

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LJS26DS- (VV)
H2.5T- (I)

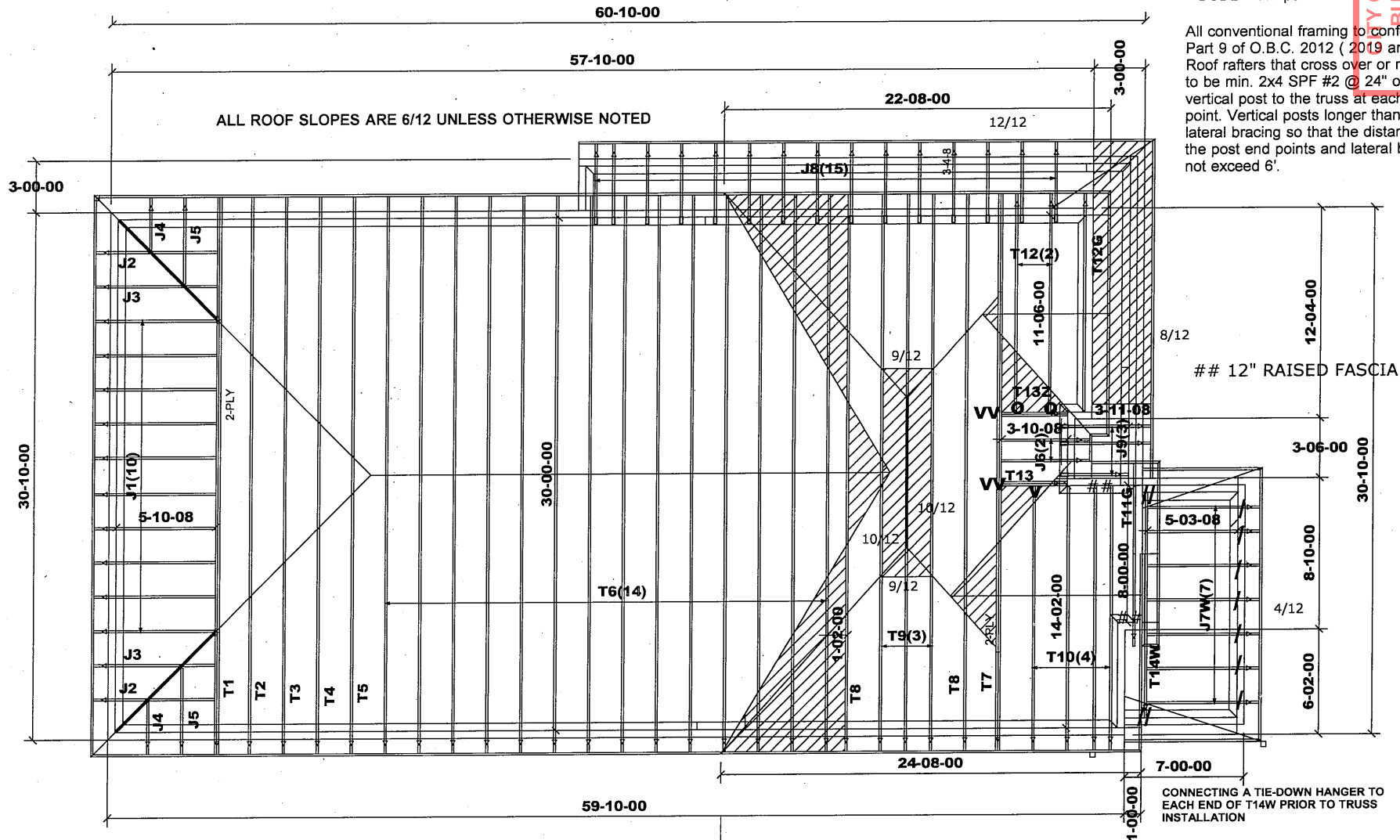


DENOTES:
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with
Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses
to be min. 2x4 SPF #2 @ 24" o/c with a
vertical post to the truss at each cross
point. Vertical posts longer than 6' to have
lateral bracing so that the distance between
the post end points and lateral bracing does
not exceed 6'.



M15638



Job Track: **53256**

Plan Log: **207463**

Layout ID: **434359**

Builder / Location:

GREENPARK HOMES / RICHMOND HILL

Model / Elevation:

ROSE 5 / 1 - STD.OR OPT.5 BED

Project: **TRINIGROUP DEVELOPMENTS**

Date: 2023-08-25 Sales: Rick DiCiano

Designer: ML/ YPG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.6.3.533

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024

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Per: Joshua Nabua

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

 **DENOTES:**
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LJS26DS- (VV)

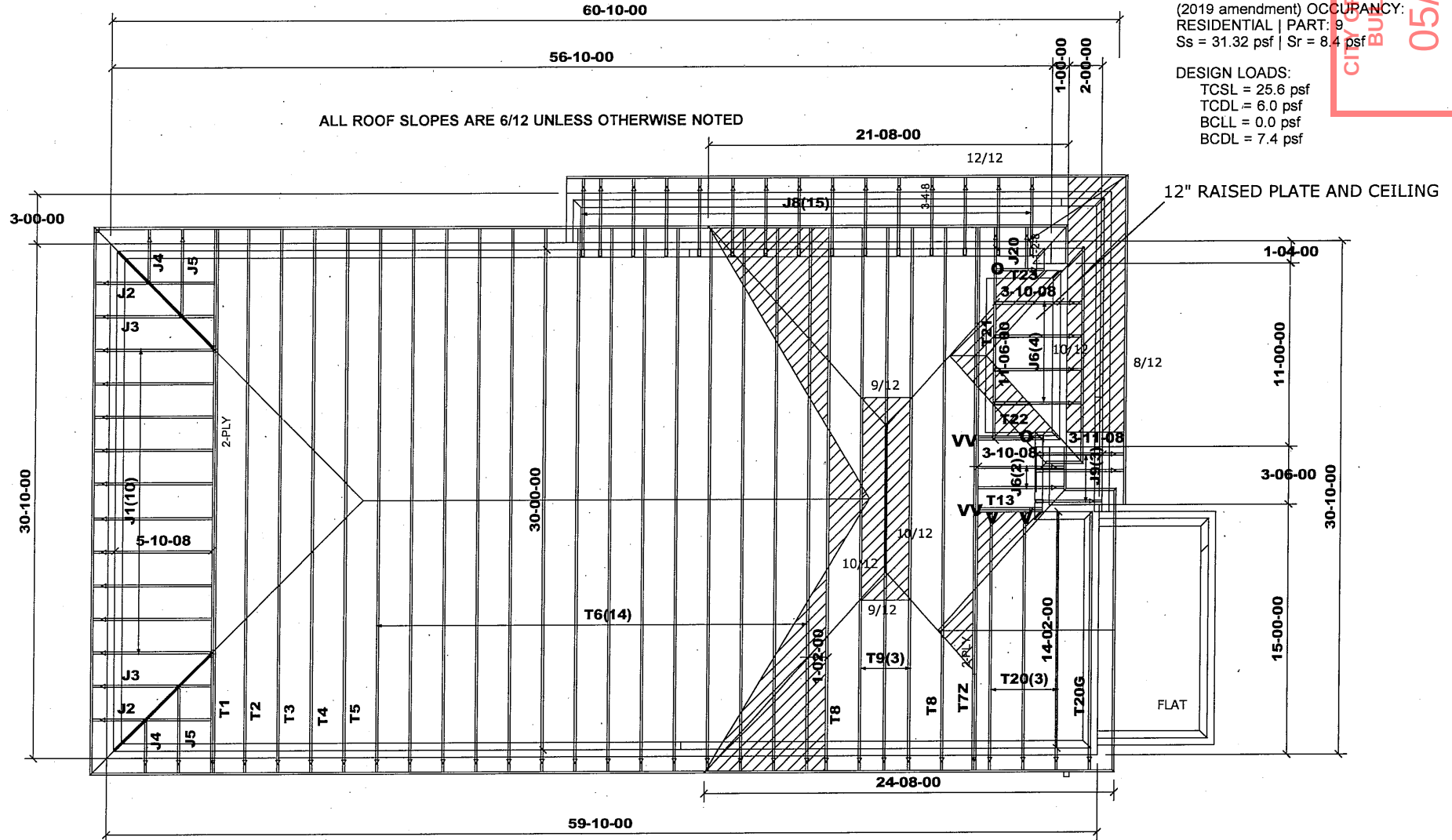
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024

RECEIVED

Per: Joshua Nabua



M15638



Job Track: **53256**

Plan Log: **207463**

Layout ID: **434360**

Builder / Location:

GREENPARK HOMES / RICHMOND HILL

Model / Elevation:

ROSE 5 / 2-STD.OR OPT.5 BED

Project: **TRINIGROUP DEVELOPMENTS**

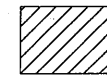
Date: 2023-08-25 Sales: Rick DiCiano Designer: YPG

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Mitek ver 8.6.3.353

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment).
Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LJS26DS - (VV)

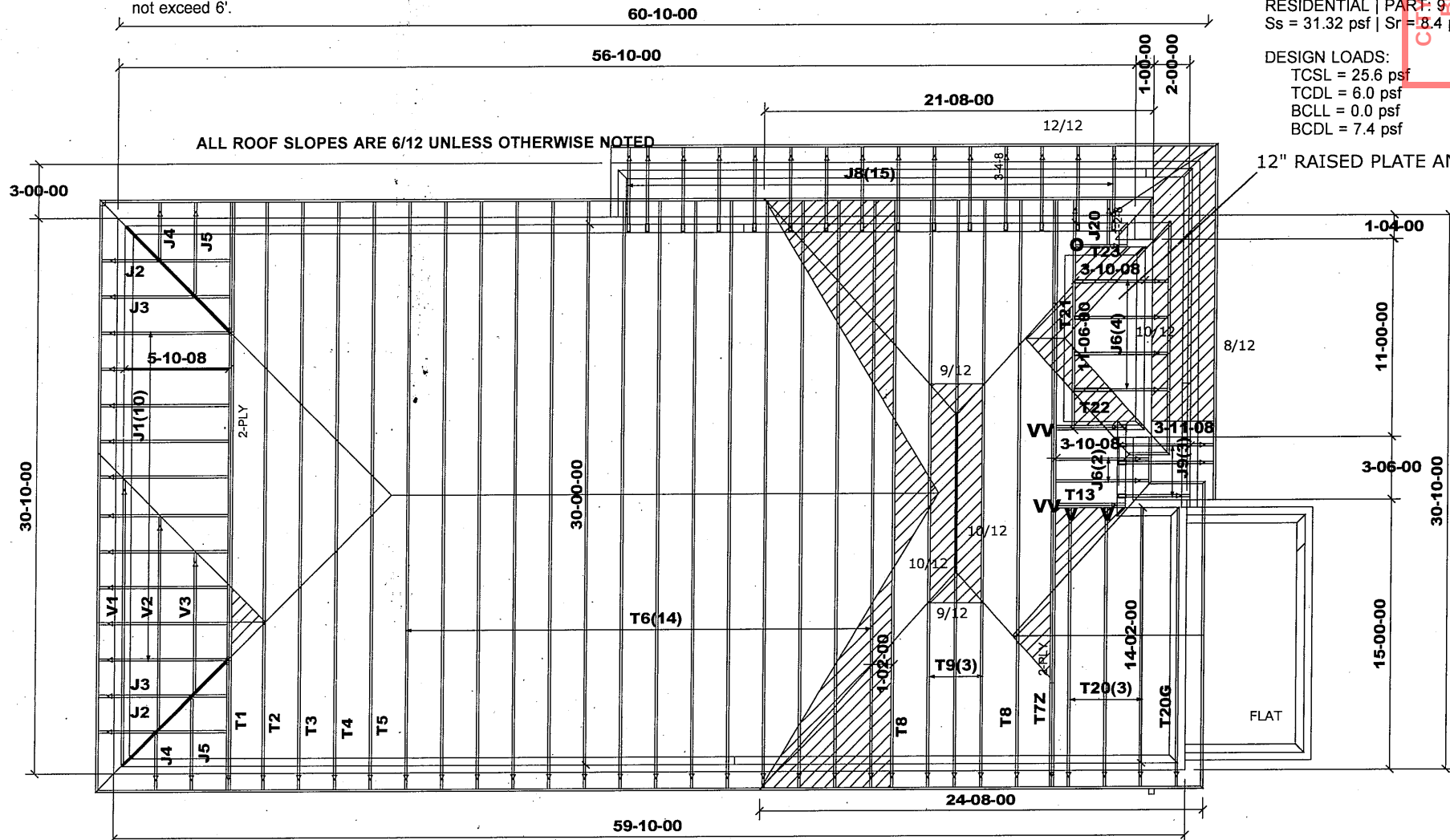


DENOTES:
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

12" RAISED PLATE AND CEILING



1000 RICHMOND HILL
 CITY OF RICHMOND DIVISION
 03/01/2024
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 Per: Joshua Nabua

M15638



Job Track: **53256**

Plan Log: **207463**

Layout ID: **434361**

Builder / Location:

GREENPARK HOMES / RICHMOND HILL

Model / Elevation:

ROSE 5 / 2- REAR UPGRADE

Project: **TRINIGROUP DEVELOPMENTS**

Date: 2023-08-25 Sales: Rick DiCiano Designer: YPG

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Mitek ver 8.6.3.353

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.



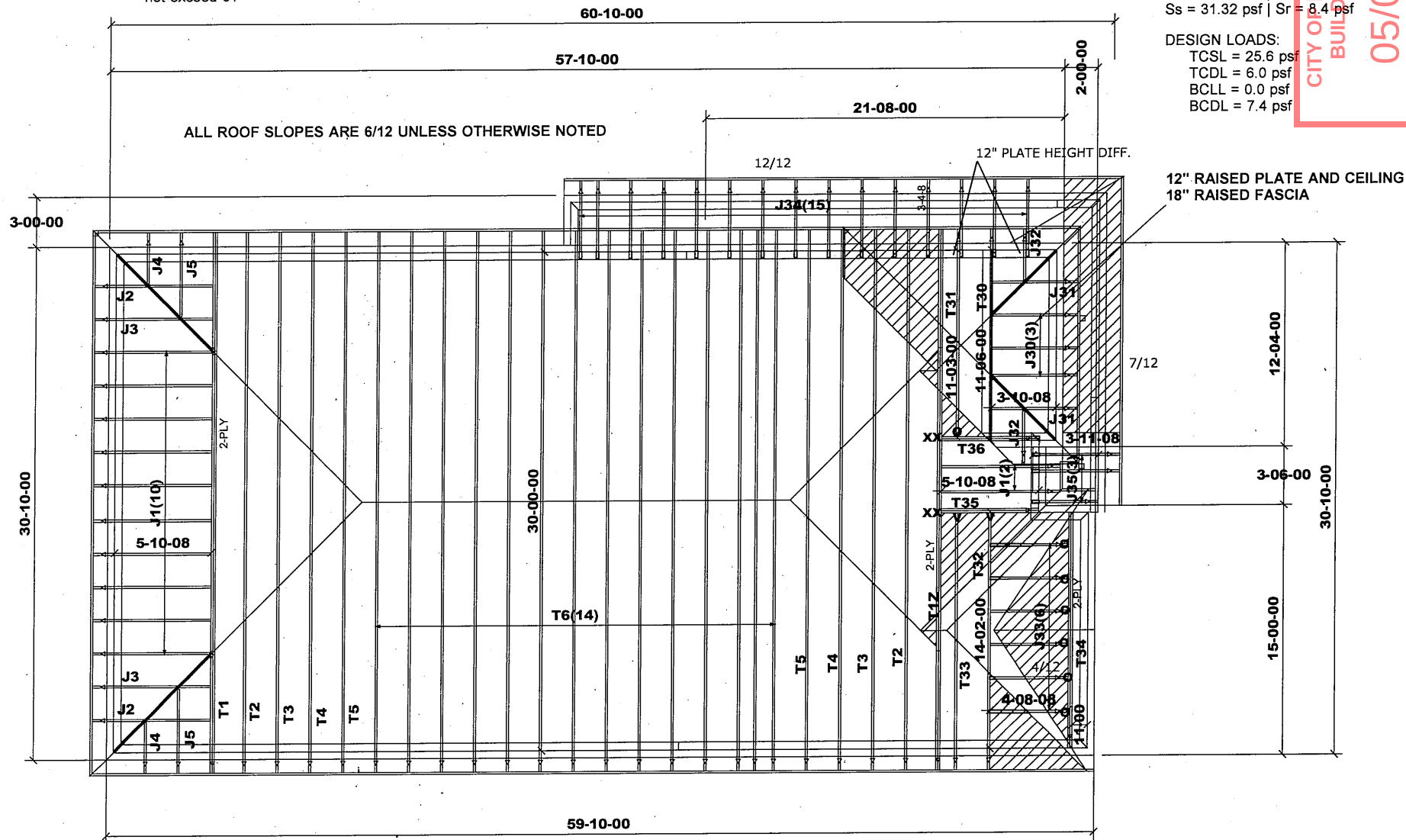
DENOTES:
CONVENTIONAL
FRAMING

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2- (XX)

DESIGN CONFORMS WITH OBC-2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: Joshua Nabua



M15638



Job Track: **53256**

Plan Log: **207463**

Layout ID: **434586**

Builder / Location:

GREENPARK HOMES / RICHMOND HILL

Project: **TRINIGROUP DEVELOPMENTS**

Date: **2023-08-25**

Sales:

Rick DiCiano

Designer: YPG

Model / Elevation:

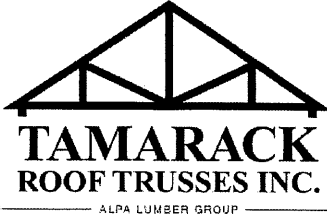
ROSE 5 / 3-STD.OR OPT.5 BED

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Mitek ver 8.6.3.353

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 1- STD.OR OPT.5 BED

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434359
 Ref #
 Page: 1 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

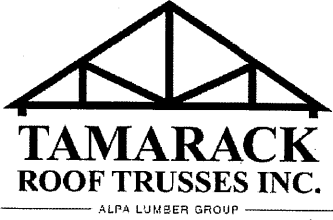
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	1	T2 Hip	6 /12	30-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.11 74.83		
	1	T3 Hip	6 /12	30-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	123.67 78.67		
	1	T4 Hip	6 /12	30-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	120.78 75.17		
	1	T5 Hip	6 /12	30-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.37 80.00		
	14	T6 Common	6 /12	30-00-00	8-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1707.34 1075.67		
	1 2-ply	T7 Hip Girder	9 /12	30-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	286.12 185.67		
	2	T8 Hip	9 /12	30-00-00	6-06-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	265.73 166.67		
	3	T9 Hip	9 /12	30-00-00	8-02-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	415.09 265.00		
	4	T10 Common	9 /12	14-02-00	7-04-00	2 x 4	1-03-08 1-03-08	1-06-04 2-06-04	264.44 172.00		
	1	T11G GABLE	9 /12	8-00-00	5-06-04	2 x 4	1-05-00 1-03-08	2-06-04 2-06-04	43.49 28.17		
	2	T12 Common	9 /12	11-06-00	5-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	98.54 66.00		
	1	T12G GABLE	9 /12	11-06-00	5-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	52.99 36.00		
	1 2-ply	T13 Jack-Open Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.82 30.67		

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

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 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 1- STD.OR OPT.5 BED

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434359
 Ref #
 Page: 2 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T13Z Jack-Closed Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.82 30.67		
	1	T14W Hip Girder	12 /12	13-02-00	2-01-02	2 x 4		1-03-04 1-03-04	51.68 34.33		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J6 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	30.9 20.33		
	7	J7W Jack-Open	4 /12	5-03-08	2-06-00	2 x 4	1-03-08	3-15 2-01-02	98.41 65.33		
	15	J8 Jack-Open	12 /12	3-04-08	5-00-15	2 x 4	1-03-08	10-02 4-02-10	182.97 120.00		
	3	J9 Jack-Open	8 /12	3-11-08	3-10-03	2 x 4	1-03-08	4-07 3-00-02	37.29 24.00		

TOTAL # TRUSS= 85

TOTAL BFT OF ALL TRUSSES= 2962.51

BFT.

TOTAL WEIGHT OF ALL TRSSES 4637.46 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	LJS26DS	
2	Hardware	LUS26-2	
11	Hardware	H2.5T	

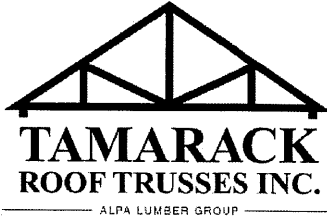
TOTAL NUMBER OF ITEMS= 16

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

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 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 2- STD .OR OPT.5 BED

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434360
 Ref #
 Page: 1 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

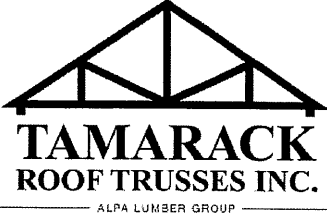
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	1	T2 Hip	6 /12	30-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.11 74.83		
	1	T3 Hip	6 /12	30-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	123.67 78.67		
	1	T4 Hip	6 /12	30-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	120.78 75.17		
	1	T5 Hip	6 /12	30-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	134.4 85.00		
	14	T6 Common	6 /12	30-00-00	8-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1757.41 1103.67		
	1 2-ply	T7Z Hip Girder	9 /12	30-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	286.12 185.67		
	2	T8 Hip	9 /12	30-00-00	6-06-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	271.69 170.00		
	3	T9 Hip	9 /12	30-00-00	8-02-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	415.09 265.00		
	1 2-ply	T13 Jack-Open Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.82 30.67		
	3	T20 Common	9 /12	14-02-00	6-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	191.65 122.50		
	1	T20G GABLE	9 /12	14-02-00	6-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	66.04 43.83		
	1	T21 Hip Girder	9 /12	11-06-00	5-10-07	2 x 4	1-03-08	1-06-04 1-06-04	63.12 43.50		
	1 2-ply	T22 Roof Special	0 /12	3-10-08	1-00-00	2 x 4		1-00-00 1-00-00	24.97 17.33		

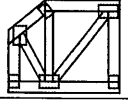
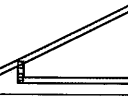
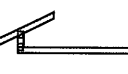
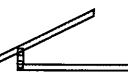
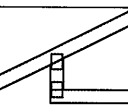
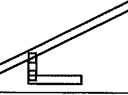
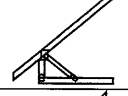
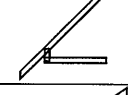
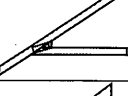
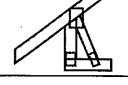
CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST					
	Lumber Yard:	TAMARACK LUMBER			Job Track:	53256
	Builder:	GREENPARK HOMES			PlanLog:	207463
	Project:	TRINIGROUP DEVELOPMENTS			Layout ID:	434360
	Location:	RICHMOND HILL			Ref #	
	Model:	ROSE 5			Page:	2 of 2
Lot #:				Date:	08-25-2023	
Elevation:	2- STD .OR OPT.5 BED			Designer:		
				Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T23 Half Hip Girder	10 /12	2-10-08	2-05-02	2 x 4		1-07-11 2-05-02	15.18 10.83		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	6	J6 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	92.69 61.00		
	15	J8 Jack-Open	12 /12	3-04-08	5-00-15	2 x 4	1-03-08	10-02 4-02-10	182.97 120.00		
	3	J9 Jack-Open	8 /12	3-11-08	3-10-03	2 x 4	1-03-08	4-07 3-00-02	37.29 24.00		
	1	J20 Jack-Open	9 /12	1-02-08	2-05-02	2 x 4	1-03-08	1-06-04 2-05-02	7.44 6.33		

TOTAL # TRUSS= 80 TOTAL BFT OF ALL TRUSSES= 2851.33 BFT. TOTAL WEIGHT OF ALL TRSSES 4477.35 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	LUS26-2	

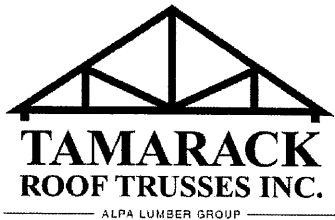
TOTAL NUMBER OF ITEMS= 5

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 2- REAR UPGRADE

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434361
 Ref #
 Page: 1 of 3
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	1	T2 Hip	6 /12	30-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.11 74.83		
	1	T3 Hip	6 /12	30-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	123.67 78.67		
	1	T4 Hip	6 /12	30-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	120.78 75.17		
	1	T5 Hip	6 /12	30-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	134.4 85.00		
	14	T6 Common	6 /12	30-00-00	8-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1757.41 1103.67		
	1 2-ply	T7Z Hip Girder	9 /12	30-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	286.12 185.67		
	2	T8 Hip	9 /12	30-00-00	6-06-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	271.69 170.00		
	3	T9 Hip	9 /12	30-00-00	8-02-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	415.09 265.00		
	1 2-ply	T13 Jack-Open Girder	10 /12	3-10-08	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	44.82 30.67		
	3	T20 Common	9 /12	14-02-00	6-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	191.65 122.50		
	1	T20G GABLE	9 /12	14-02-00	6-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	66.04 43.83		
	1	T21 Hip Girder	9 /12	11-06-00	5-10-07	2 x 4	1-03-08	1-06-04 1-06-04	63.12 43.50		
	1 2-ply	T22 Roof Special	0 /12	3-10-08	1-00-00	2 x 4		1-00-00 1-00-00	24.97 17.33		

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 2- REAR UPGRADE

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434361
 Ref #
 Page: 2 of 3
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T23 Half Hip Girder	10 /12	2-10-08	2-05-02	2 x 4		1-07-11 2-05-02	15.18 10.83		
	1	V1 Valley	6 /12	15-09-00	3-11-04	2 x 4			45.16 29.67		
	1	V2 Valley	6 /12	11-09-00	2-11-04	2 x 4			28.59 17.50		
	1	V3 Valley	6 /12	7-09-00	1-11-04	2 x 4			18.12 11.67		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	6	J6 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	92.69 61.00		
	15	J8 Jack-Open	12 /12	3-04-08	5-00-15	2 x 4	1-03-08	10-02 4-02-10	182.97 120.00		
	3	J9 Jack-Open	8 /12	3-11-08	3-10-03	2 x 4	1-03-08	4-07 3-00-02	37.29 24.00		
	1	J20 Jack-Open	9 /12	1-02-08	2-05-02	2 x 4	1-03-08	1-06-04 2-05-02	7.44 6.33		

TOTAL # TRUSS= 83

TOTAL BFT OF ALL TRUSSES= 2910.17

BFT.

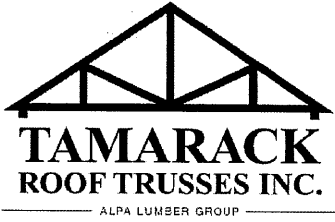
TOTAL WEIGHT OF ALL TRSSES 4569.22 LBS

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 2- REAR UPGRADE

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434361
 Ref #
 Page: 3 of 3
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	LUS26-2	
6		VTOR	

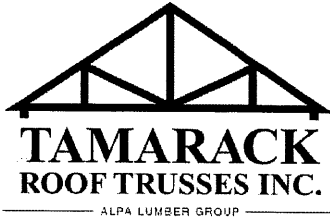
TOTAL NUMBER OF ITEMS= 11

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 3 -STD.OR OPT.5 BED

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434586
 Ref #
 Page: 1 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	1 2-ply	T1Z Hip Girder	6 /12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	2	T2 Hip	6 /12	30-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	236.22 149.67		
	2	T3 Hip	6 /12	30-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	247.35 157.33		
	2	T4 Hip	6 /12	30-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	241.56 150.33		
	2	T5 Hip	6 /12	30-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	268.8 170.00		
	14	T6 Common	6 /12	30-00-00	8-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1757.41 1103.67		
	1	T30 Hip Girder	6 /12	11-06-00	3-07-04	2 x 4	1-03-08 1-03-08	1-08-00 1-08-00	51.96 34.67		
	1	T31 Hip	6 /12	11-03-00	5-03-08	2 x 4	1-03-08 1-03-08	2-06-08 2-08-00	48.64 31.17		
	1	T32 Hip Girder	6 /12	14-02-00	3-04-12	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	65.29 43.33		
	1	T33 Hip	6 /12	14-02-00	4-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	61.03 38.67		
	1 2-ply	T34 Common Girder	4 /12	14-02-00	3-03-08	2 x 4		11-03 11-03	101.7 68.33		
	1 2-ply	T35 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T36 Flat Girder	0 /12	5-10-08	1-00-00	2 x 4		1-00-00 1-00-00	37.74 25.67		

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

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 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 3 -STD.OR OPT.5 BED

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434586
 Ref #
 Page: 2 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	3	J30 Jack-Open	6 /12	3-10-08	3-07-04	2 x 4	1-03-08	1-08-00 3-07-04	42.69 28.50		
	2	J31 Jack-Open	6 /12	1-09-07	2-06-12	2 x 4	1-03-08 2-01-01	1-08-00 2-06-12	22.47 15.33		
	2	J32 Jack-Open	6 /12	1-09-07	2-06-12	2 x 4	1-03-08 1-01	1-08-00 2-06-12	17.91 12.67		
	6	J33 Jack-Open	6 /12	4-08-08	3-04-12	2 x 4		1-00-08 3-04-12	73.12 48.00		
	15	J34 Jack-Open	12 /12	3-04-08	5-00-15	2 x 4	1-03-08	11-06 4-03-14	184.77 120.00		
	3	J35 Jack-Open	7 /12	3-11-08	3-04-13	2 x 4	1-03-08	4-05 2-08-00	36.63 24.00		

TOTAL # TRUSS= 87

TOTAL BFT OF ALL TRUSSES= 2787

BFT.

TOTAL WEIGHT OF ALL TRSSES 4382.53 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

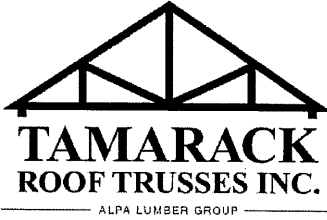
TOTAL NUMBER OF ITEMS= 11

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 3- REAR UPGRADE

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434588
 Ref #
 Page: 1 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

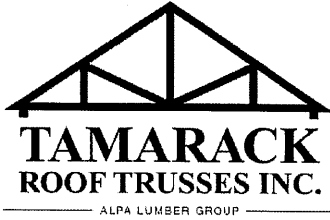
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 / 12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	1 2-ply	T1Z Hip Girder	6 / 12	30-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	271.35 173.33		
	2	T2 Hip	6 / 12	30-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	236.22 149.67		
	2	T3 Hip	6 / 12	30-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	247.35 157.33		
	2	T4 Hip	6 / 12	30-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	241.56 150.33		
	2	T5 Hip	6 / 12	30-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	268.8 170.00		
	14	T6 Common	6 / 12	30-00-00	8-08-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1757.41 1103.67		
	1	T30 Hip Girder	6 / 12	11-06-00	3-07-04	2 x 4	1-03-08 1-03-08	1-08-00 1-08-00	51.96 34.67		
	1	T31 Hip	6 / 12	11-03-00	5-03-08	2 x 4	1-03-08 1-03-08	2-06-08 2-08-00	48.64 31.17		
	1	T32 Hip Girder	6 / 12	14-02-00	3-04-12	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	65.29 43.33		
	1	T33 Hip	6 / 12	14-02-00	4-04-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	61.03 38.67		
	1 2-ply	T34 Common Girder	4 / 12	14-02-00	3-03-08	2 x 4		11-03 11-03	101.7 68.33		
	1 2-ply	T35 Jack-Closed Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T36 Flat Girder	0 / 12	5-10-08	1-00-00	2 x 4		1-00-00 1-00-00	37.74 25.67		

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

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 Per: joshua.nabua

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: TRINIGROUP DEVELOPMENTS
 Location: RICHMOND HILL
 Model: ROSE 5
 Lot #:
 Elevation: 3- REAR UPGRADE

Job Track: 53256
 PlanLog: 207463
 Layout ID: 434588
 Ref #
 Page: 2 of 2
 Date: 08-25-2023
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	12	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	201.53 128.00		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
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	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
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	2	J31 Jack-Open	6 /12	1-09-07	2-06-12	2 x 4	1-03-08 2-01-01	1-08-00 2-06-12	22.47 15.33		
	2	J32 Jack-Open	6 /12	1-09-07	2-06-12	2 x 4	1-03-08 1-01	1-08-00 2-06-12	17.91 12.67		
	6	J33 Jack-Open	6 /12	4-08-08	3-04-12	2 x 4		1-00-08 3-04-12	73.12 48.00		
	15	J34 Jack-Open	12 /12	3-04-08	5-00-15	2 x 4	1-03-08	11-06 4-03-14	184.77 120.00		
	3	J35 Jack-Open	7 /12	3-11-08	3-04-13	2 x 4	1-03-08	4-05 2-08-00	36.63 24.00		

TOTAL # TRUSS= 87

TOTAL BFT OF ALL TRUSSES= 2787

BFT.

