

LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES										BEARINGS										SPECIFIED LOADS:									
CHORDS SIZE LUMBER DESCR. SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										TOP CH. LL = 34.8 PSF									
E - B 2x4 DRY No.2										E 500 0 500 0 0 5-8 1-8										DL = 6.0 PSF									
A - C 2x4 DRY No.2										C 179 0 179 0 0 1-8 1-8										BOT CH. LL = 0.0 PSF									
E - D 2x4 DRY No.2										D 33 0 37 0 0 1-8 1-8										DL = 7.3 PSF									
DRY: SEASONED LUMBER.										SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D										TOTAL LOAD = 48.1 PSF									
PLATES (table is in inches)										UNFACTORED REACTIONS										SPACING = 240 IN.C/C									
JT TYPE PLATES W LEN Y X										1ST LCASE MAX./MIN. COMPONENT REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
B TMV+p MT20 2.0 4.0										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										THIS DESIGN COMPLIES WITH:									
E BMV+1p MT20 2.0 4.0										E 347 268 / 0 0 / 0 0 / 0 78 / 0 0 / 0										- PART 9 OF BCBC 2018, NBC-2019AE									
										C 123 105 / 0 0 / 0 0 / 0 0 / 0 18 / 0 0 / 0										- PART 9 OF OBC 2012 (2019 AMENDMENT)									
										D 26 0 / 0 0 / 0 0 / 0 0 / 0 26 / 0 0 / 0										- CSA 086-14									
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E										- TPIC 2014									
										BRACING										DESIGN ASSUMPTIONS									
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.										-OVERHANG NOT TO BE ALTERED OR CUT OFF.									
										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD									
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										ALLOWABLE DEFL.(LL)= L/360 (0.19")									
										LOADING										CALCULATED VERT. DEFL.(LL) = L/999 (0.00")									
										TOTAL LOAD CASES: (4)										ALLOWABLE DEFL.(TL)= L/360 (0.19")									
										CHORDS										CALCULATED VERT. DEFL.(TL) = L/999 (0.01")									
										MAX. FACTORED FORCE (LBS)										WEBS									
										MEMB. VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FACTORED FORCE MAX										CSI TC=0.32/1.00 (B-C:1) BC=0.07/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)									
										FR-TO FROM TO CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
										E-B -460 / 0 0.0 0.0 0.03 (4) 7.81										COMPANION LIVE LOAD FACTOR = 1.00									
										A-B 0 / 36 -119.4 -119.4 0.16 (1) 10.00										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
										B-C -27 / 0 -119.4 -119.4 0.32 (1) 6.25										NAIL VALUES									
										E-D 0 / 0 -18.2 -18.2 0.07 (4) 10.00										PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
																				MAX MIN MAX MIN MAX MIN									
																				MT20 650 371 1747 788 1987 1873									
																				PLATE PLACEMENT TOL. = 0.250 inches									
																				PLATE ROTATION TOL. = 5.0 Deg.									
																				JSI GRIP= 0.26 (B) (INPUT = 0.90)									
																				JSI METAL= 0.19 (B) (INPUT = 1.00)									

MODULUS ENGINEERING LTD.

07/04/2023

D. A. SHERMAN

100123373

LICENSED PROFESSIONAL ENGINEER

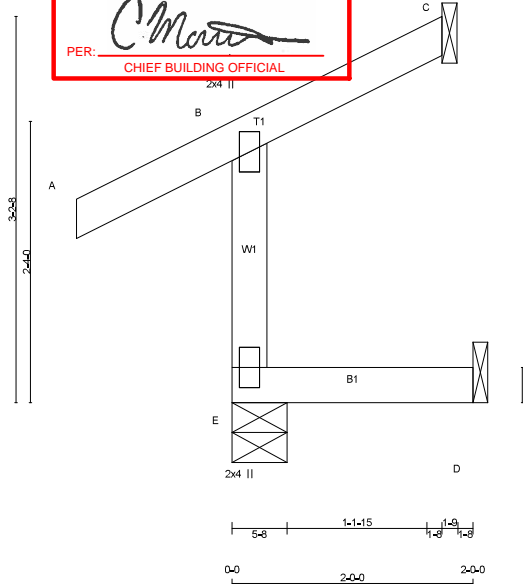
PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 04 2023
PER: *C. M...*
CHIEF BUILDING OFFICIAL

