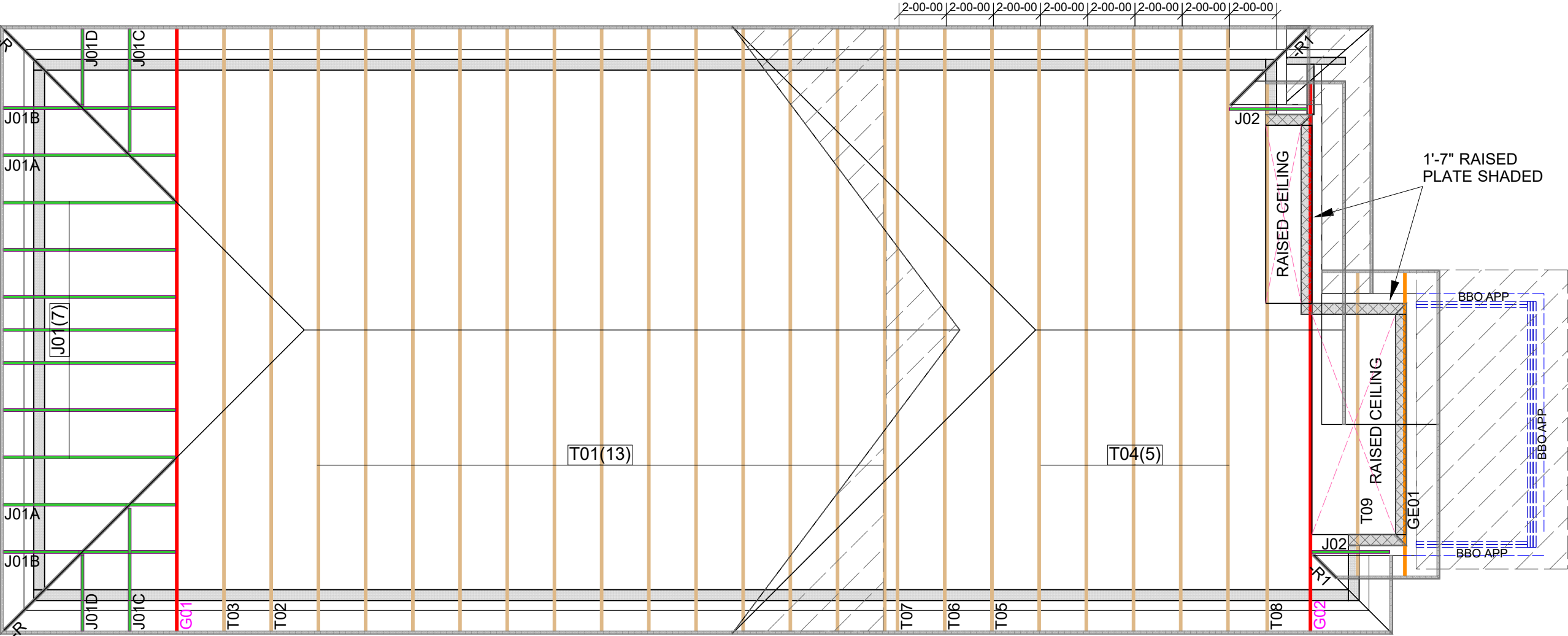


CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 04 2023
PER: 
CHIEF BUILDING OFFICIAL

MHP 23024



UPPER LEVEL: TOP OF PLATE FLUSH WITH U/S SOFFIT



CONVENTIONAL
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK GROUP
Job #	23-00081R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC OSHAWA,ON
Model	RIVER 5-1
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-07-04
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 5\RIVER 5-1\T-RIVE

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	6.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

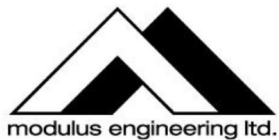
Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



**Read Carefully Prior to
Manufacture and Installation**

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)

2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.

3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses

4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)

5. Cut members to bear tightly against one another

6. Place plates on each face of truss at each joint and end using proper roller or hydraulic press. Knots and wane at locations are regulated by TPIC Appendix G

7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC

8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture

9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment

10. Connections not shown are the responsibility of others

11. Do not cut or alter truss members or plates without prior approval of an engineer

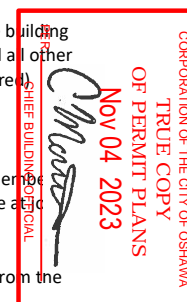
12. Install and load vertically unless otherwise noted

13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient

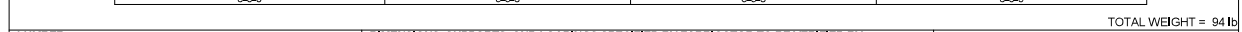
14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G

16. Building designer must review individual component drawings to ensure they are suitable for the structure

15. Not designed for solar panels unless specifically noted



MHP 23024



BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H BEARING SIZE FACTOR = 1.15 AT JNT(S) M, H (BASED ON SUPPORT DEPTH = 1-8)	GIRDER TYPE: CPrimeHp SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0
--	--

