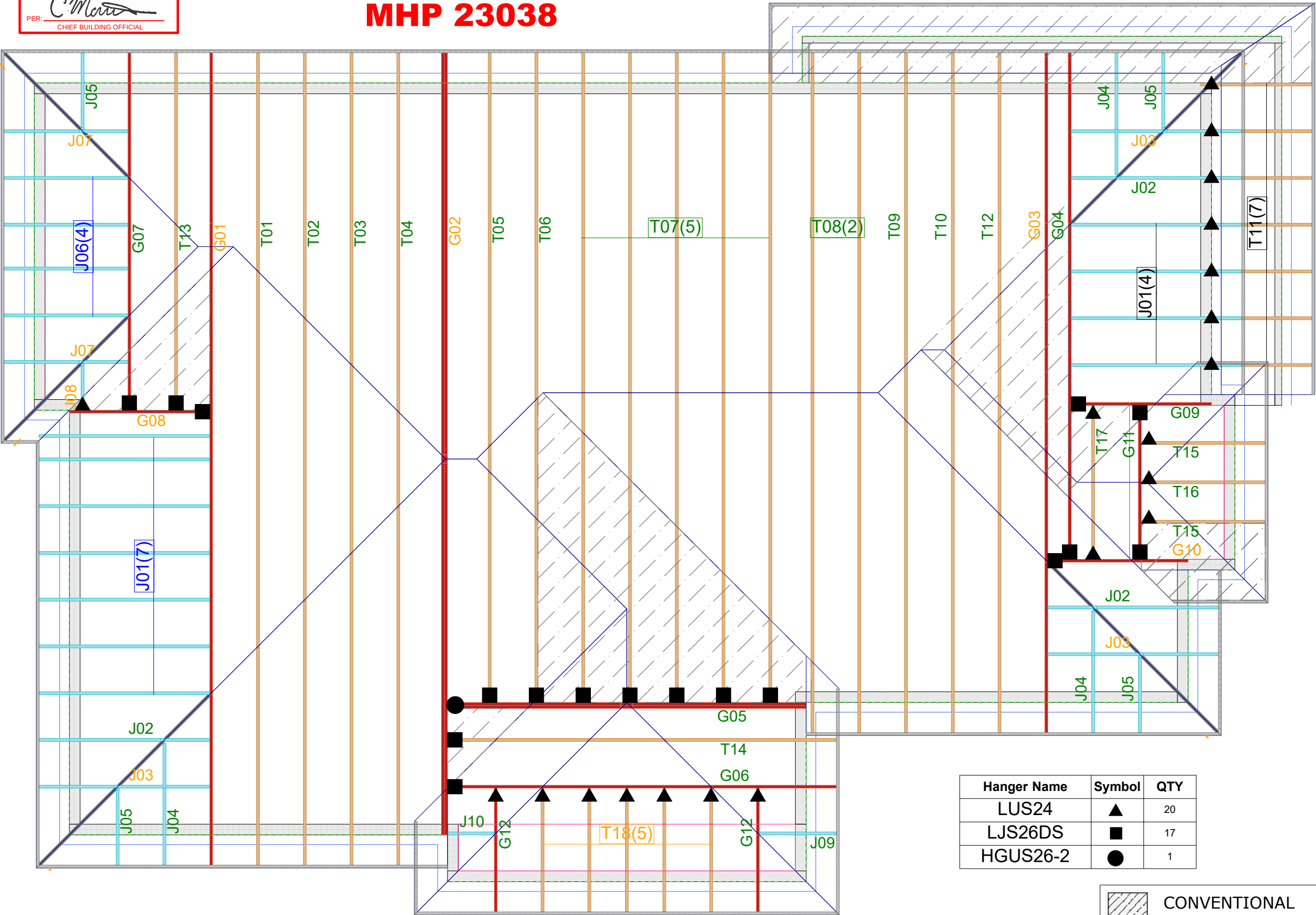


MHP 23038



Hanger Name	Symbol	QTY
LUS24	▲	20
LJS26DS	■	17
HGUS26-2	●	1



CONVENTIONAL
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK HOMES
Job #	23-00118R0
Address	ZADORRA ESTATES OSHAWA,ON
Model	VILLA 11-ELEV 3
Sales Rep	RALPH MIRIGELLO
Designer	RB
Date	6/08/23
Path	C:\MITEK\CA\JOBS\GREENPARK\ZADORRA ESTATES\VILLA 11\VILLA 11-ELEV 3\

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	6.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





Engineering Notes: Trusses

MHP 23038

NE0723
GREEN-ARK-EDBORNA
ESTATES - VILLA 11-3

CORPORATION OF THE CITY OF OSHAWA

TRUE COPY

OF PERMIT PLANS

Dec 06 2023

Charr
CHIEF BUILDING OFFICIAL

PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

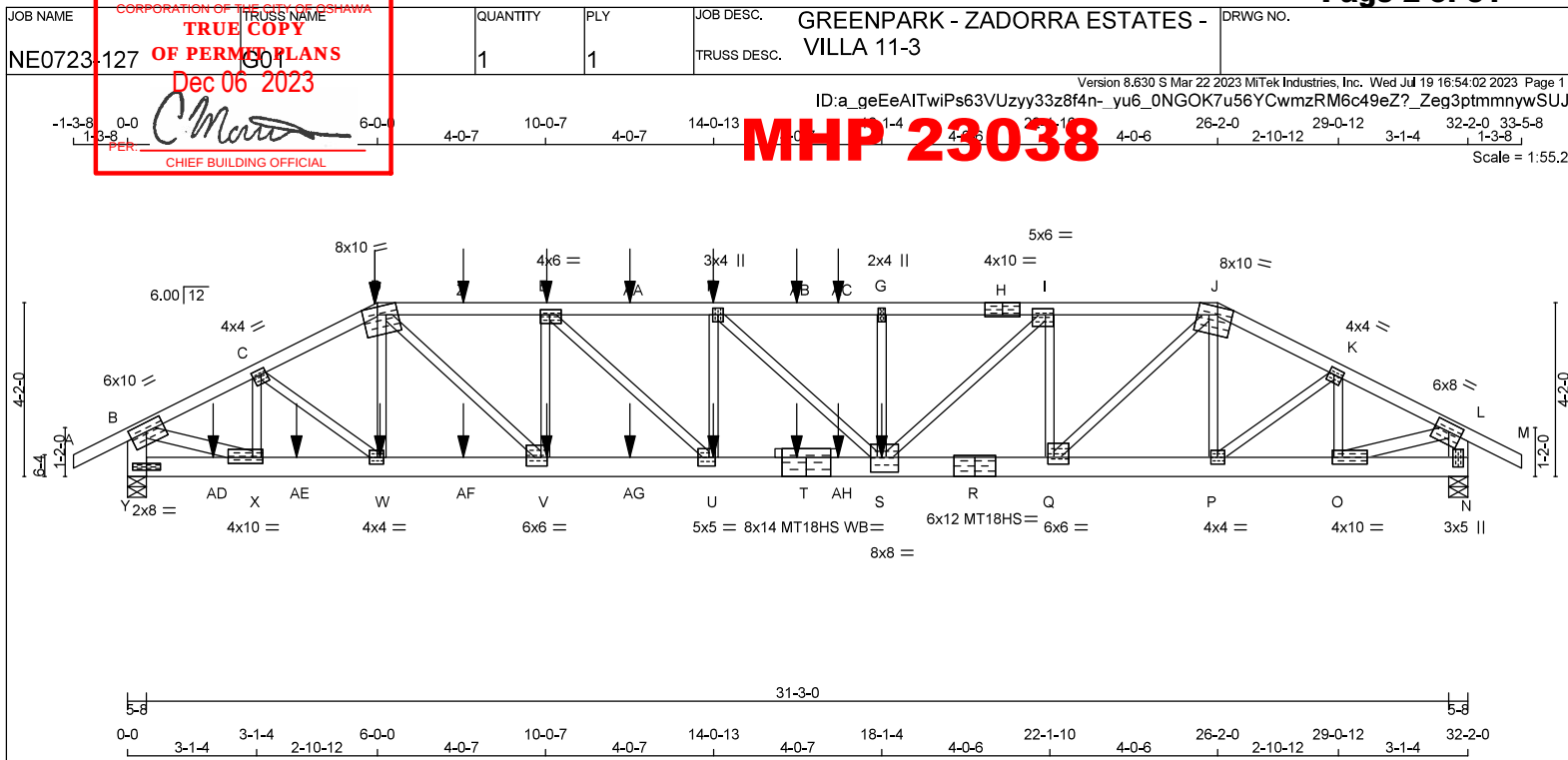
1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
2. THE COMPRESSION CHORDS ARE Laterally Braced by Continuous Rigid Diaphragm Sheathing or as Specified on the Drawing.
3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE BUILDING DESIGNER.
4. IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES SPANNING MORE THAN 12.37M (40'-7").



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	2100F 1.8E	SPF
H - J	2x4	DRY	2100F 1.8E	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - T	2x6	DRY	2100F 1.8E	SPF
T - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV4	MT20	6.0	10.0	2.50 4.75
C	CTMWW4	MT20	4.0	4.0	1.50 1.50
D	TTWV-m	MT20	8.0	10.0	Edge 4.25
E	ETMWW4	MT20	4.0	6.0	1.50 2.75
F	TMWV+T	MT20	3.0	4.0	
G	TMW+T	MT20	2.0	4.0	
H	TS4	MT20	4.0	10.0	Edge 5.00
I	TTMWW4	MT20	5.0	6.0	1.75 2.25
J	TTWV-m	MT20	8.0	10.0	Edge 4.25
K	TMWV4	MT20	4.0	4.0	1.75 1.50
L	BMWV4	MT20	6.0	8.0	2.50 3.75
N	BM+T	MT20	3.0	5.0	2.75 1.50
O	BMWV4	MT20	4.0	10.0	2.00 3.00
P	BMWV4	MT20	4.0	4.0	
Q	BMWV4	MT20	6.0	6.0	2.00 1.75
R	BS4	MT18HS	6.0	12.0	
S	BMWV4+T	MT20	8.0	8.0	4.25 2.00
T	BS4	MT18HS	8.0	14.0	
U	BMWV4	MT20	5.0	5.0	2.50 1.75
V	BMWV4	MT20	6.0	6.0	2.50 1.75
W	BMWV4	MT20	4.0	4.0	2.00 1.75
X	BMWV4	MT20	4.0	10.0	1.75 3.00
Y	BVM1-1	MT20	2.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER						
BEARINGS						
FACTORED			MAXIMUM FACTORED			INPUT
GROSS REACTION			GROSS REACTION			BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
Y	4319	0	4319	0	0	5-8
N	3879	0	3879	0	0	5-8
						4-13

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	3015	2198 / 0	0 / 0	0 / 0	0 / 0	817 / 0	0 / 0
N	2707	1985 / 0	0 / 0	0 / 0	0 / 0	722 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N
BEARING SIZE FACTOR = 1.15 AT JNT(S) Y, N (BASED ON SUPPORT DEPTH = 1-8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 36	-119.4	-119.4	0.17	10.00	X-C	-1405 / 0
B-C	-5767 / 0	-119.4	-119.4	0.63	2.31	C-W	0 / 968
C-D	-6854 / 0	-119.4	-119.4	0.74	1.95	D-D	-305 / 28
D-Z	-8338 / 0	-119.4	-119.4	0.69	2.47	D-V	0 / 3307
Z-E	-8338 / 0	-119.4	-119.4	0.69	2.47	V-E	-2200 / 0
E-AA	-9799 / 0	-119.4	-119.4	0.81	2.13	E-U	0 / 2004
AA-F	-9799 / 0	-119.4	-119.4	0.81	2.13	U-F	-1168 / 0
F-AB	-10153 / 0	-119.4	-119.4	0.89	1.97	S-G	0 / 486
AB-G	-10153 / 0	-119.4	-119.4	0.89	1.97	G-H	-851 / 0
AC-G	-10153 / 0	-119.4	-119.4	0.89	1.97	S-I	0 / 2908
G-H	-10153 / 0	-119.4	-119.4	0.74	2.19	Q-I	-2492 / 0
H-I	-10153 / 0	-119.4	-119.4	0.74	2.19	P-J	0 / 3791
I-J	-8034 / 0	-119.4	-119.4	0.55	2.70	P-Q	-304 / 0
J-K	-5921 / 0	-119.4	-119.4	0.61	2.29	P-K	0 / 854
K-L	-5134 / 0	-119.4	-119.4	0.53	2.60	K-K	-1291 / 0
L-L	-177 / 36	-119.4	-119.4	0.0	0.0	B-X	0 / 5327
Y-B	-4196 / 0	0.0	0.0	0.31	5.13	L-O	0 / 4744
N-L	-3770 / 0	0.0	0.0	0.27	5.39		0.84

YD-AD	0/0	-18.2	-18.2	0.11 (†)	10.00
AX-AE	0/0	-18.2	-18.2	0.11 (†)	10.00
XE-W	0/5169	-18.2	-18.2	0.38 (†)	10.00
AE-W	0/5169	-18.2	-18.2	0.38 (†)	10.00
WF-AF	0/5925	-18.2	-18.2	0.36 (†)	10.00
AF-V	0/5925	-18.2	-18.2	0.36 (†)	10.00
V-AG	0/8338	-18.2	-18.2	0.50 (†)	10.00
AG-U	0/8338	-18.2	-18.2	0.50 (†)	10.00
U-T	0/9799	-18.2	-18.2	0.65 (†)	10.00
T-AH	0/9799	-18.2	-18.2	0.65 (†)	10.00
AH-S	0/9799	-18.2	-18.2	0.65 (†)	10.00
S-R	0/8034	-18.2	-18.2	0.55 (†)	10.00
R-Q	0/8034	-18.2	-18.2	0.55 (†)	10.00
Q-P	0/5270	-18.2	-18.2	0.32 (†)	10.00
P-O	0/4603	-18.2	-18.2	0.34 (†)	10.00
O-N	0/0	-18.2	-18.2	0.10 (†)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX-	FACE	DIR.	TYPE	HEEL	CONN
D	6-0-0	-418	-418	—	BACK	VERT	TOTAL	—	C1
E	10-0-12	-102	-102	—	BACK	VERT	TOTAL	—	C1
F	14-0-12	-102	-102	—	BACK	VERT	TOTAL	—	C1
S	18-1-4	-1181	-1181	—	BACK	VERT	TOTAL	—	C1
T	16-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
U	14-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
V	10-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
W	6-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
Z	8-0-12	-102	-102	—	BACK	VERT	TOTAL	—	C1
AA	12-0-12	-102	-102	—	BACK	VERT	TOTAL	—	C1
AB	16-0-12	-102	-102	—	BACK	VERT	TOTAL	—	C1
AC	17-0-12	-102	-102	—	BACK	VERT	TOTAL	—	C1
AD	2-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD
CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 858 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 503 (0.77")

CSI: TC=0.89/0.97 (F-G:1), BC=0.65/0.97 (S-U:1),
WB=0.94/0.97 (B-X:1), SSI=0.35/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAIL VALUES						
	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL = 0.96 (D) (INPUT = 1.00)

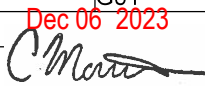
CONTINUED ON PAGE 2



JOB NAME	CORPORATION OF THE CITY OF BSHAWA	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-127	TRUE COPY OF PERMIT PLANS Dec 06 2023	1	1	TRUSS DESC.	VILLA 11-3	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 19 16:54:02 2023 Page 2

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PER: 
 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AE	4-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
AF	8-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
AG	12-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1
AH	17-0-12	-22	-22	—	BACK	VERT	TOTAL	—	C1

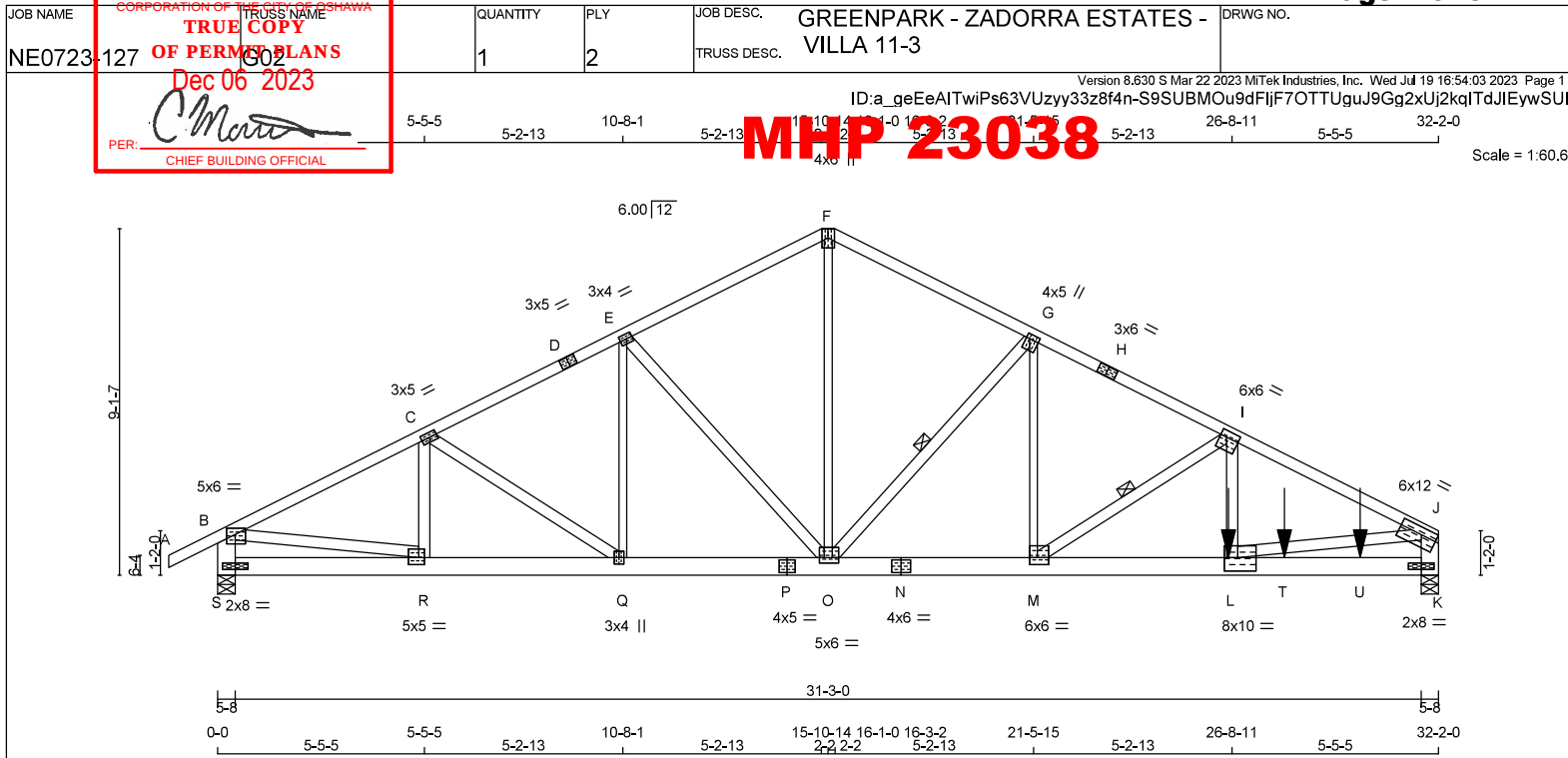
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

