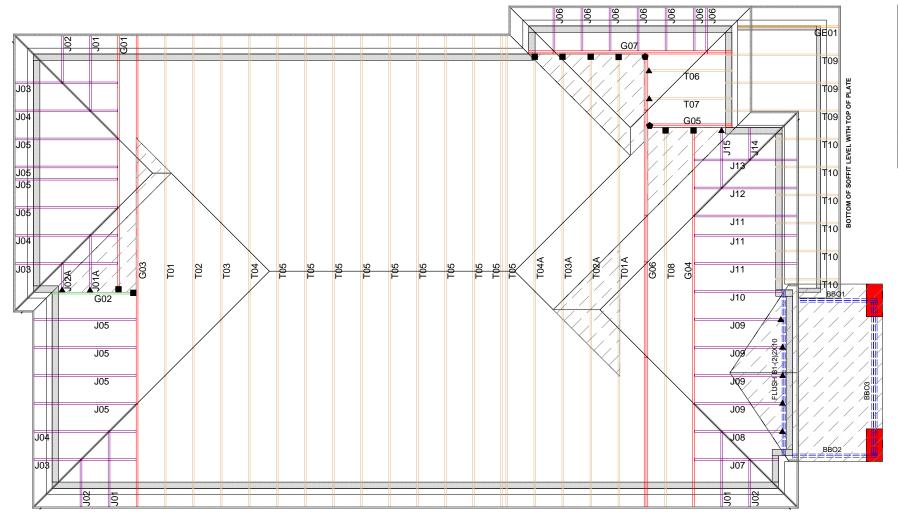


MHP 23029



Hanger Name	Symbol	QTY
LUS24	A	10
LJS26DS		8
	•	0
HGUS26-2	•	2
	Δ	0
		0
	\bigcirc	0
	Ô	0

STEEL BEAMS IN GARAGE ASSUMED TO BE DROPPED



ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/W WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6". DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB IN	FORMATION
Customer	GREENPARK HOMES
Job #	23-00114R0
Address	ZADORRA ESTATES ROSE 3 EL 3 OSHAWA,ON
Model	ROSE 3 EL 3
Sales Rep	RALPH MIRIGELLO
Designer	BB
Date	6/1/2023
Path	S:\DESIGN\KLU\CUSTOMERS\GREENPARK\ZADORRA ESTATES\MODELS\ROSE 3\ROSE 3-3\T-ROSE

DESIGN INFORMATION					
Code	NBCC 2015				
Bldg	Residential - HSB (NBCC Part 9)				
TC LL	23.3 lb/ft²				
TC DL	6.0 lb/ft ²				
BC LL	0.0 lb/ft²				
BC DL	7.3 lb/ft²				
Deflection	LL=L/360 TL=L/360				
Spacing	24" O/C unless otherwise				
Spacing	noted				
Complies With	OBC 2012 (2019 Amendment) CSA 086-14 and TPIC 2014				

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400







PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER.

IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

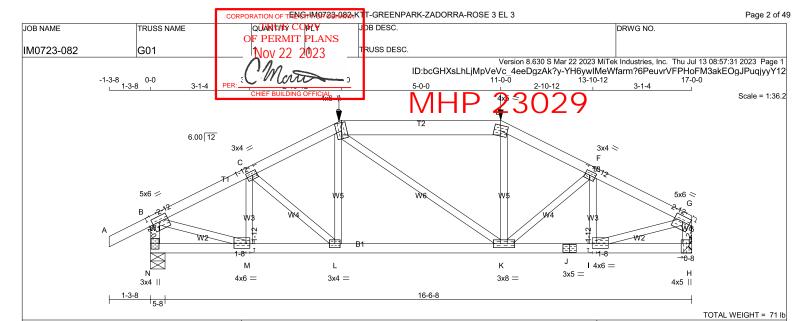
- 1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
- 2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
- 3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
- 4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

- 1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
- 2. THE COMPRESSION CHORDS ARE LATERALLY BRACED BY CONTINUOUS RIGID DIAPHRAGM SHEATHING OR AS SPECIFIED ON THE DRAWING.
- 3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE BUILDING DESIGNER.
- 4. IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES SPANNING MORE THAN 12.37M (40'-7").



LUMBER									
N. L. G. A. RULES									
CHORDS	SIZE		LUMBER	DESCR.					
A - D	2x4	DRY	No.2	SPF					
D - E	2x6	DRY	No.2	SPF					
E - G	2x4	DRY	No.2	SPF					
N - B	2x4	DRY	No.2	SPF					
H - G	2x4	DRY	No.2	SPF					
N - J	2x4	DRY	No.2	SPF					
J - H	2x4	DRY	No.2	SPF					
ALL WEBS	2x3	DRY	No.2	SPF					

DRY: SEASONED LUMBER

PLATES (table is in inches)

BMWW-t

BMWW-t

BMV1+p

JΤ	TYPE	PLATES	W	LEN	Y X
В	TMVW-t	MT20	5.0	6.0	2.00 2.75
С	TMWW-t	MT20	3.0	4.0	1.50 1.75
D	TTWW+m	MT20	4.0	6.0	
Ε	TTW-m	MT20	4.0	5.0	
F	TMWW-t	MT20	3.0	4.0	1.50 1.75
G	TMVW-t	MT20	5.0	6.0	2.00 2.75
Н	BMV1+t	MT20	4.0	5.0	Edge 0.50
1	BMWW-t	MT20	4.0	6.0	1.75 1.50
J	BS-t	MT20	3.0	5.0	

INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

MT20

MT20

MT20

3.0 4.0 4.0 6.0

3.0 4.0

1.75 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2271	0	2271	0	0	5-8	3-14
Н	2109	0	2109	0	0	MECHANIC	AL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	151 LUASE	IVIAA./I	VIIN. COMPO	NEINT REACTION	VO		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1586	1154 / 0	0/0	0/0	0/0	433 / 0	0/0
Н	1476	1059 / 0	0/0	0/0	0/0	417 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	СН	ORDS					WE	BS		
ı	MAX	. FACTOREI	D FACTO	RED				MAX. FACT	ORED	
ı	MEMB.	FORCE	E VERT. LO	DAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
ı		(LBS)	(P	LF) (CSI (LC)	UNBRA	.C	(LBS)	CSI (LC)
ı	FR-TO	` ′	FROM	ΤΌ	. ,	LENGT	H FR-TO	` '	`	•
ı	A-B	0 / 36	-119.4	-119.4	0.17(1)	10.00	M- C	-602 / 0	0.11	(1)
ı			-119.4					0 / 203	0.05	(1)
ı	C- D	-2905 / 0	-119.4	-119.4	0.26(1)	3.78	L- D	0 / 152	0.06	(4)
ı	D- E	-2598 / 0	-225.2	-225.2	0.57 (1)	4.28	D-K	0/2	0.00	(1)
ı	E-F	-2907 / 0	-119.4	-119.4	0.26(1)	3.78	K-E	0 / 153	0.06	(4)
ı	F- G	-2718 / 0	-119.4	-119.4	0.26(1)	3.91	K-F	0 / 205	0.05	(1)
ı	N- B	-2213 / 0	0.0	0.0	0.25(1)	5.58	I- F	-605 / 0	0.11	(1)
ı	H- G	-2051 / 0	0.0	0.0	0.23(1)	5.78	B- M	0 / 2535	0.63	(1)
ı							I- G	0 / 2535	0.63	(1)
ı	N- M	0/0	-34.4	-34.4	0.09(4)	10.00				
ı	M- L	0 / 2444	-34.4	-34.4	0.51(1)	10.00				
ı	L- K	0 / 2596	-34.4	-34.4	0.55(1)	10.00				
ı	K- J	0 / 2443	3 -34.4	-34.4	0.52(1)	10.00				
ı	J- I	0 / 2443	-34.4	-34.4	0.52(1)	10.00				
ı	I- H	0/0	-34.4	-34.4	0.09(4)	10.00				
ı										
ı	SPECIF	IED CONCE	NTRATED LC	ADS (LE	3S)					
ı	JT	LOC. L	.C1 MAX-	MAX-	+ F/	4CE	DIR.	TYPE	HEEL	CONN.

FRONT

VFRT

TOTAL

CONNECTION REQUIREMENTS

11-0-0

-367

-367

-367

-367

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS: 34.8 6.0 PSF PSF PSF TOP CH. 0.0 7.3 LL TOTAL LOAD 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENEND JACK TYPE: CONVENTIONAL CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.57") CALCULATED VERT. DEFL.(LL)= L/999 (0.07") ALLOWABLE DEFL.(TL)= L/360 (0.57") CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.57/0.97 (D-E:1) , BC=0.55/0.97 (K-L:1) , WB=0.63/0.97 (B-M:1) , SSI=0.37/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

C1 C1

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

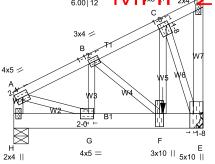
JSI GRIP= 0.88 (N) (INPUT = 0.90) JSI METAL= 0.74 (J) (INPUT = 1.00)





2-5-10 4-8-12 1-3-4 6-0-0 0 6.00 12 3x4 //

Scale = 1:36.5



TOTAL WEIGHT = 33 lb

LUMBER								
N. L. G. A. RULES								
CHORDS	SIZE		LUMBER	DESCR.				
H - A	2x4	DRY	No.2	SPF				
A - D	2x4	DRY	No.2	SPF				
E - D	2x4	DRY	No.2	SPF				
H - E	2x6	DRY	No.2	SPF				
ALL WEBS	2x3	DRY	No.2	SPF				
DRY: SEASO	ONED LI	JMBER.						

PL	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	Χ			
Α	TMVW-t	MT20	4.0	5.0	2.00	2.25			
В	TMWW-t	MT20	3.0	4.0	1.50	1.75			
С	TMWW+t	MT20	4.0	5.0	1.50	1.00			
D	TMV+p	MT20	2.0	4.0					
Ε	BMVW1+t	MT20	5.0	10.0	Edge	1.50			
F	BMWW+t	MT20	3.0	10.0					
G	BMWW-t	MT20	4.0	5.0	2.00	2.00			
Н	BMV1+p	MT20	2.0	4.0					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS A	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
BEARINGS	

	FACTORED		MAXIMUN		INPUT	REQRD	
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
ΙT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
1	1255	0	1255	0	0	5-8	1-8
	2334	0	2334	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
Н	877	635 / 0	0/0	0/0	0/0	242 / 0	0/0			
E	1631	1180 / 0	0/0	0/0	0/0	451 / 0	0/0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS WEBS MAX. FACTORED FACTORED MAX. FACTORED							ORED	
MEMB.	FORCE (LBS)	VERT. LOAD			. MEME RAC			
FR-TO	(250)	FROM TO					00.,	
H- A	-1210 / 0	0.0	0.0 0.13	3 (1) 7.	16 A- G	0 / 1239	0.31	(1)
A-B	-1301 / 0	-238.9 -23	8.9 0.22	2 (1) 5.	33 G-B	-114 / 55	0.02	(4)
	-908 / 0							
	-14 / 0							
E- D	-45 / 0	0.0	0.0 0.01	(1) 7.	81 C-E	-2129 / 0	0.47	(1)
	0 / 0 0 / 1182 0 / 799	-36.5 -3	6.5 0.03 6.5 0.44 8.2 0.39	l (1) 10.	00			
SPECIF	SPECIFIED CONCENTRATED LOADS (LBS)							
JT F	LOC. LC1 4-8-12 -1476		AAX+	FACE FRONT	DIR. VERT	TYPE TOTAL	HEEL	CONN. C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: 34.8 6.0 0.0 7.3 DI = PSF LL = DL = BOT CH. = 48.1 TOTAL LOAD

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip SIDE SETBACK = 0-0 END SETBACK = 6-0-0 END WALL WIDTH = 0-0 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-8-12 OF SPAN

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.22/0.97 (A-B:1) , BC=0.44/0.97 (F-G:1) , WB=0.50/0.97 (C-F:1) , SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873



			<u> </u>	
		CORPORATION OF TENG+1M0723+086	KTT-GREENPARK-ZADORRA-ROSE 3 EL 3	Page 4 of
JOB NAME	TRUSS NAME	QUANTING COPPY OF PERMIT PLANS	JDB DESC.	DRWG NO.
IM0723-082	G02	Nov 22 2023	TRUSS DESC.	
		M.S		23 MiTek Industries, Inc. Thu Jul 13 08:57:32 2023 Page 2 8dMGHziiN9hbCbQ42TxX4fj3JDRXvz8SM9yyY1
		PER: CHIEF BUILDING OFFICIAL		
		CHIEF BOILDING OFFICIAL	→ MHP 23029	PLATE PLACEMENT TOL. = 0.250 inches

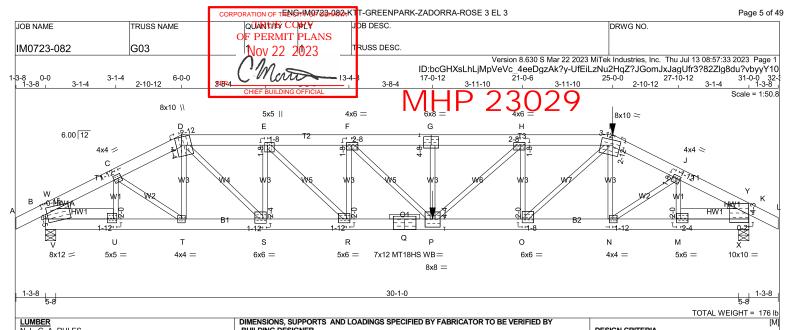
I.MATUEVIC IN 100528832

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

JSI GRIP= 0.89 (A) (INPUT = 0.90) JSI METAL= 0.47 (F) (INPUT = 1.00)

PLATE ROTATION TOL. = 5.0 Deg.



