th><th>MAX. CSI (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. CSI (LC)</th></tr></thead><tbody><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>LENGTH FR-TO</td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 53</td><td>-119.4 -119.4</td><td>0.18 (1)</td><td>10.00</td><td>Q-C</td><td>-401 / 0</td><td>0.19 (1)</td></tr><tr><td>B-C</td><td>-1972 / 0</td><td>-119.4 -119.4</td><td>0.67 (1)</td><td>3.97</td><td>C-P</td><td>0 / 1567</td><td>0.39 (1)</td></tr><tr><td>C-D</td><td>-2596 / 0</td><td>-119.4 -119.4</td><td>0.23 (1)</td><td>4.91</td><td>P-D</td><td>-1239 / 0</td><td>0.56 (1)</td></tr><tr><td>D-E</td><td>-3237 / 0</td><td>-119.4 -119.4</td><td>0.25 (1)</td><td>4.46</td><td>P-N</td><td>0 / 2553</td><td>0.63 (1)</td></tr><tr><td>E-F</td><td>-3260 / 0</td><td>-119.4 -119.4</td><td>0.19 (1)</td><td>4.52</td><td>D-N</td><td>0 / 657</td><td>0.21 (1)</td></tr><tr><td>F-G</td><td>-2712 / 0</td><td>-119.4 -119.4</td><td>0.33 (1)</td><td>3.88</td><td>N-F</td><td>0 / 1774</td><td>0.44 (1)</td></tr><tr><td>G-H</td><td>-2750 / 0</td><td>-119.4 -119.4</td><td>0.27 (1)</td><td>3.89</td><td>M-F</td><td>0 / 773</td><td>0.03 (4)</td></tr><tr><td>H-I</td><td>0 / 53</td><td>-119.4 -119.4</td><td>0.18 (1)</td><td>10.00</td><td>M-G</td><td>-76 / 0</td><td>0.02 (1)</td></tr><tr><td>R-B</td><td>-2134 / 0</td><td>0.0 0.0</td><td>0.25 (1)</td><td>5.75</td><td>B-Q</td><td>0 / 1577</td><td>0.39 (1)</td></tr><tr><td>J-H</td><td>-2302 / 0</td><td>0.0 0.0</td><td>0.27 (1)</td><td>5.56</td><td>L-H</td><td>0 / 2110</td><td>0.52 (1)</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>L-J</td><td>-89 / 0</td><td>0.01 (1)</td></tr><tr><td>R-Q</td><td>0 / 0</td><td>-18.2 -18.2</td><td>0.06 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>Q-P</td><td>0 / 1526</td><td>-18.2 -18.2</td><td>0.39 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>P-S</td><td>0 / 98</td><td>-18.2 -18.2</td><td>0.63 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>S-O</td><td>0 / 98</td><td>-18.2 -18.2</td><td>0.63 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 823</td><td>0.0 0.0</td><td>0.29 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>N-E</td><td>-517 / 0</td><td>0.0 0.0</td><td>0.19 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 2080</td><td>-18.2 -18.2</td><td>0.41 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>M-L</td><td>0 / 2151</td><td>-18.3 -18.3</td><td>0.42 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>K-L</td><td>0 / 15</td><td>0.0 0.0</td><td>0.12 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>L-G</td><td>-206 / 0</td><td>0.0 0.0</td><td>0.12 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>K-J</td><td>0 / 71</td><td>-18.2 -18.2</td><td>0.02 (1)</td><td>10.00</td><td></td><td></td><td></td></tr></tbody></table> FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX+ MAX+ FACE DIR. TYPE HEEL CONN. O 14-4-12 -446 -446 — FRONT VERT TOTAL — C1 S 12-7-12 -618 -618 — FRONT VERT TOTAL — C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.					CHORDS			WEBS			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	FR-TO		FROM TO		LENGTH FR-TO			A-B	0 / 53	-119.4 -119.4	0.18 (1)	10.00	Q-C	-401 / 0	0.19 (1)	B-C	-1972 / 0	-119.4 -119.4	0.67 (1)	3.97	C-P	0 / 1567	0.39 (1)	C-D	-2596 / 0	-119.4 -119.4	0.23 (1)	4.91	P-D	-1239 / 0	0.56 (1)	D-E	-3237 / 0	-119.4 -119.4	0.25 (1)	4.46	P-N	0 / 2553	0.63 (1)	E-F	-3260 / 0	-119.4 -119.4	0.19 (1)	4.52	D-N	0 / 657	0.21 (1)	F-G	-2712 / 0	-119.4 -119.4	0.33 (1)	3.88	N-F	0 / 1774	0.44 (1)	G-H	-2750 / 0	-119.4 -119.4	0.27 (1)	3.89	M-F	0 / 773	0.03 (4)	H-I	0 / 53	-119.4 -119.4	0.18 (1)	10.00	M-G	-76 / 0	0.02 (1)	R-B	-2134 / 0	0.0 0.0	0.25 (1)	5.75	B-Q	0 / 1577	0.39 (1)	J-H	-2302 / 0	0.0 0.0	0.27 (1)	5.56	L-H	0 / 2110	0.52 (1)						L-J	-89 / 0	0.01 (1)	R-Q	0 / 0	-18.2 -18.2	0.06 (4)	10.00				Q-P	0 / 1526	-18.2 -18.2	0.39 (1)	10.00				P-S	0 / 98	-18.2 -18.2	0.63 (1)	10.00				S-O	0 / 98	-18.2 -18.2	0.63 (1)	10.00				O-N	0 / 823	0.0 0.0	0.29 (1)	10.00				N-E	-517 / 0	0.0 0.0	0.19 (1)	7.81				N-M	0 / 2080	-18.2 -18.2	0.41 (1)	10.00				M-L	0 / 2151	-18.3 -18.3	0.42 (1)	10.00				K-L	0 / 15	0.0 0.0	0.12 (1)	10.00				L-G	-206 / 0	0.0 0.0	0.12 (1)	7.81				K-J	0 / 71	-18.2 -18.2	0.02 (1)	10.00			
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DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN.C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM *** NON STANDARD GIRDER *** ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2018 NBC2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F., G.S.L PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFLL(LL)= L/360 (0.76") CALCULATED VERT. DEFLL(LL) = L/999 (0.18") ALLOWABLE DEFLL(TL)= L/360 (0.76") CALCULATED VERT. DEFLL(TL) = L/884 (0.31") CSI TC=0.67/1.00 (B-C:1), BC=0.63/1.00 (O-P:1), WB=0.63/1.00 (N-P:1), SS=0.32/1.00 (O-P:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE LEFT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLU) MAX MN MAX MIN MAX MN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.88 (C) (INPUT = 0.90) JSI METAL= 0.47 (L) (INPUT = 1.00)																																																																																																																																																																																																								
MODULUS ENGINEERING LTD. 07/04/2023 LICENSED PROFESSIONAL ENGINEER P.R. DEAN PROVINCE OF ONTARIO REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL																																																																																																																																																																																																								
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSi-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com																																																																																																																																																																																																								