MHP 23038

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 169 = 337 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY 2100F 1.8E	SPF
S - B	2x6	DRY No.2	SPF
K - J	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - N	2x6	DRY No.2	SPF
N - K	2x6	DRY 2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF
Q - E	2x3	DRY No.2	SPF
M - G	2x3	DRY No.2	SPF
B - R	2x4	DRY 2100F 1.8E	SPF
L - J	2x4	DRY 2100F 1.8E	SPF
O - F	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-F 1	12	TOP
F-H 1	12	TOP
H-J 1	12	TOP
S-B 2	12	TOP
K-J 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P 2	12	TOP
P-N 2	12	TOP
N-K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	SIDE(0.0)
2x4 1	6	SIDE(0.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	3866	0	3866	0
K	10997	0	10997	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
S	2697	1978 / 0	0 / 0
K	7669	5640 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
 BEARING SIZE FACTOR = 1.15 AT JNT(S) S, K (BASED ON SUPPORT DEPTH = 1-8)**BRACING**TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, DBS = 6-0-0, CBF = 174 LBS.
 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF H-M, DBS = 4-0-0, CBF = 201 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 36	-119.4 -119.4 0.09 (1)	10.00	R-C	-680 / 0	0.06 (1)	
B-C	-5836 / 0	-119.4 -119.4 0.47 (1)	3.66	C-Q	-14 / 0	0.00 (1)	
C-D	-5848 / 0	-119.4 -119.4 0.46 (1)	3.84	Q-E	0 / 106	0.02 (4)	
D-E	-5848 / 0	-119.4 -119.4 0.46 (1)	3.84	E-O	-721 / 0	0.36 (1)	
E-F	-5339 / 0	-119.4 -119.4 0.44 (1)	3.83	O-G	-641 / 0	0.76 (1)	
F-G	-5342 / 0	-119.4 -119.4 0.47 (1)	3.78	M-G	0 / 4280	0.53 (1)	
G-H	-8738 / 0	-119.4 -119.4 0.75 (1)	2.79	M-I	-8021 / 0	0.82 (1)	
H-I	-8738 / 0	-119.4 -119.4 0.75 (1)	2.79	L-I	0 / 7155	0.63 (1)	
I-J	-16196 / 0	-119.4 -119.4 0.77 (1)	2.68	B-R	0 / 5295	0.22 (1)	
S-B	-3798 / 0	0.0 0.0 0.14 (1)	7.16	L-J	0 / 14644	0.60 (1)	
K-J	-9606 / 0	0.0 0.0 0.35 (1)	4.79	O-F	0 / 4228	0.52 (1)	
S-R	0 / 0	-18.2 -18.2 0.05 (1)	10.00				
R-Q	0 / 5242	-18.2 -18.2 0.39 (1)	10.00				
Q-P	0 / 5231	-18.2 -18.2 0.39 (1)	10.00				
P-O	0 / 5231	-18.2 -18.2 0.39 (1)	10.00				
O-N	0 / 7829	-18.2 -18.2 0.56 (1)	10.00				
N-M	0 / 7829	-18.2 -18.2 0.56 (1)	10.00				
M-L	0 / 14498	-18.2 -18.2 0.52 (1)	10.00				
L-T	0 / 0	-18.2 -18.2 0.57 (1)	10.00				
T-U	0 / 0	-18.2 -18.2 0.57 (1)	10.00				
U-K	0 / 0	-18.2 -18.2 0.57 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	26-7-9	-5090	-5090	—	BACK	VERT	TOTAL	—	C1
T	28-1-4	-723	-723	—	BACK	VERT	TOTAL	—	C1
U	30-1-4	-1348	-1348	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (1.07")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
 ALLOWABLE DEFL.(TL) = L/960 (1.07")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.35")CSI: TC=0.77/0.97 (I-J:1), BC=0.57/0.97 (K-L:1),
 WB=0.82/0.97 (H-M:1), SS=0.56/1.00 (K-L:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
 JSI METAL= 1.00 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	CORPORATION OF THE CITY OF BSHAWA	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-127	TRUSS NAME TRUE COPY OF PERMIT PLANS Dec 06 2023 PER: <i>C. Morris</i> CHIEF BUILDING OFFICIAL	1	2	GREENPARK - ZADORRA ESTATES - VILLA 11-3	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 19 16:54:03 2023 Page 2

ID: a_geEeAITwiPs63VUzyy33z8f4n-S9SUBMOu9dFijF7OTTUguJ9Gg2xUj2kgITdJIEywSU

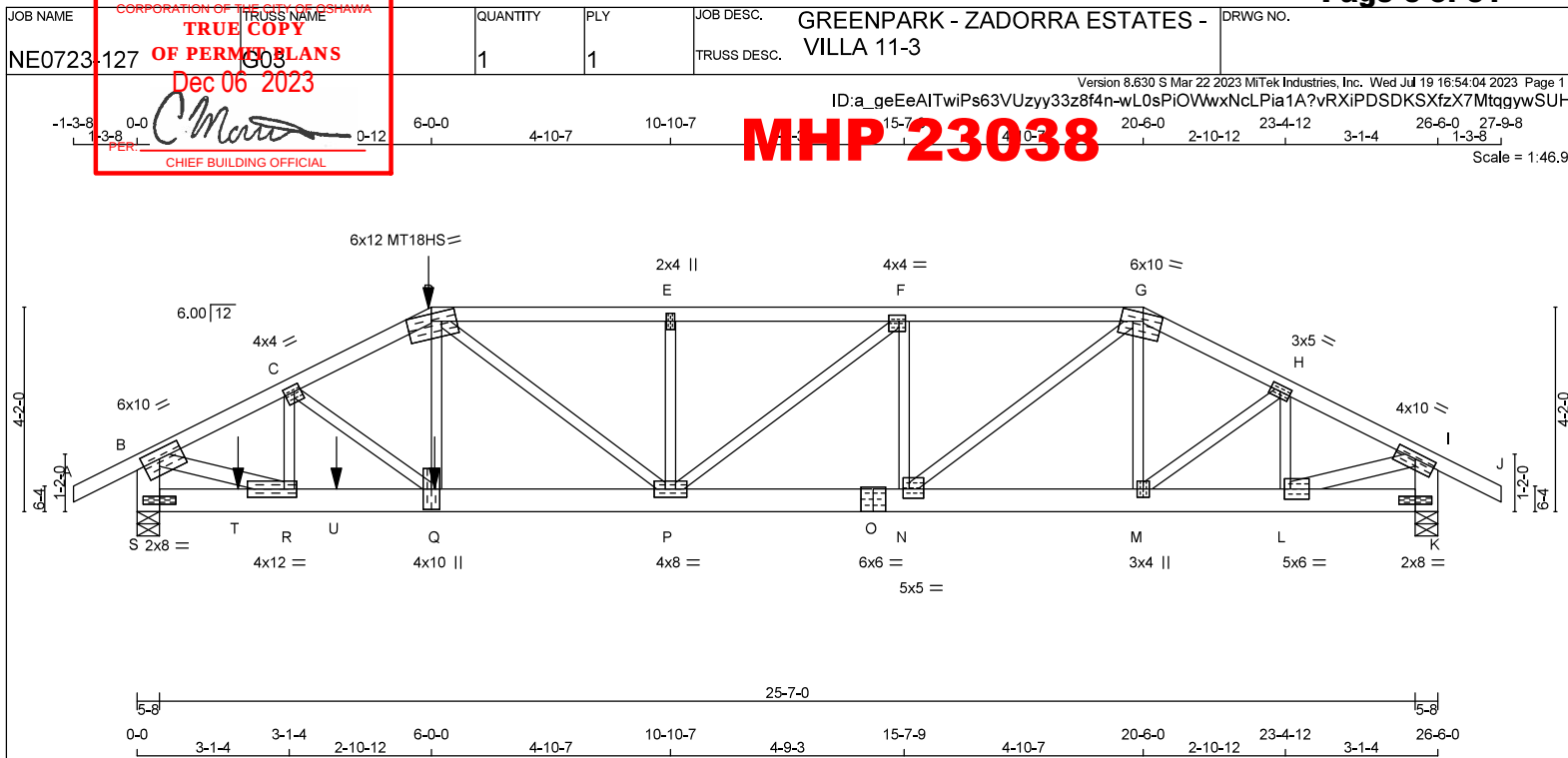
MHP 23038**PLATES** (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	3.0	5.0	1.50	2.25
D	TS-t	MT20	3.0	5.0		
E	TMWW-t	MT20	3.0	4.0	1.50	1.75
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	5.0	1.50	1.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	6.0	6.0	2.00	2.00
J	TMVW-t	MT20	6.0	12.0	2.25	Edge
K	BVM1-t	MT20	2.0	8.0		
L	BMWW-t	MT20	8.0	10.0	4.25	4.25
M	BMWW-t	MT20	6.0	6.0	2.25	2.50
N	BS-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0	1.75	1.50
P	BS-t	MT20	4.0	5.0		
Q	BMWW-t	MT20	3.0	4.0		
R	BMWW-t	MT20	5.0	5.0	2.25	1.75
S	BVM1-t	MT20	2.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
B - R	2x4	DRY	2100F 1.8E	SPF
L - I	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWW-H	MT20	6.0	10.0	2.25 4.25
C	TMWW-H	MT20	4.0	4.0	1.50 1.50
D	TTWW-m	MT18HS	6.0	12.0	Edge
E	TMWW-H	MT20	2.0	4.0	
F	TMWW-H	MT20	4.0	4.0	1.50 1.50
G	TTWW-m	MT20	6.0	10.0	2.00 4.50
H	TMWW-H	MT20	3.0	5.0	
I	TMWW-H	MT20	4.0	10.0	1.50 5.00
K	BVM1-I	MT20	2.0	8.0	
L	BMWW-H	MT20	5.0	6.0	2.50 1.50
M	BMWW-H	MT20	3.0	4.0	
N	BMWW-H	MT20	5.0	5.0	2.25 1.50
O	BS-I	MT20	6.0	6.0	
P	BMWW-H	MT20	4.0	8.0	
Q	BMWW-H	MT20	4.0	10.0	
R	BMWW-H	MT20	4.0	12.0	2.00 3.00
S	BVM1-I	MT20	2.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	4847	0	4847	0
K	2832	0	2832	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
S	3377	2503 / 0	0 / 0	0 / 0	0 / 0	874 / 0	0 / 0	
K	1974	1460 / 0	0 / 0	0 / 0	0 / 0	513 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) S, K (BASED ON SUPPORT DEPTH = 1-8")**BRACING**TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 36	-119.4	-119.4	0.17 (1)	10.00	R-C	-1456 / 0	0.25 (1)
B-C	-6575 / 0	-119.4	-119.4	0.78 (1)	1.90	C-Q	0 / 1030	0.25 (1)
C-D	-7512 / 0	-119.4	-119.4	0.91 (1)	1.44	Q-D	0 / 2633	0.65 (1)
D-E	-6618 / 0	-119.4	-119.4	0.54 (1)	2.98	D-P	-199 / 0	0.13 (1)
E-F	-6618 / 0	-119.4	-119.4	0.54 (1)	2.98	P-E	-608 / 0	0.16 (1)
F-G	-5559 / 0	-119.4	-119.4	0.47 (1)	3.33	P-F	0 / 1354	0.34 (1)
G-H	-4049 / 0	-119.4	-119.4	0.38 (1)	3.12	N-F	-1463 / 0	0.38 (1)
H-I	-3634 / 0	-119.4	-119.4	0.34 (1)	3.34	N-G	0 / 2480	0.61 (1)
I-J	0 / 36	-119.4	-119.4	0.17 (1)	10.00	M-G	-97 / 34	0.03 (1)
S-B	-4742 / 0	0.0	0.0	0.34 (1)	4.82	M-H	0 / 431	0.11 (1)
K-I	-2761 / 0	0.0	0.0	0.20 (1)	6.16	L-H	-869 / 0	0.15 (1)
						B-R	0 / 6069	0.50 (1)
						L-I	0 / 3363	0.28 (1)
S-T	0 / 0	-18.2	-18.2	0.08 (1)	10.00			
T-R	0 / 0	-18.2	-18.2	0.08 (1)	10.00			
R-U	0 / 5889	-18.2	-18.2	0.40 (1)	10.00			
U-Q	0 / 5889	-18.2	-18.2	0.40 (1)	10.00			
Q-P	0 / 6775	-18.2	-18.2	0.43 (1)	10.00			
P-O	0 / 5559	-18.2	-18.2	0.78 (1)	10.00			
O-N	0 / 5559	-18.2	-18.2	0.78 (1)	10.00			
N-M	0 / 3601	-18.2	-18.2	0.52 (1)	10.00			
M-L	0 / 3263	-18.2	-18.2	0.52 (1)	10.00			
L-K	0 / 0	-18.2	-18.2	0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-0-0	-311	-311	—	FRONT	VERT	TOTAL	—	C1
Q	6-0-12	-2226	-2226	—	FRONT	VERT	TOTAL	—	C1
T	2-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
U	4-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL	LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/846 (0.38")CSI: TC=0.91/0.97 (C-D-1), BC=0.78/0.97 (N-P-1),
WB=0.65/0.97 (D-Q-1), SSI=0.29/1.00 (F-G-1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT18HS	586	403	2455
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

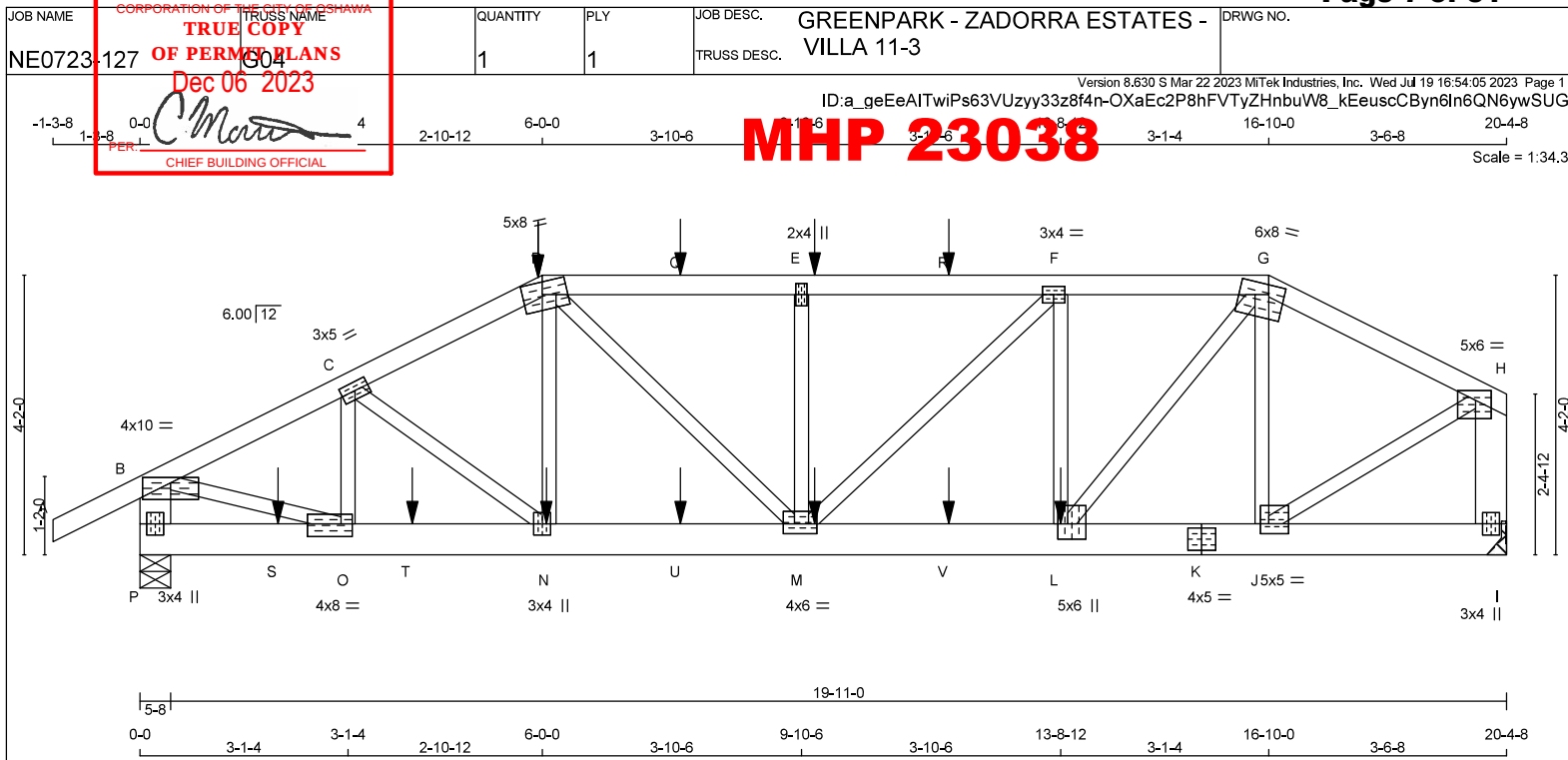
JSI GRIP= 0.90 (N) (INPUT = 0.90)

JSI METAL = 0.97 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - G	2x4	DRY No.2
G - H	2x4	DRY No.2
P - B	2x6	DRY No.2
I - H	2x6	DRY No.2
P - K	2x6	DRY No.2
K - I	2x6	DRY No.2