LUMBER									
N. L. G. A. RULES									
CHORDS	SIZE		LUMBER	DESCR.					
A - D	2x6	DRY	2100F 1.8E	SPF					
D - G	2x6	DRY	2100F 1.8E	SPF					
G - I	2x6	DRY	2100F 1.8E	SPF					
1 - L	2x6	DRY	2100F 1.8E	SPF					
B - Q	2x6	DRY	2100F 1.8E	SPF					
Q - K	2x6	DRY	2100F 1.8E	SPF					
REINFORC	ING MEN	/REDS							
HW1		DRY	No.2	SPF					
	2x6								
HW2	2x6	DRY	No.2	SPF					
	ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.								

PLATES (table is in inches)

JI	111 -	LAILS	vv			^
В	TMBMW1-m	MT20	8.0	12.0	3.25	1.00
С	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW+m	MT20	8.0	10.0	4.25	2.75
Е	TMWW+t	MT20	5.0	5.0	1.50	1.50
F	TMWW-t	MT20	4.0	6.0	1.50	2.50
G	TSW-I	MT20	6.0	8.0	4.50	4.00
Н	TMWW-t	MT20	4.0	6.0	1.50	2.50
1	TTWW-m	MT20	8.0	10.0	2.75	3.75
J	TMWW-t	MT20	4.0	4.0	1.50	1.75
K	TMBMW1-I	MT20	10.0	10.0	4.25	0.25
M	BMWW-t	MT20	5.0	6.0	2.00	2.25
Ν	BMWW-t	MT20	4.0	4.0	2.00	1.75
0	BMWW-t	MT20	6.0	6.0	2.00	1.50
Ρ	BMWWW-t	MT20	8.0	8.0	4.25	4.00
Q	BS-t	MT18HS	7.0	12.0		
R	BMWW-t	MT20	5.0	6.0	2.00	1.75
S	BMWW-t	MT20	6.0	6.0	2.25	1.75
Т	BMWW-t	MT20	4.0	4.0		
U	BMWW-t	MT20	5.0	5.0	2.00	1.75

WB - INDICATES BLOCKING REQUIRED

PROFESSIO I.MATUEVIO 100528832 VCE OF ON

READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

BUILDING DESIGNER

	FACTORED GROSS REACTION					INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
В	3777	0	3777	0	0	5-8	3-9	2x4 L
K	4766	0	4766	0	0	5-8	4-8	2x3 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./N					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
В	2637	1926 / 0	0/0	0/0	0/0	710 / 0	0/0
K	3330	2412 / 0	0/0	0/0	0/0	918 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B. K BEARING SIZE FACTOR = 1.15 AT JNT(S) B, K (BASED ON SUPPORT DEPTH = 1-8)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

HORDS		WEBS					
X. FACTORED	FACTORED				MAX. FACTO	ORED	
. FORCE	VERT. LOAD LC	MAX	MAX.	MEMB	. FORCE	MAX	
(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)	
	FROM TO		LENGTH	FR-TO			
0/0						0.17 (1)	
-3999 / 0	-119.4 -119.4	0.09(1)	5.08	C- T	0 / 733	0.18 (1)	
-5231 / 0	-119.4 -119.4	0.11(1)	4.55	T- D	-242 / 0	0.06(1)	
-5856 / 0	-119.4 -119.4	0.12(1)	4.34	D-S	0 / 3477	0.86 (1)	
-7685 / 0	-119.4 -119.4	0.19(1)	3.81	S-E	-2460 / 0	0.60(1)	
-9687 / 0	-119.4 -119.4	0.27(1)		E-R	0 / 2839	0.70(1)	
-11124 / 0	-119.4 -119.4	0.34(1)		R-F	-1861 / 0	0.46 (1)	
-11114 / 0				F-P	0 / 2038	0.50(1)	
-9550 / 0	-225.2 -225.2	0.33(1)	3.33	P- G	-619 / 0	0.15 (1)	
-7564 / 0	-119.4 -119.4	0.17 (1)	3.84	P- H	0 / 2140	0.53 (1)	
-6715 / 0	-119.4 -119.4	0.16(1)	4.05	O- H	-2439 / 0	0.60(1)	
-5101 / 0	-119.4 -119.4	0.12(1)	4.58	O- I	0 / 3819	0.95(1)	
0/0	-119.4 -119.4	0.05(1)	10.00	N- I	-353 / 25	0.09(1)	
				N- J	0 / 1000	0.25 (1)	
0 / 1795	-18.2 -18.2	0.10(1)	10.00	M- J	-1226 / 0	0.21 (1)	
0 / 1795	-18.2 -18.2	0.14(1)	10.00	V-W	0 / 65	0.00(1)	
0 / 4647						0.39 (1)	
0 / 5228						0.50(1)	
0 / 7685				X-Y	0 / 100	0.00(1)	
0 / 9687							
0 / 9687		0.64 (1)					
0 / 5963							
0 / 2279							
0 / 2279	-34.4 -34.4	0.12 (1)	10.00				
	6. FORCE (LBS) 0 / 0 -3999 / 0 -5231 / 0 -5856 / 0 -9687 / 0 -111124 / 0 -11114 / 0 -9550 / 0 -7564 / 0 -6715 / 0 -5101 / 0 0 / 1795 0 / 14647 0 / 5228 0 / 7685 0 / 9687 0 / 9687 0 / 9687 0 / 9687 0 / 9687 0 / 9687 0 / 9687	AX. FACTORED FACTORED	XX. FACTORED FACTORED FORCE CIBS CPLF CSI (LC) CSI (LC) FROM TO CSI (LC) FROM TO CSI (LC) FROM TO CSI (LC) CSI (LC)	XX. FACTORED FACTORED VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) CSI (LC) UMBRAC VERT. LOAD LC1 MAX MAX. (PLF) VERT. LOAD LENGTH VERT. LOAD LC1 MAX MAX. (PLF) VERT. LOAD LC1 MAX VERT. LOAD LC1	AX. FACTORED FACTORED VERT. LOAD LC1 MAX MAX. MEMB (PLF) CSI (LC) UNBRAC UNBRAC CSI (LC) UNBRAC UNBRAC CSI (LC) UNBRAC UNBRAC	MAX. FACTORED FACTORED FORCE CIBS FORCE CIBS (PLF CSI (LC) UNBRAC (LBS)	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	25-0-0	-367	-367		FRONT	VERT	TOTAL		C1
Ρ	17-0-12	-1631	-1631		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: TOP CH. LL =

		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE ADDT'L LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 13-11-4 OF SPAN MEASURED FROM THE RIGHT.

