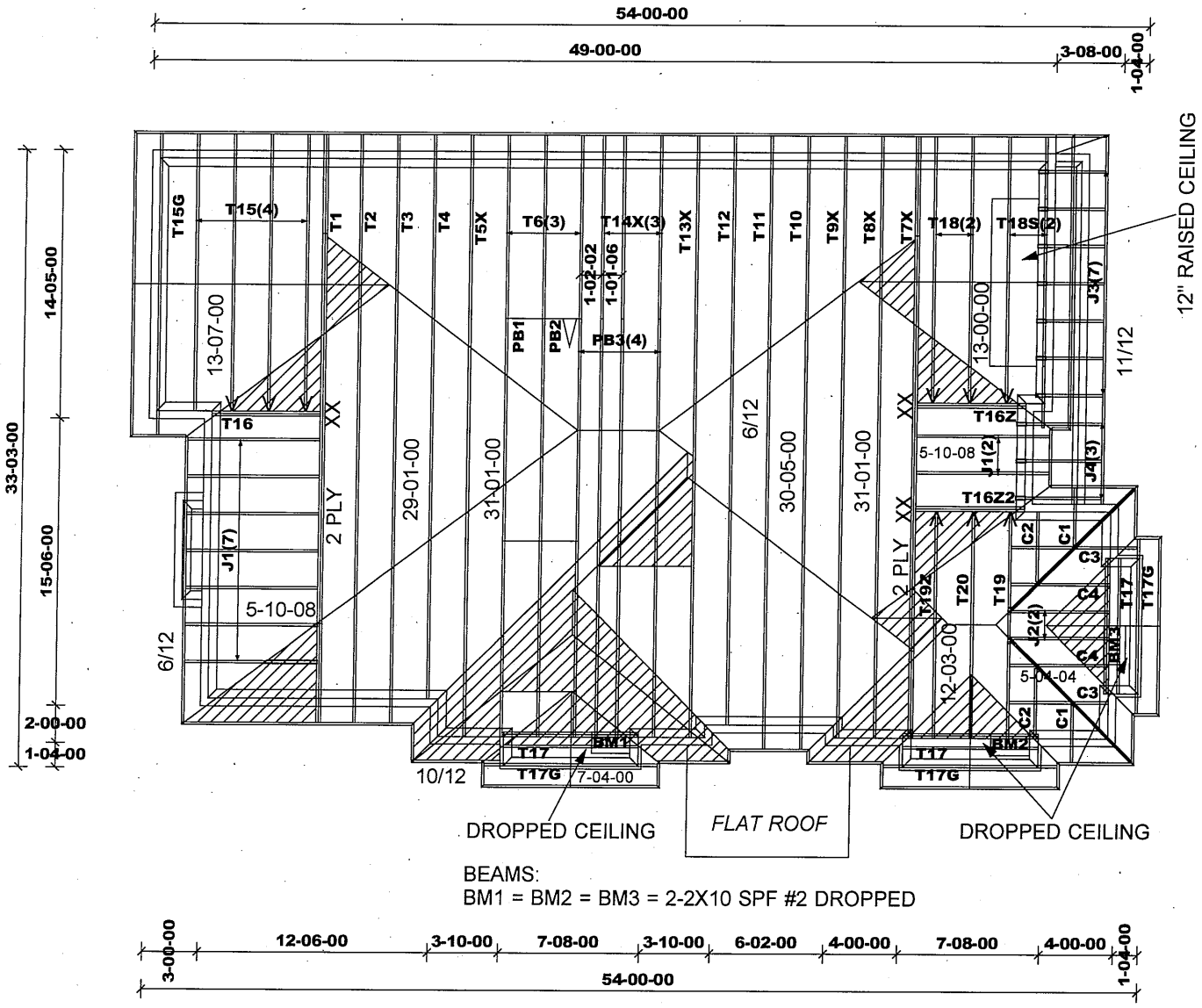
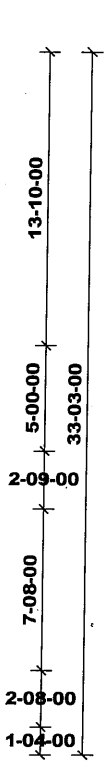


ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH OBC 2012 (2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

