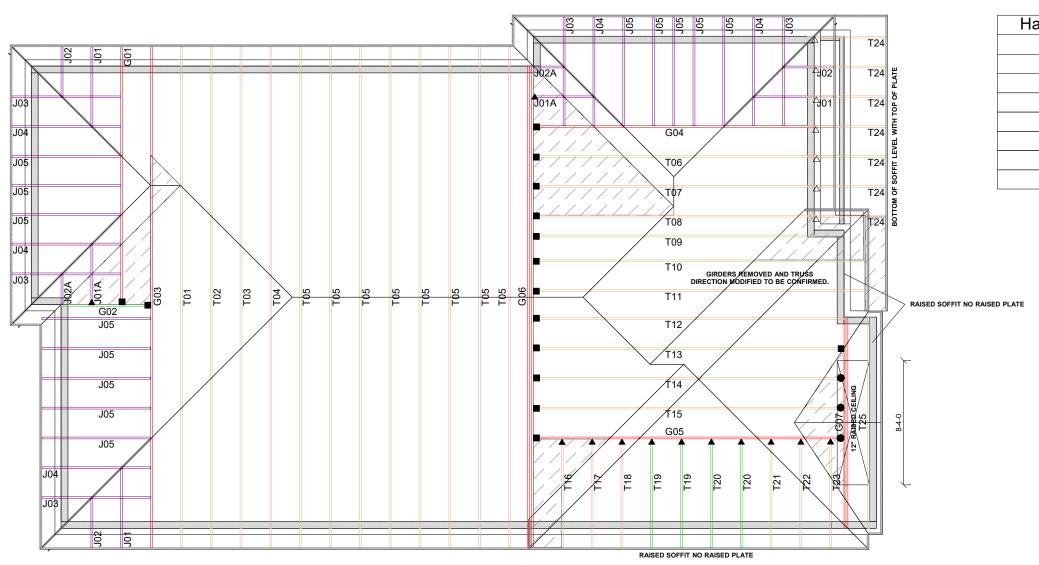


MHP 23030



Hanger Name	Symbol	QTY
LUS24		12
LJS26DS		15
HGUS26		3
		0
H2.5A	Δ	7
		0
	\bigcirc	0
	0	0

CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB IN	FORMATION
Customer	GREENPARK HOMES
Job #	23-00117R0
	ZADORRA ESTATES
Address	ROSE 6 EL 3
	OSHAWA,ON
Model	ROSE 6 EL 3
Sales Rep	RALPH MIRIGELLO
Designer	ВВ
Date	6/7/2023
Path	S:\DESIGN\KLU\CUSTOMERS\GREENPARK\ZADORRA ESTATES\MODELS\ROSE 6\ROSE 6-3\T-ROSE

DESIGN	DESIGN INFORMATION						
Code	NBCC 2015						
Bldg	Residential - HSB (NBCC Part 9)						
TC LL	34.8 lb/ft²						
TC DL	6.0 lb/ft²						
BC LL	0.0 lb/ft²						
BC DL	7.3 lb/ft²						
Deflection	LL=L/360 TL=L/360						
Spacing	24" O/C unless otherwise						
	Hoted						
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014						

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400





Engineering Notes: Trusses



PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER.

IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

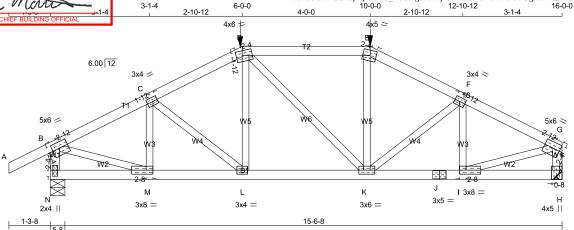
- 1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
- 2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
- 3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
- 4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

- 1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
- 2. THE COMPRESSION CHORDS ARE LATERALLY BRACED BY CONTINUOUS RIGID DIAPHRAGM SHEATHING OR AS SPECIFIED ON THE DRAWING.
- 3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE BUILDING DESIGNER.
- 4. IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES SPANNING MORE THAN 12.37M (40'-7").



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLA	ATES (table					
JT	TYPE	PLATES	W	LEN	Y X	
В	TMVW-t	MT20	5.0	6.0	2.25 2.75	
С	TMWW-t	MT20	3.0	4.0	1.50 1.75	
D	TTWW-m	MT20	4.0	6.0	1.75 2.25	
Е	TTW-m	MT20	4.0	5.0	2.00 2.25	
F	TMWW-t	MT20	3.0	4.0	1.50 1.75	
G	TMVW-t	MT20	5.0	6.0	2.25 Edge	Э
Н	BMV1+t	MT20	4.0	5.0	Edge 0.50	
1	BMWW-t	MT20	3.0	8.0	1.50 2.50	
J	BS-t	MT20	3.0	5.0		
K	BMWWW-t	MT20	3.0	6.0		
L	BMWW-t	MT20	3.0	4.0		
M	BMWW-t	MT20	3.0	8.0	1.50 2.50	
Ν	BMV1+p	MT20	2.0	4.0	2.25 1.00	

INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUN	/ FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2142	0	2142	0	0	5-8	3-7
+	1980	0	1980	0	0	MECHANIC	CAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	151 LUASE	IVIAX./I	VIIN. COMPO	NEINT REACTION	VO		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1496	1088 / 0	0/0	0/0	0/0	408 / 0	0/0
Н	1385	994 / 0	0/0	0/0	0/0	391 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	ORDS C. FACTORED	FACTO	DED			WE	B S MAX. FACTO	NDED.	
				B 4 A V	NAA.V	NAC-NAD			
MEMB.								MAX	
	(LBS)				UNBRAC		(LBS)	CSI (LC)	
FR-TO					LENGTH				
A- B	0 / 36	-119.4	-119.4	0.17 (1)	10.00	M- C	-537 / 0	0.10 (1)	
B- C	-2537 / 0	-119.4	-119.4	0.24(1)	4.05	C- L	-11 / 104	0.03(1)	
C- D	-2660 / 0	-119.4	-119.4	0.25(1)	3.96	L- D	0 / 141	0.05 (4)	
D- E	-2365 / 0	-225.2	-225.2	0.80(1)	3.05	D- K	0/0	0.00 (1)	
	-2660 / 0				3.96	K-E	0 / 142	0.05 (4)	
	-2537 / 0							0.03(1)	
	-2087 / 0				5.74			0.10 (1)	
H- G	-1925 / 0			0.21 (1)			0 / 2367	0.59 (1)	
•	102070	0.0	0.0	0.2.(1)	0.0 .		0 / 2367	0.59 (1)	
N- M	0/0	-34.4	-34 4	0.08 (4)	10.00		0 / 200 /	0.00 (1)	
M- L		-34.4		0.45 (1)					
L- K	0 / 2365	-34.4		0.47 (1)					
K- J		-34.4		0.45 (1)					
J- I		-34.4		0.45 (1)					
I- H	0/0	-34.4	-34.4	0.08 (4)	10.00				
SPECIF	SPECIFIED CONCENTRATED LOADS (LBS)								