NON STANDARD GIRDER ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT)

CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL) = L/651 (0.57")

CSI: TC=0.41/0.97 (G-H:1) , BC=0.64/0.97 (P-R:1) , WB=0.95/0.97 (I-O:1) , SSI=0.34/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

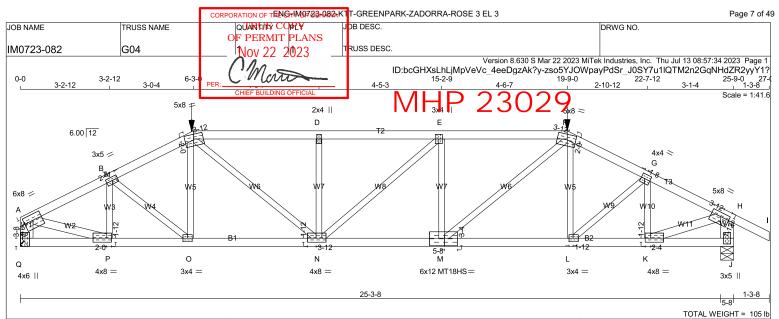


		CORPORATION OF TENG+1M0723+0824	K1 T-GREENPAF	RK-ZADORRA-ROSE	3 EL 3	Page 6 of 49
JOB NAME	TRUSS NAME	QUANTITY COPY OF PERMIT PLANS	JDB DESC.			DRWG NO.
IM0723-082	G03	Nov 22 2023 _	TRUSS DESC.			
		Maria		ID:bcGHXsLhLj		MiTek Industries, Inc. Thu Jul 13 08:57:33 2023 Page 2 LzNu2HqZ?JGomJxJagUfr3?82Zlg8du?vbyyY10
		PER: CHIEF BUILDING OFFICIAL	ا ل	МНР	23029	NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
						(PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 MT18HS 586 403 2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (R) (INPUT = 0.90) JSI METAL= 0.99 (Q) (INPUT = 1.00)







LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	2100F 1.8E	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - A	2x6	DRY	No.2	SPF
Q - M	2x4	DRY	2100F 1.8E	SPF
M - J	2x4	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - H	2x4	DRY	No.2	SPF
A - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X					
Α	TMVW-t	MT20	6.0	8.0	2.50	4.00					
В	TMWW-t	MT20	3.0	5.0	1.50	2.25					
С	TTWW-m	MT20	5.0	8.0	2.00	3.75					
D	TMW+w	MT20	2.0	4.0							
Е	TMWW+t	MT20	3.0	4.0							
F	TTWW-m	MT20	5.0	8.0	2.00	3.75					
G	TMWW-t	MT20	4.0	4.0	2.00	1.50					
Н	TMVW-t	MT20	5.0	8.0	1.75	3.75					
J	BMV1+p	MT20	3.0	5.0							
K	BMWW-t	MT20	4.0	8.0	1.75	2.25					
L	BMWW-t	MT20	3.0	4.0	1.50	1.75					
M	BSWW-I	MT18HS	6.0	12.0	3.25	5.50					
N	BMWWW-t	MT20	4.0	8.0	1.75	3.75					
0	BMWW-t	MT20	3.0	4.0							
Ρ	BMWW-t	MT20	4.0	8.0	1.75	2.00					
Q	BMV1+t	MT20	4.0	6.0	3.50						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTOR	ED	MAXIMUN	/ FACTO	INPUT	REQRD	
	GROSS RE	ACTION	TION GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	3450	0	3450	0	0	MECHANIC	CAL
J	3628	0	3628	0	0	5-8	4-2

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT Q. MINIMUM BEARING LENGTH AT JOINT Q = 3-8.

UNFACTORED REACTION	1	V.	3	

1	151 LUASE	IVIAA./	WIIN. COMPO	NEINT REACTION	<i>N</i> O		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2414	1729 / 0	0/0	0/0	0/0	686 / 0	0/0
J	2536	1832 / 0	0/0	0/0	0/0	704 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

C H	ORDS				W E	BS	
MAX	X. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	I FR-TO	1	
A-B	-4926 / 0	-119.4 -119.4	0.52 (1)	2.67	P-B	-935 / 0	0.17 (1)
B- C	-5378 / 0	-119.4 -119.4					0.12(1)
C- D	-6338 / 0	-246.3 -246.3					0.04 (4)
D-E	-6338 / 0	-246.3 -246.3					0.50 (1)
E-F	-6290 / 0	-246.3 -246.3				-1187 / 0	0.33 (1)
F-G	-5223 / 0	-119.4 -119.4				0 / 64	0.02 (4)
G- H	-4649 / 0	-119.4 -119.4				-1238 / 0	0.34 (1)
H- I	0 / 36					0 / 2141	0.53 (1)
J- H		0.0 0.0				-228 / 102	0.06 (1)
Q- A	-3376 / 0	0.0 0.0	0.24 (1)	5.66		0 / 642	0.16 (1)
						-1091 / 0	0.20 (1)
Q-P		-37.6 -37.6				0 / 4325	0.76 (1)
P- 0	0 / 4417		0.42 (1)		A-P	0 / 4529	0.80 (1)
O- N	0 / 4796		0.46 (1)				
N- M	0 / 6290		0.60 (1)				
M-L	0 / 4661		0.45 (1)				
L-K		-37.6 -37.6					
K-J	0/0	-37.6 -37.6	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

Т	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
;	6-3-0	-404	-404		FRONT	VERT	TOTAL		C1
	19-9-0	-388	-388		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



SPECIFIED LOADS:								
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTA	L LO	AD	=	48.1	PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip LEFT SETBACK = 6-3-0 RIGHT SETBACK = 6-0-0 END SETBACK = 6-3-0 END WALL WIDTH = 0-0 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14