ALL WEBS EXCEPT 2x3 DRY No.2

SPF DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	10.0	1.00	5.00
C	TMVW+4	MT20	3.0	5.0		
D	TTVW+m	MT20	5.0	8.0	2.25	3.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+4	MT20	3.0	4.0		
G	TTVW+m	MT20	6.0	8.0	1.75	2.75
H	TMVW+p	MT20	5.0	6.0	1.75	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMVW+4	MT20	5.0	5.0	1.75	1.50
K	BS+4	MT20	4.0	5.0		
L	BMVW+t	MT20	5.0	6.0	2.75	1.75
M	BMVW+4	MT20	4.0	6.0	1.75	2.00
N	BMVW+t	MT20	3.0	4.0		
O	BMVW+4	MT20	4.0	8.0	2.25	2.00
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2716	0	2716	0	0	5-8	3-5
I	2655	0	2655	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-2.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1895	1389 / 0	0 / 0	0 / 0	0 / 0	505 / 0	0 / 0
I	1852	1363 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO				
A-B	0 / 36	-119.4	-119.4	0.17 (1)	10.00	O-C	-787 / 0	0.14 (1)
B-C	-3438 / 0	-119.4	-119.4	0.32 (1)	3.44	C-N	0 / 355	0.09 (1)
C-D	-3784 / 0	-119.4	-119.4	0.34 (1)	3.28	N-D	-14 / 104	0.04 (4)
D-Q	-4094 / 0	-119.4	-119.4	0.66 (1)	2.71	D-M	0 / 1018	0.25 (1)
Q-E	-4094 / 0	-119.4	-119.4	0.66 (1)	2.71	M-E	-840 / 0	0.22 (1)
E-R	-4094 / 0	-119.4	-119.4	0.66 (1)	2.71	M-F	0 / 327	0.08 (1)
R-F	-4094 / 0	-119.4	-119.4	0.66 (1)	2.71	L-F	-737 / 0	0.19 (1)
F-G	-3861 / 0	-119.4	-119.4	0.42 (1)	3.13	L-G	0 / 2500	0.62 (1)
G-H	-2581 / 0	-119.4	-119.4	0.40 (1)	3.81	J-G	-1136 / 0	0.29 (1)
P-B	-2632 / 0	0.0	0.0	0.19 (1)	6.29	B-O	0 / 3182	0.79 (1)
I-H	-2638 / 0	0.0	0.0	0.22 (1)	6.29	J-H	0 / 2632	0.65 (1)
P-S	0 / 0	-18.2	-18.2	0.11 (1)	10.00			
S-O	0 / 0	-18.2	-18.2	0.11 (1)	10.00			
O-T	0 / 3087	-18.2	-18.2	0.51 (1)	10.00			
T-N	0 / 3087	-18.2	-18.2	0.51 (1)	10.00			
N-U	0 / 3367	-18.2	-18.2	0.50 (1)	10.00			
U-M	0 / 3367	-18.2	-18.2	0.50 (1)	10.00			
M-V	0 / 3861	-18.2	-18.2	0.61 (1)	10.00			
V-L	0 / 3861	-18.2	-18.2	0.61 (1)	10.00			
L-K	0 / 2275	-18.2	-18.2	0.38 (1)	10.00			
K-J	0 / 2275	-18.2	-18.2	0.38 (1)	10.00			
J-I	0 / 0	-18.2	-18.2	0.04 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
D	8-0-0	-418	-418	—	BACK	VERT	TOTAL	C1
E	10-0-12	-102	-102	—	BACK	VERT	TOTAL	—
L	13-8-12	-821	-821	—	BACK	VERT	TOTAL	—
M	10-0-12	-22	-22	—	BACK	VERT	TOTAL	—
N	6-0-12	-22	-22	—	BACK	VERT	TOTAL	—
Q	8-0-12	-102	-102	—	BACK	VERT	TOTAL	—
R	12-0-12	-102	-102	—	BACK	VERT	TOTAL	—
S	2-0-12	-22	-22	—	BACK	VERT	TOTAL	—
T	4-0-12	-22	-22	—	BACK	VERT	TOTAL	—
U	8-0-12	-22	-22	—	BACK	VERT	TOTAL	—
V	12-0-12	-22	-22	—	BACK	VERT	TOTAL	—

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")CSI: TC=0.68/0.97 (E-F:1), BC=0.61/0.97 (L-M:1),
WB=0.79/0.97 (B-O:1), SSI=0.32/1.00 (D-E:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

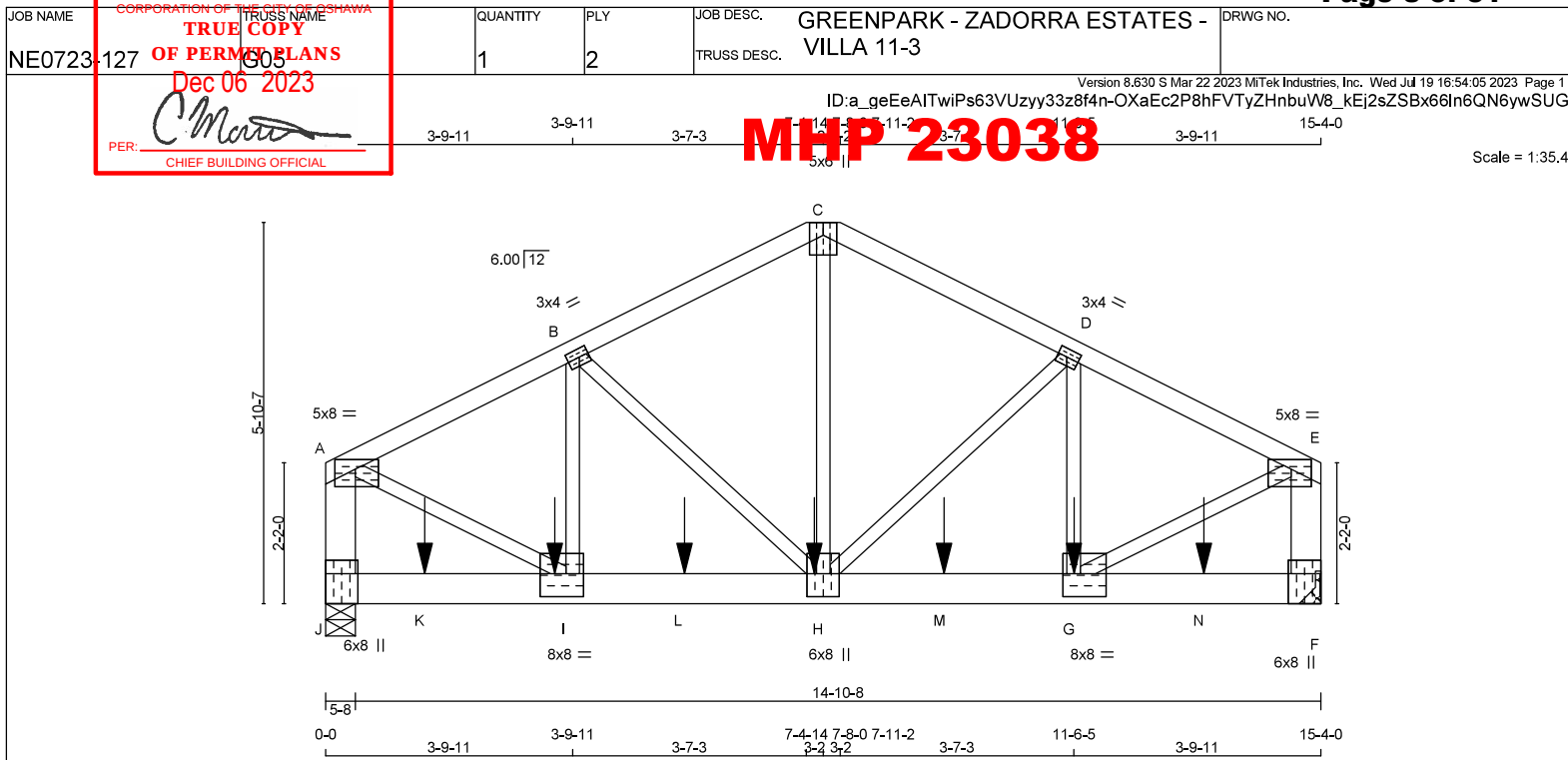
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)

JSI METAL = 0.62 (J) (INPUT = 1.00)

**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-E 1	12	TOP
J-A 2	12	TOP
F-E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J-F 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	1.75 3.75
B	TMWW-H	MT20	3.0	4.0	1.50 1.75
C	TTWW-p	MT20	5.0	6.0	Edge

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
J	7440	0	7440	0
F	7313	0	7313	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 4-0.

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
J	5193 3795 / 0 0 / 0 0 / 0 1398 / 0 0 / 0
F	5104 3730 / 0 0 / 0 0 / 0 1374 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J
 BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	-7299 / 0	-119.4	-119.4 0.33 (1)	3.37	I-B 0 / 715
B-C	-6376 / 0	-119.4	-119.4 0.29 (1)	3.63	B-H -1175 / 0
C-D	-6376 / 0	-119.4	-119.4 0.29 (1)	3.63	H-D -1186 / 0
D-E	-7308 / 0	-119.4	-119.4 0.33 (1)	3.37	G-D 0 / 727
J-A	-6514 / 0	0.0	0.0 0.26 (1)	5.75	A-I 0 / 7216
F-E	-6522 / 0	0.0	0.0 0.26 (1)	5.74	G-E 0 / 7225
				H-C	0 / 5292
J-K	0 / 0	-18.2	-18.2 0.51 (1)	10.00	
K-I	0 / 0	-18.2	-18.2 0.51 (1)	10.00	
I-L	0 / 6541	-18.2	-18.2 0.79 (1)	10.00	
L-H	0 / 6541	-18.2	-18.2 0.79 (1)	10.00	
H-M	0 / 6549	-18.2	-18.2 0.79 (1)	10.00	
M-G	0 / 6549	-18.2	-18.2 0.79 (1)	10.00	
G-N	0 / 0	-18.2	-18.2 0.51 (1)	10.00	
N-F	0 / 0	-18.2	-18.2 0.51 (1)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	11-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1
H	7-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1
I	3-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1
K	1-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1
L	5-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1
M	9-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1
N	13-6-6	-1260	-1260	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, NBC-2019AE
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.51")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
 ALLOWABLE DEFL. (TL) = L/360 (0.51")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CSI: TC=0.33/0.97 (D-E-1), BC=0.79/0.97 (H-I-1),
 WB=0.89/0.97 (E-G-1), SSI=0.36/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
 JSI METAL= 0.57 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	CORPORATION OF THE CITY OF BSHAWA	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES - VILLA 11-3	DRWG NO.
NE0723-127	TRUE COPY OF PERMIT PLANS Dec 06 2023 1905	1	2	TRUSS DESC.		

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 19 16:54:06 2023 Page 2

ID:a_geEeAITwiPs63VUzzy33z8f4n-sk8cqOQmSYdKajrz9b2NWynuoGvhwOMG_Rr_vYywsUF

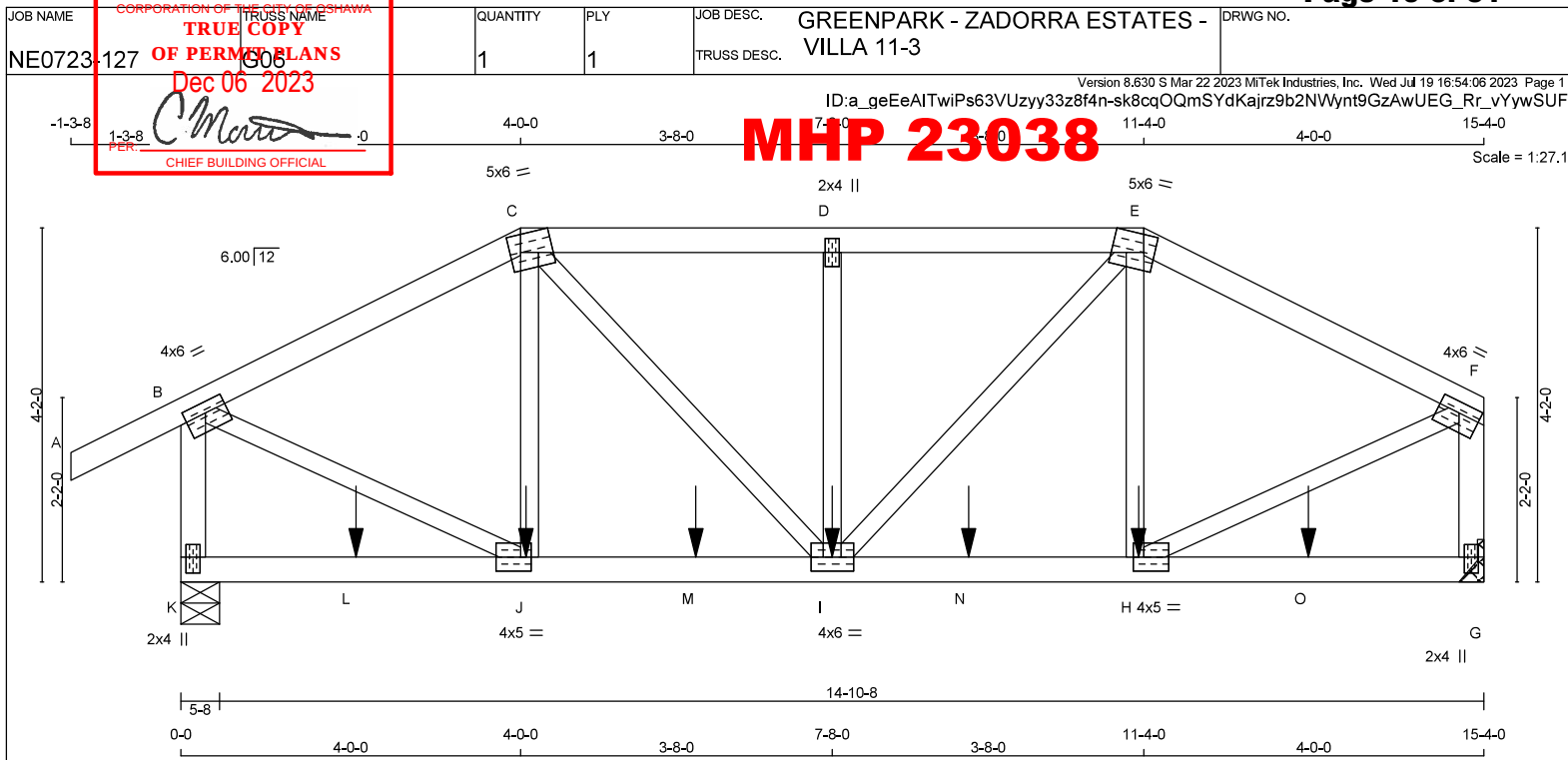
PLATES	Ita	le	is	in
JT	TYPE	PLATES	CHIEF	BUILDING
D	TMWW-H	MT20	3.0	4.0
E	TMWW-p	MT20	5.0	8.0
F	BMV1+t	MT20	6.0	8.0
G	BMWW-H	MT20	8.0	8.0
H	BMWW-H+t	MT20	6.0	8.0
I	BMWW-H	MT20	8.0	8.0
J	BMV1+t	MT20	6.0	8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

MHP 23038

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 65 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-H	MT20	4.0	6.0	1.50	3.00
C	TTVW-H-m	MT20	5.0	6.0	Edge	4.50
D	TMVW-H	MT20	2.0	4.0		
E	TTVW-H-m	MT20	5.0	6.0	Edge	4.50
F	TMVW-H	MT20	4.0	6.0	1.50	3.00
G	BMV1+p	MT20	2.0	4.0	2.25	1.00
H	BMVW-H	MT20	4.0	5.0	2.00	1.50
I	BMVW-H	MT20	4.0	6.0		
J	BMVW-H	MT20	4.0	5.0	2.00	1.50
K	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	2120	0	2120	0	0	5-8	3-6
G	1958	0	1958	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 2-14.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1473	1116 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
G	1362	1021 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS VERT. LOAD (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0 / 36	-119.4	-119.4	0.17 (1)	J-C	-113 / 27	0.03 (1)
B-C	-1871 / 0	-119.4	-119.4	0.44 (1)	C-I	0 / 740	0.18 (1)
C-D	-2179 / 0	-119.4	-119.4	0.34 (1)	I-D	-522 / 0	0.14 (1)
D-E	-2179 / 0	-119.4	-119.4	0.34 (1)	I-E	0 / 740	0.18 (1)
E-F	-1872 / 0	-119.4	-119.4	0.44 (1)	H-E	-113 / 27	0.03 (1)
K-B	-1998 / 0	0.0	0.0	0.26 (1)	B-J	0 / 1836	0.45 (1)
G-F	-1836 / 0	0.0	0.0	0.23 (1)	H-F	0 / 1836	0.45 (1)
K-L	0 / 0	-18.2	-18.2	0.32 (1)			
L-J	0 / 0	-18.2	-18.2	0.32 (1)			
J-M	0 / 1869	-18.2	-18.2	0.57 (1)			
M-I	0 / 1869	-18.2	-18.2	0.57 (1)			
I-N	0 / 1869	-18.2	-18.2	0.57 (1)			
N-H	0 / 1869	-18.2	-18.2	0.57 (1)			
H-O	0 / 0	-18.2	-18.2	0.32 (1)			
O-G	0 / 0	-18.2	-18.2	0.32 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	11-3-4	-178	-178	—	BACK	VERT	TOTAL	—	C1
I	7-8-0	-178	-178	—	BACK	VERT	TOTAL	—	C1
J	4-0-12	-178	-178	—	BACK	VERT	TOTAL	—	C1
L	2-0-12	-180	-180	—	BACK	VERT	TOTAL	—	C1
M	6-0-12	-178	-178	—	BACK	VERT	TOTAL	—	C1
N	9-3-4	-178	-178	—	BACK	VERT	TOTAL	—	C1
O	13-3-4	-180	-180	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")CSI: TC=0.44/0.97 (E-F:1), BC=0.57/0.97 (H-I:1),
WB=0.45/0.97 (F-H:1), SSI=0.23/1.00 (C-D:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