	LOC.			FACE	DIR.	TYPE	HEEL	CONN.
D	6-0-0	-367	-367	 FRONT	VERT	TOTAL		C1
E	10-0-0	-367	-367	 FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:									
TOP	CH.	LL =	34.8	PSF					
		DL =	6.0	PSF					
BOT	CH.	LL =	0.0	PSF					
		DL =	7.3	PSF					
TOTA	L LO	AD =	48.1	PSF					

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

Page 2 of 47

Scale = 1:36.0

TOTAL WEIGHT = 65 lb

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENEND JACK TYPE: CONVENTIONAL CONVENTIONAL

APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.53") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.53") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.11")

CSI: TC=0.80/0.97 (D-E:1) , BC=0.47/0.97 (K-L:1) , WB=0.59/0.97 (B-M:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90) JSI METAL= 0.70 (J) (INPUT = 1.00)





IM0723-093

Oct 909 2023

ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

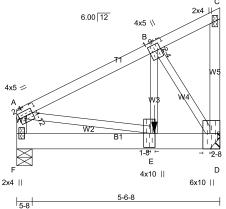
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JOB DESC. QUANTITY TRUSS DESC.

MHP 23030 Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Fri Jul 14 07:35:50 2023 Page 1 ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-nL5xndf5OEb4ne1sSeYf5lLtE?vsQCAKLS2zoPyyE7d

0-0 4-0-12 6-0-0 1-11-4 4-0-12 С

Scale = 1:34.0



TOTAL WEIGHT = 30 lb

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASO	ONED L	UMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	w	LEN	Υ	Х
Α	TMVW-t	MT20	4.0	5.0	1.75	2.25
В	TMWW+t	MT20	4.0	5.0	2.25	1.00
С	TMV+p	MT20	2.0	4.0		
D	BMVW1+t	MT20	6.0	10.0	Edge	2.50
Е	BMWW+t	MT20	4.0	10.0	5.00	1.50
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u> </u>	<u> </u>								
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD		
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG		
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
=	1423	0	1423	0	0	5-8	1-9		
)	1944	0	1944	0	0	MECHANIC	AL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	994	720 / 0	0/0	0/0	0/0	275 / 0	0/0		
D	1359	983 / 0	0/0	0/0	0/0	376 / 0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

MAX. FACTORED MAX. FACTORED MAX. FACTORED	
MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. FORCE MAX	
(LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC)	
FR-TO FROM TO LENGTH FR-TO	
F-A -1303 / 0 0.0 0.0 0.14 (1) 6.96 A-E 0 / 1319 0.33 (1)	
A-B -1414/0 -238.9 -238.9 0.58 (1) 4.53 E-B 0/1683 0.42 (1)	
B-C -73/0 -119.4 -119.4 0.47 (1) 6.25 B-D -2280/0 0.50 (1)	
D-C 0/47 0.0 0.0 0.01 (1) 10.00	
F-E 0/0 -36.5 -36.5 0.15(1) 10.00	
E-D 0 / 1296 -18.2 -18.2 0.31 (1) 10.00	
SPECIFIED CONCENTRATED LOADS (LBS)	
	NN.
E 4-0-12 -1385 -1385 FRONT VERT TOTAL C	1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: CH. LL = DL = 34.8

DL = 6.0 LL = 0.0 DL = 7.3 PSF BOT CH. = 48.1 TOTAL LOAD

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip SIDE SETBACK = 0-0 END SETBACK = 6-0-0 END WALL WIDTH = 0-0 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-0-12 OF SPAN

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.58/0.97 (A-B:1), BC=0.31/0.97 (D-E:1), WB=0.50/0.97 (B-D:1), SSI=0.43/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



CORPORATION OF THE CITY OF OSHAWA JOB NAME TRIFFEUSSARME QUANTITY OF PERMIT PLANS IM0723-093 Oct 909 2023

ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

JOB DESC.

TRUSS DESC.

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Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Fri Jul 14 07:35:50 2023 Page 2
ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-nL5xndf5OEb4ne1sSeYf5ILtE?vsQCAKLS2zoPyyE7d

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90) JSI METAL= 0.58 (E) (INPUT = 1.00)





LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	2100F 1.8E	SPF
D - H	2x6	DRY	2100F 1.8E	SPF
H - I	2x6	DRY	2100F 1.8E	SPF
1 - L	2x6	DRY	2100F 1.8E	SPF
B - R	2x6	DRY	2100F 1.8E	SPF
R - P	2x6	DRY	2100F 1.8E	SPF
P - K	2x6	DRY	2100F 1.8E	SPF
REINFORCII	NG MEN	1BERS		
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D-S	2x4	DRY	No.2	SPF
DDV 05104				

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
В	TMBMW1-m	MT20	8.0	10.0	3.25	1.00				
С	TMWW-t	MT20	4.0	4.0	2.00	1.75				
D	TTWW+m	MT20	8.0	10.0	4.25	2.50				
Ε	TMWW-t	MT20	5.0	5.0	1.50	1.50				
F	TMW+w	MT20	2.0	4.0						
G	TMWW-t	MT20	4.0	5.0	1.50	2.25				
Н	TS-t	MT20	4.0	8.0						
1	TTWW-m	MT20	8.0	10.0	3.00	3.75				
J	TMWW-t	MT20	4.0	4.0	1.75	1.75				
K	TMBMW1-I	MT20	10.0	10.0	4.25	0.25				
М	BMWW-t	MT20	5.0	6.0	2.25	2.00				
N	BMWW-t	MT20	4.0	4.0	2.00	1.75				
0	BMWW-t	MT20	6.0	6.0	2.25	1.50				
Ρ	BS-t	MT18HS	6.0	12.0						
Q	BMWWW-t	MT20	8.0	10.0	4.25	5.00				
R	BS-t	MT18HS	5.0	12.0						
S	BMWW-t	MT20	5.0	6.0	1.75	2.00				
Т	BMWW-t	MT20	4.0	4.0						
11	RM\M\M_t	MT20	5.0	5.0	2 25	1 75				