- TPIC 2014

(55 % OF 48 1 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86") CALCULATED VERT. DEFL.(LL)= L/999 (0.23") ALLOWABLE DEFL.(TL)= L/360 (0.86") CALCULATED VERT. DEFL.(TL)= L/761 (0.41")

CSI: TC=0.74/0.97 (E-F:1) , BC=0.60/0.97 (M-N:1) , WB=0.80/0.97 (A-P:1) , SSI=0.57/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 MT18HS 586 403 2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90) JSI METAL= 0.89 (A) (INPUT = 1.00)





RUSS DESC

2-8-12

IM0723-082 G05

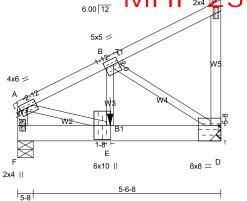
OF PERMIT PLANS Nov 22 2023

Version 8,630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:57:35 2023 Page 1 ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-R2MTmfP8au4GFcQAtkznf5Z2fsifWYtzbxN6zUyyY1 6-0-0

6.00 12 5x5 / R

Scale = 1:34.0

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LUMBER N. L. G. A. RULES CHORDS F - A LUMBER DESCR. SPF F - A A - C D - C DRY 2x4 No.2 No.2 No.2 DRY D DRY No.2 SPF ALL WEBS SPF 2x3 DRY No.2 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORD	S #ROWS	SURFACE	LOAD(PLF)						
		SPACING (IN)							
TOP CH	TOP CHORDS : (0.122"X3") SPIRAL NAILS								
F- A	1	12	TOP						
A- C	1	12	SIDE(53.4)						
C- D	1	12	TOP						
BOTTOM	M CHORDS	: (0.122"X3") SPIRAL NAILS							
F- D	2	6	SIDE(400.8)						
WEBS : (0.122"X3") SPIRAL NAILS									
242	` 4	e							

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING. PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
Α	TMVW-t	MT20	4.0	6.0	1.75	2.75
В	TMWW-t	MT20	5.0	5.0	2.00	1.75
С	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	8.0	8.0	5.50	Edge
Ε	BMWW+t	MT20	6.0	10.0	5.00	1.50
	DM//1+n	MTOO	20	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DEAL	KINGS						
	FACTOR	ED	MAXIMUN	MAXIMUM FACTORED			REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	3301	0	3301	0	0	5-8	1-14
ח	3977	0	3977	0	0	MECHANIC	:AI

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	: <u>MAX./N</u>	<u> 11N. COMPO</u>	NENT REACTION	<u> </u>		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2307	1669 / 0	0/0	0/0	0/0	638 / 0	0/0
D	2779	2011 / 0	0/0	0/0	0/0	768 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

ORDS					VV E	- B S		
. FACTORED	FACTO	RED				MAX. FACTO	RED	
FORCE	VERT. LO	AD LC1	MAX	MAX.	MEMB	. FORCE	MAX	
(LBS)	(PL	.F) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
	FROM	TO		LENGTH	FR-TO			
-3279 / 0	0.0	0.0	0.18 (1)	6.35	A-E	0 / 3964	0.49 (1)	
-4247 / 0	-238.9	-238.9	0.20(1)	4.42	E-B	0 / 3813	0.47 (1)	
-13 / 0	-119.4	-119.4	0.09(1)	6.25	B- D	-4556 / 0	0.59 (1)	
-165 / 0	0.0	0.0	0.02 (1)	7.81				
0/0				10.00				
0 / 3813	-819.9	-819.9	0.64 (1)	10.00				
	(LBS) -3279 / 0 -4247 / 0 -13 / 0 -165 / 0	FACTORED FACTOR FORCE (LBS) FROM -3279 / 0 -238.9 -13 / 0 -119.4 -165 / 0 0.0 0 0/0 -36.5	FACTORED FORCE (LBS) (PLF) FROM TO 0.0 0.0 4247 / 0 -238.9 -238.9 -313 / 0 -119.4 -119.4 -165 / 0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	FACTORED FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) FROM TO -3279 / 0 -238.9 -238.9 0.20 (1) -13 / 0 -119.4 -119.4 0.09 (1) -165 / 0 0.0 0.0 0.02 (1) 0 / 0 -36.5 -36.5 0.03 (1)	FACTORED FACTORED	FACTORED FACTORED	FACTORED FACTORED	FACTORED FACTORED

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-8-12	-2414	-2414		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

TOTAL WEIGHT = 2 X 29 = 58 lb

SPECIFIED LOADS:

TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 2-8-12 START SPAN CARRIED = 25-9-0 END DISTANCE = 6-0-0 END SPAN CARRIED = 25-9-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 2-8-12 OF SPAN MEASURED FROM THE LEFT.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.20/0.97 (A-B:1) , BC=0.64/0.97 (D-E:1) , WB=0.59/0.97 (B-D:1) , SSI=0.45/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



		CORPORATION OF TENGTIM0723H082A	KTT-GREENPARK-ZADORRA-RO	OSE 3 EL 3 Page 9 of 49
JOB NAME	TRUSS NAME	QUANTITY COPY	JDB DESC.	DRWG NO.
00011711112	111000111111111111111111111111111111111	OF PERMIT PLANS		Simo no.
IM0723-082	G05	Nov 22 2 0 23 _	TRUSS DESC.	
				Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:57:35 2023 Page 2
		(Maxin-	ID:bcGHXsLh	LjMpVeVc_4eeDgzAk?y-R2MTmfP8au4GFcQAtkznf5Z2fsifWYtzbxN6zUyyY1_

CHIEF BUILDING OFFICIAL

MHP 23029

NAIL VALUES
PLATE GRIP(DRY) SHEAR (PLI)
(PS) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.57 (E) (INPUT = 1.00)





LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
1 - L	2x4	DRY	No.2	SPF
W - A	2x4	DRY	No.2	SPF
W-P	2x8	DRY	No.2	SPF
P - K	2x8	DRY	No.2	SPF
REINFORCI	NG MEN	IBERS		
HW2	2x8	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS

CHORDS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHO		2"X3") SPIRAL NAILS	
A- C	1	12	TOP
C-F	1	12	TOP
F- G	1	12	TOP
G-I	1	12	TOP
I- L	1	12	TOP
W- A	1	12	TOP
BOTTOM	CHORDS: (0.122"X3") SPIRAL NAILS	
W-P	2	12	SIDE(61.0)
P-K	2	12	TOP `
WEBS: (0).122"X3") SF	PIRAL NAILS	
2x3	1	6	
2x8	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER**

	111100						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	6026	0	6026	0	0	MECHANIC	AL
K	3004	0	3004	0	0	5-8	1-10

