

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.



PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
2. THE COMPRESSION CHORDS ARE Laterally Braced by Continuous Rigid Diaphragm Sheathing or as Specified on the Drawing.
3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED PERMANENT. THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES $\geq 12.37\text{M}$ (40'-7").



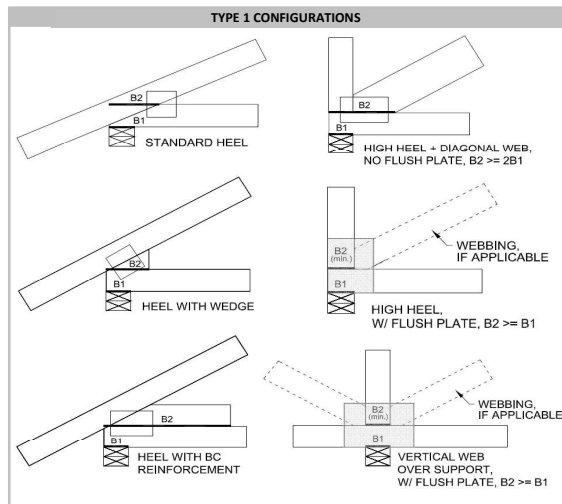
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT TRUSS BEARING CAPACITY TABLE

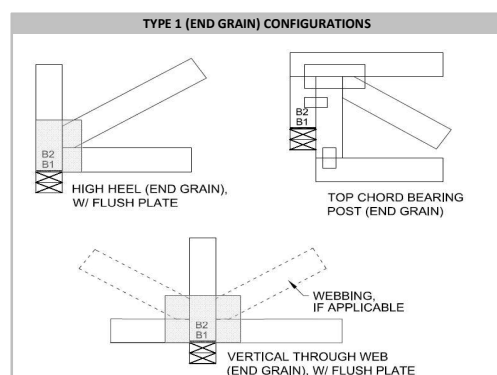
(INTERNAL USE ONLY)

PER: *C. M...* CHIEF BUILDING OFFICIAL, BY TRUSS LUMBER TYPE (SUPPORTED ON SPF #2 TOP PLATE) 7-Jun-21

BEARING	1-PLY	TYPE 1, NO FLUSH PLATE, B2 >= 2B1				TYPE 1, FLUSH PLATE			
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	PLATE (B1)								
	1 1/2"	1383		2767		4151		5534	
	2x4	3712	3228	7425	6457	11138	9685	14851	12914
	2x6	5834	5073	11668	10146	17503	15220	23337	20293
	2x8	7690	6687	15381	13375	23072	20063	30763	26750
FLUSH PLATE	BEARING PLATE (B1)								
	1 1/2"	1383		2767		4151		5534	
	2x4	3712		7425		11138		14851	
	2x6	5834		11668		17503		23337	
	2x8	7690		15381		23072		30763	
BEARING ENHANCER	TYPE 1, FLUSH PLATE + BEARING ENHANCER								
	BEARING PLATE (B1)								
	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	
	CPn-4 (Simpson)	2x4	4515	9030	13545	18065			
	CPn-6 (Simpson)	2x6	7095	14190	21285	28390			
CPn-4 (KOTT)	2x4	6007	4898	12014	9796	18021	14694	19801	19592
	2x6	8677	7075	17354	14150	26031	21225	31117	28300
	SBP4 (MiTek)	2x4	7288	11001	14714	18427			
	SBP6 (MiTek)	2x6	11030	16865	22699	28534			
	SBP6 (MiTek)	2x8	12886	20578	28269	35960			



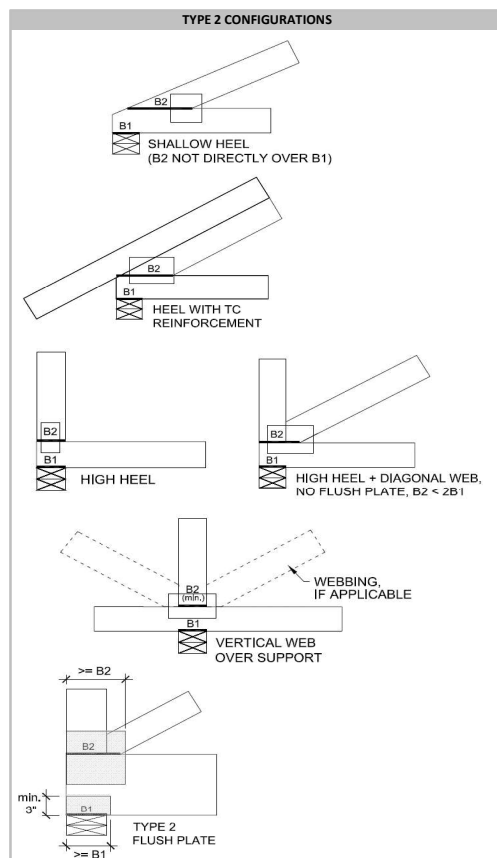
BEARING	POST (B2)	TYPE 1 (END GRAIN)				TYPE 1 (END GRAIN), FLUSH PLATE + BEARING ENHANCER			
		MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)
END GRAIN	BEARING PLATE (B1)								
	2x4	3712		7425		11138		14851	
	2x6	5834		11668		17503		23337	
	2x8	7690		15381		23072		30763	
	2x8	7690		15381		23072		30763	
END GRAIN, BEARING ENHANCER	BEARING PLATE (B1)								
	POST (B2)								
	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	
	CPn-4 (Simpson)	2x4	4515	9030	13545	18065			
	CPn-6 (Simpson)	2x6	7095	14190	21285	28390			
CPn-4 (KOTT)	2x4	6007	4898	12014	9796	18021	14694	19801	19592
	2x6	8677	7075	17354	14150	26031	21225	31117	28300
	SBP4 (MiTek)	2x4	7288	11001	14714	18427			
	SBP6 (MiTek)	2x6	11030	16865	22699	28534			
	SBP6 (MiTek)	2x8	12886	20578	28269	35960			