FR-TO	FROM	TO	LENGTH	FR-TO	
A-B	0 / 36	-119.4	-119.4	0.11 (1)	10.00
B-C	-4244 / 0	-119.4	-119.4	0.84 (1)	3.37
C-D		-225.2	-225.2	0.68 (1)	3.09
D-E	-5035 / 0	-225.2	-225.2	0.68 (1)	3.09
E-F	-5035 / 0	-119.4	-119.4	0.84 (1)	3.37
F-F	-4244 / 0	-119.4	-119.4	0.84 (1)	3.37

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED
ROOF LIVE LOAD

F-G	0 / 36	-119.4	-119.4	0.11 (1)	10.00	B-L	0 / 3844	0.95 (1)	ALLOWABLE DEFL.(LL)= L/360 (0.78") CALCULATED VERT. DEFL.(LL) = L/999 (0.17") ALLOWABLE DEFL.(TL)= L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/937 (0.29")
M-B	-2955 / 0	0.0	0.0	0.33 (1)	4.95	I-F	0 / 3844	0.95 (1)	
H-F	-2955 / 0	0.0	0.0	0.33 (1)	4.95				
M-L	0 / 0	-34.4	-34.4	0.30 (4)	10.00				CSI: TC=0.84/1.00 (B-C), BC=0.82/1.00 (I-K), WB=0.95/1.00 (F-I), SSI=0.49/1.00 (D-E)
L-K	0 / 3813	-34.4	-34.4	0.82 (1)	10.00				
K-J	0 / 3813	-34.4	-34.4	0.82 (1)	10.00				
J-I	0 / 3813	-34.4	-34.4	0.82 (1)	10.00				

<p><u>CONNECTION REQUIREMENTS</u></p> <p>1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.</p>	<p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</p>
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MODULUS ENGINEERING LTD.	NAIL VALUES				
	PLATE GRIP(DRY)		SHEAR		SECTION
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
	650	371	1747	788	1987
	1873				


 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.88 (L) (INPUT = 0.90)
 SIFMFAI = 0.87 (L) (INPUT = 1.00)

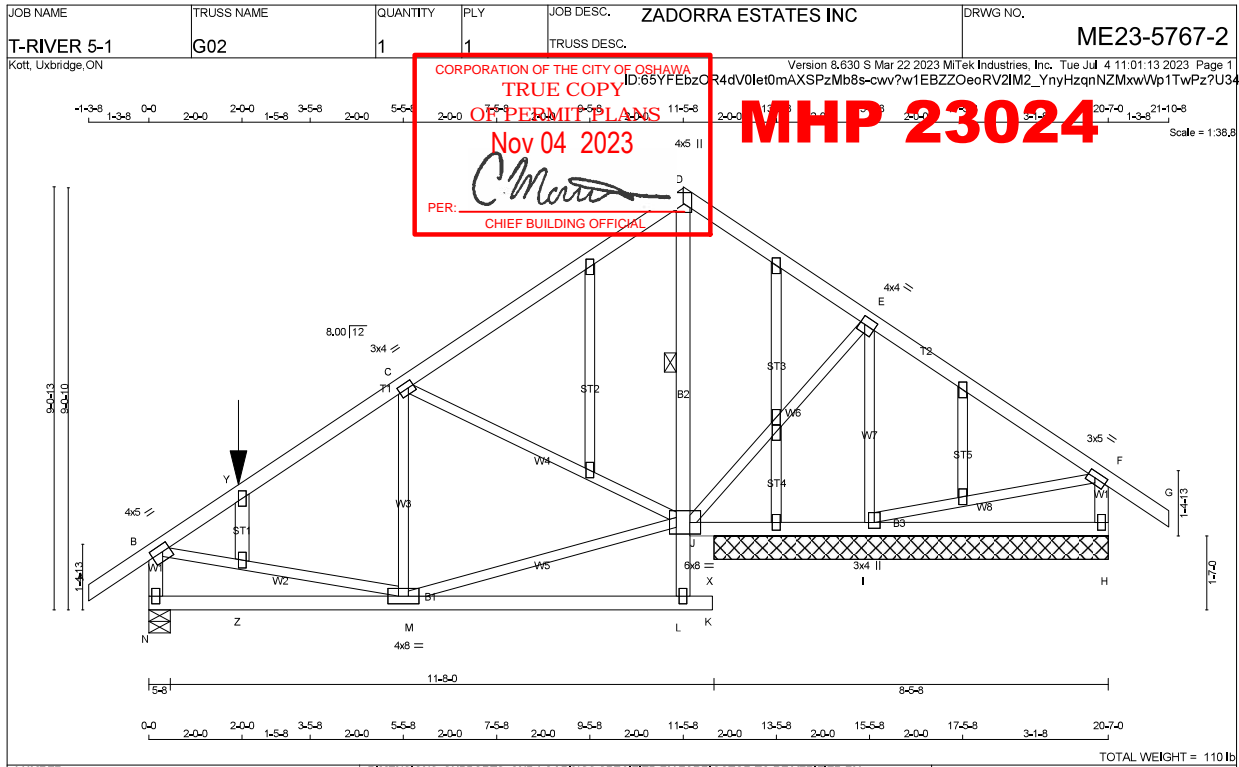

 D. A. SHERMAN
 100123373
 Mechanical Engineering
 State of California
 expires 07/04/2025




 REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD01 (VER 06/2017) BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of product is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.com and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.bcsincanada.com