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT W. MINIMUM BEARING LENGTH AT JOINT W = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	<u>иім. сомро</u>	NENT REACTION	NS .		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4212	3047 / 0	0/0	0/0	0/0	1164 / 0	0/0
K	2097	1532 / 0	0/0	0/0	0/0	565 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	HORDS				W E	BS		
M/	AX. FACTORED					MAX. FACT	ORED	
MEME	B. FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TC)	FROM TO		LENGTH	I FR-TO			
A-B	-4856 / 0	-119.4 -119.4	0.12(1)	4.26	V- B	-3610 / 0	0.34	(1)
B- C	-6859 / 0	-119.4 -119.4	0.18 (1)	3.59	B- U	0 / 3021	0.37	(1)
C- D	-7102 / 0	-119.4 -119.4	0.19 (1)	3.53	U- C	-469 / 0	0.06	
D-E	-7834 / 0	-119.4 -119.4	0.30 (1)	3.27	C- T	0 / 3440	0.43	(1)
E-F	-7743 / 0	-119.4 -119.4	0.29 (1)	3.30	T- D	-973 / 0	0.12	(1)
F- G	-5719 / 0	-119.4 -119.4	0.25 (1)	3.85	D-S	0 / 1125	0.14	(1)
G- H	-4682 / 0	-119.4 -119.4	0.19 (1)	4.25	S-E	-334 / 0	0.04	(1)
H- I	-4682 / 0	-119.4 -119.4	0.19 (1)	4.25	E-R	-143 / 0	0.03	(1)
I- J	-4304 / 0	-119.4 -119.4	0.27 (1)	4.33	R-F	-94 / 21	0.01	(1)
J-Y	-4348 / 0	-119.4 -119.4	0.27 (1)	4.31	F-Q	-3870 / 0	0.81	
Y-K	-3082 / 0	-119.4 -119.4	0.11 (1)	5.13	Q- G	0 / 2958	0.37	(1)
K-L	-1 / 0	-119.4 -119.4	0.08 (1)	10.00	G- 0	-916 / 0	0.31	(1)
W-A	-5807 / 0	0.0 0.0	0.35 (1)	4.92	O- H	-468 / 0	0.11	(1)
					O- I	0 / 1590	0.20	(1)
W-V	0/0	-140.2 -140.2	0.04(1)	10.00	N- I	0 / 184	0.02	(1)
V- U	0 / 4346	-140.2 -140.2	0.35 (1)	10.00	N- J	-89 / 0	0.02	(1)
U- T	0 / 6108	-140.2 -140.2	0.57 (1)	10.00	M- J	-526 / 0	0.05	(1)
T-S	0 / 7102	-18.2 -18.2	0.63 (1)	10.00	A- V	0 / 5589	0.69	(1)
S-R	0 / 7834	-18.2 -18.2	0.55 (1)	10.00	M-Y	0 / 2358	0.13	(1)
R-Q	0 / 7745	-18.2 -18.2	0.54 (1)	10.00	X-Y	-104 / 21	0.00	(1)
Q-P	0 / 5173	-18.2 -18.2	0.31 (1)	10.00				
P- 0	0 / 5173	-18.2 -18.2	0.31 (1)	10.00				
O- N	0 / 3829	-18.2 -18.2	0.22 (1)	10.00				
N- M	0 / 3900	-18.2 -18.2	0.25 (1)	10.00				
M- X	0 / 1603	-18.2 -18.2	0.13 (1)	10.00				
X-K	0 / 1603	-18.2 -18.2	0.11 (1)	10.00				
			50)					
		RATED LOADS (L			ND.	T)/DE		0011
JT	LOC. LC			ACE [DIR.	TYPE	HEEL	CON

FRONT

VERT

TOTAL

CONN.

CONNECTION REQUIREMENTS

-2779

LOC. 5-1-7

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

-2779

DESIGN CRITERIA

TOTAL LOAD

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: 34.8 DI = 6.0 PSF LL = DL = 0.0 7.3 BOT CH. PSF

= 48.1

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 6-0-0 END DISTANCE = 5-1-7 END SPAN CARRIED = 6-0-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.

ADDT'L LOADS BASED ON 55 % OF GSL.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14 - TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (1.03") CALCULATED VERT. DEFL.(LL)= L/999 (0.16") ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.35/0.97 (A-W:1) , BC=0.63/0.97 (S-T:1) , WB=0.81/0.97 (F-Q:1) , SSI=0.45/1.00 (T-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873



CHIEF BUILDING OFFICIAL

PLATES (table is in inches)
JT TYPE PLATES
A TMVW+t MT20 LEN Y X 6.0 1.75 Edge 5.0 2.00 1.50 8.0 2.50 2.75 4.0 4.0 6.0 3.25 2.25 6.0 2.25 2.50 4.0 Edge 3.50 4.0 1.50 1.75 8.0 1.50 4.00 6.0 2.25 2.00 W 5.0 4.0 6.0 3.0 5.0 5.0 5.0 5.0 TMWW+t TTWW+m TMWW-t MT20 MT20 MT20 TMWW-t TTWW+m MT20 MT20 TTWW-m TMW+w TTWW+m MT20 MT20 MT20 MT20 J TTWW+m
J TMWW-t
K TMBMW1-i
M BMWW+t
N, R, S
N BMWW-t
O BMWWW-i TMWW-t TMBMW1-m MT20 MT20 6.0 4.0 8.0 8.0 MT20 4.0 2.25 2.00 BMWWW-t BS-t MT20 MT20 5.0 BMWW+t BMWW+t 4.0 4.0 4.0 5.0 6.0 8.0 5.0 6.0 Q T MT20 MT20 2.50 1.75 MT20 MT20 MT20 MT20 5.0 2.50 1.50 6.0 2.50 1.50 10.0 7.25 BMWW+t BMWW+t BMV1+t 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

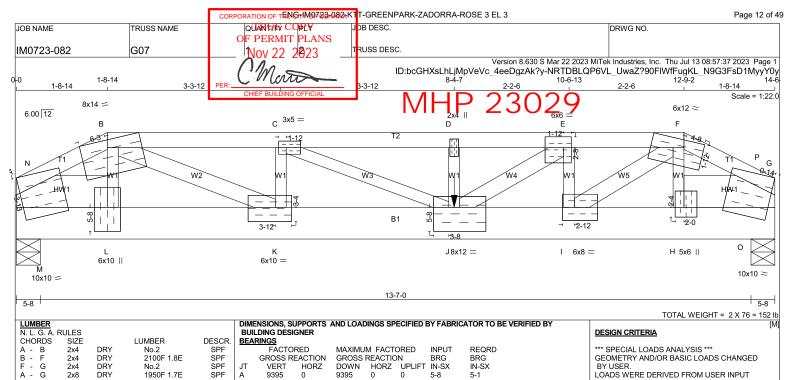
MHP 23029

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90) JSI METAL= 0.86 (A) (INPUT = 1.00)







LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	2100F 1.8E	SPF
F - G	2x4	DRY	No.2	SPF
A - G	2x8	DRY	1950F 1.7E	SPF
				_
REINFORCII	NG MEM	BERS		
HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF
				• •
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT	LAC	D	110.2	O
B - K	2x4	DRY	2100F 1.8F	SPF
J - F	2x4	DRY	No.2	SPF
i - F	2x4	DRY	No.2	SPF
	2.4	Ditti	140.2	011

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS

CHORDS	#ROWS	SURFACE	LOAD(PLF)
	:	SPACING (IN)	
TOP CHO	RDS: (0.122	"X3") SPIRAL NAILS	
A- B	1	12	TOP
B- F	1	12	TOP
F- G	1	12	TOP
воттом	CHORDS: (0.122"X3") SPIRAL NAILS	
A- G	2	5	SIDE(491.2)
WEBS: (0).122"X3") SF	PIRAL NAILS	
2x3	1	6	
2x4	1	6	
2x8	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
4	9395	0	9395	0	0	5-8	5-1
G	6850	0	6850	0	0	5-8	3-11

UNFACTORED REACTIONS

	151 LUASE	. IVIAX./N	JIIN. COMPO	NENT REACTION	VS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Α	6566	4751 / 0	0/0	0/0	0/0	1815 / 0	0/0
G	4787	3464 / 0	0/0	0/0	0/0	1323 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A. G

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	HORDS		WEBS					
		FACTORED				MAX. FACT		
MEMB.	FORCE	VERT. LOAD LO					MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC))
FR-TO		FROM TO		LENGTH	I FR-TO			
A- N	-8637 / 0	-119.4 -119.4	0.25 (1)	3.16	L- B	-186 / 0	0.01(1)	
N- B	-11364 / 0	-119.4 -119.4	0.42 (1)	2.55	B-K	0 / 11567	7 0.48 (1)	
B- C	-20573 / 0	-119.4 -119.4	0.58 (1)	2.27	K- C	-1489 / 0	0.12 (1)	
C- D	-22820 / 0	-119.4 -119.4			C- J	0 / 2491	0.31 (1)	
D- E	-22820 / 0	-119.4 -119.4	0.70 (1)	1.89	J- D	-142 / 2	0.01 (1)	
E-F	-15776 / 0	-119.4 -119.4	0.31 (1)	2.91	J- E	0 / 8695	0.77 (1)	
F-P	-9025 / 0	-119.4 -119.4	0.26 (1)	3.08	I- E	-5378 / 0	0.42 (1)	
P- G	-6700 / 0	-119.4 -119.4			I- F	0 / 9675	0.86 (1)	
			` '		H- F	-1857 / 0	0.15 (1)	
A- M	0 / 4224	-1000.6-1000.6	0.20 (1)	10.00	M- N	-2171 / 0		
M- L	0 / 4224	-1000.6-1000.6	0.24 (1)	10.00	N- L	0 / 7422	0.40 (1)	
L- K	0 / 10131	-1000.6-1000.6	0.37 (1)	10.00	0- P	-1951 / 0	0.00 (1)	
K- J	0 / 20573	-1000.6-1000.6	0.88 (1)	10.00	P- H	0 / 5949	0.32 (1)	
J- I	0 / 15776	-18.2 -18.2	0.72 (1)	10.00			. ,	
I- H	0 / 7923	-18.2 -18.2	0.28 (1)	10.00				
H- O	0 / 3301	-18.2 -18.2						
0- G	0 / 3301	-18.2 -18.2	· 0.14 (1)	10.00				
SPECI	FIED CONCENTE	RATED LOADS (L	.BS)					
IT.	100 101	MANY MANY	v	ACE 5	מוס	TVDE	HEEL C	ON

FACE

FRONT

DIB

VERT

TYPE

TOTAL

HEEL

CONN.

C1

LC1 -4212 MAX--4212





READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

NO FURTHER MODIFICATIONS WERE MADE

34.8 6.0 0.0 7.3

PSF

PSF PSF

SPECIFIED LOADS:

BOT CH. TOTAL LOAD

SPACING =

DI =

LL = DL =

= 48.1

24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0

ALL LOAD CASES.

- TPIC 2014

START SPAN CARRIED = 31-0-0 END DISTANCE = 8-4-7

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

END SPAN CARRIED = 31-0-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- ADDT'L LOADS BASED ON 55 % OF GSL. *** NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL)= L/764 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/446 (0.39")

CSI: TC=0.79/0.97 (C-D:1) , BC=0.88/0.97 (J-K:1) , WB=0.86/0.97 (F-I:1) , SSI=0.70/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873



CORPORATION OF TENGTIMO70310824K1 T-GREENPARK-ZADORRA-ROSE 3 EL 3 Page 13 of 49 QUANTINY COPY OF PERMIT PLANS JOB NAME TRUSS NAME DB DESC. DRWG NO. IM0723-082 G07 Nov 22 2023 RUSS DESC. Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:57:37 2023 Page 2 ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-NRTDBLQP6VL_UwaZ?90FlWfFugKL_N9G3FsD1MyyY0y

CHIEF BUILDING OFFICIAL

PLATES (table is in inches)
JT TYPE PLATES
A TMBMW1-m MT20 W LEN Y X
10.0 10.0 3.75 Edge
8.0 14.0 Edge 6.25
3.0 5.0 1.50 1.75
6.0 6.0 2.50 1.75
6.0 12.0 1.75 4.50
10.0 10.0 Edge 1.00
5.0 6.0 2.25 2.00
6.0 8.0 3.00 2.75
8.0 12.0 5.50 3.50
6.0 10.0 3.25 3.75
6.0 10.0 5.50 3.00 TTWW-m TMWW-t TMW+w MT20 MT20 MT20 TMWW-t TTWW-m MT20 MT20 G TMBMW1-m BMWW+t BMWW-t MT20 MT20 MT20 MT20 BMWWW-t BMWW-t MT20 MT20

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

10.0 5.50 3.00

BMWW+t

MHP 23029

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (K) (INPUT = 0.90) JSI METAL= 0.89 (L) (INPUT = 1.00)