BEARING	POST (B2)	TYPE 2, NO FLUSH PLATE, B2 < 2B1				TYPE 2, FLUSH PLATE			
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	BEARING PLATE (B1)								
	2x4	2639	2152	5279	4304	7919	6457	10588	8609
	2x6	3393	2767	6787	5534	10181	8302	13575	11069
	2x8	4147	3382	8296	6764	12444	10146	16592	13529
	2x10	5562	4535	11124	9070	16686	13606	22248	18141
	2x4	3959	3228	7919	6457	11878	9685	15838	12914
	2x6	4808	3920	9616	7840	14424	11761	19232	15681
	2x8	4808	3920	9616	7840	14424	11761	19232	15681
	2x8	5467	4458	10935	8916	16403	13375	21871	17833
	2x8	5467	4458	10935	8916	16403	13375	21871	17833
FLUSH PLATE	BEARING PLATE (B1)								
	POST (B2)								
	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	
	2x4	4672	3809	9344	7619	14016	11429	18689	15238
	2x6	4672	3809	9344	7619	14016	11429	18689	15238
	2x8	7342	5986	14684	11973	22026	17959	29368	23946
	2x10	7342	5986	14684	11973	22026	17959	29368	23946
	2x4	4672	3809	9344	7619	14016	11429	18689	15238
	2x6	4672	3809	9344	7619	14016	11429	18689	15238
	2x8	7342	5986	14684	11973	22026	17959	29368	23946

NOTES:

- Factored truss reaction shall not exceed bearing capacity corresponding to: configuration type, size of bearing surfaces, truss lumber, # of plies, and applicable enhancers.
- Values in table are in conformance with CSA O86-14 Cl. 6.5.7 and TPIC 2014-Update 2, and may be used for residential or commercial designs.
- Values in table are in conformance with MiTek Canada Detail B37821Q "SPF Bearing Capacities".
- Values in table are in conformance with Simpson Catalogue C-C-CAN2020.
- Conditions for use of table values include: standard duration (K_{sup}=1), dry lumber (K_{sup}=1), untreated lumber (K_{sup}=1), length of bearing factor not applied (K_{sup}=1).
- Size factor (K_{sup}) applied to support material calculation when acceptable. Flush plate factor (K_{sup}) applied to truss material calculation when acceptable (ie. excludes end grain).
- Flat roof factor (K_{sup}) must applied for trusses making up a flat roof system; to do so, multiply bearing capacity values by 0.75 for this application.
- Bearing plate is to be specified by the project engineer; values in table assume a bearing material of SPF #2 (or better).
- When required, flush plate must not be located further than 1/4" away from bearing surface, and must cover the entire bearing plate length (B1).
- When required, bearing enhancer must be installed as per manufacturer's guidelines.
- Type 2 bearing configurations can be converted to use Type 1 table values as outlined in TPIC 2014-Update 1 Cl. 7.5.9.
- This table is not valid after April 30, 2022.