N, L, G, A, RULES										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
CHORDS SIZE LUMBER DESCR.										BEARINGS										SPECIFIED LOADS:									
A - D 2x4 DRY No.2 SPF										FACTORED GROSS REACTION										TOP CH. LL = 34.8 PSF									
D - G 2x4 DRY No.2 SPF										MAXIMUM FACTORED GROSS REACTION										DL = 6.0 PSF									
N - B 2x4 DRY No.2 SPF										DOWN HORIZ UPLIFT IN-SX										BOT CH. LL = 0.0 PSF									
H - F 2x4 DRY No.2 SPF										N 1162 0 1162 0 5-8 1-8										DL = 7.3 PSF									
N - K 2x4 DRY No.2 SPF										H 465 0 465 0 0 8-5-8 1-10										TOTAL LOAD = 48.1 PSF									
L - D 2x4 DRY No.2 SPF										I 1262 0 1262 0 0 8-5-8 1-10																			
J - H 2x4 DRY No.2 SPF										X 300 0 300 0 0 8-5-8 1-10																			
ALL WEBS 2x3 DRY No.2 SPF																				SPACING = 24.0 IN. C/C									
EXCEPT																				GIRDER TYPE: CPrimeHip									
ALL GABLE WEBS 2x3 DRY No.2 SPF										UNFACTORED REACTIONS										SIDE SETBACK = 2'-0"									
EXCEPT ST8 2x4 DRY No.2 SPF										1ST CASE MAX./MIN. COMPONENT REACTIONS										END SETBACK = 2'-0"									
DRY: SEASONED LUMBER.										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										END WALL WIDTH = 5'-8"									
GABLE STUDS SPACED AT 2'-0" OC.										N 809 607 / 0 0 / 0 0 / 0 202 / 0 0 / 0										CORNER FRAMING TYPE: CONVENTIONAL									
										H 321 255 / 0 0 / 0 0 / 0 66 / 0 0 / 0										END JACK TYPE: CONVENTIONAL									
										I 883 632 / 0 0 / 0 0 / 0 251 / 0 0 / 0										APPLIED TO FRONT SIDE									
										X 212 142 / 0 0 / 0 0 / 0 69 / 0 0 / 0										- ADDTL LOADS BASED ON 55 % OF GSL.									
										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H, L, X										LOADS APPLIED TO FIRST 2'-0" OF SPAN									
										BRACING										MEASURED FROM THE LEFT.									
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015									
										MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.																			
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.																			
										1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-J.																			
										END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW																			
PLATES (table is in inches)										LOADING										THIS DESIGN COMPLIES WITH:									
JT TYPE PLATES W LEN Y X										TOTAL LOAD CASES: (4)										- PART 9 OF CBC 2018, NBC-2019AE									
B TMWV4 MT20 4.0 5.0 1.75 2.00																				- PART 9 OF OBC 2012 (2019 AMENDMENT)									
C TMWV4 MT20 3.0 4.0 1.50 1.50																				- CSA 086-14									
D TVV+p MT20 4.0 5.0 Edge 1.75																				- TPIC 2014									
E TMWV4 MT20 4.0 4.0 2.00 1.25																				(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD									
F TMWV4 MT20 3.0 5.0 1.50 2.00																				ALLOWABLE DEFL.(LL) = L/360 (0.40")									
H BMV1+p MT20 2.0 4.0																				CALCULATED VERT. DEFL.(LL) = L/999 (0.02")									
I BMWV1+t MT20 3.0 4.0 1.50 1.50																				ALLOWABLE DEFL.(TL) = L/360 (0.40")									
J BVWVWV4 MT20 6.0 8.0 3.00 2.75																				CALCULATED VERT. DEFL.(TL) = L/999 (0.06")									
L BMV+p MT20 2.0 4.0																				CSI: TC=0.82/1.00 (B-C:1), BC=0.23/1.00 (I-X:1), WB=0.48/1.00 (C-J:1), SS=0.31/1.00 (C-D:1)									
M BMVWV4 MT20 4.0 8.0																				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00									
N BMV1+p MT20 2.0 4.0																				COMP=1.00 SHEAR=1.00 TENS=1.00									
O P, Q, R, S, T, U, V, W																				COMPANION LIVE LOAD FACTOR = 1.00									
O NP+w MT20 2.0 4.0																				AUTOSOLVE RIGHT HEEL ONLY									
U NP+w MT20 2.0 4.0 1.75 1.00																				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																				NAIL VALUES									
																				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PSI) (PLI)									
																				MAX MIN MAX MIN MAX MIN									
																				MT20 650 371 1747 788 1987 1873									
																				PLATE PLACEMENT TOL. = 0.250 inches									
																				PLATE ROTATION TOL. = 5.0 Deg.									
																				JSI GRIP= 0.85 (E) (INPUT = 0.90)									
																				JSI METAL= 0.33 (B) (INPUT = 1.00)									

MODULUS ENGINEERING LTD.