Scale = 1:21.2



TOTAL WEIGHT = 19 lb

SPACING = 24.0 IN. C/C

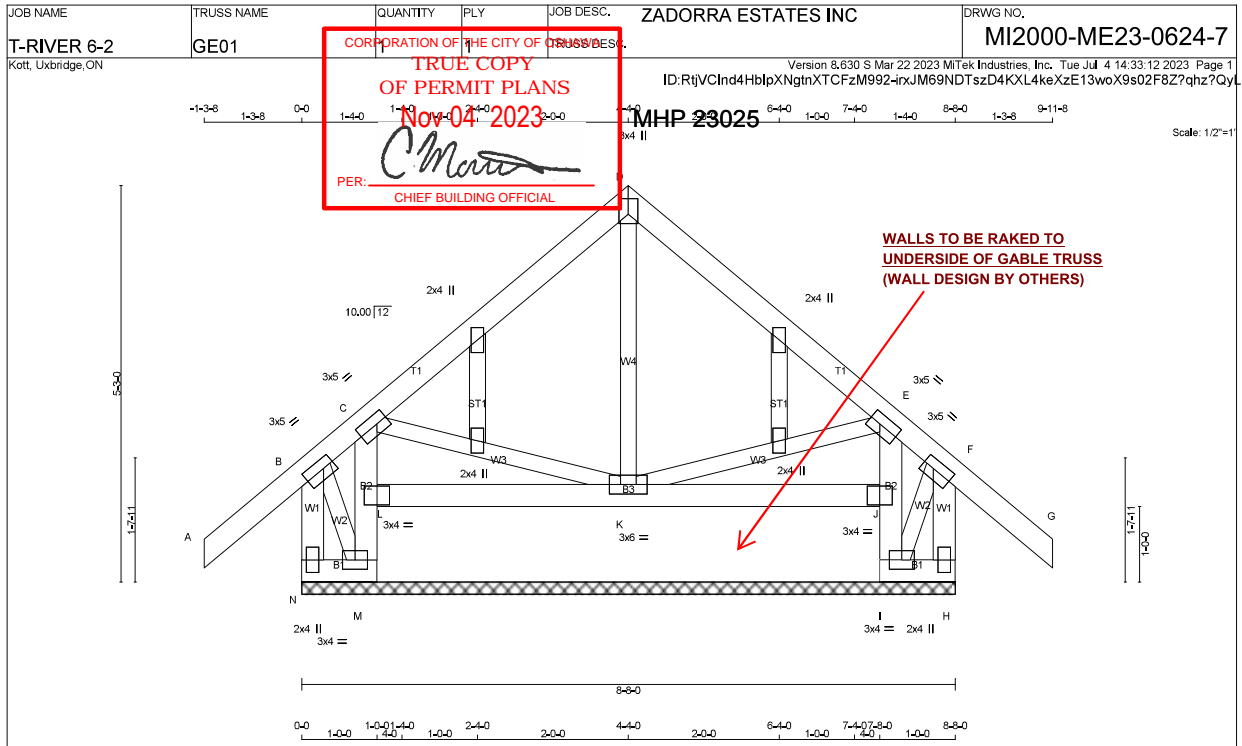
JSI METAL= 0,12 (B) (INPUT = 1,00)


NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

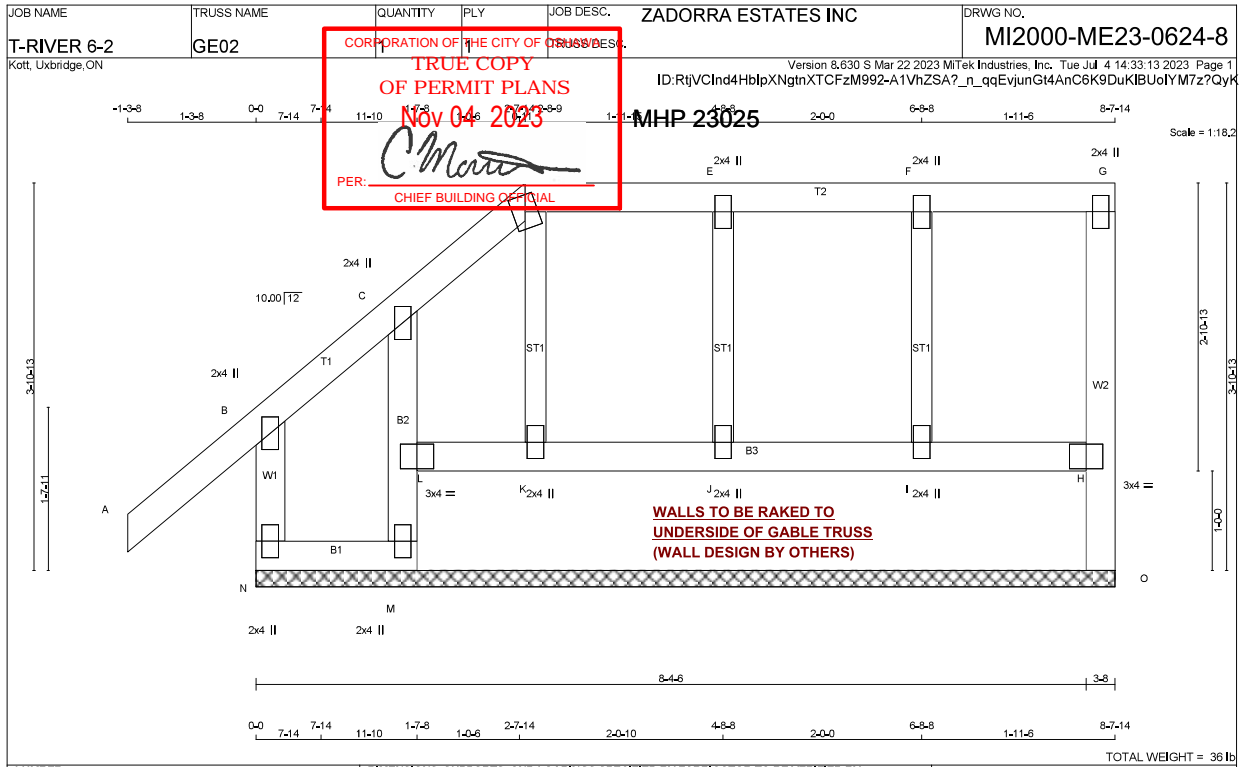
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

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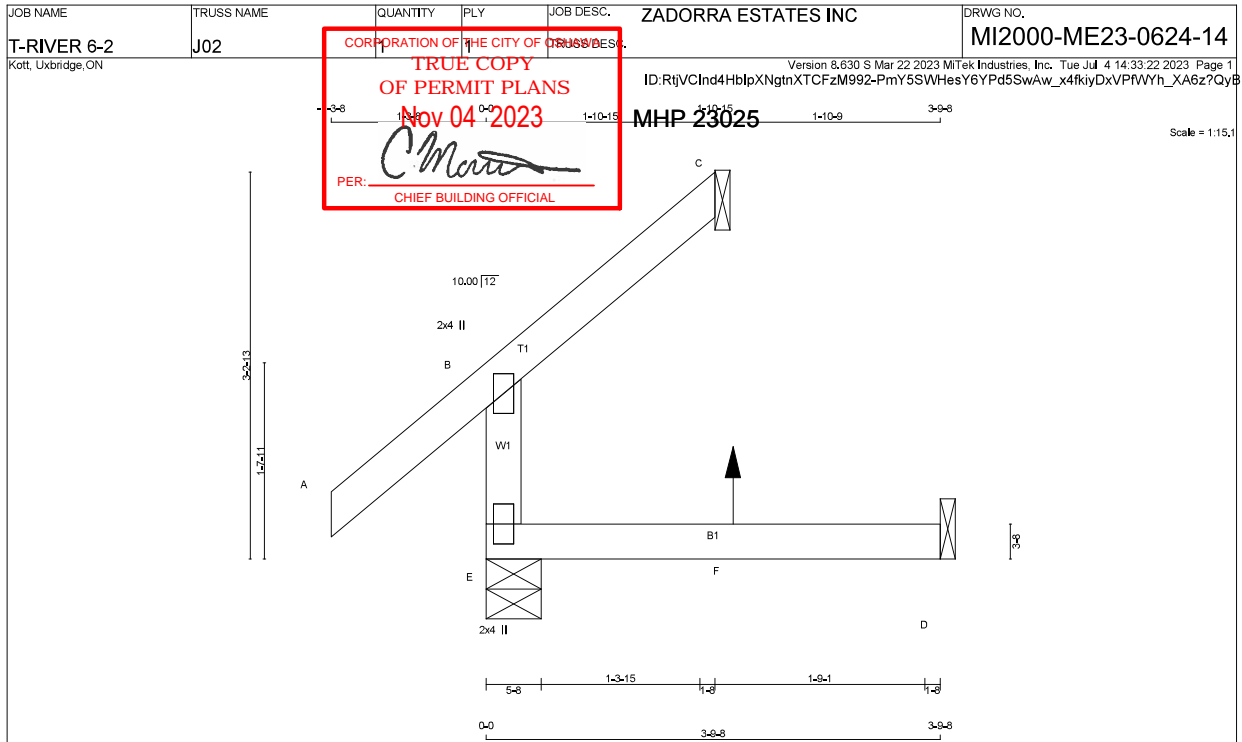


LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES										BEARINGS										SPECIFIED LOADS:									
CHORDS SIZE LUMBER										FACTORED GROSS REACTION										TOP CH. LL = 34.8 PSF									
A - D 2x4 DRY No.2										JT VERT DOWN HORIZ UPLIFT IN-SX IN-SX										DL = 6.0 PSF									
D - G 2x4 DRY No.2										N 213 0 213 0 0 8-8-0 (1-0-0) 1-8										BOT CH. LL = 0.0 PSF									
N - B 2x4 DRY No.2										I 371 0 371 0 0 8-8-0 (1-0-0) 1-8										DL = 7.3 PSF									
H - F 2x4 DRY No.2										H 213 0 213 0 0 8-8-0 (1-0-0) 1-8										TOTAL LOAD = 48.1 PSF									
N - M 2x4 DRY No.2										K 355 0 355 0 0 8-8-0 (1-0-0) 1-8																			
M - C 2x4 DRY No.2										M 371 0 371 0 0 8-8-0 (1-0-0) 1-8																			
L - J 2x4 DRY No.2																													
I - E 2x4 DRY No.2																													
I - H 2x4 DRY No.2																													
ALL WEBS 2x3 DRY No.2										VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
EXCEPT																													
ALL GABLE WEBS 2x3 DRY No.2										BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): K										THIS DESIGN COMPLIES WITH:									
DRY: SEASONED LUMBER.										UNFACTORED REACTIONS										- PART 9 OF BCBC 2018, NBC-2019AE									
GABLE STUDS SPACED AT 2'-0" OC.										1ST LCASE MAX./MIN. COMPONENT REACTIONS										- PART 9 OF OBC 2012 (2019 AMENDMENT)									
										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										- CSA 086-14									
										N 147 120 / 0 0 / 0 0 / 0 27 / 0 0 / 0										- TPIC 2014									
										I 257 199 / 0 0 / 0 0 / 0 58 / 0 0 / 0										DESIGN ASSUMPTIONS									
										H 147 120 / 0 0 / 0 0 / 0 27 / 0 0 / 0										- OVERHANG NOT TO BE ALTERED OR CUT OFF.									
										K 252 159 / 0 0 / 0 0 / 0 93 / 0 0 / 0										(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD									
										M 257 199 / 0 0 / 0 0 / 0 58 / 0 0 / 0										CSI: TC=0.18/1.00 (E-F:1), BC=0.07/1.00 (K-L:4), WB=0.07/1.00 (D-K:1), SSI=0.15/1.00 (C-D:1)									
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I, H, K, M										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10									
										BRACING										COMPANION LIVE LOAD FACTOR = 1.00									
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
										MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.										NAIL VALUES									
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
										LOADING										MAX MIN MAX MIN MAX MIN									
										TOTAL LOAD CASES: (4)										MT20 650 371 1747 788 1987 1873									
										CHORDS										PLATE PLACEMENT TOL. = 0.250 inches									
										MEMB. FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. CSI (LC) MAX. UNBRAC LENGTH										PLATE ROTATION TOL. = 5.0 Deg.									
										FR-TO FROM TO										JSI GRIP= 0.36 (D) (INPUT = 0.90)									
										A-B 0 / 53 -119.4 -119.4 0.16 (1) 10.00 K-D -287 / 0 0.07 (1)										JSI METAL= 0.07 (C) (INPUT = 1.00)									
										B-C 0 / 8 -119.4 -119.4 0.18 (1) 10.00 K-E 0 / 21 0.00 (1)																			
										C-D -55 / 0 -119.4 -119.4 0.18 (1) 6.25 C-K 0 / 21 0.00 (1)																			
										D-E -55 / 0 -119.4 -119.4 0.18 (1) 6.25 B-M 0 / 10 0.00 (1)																			
										E-F 0 / 8 -119.4 -119.4 0.18 (1) 10.00 I-F 0 / 10 0.00 (1)																			
										F-G 0 / 53 -119.4 -119.4 0.16 (1) 10.00																			
										N-B -206 / 0 0.0 0.0 0.02 (1) 7.81																			
										H-F -206 / 0 0.0 0.0 0.02 (1) 7.81																			
										N-M 0 / 0 -18.2 -18.2 0.00 (4) 10.00																			
										M-L -372 / 0 0.0 0.0 0.01 (1) 7.81																			
										L-C -348 / 0 0.0 0.0 0.01 (1) 7.81																			
										L-K 0 / 11 -18.2 -18.2 0.07 (4) 10.00																			
										K-J 0 / 11 -18.2 -18.2 0.07 (4) 10.00																			
										I-J -372 / 0 0.0 0.0 0.01 (1) 7.81																			
										J-E -348 / 0 0.0 0.0 0.01 (1) 7.81																			
										I-H 0 / 0 -18.2 -18.2 0.00 (4) 10.00																			
MODULUS ENGINEERING LTD.																													
																													
REVIEW FOR TRUSS COMPONENT ONLY																													
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL																													