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Oct 31 2023

PER: 
CHIEF BUILDING OFFICIAL

MHP 23031

STANDARD DETAIL MSD2015-H

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

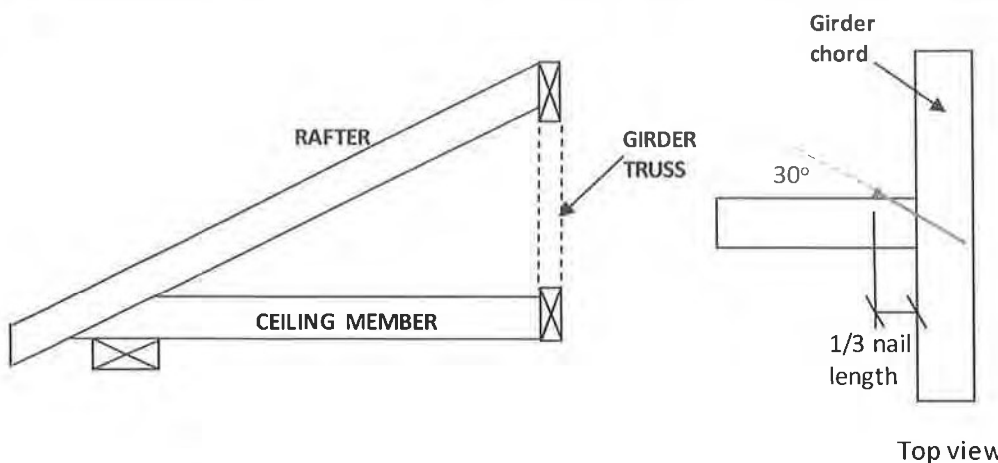


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



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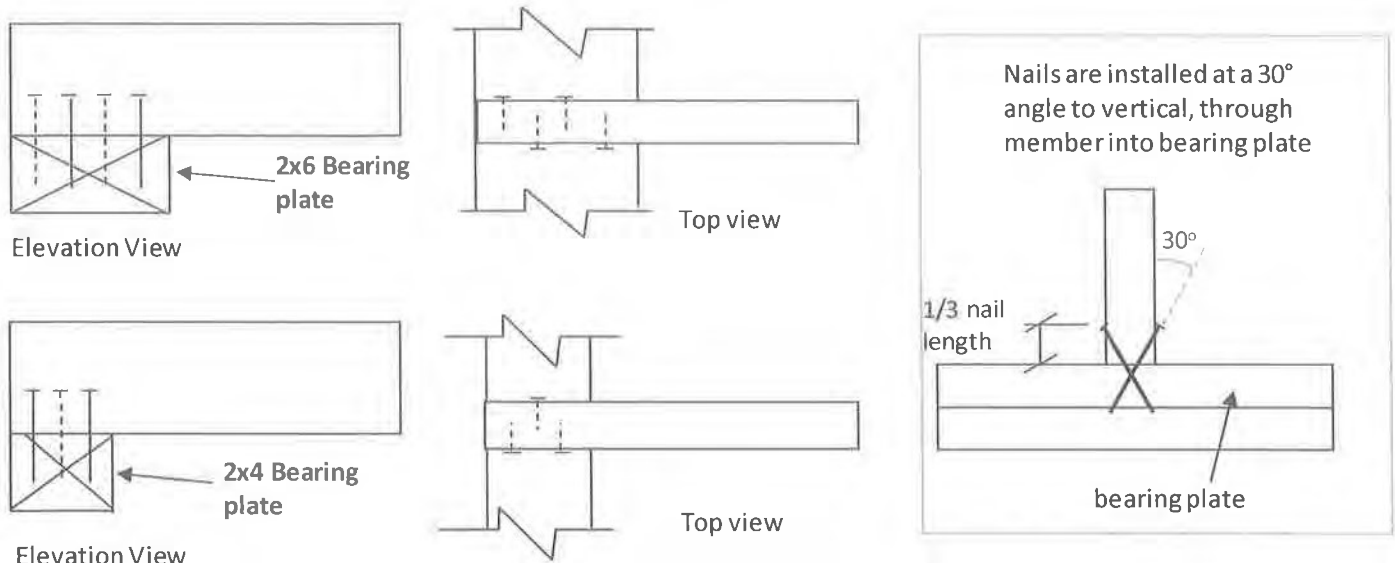
STANDARD DETAIL MSD2015-H

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.



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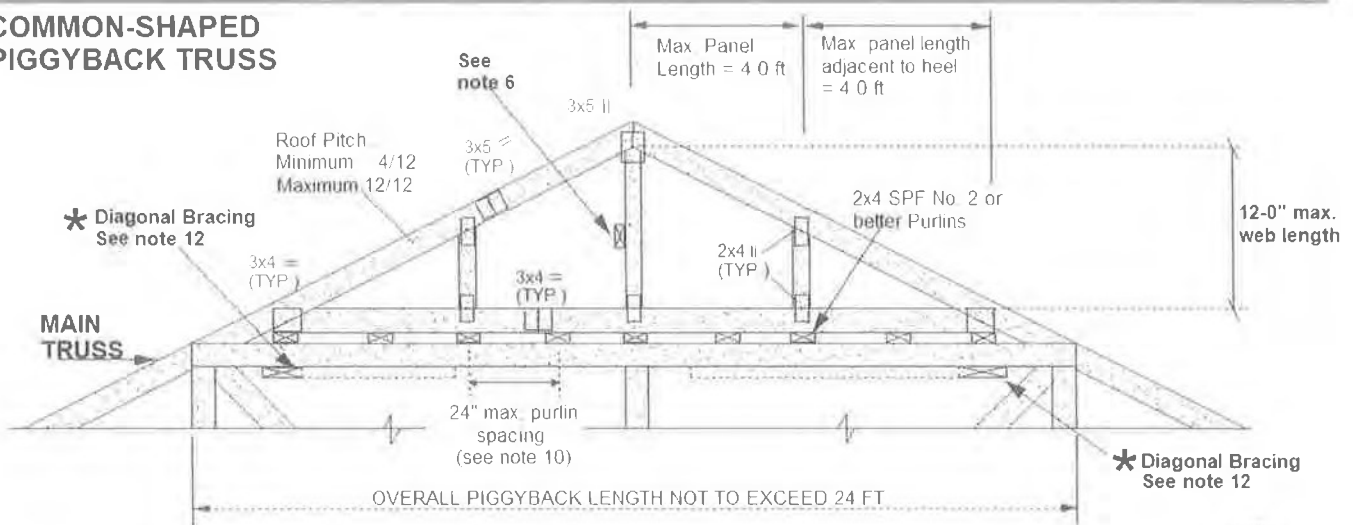
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STANDARD DETAIL MSD2015-I