07/04/2023

D. A. SHERMAN

100123373

LICENSED PROFESSIONAL ENGINEER

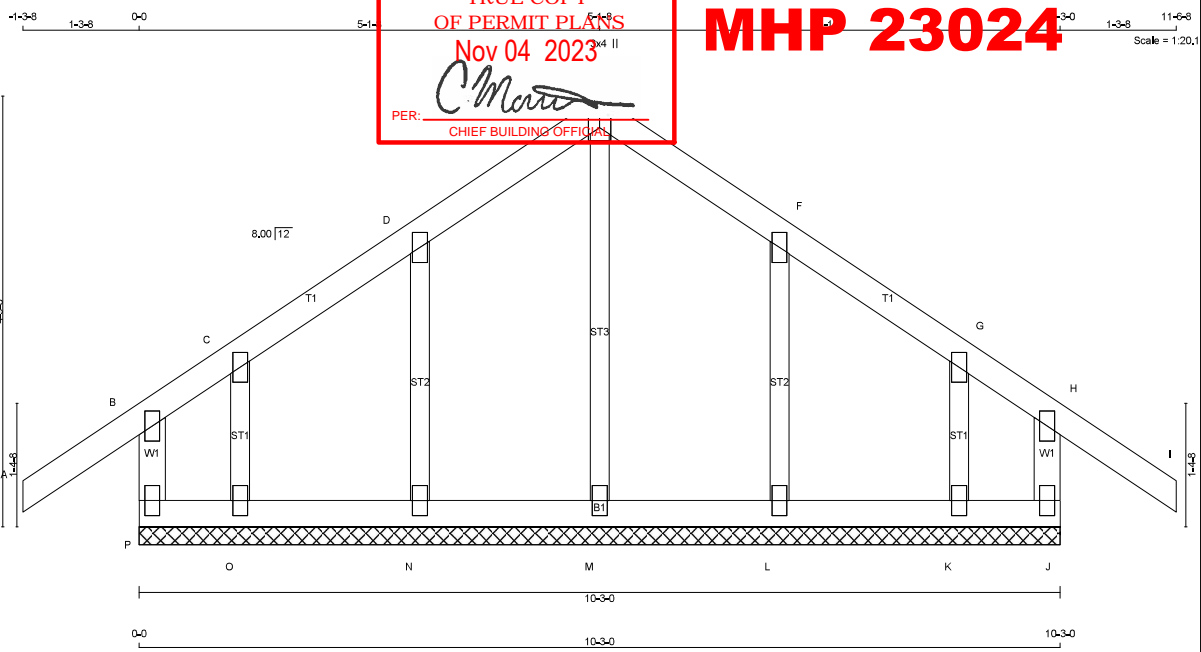
PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 04 2023
PER: *C. M...*
CHIEF BUILDING OFFICIAL

MHP 23024



TOTAL WEIGHT = 42 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY No.2	SPF
A - E	2x4	DRY No.2	SPF
E - I	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
P - J	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
GABLE STUDS SPACED AT 2'-0" O.C.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF
SPACING = 24.0 IN. G.C.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
C, D, F, G					
C TMV+w	MT20	2.0	4.0		
E TMV+p	MT20	3.0	4.0	2.25	1.50
H TMV+p	MT20	2.0	4.0		
J BMV1+p	MT20	2.0	4.0		
K, L, M, N, O					
K BMV1+w	MT20	2.0	4.0		
P BMV1+p	MT20	2.0	4.0		

LOADING
TOTAL LOAD CASES: (4)

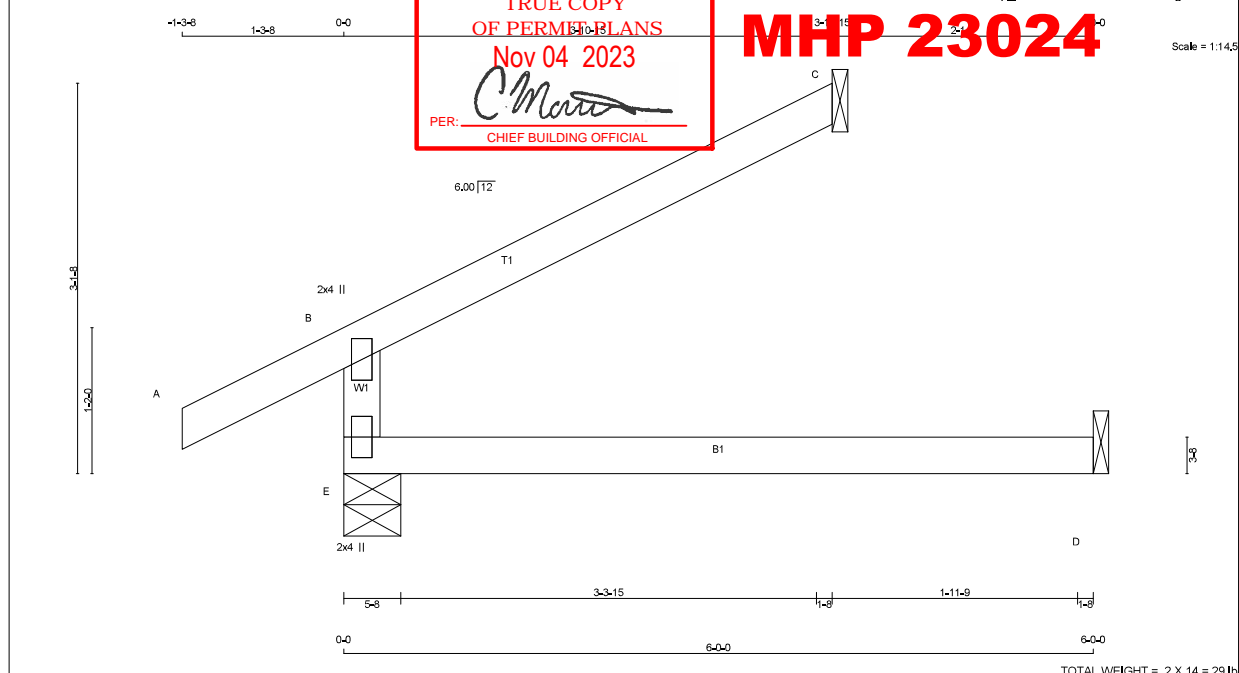
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
P-B	-254 / 0	0.0	0.06 (1)	M-E	-300 / 0
A-B	0 / 45	-119.4	0.16 (1)	N-D	-259 / 0
B-C	0 / 13	-119.4	0.10 (1)	O-C	-114 / 0
C-D	0 / 55	-119.4	0.07 (1)	L-F	-259 / 0
D-E	0 / 52	-119.4	0.07 (1)	K-G	-114 / 0
E-F	0 / 52	-119.4	0.07 (1)		
F-G	0 / 55	-119.4	0.07 (1)		
G-H	0 / 13	-119.4	0.10 (1)		
H-I	0 / 45	-119.4	0.16 (1)		
J-H	-254 / 0	0.0	0.06 (1)		
P-O	-34 / 0	-18.2	0.01 (4)		
O-N	-39 / 0	-18.2	0.02 (4)		
N-M	-46 / 0	-18.2	0.02 (4)		
M-L	-46 / 0	-18.2	0.02 (4)		
L-K	-39 / 0	-18.2	0.02 (4)		
K-J	-34 / 0	-18.2	0.01 (4)		