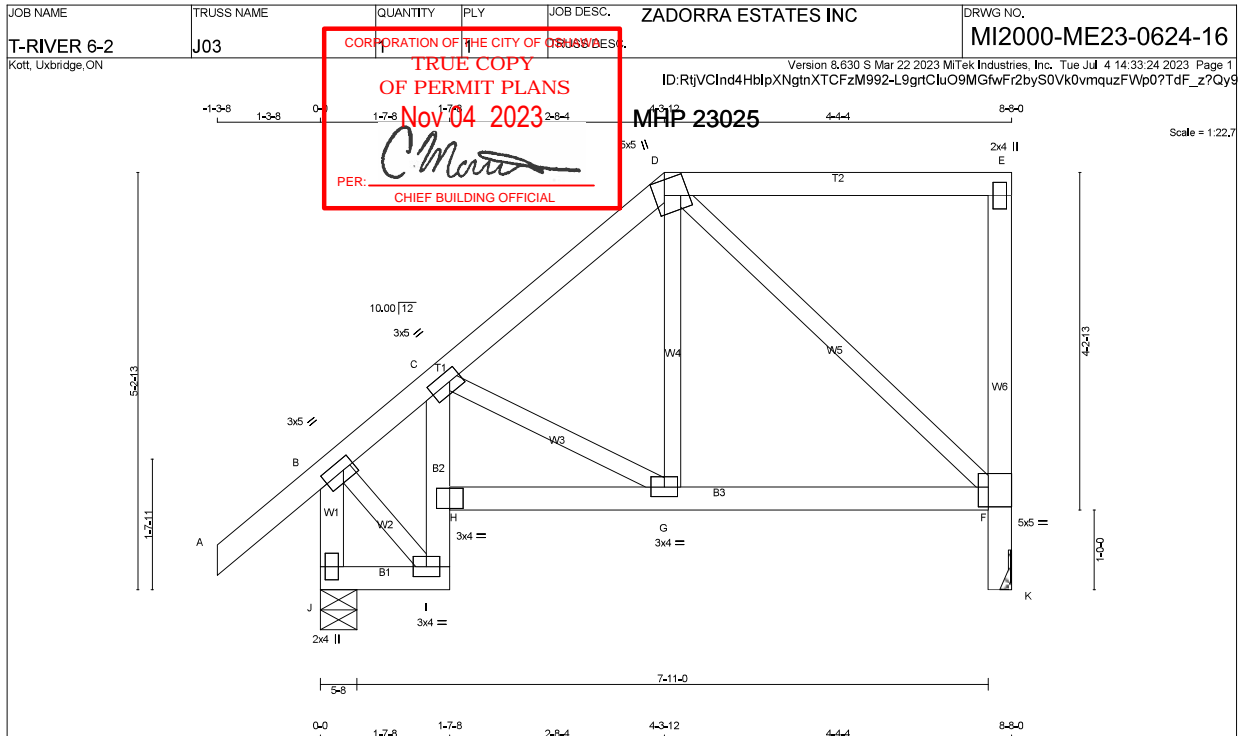


LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER DESCR. N - B 2x4 DRY No.2 SPF A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF O - G 2x4 DRY No.2 SPF N - M 2x4 DRY No.2 SPF M - C 2x4 DRY No.2 SPF L - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2'-0" OC.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): I, J, K BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4)	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIC LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 - NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.16/1.00 (A-B 1), BC=0.04/1.00 (C-L 1), WB=0.04/1.00 (F-1), SSF=0.10/1.00 (F-G 1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.22 (D) (INPUT = 0.90) JSI METAL= 0.16 (B) (INPUT = 1.00)
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B, C, G B TMV+p MT20 2.0 4.0 D TTW+m MT20 3.0 4.0 E TMW+w MT20 2.0 4.0 F TMW+w MT20 2.0 4.0 H BVMH MT20 3.0 4.0 I, J, K I BMV1+w MT20 2.0 4.0 L BVMH MT20 3.0 4.0 M BMV1+p MT20 2.0 4.0 N BMV1+p MT20 2.0 4.0	CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) MAX. UNBRACED LENGTH (LC) FROM TO FR-TO N-B -308 / 0 0.0 0.0 0.04 (1) 7.81 I-F -254 / 0 0.04 (1) A-B 0 / 53 -119.4 -119.4 0.16 (1) 10.00 J-E -243 / 0 0.04 (1) B-C -28 / 0 -119.4 -119.4 0.12 (1) 6.25 K-D -194 / 0 0.03 (1) C-D -7 / 0 -119.4 -119.4 0.04 (1) 10.00 D-E -2 / 0 -119.4 -119.4 0.06 (1) 10.00 E-F -2 / 0 -119.4 -119.4 0.06 (1) 10.00 F-G -2 / 0 -119.4 -119.4 0.06 (1) 10.00 O-H -120 / 0 0.0 0.0 0.02 (1) 7.81 H-G -99 / 0 0.0 0.0 0.02 (1) 7.81 N-M -13 / 0 -18.2 -18.2 0.02 (1) 6.25 M-L -128 / 0 0.0 0.0 0.01 (1) 7.81 L-C -99 / 0 0.0 0.0 0.04 (1) 7.81 L-K 0 / 10 -18.2 -18.2 0.03 (1) 10.00 K-J 0 / 2 -18.2 -18.2 0.01 (4) 10.00 J-I 0 / 2 -18.2 -18.2 0.02 (4) 10.00 I-H 0 / 2 -18.2 -18.2 0.02 (1) 10.00	

JOB NAME	T-RIVER 6-2 Kott, Uxbridge, ON																																				
TRUSS NAME	J01C																																				
QUANTITY	Ply																																				
JOB DESC.	ZADORRA ESTATES INC																																				
DRWG NO.	Ml2000-ME23-0624-12																																				
CORPORATION OF THE CITY OF TRUSSES DES.																																					
TRUE COPY OF PERMIT PLANS																																					
Date Nov 04 2023																																					
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PER:	CHIEF BUILDING OFFICIAL																																				
Diagram showing truss components A through E, dimensions (e.g., 6'-0", 12'-0"), and scale information (Scale = 1:14.7).																																					
LUMBER	N, L, G, A, RULES CHORDS SIZE E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 LUMBER DESCR. SPF SPF SPF DRY: SEASONED LUMBER.																																				
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX BRG IN-SX E 474 0 474 0 0 5-8 1-8 C 175 0 175 0 0 1-8 1-8 D 16 0 16 0 0 1-8 1-8 SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS 1ST LOASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 326 265 / 0 0 / 0 0 / 0 62 / 0 0 / 0 C 120 102 / 0 0 / 0 0 / 0 18 / 0 0 / 0 D 13 0 / 0 0 / 0 0 / 0 13 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6,25 FT. MAX. UNBRAVED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED, ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (5) <table border="1"><thead><tr><th colspan="3">CHORDS</th><th colspan="3">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>VERT. LOAD LC1 FROM TO</th><th>MAX. MEMB. FORCE (LBS)</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. MEMB. FORCE (LBS)</th></tr></thead><tbody><tr><td>E-B</td><td>-454 / 0</td><td>0,0 0,0 0,01 (4)</td><td>7,81</td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 96</td><td>-119,4 -119,4 0,16 (1)</td><td>10,00</td><td></td><td></td></tr><tr><td>B-C</td><td>-26 / 0</td><td>-119,4 -119,4 0,31 (1)</td><td>6,25</td><td></td><td></td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18,2 -18,2 0,02 (4)</td><td>10,00</td><td></td><td></td></tr></tbody></table> <u>CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN</u> <u>PATTERN LOADING CHECK APPLIED TO THIS TRUSS.</u>	CHORDS			WEBS			MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM TO	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. MEMB. FORCE (LBS)	E-B	-454 / 0	0,0 0,0 0,01 (4)	7,81			A-B	0 / 96	-119,4 -119,4 0,16 (1)	10,00			B-C	-26 / 0	-119,4 -119,4 0,31 (1)	6,25			E-D	0 / 0	-18,2 -18,2 0,02 (4)	10,00		
CHORDS			WEBS																																		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM TO	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. MEMB. FORCE (LBS)																																
E-B	-454 / 0	0,0 0,0 0,01 (4)	7,81																																		
A-B	0 / 96	-119,4 -119,4 0,16 (1)	10,00																																		
B-C	-26 / 0	-119,4 -119,4 0,31 (1)	6,25																																		
E-D	0 / 0	-18,2 -18,2 0,02 (4)	10,00																																		
DESIGN CRITERIA	SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN.C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2018 .NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (5% OF 48,1 P.S.F. G.S.I. PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0,19") CALCULATED VERT.DEFL(LL) = L/ 999 (0,00") ALLOWABLE DEFL.(TL)= L/360 (0,19") CALCULATED VERT.DEFL(TL) = L/ 999 (0,00") CSI TC=0,31/1,00 (B-C:1). BC=0,02/1,00 (D-E:4). WB=0,00/1,00 (n/a:0). SSI=0,20/1,00 (B-C:1) DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,10 COMP=1,10 SHEAR=1,10 TENS= 1,10 COMPANION LIVE LOAD FACTOR = 1,00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLU) (PLU) MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0,250 inches PLATE ROTATION TOL. = 5,0 Deg. JSI GRIP= 0,25 (B) (INPUT = 0,90) JSI METAL= 0,19 (B) (INPUT = 1,00)																																				
MODULUS ENGINEERING LTD. 07/04/2023 [Professional Engineer Seal] REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL																																					
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Designing bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSi-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com																																					
KOTT	[Logo]																																				