MHP 23024



TOTAL WEIGHT = 8 lb

LUMBER				DESCR.	
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	SPF	SPF
E - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	UPLIFT	BRG	IN-SX
E	346	0	346	0	0	5-8	1-8	1-8	1-8
C	50	0	50	0	-41	1-8	1-8	1-8	1-8
D	11	0	18	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
E	239	191 / 0	0 / 0	0 / 0	0 / 0
C	34	28 / -29	0 / 0	0 / 0	0 / 0
D	9	0 / -6	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT, MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	MAX. UNBRAC. LENGTH	FR-TO			
E-B	-321 / 0	0.0 0.0	0.03 (5)	7.81			
A-B	0 / 36	-119.4 -119.4	0.16 (1)	10.00			
B-C	-24 / 0	-119.4 -119.4	0.13 (1)	6.25			
E-D	0 / 0	-18.2 -18.2	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI TC=0.16/1.00 (A-B 1), BC=0.03/1.00 (D-E 5), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com





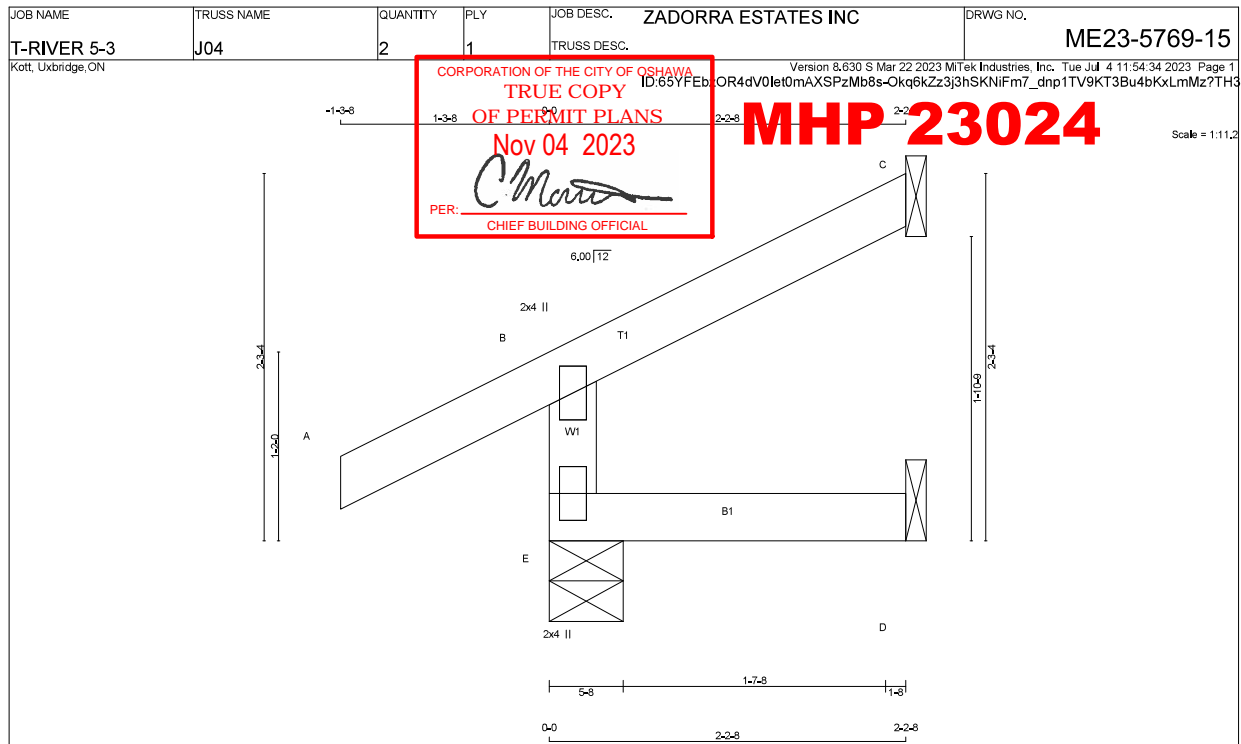
TOTAL WEIGHT = 7 lb

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from tpi.781.N.Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	349	0	349	0	5-8	1-8
C	99	0	99	0	1-8	1-8
D	18	0	20	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	241	190 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
C	68	58 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)
FR-TO						
E-B	-326 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	
B-C	-15 / 0	-119.4	-119.4	0.10 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (B-C 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

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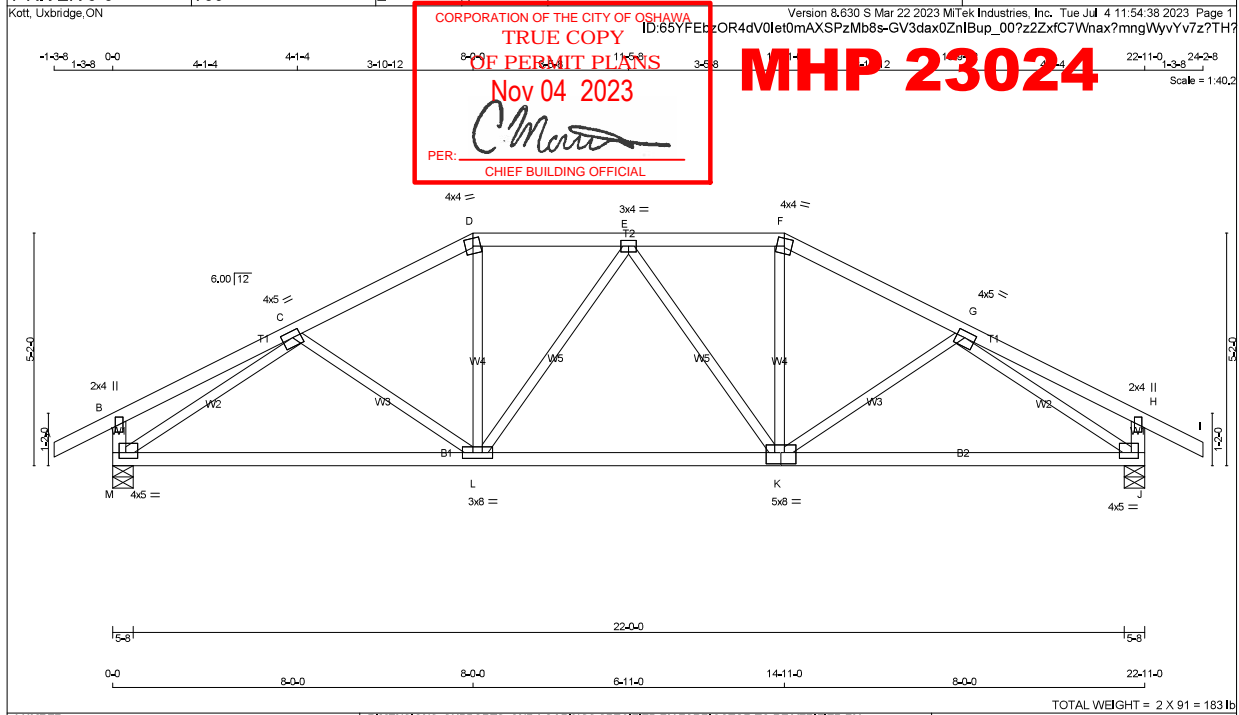