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.16/1.00 (H-I-1), BC=0.02/1.00 (M-N-4), WB=0.10/1.00 (E-M-1), SSI=0.11/1.00 (A-B-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY
NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEER'S SEAL



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPECIFIED LOADS:	TOP CH.	LL	PSF
E - B	2x4	DRY	No.2	SPF	SPF	SPF	SPF	TOP CH. LL	=	34.8	PSF
A - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF	DL	=	6.0	PSF
E - D	2x4	DRY	No.2	SPF	SPF	SPF	SPF	BOT CH. LL	=	0.0	PSF
DRY: SEASONED LUMBER.								DL	=	7.3	PSF
								TOTAL LOAD	=	48.1	PSF

PLATES (table is in inches)				SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
JT	TYPE	PLATES	W	LEN	Y	X		THIS DESIGN COMPLIES WITH:			
B	TMV+p	MT20	2.0	4.0				- PART 9 OF BCBC 2018, NBC-2019AE			
E	BMV1+p	MT20	2.0	4.0				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 086-14			
								- TPIC 2014			

UNFACTORED REACTIONS				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C				DESIGN ASSUMPTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	-OVERHANG NOT TO BE ALTERED OR CUT OFF.			
E	362	265 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD			
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	ALLOWABLE DEFL.(LL)= L/360 (0.20")			
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")			

BRACING				TOTAL LOAD CASES: (4)				ALLOWABLE DEFL.(TL)= L/360 (0.20")			
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.								CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")			
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.								CSI TC=0.31/1.00 (B-C.1) BC=0.13/1.00 (D-E.4) WB=0.00/1.00 (n/a.0) , SSI=0.20/1.00 (B-C.1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			

LOADING				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				NAIL VALUES			

MODULUS ENGINEERING LTD.

07/04/2023

D. A. SHERMAN

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PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

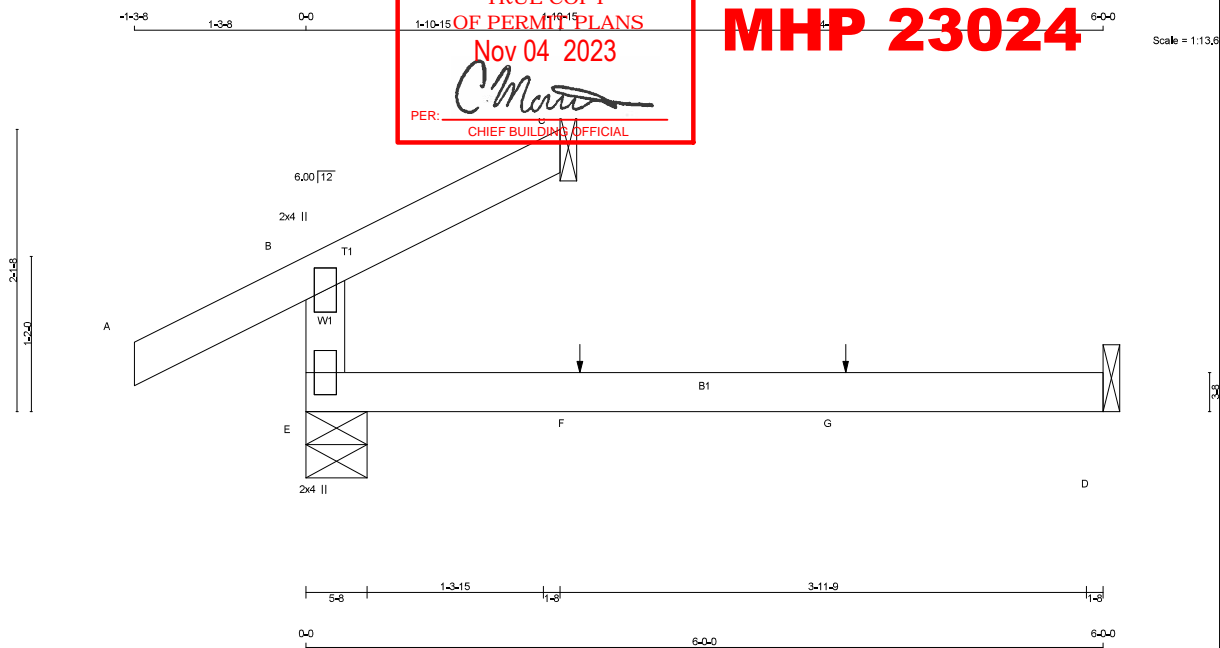
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com