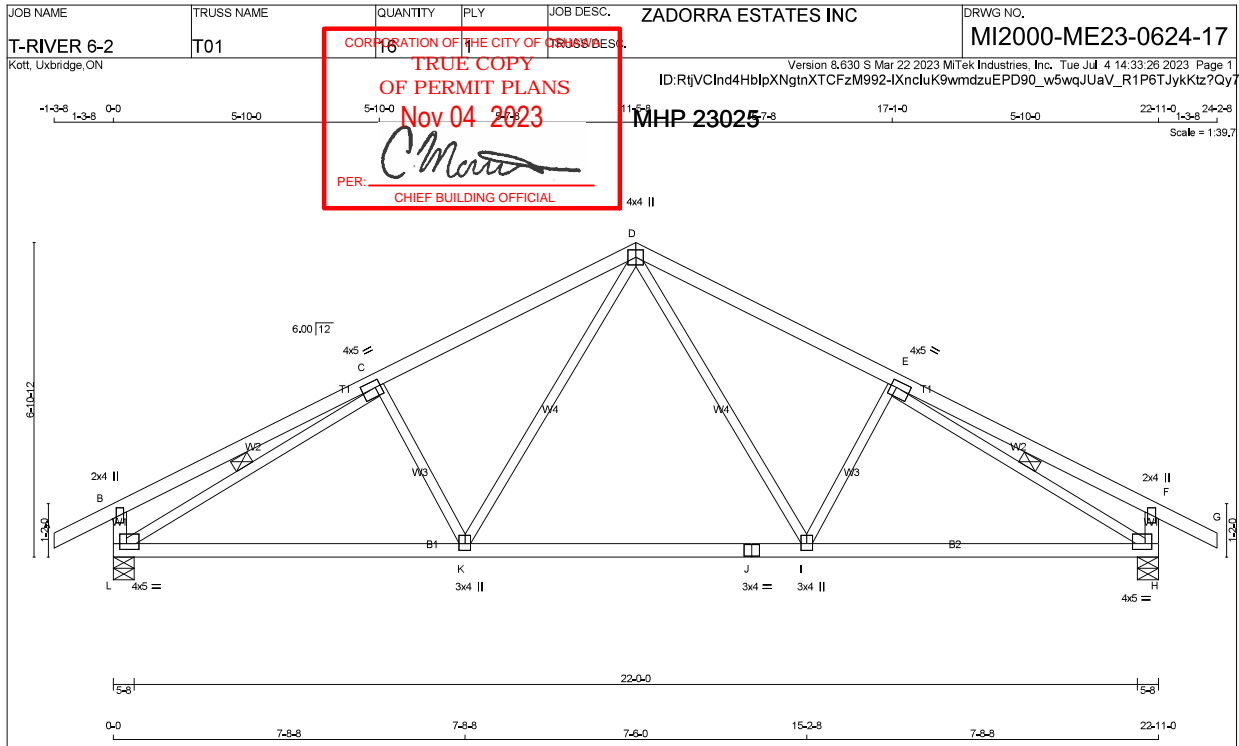
TOTAL WEIGHT = 11 T									
[M]									
LUMBER									
N. L. G. A. RULES									
CHORDS SIZE LUMBER									
DESCR. SPF									
E - B 2x4 DRY No.2									
A - C 2x4 DRY No.2									
E - D 2x4 DRY No.2									
SPF									
SPF									
SPF									
DRY: SEASONED LUMBER.									
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED MAXIMUM FACTORED INPUT REQD									
GROSS REACTION GROSS REACTION BRG BRG									
DOWN HORZ UPLIFT IN-SX IN-SX									
E 364 0 364 0 5-8 1-8									
C 67 0 67 0 -31 1-8 1-8									
D 24 0 35 0 0 1-8 1-8									
SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D									
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT									
UNFACTORED REACTIONS									
1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL									
E 252 194 / 0 0 / 0 0 / 0 59 / 0 0 / 0									
C 47 36 / -26 0 / 0 0 / 0 0 / 0 11 / 0 0 / 0									
D 20 0 / -7 0 / 0 0 / 0 0 / 0 25 / 0 0 / 0									
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (9)									
CHORDS									
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. CSI (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. CSI (LC)									
E-B -327 / 0 0.0 0.0 0.05 (5) 7.81									
A-B 0 / 53 -119.4 -119.4 0.17 (5) 10.00									
B-C -30 / 0 -119.4 -119.4 0.12 (6) 6.25									
E-F 0 / 0 -18.2 -18.2 0.06 (4) 10.00									
F-D 0 / 0 -18.2 -18.2 0.06 (4) 10.00									
FACTORED CONCENTRATED LOADS (LBS)									
JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN.									
F 2x12 8 1 15 BACK VERT TOTAL C1									
CONNECTION REQUIREMENTS									
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.									
CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN									
PATTERN LOADING CHECK APPLIED TO THIS TRUSS.									
DESIGN CRITERIA									
SPECIFIED LOADS:									
TOP CH. LL = 34.8 PSF									
DL = 6.0 PSF									
BOT CH. LL = 0.0 PSF									
DL = 7.3 PSF									
TOTAL LOAD = 48.1 PSF									
SPACING = 24.0 IN. G.C.									
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF BCBC 2018, NBC-2019AE									
- PART 9 OF OBC 2012 (2019 AMENDMENT)									
- CSA 086-14									
- TPIC 2014									
DESIGN ASSUMPTIONS									
- OVERHANG NOT TO BE ALTERED OR CUT OFF.									
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEFL.(LL)= L/360 (0.19")									
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")									
ALLOWABLE DEFL.(TL)= L/360 (0.19")									
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")									
CSI TC=0.17/1.00 (A-B 5) BC=0.06/1.00 (D-E 4) ,									
WB=0.00/1.00 (n/a 0) , SSI=0.10/1.00 (A-B 5)									
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
COMP=1.10 SHEAR=1.10 TENS=1.10									
COMPANION LIVE LOAD FACTOR = 1.00									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
NAIL VALUES									
PLATE GRIP(DRY) SHEAR SECTION									
(PSI) (PLI) (PLI)									
MAX MIN MAX MIN MAX MIN									
MT20 650 371 1747 788 1987 1873									
PLATE PLACEMENT TOL. = 0.250 inches									
PLATE ROTATION TOL. = 5.0 Deg.									
JSI GRIP= 0.22 (B) (INPUT = 0.90)									
JSI METAL= 0.17 (B) (INPUT = 1.00)									
MODULUS ENGINEERING LTD.									
07/04/2023									
P. R. WEAVER									
PROVINCE OF ONTARIO									
REVIEW FOR TRUSS COMPONENT ONLY									
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL									