REVIEW FOR TRUSS COMPONENT ONLY

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	GROSS REACTION	DOWN	UP	BRG	TOP CH. LL = 34.8 PSF	
F - I	2x4	DRY	No.2	SPF	M	VERT	0	0	5-8	DL = 6.0 PSF	
M - B	2x4	DRY	No.2	SPF	J	HORIZ	1740	0	5-8	BOT CH. LL = 0.0 PSF	
J - H	2x4	DRY	No.2	SPF			1740	0	5-8	DL = 7.3 PSF	
M - K	2x4	DRY	No.2	SPF						TOTAL LOAD = 48.1 PSF	
K - J	2x4	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF							
EXCEPT											
DRY, SEASONED LUMBER.											

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING			
JT	TYPE	PLATES	W	LEN	Y	X	1ST CASE	MAX. MIN.	COMPONENT REACTIONS		
B	TMV+p	MT20	2.0	4.0			JT	COMBINED	SNOW	LIVE	PERM.LIVE
C	TMVW+4	MT20	4.0	5.0	1.75	2.25	M	1213	892 / 0	0 / 0	0 / 0
D	TTW+m	MT20	4.0	4.0			J	1213	892 / 0	0 / 0	0 / 0
E	TMVW+4	MT20	3.0	4.0							
F	TTW+m	MT20	4.0	4.0							
G	TMVW+4	MT20	4.0	5.0	1.75	2.25					
H	TMV+p	MT20	2.0	4.0							
J	BMVW+4	MT20	4.0	5.0	1.50	1.75					
K	BSVW+4	MT20	5.0	8.0	3.00	4.00					
L	BMVW+4	MT20	3.0	8.0							
M	BMVW+4	MT20	4.0	5.0	1.50	1.75					

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	MEMB.	MAX. FACTORED	FORCE	MAX.
FR-TO	FROM	TO	CS (LC)	FR-TO	FROM	TO	CS (LC)
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	C-L	-189 / 19
B-C	0 / 23	-119.4	-119.4	0.28 (1)	10.00	L-D	0 / 506
C-D	-1933 / 0	-119.4	-119.4	0.28 (1)	4.80	L-E	-281 / 0
D-E	-1718 / 0	-119.4	-119.4	0.20 (1)	4.90	E-K	-323 / 0
E-F	-1693 / 0	-119.4	-119.4	0.20 (1)	4.93	K-F	0 / 495
F-G	-1909 / 0	-119.4	-119.4	0.26 (1)	4.63	K-G	-193 / 14
G-H	0 / 23	-119.4	-119.4	0.28 (1)	10.00	M-C	-2288 / 0
H-I	0 / 36	-119.4	-119.4	0.16 (1)	10.00	G-J	-2264 / 0
M-B	-351 / 0	0.0	0.0	0.04 (1)	7.81		
J-H	-351 / 0	0.0	0.0	0.04 (1)	7.81		
M-L	0 / 1862	-18.2	-18.2	0.45 (1)	10.00		
L-K	0 / 1877	-18.2	-18.2	0.46 (1)	10.00		
K-J	0 / 1843	-18.2	-18.2	0.50 (1)	10.00		

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")
CSI: TC=0.28/1.00 (G-H), BC=0.50/1.00 (J-K), WB=0.90/1.00 (C-M), SS=0.20/1.00 (G-H)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