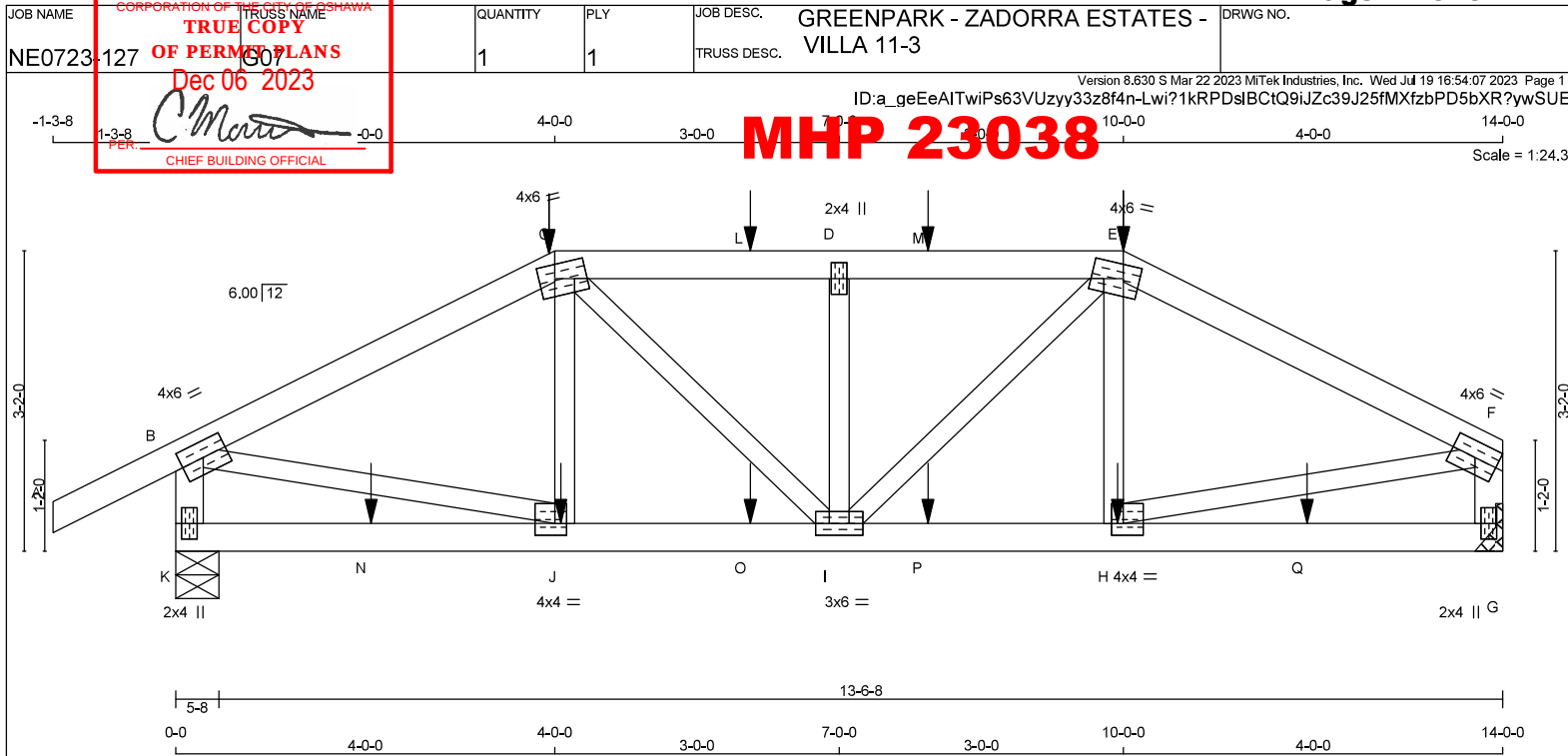
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.49 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-H	MT20	4.0	6.0	2.00	3.00
C	TTVW-H	MT20	4.0	6.0	1.75	2.00
D	TMVW-H	MT20	2.0	4.0		
E	TTVW-H	MT20	4.0	6.0	1.75	2.00
F	TMVW-H	MT20	4.0	6.0		Edge
G	BMV1-p	MT20	2.0	4.0		
H	BMVW-H	MT20	4.0	4.0	1.50	1.50
I	BMVW-H	MT20	3.0	6.0		
J	BMVW-H	MT20	4.0	4.0	1.50	1.50
K	BMV1-p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	1518	0	1518	0
G	1356	0	1356	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1058	784 / 0	0 / 0	0 / 0	0 / 0	273 / 0	0 / 0
G	947	690 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO				J-C	-215 / 33	0.04 (1)
A-B	0 / 36	-119.4	-119.4 0.17 (1)	C-I	0 / 415	0.10 (1)
B-C	-1697 / 0	-119.4	-119.4 0.43 (1)	I-D	-524 / 0	0.10 (1)
C-L	-1809 / 0	-119.4	-119.4 0.26 (1)	I-E	0 / 415	0.10 (1)
L-D	-1809 / 0	-119.4	-119.4 0.26 (1)	H-E	-215 / 33	0.04 (1)
D-M	-1809 / 0	-119.4	-119.4 0.26 (1)	B-J	0 / 1550	0.38 (1)
M-E	-1809 / 0	-119.4	-119.4 0.26 (1)	H-F	0 / 1550	0.38 (1)
E-F	-1697 / 0	-119.4	-119.4 0.43 (1)			
K-B	-1480 / 0	0.0	0.0 0.16 (1)			
G-F	-1318 / 0	0.0	0.0 0.15 (1)			

K-N	0 / 0	-18.2	-18.2 0.09 (4)	10.00
N-J	0 / 0	-18.2	-18.2 0.09 (4)	10.00
J-O	0 / 1509	-18.2	-18.2 0.30 (1)	10.00
O-I	0 / 1509	-18.2	-18.2 0.30 (1)	10.00
I-P	0 / 1509	-18.2	-18.2 0.30 (1)	10.00
P-H	0 / 1509	-18.2	-18.2 0.30 (1)	10.00
H-Q	0 / 0	-18.2	-18.2 0.09 (4)	10.00
Q-G	0 / 0	-18.2	-18.2 0.09 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-0	-202	-202	—	FRONT	VERT	TOTAL	—	C1
E	10-0-0	-202	-202	—	FRONT	VERT	TOTAL	—	C1
H	9-11-4	-10	-10	—	FRONT	VERT	TOTAL	—	C1
J	4-0-12	-10	-10	—	FRONT	VERT	TOTAL	—	C1
L	6-0-12	-41	-41	—	FRONT	VERT	TOTAL	—	C1
M	7-11-4	-41	-41	—	FRONT	VERT	TOTAL	—	C1
N	2-0-12	-10	-10	—	FRONT	VERT	TOTAL	—	C1
O	6-0-12	-10	-10	—	FRONT	VERT	TOTAL	—	C1
P	7-11-4	-10	-10	—	FRONT	VERT	TOTAL	—	C1
Q	11-11-4	-10	-10	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.47")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL) = L/360 (0.47")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.43/0.97 (E-F:1), BC=0.30/0.97 (I-J:1),
 WB=0.38/0.97 (F-H:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

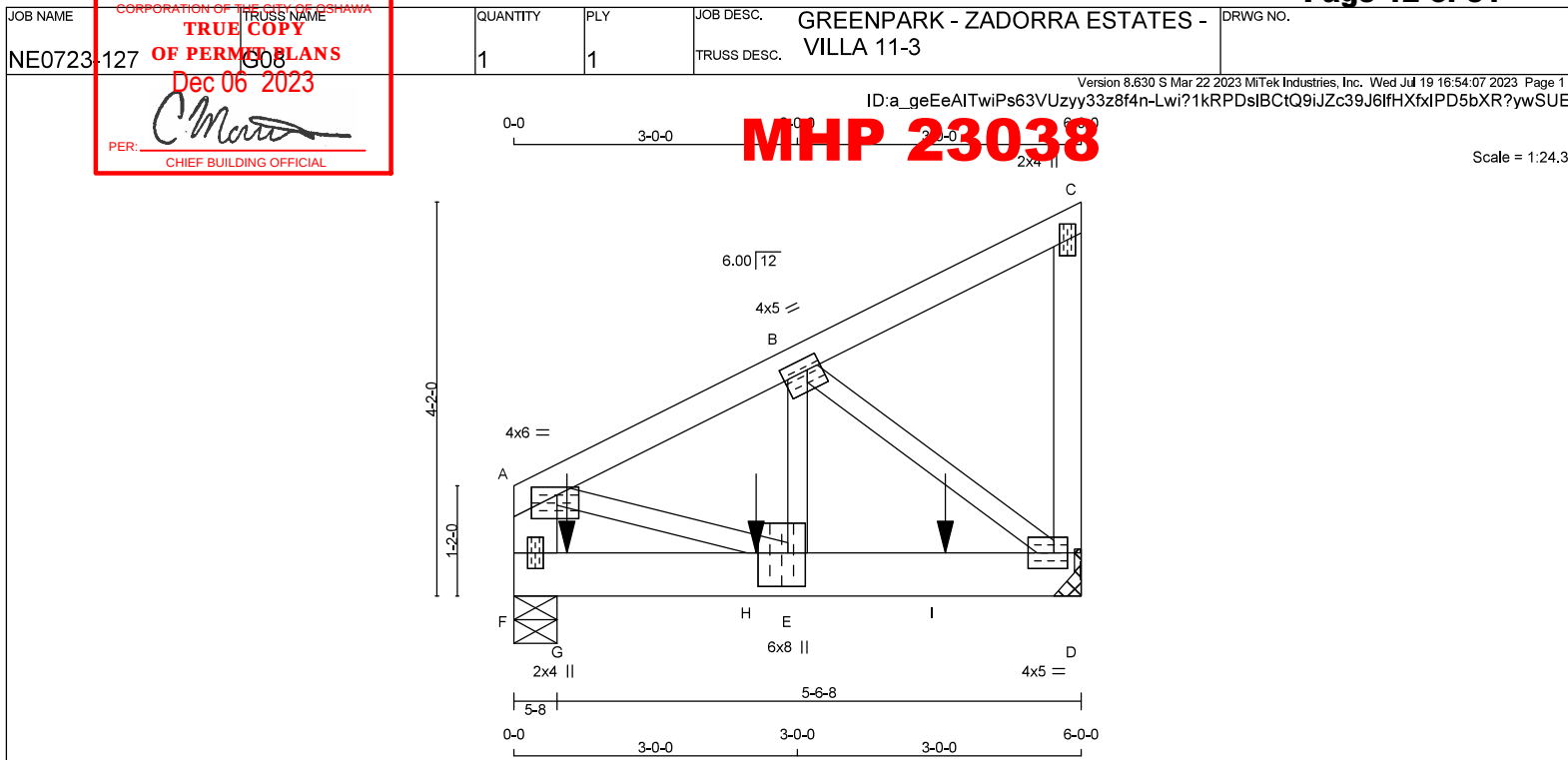
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
 JSI METAL = 0.51 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 30 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.25
B	TMVW-H	MT20	4.0	5.0	1.50	1.75
C	TMV+p	MT20	2.0	4.0		
D	BMVW+14	MT20	4.0	5.0	2.00	1.75
E	BMVW+1	MT20	6.0	8.0	4.25	2.25
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1715	0	1715	0
F	1514	0	1514	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-14.

UNFACTORED REACTIONS

	1ST LOASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1196	880 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0
F	1055	781 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-1687 / 0	-119.4	-119.4	0.19 (1)	E-B	0 / 1491	0.37 (1)
B-C	-15 / 0	-119.4	-119.4	0.15 (1)	B-D	-1910 / 0	0.47 (1)
D-C	-146 / 0	0.0	0.0	0.04 (1)	A-E	0 / 1572	0.39 (1)
F-A	-1300 / 0	0.0	0.0	0.09 (1)			
F-G	0 / 0	-18.2	-18.2	0.23 (1)			
G-H	0 / 0	-18.2	-18.2	0.23 (1)			
H-E	0 / 0	-18.2	-18.2	0.23 (1)			
E-I	0 / 1522	-18.2	-18.2	0.62 (1)			
I-D	0 / 1522	-18.2	-18.2	0.62 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	6-12	-82	-82	—	BACK	VERT	TOTAL	—	C1
H	2-6-12	-932	-932	—	BACK	VERT	TOTAL	—	C1
I	4-6-12	-659	-659	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.19/0.97 (A-B:1) , BC=0.62/0.97 (D-E:1) ,
WB=0.47/0.97 (B-D:1) , SSI=0.86/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

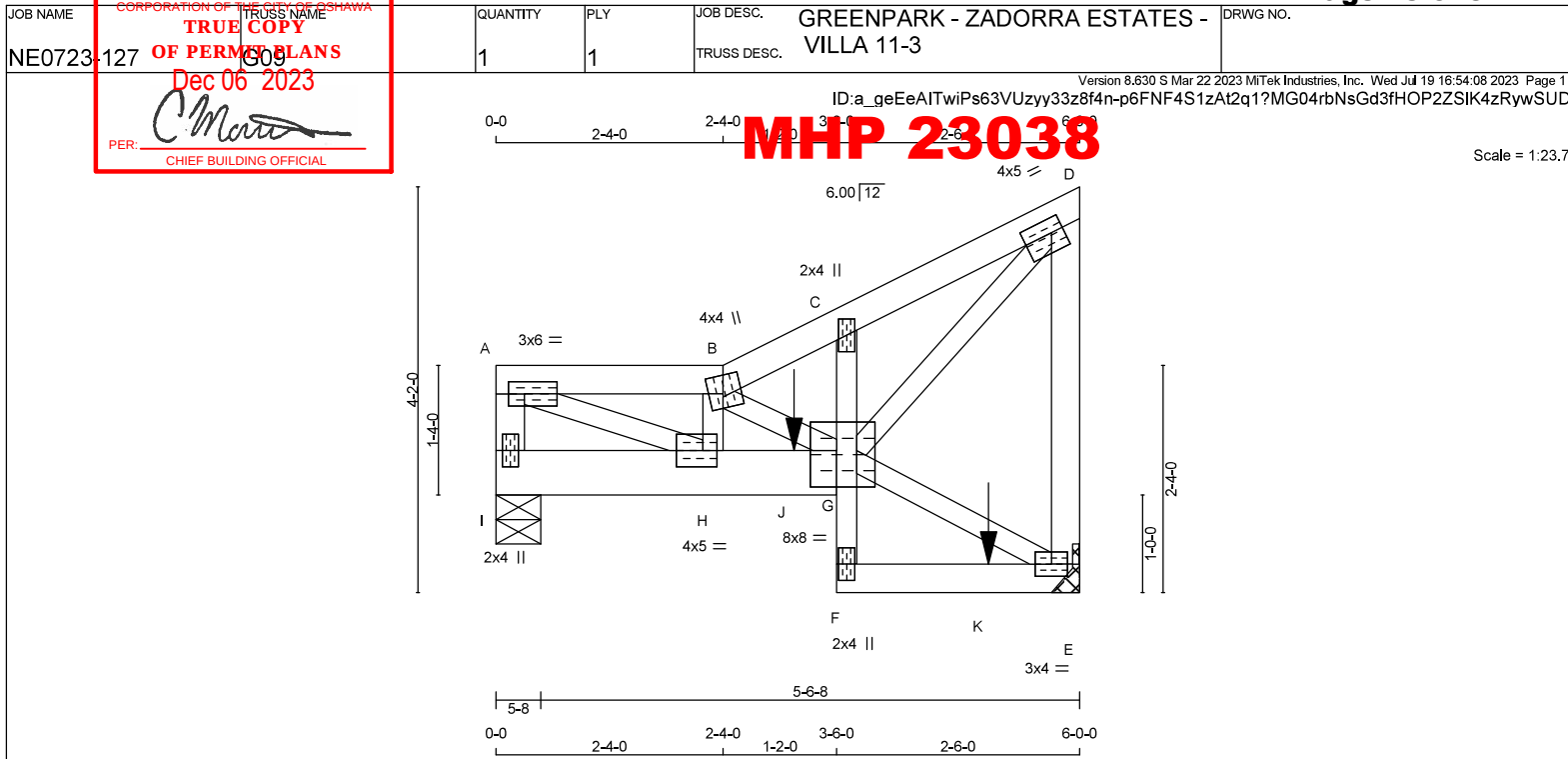
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.46 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 31 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
I - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
I - G	2x6	DRY	No.2
F - C	2x3	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.

SPF
 SPF
 SPF
 SPF
 SPF
 SPF
 SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	3.0	6.0	1.50	2.00
B	TTWW+m	MT20	4.0	4.0	2.25	1.75
C	TMV+p	MT20	2.0	4.0		
D	TMVW4	MT20	4.0	5.0	1.75	1.50
E	BMVW14	MT20	3.0	4.0		
F	BMV+p	MT20	2.0	4.0		
G	BMVWWW4	MT20	8.0	8.0	4.50	3.25
H	BMVWW4	MT20	4.0	5.0	2.00	1.75
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	885	0	885	0	0	5-8	1-8
E	1202	0	1202	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS
I	616	459 / 0
E	836	626 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORZ. LOAD (LC1)	MAX. UNBRACED LENGTH (FR-TO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. HORZ. LOAD (LC1)
FR-TO						FR-TO		
I-A	-774 / 0	0.0	0.0	0.09 (1)	7.81	A-H	0 / 1621	0.40 (1)
A-B	-1489 / 0	-119.4	-119.4	0.15 (1)	5.15	H-B	-523 / 0	0.08 (1)
B-C	-1313 / 0	-119.4	-119.4	0.05 (1)	5.55	B-G	-457 / 0	0.07 (1)
C-D	-1283 / 0	-119.4	-119.4	0.15 (1)	5.46	G-E	-38 / 0	0.01 (1)
E-D	-899 / 0	0.0	0.0	0.25 (1)	7.81	G-D	0 / 1754	0.43 (1)
I-H	0 / 0	-18.2	-18.2	0.15 (1)	10.00			
H-J	0 / 1546	-18.2	-18.2	0.46 (1)	10.00			
J-G	0 / 1546	-18.2	-18.2	0.46 (1)	10.00			
F-G	0 / 192	0.0	0.0	0.16 (1)	10.00			
G-C	-219 / 0	0.0	0.0	0.11 (1)	7.81			
F-K	0 / 34	-18.2	-18.2	0.40 (1)	10.00			
K-E	0 / 34	-18.2	-18.2	0.40 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	3-0-12	-572	-572	—	BACK	VERT	TOTAL	—	C1
K	5-0-12	-303	-303	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 34.8	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.25/0.97 (D-E-1), BC=0.46/0.97 (G-H-1), WB=0.43/0.97 (D-G-1), SSI=0.44/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

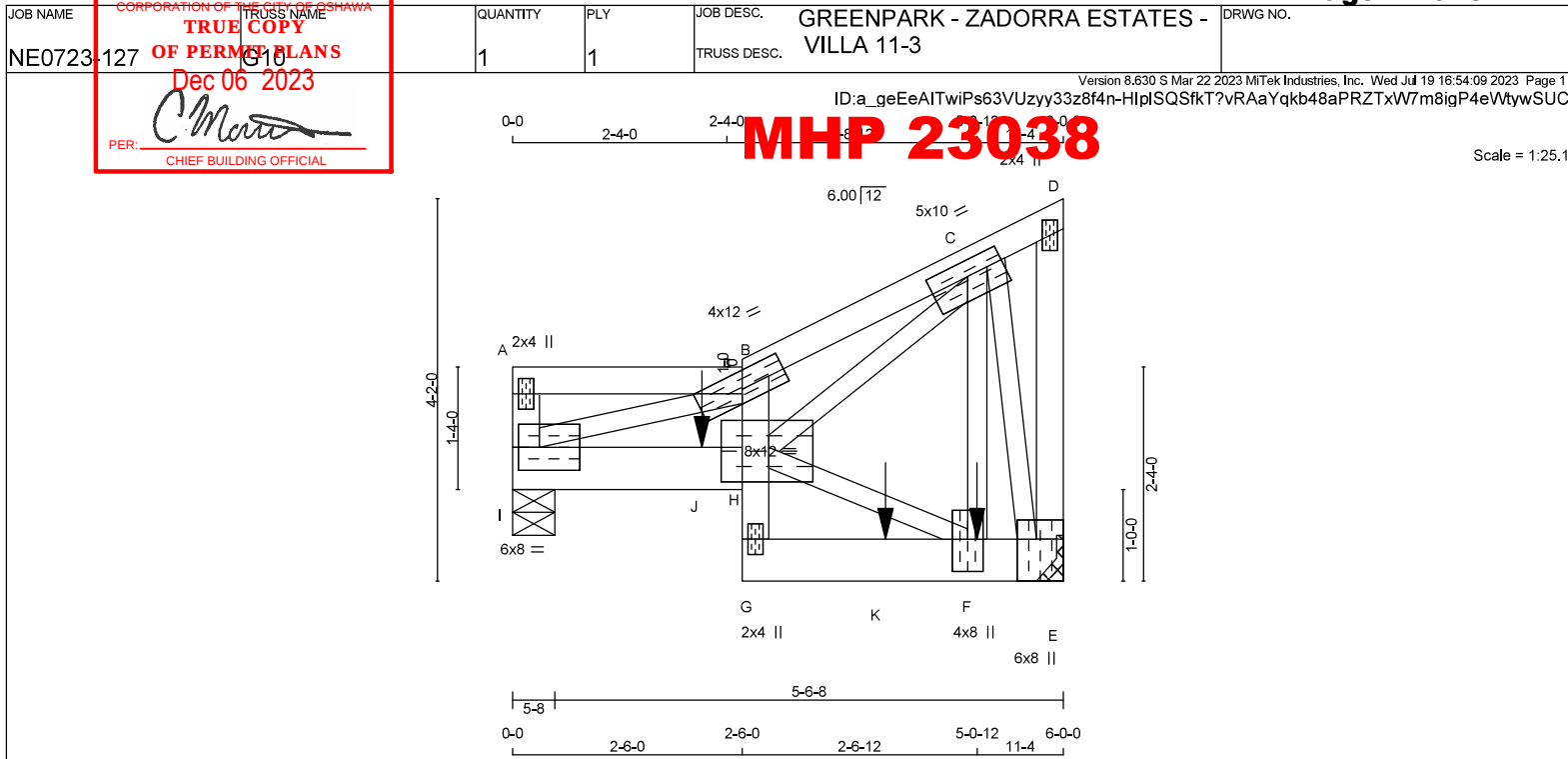
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
 JSI METAL = 0.42 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 36 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
I - A	2x4	DRY No.2
A - B	2x4	DRY No.2
B - D	2x4	DRY No.2
E - D	2x4	DRY No.2
I - H	2x6	DRY No.2
G - B	2x4	DRY No.2
G - E	2x6	DRY No.2

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

DESCR.