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER A - D 2x4 DRY No.2 D - E 2x4 DRY No.2 K - E 2x4 DRY No.2 J - B 2x4 DRY No.2 J - I 2x4 DRY No.2 I - C 2x4 DRY No.2 H - F 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY, SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD JT GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX K 583 0 583 0 0 MECHANICAL J 775 0 775 0 0 5-8 1-8 A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, MINIMUM BEARING LENGTH AT JOINT K = 1-12.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G.C. LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.29") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL)= L/960 (0.29") CALCULATED VERT. DEFL.(TL) = L/999 (0.03") CSI: TC=0.38/1.00 (D+E-1) , BC=0.35/1.00 (C+H-1) , WB=0.23/1.00 (D-F-1) , SSI=0.20/1.00 (D-E-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.74 (B) (INPUT = 0.90) JSI METAL= 0.16 (J) (INPUT = 1.00)
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW4 MT20 3.0 5.0 1.50 1.75 C TMVW4 MT20 3.0 5.0 1.50 1.75 D TMVW+m MT20 5.0 5.0 2.25 1.50 E TMV+p MT20 2.0 4.0 F BMVW4 MT20 5.0 5.0 3.00 1.50 G BMVW4 MT20 3.0 4.0 H BMVH MT20 3.0 4.0 I BMVW4 MT20 3.0 4.0 J BMV1+p MT20 2.0 4.0		UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PER LIVE WIND DEAD SOIL K 408 294 / 0 0 / 0 0 / 0 114 / 0 0 / 0 J 539 406 / 0 0 / 0 0 / 0 133 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J		BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. FACTORED VERT. LOAD (LC) MAX. FACTORED VERT. LOAD (LC) UNBRAC LENGTH FR-TO FR-TO A-B 0 / 53 -119.4 -119.4 0.16 (1) 10.00 C-G -119 / 0 0.03 (1) B-C -378 / 0 -119.4 -119.4 0.16 (1) 6.25 G-D 0 / 135 0.04 (4) C-D -420 / 0 -119.4 -119.4 0.16 (1) 6.25 D-F -434 / 0 0.23 (1) D-E 0 / 0 -119.4 -119.4 0.38 (1) 10.00 B-I 0 / 343 0.08 (1) E-F -583 / 0 0.0 0.0 0.08 (1) 7.81 F-G -260 / 0 0.0 0.0 0.08 (1) 7.81 J-B -762 / 0 0.0 0.0 0.08 (1) 7.81 J-I 0 / 0 -18.2 -18.2 0.01 (4) 10.00 I-H -213 / 0 0.0 0.0 0.35 (1) 7.81 H-C -195 / 0 0.0 0.0 0.35 (1) 7.81 H-G 0 / 425 -18.2 -18.2 0.10 (1) 10.00 G-F 0 / 323 -18.2 -18.2 0.09 (4) 10.00		WEBS MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED VERT. LOAD (LC) MAX. FACTORED VERT. LOAD (LC) UNBRAC LENGTH FR-TO FR-TO A-B 0 / 53 -119.4 -119.4 0.16 (1) 10.00 C-G -119 / 0 0.03 (1) B-C -378 / 0 -119.4 -119.4 0.16 (1) 6.25 G-D 0 / 135 0.04 (4) C-D -420 / 0 -119.4 -119.4 0.16 (1) 6.25 D-F -434 / 0 0.23 (1) D-E 0 / 0 -119.4 -119.4 0.38 (1) 10.00 B-I 0 / 343 0.08 (1) E-F -583 / 0 0.0 0.0 0.08 (1) 7.81 F-G -260 / 0 0.0 0.0 0.08 (1) 7.81 J-B -762 / 0 0.0 0.0 0.08 (1) 7.81 J-I 0 / 0 -18.2 -18.2 0.01 (4) 10.00 I-H -213 / 0 0.0 0.0 0.35 (1) 7.81 H-C -195 / 0 0.0 0.0 0.35 (1) 7.81 H-G 0 / 425 -18.2 -18.2 0.10 (1) 10.00 G-F 0 / 323 -18.2 -18.2 0.09 (4) 10.00		MODULUS ENGINEERING LTD. 07/04/2023 P. R. JEFF PROFESSIONAL ENGINEER PROVINCE OF ONTARIO REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N, L, G, A, RULES										BEARINGS										SPECIFIED LOADS:									
CHORDS SIZE LUMBER DESCR.										FACTORED MAXIMUM FACTORED INPUT REQD										TOP CH. LL = 34.8 PSF									
A - D 2x4 DRY No.2 SPF										GROSS REACTION GROSS REACTION BRG BRG										DL = 6.0 PSF									
D - G 2x4 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF									
L - B 2x4 DRY No.2 SPF										L 1740 0 1740 0 0 5-8 1-14										DL = 7.3 PSF									
H - F 2x4 DRY No.2 SPF										H 1740 0 1740 0 0 5-8 1-14										TOTAL LOAD = 48.1 PSF									
L - J 2x4 DRY No.2 SPF																													
J - H 2x4 DRY No.2 SPF																													
ALL WEBS 2x3 DRY No.2 SPF										UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C									
EXCEPT										1ST LCASE MAX./MIN. COMPONENT REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015									
DRY: SEASONED LUMBER.										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										THIS DESIGN COMPLIES WITH:									
										L 1213 892 / 0 0 / 0 0 / 0 321 / 0 0 / 0										- PART 9 OF BCBC 2018, NBC-2019AE									
										H 1213 892 / 0 0 / 0 0 / 0 321 / 0 0 / 0										- PART 9 OF OBC 2012 (2019 AMENDMENT)									
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H										- CSA 086-14									
										BRACING										- TPIC 2014									
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.										(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD									
										MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.										ALLOWABLE DEFL.(LL)= L/360 (0.76")									
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.										CALCULATED VERT. DEFL.(LL) = L/999 (0.09")									
										1 LATERAL BRACE(S) AT 1/2 LENGTH OF C ₄ , E ₄ .										ALLOWABLE DEFL.(TL)= L/360 (0.76")									
										END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX, UNBRACED LENGTH COLUMN OF THE TABLE BELOW										CALCULATED VERT. DEFL.(TL) = L/999 (0.17")									
										LOADING										CSI TC=0.63/1.00 (B-C:1), BC=0.44/1.00 (H-I:1), WB=0.72/1.00 (C-L:1), SS=0.30/1.00 (B-C:1)									
										TOTAL LOAD CASES: (4)										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10									
										CHORDS										COMPANION LIVE LOAD FACTOR = 1.00									
										MEMB. MAX. FACTORED FORCE (LBS)										WEBS									
										FACTORED VERT. LOAD LC1 MAX. MAX. UNBRAC LENGTH FR-TO										MEMB. MAX. FACTORED FORCE (LBS)									
										FR-TO FROM TO										MAX. CSI (LC)									
										A-B 0 / 36 -119.4 -119.4 0.16 (1) 10.00 D-I 0 / 679 0.15 (1)																			
										B-C 0 / 36 -119.4 -119.4 0.63 (1) 10.00 I-E -497 / 0 0.15 (1)																			
										C-D -1936 / 0 -119.4 -119.4 0.56 (1) 4.20 K-D 0 / 679 0.15 (1)																			
										D-E -1936 / 0 -119.4 -119.4 0.56 (1) 4.20 C-K -497 / 0 0.15 (1)																			
										E-F 0 / 36 -119.4 -119.4 0.63 (1) 10.00 L-C -2308 / 0 0.72 (1)																			
										F-G 0 / 36 -119.4 -119.4 0.16 (1) 10.00 E-H -2308 / 0 0.72 (1)																			
										L-B -424 / 0 0.0 0.0 0.04 (1) 7.81																			
										H-F -424 / 0 0.0 0.0 0.04 (1) 7.81																			
										L-K 0 / 1936 -18.2 -18.2 0.44 (1) 10.00																			
										K-J 0 / 1367 -18.2 -18.2 0.34 (1) 10.00																			
										J-I 0 / 1367 -18.2 -18.2 0.34 (1) 10.00																			
										I-H 0 / 1936 -18.2 -18.2 0.44 (1) 10.00																			
																				NAIL VALUES									
																				PLATE GRIP(DRY) SHEAR SECTION									
																				(PSI) (PLI) (PLI)									
																				MAX MIN MAX MIN MAX MIN									
																				MT20 650 371 1747 788 1987 1873									
																				PLATE PLACEMENT TOL. = 0.250 inches									
																				PLATE ROTATION TOL. = 5.0 Deg.									
																				JSI GRIP= 0.89 (E) (INPUT = 0.90)									
																				JSI METAL= 0.62 (C) (INPUT = 1.00)									

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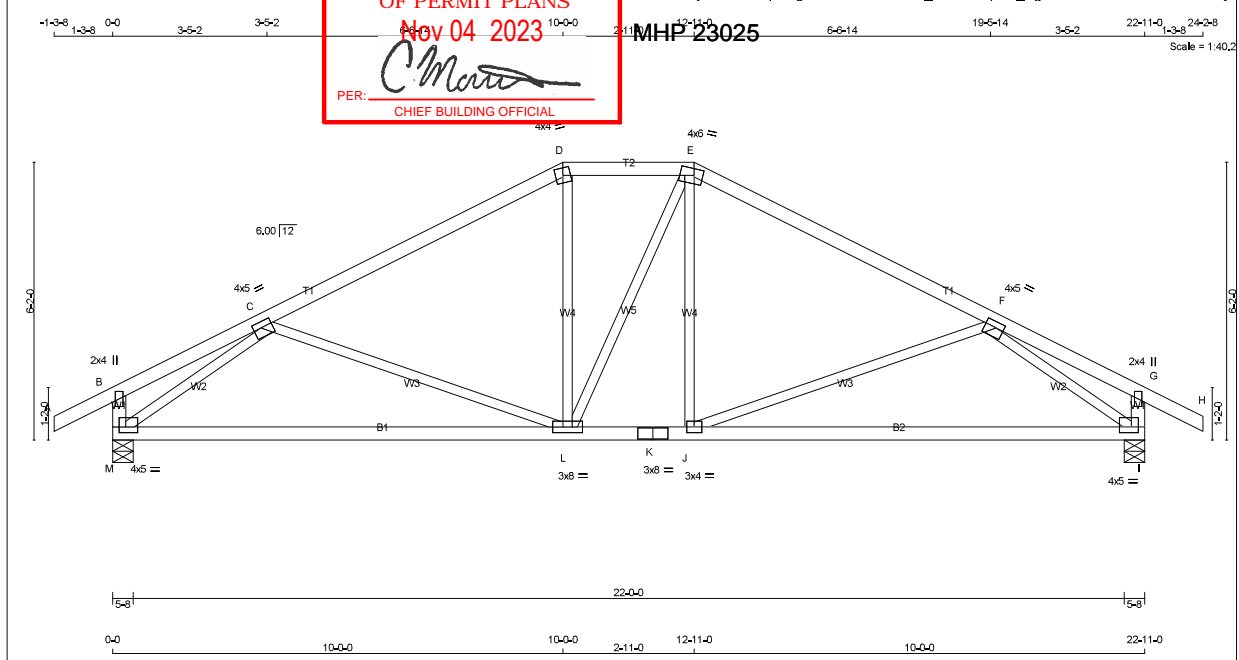
PROFESSIONAL ENGINEER