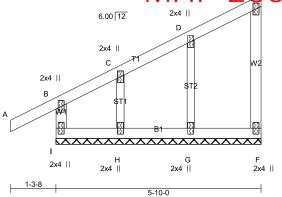


CORPORATION OF TENGTIMO7031-0824KTT-GREENPARK-ZADORRA-ROSE 3 EL 3 Page 14 of 49 TRUSS NAME QUARTERY COPLY DB DESC JOB NAME DRWG NO. OF PERMIT PLANS IM0723-082 Nov 22 2023 RUSS DESC. GE01

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:57:38 2023 Page 1 ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-rd1bOhR1tpTr649lYsXUHkBaX4sbj1?QHvbmZpyyY0x 5-10-0 6-9-8 5-10-0

Scale = 1:32.8

TOTAL WEIGHT = 24 lb



LUMBER N. L. G. A. RULES DESCR. SPF SPF SPF CHORDS SIZE LUMBER I - B A - E F - E 2x4 2x4 2x4 DRY No.2 No.2 No.2 DRY - F 2x4 DRY No.2 SPF ALL WEBS 2x3 SPF DRY No.2 ALL GABLE WEBS No.2 SPF DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	Х
В	TMV+p	MT20	2.0	4.0		
С	TMW+w	MT20	2.0	4.0		
D	TMW+w	MT20	2.0	4.0		
Ε	TMV+p	MT20	2.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
Н	BMW1+w	MT20	2.0	4.0		
1	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

lotte

CHIEF BUILDING OFFICE

ı	CHO	ORDS				WE	BS	
ı	MAX.	FACTORED	FACTORED				MAX. FACTO	RED
	MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
ı		(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
ı	FR-TO		FROM TO		LENGTH	FR-TO		
ı	I- B	-318 / 0	0.0 0.0	0.04(1)	7.81	G- D	-273 / 0	0.05 (1)
ı	A- B	0 / 36	-119.4 -119.4	0.16(1)	10.00	H- C	-165 / 0	0.02(1)
ı	B- C	-38 / 0	-119.4 -119.4	0.13(1)	6.25			
ı	C- D	-5 / 0	-119.4 -119.4	0.07(1)	10.00			
ı	D- E	-13 / 0	-119.4 -119.4	0.07(1)	6.25			
ı	F-E	-103 / 0	0.0 0.0	0.02(1)	7.81			
ı								
ı	I- H	0 / 17	-18.2 -18.2	0.05(1)	10.00			
ı	H- G	0 / 11	-18.2 -18.2	0.02(4)	10.00			
	G-F	0/4	-18.2 -18.2	0.02(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS: LL = 3 DL = LL = PSF PSF PSF 34.8 6.0 TOP CH. BOT CH. 0.0 7.3 TOTAL LOAD 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14 - TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/0.97 (A-B:1) , BC=0.05/0.97 (H-I:1) , WB=0.05/0.97 (D-G:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

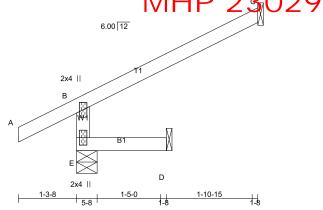
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90) JSI METAL= 0.15 (B) (INPUT = 1.00)









REQRD

BRG IN-SX

1-8

1-8

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER LUMBER N. L. G. A. RULES DESCR. SPF SPF CHORDS SIZE LUMBER E - B A - C E - D DRY FACTORED MAXIMUM FACTORED 2x4 No.2 No.2 No.2 GROSS REACTION VERT HORZ GROSS REACTION DOWN HORZ L DRY BRG UPLIFT

483

0

E C D SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C. D

0

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

	IOI LUAUL	IST LOAGE WAX./WIN. COM C			10		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	333	270 / 0	0/0	0/0	0/0	63 / 0	0/0
С	124	105 / 0	0/0	0/0	0/0	18 / 0	0/0
D	13	0/0	0/0	0/0	0/0	13 / 0	0/0

0

5-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

483

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CH	HORDS		WEBS						
MA	X. FACTORED	FACTORED	FACTORED			MAX. FACTORED			
MEMB	. FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC)	(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO	` '	, ,		
E-B	-463 / 0	0.0 0.0	0.01(4)	7.81					
A-B	0 / 36	-119.4 -119.4	0.16(1)	10.00					
B- C	-27 / 0	-119.4 -119.4							
E- D	0/0	-18.2 -18.2	0.02(4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS PSF PSF PSF 34.8 6.0 TOP CH. = BOT CH. 0.0 7.3 LL TOTAL LOAD 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 3 X 10 = 30 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL)= L/999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.33/0.97 (B-C:1) , BC=0.02/0.97 (D-E:4) , WB=0.00/0.97 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)



DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES

MT20

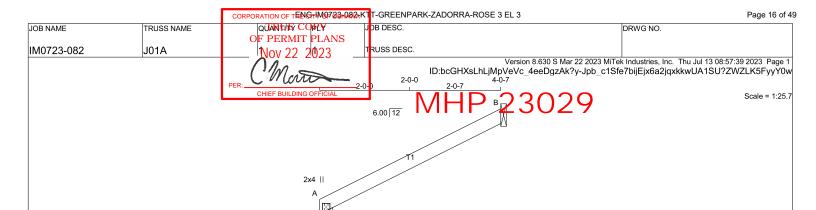
TMV+p

BMV1+p

LEN Y

2.0





LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
DRY: SEAS	ONED LI	JMBER.		

PLATES (table is in inches)
JT TYPE PLATES LEN Y 2.0 TMV+p MT20 4.0 BMV1+p

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

1-10-15

	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD		
	GROSS R	EACTION	GROSS REACTION			BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
D	229	0	229	0	0	MECHAI	VICAL		
В	212	0	212	0	0	1-8	1-8		
С	78	0	78	0	0	1-8	1-8		

1-10-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

D 2x4 ||

	1ST LCASE	MAX./	<u>MIN. COMPON</u>	NS .			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	159	123 / 0	0/0	0/0	0/0	36 / 0	0/0
В	145	123 / 0	0/0	0/0	0/0	22 / 0	0/0
С	55	35 / 0	0/0	0/0	0/0	20 / 0	0/0

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX. MEMI	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH FR-TO	o ' '		
D- A	-270 / 0	0.0 0.0	0.14(1)	7.81			
A-B	-13 / 0	-119.4 -119.4	0.24 (1)	6.25			
D-C	0/0	-18.2 -18.2	0.16(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS: LL = 3 DL = LL = PSF PSF PSF 34.8 6.0 TOP CH. 0.0 7.3 BOT CH. TOTAL LOAD 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 8 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24/0.97 (A-B:1) , BC=0.16/0.97 (C-D:1) , WB=0.00/0.97 (n/a:0) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90) JSI METAL= 0.11 (A) (INPUT = 1.00)