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OF PERMIT PLANS
Nov 04 2023
PER: *C. M...*
CHIEF BUILDING OFFICIAL

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 4 11:01:18 2023 Page 1
ID: 65YFEBzOR4dV0le0mAXSPzMb8s-ztjuzlIKN51xvDN?Xve9Ergl3rVzEkhtfg5lDbcz?U3?



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 2.0 4.0
E BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	368	0	368	0	5-8	1-8	1-8	1-8
C	86	0	86	0	1-8	1-8	1-8	1-8
D	45	0	51	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	259	177 / 0	0 / 0	0 / 0	82 / 0	0 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	9 / 0	0 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC1)	MAX. (LC)	MEMB. UNBRAC LENGTH	FR-TO	MAX. FORCE (LBS)	MAX. (LC)
FR-TO								
E-B	-304 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00			
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25			
E-F	0 / 0	-18.2	-18.2	0.13 (4)	10.00			
F-G	0 / 0	-18.2	-18.2	0.13 (4)	10.00			
G-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	1	FRONT	VERT	TOTAL	—	C1
G	4-0-12	1	1	1	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.16/1.00 (A-B 1), BC=0.13/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PU)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

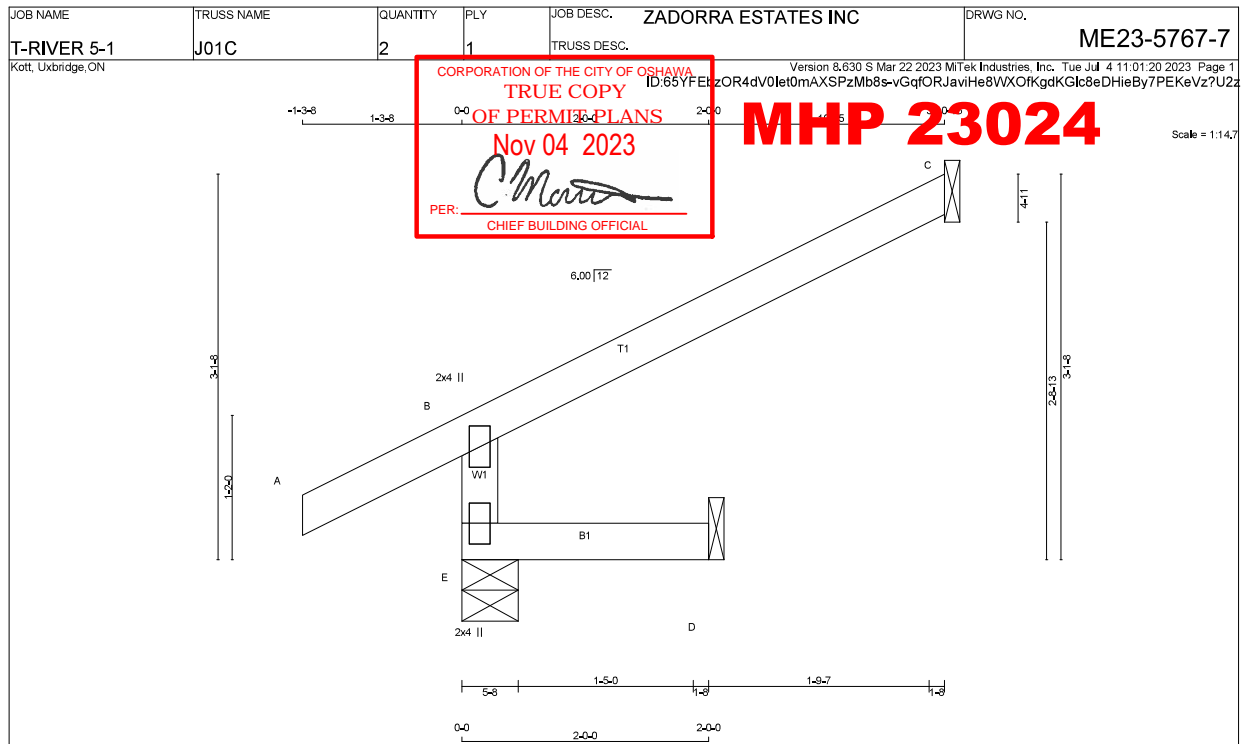


REVIEW FOR TRUSS COMPONENT ONLY

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<div>LUMBER</div> <div>N. L. G. A. RULES</div> <div>CHORDS SIZE LUMBER</div> <div>E - B 2x4 DRY No.2</div> <div>A - C 2x4 DRY No.2</div> <div>E - D 2x4 DRY No.2</div> <div>DRY: SEASONED LUMBER.</div>		<div>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</div> <div>BEARINGS</div> <div><table><tr><td></td><td>FACTORED</td><td>MAXIMUM FACTORED</td><td>INPUT</td><td>REQRD</td></tr><tr><td></td><td>GROSS REACTION</td><td>GROSS REACTION</td><td>BRG</td><td>BRG</td></tr><tr><td>JT</td><td>VERT HORZ</td><td>DOWN HORZ</td><td>UPLIFT</td><td>IN+X IN+X</td></tr><tr><td>E</td><td>474 0</td><td>474 0</td><td>0</td><td>5-3 1-3</td></tr><tr><td>C</td><td>175 0</td><td>175 0</td><td>0</td><td>1-8 1-8</td></tr><tr><td>D</td><td>16 0</td><td>18 0</td><td>0</td><td>1-8 1-8</td></tr></table></div> <div>SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D</div> <div>UNFACTORED REACTIONS</div> <div><table><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>E</td><td>326</td><td>265 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>62 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>120</td><td>102 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>18 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>13</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>13 / 0</td><td>0 / 0</td></tr></table></div> <div>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E</div> <div>BRACING</div> <div>TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6,25 FT.</div> <div>MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.</div> <div>ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.