TOTAL WEIGHT OF ALL TRSSES 4382.53 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
7	Hardware	LUS24	
2	Hardware	LJS26DS	
2	Hardware	HGUS26-2	
6		VTOR	

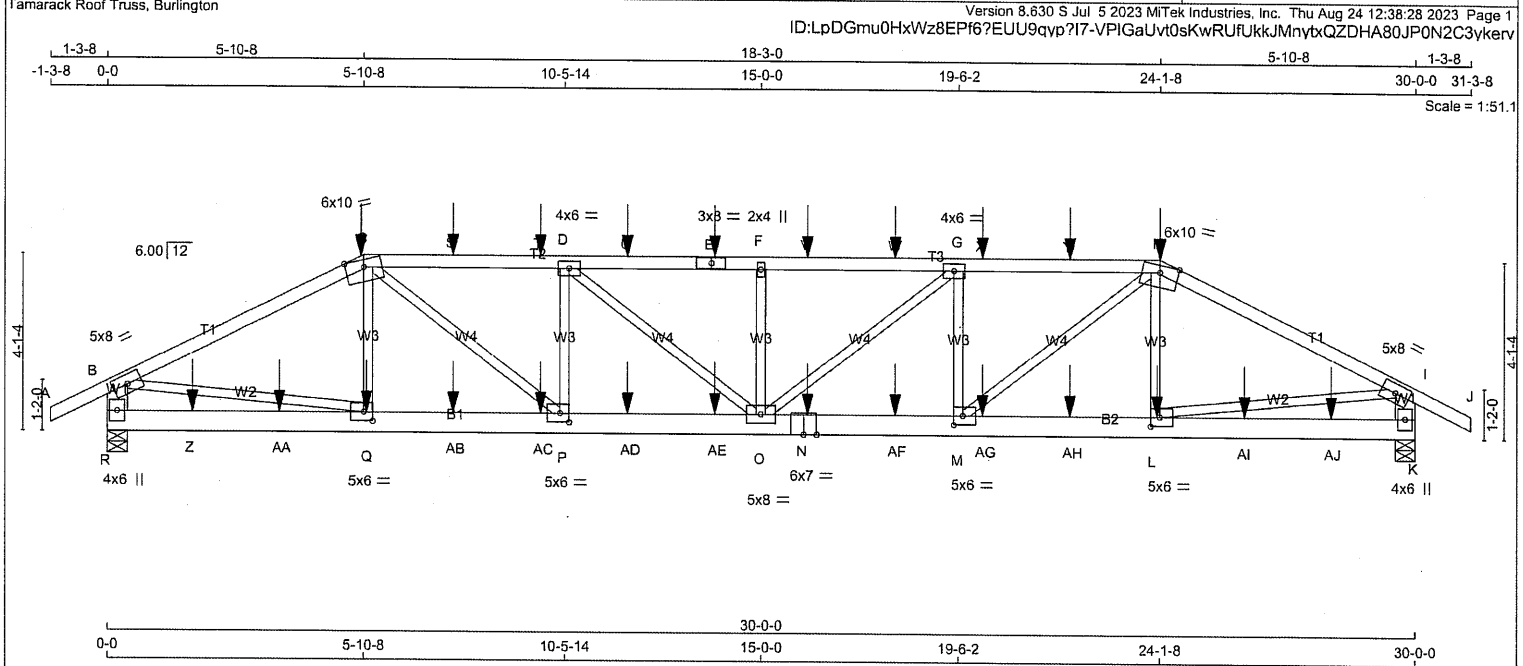
TOTAL NUMBER OF ITEMS= 17

CITY OF RICHMOND HILL
 BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T1	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.630 S Jul 5 2023 Mitek Industries, Inc. Thu Aug 24 12:38:28 2023 Page 1			
		ID:LpDGmu0HxWz8EPf6?EUU9qyp?17-VPIGaUvt0SkwRUFUkkJMyntbQZDHA80JP0N2C3yker			



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF E - H 2x4 DRY No.2 SPF H - J 2x4 DRY No.2 SPF J - B 2x6 DRY No.2 SPF B - I 2x6 DRY No.2 SPF I - N 2x6 DRY No.2 SPF N - K 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE (61.0) C-E 1 12 SIDE (61.0) E-H 1 12 SIDE (61.0) H-J 1 12 SIDE (61.0) J-B 2 12 TOP B-I 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS R-N 2 12 SIDE (183.1) N-K 2 12 SIDE (183.1) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX R 2771 0 2771 0 0 5-8 1-8 K 2770 0 2770 0 0 5-8 1-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL R 1961 1281 / 0 0 / 0 0 / 0 680 / 0 0 / 0 K 1960 1280 / 0 0 / 0 0 / 0 680 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE VERT. LOAD LC1 MAX UNBRACED MEMB. (LBS) (PLF) (LC) LENGTH FR-TO A-B 0 / 28 -91.8 -91.8 0.07 (1) 10.00 Q-C -294 / 50 0.04 (1) B-C -4074 / 0 -91.8 -91.8 0.45 (1) 4.30 C-P 0 / 1996 0.25 (1) C-S -5188 / 0 -91.8 -91.8 0.36 (1) 3.93 P-D -1107 / 0 0.14 (1) S-T -5188 / 0 -91.8 -91.8 0.36 (1) 3.93 D-O 0 / 563 0.07 (1) T-D -5188 / 0 -91.8 -91.8 0.36 (1) 3.93 O-F -603 / 0 0.08 (1) D-U -5622 / 0 -91.8 -91.8 0.38 (1) 3.77 F-G 0 / 567 0.07 (1) U-E -5622 / 0 -91.8 -91.8 0.38 (1) 3.77 M-H -1107 / 0 0.14 (1) E-F -5622 / 0 -91.8 -91.8 0.38 (1) 3.77 M-H 0 / 1994 0.25 (1) F-V -5622 / 0 -91.8 -91.8 0.38 (1) 3.77 L-H -293 / 50 0.04 (1) V-W -5622 / 0 -91.8 -91.8 0.38 (1) 3.77 B-Q 0 / 3673 0.45 (1) W-G -5622 / 0 -91.8 -91.8 0.38 (1) 3.77 L-I 0 / 3671 0.45 (1) G-X -5185 / 0 -91.8 -91.8 0.36 (1) 3.93 X-Y -5185 / 0 -91.8 -91.8 0.36 (1) 3.93 Y-H -5185 / 0 -91.8 -91.8 0.36 (1) 3.93 H-I -4073 / 0 -91.8 -91.8 0.45 (1) 4.30 I-J 0 / 28 -91.8 -91.8 0.07 (1) 10.00 J-B -2692 / 0 0.0 0.0 0.09 (1) 7.81 K-I -2691 / 0 0.0 0.0 0.09 (1) 7.81 R-Z 0 / 0 -18.5 -18.5 0.07 (4) 10.00 Z-AA 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AA-Q 0 / 0 -18.5 -18.5 0.07 (4) 10.00 Q-AB 0 / 3634 -18.5 -18.5 0.27 (1) 10.00 AB-AC 0 / 3634 -18.5 -18.5 0.27 (1) 10.00 AC-P 0 / 3634 -18.5 -18.5 0.27 (1) 10.00 P-AD 0 / 5188 -18.5 -18.5 0.39 (1) 10.00 AD-AE 0 / 5188 -18.5 -18.5 0.39 (1) 10.00 AE-O 0 / 5188 -18.5 -18.5 0.39 (1) 10.00 O-N 0 / 5185 -18.5 -18.5 0.39 (1) 10.00 N-AF 0 / 5185 -18.5 -18.5 0.39 (1) 10.00 AF-M 0 / 5185 -18.5 -18.5 0.39 (1) 10.00 M-AG 0 / 3633 -18.5 -18.5 0.27 (1) 10.00 AG-AH 0 / 3633 -18.5 -18.5 0.27 (1) 10.00 AH-L 0 / 3633 -18.5 -18.5 0.27 (1) 10.00 L-AI 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AI-AJ 0 / 0 -18.5 -18.5 0.07 (4) 10.00 AJ-K 0 / 0 -18.5 -18.5 0.07 (4) 10.00 SPECIFIED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONN. C 5-10-8 -254 -254 --- BACK VERT TOTAL --- C1 E 13-11-4 -76 -76 --- BACK VERT TOTAL --- C1				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.00") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14") ALLOWABLE DEFL.(TL)= L/360 (1.00") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26") CSI: TC=0.45/1.00 (B-C:1) , BC=0.39/1.00 (O-P:1) , WB=0.45/1.00 (B-Q:1) , SSI=0.18/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.63 (L) (INPUT = 0.90) JSI METAL= 0.41 (Q) (INPUT = 0.95)			
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STRUCTURAL COMPONENT ONLY
DWG # TR23080882

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T1	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:LpDGmu0HxWz8EPf6?EUU9qyp?17-VPIGaUvt0sKwRUfUkkJMnytxQZDHA80JP0N2C3ykerv

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	6.0	10.0	Edge	
D	TMW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW-w	MT20	2.0	4.0		
G	TMW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	6.0	10.0	Edge	
I	TMW-t	MT20	5.0	8.0		
K	BMV1-p	MT20	4.0	6.0		
L, M, P, Q						
L	BMW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	6.0	7.0		
O	BMW-t	MT20	5.0	8.0		
R	BMV1-p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	24-1-8	-254	-254	---	BACK	VERT	TOTAL	---	C1
L	24-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
N	16-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Q	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
S	7-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
T	9-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
U	11-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	16-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	18-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	20-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	22-0-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	1-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
AA	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	7-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	9-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	11-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	13-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	18-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AG	20-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AH	22-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AI	26-0-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AJ	28-0-12	-20	-20	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

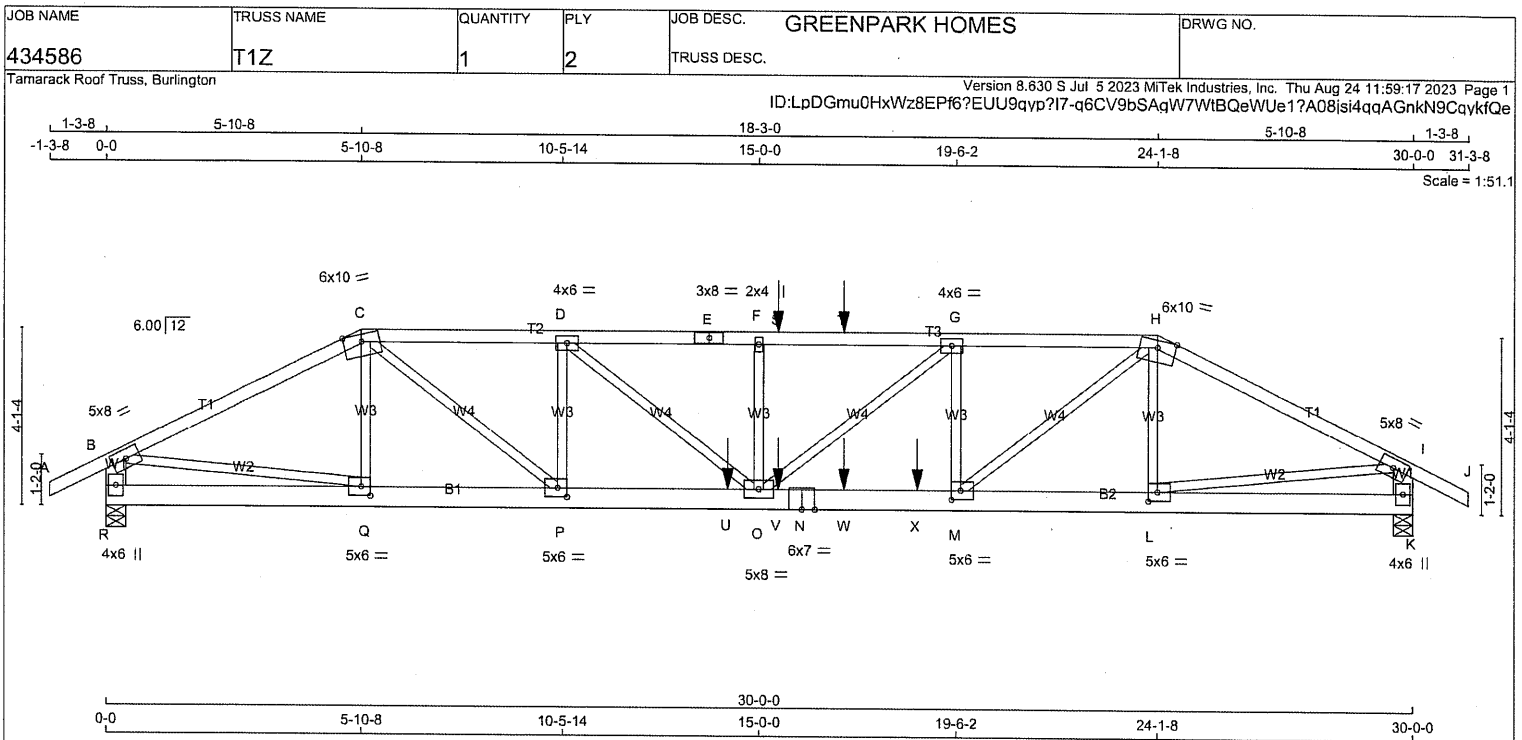


STRUCTURAL COMPONENT ONLY
DWG # TR23080882

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
E - H 1	12	SIDE(0.0)
H - J 1	12	TOP
R - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R - N 2	12	SIDE(0.0)
N - K 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UP	IN-SX
R 3396	0	0	1-13
K 3678	0	0	2-0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
R 2393	1620 / 0	0 / 0	0 / 0	0 / 0	773 / 0
K 2591	1755 / 0	0 / 0	0 / 0	0 / 0	836 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.77 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO		
A-B	0 / 28	-91.8 -91.8	0.07 (1)	10.00	Q-C	-550 / 0
B-C	-5209 / 0	-91.8 -91.8	0.53 (1)	3.81	C-P	0 / 3862
C-D	-7648 / 0	-91.8 -91.8	0.39 (1)	3.30	P-D	-2092 / 0
D-E	-9639 / 0	-91.8 -91.8	0.52 (1)	2.83	D-O	0 / 2584
E-F	-9639 / 0	-91.8 -91.8	0.52 (1)	2.83	O-F	-553 / 0
F-S	-9639 / 0	-91.8 -91.8	0.58 (1)	2.77	O-G	0 / 1506
S-T	-9639 / 0	-91.8 -91.8	0.58 (1)	2.77	M-G	-1481 / 0
T-G	-9639 / 0	-91.8 -91.8	0.58 (1)	2.77	M-H	0 / 4368
G-H	-8478 / 0	-91.8 -91.8	0.48 (1)	3.06	L-H	-640 / 0
H-I	-5700 / 0	-91.8 -91.8	0.57 (1)	3.64	B-Q	0 / 4695
I-J	0 / 28	-91.8 -91.8	0.07 (1)	10.00	L-I	0 / 5138
R-B	-3332 / 0	0.0 0.0	0.12 (1)	7.61		
K-I	-3609 / 0	0.0 0.0	0.13 (1)	7.39		
R-Q	0 / 0	-18.5 -18.5	0.04 (4)	10.00		
Q-P	0 / 4641	-18.5 -18.5	0.34 (1)	10.00		
P-U	0 / 7648	-18.5 -18.5	0.76 (1)	10.00		
U-O	0 / 7648	-18.5 -18.5	0.76 (1)	10.00		
O-V	0 / 8478	-18.5 -18.5	0.85 (1)	10.00		
V-N	0 / 8478	-18.5 -18.5	0.85 (1)	10.00		
N-W	0 / 8478	-18.5 -18.5	0.85 (1)	10.00		
W-X	0 / 8478	-18.5 -18.5	0.85 (1)	10.00		
X-M	0 / 8478	-18.5 -18.5	0.85 (1)	10.00		
M-L	0 / 5077	-18.5 -18.5	0.41 (1)	10.00		
L-K	0 / 0	-18.5 -18.5	0.05 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
S	15-5.4	-76	-76	---	FRONT	VERT	TOTAL	---
T	16-11.4	-76	-76	---	FRONT	VERT	TOTAL	---
U	14-3.8	-1274	-1274	---	FRONT	VERT	TOTAL	---
V	15-5.4	-21	-21	---	FRONT	VERT	TOTAL	---
W	16-11.4	-21	-21	---	FRONT	VERT	TOTAL	---
X	18-7.8	-1005	-1005	---	FRONT	VERT	TOTAL	---

CONNECTION REQUIREMENTS

1). C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/844 (0.43")

CSI: TC=0.58/1.00 (F-G:1), BC=0.85/1.00 (M-O:1), WB=0.64/1.00 (I-L:1), SSI=0.54/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

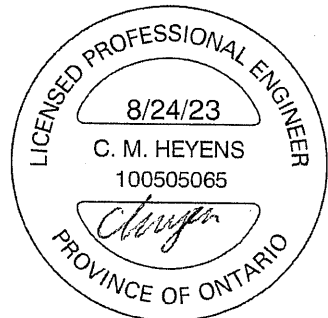
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.78 (N) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080914

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	T1Z	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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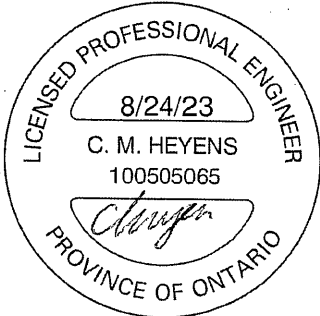
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	6.0	10.0	Edge	
D	TMVW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMVW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	6.0	10.0	Edge	
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	4.0	6.0		
L, M, P, Q						
L	BMVW-t	MT20	5.0	6.0	2.50	2.50
N	BS-t	MT20	6.0	7.0		
O	BMVW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080914

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T2	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

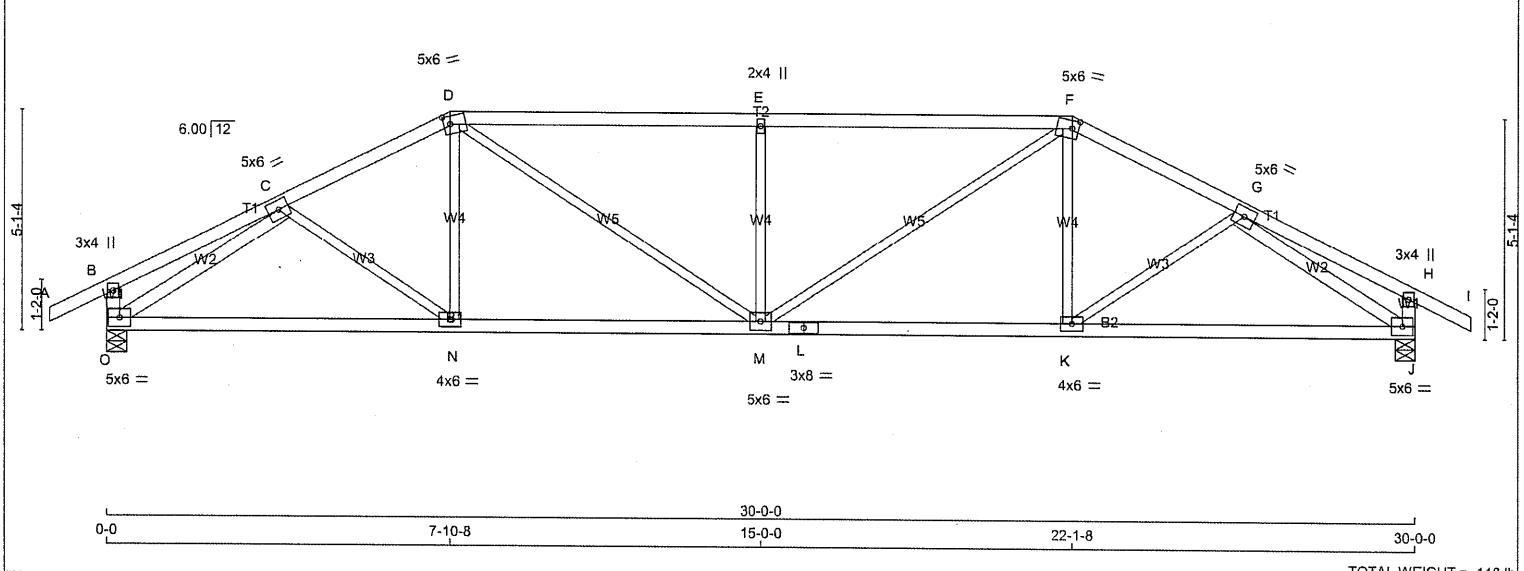
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1-3-8 7-10-8 14-3-0 7-10-8 1-3-8

-1-3-8 0-0 4-0-8 7-10-8 15-0-0 22-1-8 25-11-8 30-0-0 31-3-8

Scale = 1:51.1



LUMBER				TOTAL WEIGHT = 118 lb			
N. L. G. A. RULES				(M/F)			
CHORDS	SIZE	LUMBER	DESCR.				
A - D	2x4	DRY	No.2				
D - F	2x4	DRY	No.2				
F - I	2x4	DRY	No.2				
O - B	2x4	DRY	No.2				
J - H	2x4	DRY	No.2				
O - L	2x4	DRY	No.2				
L - J	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2				
EXCEPT							
O - C	2x4	DRY	No.2				
G - J	2x4	DRY	No.2				
DRY: SEASONED LUMBER.							
PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	5.0	6.0			
D	TTWW-m	MT20	5.0	6.0	2.25	1.75	
E	TMW+w	MT20	2.0	4.0			
F	TTWW-m	MT20	5.0	6.0	2.25	1.75	
G	TMWW-t	MT20	5.0	6.0			
H	TMV+p	MT20	3.0	4.0			
J	BMVW1-t	MT20	5.0	6.0			
K	BMWW-t	MT20	4.0	6.0			
L	BS-t	MT20	3.0	8.0			
M	BMWWW-t	MT20	5.0	6.0			
N	BMWW-t	MT20	4.0	6.0			
O	BMVW1-t	MT20	5.0	6.0			
NOTES- (1)							
1) Lateral braces to be a minimum of 2X4 SPF #2.							
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
FACTORED							
GROSS REACTION							
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1778	0	1778	0	0	5-8	1-15
J	1778	0	1778	0	0	5-8	1-15
UNFACTORED REACTIONS							
1ST LCASE							
COMBINED							
O	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0
J	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J							
BRACING							
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.							
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.							
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.							
LOADING							
TOTAL LOAD CASES: (4)							
CHORDS							
MAX. FACTORED							
FORCE							
MEMB.							
FR-TO							
A-B							
B-C							
C-D							
D-E							
E-F							
F-G							
G-H							
H-I							
O-B							
J-H							
O-N							
N-M							
M-L							
L-K							
K-J							
WEBS							
MAX. FACTORED							
FORCE							
MEMB.							
FR-TO							
C-N							
N-D							
D-M							
M-E							
M-F							
K-F							
K-G							
O-C							
G-J							
DESIGN CRITERIA							
SPECIFIED LOADS:							
TOP CH.							
LL =							
DL =							
BOT CH.							
LL =							
DL =							
TOTAL LOAD =							
SPACING =							
24.0							
IN. C/C							
LOADING IN FLAT SECTION BASED ON A SLOPE							
OF 6.00/12							
THIS TRUSS IS DESIGNED FOR RESIDENTIAL							
OR SMALL BUILDING REQUIREMENTS OF PART							
9, NBCC 2015							
THIS DESIGN COMPLIES WITH:							
- PART 9 OF BCBC 2018 , NBC-2019AE							
- PART 9 OF OBC 2012 (2019 AMENDMENT)							
- CSA 086-14							
- TPC 2014							
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.							
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED							
ROOF LIVE LOAD							
ALLOWABLE DEFL.(LL) = L/360 (1.00")							
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")							
ALLOWABLE DEFL.(TL) = L/360 (1.00")							
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")							
CSI: TC=0.76/1.00 (D-E:1) . BC=0.48/1.00 (M-N:1) .							
WB=0.69/1.00 (C-O:1) , SSI=0.32/1.00 (D-E:1)							
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10							
COMP=1.10 SHEAR=1.10 TENS= 1.10							
COMPANION LIVE LOAD FACTOR = 1.00							
AUTOSOLVE HEELS OFF							
TRUSS PLATE MANUFACTURER IS NOT							
RESPONSIBLE FOR QUALITY CONTROL IN THE							
TRUSS MANUFACTURING PLANT .							
NAIL VALUES							
PLATE GRIP(DRY) SHEAR SECTION							
(PSI) (PLI) (PLI)							
MAX MIN MAX MIN MAX MIN							
MT20 650 371 1747 788 1987 1873							
PLATE PLACEMENT TOL. = 0.250 inches							
PLATE ROTATION TOL. = 5.0 Deg.							
JSI GRIP= 0.89 (M) (INPUT = 0.90)							
JSI METAL= 0.58 (L) (INPUT = 0.95)							

8/24/23

C. M. HEYENS

100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

DWG # TR23080883

CITY OF RICHMOND HILL

BUILDING DIVISION

05/01/2024

RECEIVED

Per: joshua.nabua

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T3	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

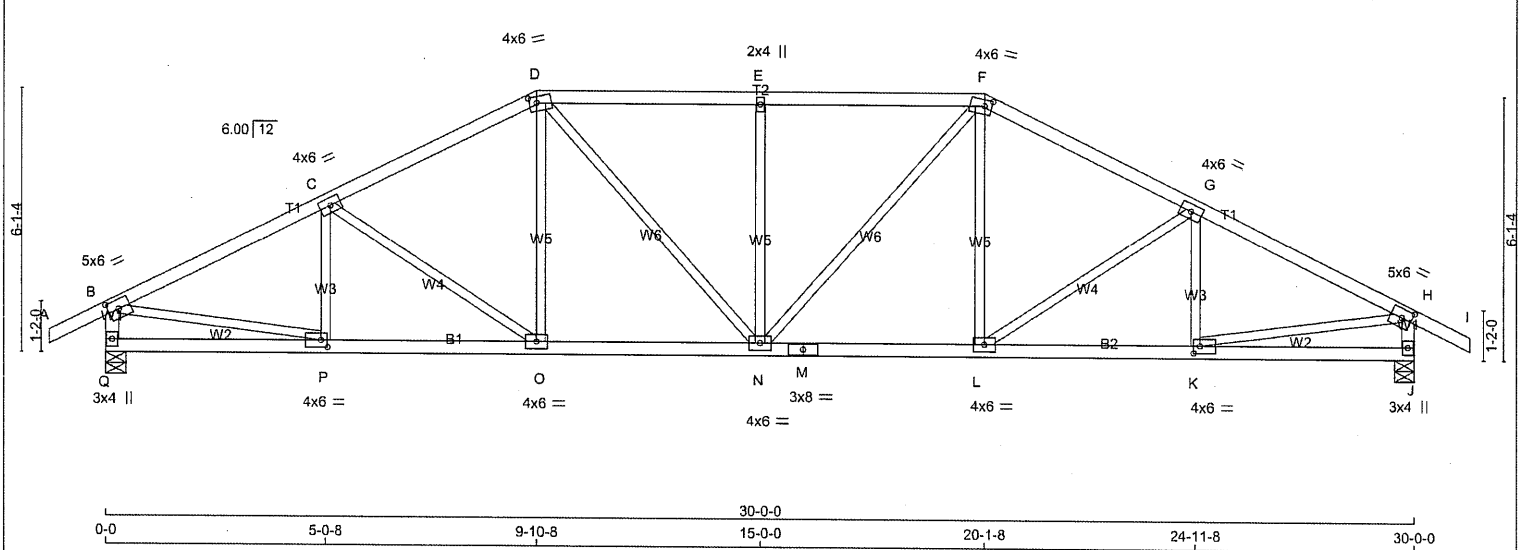
Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:31 2023 Page 1

ID:LpDGmu0HxWz8EPf6?EUU9qyp?17-v RPCVxliniVlxN3Pst3PbVUzmErNVEI5 cipOykers

1-3-8 9-10-8 10-3-0 9-10-8 1-3-8

-1-3-8 0-0 5-0-8 9-10-8 15-0-0 20-1-8 24-11-8 30-0-0 31-3-8

Scale = 1:51.1



LUMBER				TOTAL WEIGHT = 124 lb			
N. L. G. A. RULES				(M/F)			
CHORDS	SIZE	LUMBER	DESCR.				
A - D	2x4	DRY	No.2				
D - F	2x4	DRY	No.2				
F - I	2x4	DRY	No.2				
Q - B	2x4	DRY	No.2				
J - H	2x4	DRY	No.2				
Q - M	2x4	DRY	No.2				
M - J	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2				
EXCEPT			SPF				
DRY: SEASONED LUMBER.							
PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW-t	MT20	5.0	6.0	2.50	2.75	
C	TMVW-t	MT20	4.0	6.0			
D	TTWW-m	MT20	4.0	6.0	1.75	2.00	
E	TMVW-w	MT20	2.0	4.0			
F	TTWW-m	MT20	4.0	6.0	1.75	2.00	
G	TMVW-t	MT20	4.0	6.0			
H	TMVW-t	MT20	5.0	6.0	2.50	2.75	
J	BMV1+p	MT20	3.0	4.0			
K	BMVW-t	MT20	4.0	6.0	2.00	1.75	
L	BMVW-t	MT20	4.0	6.0			
M	BS-t	MT20	3.0	8.0			
N	BMVW-w	MT20	4.0	6.0			
O	BMVW-t	MT20	4.0	6.0			
P	BMVW-t	MT20	4.0	6.0	2.00	1.75	
Q	BMV1+p	MT20	3.0	4.0			
NOTES- (1)							
1) Lateral braces to be a minimum of 2X4 SPF #2.							
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
FACTORED							
GROSS REACTION							
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1778	0	1778	0	0	5-8	2-4
J	1778	0	1778	0	0	5-8	2-4
UNFACTORED REACTIONS							
1ST CASE							
COMBINED							
Q	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0
J	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J							
BRACING							
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.							
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.							
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.							
LOADING							
TOTAL LOAD CASES: (4)							
CHORDS							
MAX. FACTORED							
MEMB. FORCE (LBS)							
FR-TO							
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P-C	-271 / 0
B-C	-2364 / 0	-91.8	-91.8	0.36 (1)	4.15	C-O	-295 / 0
C-D	-2139 / 0	-91.8	-91.8	0.34 (1)	4.34	O-D	0 / 265
D-E	-2187 / 0	-91.8	-91.8	0.37 (1)	4.26	D-N	0 / 442
E-F	-2187 / 0	-91.8	-91.8	0.37 (1)	4.26	N-E	-574 / 0
F-G	-2139 / 0	-91.8	-91.8	0.34 (1)	4.34	N-F	0 / 442
G-H	-2364 / 0	-91.8	-91.8	0.36 (1)	4.15	L-F	0 / 265
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-295 / 0
Q-B	-1736 / 0	0.0	0.0	0.18 (1)	6.32	K-G	-271 / 0
J-H	-1736 / 0	0.0	0.0	0.18 (1)	6.32	B-P	0 / 2165
						K-H	0 / 2165
Q-P	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
P-O	0 / 2134	-18.5	-18.5	0.40 (1)	10.00		
O-N	0 / 1895	-18.5	-18.5	0.36 (1)	10.00		
N-M	0 / 1895	-18.5	-18.5	0.36 (1)	10.00		
M-L	0 / 1895	-18.5	-18.5	0.36 (1)	10.00		
L-K	0 / 2134	-18.5	-18.5	0.40 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
WEBS							
MAX. FACTORED							
MEMB. FORCE (LBS)							
FR-TO							
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P-C	-271 / 0
B-C	-2364 / 0	-91.8	-91.8	0.36 (1)	4.15	C-O	-295 / 0
C-D	-2139 / 0	-91.8	-91.8	0.34 (1)	4.34	O-D	0 / 265
D-E	-2187 / 0	-91.8	-91.8	0.37 (1)	4.26	D-N	0 / 442
E-F	-2187 / 0	-91.8	-91.8	0.37 (1)	4.26	N-E	-574 / 0
F-G	-2139 / 0	-91.8	-91.8	0.34 (1)	4.34	N-F	0 / 442
G-H	-2364 / 0	-91.8	-91.8	0.36 (1)	4.15	L-F	0 / 265
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L-G	-295 / 0
Q-B	-1736 / 0	0.0	0.0	0.18 (1)	6.32	K-G	-271 / 0
J-H	-1736 / 0	0.0	0.0	0.18 (1)	6.32	B-P	0 / 2165
						K-H	0 / 2165
Q-P	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
P-O	0 / 2134	-18.5	-18.5	0.40 (1)	10.00		
O-N	0 / 1895	-18.5	-18.5	0.36 (1)	10.00		
N-M	0 / 1895	-18.5	-18.5	0.36 (1)	10.00		
M-L	0 / 1895	-18.5	-18.5	0.36 (1)	10.00		
L-K	0 / 2134	-18.5	-18.5	0.40 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00		
DESIGN CRITERIA							
SPECIFIED LOADS:							
TOP CH. LL = 25.6 PSF							
DL = 8.0 PSF							
BOT CH. LL = 0.0 PSF							
DL = 7.4 PSF							
TOTAL LOAD = 39.0 PSF							
SPACING = 24.0 IN. C/C							
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12							
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015							
THIS DESIGN COMPLIES WITH:							
- PART 9 OF CBC2018, NBC-2019AE							
- PART 9 OF OBC 2012 (2019 AMENDMENT)							
- CSA 086-14							
- TPIC 2014							
(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD							
ALLOWABLE DEFL.(LL) = L/360 (1.00")							
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")							
ALLOWABLE DEFL.(TL) = L/360 (1.00")							
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")							
CSI: TC=0.37/1.00 (D-E:1) BC=0.40/1.00 (K-L:1) WB=0.49/1.00 (H-K:1) SSI=0.23/1.00 (D-E:1)							
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10							
COMPANION LIVE LOAD FACTOR = 1.00							
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.							
NAIL VALUES							
PLATE GRIP(DRY) SHEAR SECTION							
(PSI) (PLI) (PLI)							
MAX MIN MAX MIN MAX MIN							
MT20 650 371 1747 788 1987 1873							
PLATE PLACEMENT TOL. = 0.250 inches							
PLATE ROTATION TOL. = 5.0 Deg.							
JSI GRIP= 0.89 (K) (INPUT = 0.90)							
JSI METAL= 0.57 (B) (INPUT = 0.95)							