SPF
 SPF
 SPF
 SPF
 SPF
 SPF
 SPF**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TTVVH	MT20	4.0	12.0	2.25	6.00
C	TMVWWH	MT20	5.0	10.0	2.00	5.00
D	TMV+p	MT20	2.0	4.0		
E	BMVW1+t	MT20	6.0	8.0	Edge	2.50
F	BMVWW+t	MT20	4.0	8.0	4.25	2.00
G	BMV+p	MT20	2.0	4.0		
H	BMVWWH	MT20	8.0	12.0	4.50	6.25
I	BMVW1+t	MT20	6.0	8.0	3.00	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
I	1507	0	1507	0
E	3216	0	3216	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT
 JOINT E = 3-8.**UNFACTORED REACTIONS**

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1049	783 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
E	2240	1664 / 0	0 / 0	0 / 0	0 / 0	576 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 7.77 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX. (PLF)	MAX. (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX. (LC)
FR-TO															
I-A	-158 / 0		0.0	0.0	0.02	(1)	7.81	I-B	-4112 / 0		0.70	(1)			
A-B	0 / 0		-119.4	-119.4	0.14	(1)	10.00	H-F	0 / 477		0.11	(1)			
B-C	-3804 / 0		-119.4	-119.4	0.24	(1)	3.42	H-C	0 / 3391		0.76	(1)			
C-D	-16 / 0		-119.4	-119.4	0.04	(1)	6.25	F-C	0 / 2115		0.48	(1)			
E-D	-22 / 0		0.0	0.0	0.01	(1)	7.81	C-E	-2868 / 0		0.62	(1)			
I-J	0 / 3890		-18.2	-18.2	0.72	(1)	10.00								
J-H	0 / 3890		-18.2	-18.2	0.72	(1)	10.00								
G-H	0 / 369		0.0	0.0	0.63	(1)	10.00								
H-B	-743 / 0		0.0	0.0	0.45	(1)	7.77								
G-K	0 / 358		-18.2	-18.2	0.38	(1)	10.00								
K-F	0 / 358		-18.2	-18.2	0.38	(1)	10.00								
F-E	0 / 800		-18.2	-18.2	0.37	(1)	10.00								

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	5-0-12	-1838	-1838	—	FRONT	VERT	TOTAL	—	C1
J	2-0-12	-572	-572	—	FRONT	VERT	TOTAL	—	C1
K	4-0-12	-302	-302	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE
 OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD
 CASES.THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
 BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/757 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/453 (0.16")CSI: TC=0.24/0.97 (B-C-1), BC=0.72/0.97 (H-I-1),
 WB=0.76/0.97 (C-H-1), SSI=0.42/1.00 (B-H-1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
 FOR QUALITY CONTROL IN THE TRUSS
 MANUFACTURING PLANT.**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
			788
			1987
			1873

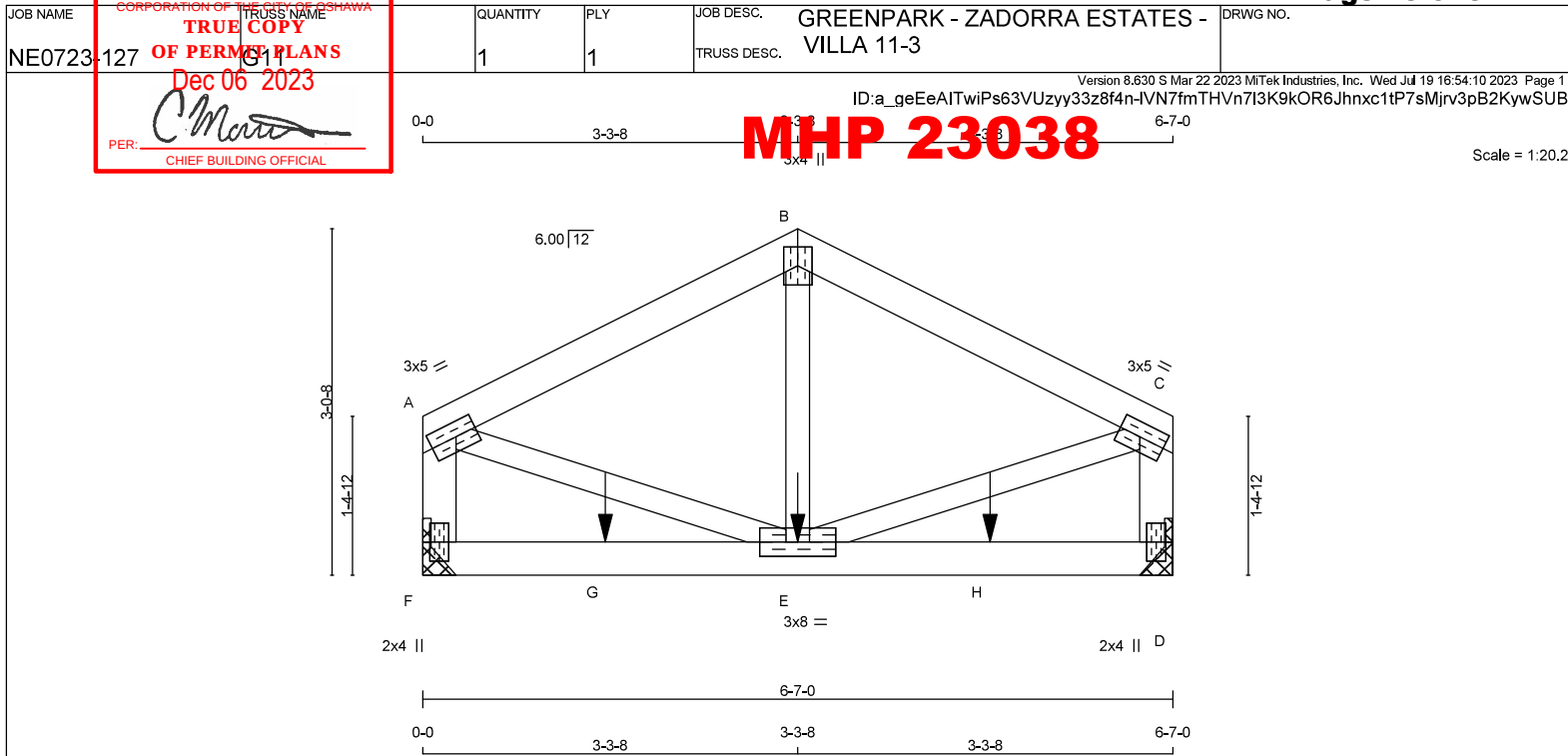
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
 JSI METAL = 0.79 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 25 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVWH	MT20	3.0	5.0	1.50	2.25
B	TTW+p	MT20	3.0	4.0		
C	TMVWH	MT20	3.0	5.0	1.50	2.25
D	BMV1+p	MT20	2.0	4.0		
E	BMVWHH	MT20	3.0	8.0		
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	839	0	839	0	0
D	839	0	839	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	583	438 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
D	583	438 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO	A-B	-714 / 0	-119.4	-119.4	0.25 (1)	6.25	E-B	0 / 246
	B-C	-714 / 0	-119.4	-119.4	0.25 (1)	6.25	A-E	0 / 673
	F-A	-727 / 0	0.0	0.0	0.08 (1)	7.81	E-C	0 / 673
	D-C	-727 / 0	0.0	0.0	0.08 (1)	7.81		
	F-G	0 / 0	-18.2	-18.2	0.25 (1)	10.00		
	G-E	0 / 0	-18.2	-18.2	0.25 (1)	10.00		
	E-H	0 / 0	-18.2	-18.2	0.25 (1)	10.00		
	H-D	0 / 0	-18.2	-18.2	0.25 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-3-8	-178	-178	—	BACK	VERT	TOTAL	—	C1
G	1-7-4	-178	-178	—	BACK	VERT	TOTAL	—	C1
H	4-11-12	-178	-178	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/960 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.25/0.97 (B-C-1), BC=0.25/0.97 (E-F-1),
WB=0.17/0.97 (A-E-1), SSI=0.18/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

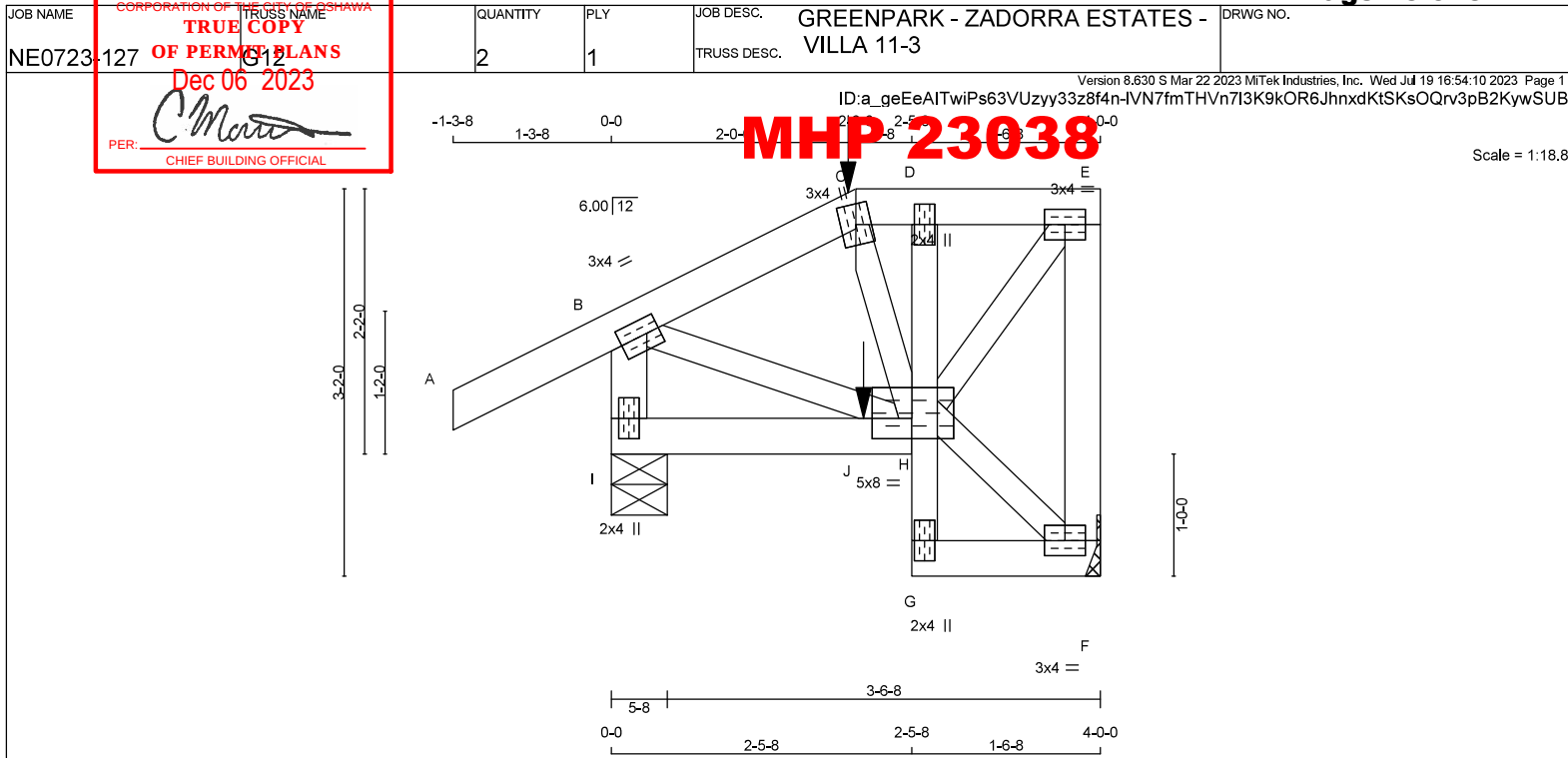
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 23 = 46 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
G - D	2x3	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	3.0	4.0	1.50	1.25
C	TTV+m	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
E	TMVW4	MT20	3.0	4.0		
F	BMVW14	MT20	3.0	4.0		
G	BMV+p	MT20	2.0	4.0		
H	BMVWWVW	MT20	5.0	8.0	2.00	2.50
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	278	0	278	0
I	440	0	440	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	195	139 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
I	306	234 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 36	-119.4 -119.4	0.17 (1) 10.00	C-H	-65 / 0	0.01 (1)	
B-C	-180 / 0	-119.4 -119.4	0.09 (1) 6.25	H-F	-4 / 0	0.00 (1)	
C-D	-142 / 0	-119.4 -119.4	0.01 (1) 6.25	H-E	0 / 231	0.06 (1)	
D-E	-140 / 0	-119.4 -119.4	0.04 (1) 6.25	B-H	0 / 170	0.04 (1)	
F-E	-263 / 0	0.0 0.0	0.05 (1) 7.81				
I-B	-416 / 0	0.0 0.0	0.05 (1) 7.81				
I-J	0 / 0	-18.2 -18.2	0.04 (4) 10.00				
J-H	0 / 0	-18.2 -18.2	0.04 (4) 10.00				
G-H	0 / 15	0.0 0.0	0.01 (1) 10.00				
H-D	-139 / 0	0.0 0.0	0.01 (1) 7.81				
G-F	0 / 3	-18.2 -18.2	0.01 (4) 10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-0-0	-3	-3	—	FRONT	VERT	TOTAL	—	C1
J	2-0-12	-2	-2	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")CSI: TC=0.17/0.97 (A-B:1), BC=0.04/0.97 (H-I:1),
WB=0.06/0.97 (E-H:1), SS=0.12/1.00 (A-B:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

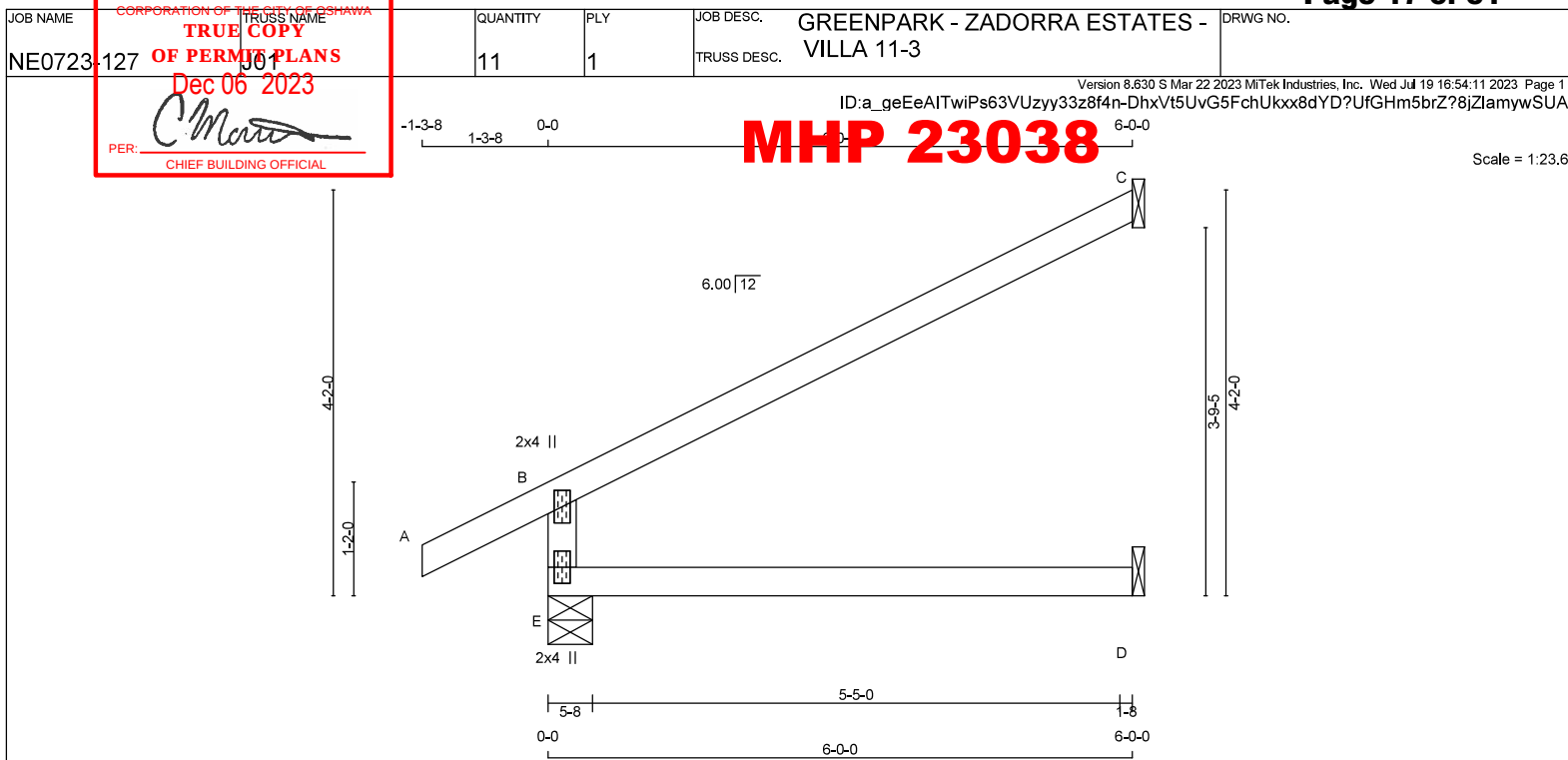
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (H) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	674	0	674	0	0	5-8	1-8
C	269	0	269	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	468	355 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
C	184	157 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
E-B	-610 / 0	0.0 0.0	0.13 (4)	7.81			
A-B	0 / 36	-119.4 -119.4	0.16 (1)	10.00			
B-C	-40 / 0	-119.4 -119.4	0.73 (1)	6.25			
E-D	0 / 0	-18.2 -18.2	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.73/0.97 (B-C:1), BC=0.13/0.97 (D-E:4), WB=0.00/0.97 (n/a:0), SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