18	PROFESSIONA	2
LICEN	I.MATIJEVIC 100528832	INEER
Page 1	WINCE OF ONTO	

READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

30-1-0

	FACTOR	ED	MAXIMUN	/ FACTO	RED	INPUT	REQRD		
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG	HEEL	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE	
В	3719	0	3719	0	0	5-8	3-8	2x3 L	
K	4551	0	4551	0	0	5-8	4-5	2x3 R	

UNFACTORED REACTIONS

	1ST LCASE	SEMAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
В	2596	1897 / 0	0/0	0/0	0/0	699 / 0	0/0
K	3180	2303 / 0	0/0	0/0	0/0	877 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B. K BEARING SIZE FACTOR = 1.15 AT JNT(S) B, K (BASED ON SUPPORT DEPTH = 1-8)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	C F	IORDS				W E	BS	
		X. FACTORED	FACTORED				MAX. FACTO	RED
	мемв.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB	. FORCE	MAX
		(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
	FR-TO		FROM TO		LENGTH	FR-TO	, ,	, ,
	A-B	0/0	-119.4 -119.4	0.05(1)	10.00	U- C	-954 / 0	0.16 (1)
	B- W	-3926 / 0	-119.4 -119.4	0.09(1)	5.12	C-T	0 / 691	0.17 (1)
	W- C	-5147 / 0	-119.4 -119.4	0.11(1)	4.58	T- D	-267 / 0	0.07 (1)
	C- D	-5735 / 0	-119.4 -119.4	0.12(1)	4.38		0 / 4034	0.71 (1)
	D- E	-8371 / 0	-119.4 -119.4	0.27(1)	3.61	S-E	-2259 / 0	0.55 (1)
	E-F	-10531 / 0	-119.4 -119.4			E-Q		0.66 (1)
	F- G	-10531 / 0	-225.2 -225.2			Q-F		0.17 (1)
	G- H	-9245 / 0	-225.2 -225.2			Q- G		0.41 (1)
	H- I	-9245 / 0	-225.2 -225.2				-2190 / 0	0.54 (1)
	I- J	-7174 / 0	-119.4 -119.4			O- I		0.91 (1)
	J- Y	-6383 / 0	-119.4 -119.4			N- I		0.07 (1)
	Y- K	-4862 / 0	-119.4 -119.4			N- J		0.23 (1)
	K-L	0/0	-119.4 -119.4	0.05 (1)	10.00		-1165 / 0	0.20 (1)
						V-W	0 / 53	0.00 (1)
	B- V	0 / 1763		0.09(1)		W- U		0.38 (1)
	V- U	0 / 1763				M-Y		0.48 (1)
	U- T	0 / 4572	-18.2 -18.2			X-Y	0 / 100	0.00 (1)
	T-S	0 / 5119		0.30(1)				
	S-R	0 / 8371		0.54 (1)				
	R-Q	0 / 8371		0.54 (1)				
	Q-P	0 / 9244		0.60 (1)				
_	P- 0	0 / 9244		0.60 (1)				
	O- N	0 / 6407		0.39 (1)				
	N- M	0 / 5669		0.35 (1)				
	M- X	0 / 2173		0.16 (1)				
	X-K	0 / 2173	-34.4 -34.4	0.11 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
1	25-0-0	-367	-367		FRONT	VERT	TOTAL		C1
Q	16-0-12	-1359	-1359		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

5-8

TOTAL WEIGHT = 173 lb

SPECIFIED LOADS:

DI = 6.0 PSF LL = DL = 0.0 7.3 BOT CH. TOTAL LOAD = 48.1

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE ADDT'L LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 14-11-4 OF SPAN MEASURED FROM THE RIGHT.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/ 678 (0.55")

CSI: TC=0.41/0.97 (F-G:1) , BC=0.60/0.97 (O-Q:1) , WB=0.91/0.97 (I-O:1) , SSI=0.38/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



CORPORATION OF THE CITY OF OSHAWA JOB NAME TRIFFEUSSARME QUANTITY OF PERMIT PLANS IM0723-093 Oct 909 2023

ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

JOB DESC.

TRUSS DESC.

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ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-GYfJ_zgj9YjxPoc20L3ueyu5kOAa9YzTa6nWKryyE7c

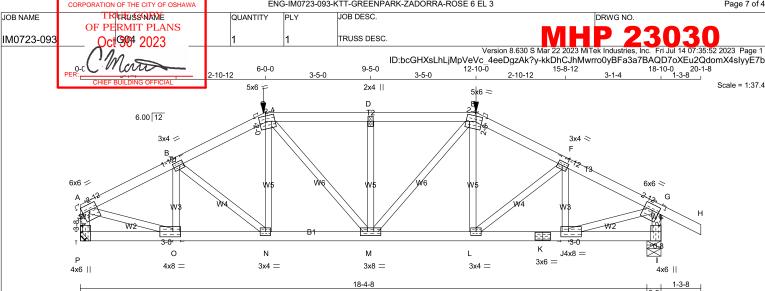
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90) JSI METAL= 0.98 (R) (INPUT = 1.00)







LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EVCEDT				I

DRY: SEASONED LUMBER

PLATES (table is in inches)

BMWW-t

BMWWW-t BMWW-t

J١	TYPE	PLATES	W	LEN	YX	(
Α	TMVW-t	MT20	6.0	6.0	2.25 E	Edge
В	TMWW-t	MT20	3.0	4.0	1.50 1	.75
С	TTWW-m	MT20	5.0	6.0	2.00 2	2.25
D	TMW+w	MT20	2.0	4.0		
Е	TTWW-m	MT20	5.0	6.0	2.00 2	2.25
F	TMWW-t	MT20	3.0	4.0	1.50 1	.75
G	TMVW-t	MT20	6.0	6.0	2.25 2	.75
1	BMV1+t	MT20	4.0	6.0	Edge 0	.50
J	BMWW-t	MT20	4.0	8.0	2.00 3	5.00
K	BS-t	MT20	3.0	6.0		

RMWW-t MT20 4.0 8.0 2.00 3.00 3.50 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

3.0 4.0

3.0 8.0 4.0

MT20

MT20 MT20

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u> </u>	LAIMINGO								
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD		
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
Р	2347	0	2347	0	0	MECHANIC	CAL		
l	2509	0	2509	0	0	5-8	4-11		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT P. MINIMUM BEARING LENGTH AT JOINT P = 3-8.