8/24/23

C. M. HEYENS

100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

DWG # TR23080884

CITY OF RICHMOND HILL

BUILDING DIVISION

05/01/2024

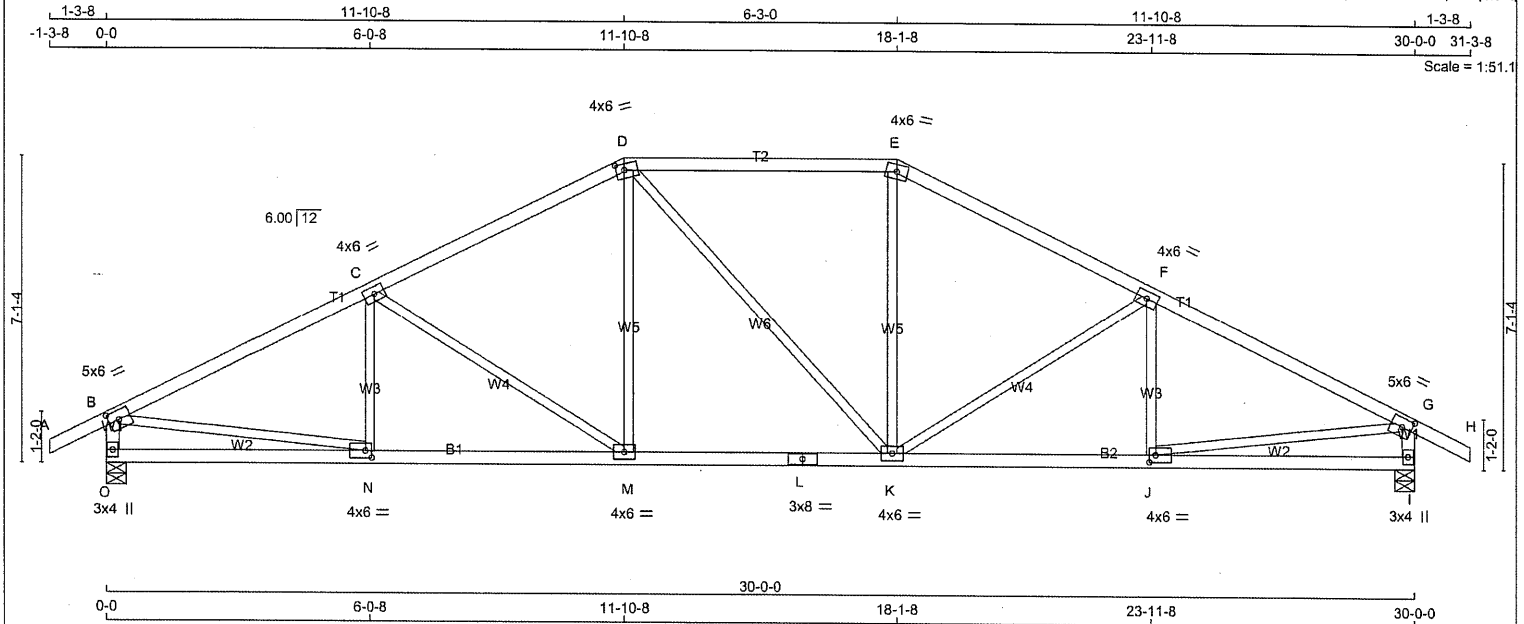
RECEIVED

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T4	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTWW-m	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1-p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0	2.00	1.75
K	BMVW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	6.0	2.00	1.75
O	BMV1-p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
O	1778	0	1778	0
I	1778	0	1778	0

UNFACTORED REACTIONS

	1ST LCASE	MAX / MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0
I	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	N-C	-191 / 33	0.05 (1)
B-C	-2395 / 0	-91.8	-91.8 0.53 (1)	3.93	C-M	-503 / 0	0.48 (1)
C-D	-1983 / 0	-91.8	-91.8 0.48 (1)	4.30	M-D	0 / 386	0.09 (1)
D-E	-1753 / 0	-91.8	-91.8 0.51 (1)	4.44	D-K	0 / 0	0.00 (1)
E-F	-1983 / 0	-91.8	-91.8 0.48 (1)	4.30	K-E	0 / 387	0.09 (1)
F-G	-2394 / 0	-91.8	-91.8 0.53 (1)	3.93	K-F	-502 / 0	0.48 (1)
G-H	0 / 28	-91.8	-91.8 0.12 (1)	10.00	J-F	-191 / 33	0.05 (1)
O-B	-1730 / 0	0.0	0.0 0.17 (1)	6.33	B-N	0 / 2189	0.49 (1)
I-G	-1730 / 0	0.0	0.0 0.17 (1)	6.33	J-G	0 / 2189	0.49 (1)
O-N	0 / 0	-18.5	-18.5 0.15 (4)	10.00			
N-M	0 / 2167	-18.5	-18.5 0.42 (1)	10.00			
M-L	0 / 1752	-18.5	-18.5 0.36 (1)	10.00			
L-K	0 / 1752	-18.5	-18.5 0.36 (1)	10.00			
K-J	0 / 2167	-18.5	-18.5 0.42 (1)	10.00			
J-I	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC2018, NBC2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.19")

CSI: TC=0.53/1.00 (B-C:1), BC=0.42/1.00 (J-K:1), WB=0.49/1.00 (B-N:1), SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080885

CITY OF RICHMOND HILL
BUILDING DIVISION

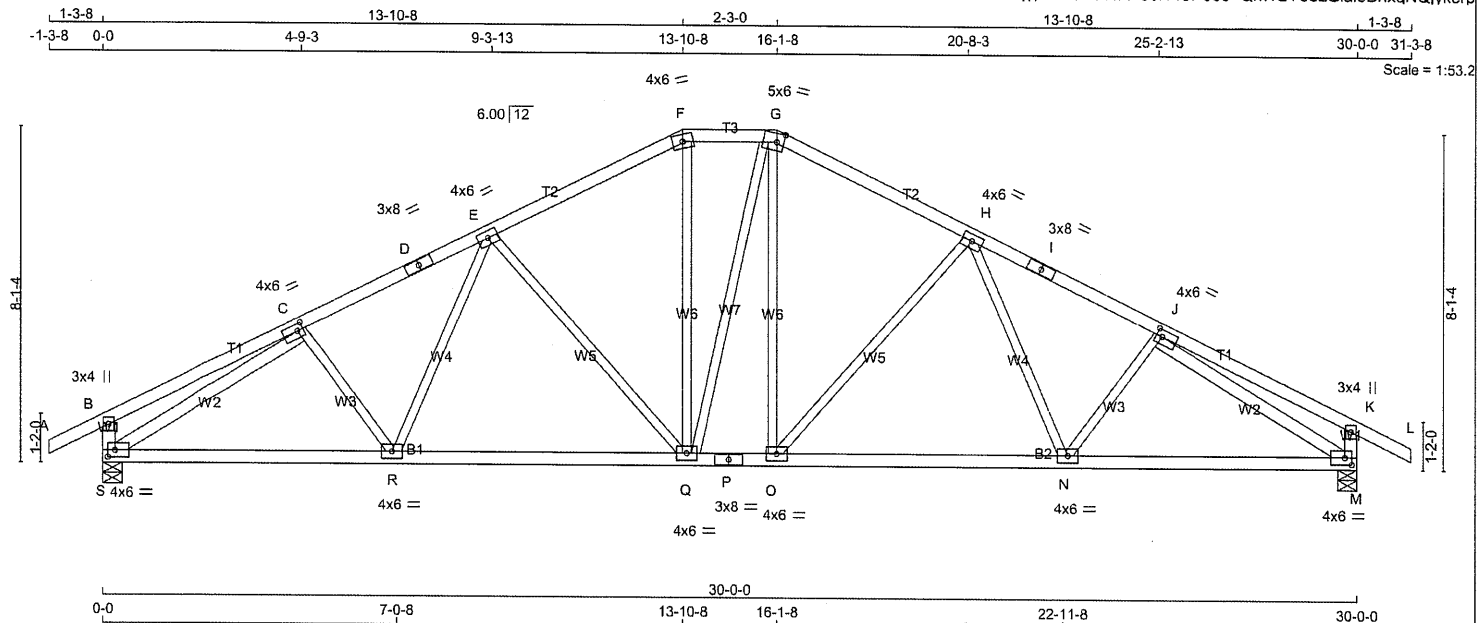
05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	T5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LpDGMu0HxVz8EP6?EUU9qyp?I7-JZ6YrX ebi449P6e5 Qm1D70ezGlaloBnxqNQiykerp



TOTAL WEIGHT = 134 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
S - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
S - C	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0	2.00	1.75
D TS-t	MT20	3.0	8.0		
E TMWW-t	MT20	4.0	6.0		
F TTW-m	MT20	4.0	6.0		
G TTWW-m	MT20	5.0	6.0	2.50	2.00
H TMWW-t	MT20	4.0	6.0		
I TS-t	MT20	3.0	8.0		
J TMWW-t	MT20	4.0	6.0	2.00	1.75
K TMV+p	MT20	3.0	4.0		
M BMVV-t	MT20	4.0	6.0	2.00	2.00
N, O, R					
P BMWW-t	MT20	4.0	6.0		
N BS-t	MT20	3.0	8.0		
Q BMWW-t	MT20	4.0	6.0		
S BMVV-t	MT20	4.0	6.0	2.00	2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
S	1778	0	1778	0	0	5-8	1-15	1-15	1-15
M	1778	0	1778	0	0	5-8	1-15	1-15	1-15

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN.	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
S	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0
M	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO	PLF	CSI (LC)	FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	C-R	-65 / 41	0.02 (1)
B-C	0 / 16	-91.8	-91.8	0.23 (1)	R-E	0 / 212	0.05 (4)
C-D	-2281 / 0	-91.8	-91.8	0.27 (1)	E-Q	-579 / 0	0.60 (1)
D-E	-2281 / 0	-91.8	-91.8	0.27 (1)	Q-F	0 / 517	0.12 (1)
E-F	-1787 / 0	-91.8	-91.8	0.26 (1)	Q-G	0 / 8	0.00 (1)
F-G	-1588 / 0	-91.8	-91.8	0.09 (1)	O-G	0 / 509	0.11 (1)
G-H	-1785 / 0	-91.8	-91.8	0.26 (1)	O-H	-582 / 0	0.60 (1)
H-I	-2282 / 0	-91.8	-91.8	0.27 (1)	H-N	0 / 216	0.05 (4)
I-J	-2282 / 0	-91.8	-91.8	0.27 (1)	N-J	-65 / 41	0.02 (1)
J-K	0 / 16	-91.8	-91.8	0.23 (1)	S-C	-2521 / 0	0.95 (1)
K-L	0 / 28	-91.8	-91.8	0.12 (1)	J-M	-2522 / 0	0.95 (1)
S-B	-301 / 0	0.0	0.0	0.03 (1)			
M-K	-301 / 0	0.0	0.0	0.03 (1)			
S-R	0 / 2080	-18.5	-18.5	0.44 (1)			
R-Q	0 / 1961	-18.5	-18.5	0.42 (1)			
Q-P	0 / 1586	-18.5	-18.5	0.33 (1)			
P-O	0 / 1586	-18.5	-18.5	0.33 (1)			
O-N	0 / 1961	-18.5	-18.5	0.43 (1)			
N-M	0 / 2081	-18.5	-18.5	0.45 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.27/1.00 (H-J:1), BC=0.45/1.00 (M-N:1), WB=0.95/1.00 (J-M:1), SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080886

CITY OF RICHMOND HILL
BUILDING DIVISION

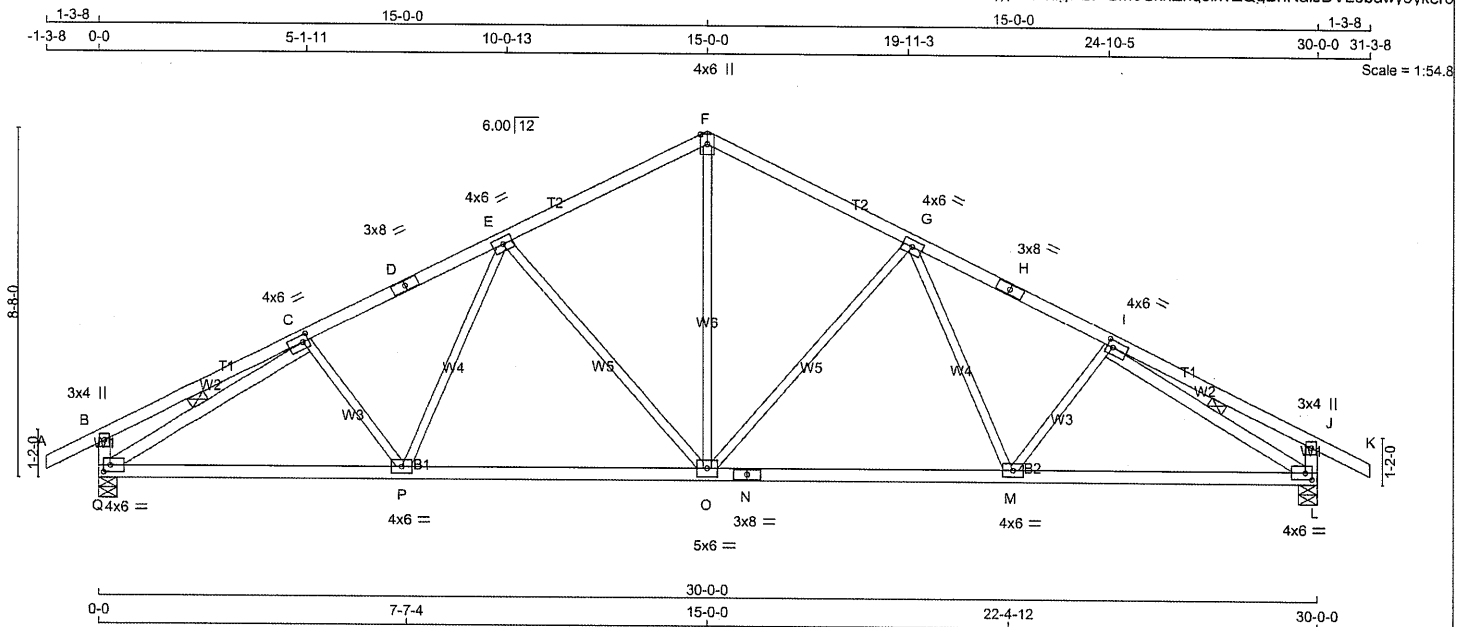
05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	T6	14	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 14 X 126 = 1757 lb [M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
Q - C	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TMVW-t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMVW-t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	4.0	6.0	2.00	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	6.0	2.00	2.00
M	BMVW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW1-t	MT20	4.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	1778	0	1778	0
L	1778	0	1778	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
Q	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0	
L	1255	837 / 0	0 / 0	0 / 0	0 / 0	418 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, H-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. LENGTH	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00	O-F	0 / 1119	0.25 (1)	
B-C	0 / 18	-91.8 -91.8 0.28 (1)	10.00	O-G	-650 / 0	0.79 (1)	
C-D	-2260 / 0	-91.8 -91.8 0.31 (1)	4.28	G-M	0 / 274	0.06 (1)	
D-E	-2260 / 0	-91.8 -91.8 0.31 (1)	4.28	M-I	-125 / 26	0.04 (1)	
E-F	-1689 / 0	-91.8 -91.8 0.30 (1)	4.82	E-O	-650 / 0	0.79 (1)	
F-G	-1689 / 0	-91.8 -91.8 0.30 (1)	4.82	P-E	0 / 274	0.06 (1)	
G-H	-2260 / 0	-91.8 -91.8 0.31 (1)	4.28	C-P	-125 / 26	0.04 (1)	
H-I	-2260 / 0	-91.8 -91.8 0.31 (1)	4.28	Q-C	-2525 / 0	0.47 (1)	
I-J	0 / 18	-91.8 -91.8 0.28 (1)	10.00	I-L	-2525 / 0	0.47 (1)	
J-K	0 / 28	-91.8 -91.8 0.12 (1)	10.00				
Q-B	-314 / 0	0.0 0.0 0.03 (1)	7.81				
L-J	-314 / 0	0.0 0.0 0.03 (1)	7.81				
Q-P	0 / 2097	-18.5 -18.5 0.46 (1)	10.00				
P-O	0 / 1917	-18.5 -18.5 0.43 (1)	10.00				
O-N	0 / 1917	-18.5 -18.5 0.43 (1)	10.00				
N-M	0 / 1917	-18.5 -18.5 0.43 (1)	10.00				
M-L	0 / 2097	-18.5 -18.5 0.46 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
DL	= 8.0 PSF	
BOT CH.	LL	= 0.0 PSF
DL	= 7.4 PSF	
TOTAL LOAD	= 39.0 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.31/1.00 (C-E:1), BC=0.46/1.00 (P-Q:1), WB=0.79/1.00 (E-O:1), SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.59 (N) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080887

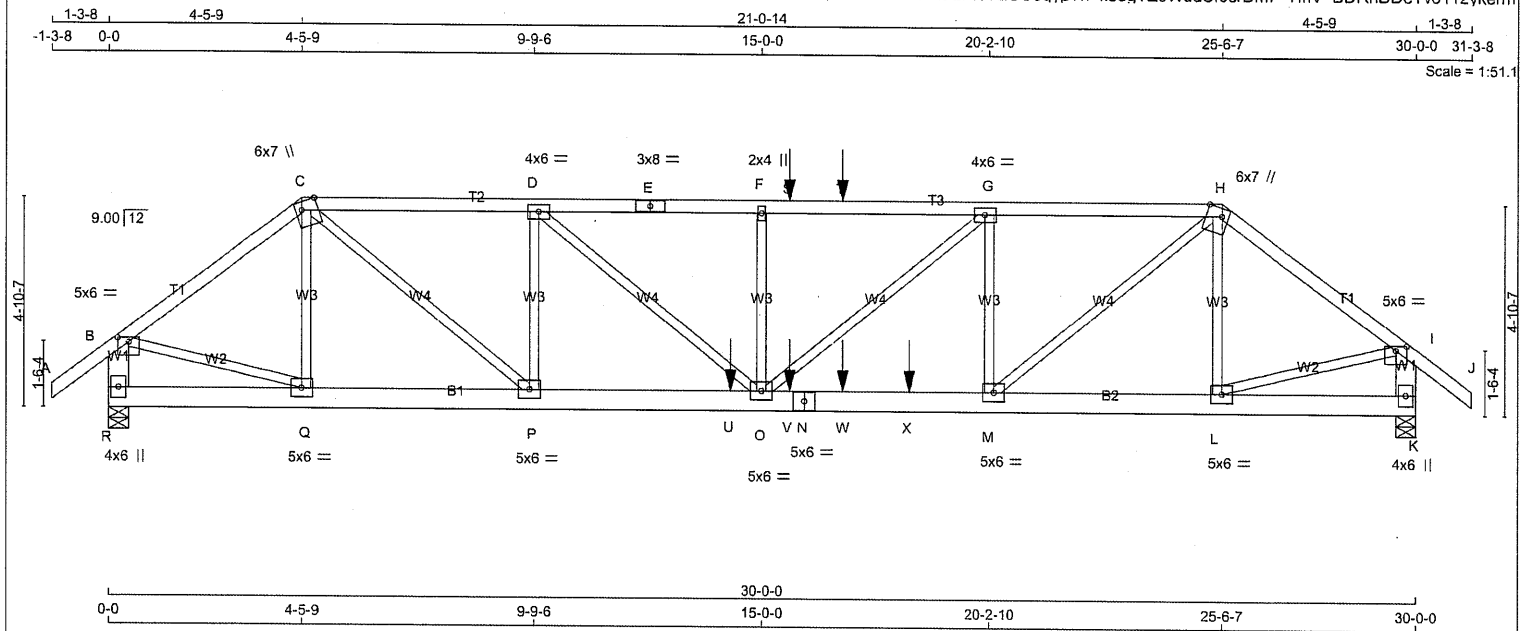
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME 434359	TRUSS NAME T7	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 143 = 286 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - B	2x6	DRY	No.2	SPF	
B - I	2x6	DRY	No.2	SPF	
I - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
E - H 1	12	SIDE(0.0)
H - J 1	12	TOP
J - B 2	12	TOP
B - I 2	12	TOP
I - N 2	12	SIDE(0.0)
N - K 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 2631	DOWN 2836	0	1-8
R	HORZ 0	HORZ 0	0	1-8
K	0	0	0	1-9

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED SNOW LIVE	PERM. LIVE	WIND
R	1858 1236 / 0	0 / 0	0 / 0
K	2001 1339 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 38	-91.8	-91.8	0.07 (1)	Q-C	-442 / 0	0.07 (1)	
B-C	-2849 / 0	-91.8	-91.8	0.22 (1)	C-P	0 / 2703	0.33 (1)	
C-D	-4333 / 0	-91.8	-91.8	0.30 (1)	P-D	-1633 / 0	0.28 (1)	
D-E	-5621 / 0	-91.8	-91.8	0.36 (1)	D-O	0 / 1701	0.21 (1)	
E-F	-5621 / 0	-91.8	-91.8	0.36 (1)	O-F	-585 / 0	0.10 (1)	
F-S	-5621 / 0	-91.8	-91.8	0.40 (1)	S-T	0 / 995	0.12 (1)	
S-T	-5621 / 0	-91.8	-91.8	0.40 (1)	T-G	-1227 / 0	0.21 (1)	
T-G	-5621 / 0	-91.8	-91.8	0.40 (1)	G-H	0 / 3161	0.39 (1)	
G-H	-4867 / 0	-91.8	-91.8	0.36 (1)	H-I	-591 / 0	0.10 (1)	
H-I	-3083 / 0	-91.8	-91.8	0.23 (1)	I-J	0 / 2341	0.29 (1)	
I-J	0 / 38	-91.8	-91.8	0.07 (1)	J-K	0 / 2533	0.31 (1)	
J-K	-2599 / 0	0.0	0.0	0.09 (1)				
K-I	-2765 / 0	0.0	0.0	0.10 (1)				

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
R-Q	0 / 0	-18.5	-18.5	0.03 (4)	Q-P	-18.5	-18.5	0.16 (1)
Q-P	0 / 2264	-18.5	-18.5	0.43 (1)	P-U	0 / 4333	-18.5	0.43 (1)
P-U	0 / 4333	-18.5	-18.5	0.43 (1)	U-O	0 / 4333	-18.5	0.43 (1)
U-O	0 / 4333	-18.5	-18.5	0.43 (1)	O-V	0 / 4867	-18.5	0.43 (1)
O-V	0 / 4867	-18.5	-18.5	0.43 (1)	V-N	0 / 4867	-18.5	0.43 (1)
V-N	0 / 4867	-18.5	-18.5	0.43 (1)	N-W	0 / 4867	-18.5	0.43 (1)
N-W	0 / 4867	-18.5	-18.5	0.43 (1)	W-X	0 / 4867	-18.5	0.43 (1)
W-X	0 / 4867	-18.5	-18.5	0.43 (1)	X-M	0 / 4867	-18.5	0.43 (1)
X-M	0 / 4867	-18.5	-18.5	0.43 (1)	M-L	0 / 2448	-18.5	0.29 (1)
M-L	0 / 2448	-18.5	-18.5	0.29 (1)	L-K	0 / 0	-18.5	0.03 (1)
L-K	0 / 0	-18.5	-18.5	0.03 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	15-7-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
T	16-10-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
U	14-3-8	-508	-508	---	FRONT	VERT	TOTAL	---	C1
V	15-7-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
W	16-10-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
X	18-4-8	-691	-691	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.40/1.00 (F-G:1), BC=0.64/1.00 (M-O:1), WB=0.39/1.00 (H-M:1), SSI=0.25/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (O) (INPUT = 0.90)
JSI METAL= 0.42 (N) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080888

CITY OF RICHMOND HILL
BUILDING DIVISION

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T7	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:37 2023 Page 2
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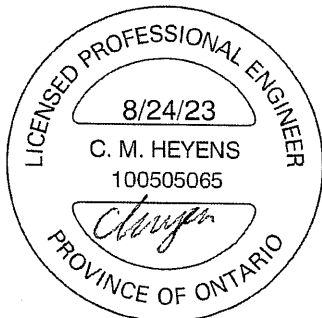
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	7.0	Edge	4.25
D	TMWV-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWV-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	7.0	Edge	4.25
I	TMWV-p	MT20	5.0	6.0	1.25	3.00
K	BMV1+p	MT20	4.0	6.0		
L, M, P, Q						
L	BMWV-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWV-t	MT20	5.0	6.0		
R	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

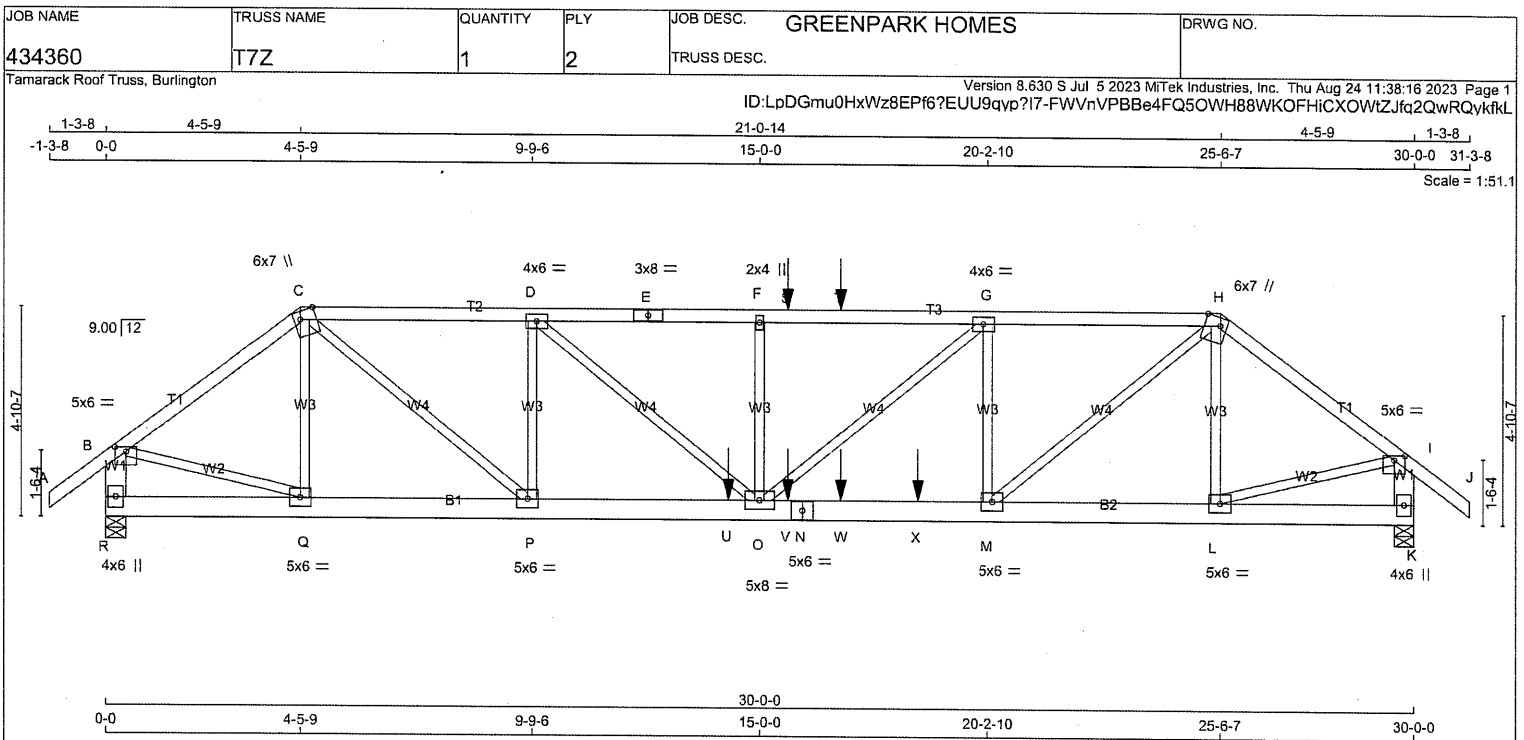


STRUCTURAL COMPONENT ONLY
DWG # TR23080888

CITY OF RICHMOND HILL
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
R - N	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - E 1	12	TOP
E - H 1	12	TOP
H - J 1	12	TOP
R - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R - N 2	12	SIDE(0.0)
N - K 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED DOWN	MAXIMUM FACTORED UP	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT 2831	HORZ 0	0	5-8	1-9
R	3022	0	0	5-8	1-10

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	0/0	0/0	0/0	0/0
R	1996	1347 / 0	0/0	0/0	648 / 0	0/0
K	2130	1438 / 0	0/0	0/0	692 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 38	-91.8	-91.8	0.07 (1)	10.00	Q-C	-507 / 0	0.09 (1)
B-C	-3097 / 0	-91.8	-91.8	0.23 (1)	5.04	C-P	0 / 3033	0.38 (1)
C-D	-4781 / 0	-91.8	-91.8	0.32 (1)	4.15	P-D	-1769 / 0	0.30 (1)
D-E	-8230 / 0	-91.8	-91.8	0.40 (1)	3.64	D-O	0 / 1912	0.24 (1)
E-F	-8230 / 0	-91.8	-91.8	0.40 (1)	3.64	O-F	-583 / 0	0.10 (1)
F-S	-8230 / 0	-91.8	-91.8	0.43 (1)	3.59	O-G	0 / 1282	0.16 (1)
S-T	-8230 / 0	-91.8	-91.8	0.43 (1)	3.59	M-G	-1413 / 0	0.24 (1)
T-G	-8230 / 0	-91.8	-91.8	0.43 (1)	3.59	M-H	0 / 3425	0.42 (1)
G-H	-5259 / 0	-91.8	-91.8	0.38 (1)	3.92	L-H	-619 / 0	0.10 (1)
H-I	-3321 / 0	-91.8	-91.8	0.24 (1)	4.90	B-Q	0 / 2544	0.31 (1)
I-J	0 / 38	-91.8	-91.8	0.07 (1)	10.00	L-I	0 / 2729	0.34 (1)
R-B	-2796 / 0	0.0	0.0	0.10 (1)	7.81			
K-I	-2975 / 0	0.0	0.0	0.11 (1)	7.81			
R-Q	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
Q-P	0 / 2461	-18.5	-18.5	0.18 (1)	10.00			
P-U	0 / 4782	-18.5	-18.5	0.47 (1)	10.00			
U-O	0 / 4782	-18.5	-18.5	0.47 (1)	10.00			
O-V	0 / 5259	-18.5	-18.5	0.63 (1)	10.00			
V-N	0 / 5259	-18.5	-18.5	0.63 (1)	10.00			
N-W	0 / 5259	-18.5	-18.5	0.63 (1)	10.00			
W-X	0 / 5259	-18.5	-18.5	0.63 (1)	10.00			
X-M	0 / 5259	-18.5	-18.5	0.63 (1)	10.00			
M-L	0 / 2637	-18.5	-18.5	0.28 (1)	10.00			
L-K	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	15-7-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
T	16-10-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
U	14-3-8	-802	-802	---	FRONT	VERT	TOTAL	---	C1
V	15-7-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
W	16-10-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
X	18-7-8	-664	-664	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.43/1.00 (F-G:1) . BC=0.63/1.00 (M-O:1) .