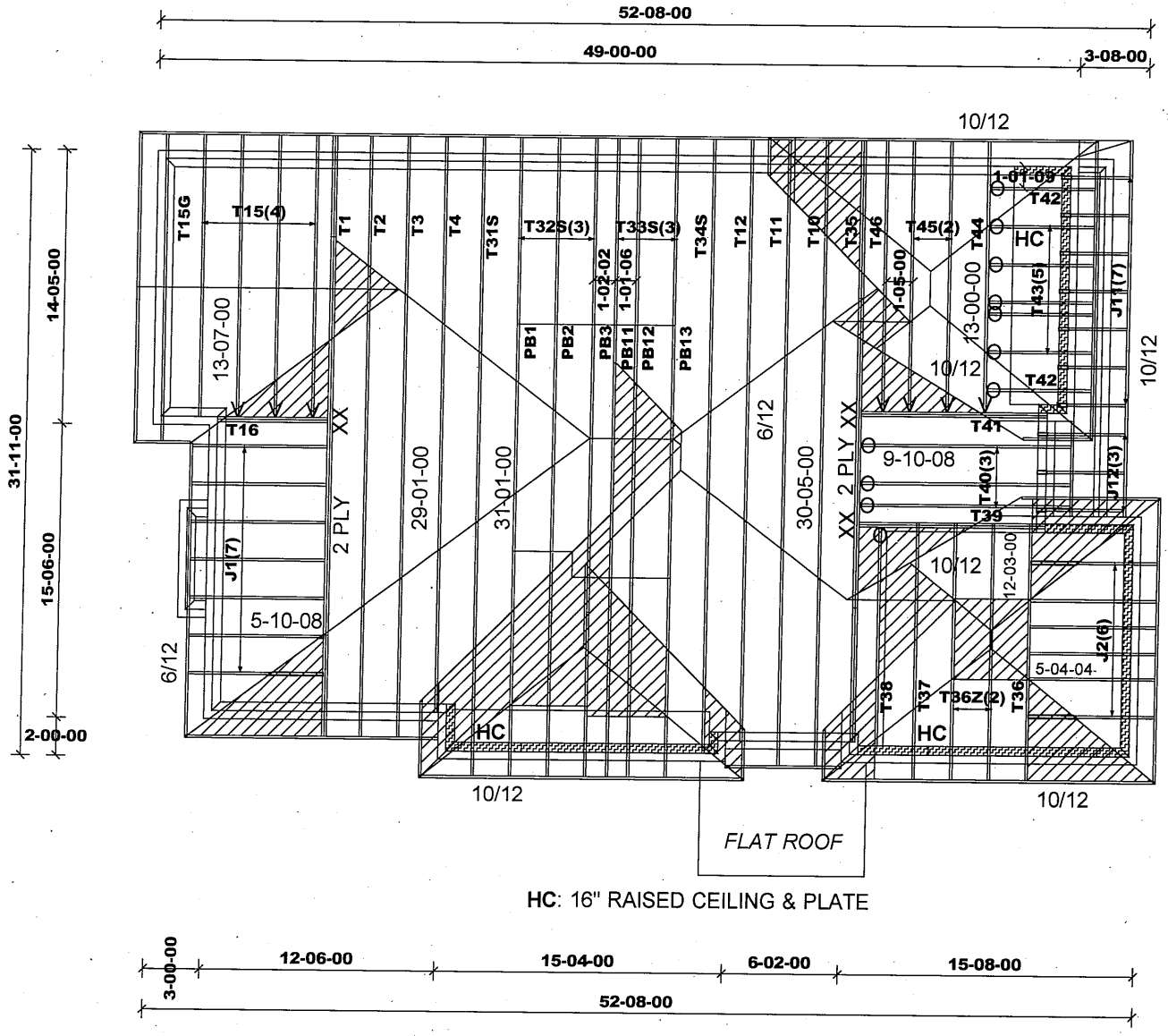
BEAMS:
BM1 = BM2 = BM3 = 2-2X10 SPF #2 DROPPED

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

M15496

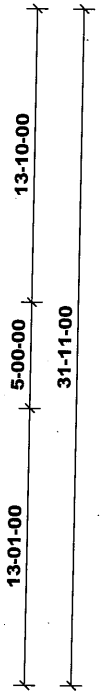
TAMARACK ROOF TRUSSES INC.	Job Track: 53256	Builder / Location: GREENPARK HOMES / RICHMOND HILL	Model / Elevation: VILLA 10 / 1
	Plan Log: 207153	Project: TRINIGROUP DEVELOPMENTS	
	Layout ID: 432495	Date: 2023-06-14 Sales: Rick DiCiano Designer: LC	

Mitek ver 8.5.3.233



HC: 16" RAISED CEILING & PLATE

ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

1X

M15496

TAMARACK ROOF TRUSSES INC.