UNFACTORED REACTIONS

	IST LUASE		VIIIN. COIVIPOI	NO ON				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
Ρ	1642	1179 / 0	0/0	0/0	0/0	463 / 0	0/0	
1	1753	1274 / 0	0/0	0/0	0/0	479 / 0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

WERS

LOADING TOTAL LOAD CASES: (4)

CHORDS

СН	UKUS					VV E	85		
MAX	K. FACTORED	FACTO	RED				MAX. FACT	ORED	
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	_F)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO	, ,	FROM	ΤΌ		LENGTH	FR-TO	, ,		, ,
A-B	-3061 / 0	-119.4	-119.4	0.29(1)	3.67	O-B	-672 / 0	0.12	(1)
B- C	-3301 / 0	-119.4	-119.4	0.29 (1)	3.55	B- N	0 / 239	0.06	
C-D	-3386 / 0	-225.2	-225.2	0.62 (1)	3.00	N- C	-19 / 109	0.04	(4)
D-E	-3386 / 0	-225.2	-225.2	0.62 (1)	3.00	C- M	0 / 679	0.17	(1)
E-F	-3301 / 0	-119.4	-119.4	0.29 (1)	3.55	M- D	-921 / 0	0.25	(1)
F- G	-3061 / 0	-119.4	-119.4	0.29 (1)	3.67	M- E	0 / 679	0.17	(1)
G- H	0 / 36	-119.4	-119.4	0.17 (1)	10.00	L-E	-19 / 109	0.04	(4)
P- A	-2290 / 0	0.0	0.0	0.25 (1)	5.50	L-F	0 / 239	0.06	(1)
I- G	-2452 / 0	0.0	0.0	0.27 (1)	5.33	J- F	-672 / 0	0.12	(1)
						A- O	0 / 2853	0.71	(1)
P- 0	0/0	-34.4	-34.4	0.08(4)	10.00	J- G	0 / 2853	0.71	(1)
O- N	0 / 2750	-34.4	-34.4	0.53(1)	10.00				
N- M	0 / 2936	-34.4	-34.4	0.57 (1)	10.00				
M- L	0 / 2936	-34.4	-34.4	0.57 (1)	10.00				
L-K	0 / 2750	-34.4	-34.4	0.53 (1)	10.00				
K-J	0 / 2750	-34.4	-34.4	0.53(1)	10.00				
J- I	0/0	-34.4	-34.4	0.08 (4)	10.00				
	SPECIFIED CONCENTRATED LOADS (LBS)								
							T. (D.E.		
l JT	LOC. LC	1 MAX-	MAX	+ F/	ACE [DIR.	TYPE	HEEL	CON

וו	LUC.	LUI	IVIAA-	IVIAX+	FACE	DIK.	ITPE	HEEL	CONN.
С	6-0-0	-367	-367		FRONT	VERT	TOTAL		C1
Ē	12-10-0	-367	-367		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:								
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSI			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSI			
TOTA	I IO	AD	=	48 1	PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

TOTAL WEIGHT = 78 lb

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENEND JACK TYPE: CONVENTIONAL CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

CSA 086-14 - TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.63") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10") ALLOWABLE DEFL.(TL)= L/360 (0.63") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.62/0.97 (C-D:1) , BC=0.57/0.97 (M-N:1) , WB=0.71/0.97 (A-O:1) , SSI=0.40/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90) JSI METAL= 0.85 (K) (INPUT = 1.00)





5x6 =

CORPORATION OF THE CITY OF OSHAWA TRITHUSSARME JOB NAME OF PERMIT PLANS Oct 905 2023 IM0723-093 linte

JOB DESC QUANTITY TRUSS DESC.

5-1-0

6.00 12

2-0-0

5x6 =

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ID:3suF8DUMbbxFEOWsk3nNlizALn9-Cwn4Pfh_h9zfe6mR7m5MjNzMUCqqdVYm1PGdPkyyE7a 11-5-12 15-1-0 17-10-4 20-10-0 2-9-4 2-11-12

Ε 5x6 \\ 4x5 II A 5x6 Ⅱ 3x4 // G 170 12x12 || BL Κ Р 0 Ν М Q 3x6 = 4x6 || 4x5 || 3x4 || 4x6 = 4x5 = 2x4 || 4x6 ||

2x4 ||

TOTAL WEIGHT = 107 lb

Scale = 1:44.5

LUMBER						
N. L. G. A. R	ULES					
CHORDS	SIZE		LUMBER	DESCR.		
Q - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - H	2x6	DRY	No.2	SPF		
I - H	2x4	DRY	No.2	SPF		
Q - K	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
BEARING BI	OCKS					
BL1	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PL/	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X			
Α	TMVW+p	MT20	5.0	6.0	2.25	1.75			
В	TMWW+t	MT20	4.0	5.0	2.00	1.50			
С	TTWW+m	MT20	5.0	6.0	3.00	2.00			
D	TTWW-m	MT20	5.0	6.0	2.50	2.25			
Е	TMW+w	MT20	2.0	4.0					
F	TTWW-m	MT20	5.0	6.0	2.00	1.75			
G	TMWW+t	MT20	3.0	4.0	1.75	0.75			
Н	TMVWK1+w	MT20	12.0	12.0	Edge	5.50			
1	BMV+p	MT20	2.0	4.0					
J	BMWW-t	MT20	4.0	5.0	1.75	1.50			
K	BS-t	MT20	3.0	6.0					
L	BMWW-t	MT20	3.0	4.0					
M	BMWWW-t	MT20	4.0	6.0	1.75	1.50			
Ν	BMWW+t	MT20	3.0	4.0					
0	BMWW+t	MT20	4.0	5.0	2.00	1.50			
Ρ	BMWW+t	MT20	4.0	6.0	1.50	1.50			
Q	BMV1+t	MT20	4.0	6.0	3.50				
R	KP-p	MT20	3.0	6.0		3.00			

 $\mbox{Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. }$



READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

20-10-0

	FACTO	RED	MAXIMUI	M FACTO	DRED	INPUT	REQRD
	GROSS RE	EACTION	GROSS I	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2677	0	2677	0	0	MECHANIC	AL
S(H)	2612	0	2612	0	0	MECHANIC	AL
							(** SEE "BEARING NOTE" **)

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT Q, S(H). MINIMUM BEARING LENGTH AT JOINT Q = 3-8, JOINT S(H) = 5-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX	<u>/MIN. COMPON</u>	<u>ENT REACTION</u>	NS .		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1871	1353 / 0	0/0	0/0	0/0	517 / 0	0/0
S(H)	1826	1321 / 0	0/0	0/0	0/0	505 / 0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