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STANDARD PIGGYBACK TRUSS

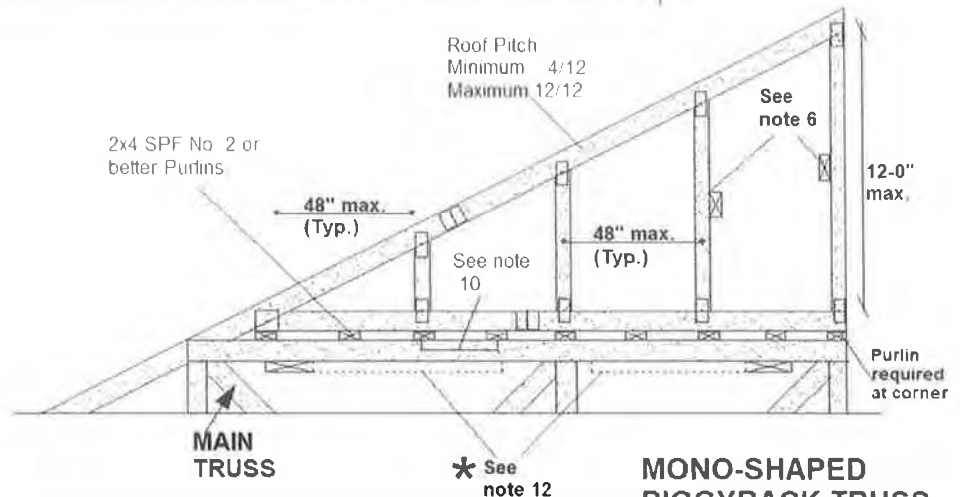
COMMON-SHAPED
PIGGYBACK TRUSS

Specified Load Rating:

Top chord live:	60.0 PSF
Top chord dead:	6.0 PSF
Bottom chord live:	0.0 PSF
Bottom chord dead:	7.3 PSF

Truss spacing not to exceed 24 inches cc

The design requirements and limits shown on this page are applicable to both common shaped and mono-shaped piggybacks. Diagrams are for illustration purposes only.

MONO-SHAPED
PIGGYBACK TRUSS

NOTES:

1. This detail is applicable only to projects conforming to **PART 9 NBCC2015** that do not require a wind analysis to be incorporated into the truss design. Piggyback length not to exceed 24 ft.
2. All piggyback truss lumber to be SPF or D. Fir species and No. 2 DRY (or better) grade.
3. Piggyback top chord size shall be 2x4. Vertical webs and bottom chord sizes shall be 2x3 or 2x4.
4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint. Splices shall be located at 1/4 length of any permissible panel.
5. Maximum web length not to exceed 12.0 ft. Maximum panel lengths not to exceed 48 inches.
6. One continuous lateral brace required at $\frac{1}{2}$ length of all vertical members longer than 6.0 ft. Scab brace or L-brace substitutions are permitted. Scab braces shall be limited to 10 ft. long webs or less. Scab and L-braces shall cover 90% of web length and connected as per MITEK detail MSD2015-K.
7. Piggyback truss must be installed directly on top of each main truss.
8. All plates shall be MiTek MT20, centered at each joint and pressed into both faces of truss.
9. **DO NOT** cantilever mono-shaped piggyback truss over the end of main truss. Piggyback length must match the length of the flat chord of the main truss.
10. Purlin spacing to be equal to main truss maximum unbraced top chord length (as shown on main truss engineering drawing), but not to exceed 24 inches c/c.
11. Purlin design and overall roof system bracing shall be specified by the building designer.
12. Diagonal bracing required. See **'BCSI CANADA -B2C: FIELD ASSEMBLY & OTHER SPECIAL CONDITIONS'**