</div> <div>LOADING</div> <div>TOTAL LOAD CASES: (5)</div> <div><table><tr><td colspan="2">CHORDS</td><td colspan="2">FACTORED</td><td colspan="2">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>VERT. LOAD (PLF)</td><td>LC1 MAX (LC)</td><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td>FR-TO</td><td></td></tr><tr><td>E-B</td><td>-454 / 0</td><td>0.0</td><td>0.0</td><td>0.01 (4)</td><td>7.81</td></tr><tr><td>A-B</td><td>0 / 36</td><td>-119.4</td><td>-119.4</td><td>0.16 (1)</td><td>10.00</td></tr><tr><td>B-C</td><td>-26 / 0</td><td>-119.4</td><td>-119.4</td><td>0.31 (1)</td><td>6.25</td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.2</td><td>-18.2</td><td>0.02 (4)</td><td>10.00</td></tr></table></div> <div>CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN</div> <div>PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.</div>			FACTORED	MAXIMUM FACTORED	INPUT	REQRD		GROSS REACTION	GROSS REACTION	BRG	BRG	JT	VERT HORZ	DOWN HORZ	UPLIFT	IN+X IN+X	E	474 0	474 0	0	5-3 1-3	C	175 0	175 0	0	1-8 1-8	D	16 0	18 0	0	1-8 1-8	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0	C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	CHORDS		FACTORED		WEBS		MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO		FROM	TO	FR-TO		E-B	-454 / 0	0.0	0.0	0.01 (4)	7.81	A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	B-C	-26 / 0	-119.4	-119.4	0.31 (1)	6.25	E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00	<div>DESIGN CRITERIA</div> <div>SPECIFIED LOADS:</div> <div>TOP CH. LL = 34.8 PSF</div> <div>DL = 6.0 PSF</div> <div>BOT CH. LL = 0.0 PSF</div> <div>DL = 7.3 PSF</div> <div>TOTAL LOAD = 48.1 PSF</div> <div>SPACING = 24.0 IN. G/C</div> <div>THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015</div> <div>THIS DESIGN COMPLIES WITH:</div> <div>PART 9 OF CBC 2018, NBC-2019AE</div> <div>PART 9 OF OBC 2012 (2019 AMENDMENT)</div> <div>CSA 086-14</div> <div>TPIC 2014</div> <div>DESIGN ASSUMPTIONS</div> <div>OVERHANG NOT TO BE ALTERED OR CUT OFF.</div> <div>(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD</div> <div>ALLOWABLE DEFL.(LL)= L/360 (0.19")</div> <div>CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")</div> <div>ALLOWABLE DEFL.(TL)= L/360 (0.19")</div> <div>CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")</div> <div>CSI TC=0.31/1.00 (B-C:1) BC=0.02/1.00 (D-E:4) ,</div> <div>WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1)</div> <div>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10</div> <div>COMP=1.10 SHEAR=1.10 TENS= 1.10</div> <div>COMPANION LIVE LOAD FACTOR = 1.00</div> <div>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</div> <div>NAIL VALUES</div> <div>PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)</div> <div>MAX MIN MAX MIN MAX MIN</div> <div>MT20 650 371 1747 788 1987 1873</div> <div>PLATE PLACEMENT TOL. = 0,250 inches</div> <div>PLATE ROTATION TOL. = 5.0 Deg.</div> <div>JSI GRIP= 0.25 (B) (INPUT = 0.90)</div> <div>JSI METAL= 0.19 (B) (INPUT = 1.00)</div>
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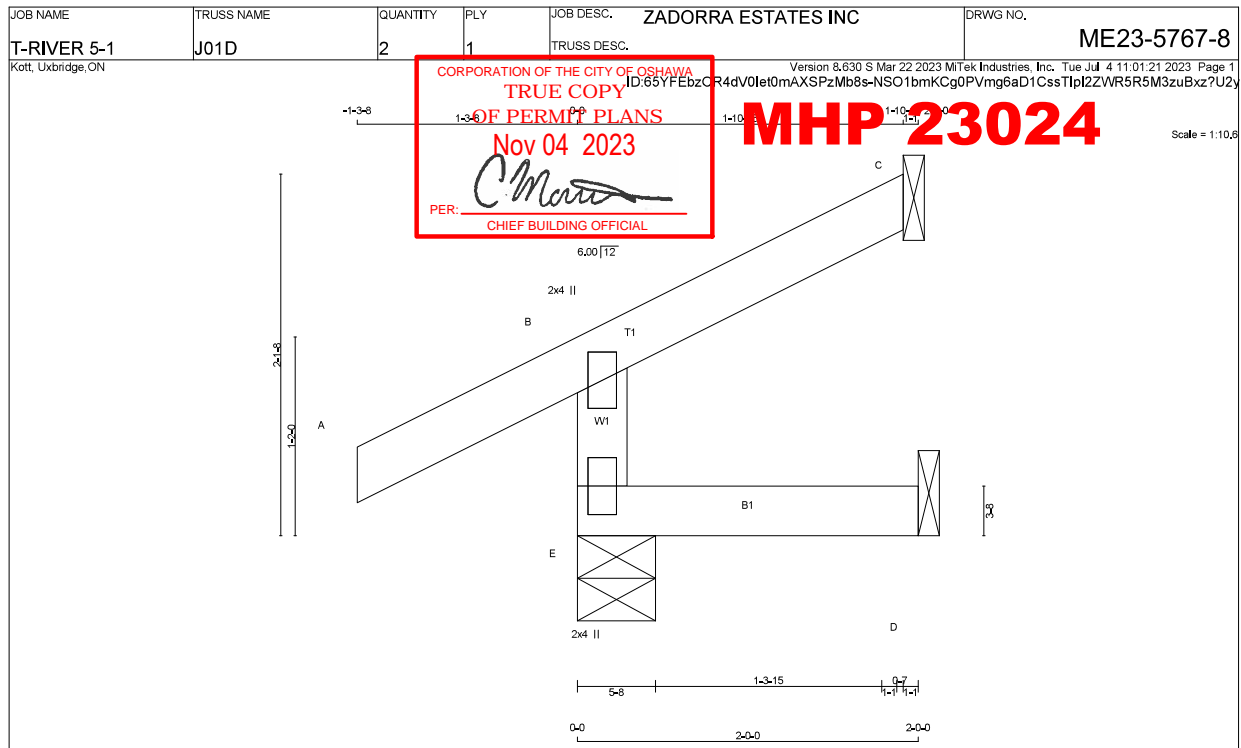
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LICENSED PROFESSIONAL ENGINEER