СН	ORDS				W E	EBS	
MAX	(. FACTORED	FACTORED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LOAD L	.C1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)					(LBS)	CSI (LC)
FR-TO		FROM TO					
Q- A	-2535 / 0	0.0				0 / 2884	0.71 (1)
A-B	-1582 / 0	-119.4 -119			P-B	-1980 / 0	0.55 (1)
B- C	-2607 / 0	-119.4 -119				0 / 1926	0.48 (1)
C-D	-2888 / 0	-119.4 -119				-1429 / 0	0.39(1)
D-E	-2877 / 0	-119.4 -119	.4 0.49 (1)	3.53	C- N	-131 / 0	0.04(1)
E-F	-2876 / 0	-119.4 -119	.4 0.46 (1)	3.53	N- D	0 / 643	0.16 (1)
F-G	-2569 / 0	-119.4 -119	.4 0.11 (1)	5.03	D- M	0 / 418	0.10 (1)
G- H	-2158 / 0	-119.4 -119			M- E	-538 / 0	0.22(1)
I- R	0 / 139				M- F	0 / 1004	0.25 (1)
R-H	0 / 139	0.0	.0 0.42 (1)	10.00	L- F	0 / 49	0.02 (4)
					L- G	0 / 525	0.13 (1)
Q-P	0/0					-954 / 0	0.22 (1)
P- 0	0 / 1582	-140.2 -140				0 / 2125	0.38 (1)
O- N	0 / 2646	-140.2 -140				-2663 / 0	0.20(1)
N- M	0 / 2597	-140.2 -140			R-S	0 / 516	0.00 (1)
M-L	0 / 2276	-140.2 -140					
L-K	0 / 1935	-140.2 -140					
K-J	0 / 1935	-140.2 -140					
J- I	0 / 308	-140.2 -140	.2 0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:								
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSI			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSI			
TOTA	I IO	AD	=	48 1	PSF			

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 6-0-0 END DISTANCE = 20-10-0 END SPAN CARRIED = 6-0-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.68") CALCULATED VERT. DEFL.(LL)= L/999 (0.11") ALLOWABLE DEFL.(TL)= L/360 (0.68") CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.71/0.97 (A-Q:1) , BC=0.74/0.97 (M-N:1) , WB=0.71/0.97 (A-P:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR (PSI) (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90) JSI METAL= 0.70 (P) (INPUT = 1.00)



CORPORATION OF THE CITY OF OSHAWA TRITHUSSARME JOB NAME OF PERMIT PLANS Oct 90 2023 IM0723-093

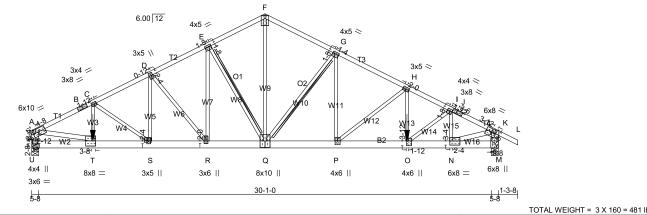
TRUSS DESC.

-12

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7-10-8 11-8-4 15-6-0

Scale = 1:76.5



LUMBER N. L. G. A. RULES LUMBER DESCR. SPF CHORDS DRY A - B B - F No.2 B F DRY SPF 2x4 DRY No.2 DRY DRY SPF SPF М 2x6 No.2 DRY SPF ALL WEBS EXCEPT SPF DRY No.2 DRY

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF $\underline{\mathbf{3}}$ TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORD	S #ROWS		LOAD(PLF)
		SPACING (IN)	
TOP CH	IORDS : (0.1	22"X3") SPIRAL NA	NLS
A-B	1	12	SIDE(71.2)
B- F	1	12	SIDE(71.2)
F- J	1	12	TOP
J- L	1	12	TOP
U- A	2	12	TOP
M- K	2	12	TOP
BOTTO	M CHORDS	: (0.122"X3") SPIRA	L NAILS
U- Q	2	5	SIDE(758.4)
Q- M	2	4	SIDE(867.5)
WEBS:	(0.122"X3")	SPIRAL NAILS	, ,
2x3	1	6	
244	4	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u> </u>	111100						
	FACTORED		MAXIMUN	/ FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	11948	0	11948	0	0	5-8	5-8
M	10982	0	10982	0	0	5-8	5-8

UNFACTORED REACTIONS

AMAY /MINI COMPONENT REA

l .	151 LUASE	: IVIAX./I	VIIN. COMPO	NENT REACTION	V 5		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	8350	6042 / 0	0/0	0/0	0/0	2308 / 0	0/0
М	7672	5565 / 0	0/0	0/0	0/0	2107 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U. M. BEARING SIZE FACTOR = 1.15 AT JNT(S) U, M (BASED ON SUPPORT DEPTH = 1-8)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.28 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-Q, G-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3 COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

WEBS

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

CHORDS

MA	AX. FACTORED	FACTORE					MAX. FACT		
MEME	FORCE	VERT. LOAD	LC1 M	1AX	MAX.	MEMB	. FORCE	MAX	(
	(LBS)	(PLF)	CSI	(LC)	UNBRAC	;	(LBS)	CSI	(LC)
FR-TC) ' '	FROM TO			LENGTH	FR-TO			
A-B	-18327 / 0	-238.9 -23	8.9 0.7	76 (1)	2.28	T- C	0 / 127	0.01	(4)
B- C	-18327 / 0	-238.9 -23				C-S	-840 / 0	0.10	(1)
C- D	-17599 / 0	-119.4 -11	9.4 0.5	54 (1)	2.56	S- D	0 / 2955	0.22	(1)
D- E	-15408 / 0	-119.4 -11	9.4 0.4	13 (1)	2.84	D-R	-3095 / 0	0.64	(1)
E-F	-12698 / 0	-119.4 -11					0 / 4957	0.37	(1)
F- G	-12716 / 0	-119.4 -11	9.4 0.4	12 (1)	3.17	E-Q	-4873 / 0	0.58	(1)
G- H	-16357 / 0	-119.4 -11	9.4 0.5	59 (1)		Q-F		7 0.82	(1)
H- I	-18608 / 0	-119.4 -11	9.4 0.5	51 (1)		Q- G	-5410 / 0	0.66	
I- J	-15560 / 0	-119.4 -11				P- G	0 / 5305		
J- K	-15560 / 0	-119.4 -11					-2594 / 0	0.54	(1)
		-119.4 -11				O- H			(1)
					5.35		0 / 3521		
M-K	-10757 / 0	0.0	0.0 0.2	23 (1)	5.57	N- I	-3879 / 0	0.21	(1)
						A- T	0 / 1670	3 0.89	(1)
U- T	0/0	-36.5 -3	6.5 0.0	09 (1)	10.00	N-K	0 / 1432	9 0.77	(1)
T-S	0 / 16407	-581.9 -58	1.9 0.8	37 (1)	10.00				
S-R	0 / 15731	-581.9 -58	1.9 0.7	75 (1)	10.00				
R-Q	0 / 13784	-666.5 -66	6.5 0.7	75 (1)	10.00				
Q-P	0 / 14642	-666.5 -66	6.5 0.8	39 (1)	10.00				
P-O	0 / 16661	-666.5 -66	6.5 0.9	97 (1)	10.00				
O- N	0 / 13911	-18.2 -1	8.2 0.6	88 (1)	10.00				
N- M	0/0	-18.2 -1	8.2 0.1	14 (1)	10.00				
SPEC	IFIED CONCENT	RATED I OADS	S (LBS)						
		MAX- N	AAX+	F/	ACE E)IR	TYPE	HEEL	CONN.
	24-11-4 -1890					RT			C1
	4-0-12 -1642	-1642				RT	TOTAL		C1
· .	1012				J V L		101/L		51
CONN	IECTION REQUIRE	EMENTS							