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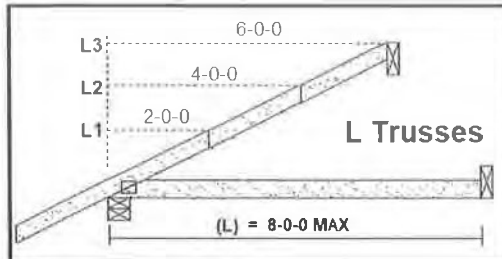
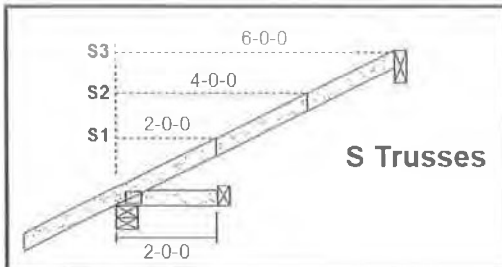
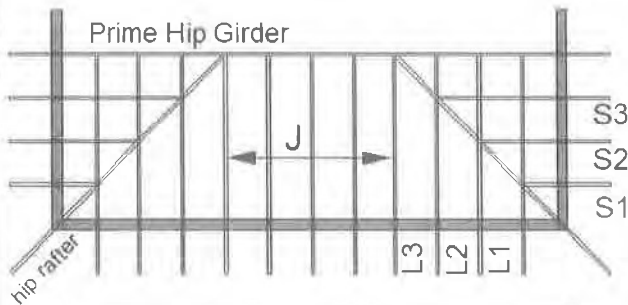
STANDARD DETAIL MSD2015-J

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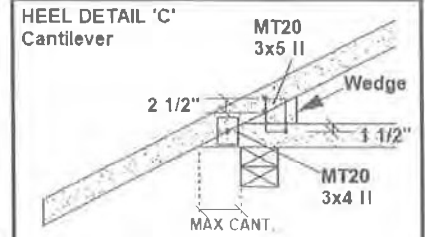
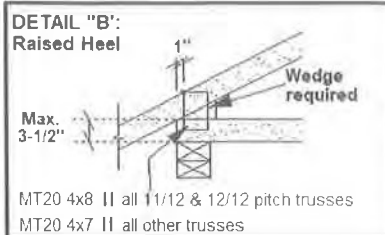
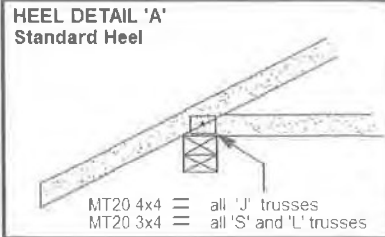
STANDARD HIP END FRAMING

PLAN VIEW



Specified Load Rating:

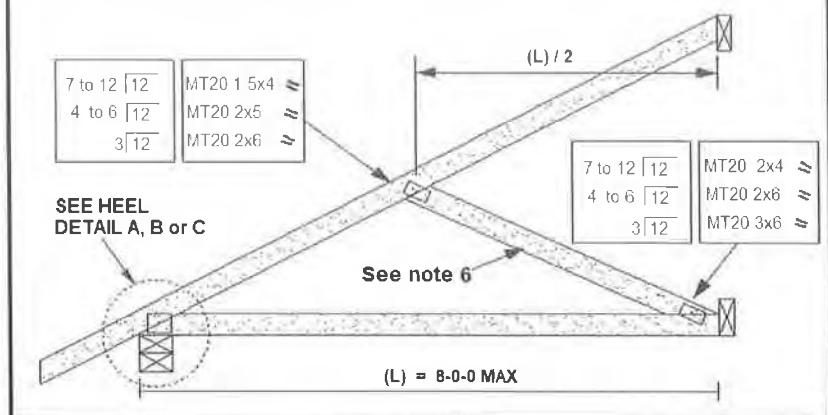
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL 'C'

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 X 5	2 X 3
4/12	14"	3 X 5	2 X 3
5/12	12"	3 X 5	2 X 4
6/12	10"	3 X 5	2 X 4
7/12	9"	3 X 5	2 X 6
8/12	8 5/8"	3 X 5	2 X 6
9/12	8"	3 X 5	2 X 6
10/12	7 5/8"	3 X 5	2 X 6

J Trusses



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NOTES:

- This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
- Overhang length shall not exceed 24 inches.
- All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
- All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
- Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
- For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
- All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'