P. R. WEAH

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL



TOTAL WEIGHT = 92 lb [M]F

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW4	MT20	4.0	5.0	1.50	2.00
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMWW4	MT20	4.0	5.0	1.50	2.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW14	MT20	4.0	5.0	1.50	1.75
J	BMWW4	MT20	3.0	4.0		
K	BS4	MT20	3.0	8.0		
L	BMWWW4	MT20	3.0	8.0		
M	BMVW14	MT20	4.0	5.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	DOWN	UP	UP
M	1740	0	1740	0
I	1740	0	1740	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS	DEAD	SOIL
M	1213	892 / 0	0 / 0	0 / 0	321 / 0
I	1213	892 / 0	0 / 0	0 / 0	321 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	C-L	-390 / 0	0.42 (1)
B-C	0 / 62	-119.4	-119.4	0.64 (1)	10.00	L-D	0 / 269	0.06 (1)
C-D	-1752 / 0	-119.4	-119.4	0.70 (1)	4.10	L-E	0 / 1	0.00 (1)
D-E	-1543 / 0	-119.4	-119.4	0.16 (1)	5.16	J-F	0 / 268	0.06 (1)
E-F	-1752 / 0	-119.4	-119.4	0.70 (1)	4.10	J-F	-390 / 0	0.42 (1)
F-G	0 / 62	-119.4	-119.4	0.64 (1)	10.00	M-C	-2386 / 0	0.70 (1)
G-H	0 / 36	-119.4	-119.4	0.16 (1)	10.00	F-I	-2386 / 0	0.70 (1)
H-I	-223 / 0	0.0	0.0	0.02 (1)	7.81			
I-G	-223 / 0	0.0	0.0	0.02 (1)	7.81			
M-L	0 / 1903	-18.2	-18.2	0.52 (1)	10.00			
L-K	0 / 1543	-18.2	-18.2	0.50 (4)	10.00			
K-J	0 / 1543	-18.2	-18.2	0.50 (4)	10.00			
J-I	0 / 1903	-18.2	-18.2	0.53 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/813 (0.34")

CSI: TC=0.70/1.00 (C-D-1), BC=0.53/1.00 (I-J-1), WB=0.70/1.00 (C-M-1), SS=0.33/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PU)
MT20	MAX 650	MIN 371	MAX 1747
	MIN 550	MAX 788	MIN 1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.89 (K) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

07/04/2023

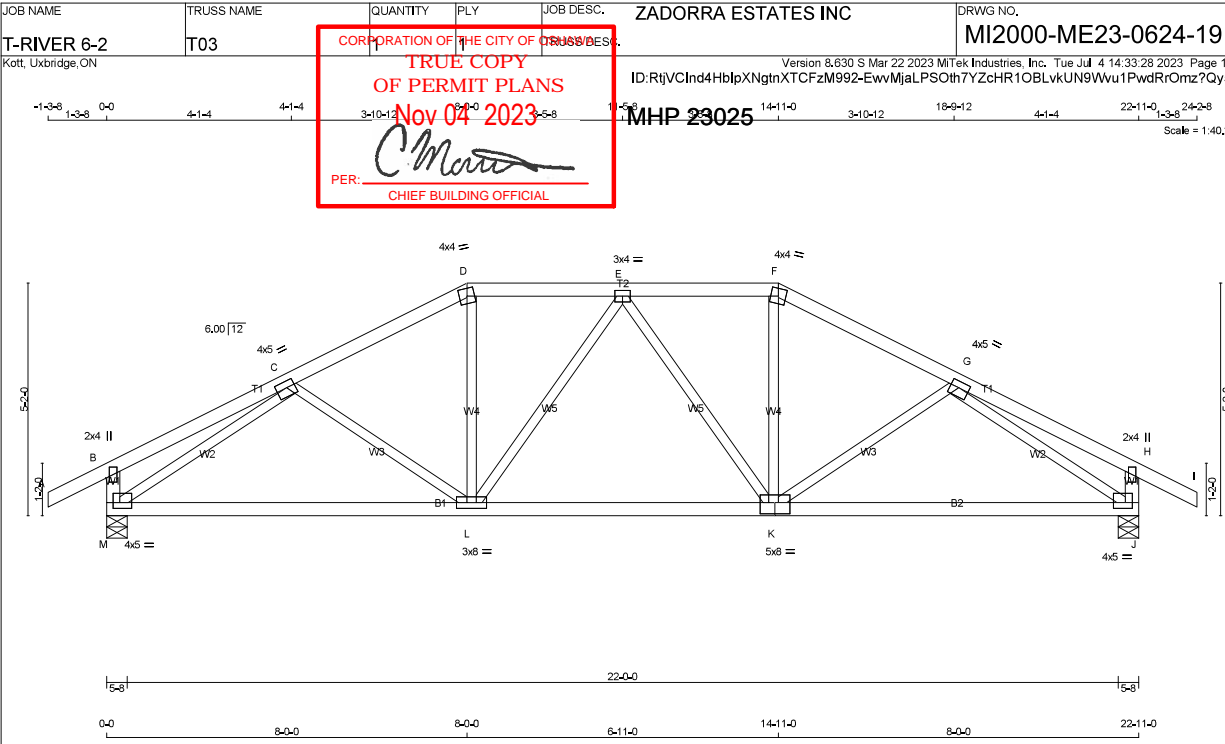
P. R. JEFF

PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL



LUMBER			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
M - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
C	TMVW4	MT20	4.0	5.0 1.75 2.25
D	TTW-m	MT20	4.0	4.0
E	TMVW4	MT20	3.0	4.0
F	TTW-m	MT20	4.0	4.0
G	TMVW4	MT20	4.0	5.0 1.75 2.25
H	TMV+p	MT20	2.0	4.0
J	BMVW4	MT20	4.0	5.0 1.50 1.75
K	BSVW4	MT20	5.0	8.0 3.00 4.00
L	BMVW4	MT20	3.0	8.0
M	BMVW4	MT20	4.0	5.0 1.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG					
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
M	1740	0	1740	0	0	5-8	1-14		
J	1740	0	1740	0	0	5-8	1-14		

UNFACTORED REACTIONS							
JT	1ST CASE	MAX. MIN.	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1213	892 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
J	1213	892 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 36	-119.4 -119.4	0.16 (1)	C-L	-189 / 19	0.08 (1)	
B-C	0 / 23	-119.4 -119.4	0.28 (1)	L-D	0 / 506	0.11 (1)	
C-D	-1933 / 0	-119.4 -119.4	0.28 (1)	L-E	-281 / 0	0.17 (1)	
D-E	-1718 / 0	-119.4 -119.4	0.20 (1)	E-K	-323 / 0	0.20 (1)	
E-F	-1693 / 0	-119.4 -119.4	0.20 (1)	K-F	0 / 495	0.11 (1)	
F-G	-1909 / 0	-119.4 -119.4	0.26 (1)	K-G	-193 / 14	0.08 (1)	
G-H	0 / 23	-119.4 -119.4	0.28 (1)	M-C	-2288 / 0	0.90 (1)	
H-I	0 / 36	-119.4 -119.4	0.16 (1)	G-J	-2264 / 0	0.89 (1)	
M-B	-351 / 0	0.0	0.0 0.04 (1)				
J-H	-351 / 0	0.0	0.0 0.04 (1)				
M-L	0 / 1862	-18.2 -18.2	0.45 (1)				
L-K	0 / 1877	-18.2 -18.2	0.46 (1)				
K-J	0 / 1843	-18.2 -18.2	0.50 (1)				