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: DI = 6.0

PSF LL = DL = 0.0 7.3 BOT CH. TOTAL LOAD = 48.1

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip SIDE SETBACK = 0-0 END SETBACK = 6-0-0 END WALL WIDTH = 0-0 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE

- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-0-12 OF SPAN

GIRDER TYPE: CStdGirder START DISTANCE = 4-0-12 START SPAN CARRIED = 18-10-0 END DISTANCE = 11-6-4 END SPAN CARRIED = 18-10-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder START DISTANCE = 11-5-8 START SPAN CARRIED = 20-10-0 END DISTANCE = 24-11-4 END SPAN CARRIED = 20-10-0 END WALL WIDTH = 0-0 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03") CALCULATED VERT. DEFL.(LL)= L/999 (0.27") ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL)= L/794 (0.47")

CSI: TC=0.76/0.97 (A-C:1) , BC=0.97/0.97 (O-P:1) , WB=0.89/0.97 (A-T:1) , SSI=0.38/1.00 (P-Q:1)



JOB NAME

CORPORATION OF THE CITY OF OSHAWA

TRICKUS NAME

OF PERMIT PLANS

Oct 90 2023

QUANTITY PLY JOB DESC.

1 3 TRUSS DESC.

ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

DRWG NO.

MHP 23030

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Fri Jul 14 07:35:54 2023 Page 2

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Fri Jul 14 07:35:54 2023 Page 2 ID:bcGHXsLhLjMpVeVc 4eeDgzAk?y-g7LSc?icST5WGGLdhUdbGbWXVc5UMwzvG30AxAyyE7Z

PLATES (table is in phospile Building OFFICIAL
IT TYPE BLATES W. LEN X X

IM0723-093

PLATES (table JT TYPE A TMVW-t LEN Y X 10.0 2.75 4.50 PLATES MT20 6.0 ABCDEF TS-t TMWW-t TMWW+t MT20 MT20 MT20 3.0 3.0 3.0 4.0 8.0 4.0 5.0 5.0 8.0 5.0 4.0 8.0 8.0 1.50 1.75 2.25 0.75 1.50 1.50 TMWW-t MT20 MT20 Edge 1.50 1.25 1.50 2.00 1.50 1.75 6.0 G TMWW-t 4.0 3.0 4.0 MT20 MT20 MT20 TMWW-t TMWW-t TS-t TMVW-t MT20 MT20 3.0 1.50 3.25 2.50 3.75 JKMNOPQRST BMV1+t BMWW-t MT20 MT20 6.0 6.0 8.0 8.0 Edge 0.50 3.25 2.25 6.0 6.0 BMWW+t MT20 4.0 3.75 1.75 BMWW+t BSWWW+l MT20 MT20 4.0 10.0 2.00 1.50 2.25 1.50 4.00 3.50 2.25 2.00 2.50 2.75 3.0 3.0 8.0 4.0 6.0 5.0 8.0 4.0 BMWW+t MT20 MT20 BMWW+t MT20 MT20 BMWW-t BMV1+p TP-t MT20 3.0 6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

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COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 $\begin{array}{c|ccccc} \text{NAIL Values} & \text{NAIL Value} \\ \text{PLATE} & \text{GRIP}(\text{DRY}) & \text{SHEAF} & \text{SECTION} \\ & (\text{PSI}) & (\text{PLI}) & (\text{PLI}) \\ & \text{MAX} & \text{MIN} & \text{MAX} & \text{MIN} & \text{MAX} & \text{MIN} \\ \text{MT20} & 650 & 371 & 1747 & 788 & 1987 & 1873 \\ \end{array}$

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90) JSI METAL= 0.93 (K) (INPUT = 1.00)





CORPORATION OF THE CITY OF OSHAWA TRITHUSSARME JOB NAME OF PERMIT PLANS Oct 907 2023 IM0723-093

JOB DESC. QUANTITY TRUSS DESC.

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ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-8JvqqKjEDmDNtQwqFB8qoo2oV0Ti5PR3VjlkTdyyE7Y 2-11-0 5-0-0 7-1-0 8-1-4 11-3-0 14-2-0 1-4-4 1-6-12 6x8 || 3x4 >

Е 4.00 12 D 3x4 > 3x5 = G С 5x6 > 6x6 = Н В 5x6 = 4x6 =W11 Р 0 6x10 = 6x10 =4x5 = 6x10 || 4x4 = Κ 5x10 = 2x4 || 2x4 || 5x8 = 3x4 || 13-3-0

TOTAL WEIGHT = 2 X 75 = 150 lb

Scale = 1:33.7

LUMBER N.I.G.A.R				
CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	2100F 1.8E	SPF
E - I	2x4	DRY	2100F 1.8E	SPF
T - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
T - R	2x6	DRY	2100F 1.8E	SPF
R - C	2x3	DRY	No.2	SPF
Q - M	2x8	DRY	No.2	SPF
L - G	2x3	DRY	No.2	SPF
L - J	2x6	DRY	2100F 1.8E	SPF
ALL MEDO	00	DDV	N- O	ODE
ALL WEBS	2x3	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
М - Н	2x4	DRY	No.2	SPF
I				