WB=0.42/1.00 (H-M:1) . SSI=0.37/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)

JSI METAL= 0.56 (N) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080899

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BUILDING DIVISION
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JOB NAME 434360	TRUSS NAME T7Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Jul 5 2023 MTek Industries, Inc. Thu Aug 24 11:38:16 2023 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	7.0	Edge	4.25
D	TMWV-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWV-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	7.0	Edge	4.25
I	TMWV-p	MT20	5.0	6.0	1.25	3.00
K	BMV1+p	MT20	4.0	6.0		
L, M, P, Q						
L	BMWV-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWV-t	MT20	5.0	8.0		
R	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080899

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

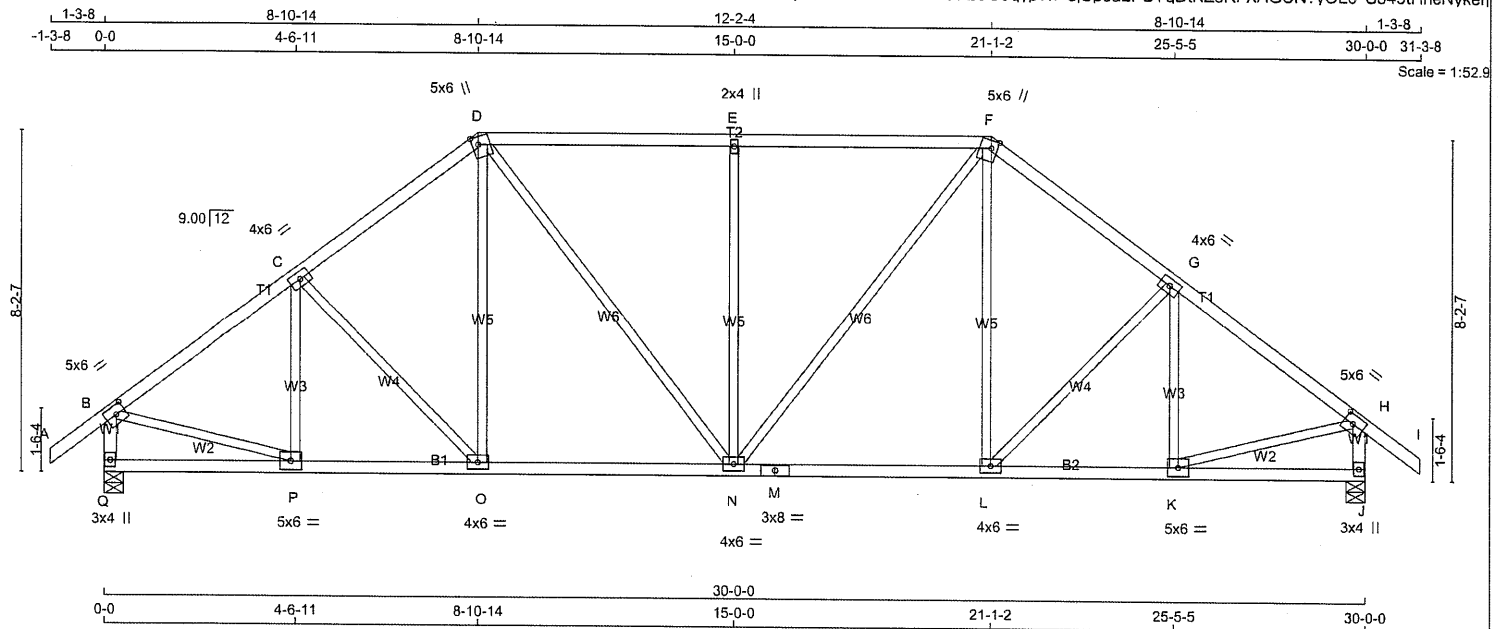
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Per: joshua.nabua

STRUCTURAL COMPONENT ONLY
DWG # TR23080889

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	T9	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 138 = 415 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.75
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
Q	1780	0	1780	0
J	1780	0	1780	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN	COMPONENT REACTIONS				
Q	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1257	838 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
J	1257	838 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MAX. FACTORED	MEMB.	MAX. FACTORED	FACTORED
	FORCE (LBS)	VERT. (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH		FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 38	-91.8	-91.8	0.12 (1)	10.00	P-C	-293 / 0
B-C	-1768 / 0	-91.8	-91.8	0.27 (1)	4.77	C-O	-175 / 0
C-D	-1678 / 0	-91.8	-91.8	0.27 (1)	4.87	O-D	0 / 226
D-E	-1627 / 0	-91.8	-91.8	0.48 (1)	4.63	D-N	0 / 504
E-F	-1627 / 0	-91.8	-91.8	0.48 (1)	4.63	N-E	-685 / 0
F-G	-1678 / 0	-91.8	-91.8	0.27 (1)	4.87	N-F	0 / 504
G-H	-1768 / 0	-91.8	-91.8	0.27 (1)	4.77	L-F	0 / 226
H-I	0 / 38	-91.8	-91.8	0.12 (1)	10.00	L-G	-175 / 0
Q-B	-1743 / 0	0.0	0.0	0.18 (1)	6.31	K-G	-293 / 0
J-H	-1743 / 0	0.0	0.0	0.18 (1)	6.31	B-P	0 / 1486
						K-H	0 / 1486
Q-P	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
P-O	0 / 1437	-18.5	-18.5	0.29 (1)	10.00		
O-N	0 / 1319	-18.5	-18.5	0.28 (1)	10.00		
N-M	0 / 1319	-18.5	-18.5	0.28 (1)	10.00		
M-L	0 / 1319	-18.5	-18.5	0.28 (1)	10.00		
L-K	0 / 1437	-18.5	-18.5	0.29 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.48/1.00 (D-E:1) . BC=0.29/1.00 (O-P:1) .
WB=0.90/1.00 (E-N:1) . SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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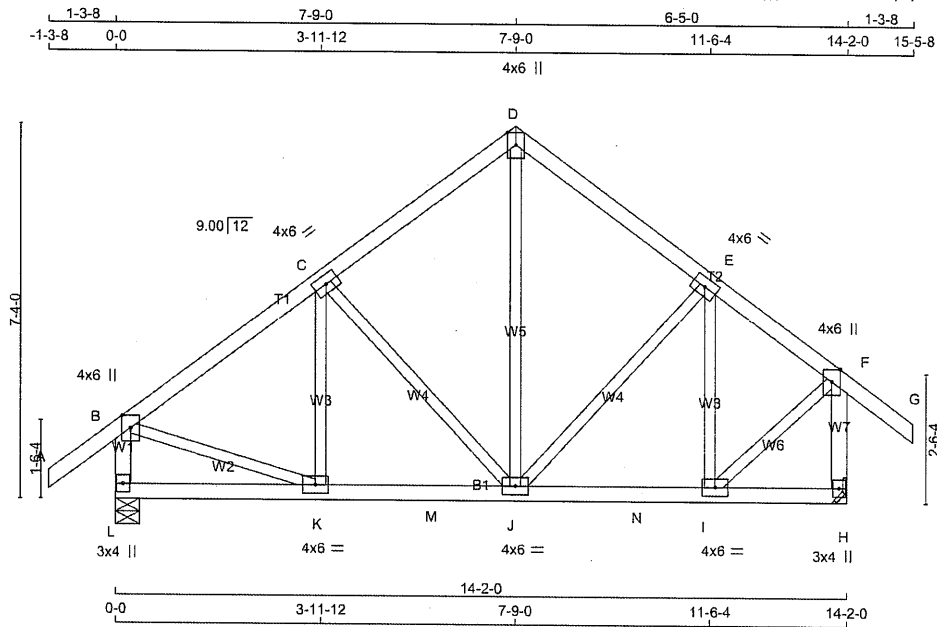
Per: joshua.nabua



STRUCTURAL COMPONENT ONLY
DWG # TR23080890

JOB NAME 434359	TRUSS NAME T10	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:43.2

TOTAL WEIGHT = 4 X 70 = 281 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
L	1189	0	1189	0
H	1056	0	1056	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
L	865	433/0	0/0	0/0	432/0	0/0
H	758	433/0	0/0	0/0	325/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)		LENGTH		(LBS)	CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 38	-91.8	-91.8	0.14 (1)	10.00	K-C	0 / 218	0.08 (4)
B-C	-981 / 0	-91.8	-91.8	0.27 (1)	5.92	C-J	-409 / 0	0.22 (1)
C-D	-688 / 0	-91.8	-91.8	0.25 (1)	6.25	J-D	0 / 523	0.14 (4)
D-E	-681 / 0	-91.8	-91.8	0.19 (1)	6.25	J-E	0 / 57	0.02 (4)
E-F	-632 / 0	-91.8	-91.8	0.18 (1)	6.25	I-E	-375 / 0	0.12 (1)
F-G	0 / 38	-91.8	-91.8	0.14 (1)	10.00	B-K	0 / 839	0.21 (1)
L-B	-1113 / 0	0.0	0.0	0.13 (1)	7.48	I-F	0 / 685	0.17 (1)
H-F	-1035 / 0	0.0	0.0	0.15 (1)	7.69			
L-K	0 / 0	-38.5	-75.6	0.27 (4)	10.00			
K-M	0 / 804	-75.6	-96.0	0.39 (4)	10.00			
M-J	0 / 804	-64.7	-64.7	0.39 (4)	10.00			
J-N	0 / 527	-64.7	-18.5	0.24 (4)	10.00			
N-I	0 / 527	-18.5	-18.5	0.24 (4)	10.00			
I-H	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.27/1.00 (B-C:1), BC=0.39/1.00 (J-K:4), WB=0.22/1.00 (C-J:1), SSI=0.24/1.00 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.80 (B) (INPUT= 0.90)
JSI METAL= 0.51 (B) (INPUT= 0.95)



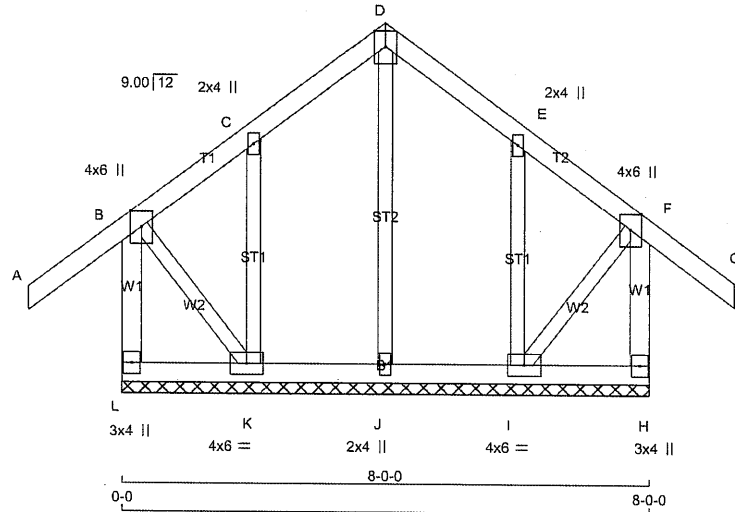
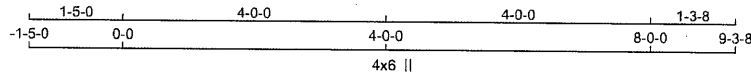
STRUCTURAL COMPONENT ONLY
DWG # TR23080891

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T11G	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 43 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW1-l	MT20	4.0	6.0		
J	BMVW1+w	MT20	2.0	4.0		
K	BMVW1-l	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
L-B	-260 / 0	0.0	0.0 0.03 (1)	J-D	-181 / 0	0.08 (1)	
A-B	0 / 41	-91.8	-91.8 0.15 (1)	K-C	-149 / 0	0.04 (1)	
B-C	-20 / 0	-91.8	-91.8 0.14 (1)	I-E	-161 / 0	0.04 (1)	
C-D	0 / 4	-91.8	-91.8 0.05 (1)	B-K	-7 / 0	0.00 (1)	
D-E	0 / 3	-91.8	-91.8 0.05 (1)	I-F	-7 / 0	0.00 (1)	
E-F	-14 / 0	-91.8	-91.8 0.12 (1)				
F-G	0 / 38	-91.8	-91.8 0.12 (1)				
H-F	-238 / 0	0.0	0.0 0.03 (1)				
L-K	0 / 0	-18.5	-18.5 0.02 (4)				
K-J	-8 / 0	-18.5	-18.5 0.02 (4)				
J-I	-8 / 0	-18.5	-18.5 0.02 (4)				
I-H	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
LL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (I-J:4), WB=0.08/1.00 (D-J:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (F) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080892

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

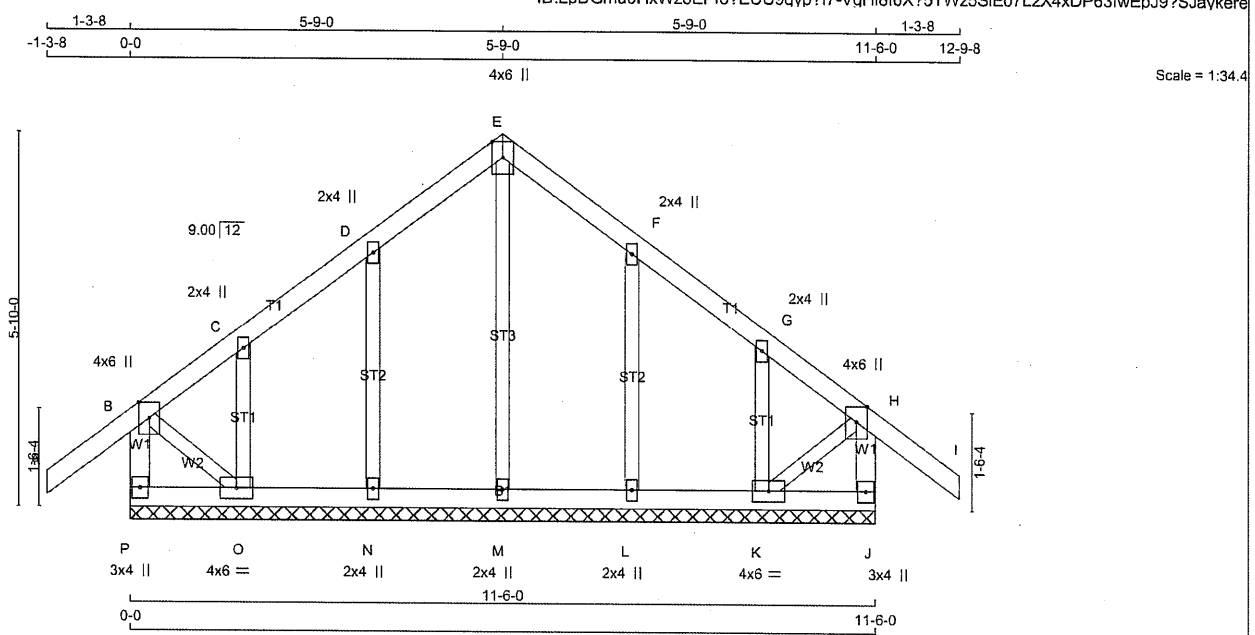
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Per: joshua.nabua

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T12G	1	1	GREENPARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				TOP CH. LL = 25.6 PSF			
P - B	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				DL = 6.0 PSF			
A - E	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				BOT CH. LL = 0.0 PSF			
E - I	2x4	DRY	No.2	BRACING				DL = 7.4 PSF			
J - H	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				TOTAL LOAD = 39.0 PSF			
P - J	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				SPACING = 24.0 IN. C/C			
ALL WEBS	2x3	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
ALL GABLE WEBS	2x3	DRY	No.2	LOADING				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				TOTAL LOAD CASES: (4)				- PART 9 OF CBC 2018, NBC-2019AE			
GABLE STUDS SPACED AT 2-0-0 OC.								- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 086-14			
								- TPIC 2014			
								DESIGN ASSUMPTIONS			
								- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
								(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
								CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (N-O:4) ,			
								WB=0.07/1.00 (E-M:1) , SSI=0.08/1.00 (A-B:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS= 1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.16 (B) (INPUT = 0.90)			
								JSI METAL= 0.12 (F) (INPUT = 0.95)			



CITY OF RICHMOND HILL
BUILDING DIVISION

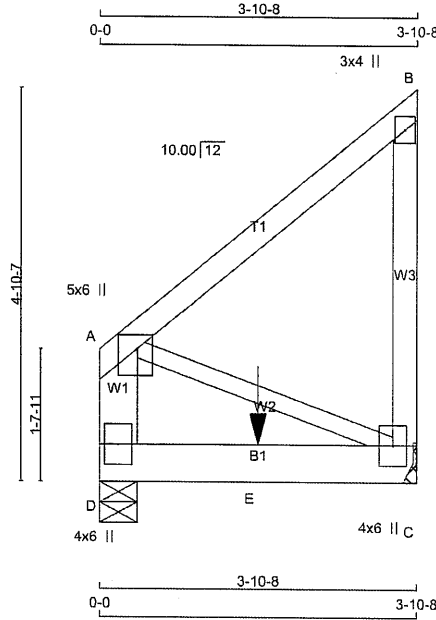
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Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T13	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:27.3

TOTAL WEIGHT = 2 X 22 = 45 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
D - A	2x6	DRY No.2	SPF	
A - B	2x4	DRY No.2	SPF	
C - B	2x4	DRY No.2	SPF	
D - C	2x6	DRY No.2	SPF	
ALL WEBS	2x3	DRY No.2	SPF	
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 2	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMW+p	MT20	5.0	6.0	2.00	2.25
B TMV+p	MT20	3.0	4.0		
C BMW1+p	MT20	4.0	6.0		
D BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	732	0	732	0	0
C	732	0	732	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	522	316 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0
C	523	316 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
D-A	-178 / 0	0.0	0.0	0.01 (1)	7.81	A-C	0 / 0
A-B	0 / 0	-91.8	-91.8	0.12 (1)	10.00		
C-B	-178 / 0	0.0	0.0	0.03 (1)	7.81		
D-E	0 / 0	-18.5	-18.5	0.35 (1)	10.00		
E-C	0 / 0	-18.5	-18.5	0.35 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	-743	-743		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.12/1.00 (A-B:1), BC=0.35/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080895

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	T13	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080895

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

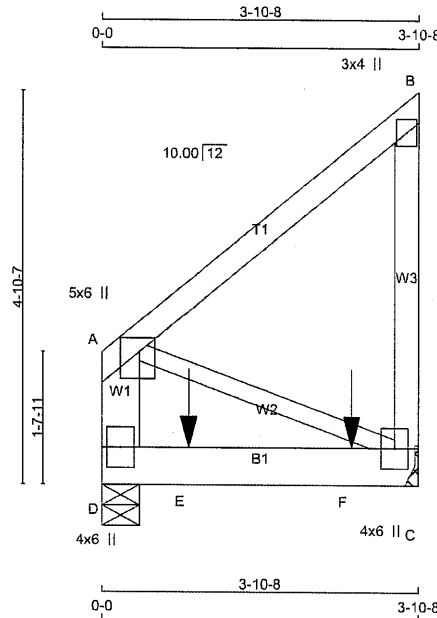
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	T13Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:48 2023 Page 1

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Scale = 1:27.3

TOTAL WEIGHT = 2 X 22 = 45 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 2	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	5.0	6.0	2.00	2.25
B TMV+p	MT20	3.0	4.0		
C BMVW1+p	MT20	4.0	6.0		
D BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	UPLIFT	IN-SX
D 908	0	908	1-8
C 1005	0	1005	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED						
D 639	440 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
C 706	487 / 0	0 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
D-A	-178 / 0	0.0 0.0 0.01 (1)	A-C	0 / 0
A-B	0 / 0	-91.8 -91.8 0.13 (1)		0.00 (1)
C-B	-178 / 0	0.0 0.0 0.03 (1)		
D-E	0 / 0	-18.5 -18.5 0.28 (1)		10.00
E-F	0 / 0	-18.5 -18.5 0.28 (1)		10.00
F-C	0 / 0	-18.5 -18.5 0.28 (1)		10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-521	-521		FRONT	VERT	TOTAL	---	C1
F	3-0-12	-522	-522		FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.28/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.04 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080896

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	T13Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:48 2023 Page 2
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NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



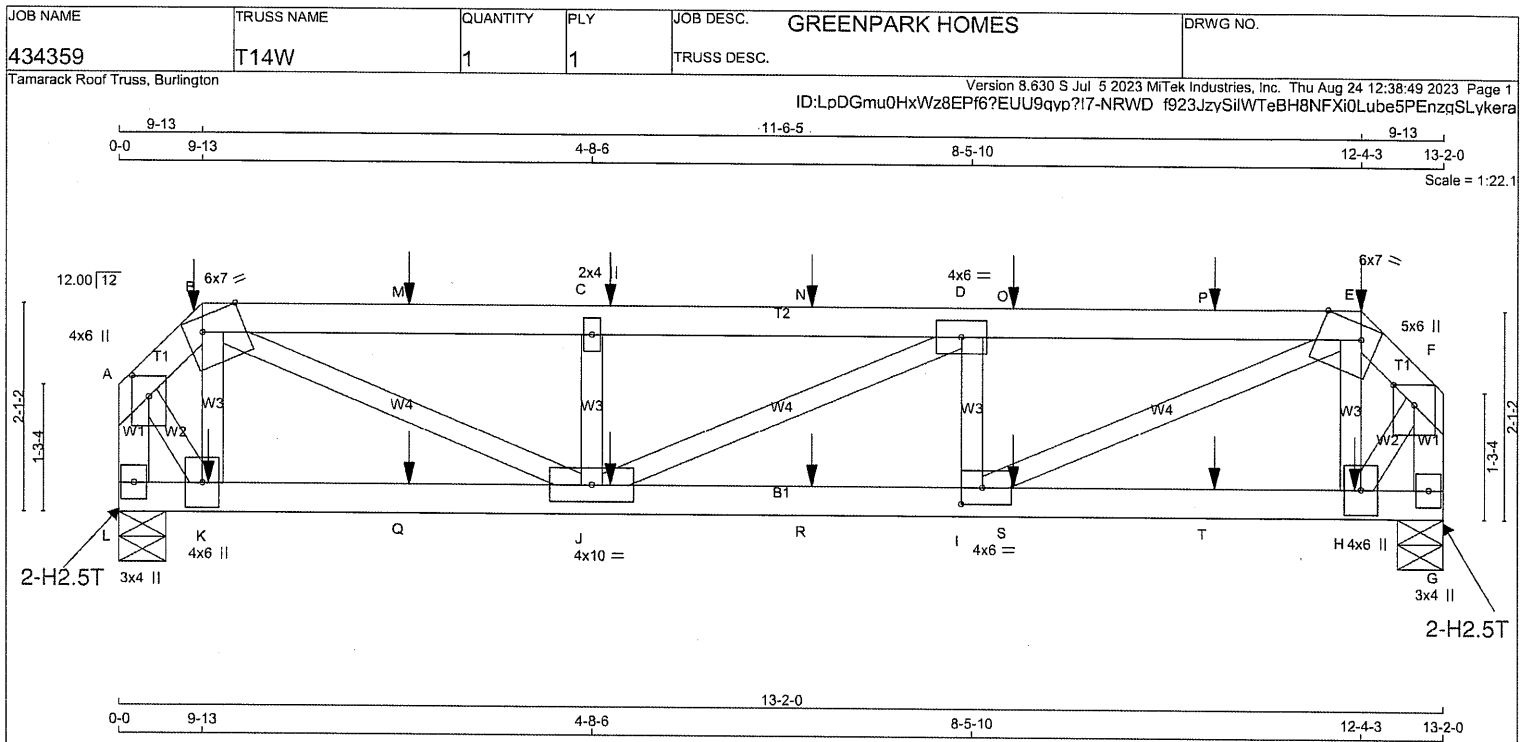
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BUILDING DIVISION

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	T14W	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	2-10-9	-45	-45	55	FRONT	VERT	TOTAL	---	C1
R	6-10-9	-45	-45	55	FRONT	VERT	TOTAL	---	C1
S	8-10-9	-45	-45	55	FRONT	VERT	TOTAL	---	C1
T	10-10-9	-45	-45	55	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.



STRUCTURAL COMPONENT ONLY
DWG # TR23080897

CITY OF RICHMOND HILL
BUILDING DIVISION

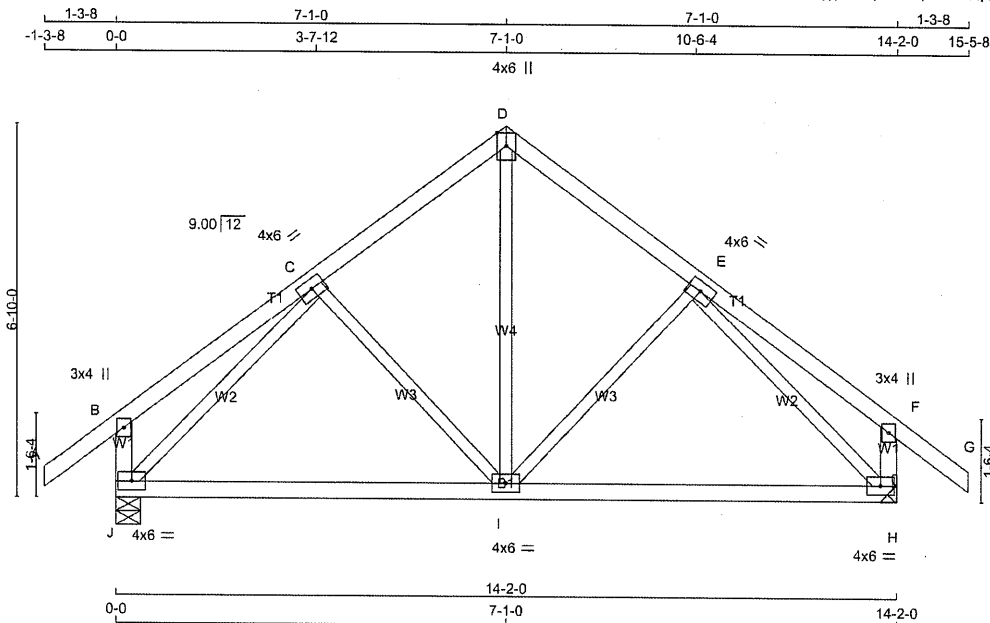
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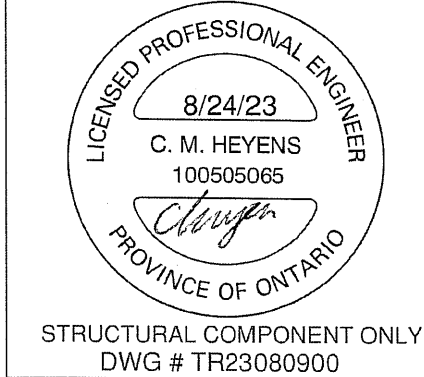
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434360	T20	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 11:38:17 2023 Page 1
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH.	LL =	25.6	PSF
A - D	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	DL	=	6.0	PSF
D - G	2x4	DRY	No.2	JT VERT	HORZ	DOWN	HORZ	BOT CH.	LL =	0.0	PSF
J - B	2x4	DRY	No.2	J 908	0	908	0	DL	=	7.4	PSF
H - F	2x4	DRY	No.2	SPF				TOTAL LOAD	=	39.0	PSF
J - H	2x4	DRY	No.2	SPF				SPACING = 24.0 IN. C/C			
ALL WEBS	2x3	DRY	No.2	A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015			
EXCEPT								THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.								- PART 9 OF CBC2018, NBC-2019AE			
								- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 086-14			
								- TPIC 2014			
								(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
								ALLOWABLE DEFL.(LL)= L/360 (0.47")			
								CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")			
								ALLOWABLE DEFL.(TL)= L/360 (0.47")			
								CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")			
								CSI: TC=0.18/1.00 (E-F:1) , BC=0.31/1.00 (H-I:4) ,			
								WB=0.36/1.00 (C-J:1) , SSI=0.13/1.00 (C-D:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.47 (E) (INPUT = 0.90)			
								JSI METAL= 0.18 (C) (INPUT = 0.95)			

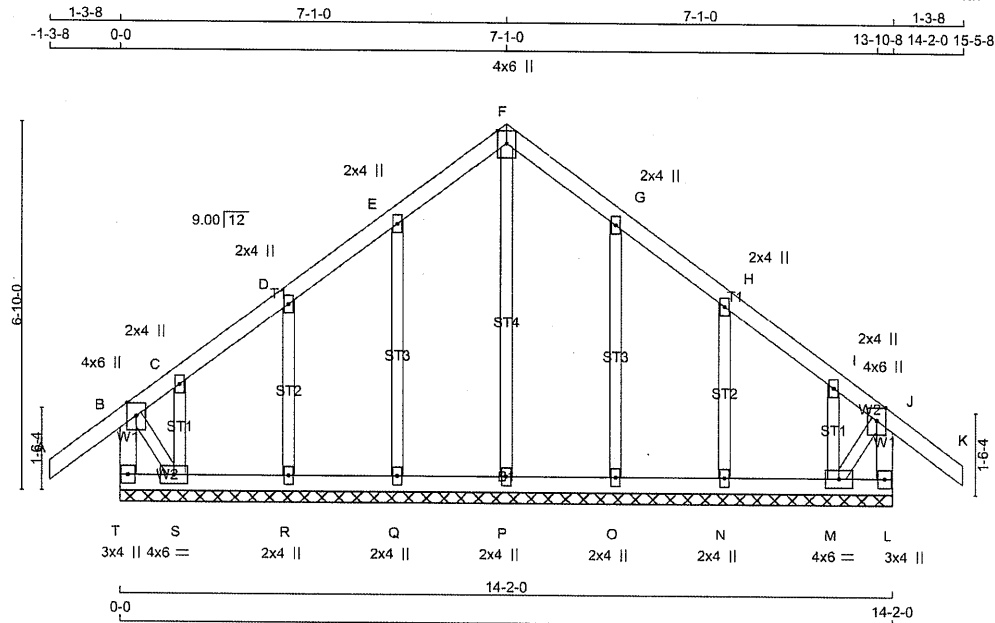


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434360	T20G	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 11:38:18 2023 Page 1
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Scale = 1:40.9