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 34.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.3 PSF
TOTAL LOAD	= 48.1 PSF

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 - NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")

CSI: TC=0.28/1.00 (G-H 1), BC=0.50/1.00 (J-K 1), WB=0.90/1.00 (C-M 1), SS=0.20/1.00 (G-H 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.59 (C) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

07/04/2023

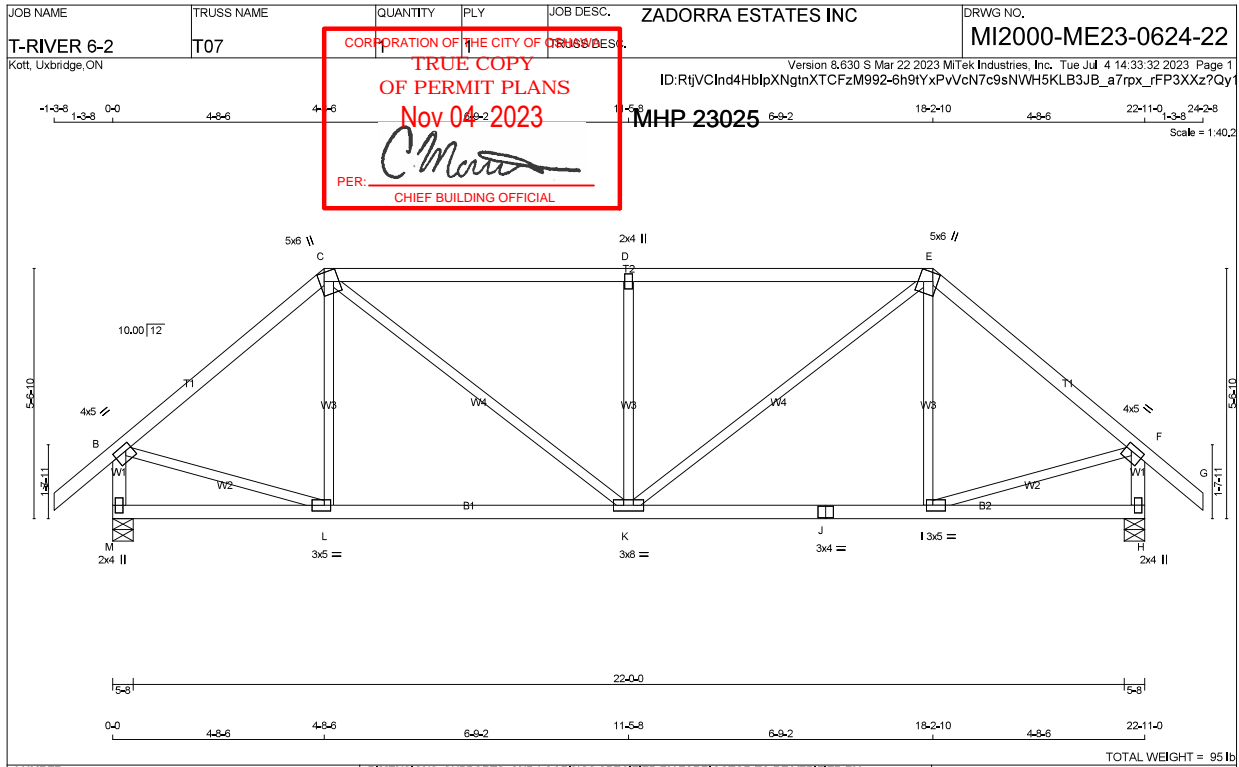
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LUMBER
N, L, G, A, RULES
CHORDS SIZE LUMBER DESCR.

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	5.0	1.50	1.75
C	TTVW+m	MT20	5.0	6.0	2.25	1.25
D	TMVW4	MT20	2.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.25	1.25
F	TMVW4	MT20	4.0	5.0	1.50	1.75
H	BMV1+p	MT20	2.0	4.0		
I	BMVW4	MT20	3.0	5.0	1.50	1.75
J	BS4	MT20	3.0	4.0		
K	BMVW4	MT20	3.0	5.0	1.50	1.75
L	BMVW4	MT20	3.0	5.0	1.50	1.75
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
M	1743	0	1743	0
H	1743	0	1743	0

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1216	894 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
H	1216	894 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00	L-C	-192 / 43
B-C	-1488 / 0	-119.4	-119.4	0.54 (1)	4.69	C-K	0 / 907
C-D	-1853 / 0	-119.4	-119.4	0.68 (1)	4.83	K-D	-990 / 0
D-E	-1853 / 0	-119.4	-119.4	0.68 (1)	4.83	K-E	0 / 907
E-F	-1488 / 0	-119.4	-119.4	0.54 (1)	4.69	I-E	-192 / 43
F-G	0 / 53	-119.4	-119.4	0.16 (1)	10.00	B-L	0 / 1182
M-B	-1712 / 0	0.0	0.0	0.18 (1)	6.36	I-F	0 / 1182
H-F	-1712 / 0	0.0	0.0	0.18 (1)	6.36		
M-L	0 / 0	-18.2	-18.2	0.14 (4)	10.00		
L-K	0 / 1137	-18.2	-18.2	0.26 (1)	10.00		
K-J	0 / 1137	-18.2	-18.2	0.26 (1)	10.00		
J-I	0 / 1137	-18.2	-18.2	0.26 (1)	10.00		
I-H	0 / 0	-18.2	-18.2	0.14 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.68/1.00 (D-E-1), BC=0.26/1.00 (K-L-1), WB=0.46/1.00 (D-K-1), SS=0.39/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.51 (F) (INPUT = 1.00)

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ID:RtjVCInd4HblpXNgtnXTCFzM992-TfymbfS2K9?QjwLIqhV2EnHX?GoWAHk?X6pBkz?Qxy



TOTAL WEIGHT = 43 lb

JSI GRIP= 0.64 (L) (INPUT = 0.90)
JSI METAL= 0.16 (M) (INPUT = 1.00)

MEMB.	CHORDS			WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CS1 (%)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 53	-119.4 -119.4	0.16 (1)	10.00	J-D	0 / 167	0.04 (1)
B-C	-307 / 0	-119.4 -119.4	0.16 (1)	6.25	J-E	-208 / 0	0.05 (1)
D-E	-434 / 0	-119.4 -119.4	0.20 (1)	6.25	C-J	-115 / 0	0.03 (1)
C-D	-433 / 0	-119.4 -119.4	0.20 (1)	6.25	B-L	0 / 397	0.09 (1)
E-F	-281 / 0	-119.4 -119.4	0.11 (1)	6.25	H-F	0 / 480	0.11 (1)
M-B	-767 / 0	0.0 0.0	0.08 (1)	7.81			
G-F	-576 / 0	0.0 0.0	0.06 (1)	7.81			
M-L	0 / 0	-18.2 -18.2	0.00 (4)	10.00			
L-K	-325 / 0	0.0 0.0	0.30 (1)	7.81			
K-C	-299 / 0	0.0 0.0	0.30 (1)	7.81			
K-J	0 / 431	-18.2 -18.2	0.10 (1)	10.00			
J-I	0 / 521	-18.2 -18.2	0.12 (1)	10.00			
H-I	-394 / 0	0.0 0.0	0.36 (1)	7.81			
I-E	-369 / 0	0.0 0.0	0.36 (1)	7.81			
H-G	0 / 0	-18.2 -18.2	0.00 (4)	10.00			

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com