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STANDARD DETAIL MSD2015-K

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STANDARD GABLE END DETAIL

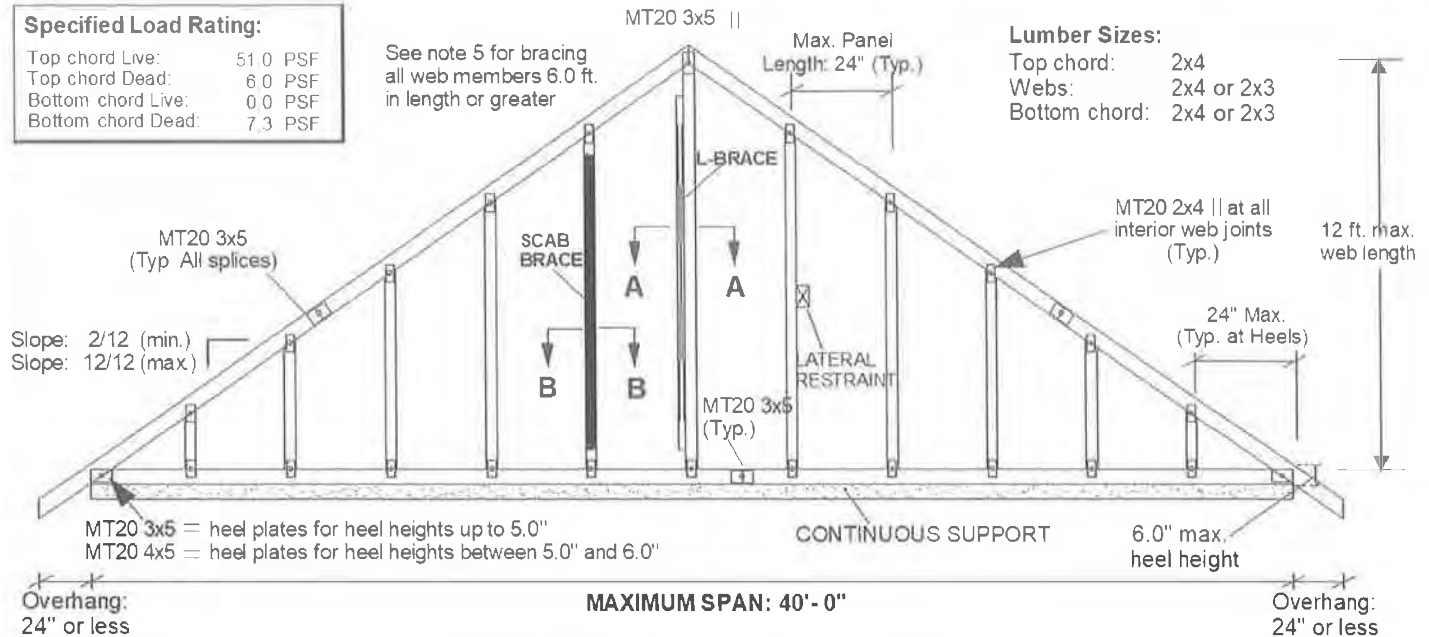
Specified Load Rating:

Top chord Live:	51.0 PSF
Top chord Dead:	6.0 PSF
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF

See note 5 for bracing
all web members 6.0 ft.
in length or greater

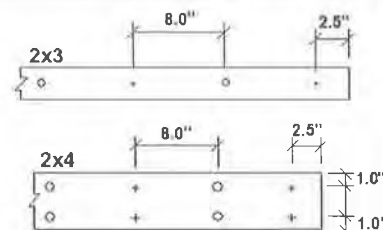
Lumber Sizes:

Top chord:	2x4
Webs:	2x4 or 2x3
Bottom chord:	2x4 or 2x3

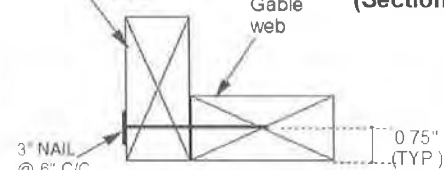
SCAB BRACE DETAIL
(Section B-B)

Gable Web

SPF No. 2 DRY Scab, same
size as web. Scab brace
must cover 90% of web length



- + 0.122"x3.0" nail driven from front face
- o 0.122"x3.0" nail driven from back face

2x4 SPF No. 2
DRY L-BraceL BRACE DETAIL
(Section A-A)

Fasten L-Brace to narrow edge of web with one row of 0.122" x 3.0" nails spaced at 6.0" c/c along entire length of web. Brace must cover 90% of the web length. Respect a 2.5" minimum end distance.

Notes:

1. This detail is only valid for projects conforming to **Part 9, NBCC 2015** that do not require a wind analysis to be incorporated into the design of the truss.
2. This detail is for vertical (gravity) load rating of the truss only. Truss must be continuously supported over the entire length of bottom chord.
3. Maximum web length not to exceed 12.0 ft. Spacing of gable stud webs in the truss not to exceed 24 inches cc.
4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint.
5. Lateral restraint required at half-length of all webs over 6.0 ft. long. Alternatively install an L-Brace or scab brace as shown above. Scab braces shall be limited to 10 ft. long webs or less.
6. All plates are MITEK MT20 pressed into both faces of truss.
7. All lumber to be SPF (or D-Fir) DRY and of No.2 grade or better.
8. Additional building bracing is typically installed to brace the face of the end wall assembly. See BCSI Canada 'Building Designer Responsibilities for Gable End Frame Bracing' for additional information on building bracing for gable-end assemblies.