5-8

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS

CHORDS	#ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	, ,
TOP CHO		2"X3") SPIRAĹ NAILS	
A-E	1 `	12	TOP
E-I	1	12	TOP
T- A	2	12	TOP
J- I	2	12	TOP
воттом	CHORDS: (0.122"X3") SPIRAL NAILS	
T-R	2	8	SIDE(324.1)
L- J	2	12	TOP
R- C	1	12	TOP
G- L	1	12	TOP
Q- M	2	8	SIDE(324.1)
WEBS: (0.122"X3") SI	PIRAL NAILS	
2x3	1 '	6	
2 v /	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMU	M FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Т	5844	0	5844	0	0	5-8	2-12
J	3973	0	3973	0	0	5-8	1-14

UNF	ACTORED RE	ACTIONS					
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	NS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Т .	4084	2955 / 0	0/0	0/0	0/0	1129 / 0	0/0
J	2776	2009 / 0	0/0	0/0	0/0	768 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T. J BEARING SIZE FACTOR = 1.15 AT JNT(S) T, J (BASED ON SUPPORT DEPTH = 1-8)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C	HORDS					W E	BS	
M/	XX. FACTORED	FACTOR	RED				MAX. FACTO	RED
MEME	. FORCE	VERT. LOA	AD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PLI	F)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TC) ' '	FROM .	ΤΌ		LENGTH	FR-TO		
A-B	-4711 / 0	-119.4 -	-119.4	0.15 (1)	5.16	S-B	-6494 / 0	0.52(1)
B- C	-12584 / 0	-119.4 -	-119.4	0.29(1)	3.25	C-P	-1980 / 0	0.18 (1)
C- D	-10880 / 0	-119.4 -	-119.4	0.24(1)	3.55	P- D	0 / 1442	0.18 (1)
D-E	-9353 / 0	-119.4 -	-119.4	0.13(1)	3.92	D- O	-1947 / 0	0.18 (1)
E-F	-9330 / 0	-119.4 -	-119.4	0.13(1)	3.92	O-E	0 / 5627	0.70 (1)
F-G	-9950 / 0	-119.4 -	-119.4	0.23(1)		0- F	-1306 / 0	0.11 (1)
G- H	-10137 / 0	-119.4 -	-119.4	0.20(1)	3.70	N- F	0 / 1216	0.15 (1)
H- I	-3441 / 0	-119.4 -	-119.4	0.11 (1)		N- G	-380 / 0	0.04 (1)
T- A	-5168 / 0	0.0	0.0	0.19(1)	6.34	K- H	-5474 / 0	0.44 (1)
J- I	-3806 / 0	0.0	0.0	0.14 (1)	7.16	A-S		0.69 (1)
						K- I		0.51 (1)
T-S	0/0			0.07 (1)		B- Q		0.75 (1)
S-R	0 / 181			0.06 (1)		S- Q		0.69 (1)
R-Q	0 / 611			0.36(1)		M- H		0.64 (1)
Q-C	0 / 1273	0.0		0.44 (1)		M-K	0 / 4064	0.50 (1)
Q-P	0 / 12123			0.84 (1)				
P- 0	0 / 10348			0.71 (1)				
O- N	0 / 9435	-666.5 -						
N- M	0 / 9796			0.78 (1)				
L- M	0 / 201			0.25 (1)				
M- G	-215 / 0			0.23 (1)				
L-K	0 / 143			0.05 (1)				
K-J	0/0	-18.2	-18.2	0.05 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS) MÀX+ FACE DIR TYPF HEEL -1826 -1826 FRONT TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

5-8

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: 34.8 DI = 6.0 PSF LL = DL = 0.0 7.3 BOT CH. TOTAL LOAD = 48.1

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 20-10-0 END DISTANCE = 8-1-4 END SPAN CARRIED = 20-10-0 END WALL WIDTH = 0-0 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: PART 9 OF BCBC 2018 NBC-2019AF - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

CONN

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/817 (0.21")

CSI: TC=0.29/0.97 (B-C:1), BC=0.84/0.97 (P-Q:1) , WB=0.75/0.97 (B-Q:1) , SSI=0.56/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



CORPORATION OF THE CITY OF OSHAWA JOB NAME TRITHUSSARME OF PERMIT PLANS

Oct 907 2023

QUANTITY PLY JOB DESC. TRUSS DESC.

ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

MHP 23030 Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Fri Jul 14 07:35:55 2023 Page 2

JSI GRIP= 0.89 (B) (INPUT = 0.90) JSI METAL= 0.90 (B) (INPUT = 1.00)

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ID:bcGHXsLhLjMpVeVc 4eeDgzAk?y-8JvqqKjEDmDNtQwqFB8qoo2oV0Ti5PR3VjlkTdyyE7Y

PLATES (table JT TYPE A TMVW-p LENK OFFICIAL

LEN Y X
6.0 1.50 2.75
6.0 2.00 2.50
5.0 1.50 2.00
1.75
8.0 Edge
4.0 1.50 1.25
4.0 1.50 2.75
4.0 2.25 1.00
8.0 1.75 2.75
4.0 2.25 1.00
4.0 1.00
10.0 2.25 2.00
10.0 3.00 5.25
4.0 3.00 5.25
4.0 4.0 1.75 3.00
10.0 1.75 5.00
4.0 1.75 5.00 PLATES MT20 W 5.0 6.0 3.0 4.0 6.0 3.0 5.0 4.0 ABCDEF MT20 MT20 MT20 MT20 MT20 TMWW-t TMVW-t TMWW-t TTW+p TMWW-t TMVW-t TMVW-t TMVW-p BMV1+p BMWWW-t G MT20 MT20 MT20 MT20 MT20 2.0 BMV+p BVMWW-I MT20 MT20 2.0 6.0 4.0 6.0 4.0 6.0 2.0 5.0 3.0

IM0723-093

BMWW-t BMWWW+t BMWW-t BVMWW-I BMV+p BMWWW-t

BMV1+p

Q R S T

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

MT20 MT20 MT20

MT20 MT20 MT20





ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

JOB DESC

0-0

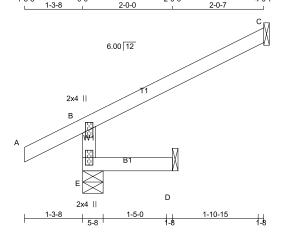
TRUSS DESC.