TOTAL WEIGHT = 66 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				TOP CH. LL = 25.8 PSF			
T - B	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				DL = 8.0 PSF			
A - F	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				BOT CH. LL = 0.0 PSF			
F - K	2x4	DRY	No.2	BRACING				DL = 7.4 PSF			
L - J	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				TOTAL LOAD = 39.0 PSF			
T - L	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				SPACING = 24.0 IN. C/C			
ALL WEBS	2x3	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
ALL GABLE WEBS	2x3	DRY	No.2	LOADING				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				TOTAL LOAD CASES: (4)				- PART 9 OF CBC 2018, NBC-2019AE			
GABLE STUDS SPACED AT 2-0-0 OC.				CHORDS				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
				MEMB. MAX. FACTORED FORCE (LBS)				- CSA 086-14			
				VERT. LOAD LC1 MAX (PLF)				- TPIC 2014			
				FROM TO				DESIGN ASSUMPTIONS			
				CSI (LC)				- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
				UNBRAC LENGTH				(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
				FR-TO				CSI: TC=0.12/1.00 (J-K:1), BC=0.02/1.00 (R-S:4), WB=0.11/1.00 (F-P:1), SSI=0.08/1.00 (J-K:1)			
				T-B				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				A-B				COMPANION LIVE LOAD FACTOR = 1.00			
				B-C				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				C-D				NAIL VALUES			
				D-E				PLATE GRIP(DRY) SHEAR SECTION			
				E-F				(PSI) (PLI) (PLI)			
				F-G				MAX MIN MAX MIN MAX MIN			
				G-H				MT20 650 371 1747 788 1987 1873			
				H-I				PLATE PLACEMENT TOL. = 0.250 inches			
				I-J				PLATE ROTATION TOL. = 5.0 Deg.			
				J-K				JSI GRIP= 0.16 (J) (INPUT = 0.90)			
				K-L				JSI METAL= 0.11 (E) (INPUT = 0.95)			
				L-J							
				T-S							
				S-R							
				R-Q							
				Q-P							
				P-O							
				O-N							
				N-M							
				M-L							

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	6.0	Edge	
J TMVW+p	MT20	4.0	6.0	Edge	
L BMV1+p	MT20	3.0	4.0		
M BMVW1-t	MT20	4.0	6.0		
N, O, P, Q, R					
N BMV1+w	MT20	2.0	4.0		
S BMVW1-t	MT20	4.0	6.0		
T BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080901

CITY OF RICHMOND HILL
BUILDING DIVISION

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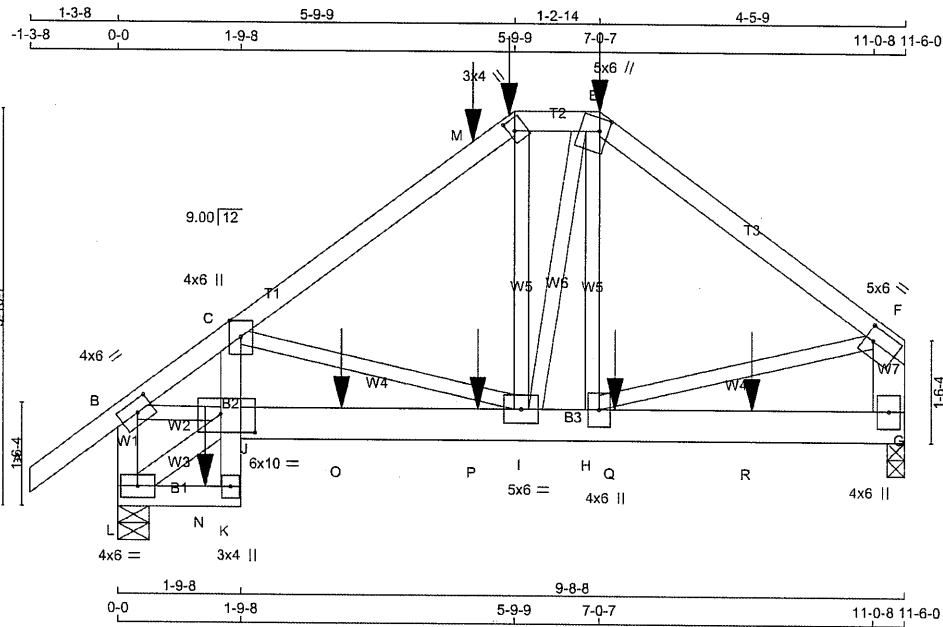
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434360	T21	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:32.6

TOTAL WEIGHT = 63 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x6	DRY	No.2
L - K	2x4	DRY	No.2
K - C	2x4	DRY	No.2
J - G	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
L - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.75
C	TMVW+p	MT20	4.0	6.0	Edge	2.00
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TTW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-t	MT20	5.0	6.0	2.50	1.50
G	BMV1+p	MT20	4.0	6.0		
H	BMVW+t	MT20	4.0	6.0		
I	BMVW+t	MT20	5.0	6.0		
J	BMVW-t	MT20	6.0	10.0	3.25	6.00
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1076	0	1076	0	0	3-0	1-8
L	1250	0	1250	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM.LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
G	757	517 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0				
L	877	614 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC1)	MEMB.	FORCE (LBS)	MAX. (LC1)	MAX. (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 38	-91.8	-91.8 0.14 (1)	10.00	C-I	-487 / 0	0.17 (1)
B-C	-1574 / 0	-91.8	-91.8 0.22 (1)	4.98	I-D	0 / 89	0.03 (4)
C-M	-1056 / 0	-91.8	-91.8 0.29 (1)	5.73	I-E	0 / 111	0.03 (1)
M-D	-1056 / 0	-91.8	-91.8 0.29 (1)	5.73	H-E	-98 / 30	0.03 (1)
D-E	-651 / 0	-91.8	-91.8 0.04 (1)	6.25	L-J	-58 / 0	0.01 (1)
E-F	-1037 / 0	-91.8	-91.8 0.38 (1)	5.62	B-J	0 / 1271	0.31 (1)
L-B	-1169 / 0	0.0	0.0 0.13 (1)	7.34	H-F	0 / 852	0.21 (1)
G-F	-1030 / 0	0.0	0.0 0.07 (1)	7.81			
L-N	0 / 47	-18.5	-18.5 0.08 (1)	10.00			
N-K	0 / 47	-18.5	-18.5 0.08 (1)	10.00			
K-J	0 / 125	0.0	0.0 0.10 (1)	10.00			
J-C	0 / 122	0.0	0.0 0.10 (1)	10.00			
J-O	0 / 1313	-18.5	-18.5 0.21 (1)	10.00			
O-P	0 / 1313	-18.5	-18.5 0.21 (1)	10.00			
P-I	0 / 1313	-18.5	-18.5 0.21 (1)	10.00			
I-H	0 / 826	-18.5	-18.5 0.13 (1)	10.00			
H-Q	0 / 0	-18.5	-18.5 0.06 (4)	10.00			
Q-R	0 / 0	-18.5	-18.5 0.06 (4)	10.00			
R-G	0 / 0	-18.5	-18.5 0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-9-9	-34	-34	---	FRONT	VERT	DEAD	---	C1
D	5-9-9	-145	-145	---	FRONT	VERT	SNOW	---	C1
E	7-0-7	-34	-34	---	FRONT	VERT	DEAD	---	C1
E	7-0-7	-64	-64	---	BACK	VERT	TOTAL	---	C1
E	7-0-7	-145	-145	---	FRONT	VERT	SNOW	---	C1
M	5-3-4	-74	-74	---	BACK	VERT	TOTAL	---	C1
N	1-3-4	-99	-99	---	BACK	VERT	TOTAL	---	C1
O	3-3-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
P	5-3-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
Q	7-3-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
R	9-3-4	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.38/1.00 (E-F:1), BC=0.21/1.00 (I-J:1), WB=0.31/1.00 (B-J:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

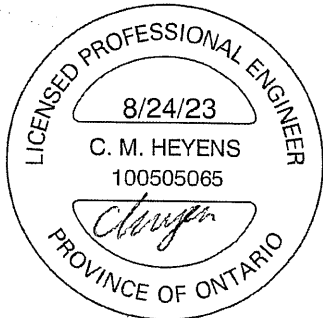
NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

RECEIVED CONTINUED ON PAGE 2

Per: joshua.nabua



STRUCTURAL COMPONENT ONLY
DWG # TR23080902

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434360	T21	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 11:38:20 2023 Page 2
ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-8HILnEiiJlsa0q3N bGZ5SO 9supP?EkqO8aBykfkH

JSI GRIP= 0.75 (F) (INPUT = 0.90)

JSI METAL= 0.39 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY

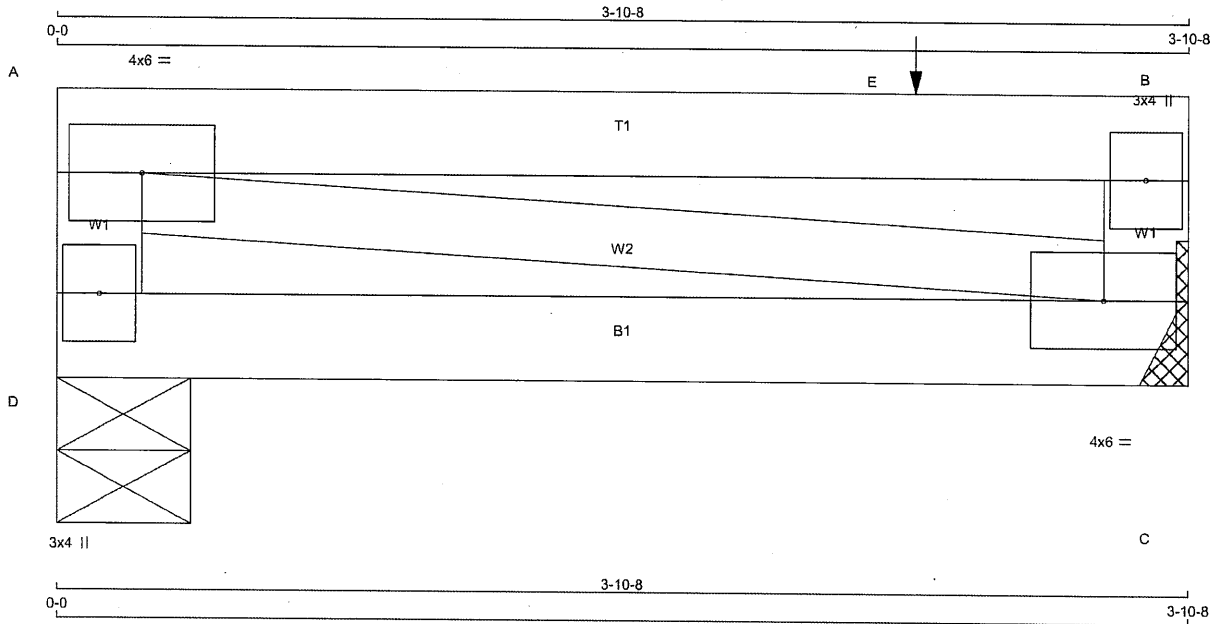
DWG # TR23080902

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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Per: joshua.nabua



LUMBER N. L. G. A. RULES <table><tr><td>CHORDS</td><td>SIZE</td><td>LUMBER</td><td>DESCR.</td></tr><tr><td>D - A</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>A - B</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>C - B</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>D - C</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr></table> ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.							CHORDS	SIZE	LUMBER	DESCR.	D - A	2x4 DRY	No.2	SPF	A - B	2x4 DRY	No.2	SPF	C - B	2x4 DRY	No.2	SPF	D - C	2x4 DRY	No.2	SPF	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td></td><td colspan="2">FACTORED GROSS REACTION</td><td colspan="2">MAXIMUM FACTORED GROSS REACTION</td><td>INPUT BRG</td><td>REQRD BRG</td></tr><tr><td>JT</td><td>VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td><td>UPLIFT</td><td>IN-SX IN-SX</td></tr><tr><td>D</td><td>452</td><td>0</td><td>452</td><td>0</td><td>0</td><td>5-8 1-8</td></tr><tr><td>C</td><td>962</td><td>0</td><td>962</td><td>0</td><td>0</td><td>MECHANICAL</td></tr></table> A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.								FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX IN-SX	D	452	0	452	0	0	5-8 1-8	C	962	0	962	0	0	MECHANICAL	DESIGN CRITERIA SPECIFIED LOADS: <table><tr><td>TOP CH.</td><td>LL = 25.6 PSF</td></tr><tr><td></td><td>DL = 6.0 PSF</td></tr><tr><td>BOT CH.</td><td>LL = 0.0 PSF</td></tr><tr><td></td><td>DL = 7.4 PSF</td></tr><tr><td>TOTAL LOAD</td><td>= 39.0 PSF</td></tr></table> SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF CBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01") CSI: TC=0.56/1.00 (A-B:1) , BC=0.04/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.36/1.00 (A-B:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES <table><tr><td>PLATE</td><td>GRIP(DRY)</td><td>SHEAR</td><td>SECTION</td></tr><tr><td></td><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td></tr><tr><td></td><td>MAX MIN</td><td>MAX MIN</td><td>MAX MIN</td></tr><tr><td>MT20</td><td>650 371</td><td>1747 788</td><td>1987 1873</td></tr></table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.18 (B) (INPUT = 0.90) JSI METAL= 0.06 (B) (INPUT = 0.95)							TOP CH.	LL = 25.6 PSF		DL = 6.0 PSF	BOT CH.	LL = 0.0 PSF		DL = 7.4 PSF	TOTAL LOAD	= 39.0 PSF	PLATE	GRIP(DRY)	SHEAR	SECTION		(PSI)	(PLI)	(PLI)		MAX MIN	MAX MIN	MAX MIN	MT20	650 371	1747 788	1987 1873																														
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DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: <table><tr><td>CHORDS #ROWS</td><td>SURFACE SPACING (IN)</td><td>LOAD(PLF)</td></tr><tr><td colspan="3">TOP CHORDS : (0.122"x3") SPIRAL NAILS</td></tr><tr><td>D-A 1</td><td>12</td><td>TOP</td></tr><tr><td>A-B 1</td><td>12</td><td>TOP</td></tr><tr><td>B-C 1</td><td>12</td><td>TOP</td></tr><tr><td colspan="3">BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS</td></tr><tr><td>D-C 1</td><td>12</td><td>TOP</td></tr><tr><td colspan="3">WEBS : (0.122"x3") SPIRAL NAILS</td></tr><tr><td>2x3 1</td><td>6</td><td></td></tr></table> NAILS TO BE DRIVEN FROM ONE SIDE ONLY. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.							CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)	TOP CHORDS : (0.122"x3") SPIRAL NAILS			D-A 1	12	TOP	A-B 1	12	TOP	B-C 1	12	TOP	BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			D-C 1	12	TOP	WEBS : (0.122"x3") SPIRAL NAILS			2x3 1	6		UNFACTORED REACTIONS <table><tr><td></td><td>1ST LCASE</td><td colspan="6">MAX /MIN. COMPONENT REACTIONS</td></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>D</td><td>319</td><td>212 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>107 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>679</td><td>453 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>225 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><td colspan="2">CHORDS</td><td colspan="2">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>FACTORED VERT. LOAD (PLF)</td><td>MAX. FACTORED FORCE (LBS)</td></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td>FR-TO</td></tr><tr><td>D-A</td><td>-416 / 0</td><td>0.0 0.0 0.02 (1)</td><td>7.81 0 / 0</td></tr><tr><td>A-E</td><td>0 / 0</td><td>-91.8 -91.8 0.56 (1)</td><td>10.00</td></tr><tr><td>E-B</td><td>0 / 0</td><td>-91.8 -91.8 0.56 (1)</td><td>10.00</td></tr><tr><td>C-B</td><td>-926 / 0</td><td>0.0 0.0 0.05 (1)</td><td>7.81</td></tr><tr><td>D-C</td><td>0 / 0</td><td>-18.5 -18.5 0.04 (4)</td><td>10.00</td></tr></table> SPECIFIED CONCENTRATED LOADS (LBS) <table><tr><td>JT</td><td>LOC.</td><td>LC1</td><td>MAX-</td><td>MAX+</td><td>FACE</td><td>DIR.</td><td>TYPE</td><td>HEEL</td><td>CONN.</td></tr><tr><td>E</td><td>2-11-4</td><td>-696</td><td>-696</td><td>-</td><td>TOP</td><td>VERT</td><td>TOTAL</td><td>-</td><td>C1</td></tr></table> CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.								1ST LCASE	MAX /MIN. COMPONENT REACTIONS						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	D	319	212 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0	C	679	453 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0	CHORDS		WEBS		MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FR-TO		FROM TO	FR-TO	D-A	-416 / 0	0.0 0.0 0.02 (1)	7.81 0 / 0	A-E	0 / 0	-91.8 -91.8 0.56 (1)	10.00	E-B	0 / 0	-91.8 -91.8 0.56 (1)	10.00	C-B	-926 / 0	0.0 0.0 0.05 (1)	7.81	D-C	0 / 0	-18.5 -18.5 0.04 (4)	10.00	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	E	2-11-4	-696	-696	-	TOP	VERT	TOTAL	-	C1
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STRUCTURAL COMPONENT ONLY
DWG # TR23080903

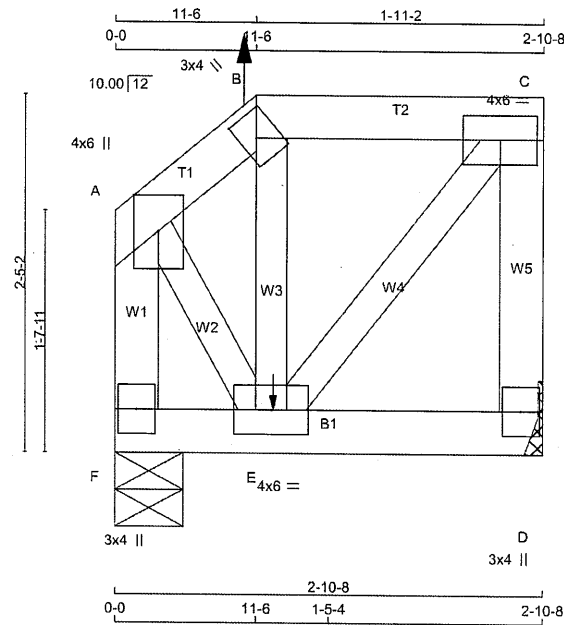
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434360	T23	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Jul 5 2023 MjTek Industries, Inc. Thu Aug 24 11:38:22 2023 Page 1
ID:LpDGmu0HxWz8EPf6?EUU9qyp?17-4qs3ITGyEw?apJ RUPdkeWXpUyaUHM7XC tEf4ykrfKf



TOTAL WEIGHT = 15 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTW+h	MT20	3.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX
D	155	0	155	0	0	MECHANICAL	
F	151	0	151	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	109	74 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
F	106	74 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-62 / 0	-91.8	-91.8	0.02 (1)	E-B	-81 / 20	0.01 (1)
B-C	-43 / 0	-91.8	-91.8	0.06 (1)	A-E	0 / 75	0.02 (1)
D-C	-139 / 0	0.0	0.0	0.02 (1)	E-C	0 / 66	0.02 (1)
F-A	-142 / 0	0.0	0.0	0.02 (1)			
F-E	0 / 0	-18.5	-18.5	0.01 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	11-6	9	1	100	FRONT	VERT	TOTAL	---	C1
E	1-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.06/1.00 (B-C:1), BC=0.01/1.00 (D-E:4), WB=0.02/1.00 (A-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 0.95)

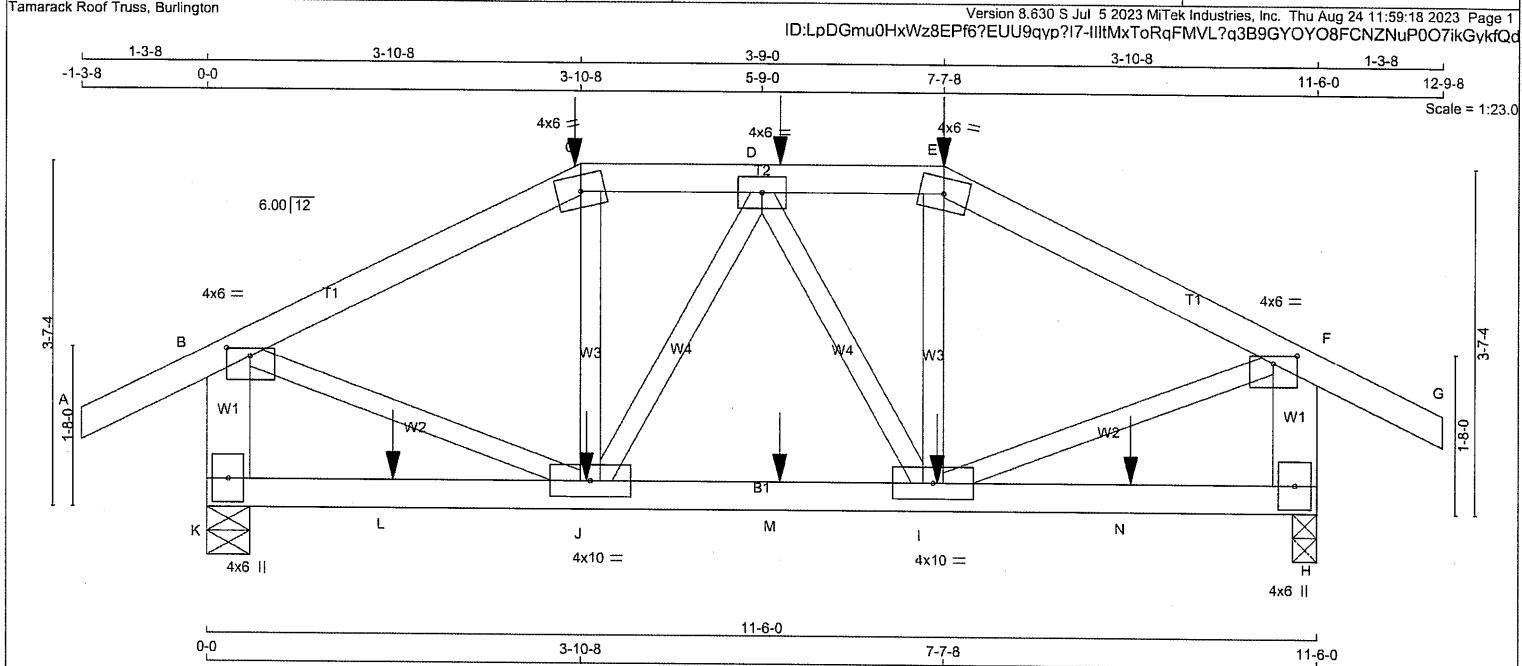


STRUCTURAL COMPONENT ONLY
DWG # TR23080904

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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Per: joshua.nabua



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TTW-m	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-p	MT20	4.0	6.0	1.00	3.00
H	BMV1+p	MT20	4.0	6.0		
I	BMVWW-t	MT20	4.0	10.0		
J	BMVWW-t	MT20	4.0	10.0		
K	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
K	1054	0	1054	0	5-8	1-8
H	1054	0	1054	0	3-0	1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
K	743	500 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0
H	743	500 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	J-C	-37 / 51	0.02 (4)
B-C	-945 / 0	-91.8	-91.8	0.28 (1)	5.98	I-E	-36 / 52	0.02 (4)
C-D	-843 / 0	-91.8	-91.8	0.05 (1)	6.25	B-J	0 / 892	0.22 (1)
D-E	-844 / 0	-91.8	-91.8	0.05 (1)	6.25	I-F	0 / 893	0.22 (1)
E-F	-946 / 0	-91.8	-91.8	0.28 (1)	5.98	J-D	-161 / 0	0.04 (1)
F-G	0 / 28	-91.8	-91.8	0.13 (1)	10.00	D-I	-159 / 0	0.04 (1)
K-B	-1015 / 0	0.0	0.0	0.08 (1)	7.81			
H-F	-1015 / 0	0.0	0.0	0.08 (1)	7.81			
K-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
L-J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
J-M	0 / 919	-18.5	-18.5	0.20 (1)	10.00			
M-I	0 / 919	-18.5	-18.5	0.20 (1)	10.00			
I-N	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
N-H	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-145	-145		BACK	VERT	TOTAL		C1
D	5-11-4	-59	-59		BACK	VERT	TOTAL		C1
E	7-7-8	-145	-145		BACK	VERT	TOTAL		C1
I	7-6-12	-14	-14		BACK	VERT	TOTAL		C1
J	3-11-4	-14	-14		BACK	VERT	TOTAL		C1
L	1-11-4	-14	-14		BACK	VERT	TOTAL		C1
M	5-11-4	-14	-14		BACK	VERT	TOTAL		C1
N	9-6-12	-14	-14		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.28/1.00 (E-F:1), BC=0.20/1.00 (I-J:1), WB=0.22/1.00 (F-I:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

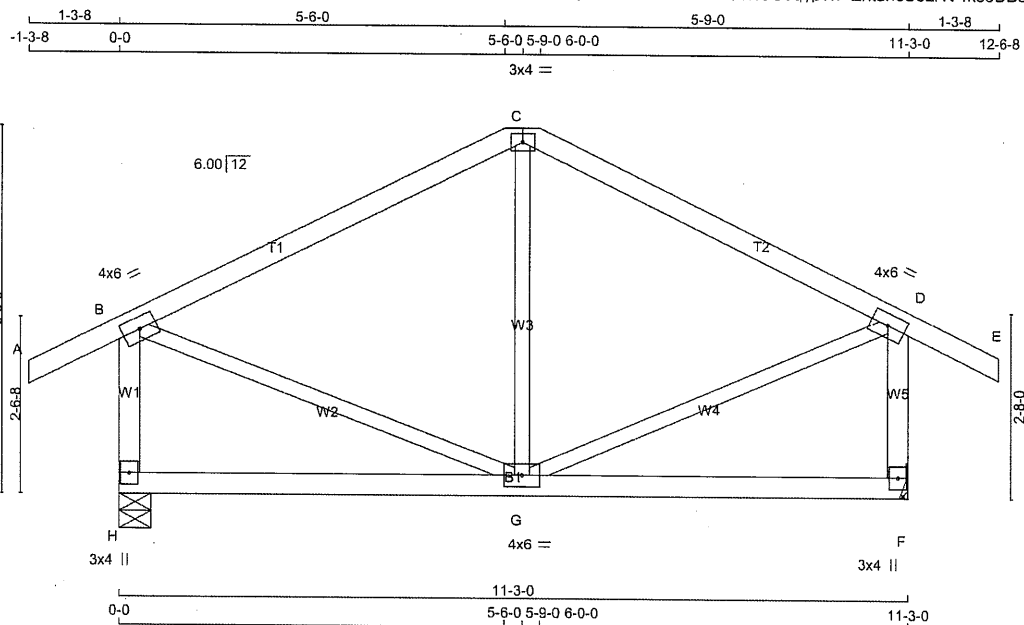
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080915

**CITY OF RICHMOND HILL
BUILDING DIVISION**
05/01/2024
RECEIVED
Per: joshua.nabua



TOTAL WEIGHT = 49 lb
(M)[F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW-t	MT20	4.0	6.0	
C	TTW-p	MT20	3.0	4.0	
D	TMW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW-t	MT20	4.0	6.0	
H	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	745	0	745	0	5-8
F	745	0	745	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN	COMPONENT	REACTIONS				
H	524	357 / 0	0 / 0	PERM LIVE	WIND	DEAD	SOIL	
F	524	357 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 28	-91.8	-91.8 0.12 (1)	B-G	0 / 380
B-C	-396 / 0	-91.8	-91.8 0.39 (1)	G-D	0 / 386
C-D	-396 / 0	-91.8	-91.8 0.36 (1)	G-C	-161 / 37
D-E	0 / 28	-91.8	-91.8 0.12 (1)		
H-B	-703 / 0	0.0	0.0 0.09 (1)		
F-D	-706 / 0	0.0	0.0 0.10 (1)		
H-G	0 / 0	-18.5	-18.5 0.17 (4)		
G-F	0 / 0	-18.5	-18.5 0.17 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.17/1.00 (G-H:4) ,
WB=0.09/1.00 (D-G:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L5 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (G) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 0.95)



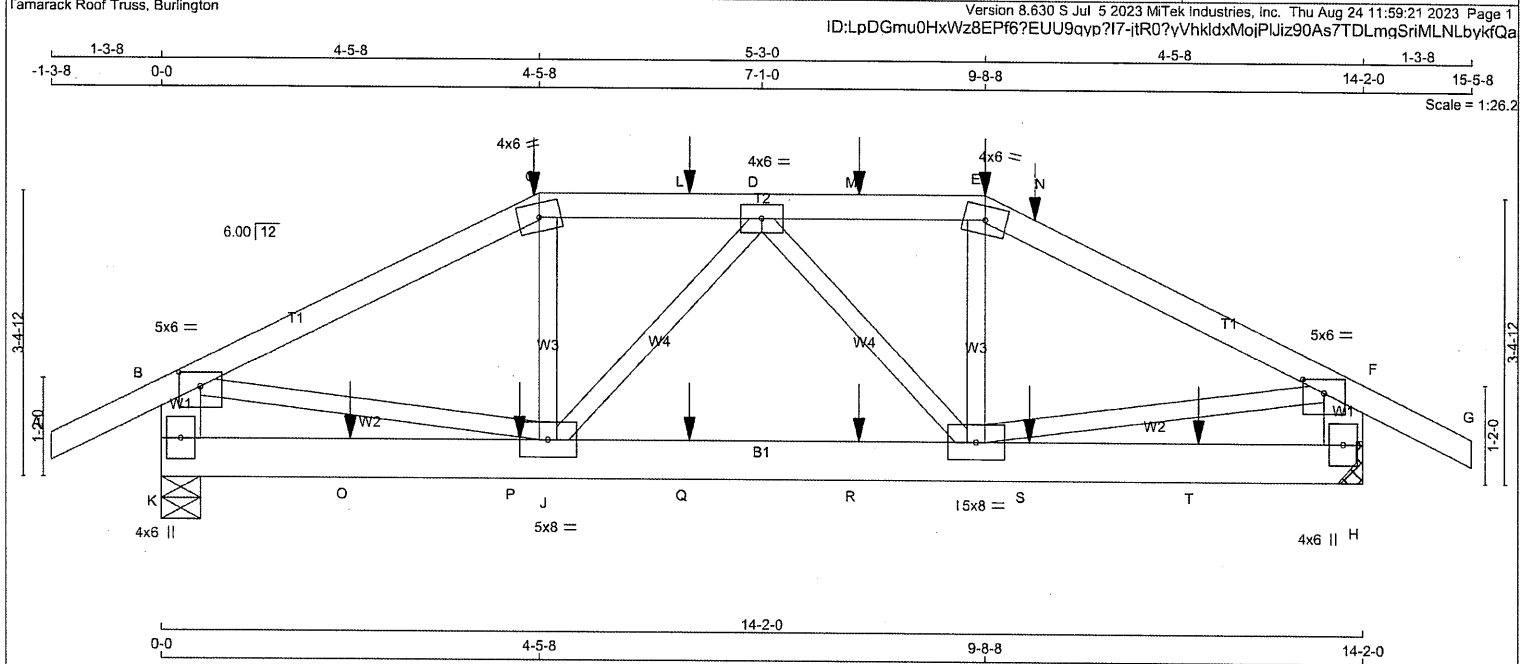
STRUCTURAL COMPONENT ONLY
DWG # TR23080916

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME 434586	TRUSS NAME T32	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 11:59:21 2023 Page 1		ID: LpDgmu0HxWz8EPf6?EUU9qyp?I7-jtR0?yVhklxdxMojPIJiz90As7TDLmgSriMLNLbykfQa	



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	2.00	3.00
C	TTW-m	MT20	4.0	6.0		
D	TMWV-t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMWV-p	MT20	5.0	6.0	2.00	3.00
H	BMV1+p	MT20	4.0	6.0		
I	BMWVW-t	MT20	5.0	8.0		
J	BMWVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
K	1520	0	1520	0	5-8
H	1545	0	1545	0	1-10