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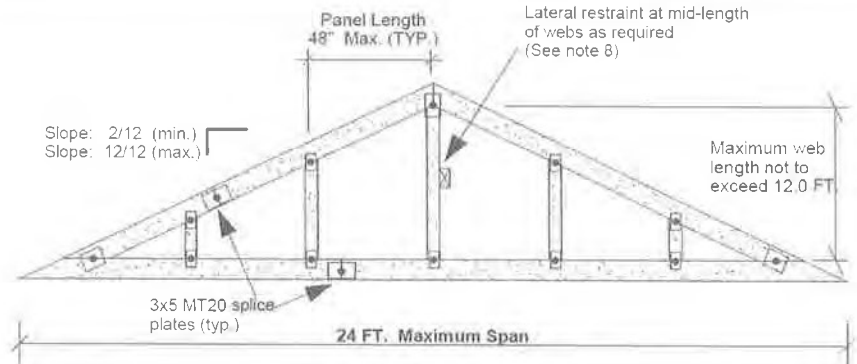
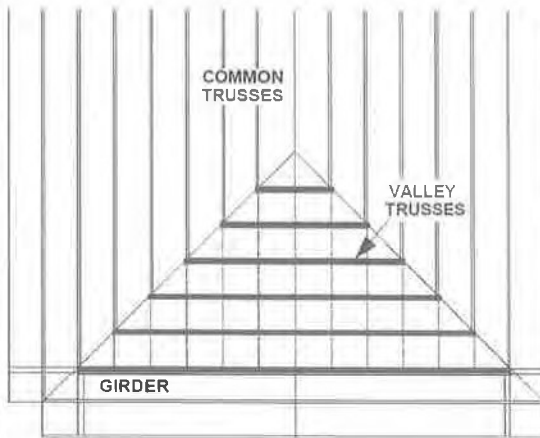
STANDARD DETAIL MSD2015-L

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Expiry: APRIL 30, 2024

VALLEY SET DETAIL

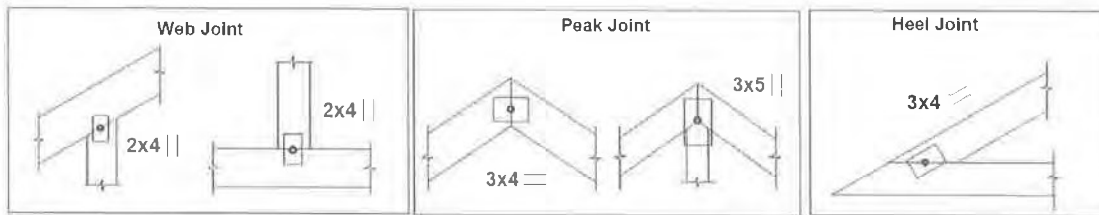
VALLEY SET ROOF FRAMING PLAN



VALLEY TRUSS LUMBER:

Top Chord	2x4
Bottom Chord	2x3 or 2x4
Web	2x3 or 2x4

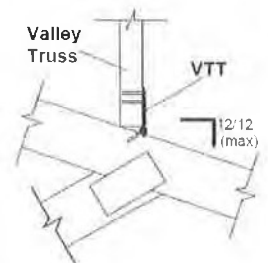
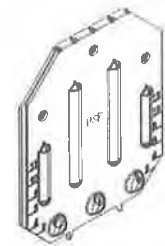
Splices (if required) shall be located at 1/4 point of a panel. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint. If beveling the bottom chord, splice plates shall be flush with top edge of bottom chord.



NOTES:

- Specified roof loads shall not exceed the maximum limits shown below:

Top chord:	Live =	70.0 PSF
Top Chord:	Dead =	6.0 PSF
Bottom Chord:	Live =	0.0 PSF
Bottom Chord:	Dead =	7.3 PSF
- Valley truss design assumes continuous support by the truss system underneath. Spacing of all valley trusses as well as the underlying trusses shall not exceed 24 in. c/c.
- Vertical web spacing in valley trusses not to exceed 48 in. c/c. Web lengths not to exceed 12 ft.
- All lumber to be DRY No. 2 grade or better, SPF or D-Fir.
- Bottom chord may be beveled to match the slope of the intersecting roof. If beveling, a minimum 2x4 bottom chord is required, with a maximum bevel slope of 4/12 (spliced chord) or 8/12 (non-spliced chord). For bevel slopes exceeding the 2x4 limits, use a 2x6 bottom chord.
- Truss plates are MITEK MT20 pressed into both faces of the truss and centered at each joint.
- Use MiTek VTT Valley Truss Ties to attach each valley truss at the intersection point between valley chord and underlying truss chord. Install clips as per product installation instructions. Alternatively, toe-nail valley truss bottom chord at all intersection points with each underlying truss, using two 0.122"x3.25" nails per connection.
- One continuous lateral brace is required at 1/2 length of all webs that exceed 6.0 ft. in length.
- This detail is only valid for residential projects conforming to PART 9 - NBCC2015, that do not require a wind analysis to be incorporated into the design of the trusses.

MITEK VTT
(Valley Truss Tie)

Install as per MiTek product installation instructions

PEO
Certificate No 10889485



WARNING

WARNING

General Notes

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and bracing. Follow the recommendations for handling, installing and temporary restraint and bracing of trusses. Refer to BCSi CANADA - Guide to Good Practice for Handling, Installing, Restraint and Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSi B3C Summary Sheet for more information. An other permanent bracing design is the responsibility of the building designer.

NOTICE The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

NOTICE Les conséquences d'une manipulation, d'une installation, d'une installation, d'un renforcement ou d'un autre bracement inadéquat peuvent entraîner l'effondrement de la structure, ou pire, des blessures graves ou la mort.