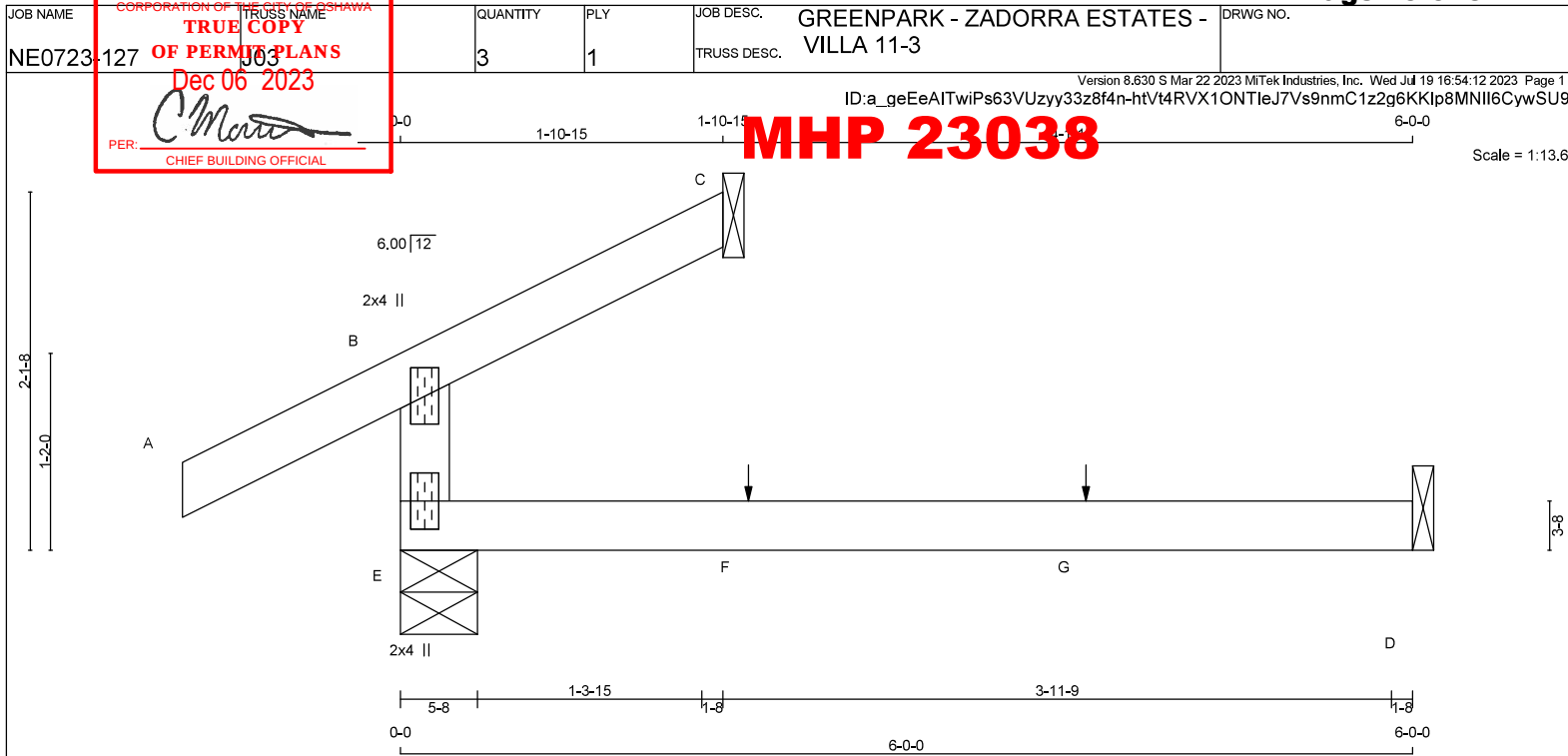
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



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IN THE DESIGN OF THIS COMPONENT.**





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	368	0	368	0	0	5-8	1-8
C	86	0	86	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO		FR-TO		
E-B	-304 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25		
E-F	0 / 0	-18.2	-18.2	0.13 (4)	10.00		
F-G	0 / 0	-18.2	-18.2	0.13 (4)	10.00		
G-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1
G	4-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.16/0.97 (A-B:1), BC=0.13/0.97 (D-E:4),
 WB=0.00/0.97 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

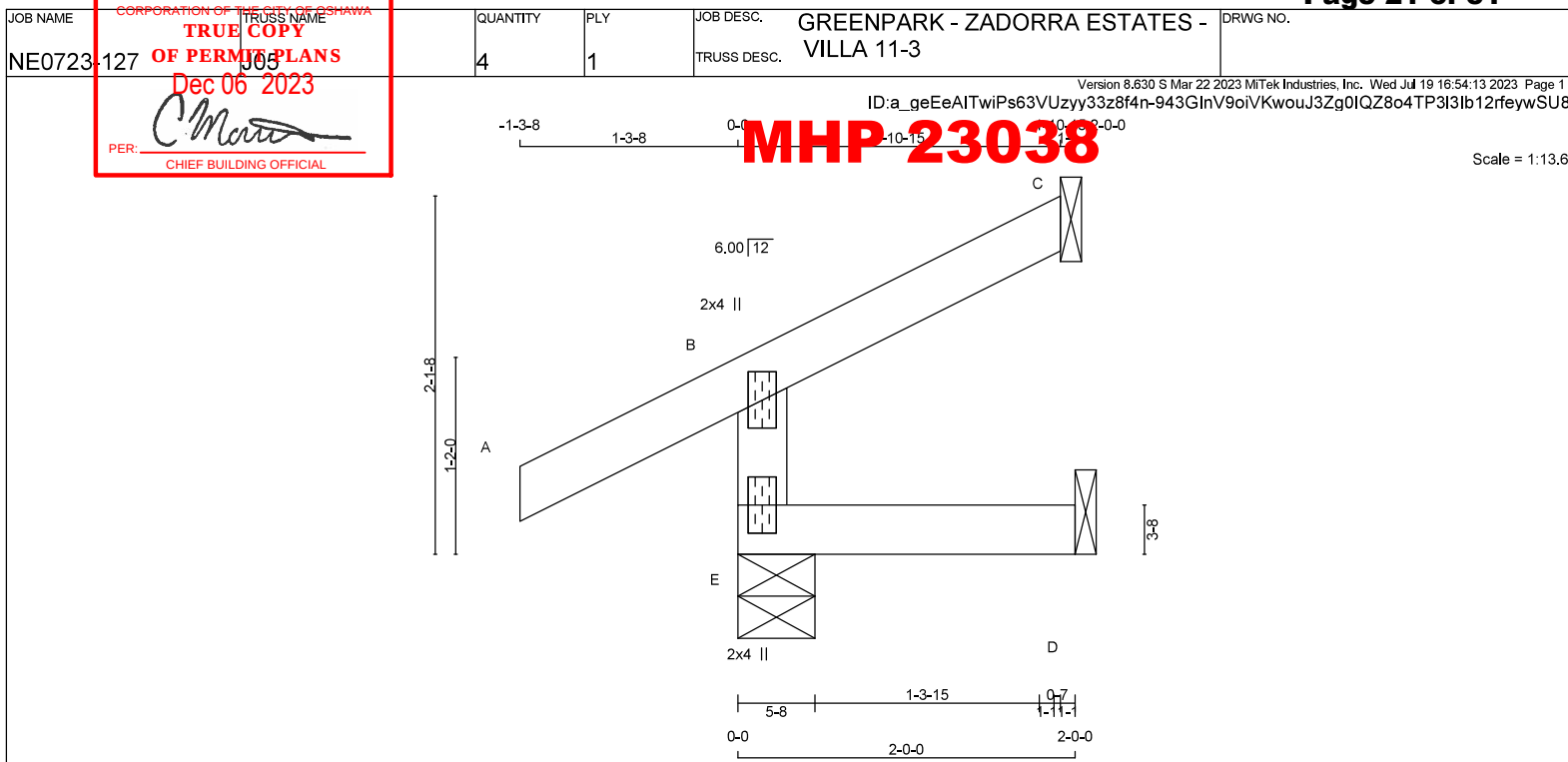
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
 JSI METAL= 0.13 (B) (INPUT = 1.00)



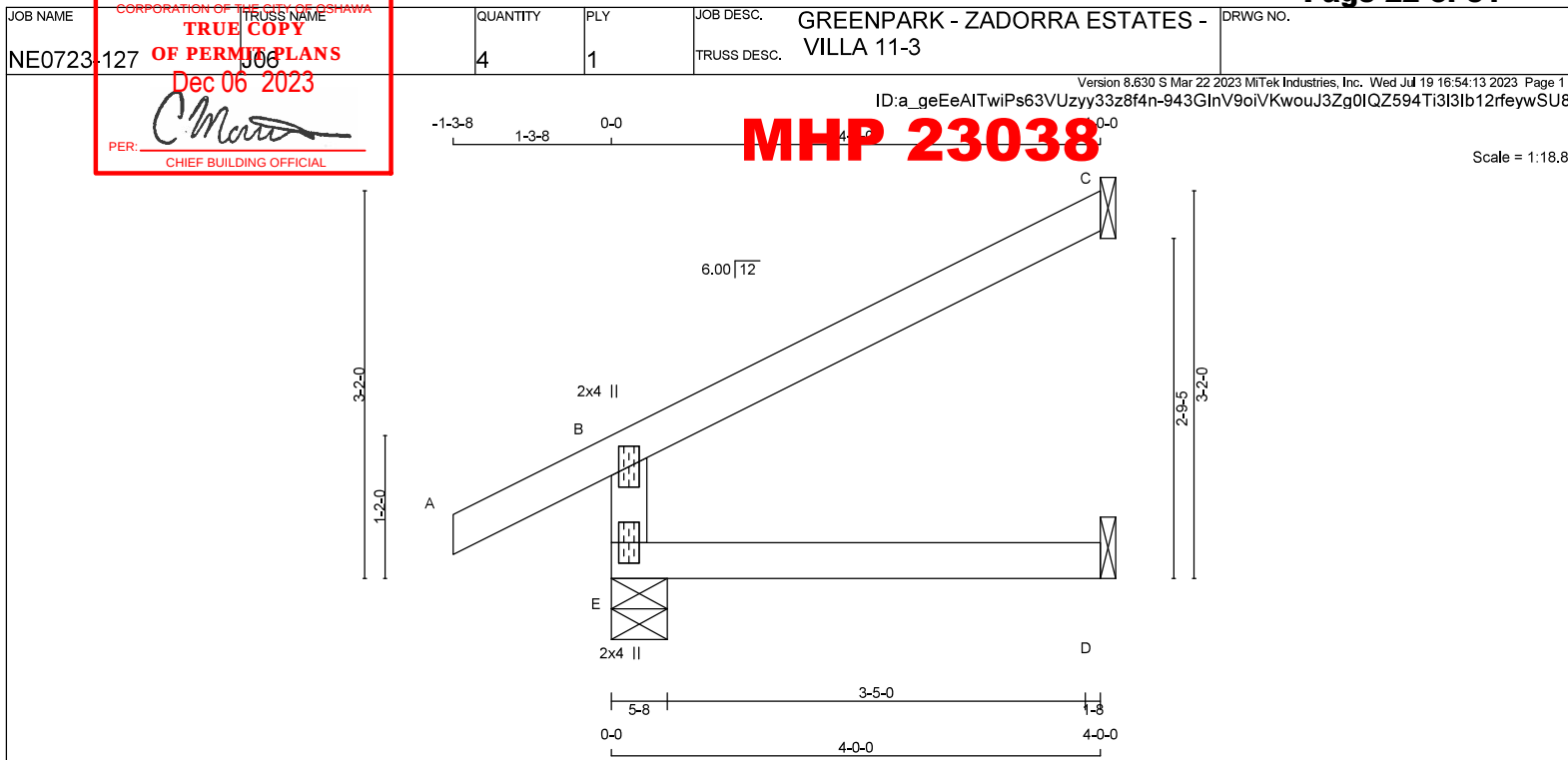
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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**READ ALL NOTES ON THIS PAGE AND ON THE
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IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 4 X 12 = 49 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	503	0	503	0	5-8	1-8
C	179	0	179	0	1-8	1-8
D	31	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	348	268 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0	0 / 0
C	123	105 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
E-B	-460 / 0	0.0	0.0 0.05 (4)	7.81	
A-B	0 / 36	-119.4	-119.4 0.16 (1)	10.00	
B-C	-27 / 0	-119.4	-119.4 0.32 (1)	6.25	
E-D	0 / 0	-18.2	-18.2 0.06 (4)	10.00	

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")CSI: TC=0.32/0.97 (B-C:1), BC=0.06/0.97 (D-E:4),
WB=0.00/0.97 (n/a:0), SSH=0.21/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

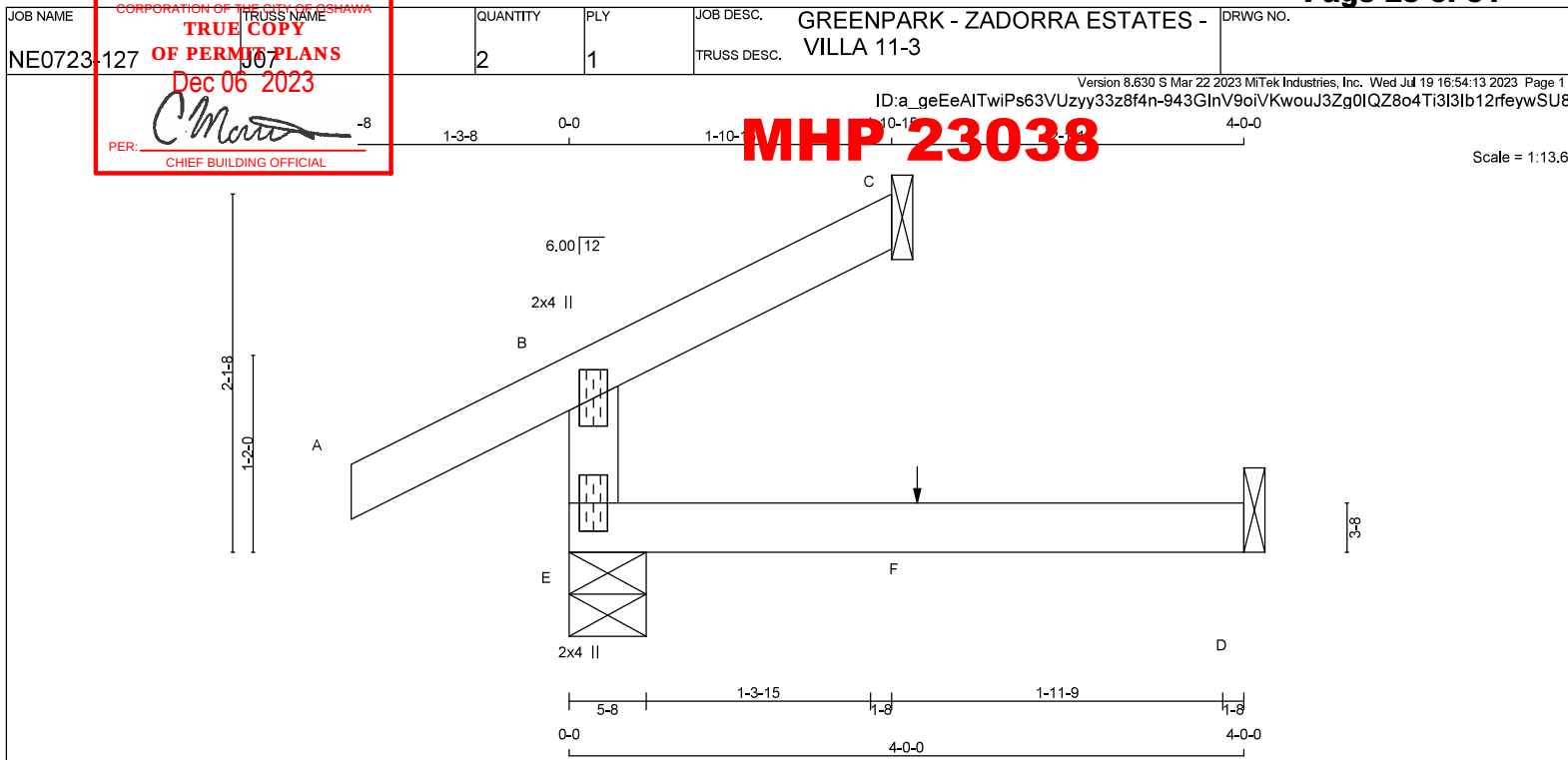
JSI GRIP= 0.26 (B) (INPUT = 0.90)

JSI METAL= 0.19 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 10 = 19 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	346	0	346	0	0	5-8	1-8
C	86	0	86	0	0	1-8	1-8
D	31	0	35	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	241	177 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM	TO				
E-B	-304 / 0	0.0	0.0	0.05 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25		
E-F	0 / 0	-18.2	-18.2	0.06 (4)	10.00		
F-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.16/0.97 (A-B-1) , BC=0.06/0.97 (D-E-4) ,
 WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
 FOR QUALITY CONTROL IN THE TRUSS
 MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

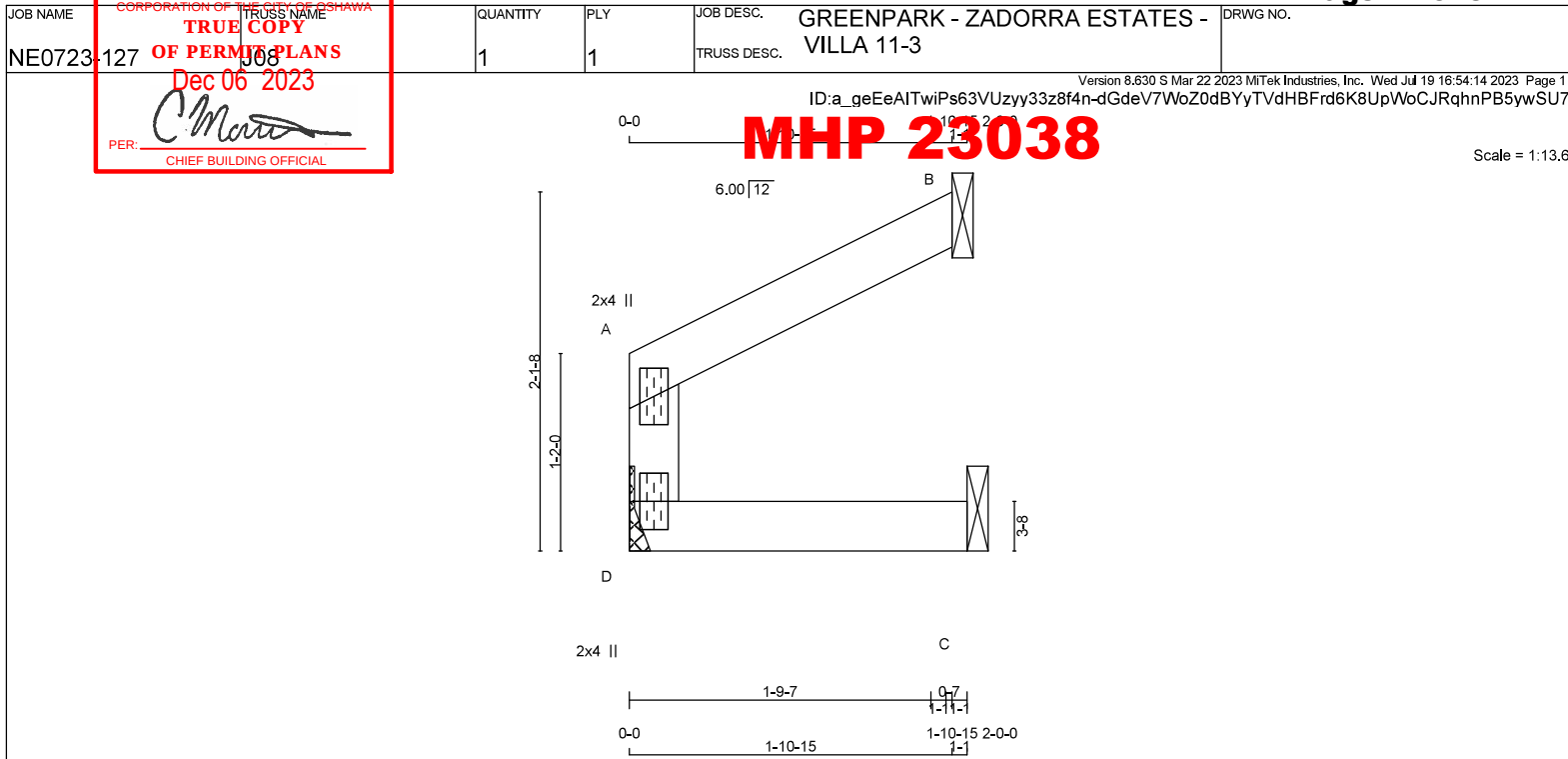
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
 JSI METAL= 0.13 (B) (INPUT = 1.00)