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-11.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1069	733 / 0	0 / 0	0 / 0	0 / 0	336 / 0	0 / 0
H	1086	746 / 0	0 / 0	0 / 0	0 / 0	340 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LC1)	MAX. (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 28	-91.8	-91.8 0.13 (1)	10.00	J-C	0 / 143	0.05 (4)
B-C	-1882 / 0	-91.8	-91.8 0.43 (1)	4.38	J-D	-294 / 0	0.08 (1)
C-L	-1690 / 0	-91.8	-91.8 0.21 (1)	4.86	D-I	-281 / 0	0.08 (1)
L-D	-1690 / 0	-91.8	-91.8 0.21 (1)	4.86	I-E	0 / 134	0.05 (4)
D-M	-1698 / 0	-91.8	-91.8 0.21 (1)	4.85	B-J	0 / 1706	0.42 (1)
M-E	-1698 / 0	-91.8	-91.8 0.21 (1)	4.85	I-F	0 / 1714	0.42 (1)
E-N	-1870 / 0	-91.8	-91.8 0.49 (1)	4.30			
N-F	-1870 / 0	-91.8	-91.8 0.49 (1)	4.30			
F-G	0 / 28	-91.8	-91.8 0.13 (1)	10.00			
K-B	-1455 / 0	0.0	0.0 0.10 (1)	7.81			
H-F	-1475 / 0	0.0	0.0 0.10 (1)	7.81			
K-O	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
O-P	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
P-J	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
J-Q	0 / 1876	-18.5	-18.5 0.31 (1)	10.00			
Q-R	0 / 1876	-18.5	-18.5 0.31 (1)	10.00			
R-I	0 / 1876	-18.5	-18.5 0.31 (1)	10.00			
I-S	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
S-T	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
T-H	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-5-8	-36	-36	---	FRONT	VERT	DEAD	---	C1
C	4-5-8	-77	-77	---	FRONT	VERT	TOTAL	---	C1
C	4-5-8	-150	-150	---	FRONT	VERT	SNOW	---	C1
E	9-8-8	-36	-36	---	FRONT	VERT	DEAD	---	C1
E	9-8-8	-150	-150	---	FRONT	VERT	SNOW	---	C1
L	6-2-12	-73	-73	---	FRONT	VERT	TOTAL	---	C1
M	8-2-12	-73	-73	---	FRONT	VERT	TOTAL	---	C1
N	10-2-12	-88	-88	---	FRONT	VERT	TOTAL	---	C1
O	2-2-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1
P	4-2-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1
Q	6-2-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1
R	8-2-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1
S	10-2-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1
T	12-2-12	-33	-33	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.49/1.00 (E-F:1), BC=0.31/1.00 (I-J:1), WB=0.42/1.00 (F-I:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
MT20 650 375 1747 888 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (J) (INPUT = 0.90)
JSI METAL= 0.38 (F) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080917

RECEIVED
Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	T32	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 11:59:21 2023 Page 2
 ID:LpDGmu0HxWz8EPf6?EUU9gyp?I7-jtR0?yVhkldxMojPIJiz90As7TDLmgSriMLNLbykfQa

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

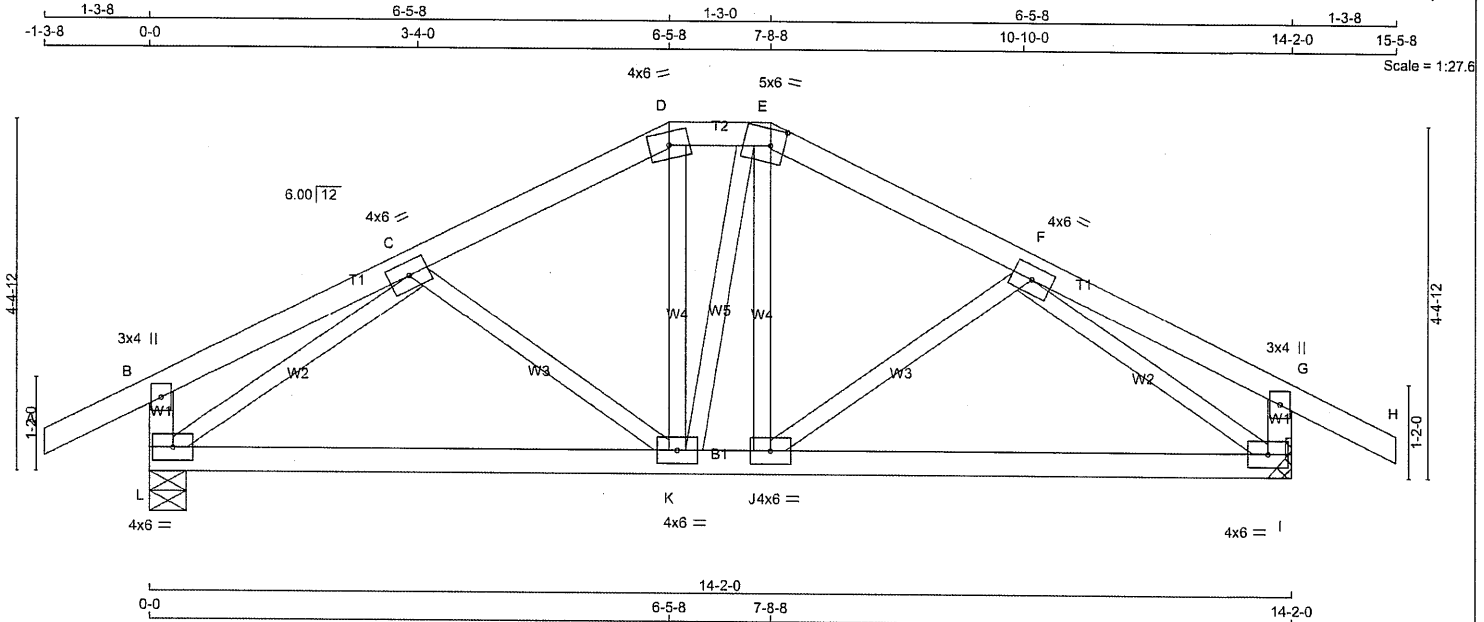


STRUCTURAL COMPONENT ONLY
DWG # TR23080917

CITY OF RICHMOND HILL
 BUILDING DIVISION
 05/01/2024
 RECEIVED
 Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	T33	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 11:59:22 2023 Page 1
ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-B4?OCiWJV3lo ylc1ECIEI6ltadV9z?x05wt2yktQZ



TOTAL WEIGHT = 61 lb
[M/JF]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.00
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	905	0	905	0	0
I	905	0	905	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	638	432 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0
I	638	432 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 28	-91.8 -91.8	0.12 (1)	C-K	-149 / 0
B-C	0 / 14	-91.8 -91.8	0.14 (1)	K-D	0 / 171
C-D	-780 / 0	-91.8 -91.8	0.11 (1)	K-E	0 / 5
D-E	-688 / 0	-91.8 -91.8	0.02 (1)	J-E	0 / 165
E-F	-778 / 0	-91.8 -91.8	0.11 (1)	J-F	-149 / 0
F-G	0 / 14	-91.8 -91.8	0.14 (1)	L-C	-1011 / 0
G-H	0 / 28	-91.8 -91.8	0.12 (1)	F-I	-1010 / 0
L-B	-242 / 0	0.0 0.0	0.02 (1)		
I-G	-242 / 0	0.0 0.0	0.02 (1)		
L-K	0 / 803	-18.5 -18.5	0.25 (4)		
K-J	0 / 687	-18.5 -18.5	0.25 (4)		
J-I	0 / 802	-18.5 -18.5	0.24 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.07")

CSI: TC=0.14/1.00 (F-G:1) . BC=0.25/1.00 (J-K:4) .
WB=0.28/1.00 (C-L:1) . SSI=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (L) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080918

CITY OF RICHMOND HILL
BUILDING DIVISION

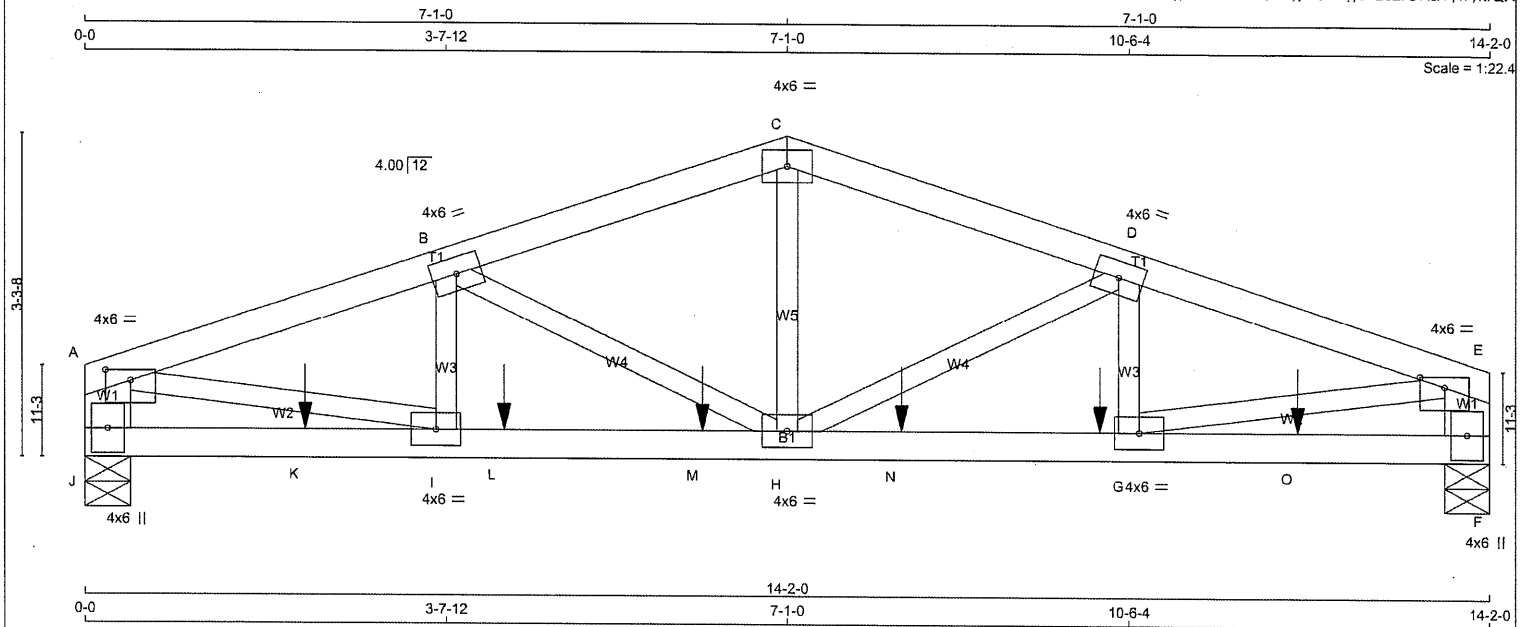
05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434586	T34	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
J - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
J - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - E	1	12
J - A	2	12
F - E	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F	1	12
WEBS : (0.122"x3") SPIRAL NAILS		
D - G	1	6
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1484	0	1484	0	0
F	1524	0	1524	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN	COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE
J	1045	714 / 0	0 / 0	0 / 0
F	1072	734 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	-2587 / 0	-91.8 -91.8	0.11 (1)	H-C	0 / 1033
B-C	-2061 / 0	-91.8 -91.8	0.09 (1)	H-D	-603 / 0
C-D	-2061 / 0	-91.8 -91.8	0.09 (1)	G-D	0 / 89
D-E	-2606 / 0	-91.8 -91.8	0.11 (1)	B-H	-583 / 0
J-A	-1390 / 0	0.0 0.0	0.05 (1)	I-B	0 / 73
F-E	-1399 / 0	0.0 0.0	0.05 (1)	A-I	0 / 2498
				G-E	0 / 2516
J-K	0 / 0	-18.5 -18.5	0.12 (1)		
K-I	0 / 0	-18.5 -18.5	0.12 (1)		
I-L	0 / 2461	-18.5 -18.5	0.33 (1)		
L-M	0 / 2461	-18.5 -18.5	0.33 (1)		
M-H	0 / 2461	-18.5 -18.5	0.33 (1)		
H-N	0 / 2479	-18.5 -18.5	0.33 (1)		
N-G	0 / 2479	-18.5 -18.5	0.33 (1)		
G-O	0 / 0	-18.5 -18.5	0.15 (1)		
O-F	0 / 0	-18.5 -18.5	0.15 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	10-2-12	-169	-169	---	BACK	VERT	TOTAL	---	C1
K	2-2-12	-169	-169	---	BACK	VERT	TOTAL	---	C1
L	4-2-12	-169	-169	---	BACK	VERT	TOTAL	---	C1
M	6-2-12	-169	-169	---	BACK	VERT	TOTAL	---	C1
N	8-2-12	-169	-169	---	BACK	VERT	TOTAL	---	C1
O	12-2-12	-169	-169	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC 2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.11/1.00 (D-E:1), BC=0.33/1.00 (G-H:1), WB=0.31/1.00 (E-G:1), SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.28 (G) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080919

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	T34	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.25	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TTW-p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	4.0	6.0	1.25	3.00
F	BMV1+p	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080919

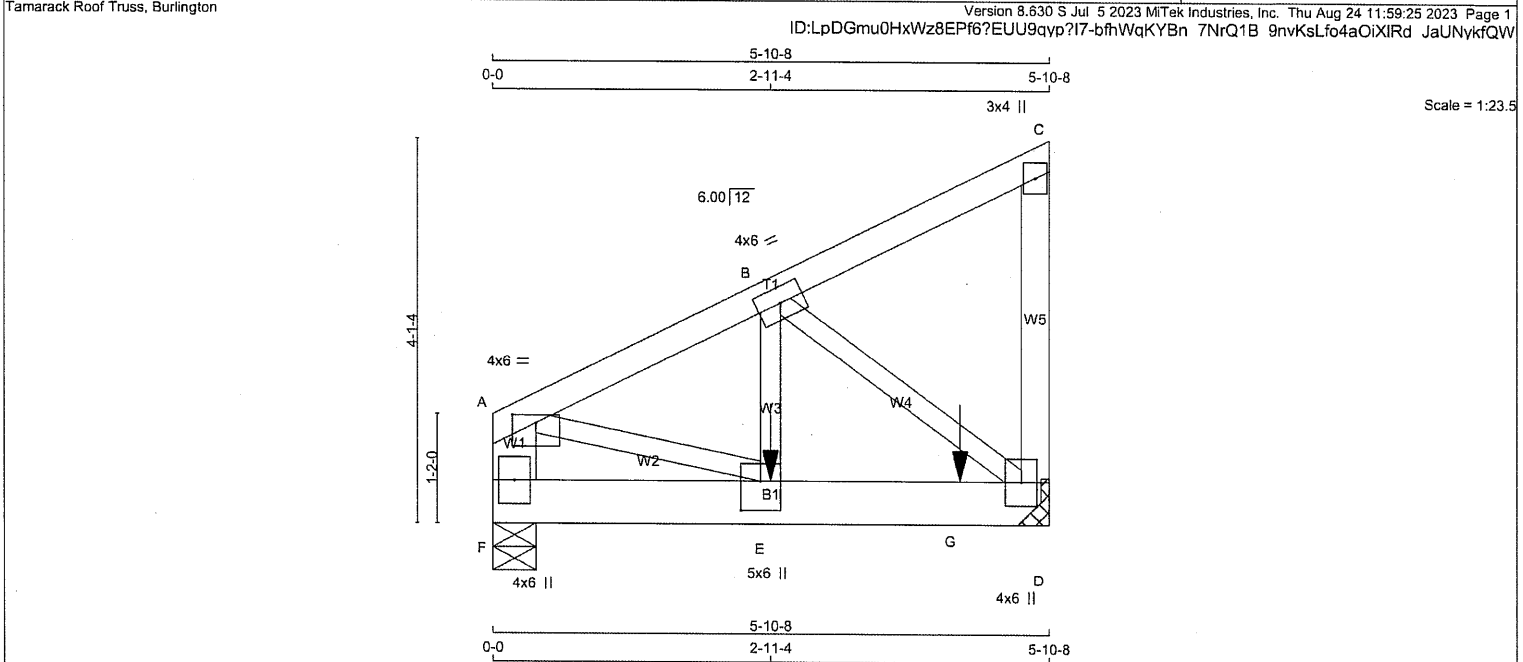
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434586	T35	1	2	TRUSS DESC.		



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x6 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - D	2x6 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 2	12	TOP
A-C 1	12	TOP
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B-E 1	3	SIDE(215.1)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
F	1229 0	1229 0	5-8	1-8
D	1833 0	1833 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM.LIVE	WIND	DEAD	SOIL
F	864	592 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
D	1289	886 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO							
F-A	-1202 / 0	0.0	0.0 0.04 (1)	7.81	A-E	0 / 1475	0.18 (1)
A-B	-1584 / 0	-91.8	-91.8 0.06 (1)	6.25	E-B	0 / 1488	0.18 (1)
B-C	-10 / 0	-91.8	-91.8 0.05 (1)	10.00	B-D	-1794 / 0	0.21 (1)
D-C	-113 / 0	0.0	0.0 0.01 (1)	7.81			
FR-FR							
F-E	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
E-G	0 / 1426	-18.5	-18.5 0.31 (1)	10.00			
G-D	0 / 1426	-18.5	-18.5 0.31 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-11-4	-1072	-1072	---	BACK	VERT	TOTAL	---	C1
G	4-11-4	-624	-624	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF	
DL	25.6	PSF	
DL	6.0	PSF	
BOT CH. <td>LL</td> <td>0.0</td> <td>PSF</td>	LL	0.0	PSF
DL	7.4	PSF	
TOTAL LOAD		39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.31/1.00 (D-E:1) , WB=0.21/1.00 (B-D:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (E) (INPUT = 0.90)

JSI METAL= 0.32 (D) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER

8/24/23

C. M. HEYENS

100505065

PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY

DWG # TR23080920

CITY OF RICHMOND HILL

BUILDING DIVISION

05/01/2024

RECEIVED

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434586	T35	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMWW+t	MT20	5.0	6.0	3.75	2.50
F	BMV1+p	MT20	4.0	6.0		

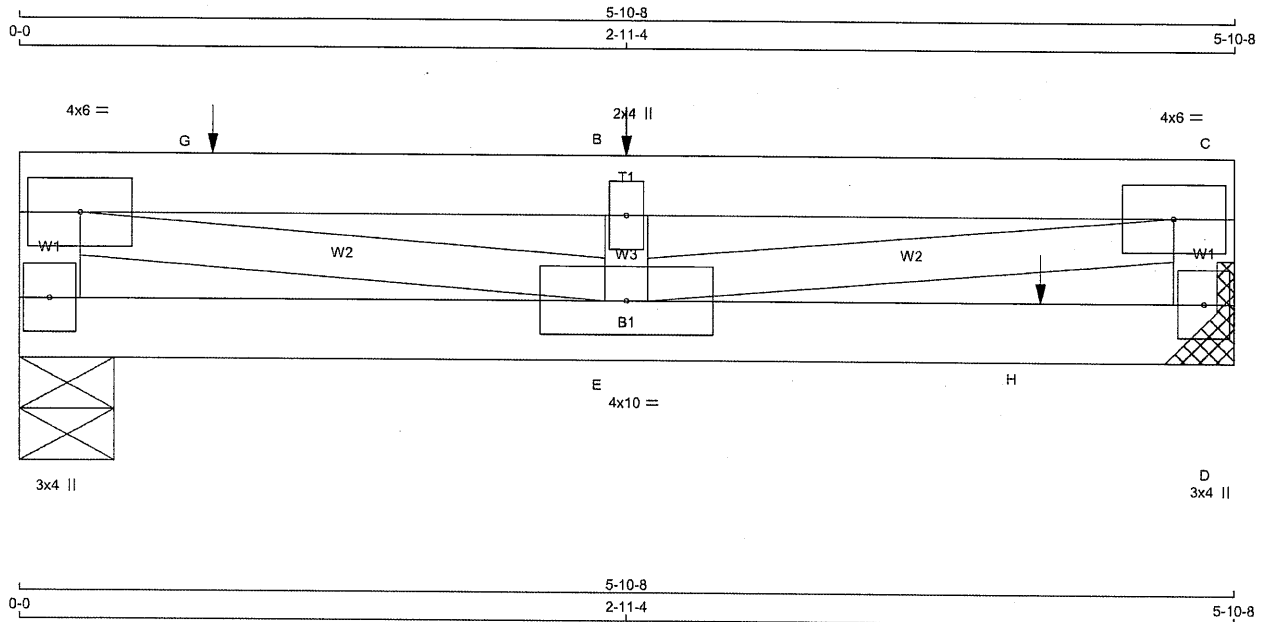
NOTES- (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY
 DWG # TR23080920

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
 Per: joshua.nabua



TOTAL WEIGHT = 2 X 19 = 38 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	SIDE(0.0)
C-D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 1	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMVW+w	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	10.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	1090	0	1090	0	0	5-8	1-8		
D	1448	0	1448	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	788	518 / 0	0 / 0	0 / 0	0 / 0	250 / 0	0 / 0
D	1020	691 / 0	0 / 0	0 / 0	0 / 0	329 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
F-A	-1042 / 0	0.0	0.0 0.06 (1)	A-E	0 / 3150
A-G	-3062 / 0	-91.8	-91.8 0.22 (1)	E-B	-1232 / 0
G-B	-3062 / 0	-91.8	-91.8 0.22 (1)	B-C	0 / 3150
B-C	-3062 / 0	-91.8	-91.8 0.15 (1)	C-D	
D-C	-905 / 0	0.0	0.0 0.05 (1)		
F-E	0 / 0	-18.5	-18.5 0.05 (1)		
E-H	0 / 0	-18.5	-18.5 0.37 (1)		
H-D	0 / 0	-18.5	-18.5 0.37 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-11-4	-680	-680	---	TOP	VERT	TOTAL	---	C1
G	11-4	-140	-140	---	BACK	VERT	TOTAL	---	C1
H	4-11-4	-510	-510	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/926 (0.08")

CSI: TC=0.22/1.00 (A-B:1), BC=0.37/1.00 (D-E:1), WB=0.39/1.00 (A-E:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371 1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080921

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	T36	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY

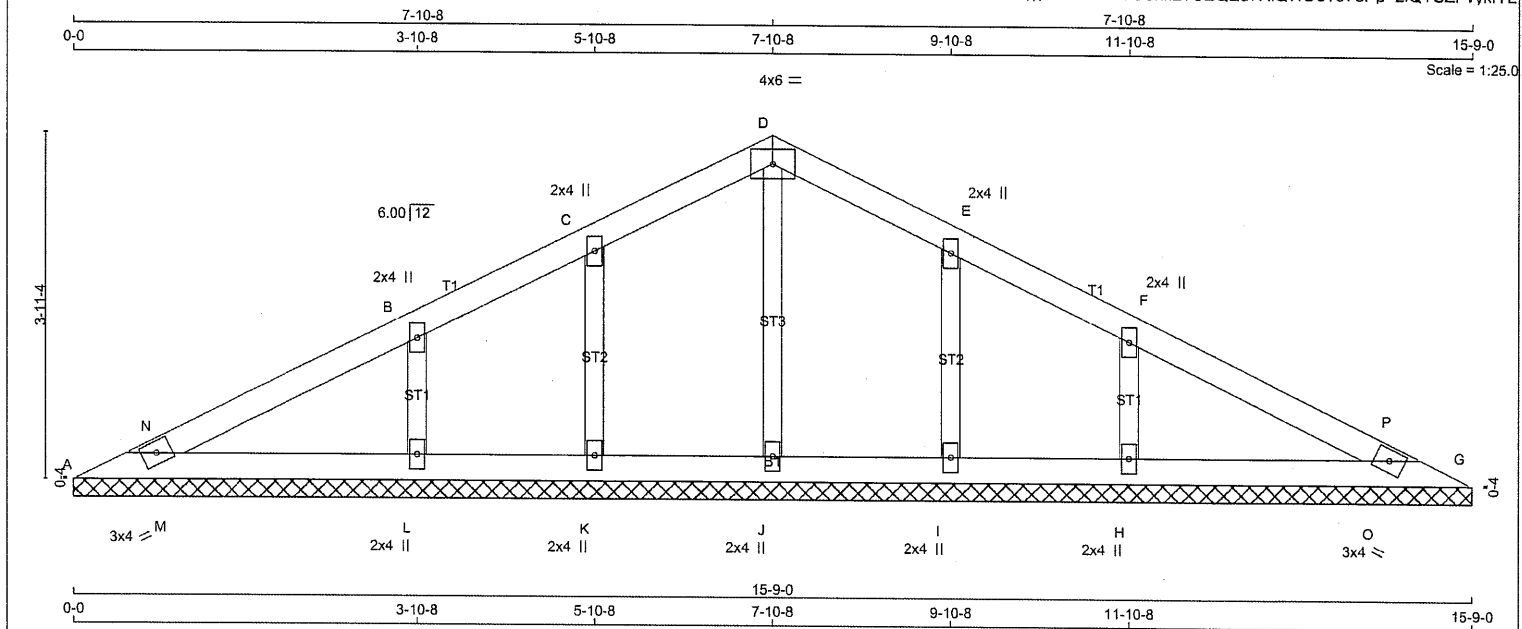
DWG # TR23080921

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

A - D 2x4 DRY No.2 SPF

D - G 2x4 DRY No.2 SPF

A - G 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

A TBM1-h MT20 3.0 4.0

B, C, E, F

B TMW+w MT20 2.0 4.0

D TTW-p MT20 4.0 6.0

G TBM1-h MT20 3.0 4.0

H, I, J, K, L

H BMW1+w MT20 2.0 4.0

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

A 133 0 133 0 0 15-9-0 (15-8-0)8

G 133 0 133 0 0 15-9-0 (15-8-0)8

J 329 0 329 0 0 15-9-0 (15-8-0)8

K 169 0 169 0 0 15-9-0 (15-8-0)8

L 398 0 398 0 0 15-9-0 (15-8-0)8

I 169 0 169 0 0 15-9-0 (15-8-0)8

H 398 0 398 0 0 15-9-0 (15-8-0)8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

A 94 62/0 0/0 0/0 0/0 31/0 0/0

G 94 62/0 0/0 0/0 0/0 31/0 0/0

J 234 148/0 0/0 0/0 0/0 85/0 0/0

K 119 81/0 0/0 0/0 0/0 38/0 0/0

L 281 183/0 0/0 0/0 0/0 98/0 0/0

I 119 81/0 0/0 0/0 0/0 38/0 0/0

H 281 183/0 0/0 0/0 0/0 98/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, K, L, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED FORCE VERT. LOAD LC1 MAX. FACTORED (PLF) CSI (LC) UNBRAC LENGTH FR-TO

MEMB. (LBS)

FR-TO

A-N 0/92 -91.8 -91.8 0.05 (1) 10.00 J-D -272/0 0.06 (1)

N-B 0/124 -91.8 -91.8 0.13 (1) 10.00 K-C -171/0 0.03 (1)

B-C 0/107 -91.8 -91.8 0.13 (1) 10.00 L-B -279/0 0.04 (1)

C-D 0/118 -91.8 -91.8 0.07 (1) 10.00 I-E -171/0 0.03 (1)

D-E 0/118 -91.8 -91.8 0.07 (1) 10.00 H-F -279/0 0.04 (1)

E-F 0/107 -91.8 -91.8 0.13 (1) 10.00 M-N -94/3 0.00 (1)

F-P 0/124 -91.8 -91.8 0.13 (1) 10.00 O-P -94/3 0.00 (1)

P-G 0/92 -91.8 -91.8 0.05 (1) 10.00

WEBS

MAX. FACTORED FORCE VERT. LOAD LC1 MAX. FACTORED (PLF) CSI (LC) UNBRAC LENGTH FR-TO

MEMB. (LBS)

FR-TO

A-M -106/0 -18.5 -18.5 0.10 (1) 6.25

M-L -95/0 -18.5 -18.5 0.10 (1) 6.25

L-K -106/0 -18.5 -18.5 0.07 (1) 6.25

K-J -111/0 -18.5 -18.5 0.02 (4) 6.25

J-I -111/0 -18.5 -18.5 0.02 (4) 6.25

I-H -106/0 -18.5 -18.5 0.07 (1) 6.25

H-O -95/0 -18.5 -18.5 0.10 (1) 6.25

O-G -106/0 -18.5 -18.5 0.10 (1) 6.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (F-P:1) , BC=0.10/1.00 (L-M:1) , WB=0.06/1.00 (D-J:1) , SSI=0.12/1.00 (B-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

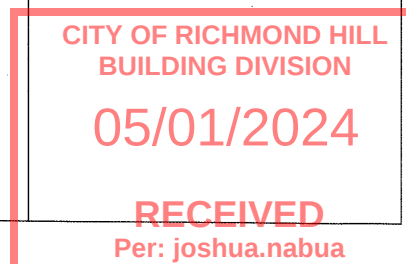
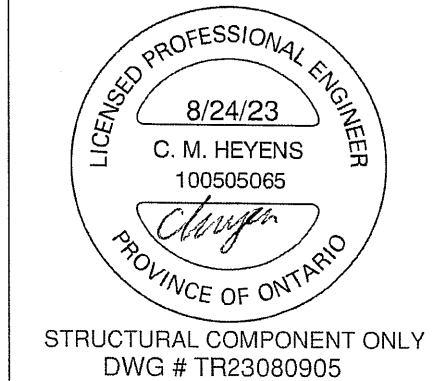
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)

JSI METAL= 0.12 (F) (INPUT = 0.95)

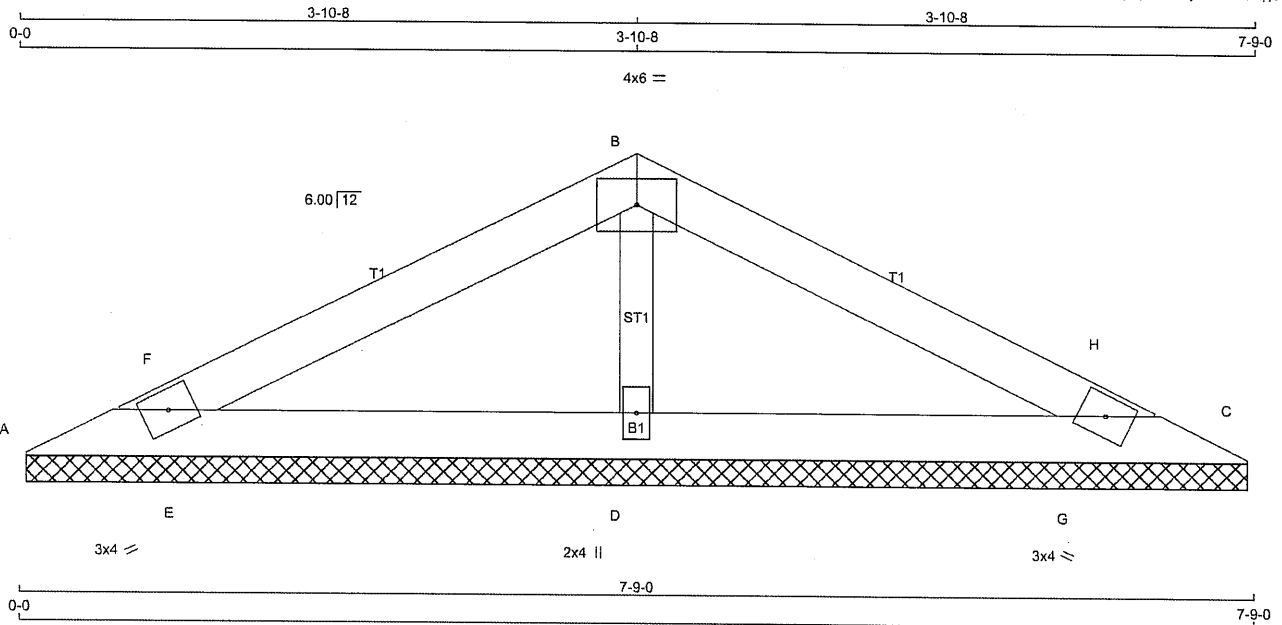




RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434361	V3	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:14.0