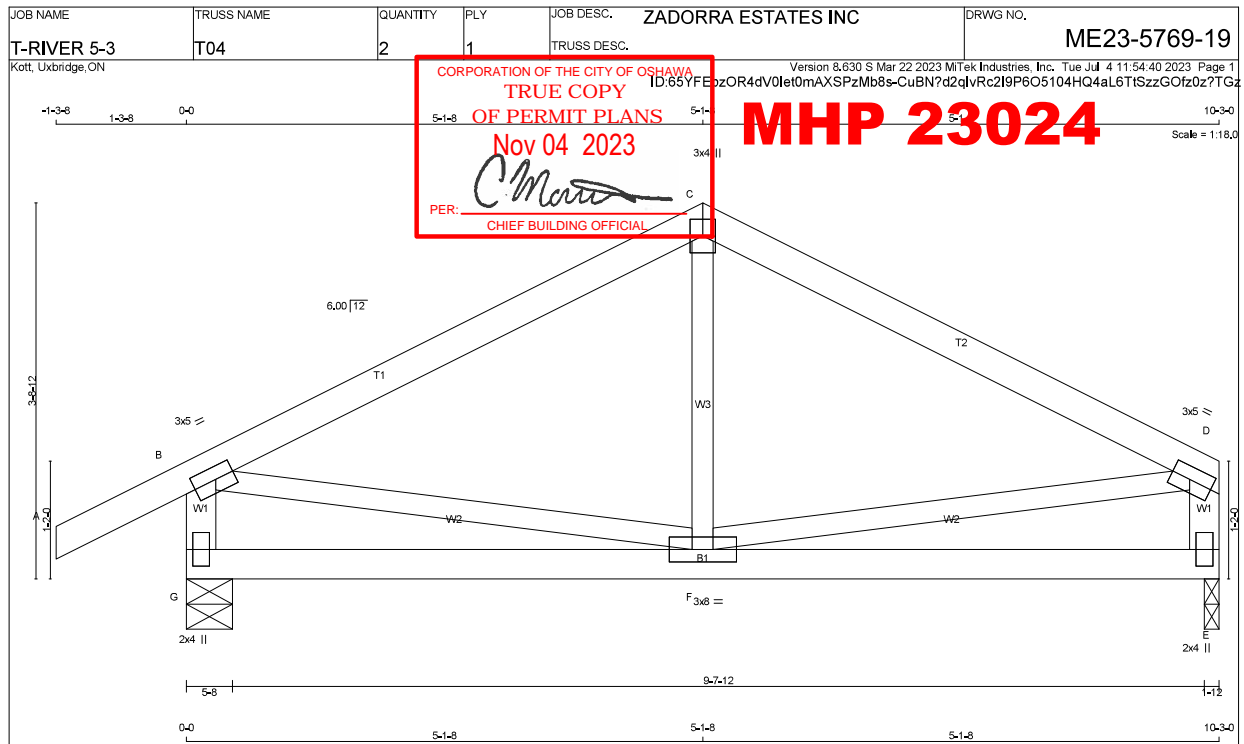
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.59 (C) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL



TOTAL WEIGHT = 2 X 39 = 77 LBS										[MIF]																			
LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER DESCR.										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
A - C 2x4 DRY No.2 SPF										FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD BRG BRG										SPECIFIED LOADS:									
C - D 2x4 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										TOP CH. LL = 34.8 PSF									
G - B 2x4 DRY No.2 SPF										G 868 0 868 0 0 5-8 1-8										DL = 6.0 PSF									
E - D 2x4 DRY No.2 SPF										E 706 0 706 0 0 1-12 1-8										BOT CH. LL = 0.0 PSF									
G - E 2x4 DRY No.2 SPF																				DL = 7.3 PSF									
ALL WEBS 2x3 DRY No.2 SPF																				TOTAL LOAD = 48.1 PSF									
EXCEPT																													
DRY: SEASONED LUMBER.																													
PLATES (table is in inches)																													
JT TYPE PLATES W LEN Y X																													
B TMVV4 MT20 3.0 5.0 1.50 2.25																													
C TTW+p MT20 3.0 4.0																													
D TMVV4 MT20 3.0 5.0 1.50 2.25																													
E BMV1+p MT20 2.0 4.0																													
F BMVWW4 MT20 3.0 6.0																													
G BMV1+p MT20 2.0 4.0																													