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 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
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 IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REGRD
D	133	0	133	0	0	MECHANICAL	BRG
B	107	0	107	0	0	1-8	BRG
C	25	0	25	0	0	1-8	BRG

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
D	93	67 / 0	0 / 0
B	73	61 / 0	0 / 0
C	19	5 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO			FROM	TO	UNBRACED LENGTH	FR-TO	
D-A	-122 / 0	0.0	0.0	0.02 (1)	7.81		
A-B	-3 / 0	-119.4	-119.4	0.05 (1)	10.00		
D-C	0 / 0	-18.2	-18.2	0.02 (1)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.05/0.97 (A-B:1) , BC=0.02/0.97 (C-D:1) ,
 WB=0.00/0.97 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

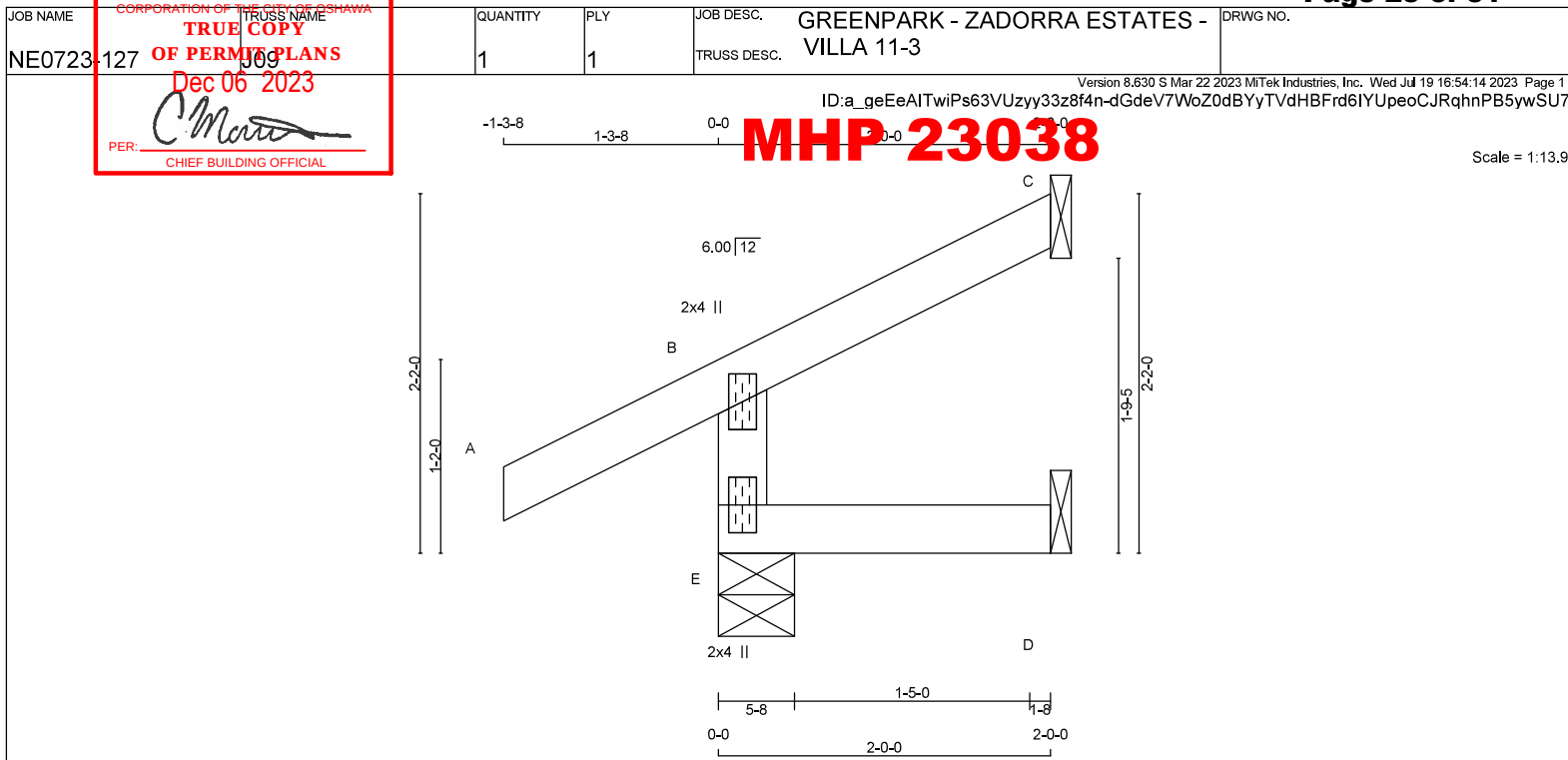
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
 JSI METAL = 0.05 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	331	331	0	5-8
C	90	90	0	1-8
D	16	18	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	229	181 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0	0 / 0
C	62	53 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						FR-TO			
E-B	-311 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00				
B-C	-13 / 0	-119.4	-119.4	0.08 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16/0.97 (A-B:1) , BC=0.02/0.97 (D-E:4) ,
 WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PL)	(PSI)	(PL)	(PL)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

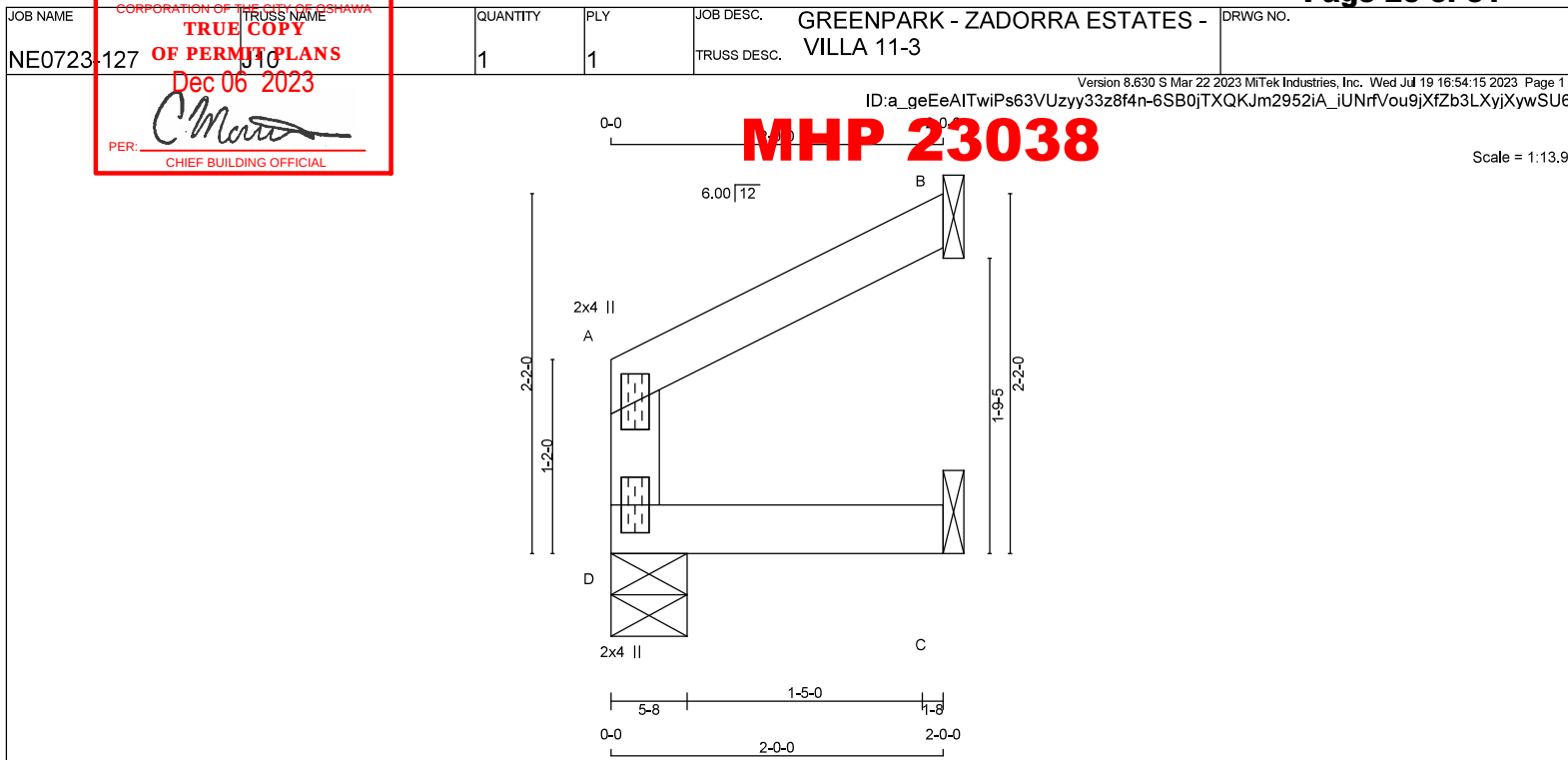
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
 JSI METAL= 0.13 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	138	0	138	0	5-8	1-8
B	111	0	111	0	1-8	1-8
C	27	0	27	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	96	70 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	76	64 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
C	20	6 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO							
D-A	-128 / 0	0.0	0.0	0.02 (1)	7.81		
A-B	-4 / 0	-119.4	-119.4	0.06 (1)	10.00		
D-C	0 / 0	-18.2	-18.2	0.03 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, NBC-2019AE
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.06/0.97 (A-B:1), BC=0.03/0.97 (C-D:1),
 WB=0.00/0.97 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

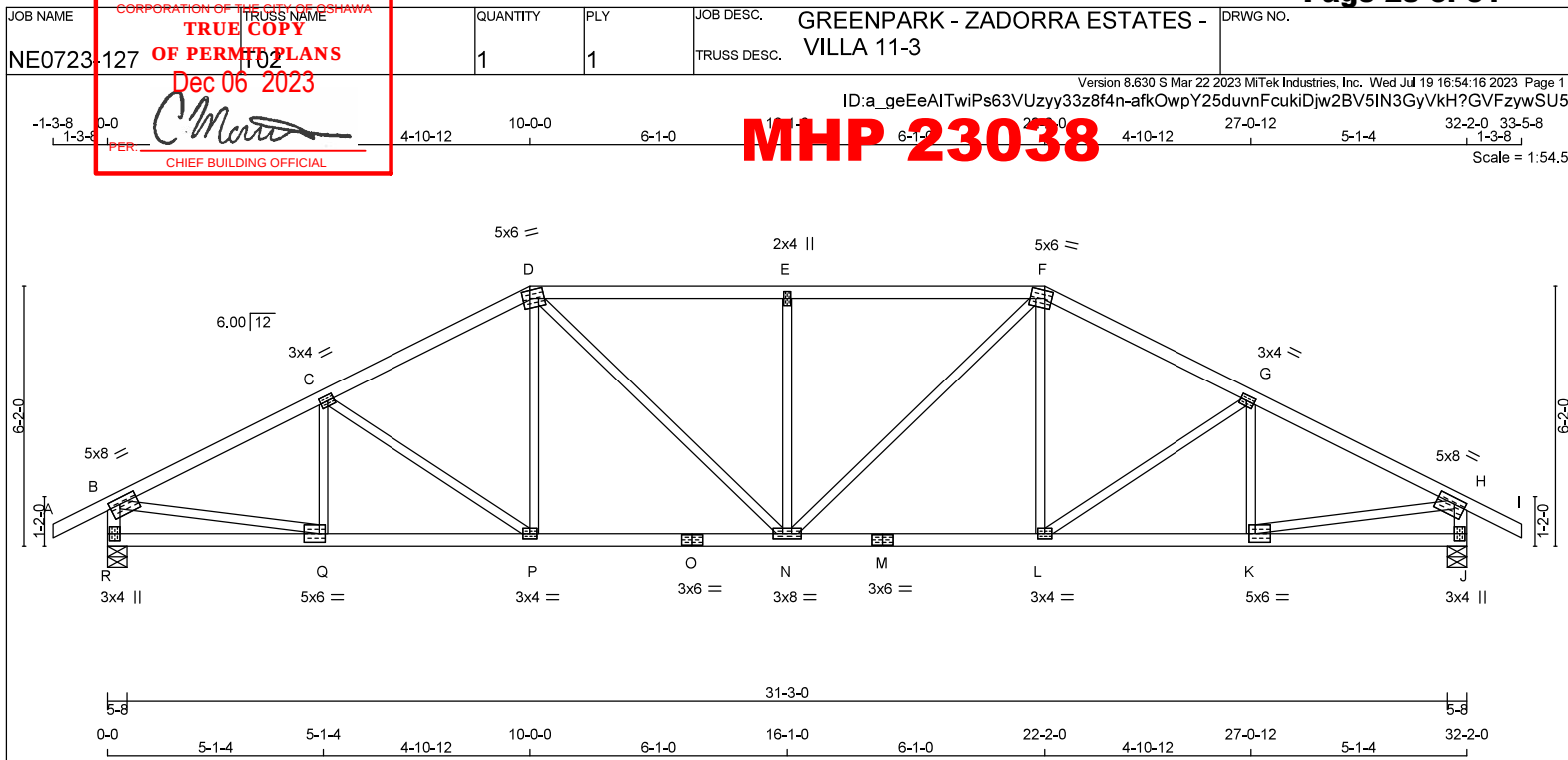
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
 JSI METAL = 0.05 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW4	MT20	5.0	8.0	1.75	3.00
C TMWW4	MT20	3.0	4.0	1.50	1.75
D TTWW-m	MT20	5.0	6.0	2.25	2.00
E TMW+w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	2.25	2.00
G TMWW4	MT20	3.0	4.0	1.50	1.75
H TMWW4	MT20	5.0	8.0	1.75	3.00
J BMV1+p	MT20	3.0	4.0	2.00	
K BMWW4	MT20	5.0	6.0	2.50	1.75
L BMWW4	MT20	3.0	4.0		
M BS4	MT20	3.0	6.0		
N BMWWWW4	MT20	3.0	8.0		
O BS4	MT20	3.0	6.0		
P BMWW4	MT20	3.0	4.0		
Q BMWW4	MT20	5.0	6.0	2.50	1.75
R BMV1+p	MT20	3.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
R	2376	0	2376	0
J	2376	0	2376	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1658	1214 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0	0 / 0
J	1658	1214 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0 / 36	-119.4	-119.4 0.16 (1)	Q-C	-400 / 0	0.09 (1)	10.00
B-C	-3202 / 0	-119.4	-119.4 0.52 (1)	C-P	-346 / 0	0.22 (1)	10.00
C-D	-2946 / 0	-119.4	-119.4 0.49 (1)	P-D	0 / 305	0.07 (1)	10.00
D-E	-3128 / 0	-119.4	-119.4 0.73 (1)	D-N	0 / 719	0.16 (1)	10.00
E-F	-3128 / 0	-119.4	-119.4 0.73 (1)	N-E	-892 / 0	0.54 (1)	10.00
F-G	-2946 / 0	-119.4	-119.4 0.49 (1)	N-F	0 / 719	0.16 (1)	10.00
G-H	-3202 / 0	-119.4	-119.4 0.52 (1)	L-F	0 / 305	0.07 (1)	10.00
H-I	0 / 36	-119.4	-119.4 0.16 (1)	L-G	-346 / 0	0.22 (1)	10.00
R-B	-2332 / 0	0.0	0.0 0.24 (1)	K-G	-400 / 0	0.09 (1)	10.00
J-H	-2332 / 0	0.0	0.0 0.24 (1)	B-Q	0 / 2931	0.66 (1)	10.00
				K-H	0 / 2931	0.66 (1)	10.00
R-Q	0 / 0	-18.2	-18.2 0.10 (4)				
Q-P	0 / 2891	-18.2	-18.2 0.53 (1)				
P-O	0 / 2611	-18.2	-18.2 0.49 (1)				
O-N	0 / 2611	-18.2	-18.2 0.49 (1)				
N-M	0 / 2611	-18.2	-18.2 0.49 (1)				
M-L	0 / 2611	-18.2	-18.2 0.49 (1)				
L-K	0 / 2891	-18.2	-18.2 0.53 (1)				
K-J	0 / 0	-18.2	-18.2 0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.73/0.97 (D-E:1), BC=0.53/0.97 (K-L:1),
WB=0.66/0.97 (H-K:1), SS=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