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ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-cVTC1gks_4MEVZU0puf3L0byeP0oq1RCjNVH?3yyE7X 2-0-0 4-0-7

Scale = 1:25.7

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LUMBER N. L. G. A. RULES DESCR. SPF SPF SPF CHORDS SIZE LUMBER E - B A - C E - D DRY 2x4 No.2 No.2 No.2 DRY DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES LEN Y q+VMT MT20 2.0 BMV1+p

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

DEAL	VIIAGO						
	FACTOR	RED	MAXIMUN	M FACTO	DRED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	483	0	483	0	0	5-8	1-8
С	181	0	181	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C. D

UNFACTORED REACTIONS

| MAX./MIN. COMPONENT REACTIONS | SNOW LIVE | PERM.LIVE | WIND | 270 / 0 | 0 / 0 | 0 / 0 | COMBINED SOIL 0/0 DEAD Ē 270 / 0 333 63 / 0 105 / 0 0/0 C D 0/0 13 / 0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

QUANTITY

-1-3-8

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHC	RDS					WE	BS		
MAX.	FACTORED	FACTORE	D				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD	LC.	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)	
FR-TO		FROM TO)		LENGTH	FR-TO			
E-B	-463 / 0	0.0	0.0	0.01 (4)	7.81				
A- B	0 / 36	-119.4 -1	19.4	0.16 (1)	10.00				
B- C	-27 / 0	-119.4 -1	19.4	0.33 (1)	6.25				
E- D	0/0	-18.2 -	18.2	0.02(4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS LL DL 34.8 6.0 TOP CH. = BOT CH. 0.0 7.3 PSF LL DL TOTAL LOAD 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

TOTAL WEIGHT = 3 X 10 = 30 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.33/0.97 (B-C:1) , BC=0.02/0.97 (D-E:4) , WB=0.00/0.97 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)





ENG-IM0723-093-KTT-GREENPARK-ZADORRA-ROSE 6 EL 3

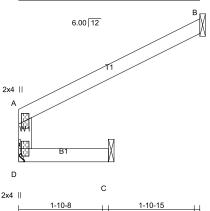
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QUANTITY JOB DESC TRUSS DESC.

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0-0 2-0-0 4-0-7 2-0-7

Scale = 1:25.7



TOTAL WEIGHT = 2 X 8 = 17 lb

LUMBER N. L. G. A. RULES CHORDS D - A A - B D - C DESCR. SPF SIZE LUMBER DRY 2x4 No.2 No.2 No.2 DRY SPF DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES

LEN Y 2.0 TMV+p MT20 4.0 BMV1+p

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTO	RED	MAXIMU	M FACTO	ORED	INPUT	REQRD
	GROSS REACTION		GROSS	REACTIC	BRG	BRG	
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
)	229	0	229	0	0	MECHANIC	CAL
3	212	0	212	0	0	1-8	1-8
2	78	0	78	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	<u>/IIN. COMPO</u>	<u>NENT REACTION</u>	NS .		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	159	123 / 0	0/0	0/0	0/0	36 / 0	0/0
В	145	123 / 0	0/0	0/0	0/0	22 / 0	0/0
С	55	35 / 0	0/0	0/0	0/0	20 / 0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX.	FACTORED	FACTORED			MAX	(. FACTO	RED	
MEMB.	FORCE	VERT. LOAD LO	1 MAX	MAX. N	иЕМВ.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH F	R-TO			
D- A	-270 / 0	0.0 0.0	0.14 (1)	7.81				
A- B	-13 / 0	-119.4 -119.4	0.24 (1)	6.25				
D- C	0/0	-18.2 -18.2	0.16(1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS LL = 3 DL = LL = PSF PSF PSF 34.8 6.0 TOP CH. 0.0 7.3 BOT CH. TOTAL LOAD 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24/0.97 (A-B:1) , BC=0.16/0.97 (C-D:1) , WB=0.00/0.97 (n/a:0) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90) JSI METAL= 0.11 (A) (INPUT = 1.00)





0-0

QUANTITY JOB DESC. TRUSS DESC. Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Fri Jul 14 07:35:58 2023 Page 1 ID:bcGHXsLhLjMpVeVc_4eeDgzAk?y-YuazSMl6WhcyktePwJhXQRgLtDiGlxxVBh_O4xyyE7V

2-0-0 2-0-7 0₀7

D

С 6.00 12 2x4 || В W В1

Scale = 1:18.7

TOTAL WEIGHT = 3 X 7 = 22 lb

LUMBER								
N. L. G. A.	RULES							
CHORDS	SIZE		LUMBER	DESCR.				
E - B	2x4	DRY	No.2	SPF				
A - C	2x4	DRY	No.2	SPF				
E - D	2x4	DRY	No.2	SPF				
DRY: SEAS	DRY: SEASONED LUMBER.							

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN Y	Х
В	TMV+p	MT20	2.0	4.0	
Е	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u> </u>	<u>BEARINGS</u>									
		FACTORED		MAXIMUM FACTORED			INPUT	REQRD		
	GROSS REACTION			GROSS REACTION			BRG	BRG		
J.	Т	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
Е		334	0	334	0	0	5-8	1-8		
С		92	0	92	0	0	1-8	1-8		
D)	16	0	18	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C. D

UNFACTORED REACTIONS

-1-3-8

1-3-8

1-3-8

	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E	230	183 / 0	0/0	0/0	0/0	48 / 0	0/0	
C	63	54 / 0	0/0	0/0	0/0	9/0	0/0	
D	13	0/0	0/0	0/0	0/0	13 / 0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

CHC	RDS	WEBS					
MAX.	FACTORED	FACTORED		MAX. FACTOR	RED		
MEMB.	FORCE	VERT. LOAD LC1 MA	X MAX. MEME	FORCE	MAX		
	(LBS)	(PLF) CSI (L	.C) UNBRAC	(LBS)	CSI (LC)		
FR-TO		FROM TO	LENGTH FR-TO)			
E-B	-313 / 0	0.0 0.0 0.01	(4) 7.81				
A- B	0 / 36	-119.4 -119.4 0.16	(1) 10.00				
B- C	-13 / 0	-119.4 -119.4 0.08	(1) 6.25				
E- D	0/0	-18.2 -18.2 0.02	(4) 10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:							
TOP	CH.	LL	=	34.8	PS		
		DL	=	6.0	PS		
BOT	CH.	LL	=	0.0	PS		
		DL	=	7.3	PS		
TOTA	L LO	AD	=	48.1	PS		

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.00") CSI: TC=0.16/0.97 (A-B:1), BC=0.02/0.97 (D-E:4),

WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (A-B:1) DOL LUMBER=1 00 NAII =1 00 LS BEND=1 10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