NOTICE Exercise care when removing handling and handling trusses to avoid damaging trusses and personnel. Wear personal protective equipment for the eyes, feet, hands and head when working with trusses.

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NOTICE Exercer une attention particulière lors du retrait des supports et des trusses pour éviter d'endommager les trusses et les personnes. Porter une attention particulière lors des conditions ventouseuses du site des lignes électriques et des aéroports.

NOTICE Use special care in windy weather or near power lines and airports. Use caution when working near power lines and airports. Use caution when working near power lines and airports.

NOTICE Utilisez une attention particulière lors des conditions ventouseuses du site des lignes électriques et des aéroports.

NOTICE Utilisez un équipement de levage approprié. Utilisez l'équipement de levage et de levage approprié.

NOTICE Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored for more than one week, place blocking of sufficient height beneath the slack of trusses at (2.4 m) or (8 ft) on center (see).

NOTICE Les trusses peuvent être déchargées directement sur le sol au moment de la livraison ou stockées temporairement en contact avec le sol après la livraison. Si les trusses sont stockées temporairement pendant plus d'une semaine, placez des blocs d'une hauteur suffisante sous le jeu de trusses à une distance de 2,4 m (8 pi) à 3 m (10 pi) d'entretoises.

NOTICE For trusses stored for more than one week, cover bundles to protect from the environment. Pour les trusses entreposées pendant plus d'une semaine, couvrez les lots pour les protéger des intempéries.

NOTICE Refer to BCSi CANADA for more detailed information pertaining to handling and storage of trusses. Reportez-vous au BCSi CANADA pour obtenir des renseignements détaillés concernant la manipulation et l'entreposage des trusses au stade de la construction.

Remarques générales

Les trusses ne sont pas marquées d'aucune façon pour identifier la fréquence ou l'emplacement de la retenue latérale temporaire et du contreventement diagonal. Suivez les recommandations pour la manipulation, l'installation et le contreventement temporaire des trusses. Reportez-vous au BCSi CANADA - Guide to Good Practice for Handling, Installing, Restraint and Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Les dessins de conception de trusses peuvent préciser des emplacements de retenue latérale temporaire ou de renforcement pour des membres de trusses individuelles. Reportez-vous à la fiche sommaire BCSi B3C pour obtenir plus de renseignements. La conception du bracement est la responsabilité de toute autre conception de contreventement.

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Hoisting and Placement of Truss Bundle

NOTICE Do not use the crane. NE S'UTILISER PAS la grue.

NOTICE NEVER use bracing to lift a bundle. N'UTILISER JAMAIS de brayures métalliques seuls pour soulever un lot.

A single lift point may be used for bundles of top chord pitch trusses up to 45 (13.7 m) and parallel chord trusses up to 45 (13.7 m) and parallel chord trusses up to 45 (13.7 m).

Use at least two lift points for bundles of top chord pitch trusses up to 45 (13.7 m) and parallel chord trusses up to 45 (13.7 m). Use at least three lift points for bundles of top chord pitch trusses >45 (13.7 m) and parallel chord trusses >45 (13.7 m).

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Steps to Setting Trusses

Étapes pour placer les fermes

1) Install ground bracing. 2) Set first truss and attach supports to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install top chord member plate diagonal bracing to stabilize the first two trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process with other trusses until all trusses are set.

1) Installer le contreventement de sol. 2) Placer la première ferme, puis fixer la solidement au contreventement de sol. 3) Placer les quatre fermes suivantes avec la retenue latérale temporaire de la membrure courte (ci-dessous). 4) Installer le contreventement diagonal de la membrure supérieure (ci-dessous). 5) Installer le contreventement diagonal de la membrure supérieure de la première ferme (ci-dessous) pour stabiliser les deux premières fermes. 6) Installer la membrure inférieure de la première ferme (ci-dessous). 7) Répéter la procédure par tranches de quatre fermes jusqu'à ce que toutes les fermes soient installées.

NOTICE Refer to BCSi B3C for more information. Reportez-vous au BCSi B3C pour plus de renseignements.

Restraint/Bracing for All Planes of Trusses
Retenue et contreventement pour tous les plans de fermes

Members used for lateral restraint and bracing shall be 2x4 dimension lumber. Attach to each truss with at least 3 common nails (16d x 3) or 3x3 or 3x4 plate (131 x 31 mm).

Les membres utilisés pour la retenue latérale et le contreventement doivent être des membrures de 2x4 (51 mm x 89 mm). Attacher aux trusses avec au moins 3 clous communs (16d x 3) ou 3x3 ou 3x4 plaque (131 x 31 mm).

The restraint and bracing method is for all houses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs.

Cette méthode de retenue et de contreventement convient à toutes les fermes, sauf les fermes à membrure parallèle 3x2 et 4x2 (PCTs). Reportez-vous à la partie supérieure de la colonne suivante pour la retenue et le contreventement temporaires des PCTs.

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Restraint & Bracing for 3x2 and 4x2 Parallel Chord Trusses

Retenue et contreventement pour les fermes à membrure parallèle 3x2 et 4x2

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