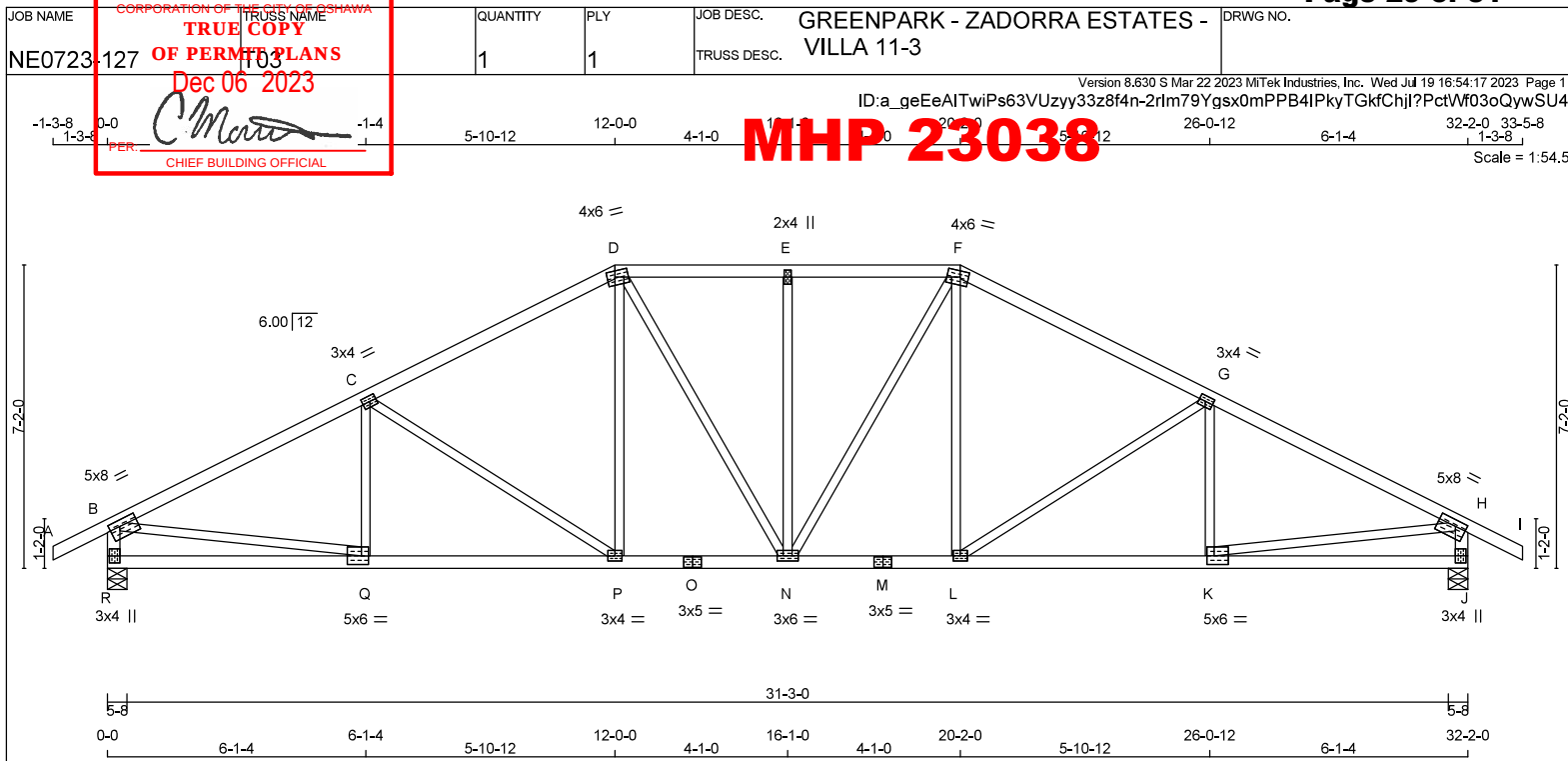
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (F) (INPUT = 0.90)
JSI METAL = 0.82 (O) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	SIZE	LUMBER
CHORDS		
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
R - B	2x4	DRY No.2
J - H	2x4	DRY No.2
R - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW4	MT20	5.0	8.0	1.75	3.00
C	TMWW4	MT20	3.0	4.0	1.50	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMWW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMWW4	MT20	3.0	4.0	1.50	1.75
H	TMWW4	MT20	5.0	8.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0	2.00	
K	BMWW4	MT20	5.0	6.0	2.50	2.00
L	BMWW4	MT20	3.0	4.0		
M	BS4	MT20	3.0	5.0		
N	BMWWWW4	MT20	3.0	6.0		
O	BS4	MT20	3.0	5.0		
P	BMWW4	MT20	3.0	4.0		
Q	BMWW4	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT				
R	2376	0	2376	0
J	2376	0	2376	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
R	1658	1214 / 0	0 / 0	444 / 0	0 / 0
J	1658	1214 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	Q-C	-297 / 29	0.08 (1)	
B-C	-3251 / 0	-119.4	-119.4	0.77 (1)	3.04	C-P	-614 / 0	0.61 (1)	
C-D	-2753 / 0	-119.4	-119.4	0.68 (1)	3.38	P-D	0 / 436	0.10 (1)	
D-E	-2624 / 0	-119.4	-119.4	0.32 (1)	3.95	D-N	0 / 373	0.08 (1)	
E-F	-2624 / 0	-119.4	-119.4	0.32 (1)	3.95	N-E	-588 / 0	0.53 (1)	
F-G	-2753 / 0	-119.4	-119.4	0.68 (1)	3.38	N-F	0 / 373	0.08 (1)	
G-H	-3251 / 0	-119.4	-119.4	0.77 (1)	3.04	L-F	0 / 436	0.10 (1)	
H-I	0 / 36	-119.4	-119.4	0.16 (1)	10.00	L-G	-614 / 0	0.61 (1)	
R-B	-2327 / 0	0.0	0.0	0.24 (1)	5.52	K-G	-297 / 29	0.08 (1)	
J-H	-2327 / 0	0.0	0.0	0.24 (1)	5.52	B-Q	0 / 2969	0.67 (1)	
						K-H	0 / 2969	0.67 (1)	
R-Q	0 / 0	-18.2	-18.2	0.15 (4)	10.00				
Q-P	0 / 2941	-18.2	-18.2	0.53 (1)	10.00				
P-O	0 / 2433	-18.2	-18.2	0.45 (1)	10.00				
O-N	0 / 2433	-18.2	-18.2	0.45 (1)	10.00				
N-M	0 / 2433	-18.2	-18.2	0.45 (1)	10.00				
M-L	0 / 2433	-18.2	-18.2	0.45 (1)	10.00				
L-K	0 / 2941	-18.2	-18.2	0.53 (1)	10.00				
K-J	0 / 0	-18.2	-18.2	0.15 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.77/0.97 (G+H:1), BC=0.53/0.97 (K+L:1),
WB=0.67/0.97 (H+K:1), SS=0.31/1.00 (G+H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL)
(PSI) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

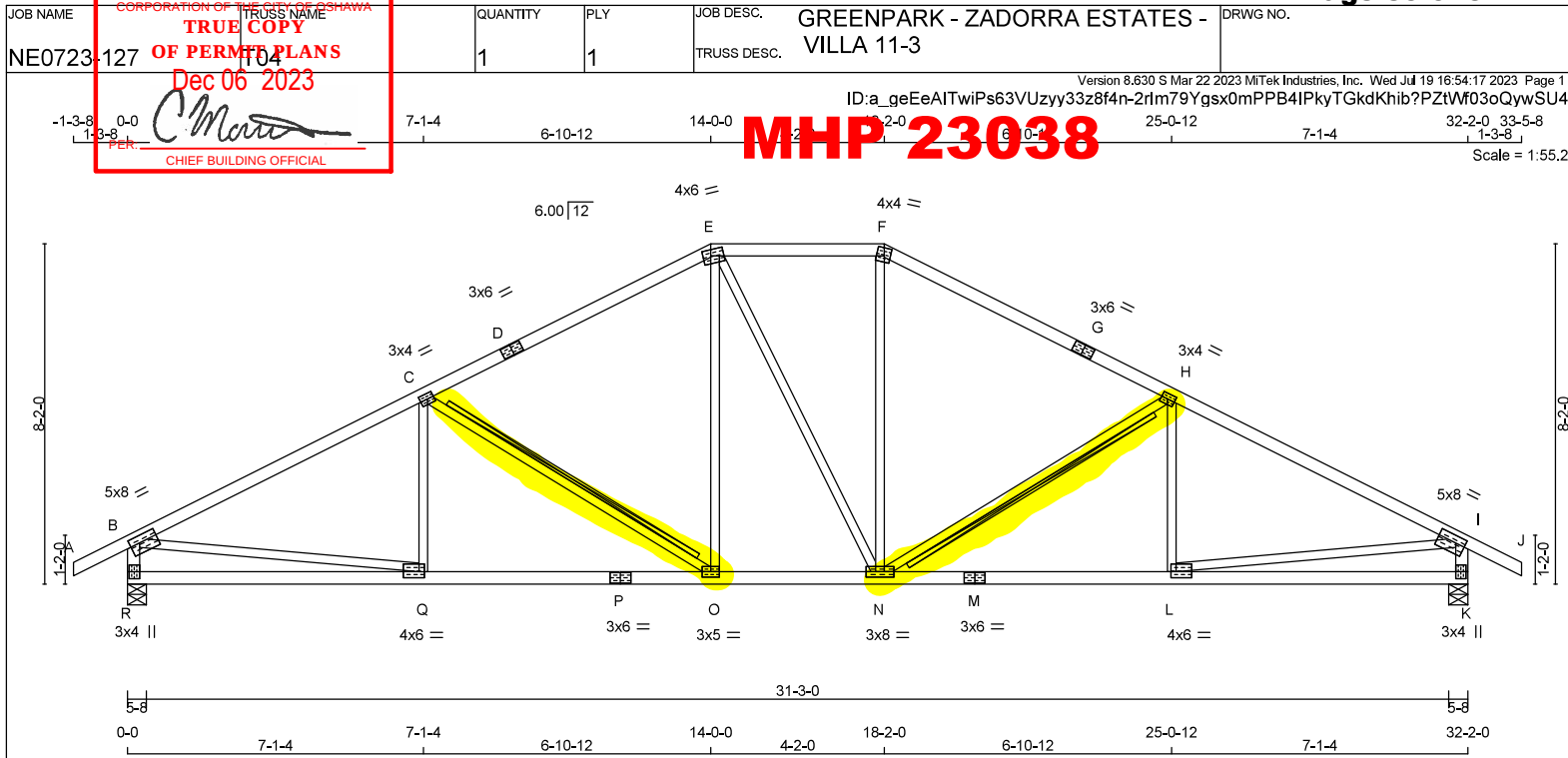
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (D) (INPUT = 0.90)
JSI METAL = 0.70 (O) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	2100F 1.8E	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	8.0	2.00	2.75
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TS4	MT20	3.0	6.0		
E	TTVW-m	MT20	4.0	6.0	1.75	2.25
F	TTVW-m	MT20	4.0	4.0	2.25	1.75
G	TS4	MT20	3.0	6.0		
H	TMVW4	MT20	3.0	4.0	1.50	1.75
I	TMVW4	MT20	5.0	8.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0	2.00	
L	BMVW4	MT20	4.0	6.0	1.75	1.50
M	BS4	MT20	3.0	6.0		
N	BMVW4	MT20	3.0	6.0		
O	BMVW4	MT20	3.0	5.0		
P	BS4	MT20	3.0	6.0		
Q	BMVW4	MT20	4.0	6.0	1.75	1.50
R	BMV1+p	MT20	3.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2376	0	2376	0
K	2376	0	2376	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1658	1214 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1658	1214 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-O, H-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 36	-119.4 -119.4	0.10 (1)	Q-C	-214 / 71	0.07 (1)	
B-C	-3265 / 0	-119.4 -119.4	0.64 (1)	C-O	-872 / 0	0.46 (1)	
C-D	-2535 / 0	-119.4 -119.4	0.95 (1)	O-E	0 / 558	0.13 (1)	
D-E	-2535 / 0	-119.4 -119.4	0.95 (1)	E-N	0 / 2	0.00 (1)	
E-F	-2233 / 0	-119.4 -119.4	0.33 (1)	N-F	0 / 561	0.13 (1)	
F-G	-2536 / 0	-119.4 -119.4	0.95 (1)	N-H	-870 / 0	0.46 (1)	
G-H	-2536 / 0	-119.4 -119.4	0.95 (1)	H-I	-216 / 71	0.07 (1)	
H-I	-3265 / 0	-119.4 -119.4	0.64 (1)	B-Q	0 / 2983	0.67 (1)	
I-J	0 / 36	-119.4 -119.4	0.10 (1)	L-I	0 / 2983	0.67 (1)	
R-B	-2322 / 0	0.0	0.0 0.23 (1)				
K-I	-2322 / 0	0.0	0.0 0.23 (1)				
R-Q	0 / 0	-18.2 -18.2	0.22 (4)				
Q-P	0 / 2962	-18.2 -18.2	0.58 (1)				
P-O	0 / 2962	-18.2 -18.2	0.58 (1)				
O-N	0 / 2232	-18.2 -18.2	0.43 (1)				
N-M	0 / 2961	-18.2 -18.2	0.57 (1)				
M-L	0 / 2961	-18.2 -18.2	0.57 (1)				
L-K	0 / 0	-18.2 -18.2	0.22 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.95/0.97 (F-H:1), BC=0.58/0.97 (O-Q:1),
WB=0.67/0.97 (B-Q:1), SSI=0.37/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

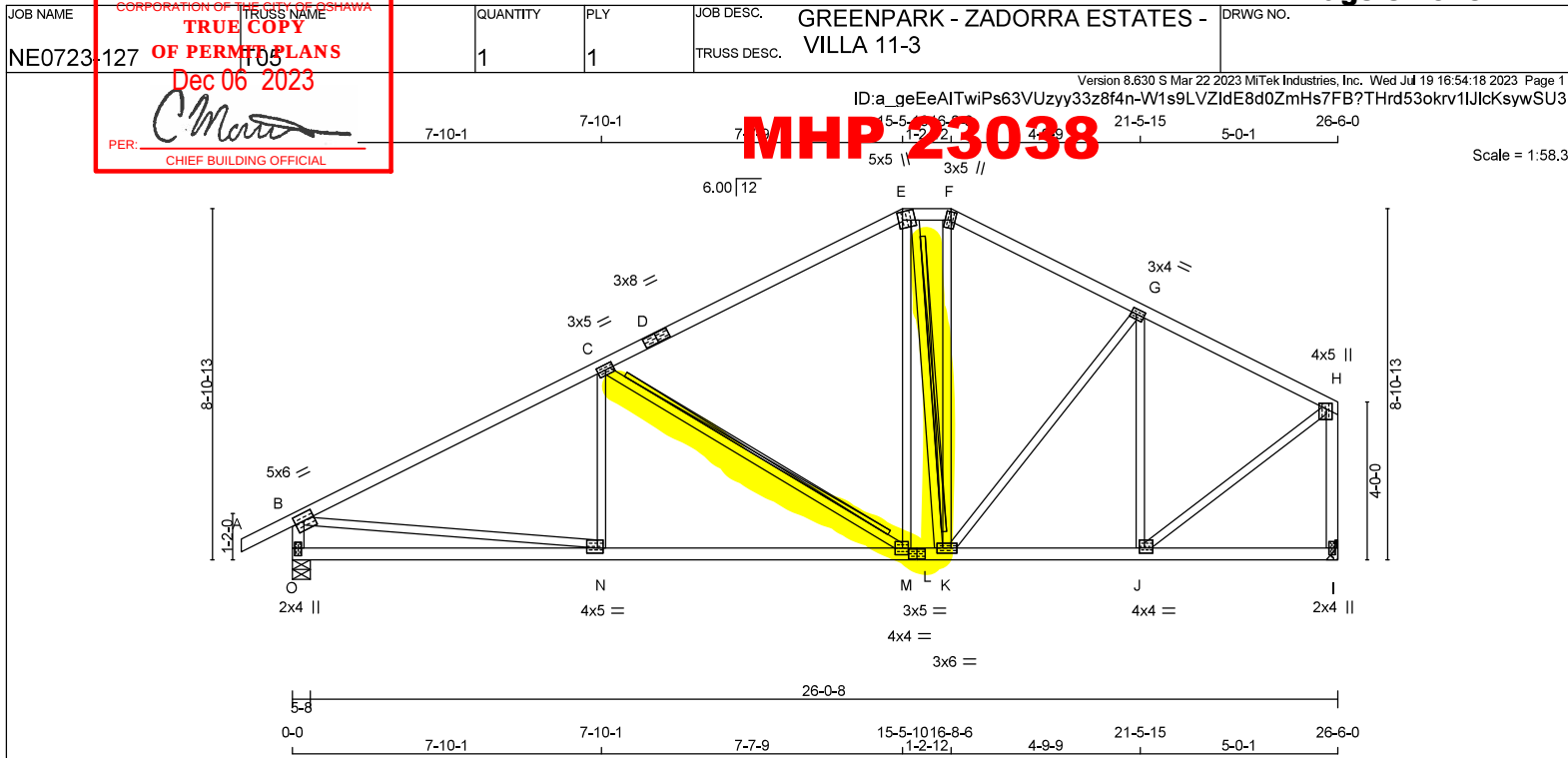
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.95 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - E	2x4	DRY	2100F 1.8E	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	6.0	2.25	2.75
C	TMVW4	MT20	3.0	5.0		
D	TS4	MT20	3.0	8.0		
E	TTVW+m	MT20	5.0	5.0	2.50	1.25
F	TTVW+m	MT20	3.0	5.0	2.50	1.25
G	TMVW4	MT20	3.0	4.0	1.50	1.50
H	TMVW+p	MT20	4.0	5.0	1.75	1.75
I	BMV1+p	MT20	2.0	4.0		
J	BMVW4	MT20	4.0	4.0	1.50	1.50
K	BMVW4	MT20	3.0	6.0		
L	BS4	MT20	3.0	5.0		
M	BMVW4	MT20	4.0	4.0	2.00	1.75
N	BMVW4	MT20	4.0	5.0	1.50	1.75
O	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1986	0	1986	0	0	5-8	2-15
I	1824	0	1824	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-7.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1386	1017 / 0	0 / 0	0 / 0	0 / 0	0 / 0	369 / 0	0 / 0
I	1275	922 / 0	0 / 0	0 / 0	0 / 0	0 / 0	352 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-M, E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. UNBRACED LENGTH (FR-TO)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC1)
FR-TO						FR-TO			
A-B	0 / 36	-119.4	-119.4	0.10 (1)	10.00	N-C	-89 / 117	0.04 (4)	
B-C	-2535 / 0	-119.4	-119.4	0.73 (1)	4.20	C-M	-1115 / 0	0.73 (1)	
C-D	-1575 / 0	-119.4	-119.4	0.65 (1)	5.16	M-E	0 / 639	0.14 (1)	
D-E	-1575 / 0	-119.4	-119.4	0.65 (1)	5.16	E-K	-374 / 0	0.22 (1)	
E-F	-1322 / 0	-119.4	-119.4	0.04 (1)	5.57	K-F	0 / 373	0.08 (1)	
F-G	-1506 / 0	-119.4	-119.4	0.40 (1)	4.85	K-G	0 / 98	0.02 (1)	
G-H	-1374 / 0	-119.4	-119.4	0.39 (1)	5.03	J-G	-836 / 0	0.58 (1)	
O-B	-1928 / 0	0.0	0.0	0.19 (1)	5.98	B-N	0 / 2325	0.52 (1)	
I-H	-1785 / 0	0.0	0.0	0.47 (1)	6.17	J-H	0 / 1562	0.35 (1)	
O-N	0 / 0	-18.2	-18.2	0.28 (4)	10.00				
N-M	0 / 2311	-18.2	-18.2	0.51 (1)	10.00				
M-L	0 / 1370	-18.2	-18.2	0.30 (1)	10.00				
L-K	0 / 1370	-18.2	-18.2	0.30 (1)	10.00				
K-J	0 / 1257	-18.2	-18.2	0.28 (1)	10.00				
J-I	0 / 0	-18.2	-18.2	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.73/0.97 (B-C:1), BC=0.51/0.97 (M-N:1), WB=0.73/0.97 (C-M:1), SSI=0.40/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PL)	(PL)	(PL)	(PL)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (G) (INPUT = 0.90)
JSI METAL = 0.66 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