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COMPONENT REACTIONS	WIND	DEAD	SOIL	E	COMBINED	SNOW LIVE PERM.LIVE	0/0	47/0	0/0	C	224	177/0 0/0	0/0	0/0	0/0	E	59	50/0 0/0	0/0	9/0	0/0	D	13	0/0 0/0	0/0	13/0	0/0	CHORDS			WEBS			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO		FROM TO		FR-TO		E-B	-304 / 0	0.0 0.0	0.01 (4)		7.81	A-B	0 / 36	-119.4 -119.4	0.16 (1)		10.00	B-C	-12 / 0	-119.4 -119.4	0.07 (1)		6.25	E-D	0 / 0	-18.2 -18.2	0.02 (4)		10.00	<div>DESIGN CRITERIA</div> <div>SPECIFIED LOADS:</div> <table><tr><td>TOP CH. LL</td><td>=</td><td>34.8</td><td>PSF</td></tr><tr><td>DL</td><td>=</td><td>6.0</td><td>PSF</td></tr><tr><td>BOT CH. 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DEFL.(TL) = L/ 999 (0.00")</div> <div>CSI TC=0.16/1.00 (A-B 1), BC=0.02/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.11/1.00 (A-B 1)</div> <div>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10</div> <div>COMPANION LIVE LOAD FACTOR = 1.00</div> <div>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.</div> <div>NAIL VALUES</div> <table><tr><td>PLATE GRIP(DRY)</td><td>SHEAR</td><td>SECTION</td></tr><tr><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td></tr><tr><td>MAX MIN</td><td>MAX MIN</td><td>MAX MIN</td></tr><tr><td>MT20 650 371</td><td>1747 788</td><td>1987 1873</td></tr></table> <div>PLATE PLACEMENT TOL. = 0.250 inches</div> <div>PLATE ROTATION TOL. = 5.0 Deg.</div> <div>JSI GRIP= 0.17 (B) (INPUT = 0.90)</div> <div>JSI METAL= 0.13 (B) (INPUT = 1.00)</div>										TOP CH. LL	=	34.8	PSF	DL	=	6.0	PSF	BOT CH. 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PROVINCE OF ONTARIO

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