TOTAL WEIGHT = 18 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0			
B	TTW-p	MT20	4.0	6.0			
C	TBM1-h	MT20	3.0	4.0			
D	BMW1+w	MT20	2.0	4.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	86	0	86	0	7-8-0	1-8
C	86	0	86	0	7-8-0	1-8
D	674	0	674	0	7-8-0	1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	60	40 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
C	60	40 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	477	313 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-F	0 / 258	-91.8 -91.8	0.05 (1)	10.00	D-B	-493 / 0	0.07 (1)
F-B	0 / 268	-91.8 -91.8	0.16 (1)	10.00	E-F	-151 / 0	0.00 (1)
B-H	0 / 268	-91.8 -91.8	0.16 (1)	10.00	G-H	-151 / 0	0.00 (1)
H-C	0 / 258	-91.8 -91.8	0.05 (1)	10.00			
A-E	-255 / 0	-18.5 -18.5	0.12 (1)	6.25			
E-D	-241 / 0	-18.5 -18.5	0.12 (1)	6.25			
D-G	-241 / 0	-18.5 -18.5	0.12 (1)	6.25			
G-C	-255 / 0	-18.5 -18.5	0.12 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (B-F:1), BC=0.12/1.00 (D-E:1), WB=0.07/1.00 (B-D:1), SSI=0.09/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

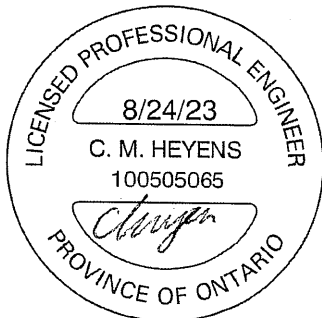
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.15 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080907

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

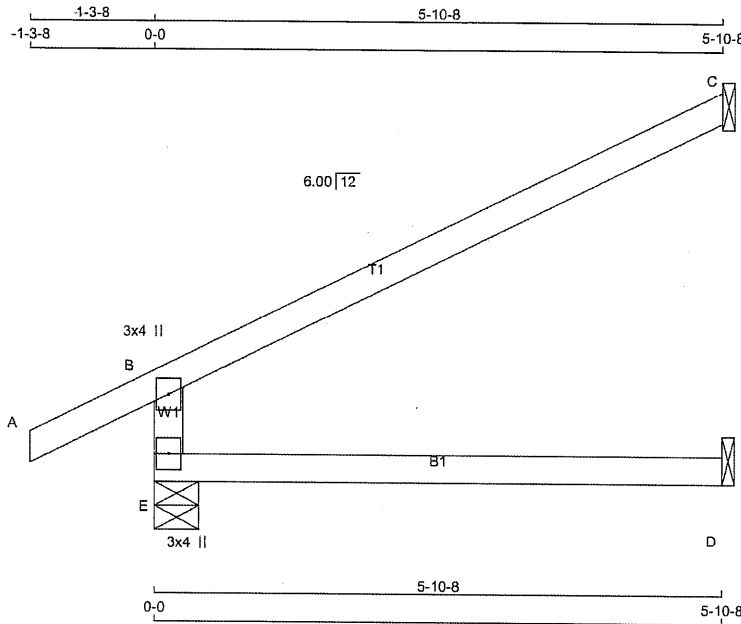
RECEIVED
Per: joshua.nabua

JOB NAME 434359	TRUSS NAME J1	QUANTITY 10	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:23.1

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	SPECIFIED LOADS:		
E - B	2x4	DRY	No.2	SPF	JT VERT	DOWN	UPLIFT	IN-SX	TOP CH. LL = 25.6 PSF		
A - C	2x4	DRY	No.2	SPF	E 525	0	0	5-8	DL = 6.0 PSF		
E - D	2x4	DRY	No.2	SPF	C 202	0	0	1-8	BOT CH. LL = 0.0 PSF		
DRY: SEASONED LUMBER.					D 45	0	0	1-8	DL = 7.4 PSF		
									TOTAL LOAD = 39.0 PSF		

TOTAL WEIGHT = 10 X 17 = 168 lb

[M]

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	FR-TO
FR-TO		FROM	TO	FR-TO			
E-B	-461 / 0	0.0	0.0 0.13 (4)	7.81			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00			
B-C	-30 / 0	-91.8	-91.8 0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080873

CITY OF RICHMOND HILL
BUILDING DIVISION

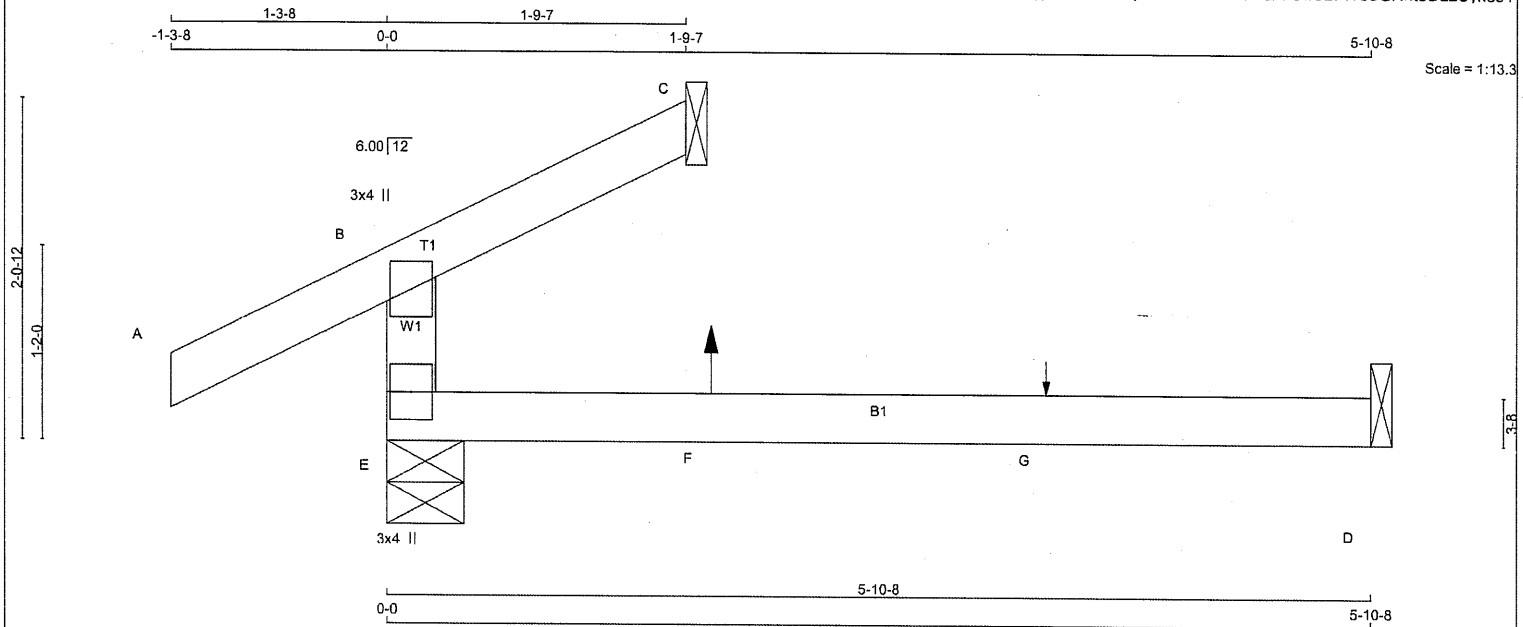
05/01/2024

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Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	J2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:17 2023 Page 1
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LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	TOP CH.	LL	=	25.6 PSF
E - B	2x4	DRY	No.2	DL	=	6.0 PSF	
A - C	2x4	DRY	No.2	BOT CH.	LL	=	0.0 PSF
E - D	2x4	DRY	No.2	DL	=	7.4 PSF	
DRY: SEASONED LUMBER.				TOTAL LOAD	=	39.0 PSF	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	284	0	284	0	0	5-8	1-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
JT	COMBINED	137 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0	0 / 0	0 / 0
E	200	137 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0	0 / 0
C	46	21 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	0 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO		FR-TO		
E-B	-227 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-9 / 9	-91.8	-91.8	0.08 (4)	10.00		
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00		
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	BACK	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

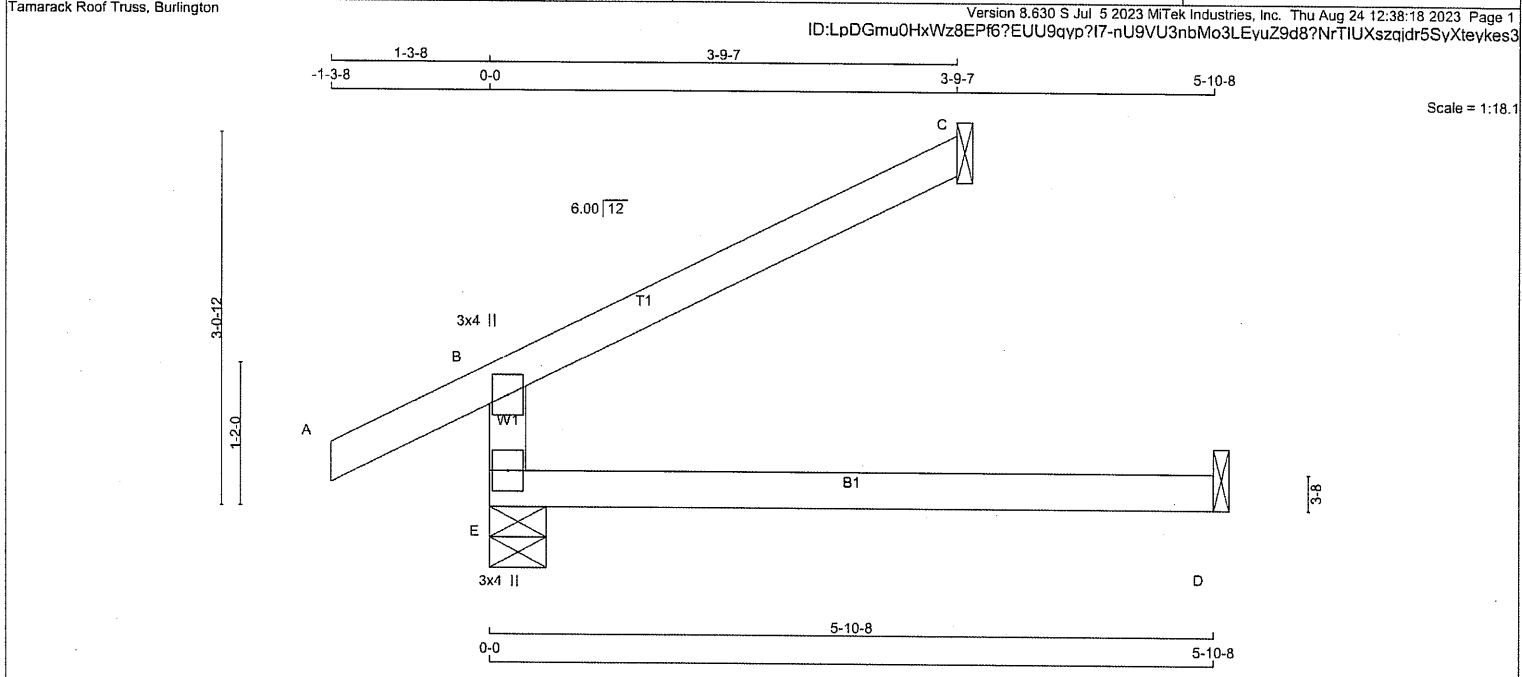


STRUCTURAL COMPONENT ONLY
DWG # TR23080874

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th>JT</th><th>VERT</th><th>HORZ</th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr><tr><td>E</td><td>405</td><td>0</td><td>405</td><td>0</td><td>5-8</td><td>1-8</td></tr><tr><td>C</td><td>130</td><td>0</td><td>130</td><td>0</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>45</td><td>0</td><td>50</td><td>0</td><td>1-8</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><th>JT</th><th>1ST LCASE</th><th>SNOW</th><th>LIVE</th><th>PERM. LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>286</td><td>190 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>96 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>90</td><td>73 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>17 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>36</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>36 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. CSI (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRAC LENGTH</th><th>MAX. CSI (LC)</th></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>E-B</td><td>-342 / 0</td><td>0.0</td><td>0.0 0.13 (4)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 28</td><td>-91.8</td><td>-91.8 0.12 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>B-C</td><td>-19 / 0</td><td>-91.8</td><td>-91.8 0.22 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.5</td><td>-18.5 0.13 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></table>				JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	E	405	0	405	0	5-8	1-8	C	130	0	130	0	1-8	1-8	D	45	0	50	0	1-8	1-8	JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0	C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0	D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	MAX. CSI (LC)	FR-TO		FROM TO		FR-TO				E-B	-342 / 0	0.0	0.0 0.13 (4)	7.81				A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00				B-C	-19 / 0	-91.8	-91.8 0.22 (1)	6.25				E-D	0 / 0	-18.5	-18.5 0.13 (4)	10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03") CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES			
JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG																																																																																																																									
E	405	0	405	0	5-8	1-8																																																																																																																									
C	130	0	130	0	1-8	1-8																																																																																																																									
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JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL																																																																																																																								
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0																																																																																																																								
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0																																																																																																																								
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0																																																																																																																								
CHORDS				WEBS																																																																																																																											
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	MAX. CSI (LC)																																																																																																																								
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STRUCTURAL COMPONENT ONLY
DWG # TR23080875

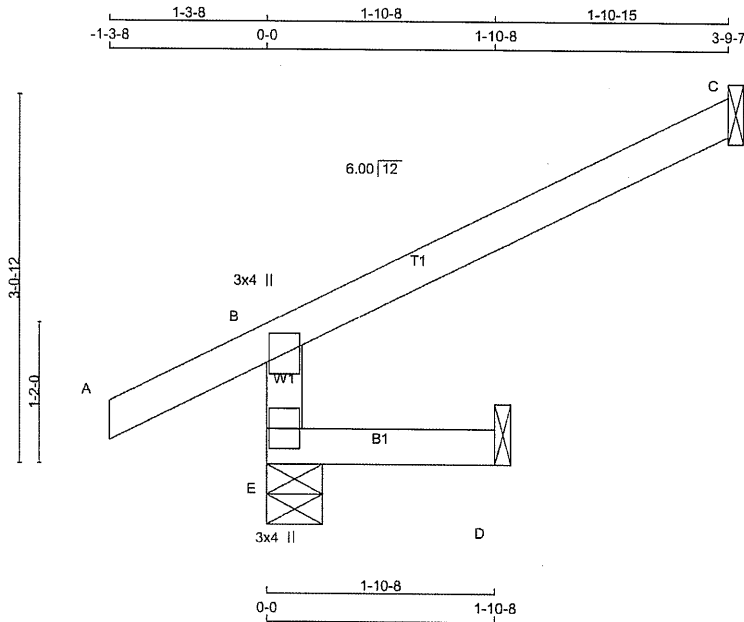
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME 434359	TRUSS NAME J5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:21 2023 Page 1
ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-C3qd64qUfjRw5Pc8qmh?U5pikws14NHoQBBUzykes0



Scale = 1:18.3

TOTAL WEIGHT = 2 X 10 = 19 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	361	0	361	0	0	5-8	1-8
C	130	0	130	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
E-B	-342 / 0	0.0	0.0 0.01 (4)	7.81	
A-B	0 / 28	-91.8	-91.8 0.13 (5)	10.00	
B-C	-19 / 0	-91.8	-91.8 0.22 (1)	6.25	
E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080877

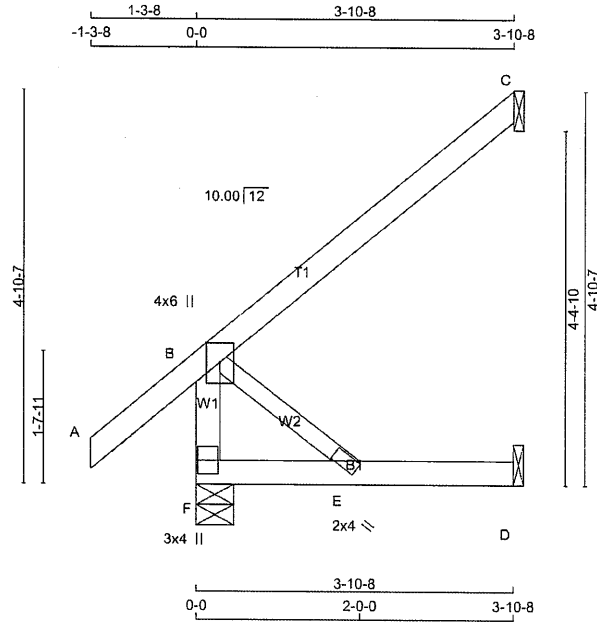
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	J6	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:22 2023 Page 1
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Scale = 1:27.3

TOTAL WEIGHT = 2 X 15 = 31 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	Edge
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQD
JT	VERT	JT	HORZ	BRG	BRG
F	341	0	341	0	0
C	178	0	178	0	0
D	36	0	40	0	0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
F	239	170/0	0/0	0/0	0/0	0/0	69/0	0/0
C	122	99/0	0/0	0/0	0/0	0/0	23/0	0/0
D	29	0/0	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-B	-305/0	0.0	0.0 0.03 (1)	B-E	0/0
A-B	0/41	-91.8	-91.8 0.14 (5)		
B-C	0/0	-91.8	-91.8 0.23 (1)		
F-E	0/0	-18.5	-18.5 0.08 (4)		
E-D	0/0	-18.5	-18.5 0.08 (4)		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080878

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

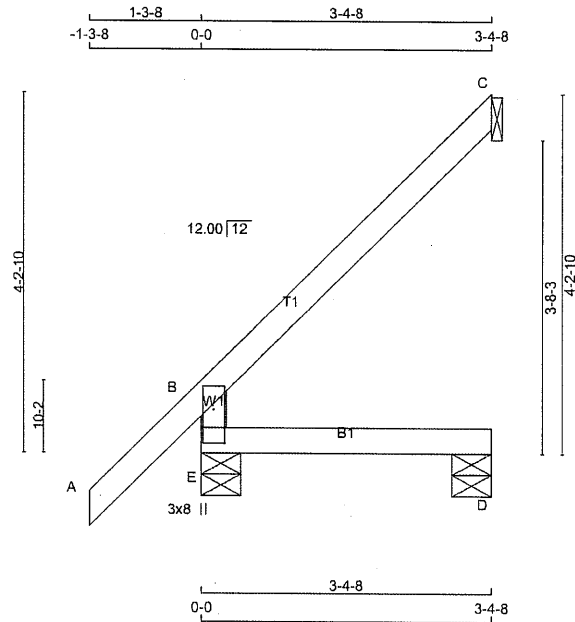
RECEIVED
Per: joshua.nabua

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
434359	J8	15	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:38:25 2023 Page 1
ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-4q48ySt jxxMa0ww3bmFAKFWRLHzzuMsi29Ockykery



Scale = 1:25.9

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	TMBMV1+p	MT20	3.0	8.0	4.75	1.50

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	358	358	0	0
C	116	116	0	0
D	26	29	0	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	251	179 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
C	80	65 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
E-B	-321 / 0	0.0	0.0	0.04 (4)	7.81		
A-B	0 / 45	-91.8	-91.8	0.14 (5)	10.00		
B-C	-27 / 0	-91.8	-91.8	0.18 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 15 X 12 = 183 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.18/1.00 (B-C:1), BC=0.04/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080880

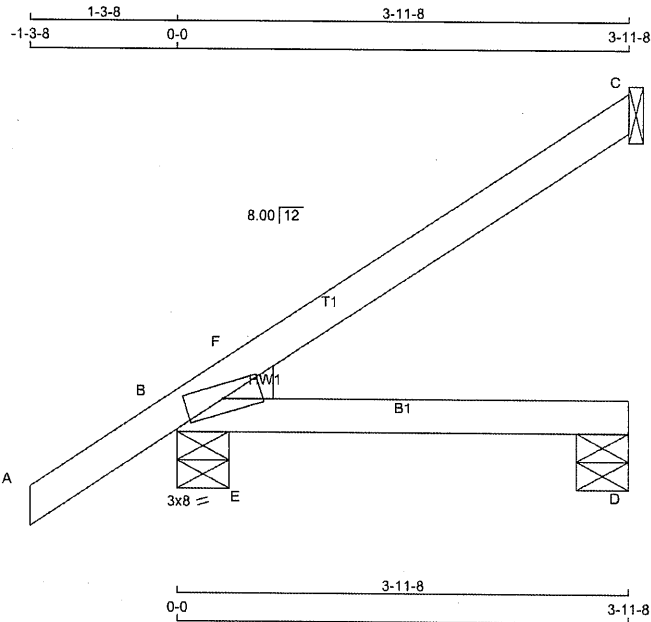
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434359	J9	3	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:19.5

TOTAL WEIGHT = 3 X 12 = 37 lb

LUMBER

N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - C	2x4	DRY	No.2		
B - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	154	0	154	0	1-8	1-8	
B	344	0	344	0	5-8	1-8	2x4 L
D	64	0	64	0	5-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE	PERM. LIVE	WIND			
C	107	83/0	0/0	0/0	0/0	0/0	24/0	0/0
B	241	172/0	0/0	0/0	0/0	0/0	70/0	0/0
D	48	19/0	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS		
MAX. FACTORED		FACTORED			MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	CSI (LC)	FR-TO		
A-B	0 / 34	-91.8	-91.8	0.14 (5)	10.00	E-F	-174 / 0
B-F	-17 / 0	-91.8	-91.8	0.04 (4)	6.25		0.00 (1)
F-C	0 / 3	-91.8	-91.8	0.18 (1)	10.00		
B-E	0 / 0	-18.5	-18.5	0.15 (1)	10.00		
E-D	0 / 0	-18.5	-18.5	0.15 (1)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.18/1.00 (C-F:1), BC=0.15/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.14/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL = 0.03 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080881

CITY OF RICHMOND HILL
BUILDING DIVISION

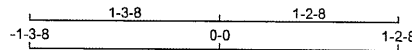
05/01/2024

RECEIVED

Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434360	J20	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:15.1

LUMBER					
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	Edge
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	265	0	265	0	5-8
C	-17	0	0	-66	1-8
D	11	0	13	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	184	142 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0
C	-12	0 / -40	0 / 0	0 / 0	0 / 0	0 / -2	0 / 0
D	9	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
F-B	-254 / 0	B-E	0 / 0
A-B	0 / 38		
B-C	-43 / 0		
F-E	0 / 0		
E-D	0 / 0		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.01/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080898

CITY OF RICHMOND HILL
BUILDING DIVISION

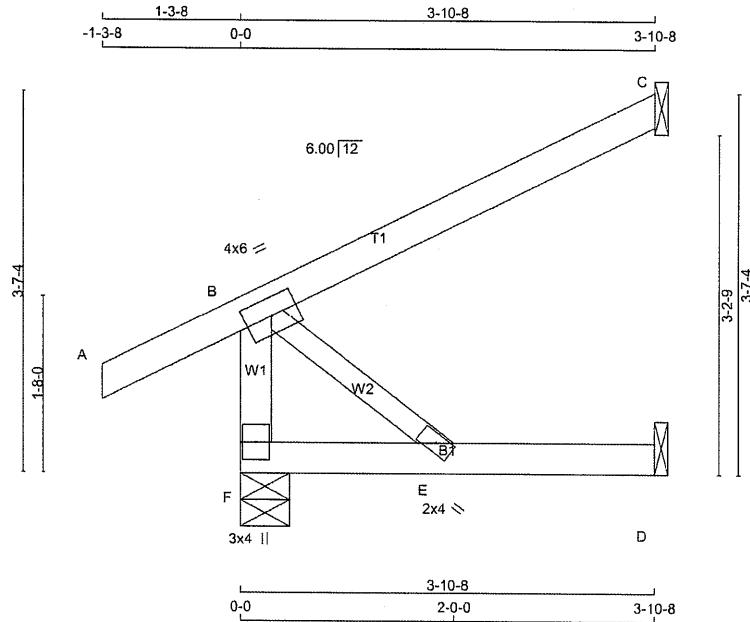
05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME 434586	TRUSS NAME J30	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:20.8

TOTAL WEIGHT = 3 X 14 = 43 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW-w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	338	0	338	0	0	5-8	1-8
C	178	0	178	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	MAX./MIN.	COMPONENT REACTIONS			
F	237	169 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
F-B	-302 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0
A-B	0 / 28	-91.8	-91.8	0.14 (5)	10.00		0.00 (1)
B-C	0 / 0	-91.8	-91.8	0.23 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
E-D	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.08/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080908

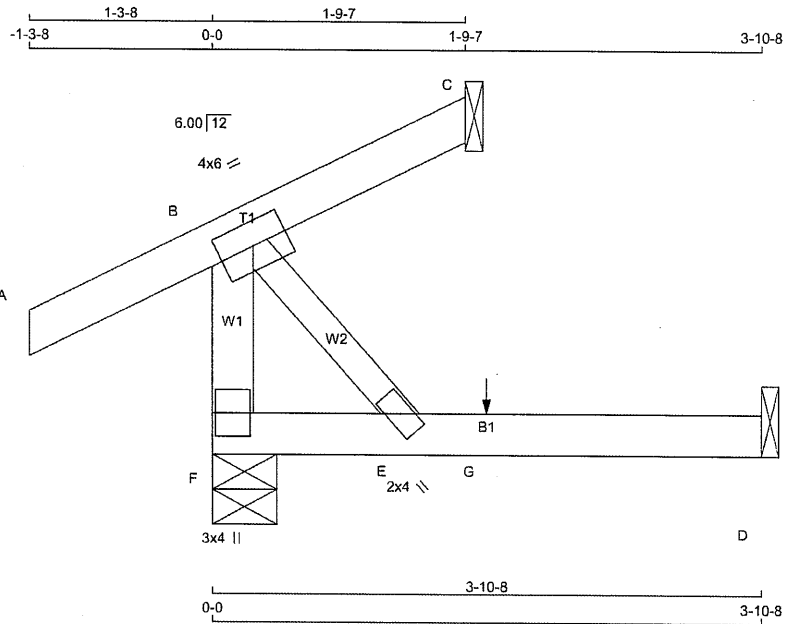
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	J31	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:15.8

TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	JT	HORZ	IN-SX	IN-SX
F	290	0	290	0	1-8
C	35	0	35	0	1-8
D	36	0	40	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS				
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
F	203	142 / 0	0 / 0	0 / 0	0 / 0	62 / 0
C	24	19 / -26	0 / 0	0 / 0	0 / 0	5 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
F-B	-254 / 0	0.0	0.0 0.03 (1)	B-E	0 / 0
A-B	0 / 28	-91.8	-91.8 0.14 (5)		
B-C	-21 / 0	-91.8	-91.8 0.13 (5)		
F-E	0 / 0	-18.5	-18.5 0.07 (4)		
E-G	0 / 0	-18.5	-18.5 0.08 (4)		
G-D	0 / 0	-18.5	-18.5 0.08 (4)		

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		TYPE	HEEL	CONN.
JT	LOC.	LC1	MAX- MAX+	FRONT VERT	TOTAL	C1
G	1-11-4	1	1			

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (A-B:5), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.10/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)			
MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

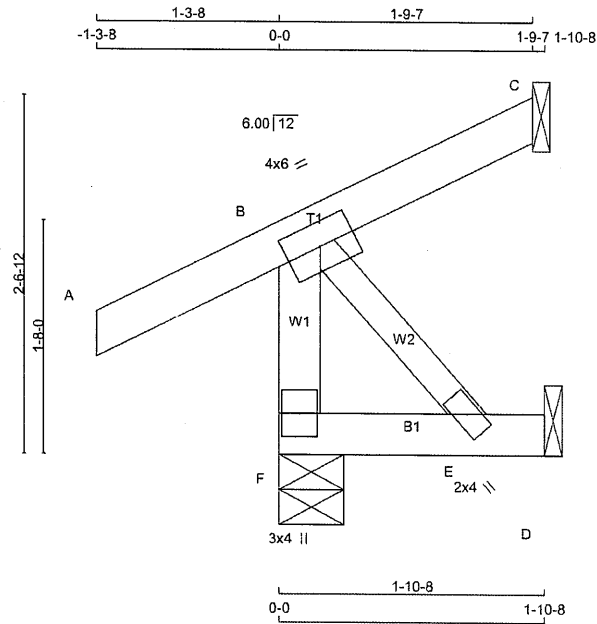


STRUCTURAL COMPONENT ONLY
DWG # TR23080909

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	J32	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:15.8

TOTAL WEIGHT = 2 X 9 = 18 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	JT	HORZ	IN-SX	IN-SX
F	271	0	271	0	1-8
C	35	0	35	0	1-8
D	17	0	19	0	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	189	142 / 0	0 / 0	0 / 0	0 / 0	47 / 0
C	24	19 / -26	0 / 0	0 / 0	0 / 0	5 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED
FR-TO					FR-TO		
F-B	-254 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 28	-91.8	-91.8	0.12 (1)			
B-C	-21 / 0	-91.8	-91.8	0.11 (1)			
F-E	0 / 0	-18.5	-18.5	0.02 (4)			
E-D	0 / 0	-18.5	-18.5	0.02 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080910

CITY OF RICHMOND HILL
BUILDING DIVISION

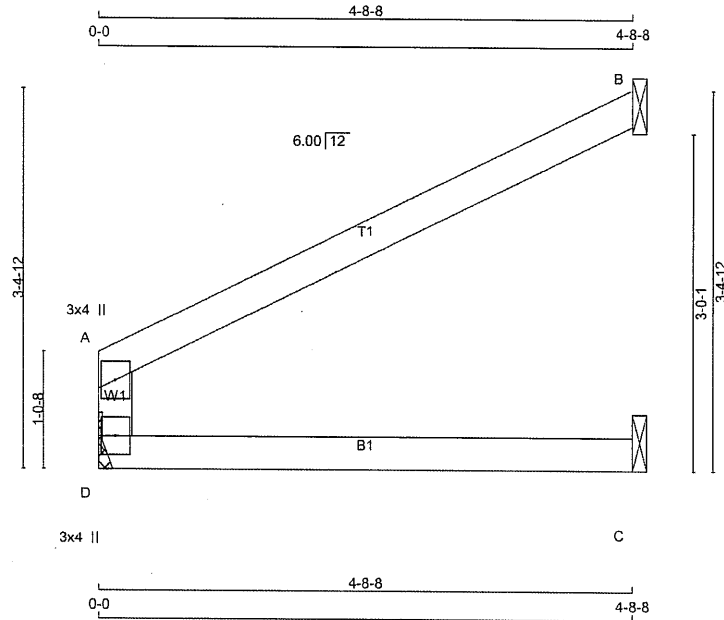
05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME 434586	TRUSS NAME J33	QUANTITY 6	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:19.7

TOTAL WEIGHT = 6 X 12 = 73 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

NOTES-

- (1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	260	0	260	0
B	197	0	197	0
C	63	0	63	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	184	120 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
B	136	107 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
C	48	13 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE	FORCE
	(LBS)	(LBS)
FR-TO	FROM	TO
D-A	-235 / 0	0.0 0.0 0.10 (1) 7.81
A-B	-9 / 0	-91.8 -91.8 0.28 (1) 10.00
D-C	0 / 0	-18.5 -18.5 0.14 (1) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
DL	= 6.0 PSF	
BOT CH.	LL	= 0.0 PSF
DL	= 7.4 PSF	
TOTAL LOAD	= 39.0 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.28/1.00 (A-B:1), BC=0.14/1.00 (C-D:1),
WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (D) (INPUT = 0.90)
JSI METAL = 0.07 (A) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080911

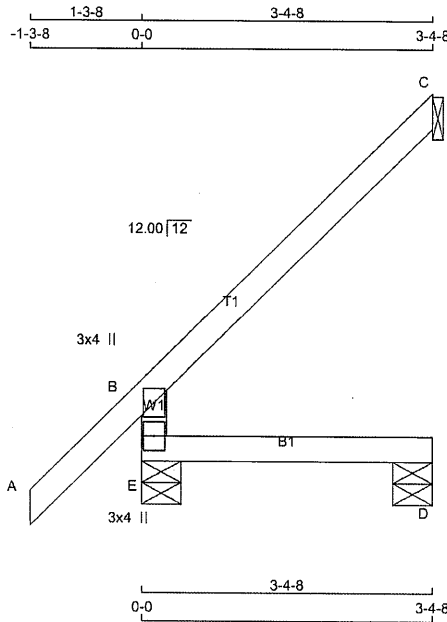
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME 434586	TRUSS NAME J34	QUANTITY 15	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:25.9

TOTAL WEIGHT = 15 X 12 = 185 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	358	0	358	0	5-8	1-8
C	116	0	116	0	1-8	1-8
D	26	0	29	0	5-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE SNOW		MAX / MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		VERT	HORZ	LIVE	PERM. LIVE			
E	250	179 / 0	0 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
C	80	65 / 0	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO					
E-B	-321 / 0	0.0	0.0	0.04 (4)	7.81			
A-B	0 / 45	-91.8	-91.8	0.14 (5)	10.00			
B-C	-27 / 0	-91.8	-91.8	0.18 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.18/1.00 (B-C:1), BC=0.04/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080912

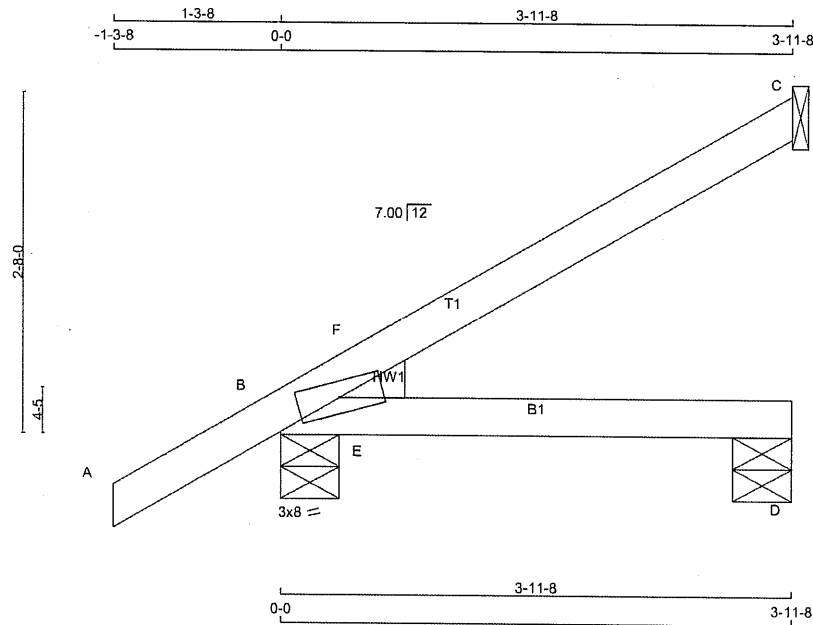
CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434586	J35	3	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:17.3

TOTAL WEIGHT = 3 X 12 = 37 lb

LUMBER

N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - C	2x4	DRY	No.2		
B - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBH1-m	MT20	3.0	8.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	
JT	153	0	153	0	1-8	1-8	
C	343	0	343	0	5-8	1-8	2x4 L
B	65	0	65	0	5-8	1-8	
D							

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

	1ST CASE	MAX / MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	82 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
C	106	171 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
B	241	19 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	48						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. FORCE (LBS)	VERT. LOAD (PLF)
FR-TO								
A-B	0 / 30	-91.8	-91.8	0.14 (5)	10.00	E-F	-154 / 2	0.00 (1)
B-F	-16 / 0	-91.8	-91.8	0.04 (4)	6.25			
F-C	0 / 3	-91.8	-91.8	0.17 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.15 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.15 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.17/1.00 (C-F:1), BC=0.15/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SSI=0.13/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 0.95)



STRUCTURAL COMPONENT ONLY
DWG # TR23080913

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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Per: joshua.nabua

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

CITY OF RICHMOND HILL
BUILDING DIVISION

05/01/2024

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TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MAXIMUM NUMBER OF TOE NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

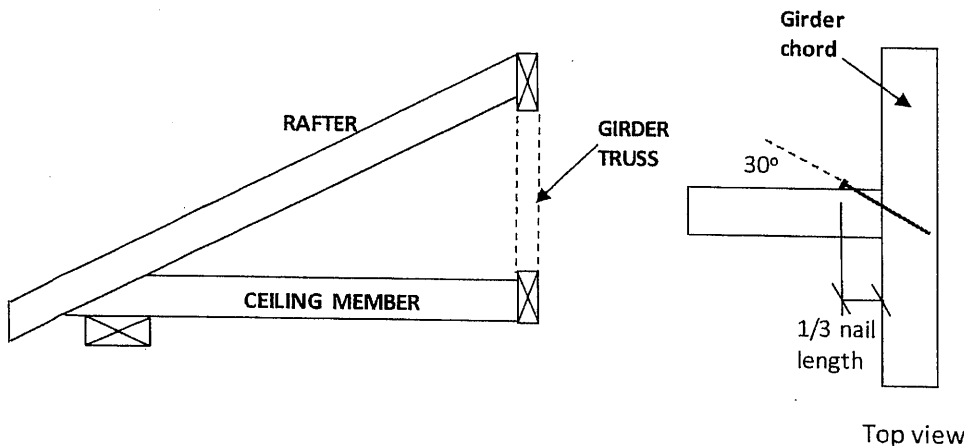


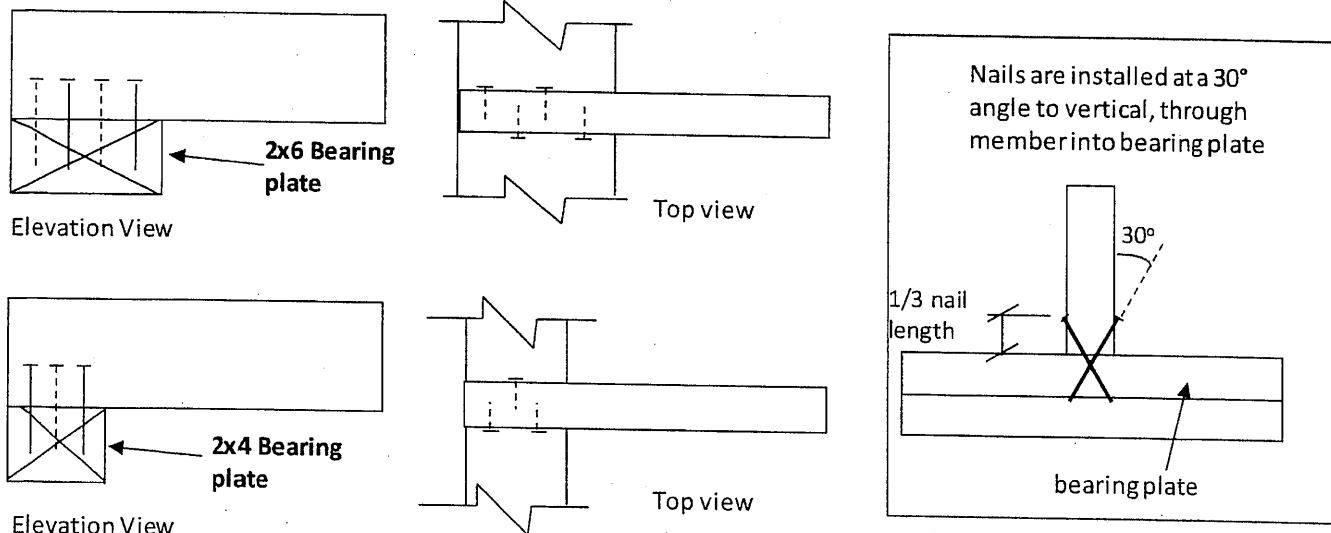
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

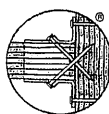
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

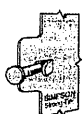
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

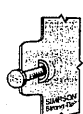
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

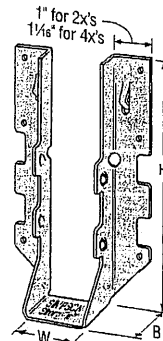
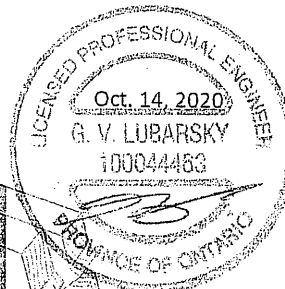
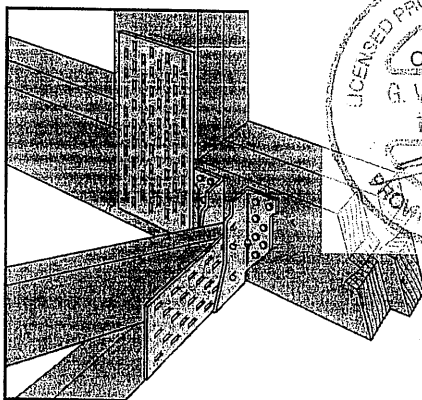


Double-Shear Nailing Side View; Do not bend tab

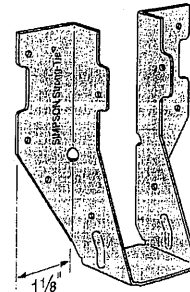


Dome Double-Shear Nailing Side View (available on some models)

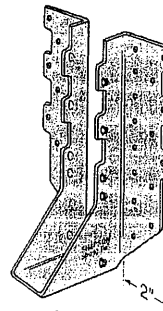
Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



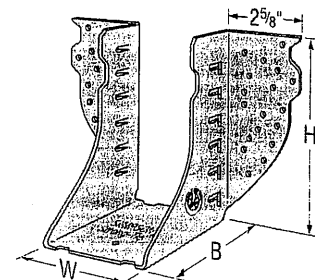
LUS28



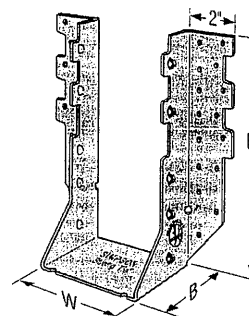
LU26L



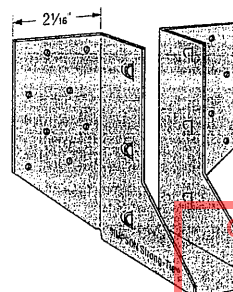
HUS210 (HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

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TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

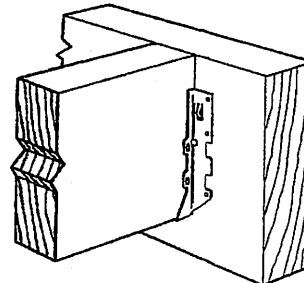
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

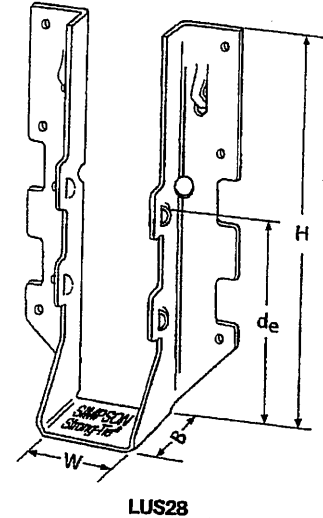
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

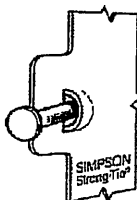


Typical LUS
Installation



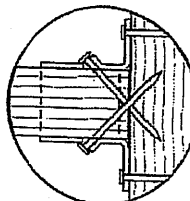
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1 1/8	3 1/8	1 3/4	1 1/8	(4) 10d	(2) 10d	(K _u =1.15)	(K _n =1.00)	(K _u =1.15)	(K _n =1.00)
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	710	1630	645	1155
LUS26	18	1 1/8	4 1/8	1 3/4	3 1/8	(4) 10d	(4) 10d	835	2020	590	1435
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1420	2170	1290	1630
LUS26-3	18	4 1/8	4 1/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS28	18	1 1/8	6 1/8	1 3/4	3 3/8	(6) 10d	(6) 10d	1720	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS28-3	18	4 1/8	6 1/8	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

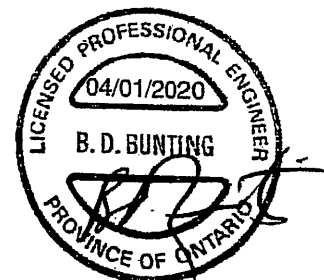


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

CITY OF RICHMOND HILL
BUILDING DIVISION

(800) 999-5099

strongtie.com

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

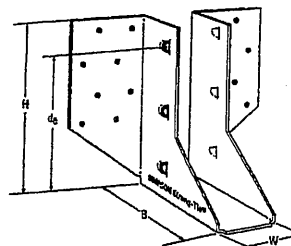
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

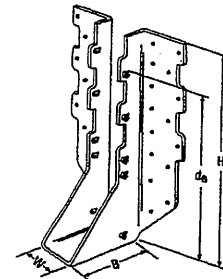
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

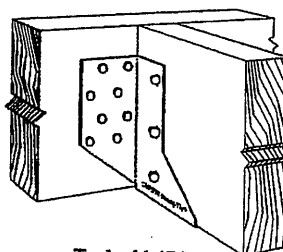
- See current catalogue for options



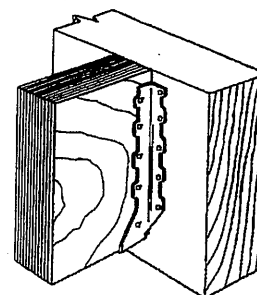
LJS26DS



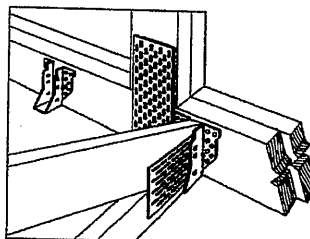
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



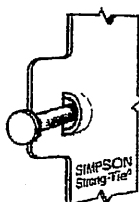
Typical HUS
Installation



Typical HUS installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

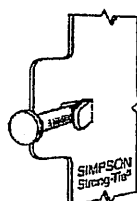
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/8	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/8	7 3/16	3	6 3/16	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 1/8	9 3/16	3	7 3/16	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

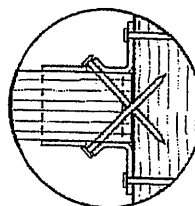


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

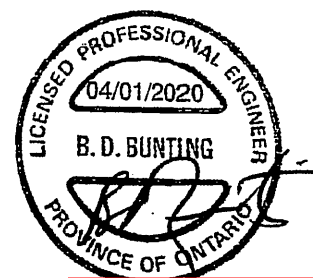
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

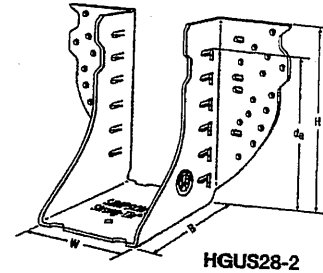
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

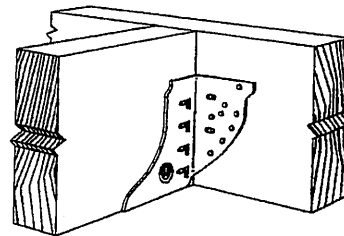
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

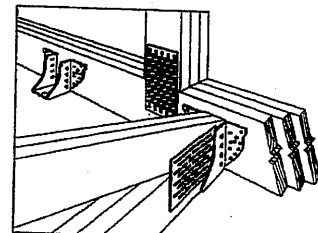
- See current catalogue for options



HGUS28-2



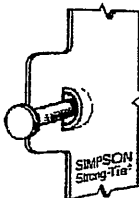
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

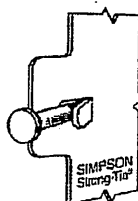
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
HGUS26	12	1½	5½	5	4¾	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_o is the distance from the seat of the hanger to the highest joist nail.

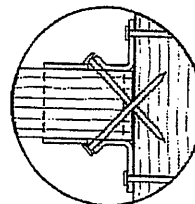


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

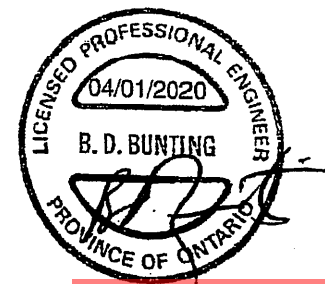
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

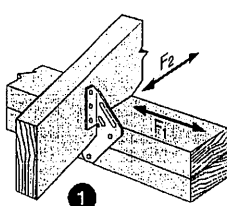
**CITY OF RICHMOND HILL
BUILDING DIVISION**

(800) 999-5099
strongtie.com

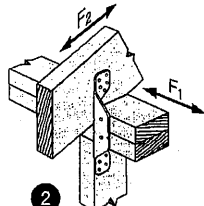
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H/TSP

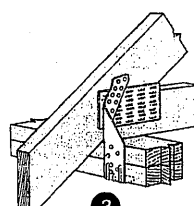
Seismic and Hurricane Ties (cont.)



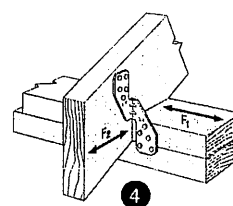
1 H1 Installation



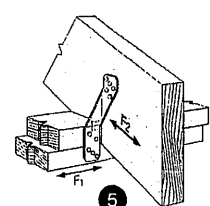
2 H2A Installation



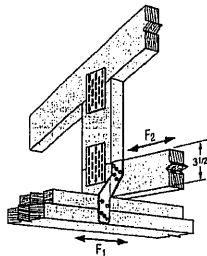
3 TSP Installation



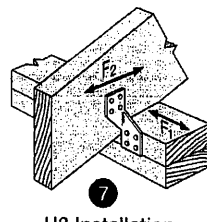
4 H2.5A Installation
(Nails into both top plates)



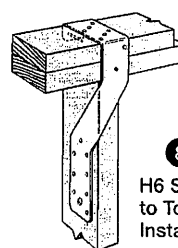
5 H2.5T Installation
(Nails into both top plates)



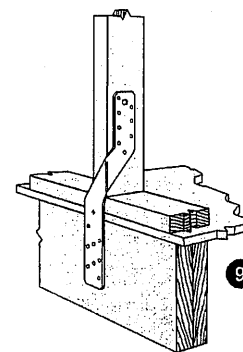
6 H2.5T Installation



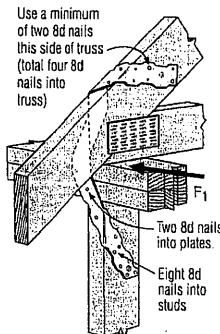
7 H3 Installation
(Nails into upper top plate)



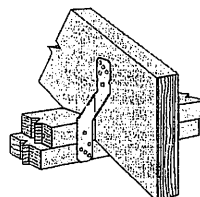
8 H6 Stud
to Top Plate
Installation



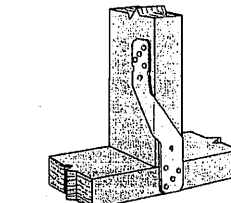
9 H6 Stud to
Band Joist
Installation



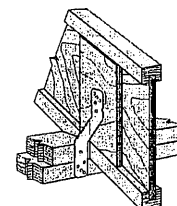
10 H7Z Installation



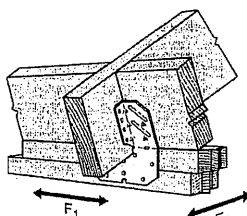
11 H8 Attaching
Rafter to Double
Top Plates



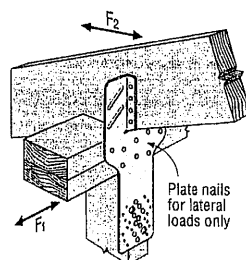
12 H8 attaching Stud to Sill
(4) 8d into plate, (5) 8d into stud



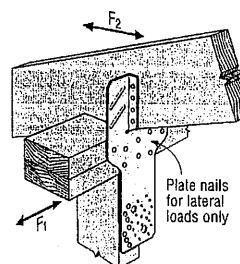
13 H8 attaching
I-Joist to Double
Top Plates



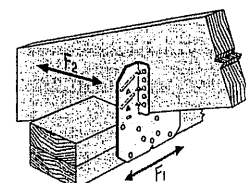
14 H10A Field-Bent
Installation



15 H10S Installation

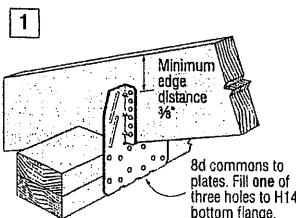


16 H10S Installation
with Stud Offset

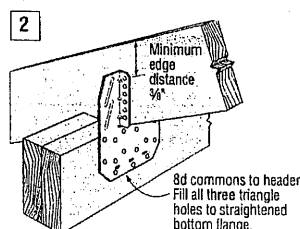


17 H10A
Installation

H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

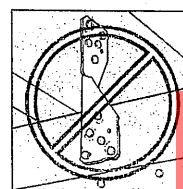


18 H14 Installation to
Double Top Plates



19 H14 Installation
to Double 2x Header

Avoid a Misinstallation



Do not make
new holes or
overdrive nails.

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05/01/2024

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Straps and Ties

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

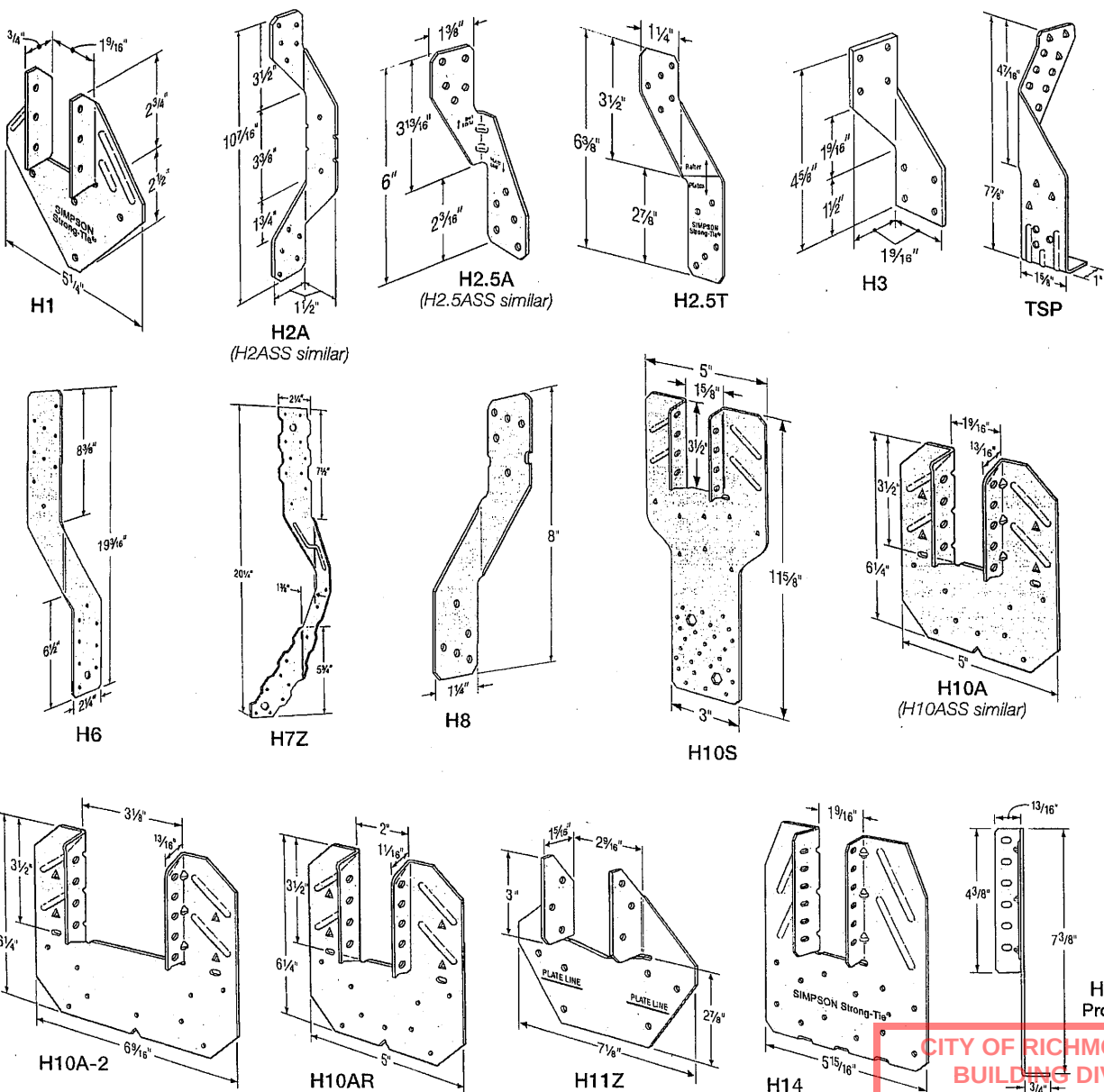
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit strongtie.com.

Installation:

- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



Straps and Ties

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

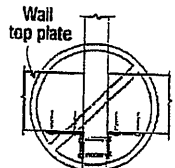
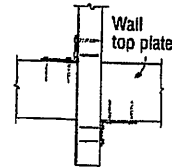
• Nails: $8d = 0.131"$ dia. x $2\frac{1}{2}"$ long common wire, $8d \times 1\frac{1}{2}" = 0.131" \times 1\frac{1}{2}"$ long, $10d \times 1\frac{1}{2}" = 0.146" \times 1\frac{1}{2}"$ long

• H1 can be installed with flanges facing outwards

• Hurricane ties do not replace solid blocking

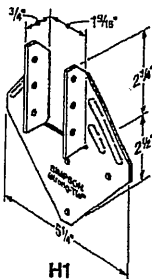
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0 .

Hurricane Tie Installations to Achieve Twice the Load (Top View)

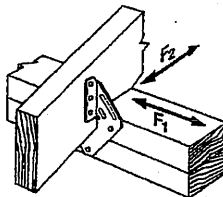


Install diagonally across from each other for minimum 2x truss.

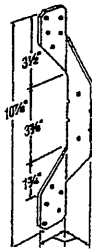
Nailing into both sides of a single ply 2x truss may cause the wood to split.



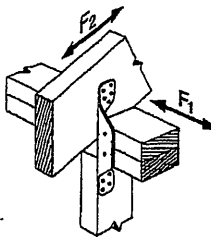
H1



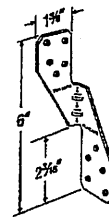
H1 Installation



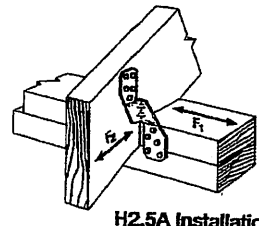
H2A



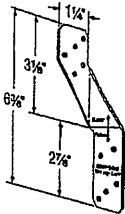
H2A Installation



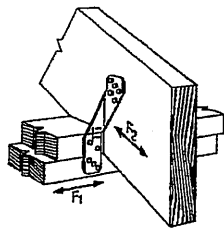
H2.5A



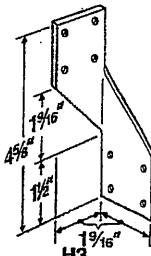
H2.5A Installation



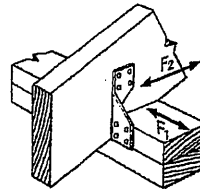
H2.5T



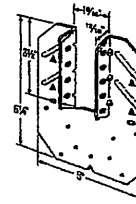
H2.5T Installation
(Nails into both top plates)



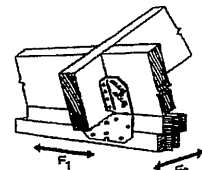
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
										(K _p =1.15)
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.

2. Factored resistances are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

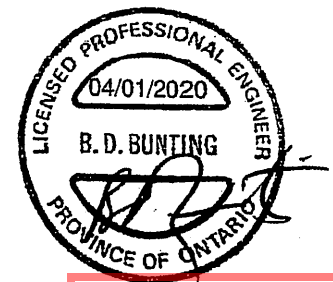


LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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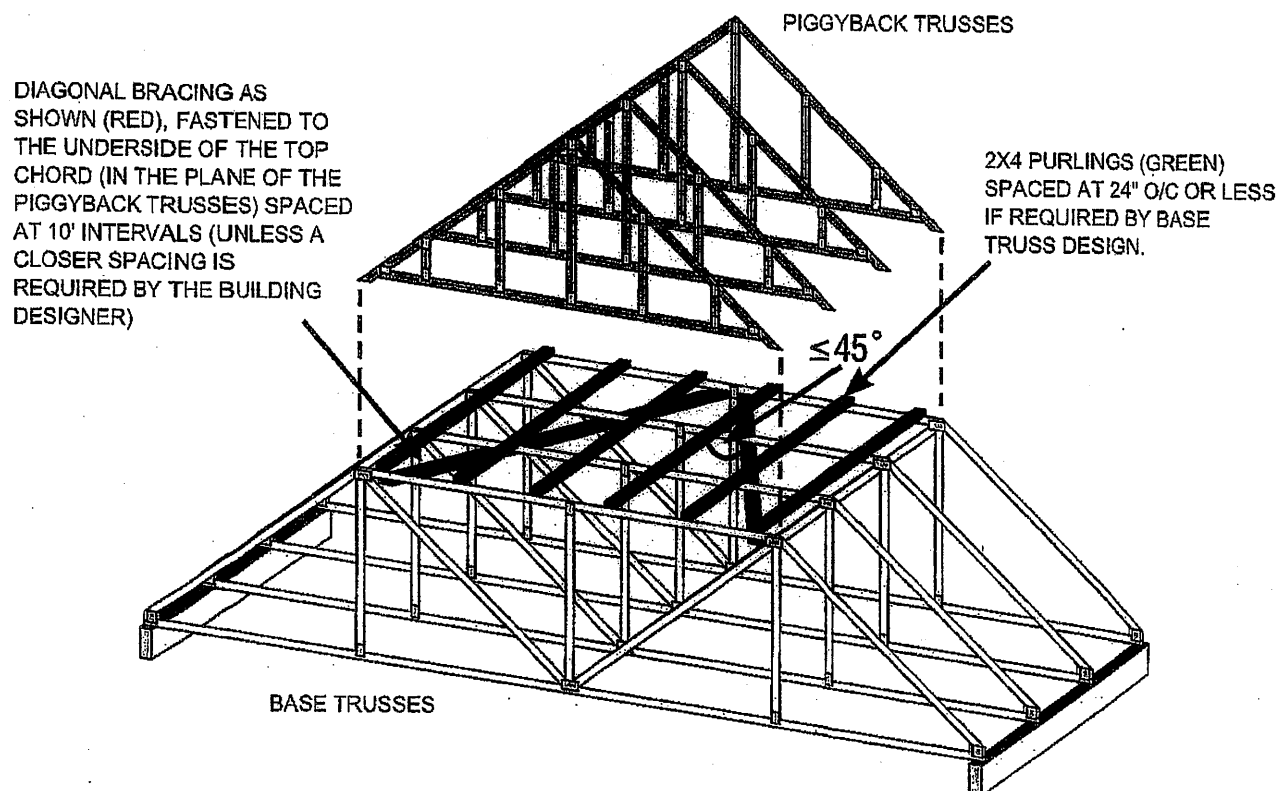
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

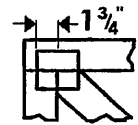
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05/02/2024

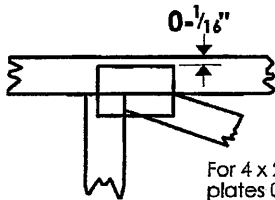
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Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

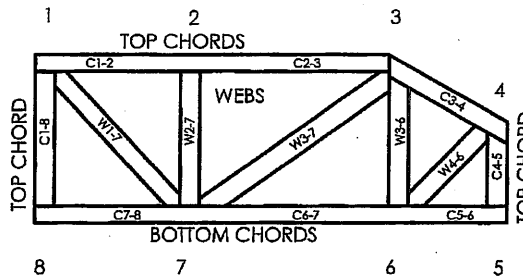


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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