Job Track:	53256
Plan Log:	207153
Layout ID:	432499

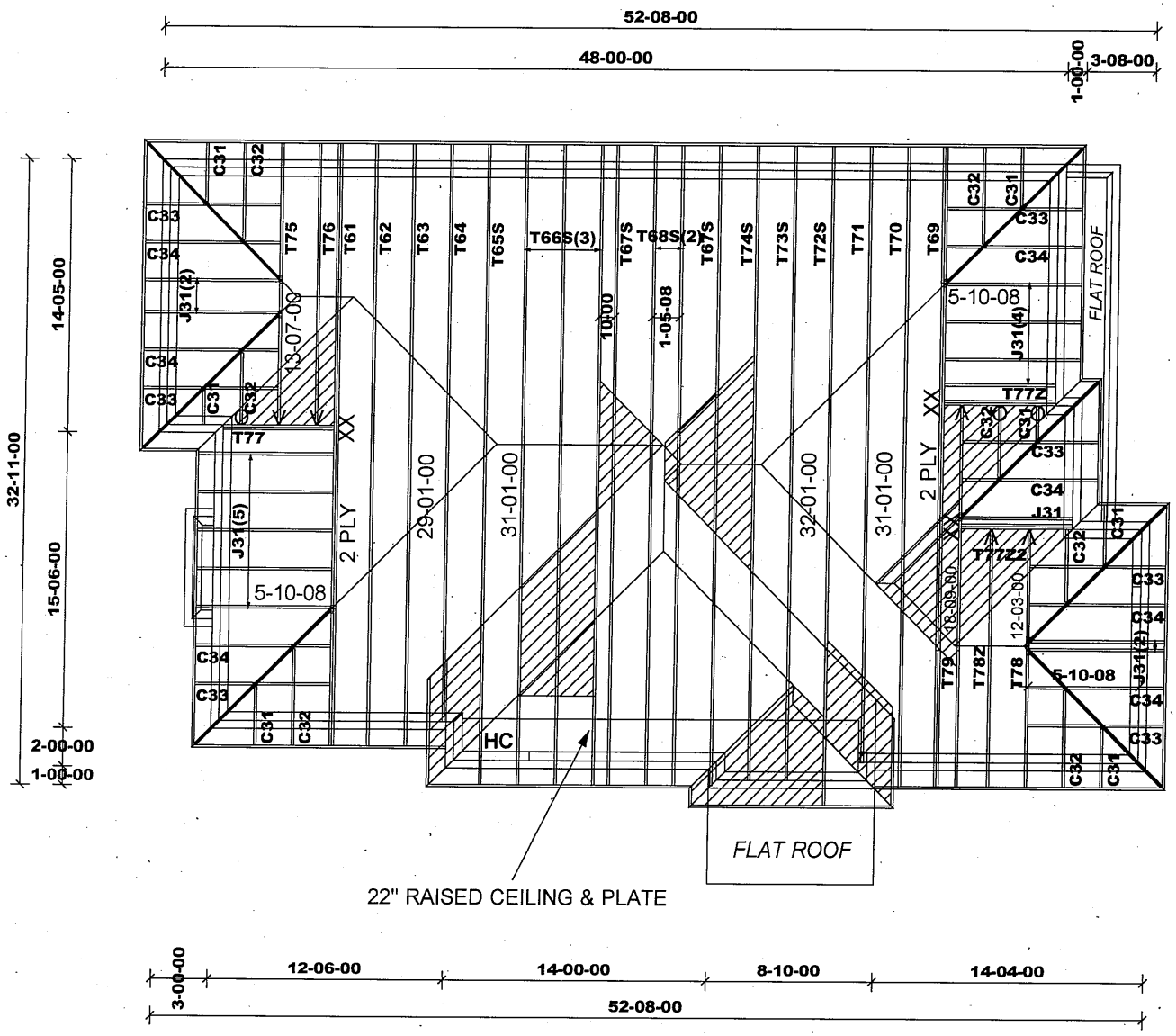
Builder / Location: GREENPARK HOMES / RICHMOND HILL

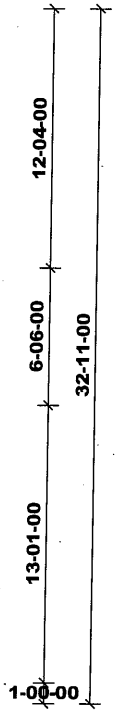
Model / Elevation: VILLA 10 / 2

Project: TRINIGROUP DEVELOPMENTS

Date: 2023-06-14 Sales: Rick DiCiano Designer: LC

Mitek ver 8.5.3.233





ALL ROOF PITCHES ARE 6/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

M15496

TAMARACK ROOF TRUSSES INC.

Job Track: 53256	Builder / Location: GREENPARK HOMES / RICHMOND HILL		Model / Elevation: VILLA 10 / 3
Plan Log: 207153	Project: TRINIGROUP DEVELOPMENTS		
Layout ID: 432501	Date: 2023-06-14	Sales: Rick DiCiano	Designer: LC/TT

Mitek ver 8.5.3.233

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207153
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432495
Location:	RICHMOND HILL	Ref #	
Model:	VILLA 10	Page:	1 of 3
Lot #:		Date:	06-14-2023
Elevation:	1	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	8 /12	**29-01-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	271.08 168.67		
	1	T2 Hip	8 /12	**29-01-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	116.29 73.33		
	1	T3 Hip	8 /12	**29-01-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	129.44 81.67		
	1	T4 Hip	8 /12	**29-01-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	128.23 81.17		
	1	T5X Hip	8 /12	**31-01-00**	8-01-04	2 x 4	1-03-08	1-04-13 0-13	133.48 83.50		
	3	T6 Piggyback Base	8 /12	**31-01-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-07-11	413.35 264.50		
	1 2-ply	T7X Hip Girder	8 /12	**31-01-00**	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 11-08	285.25 179.00		
	1	T8X Hip	8 /12	**31-01-00**	5-01-04	2 x 4	1-03-08	1-04-13 11-08	122.72 77.00		
	1	T9X Hip	8 /12	**31-01-00**	6-01-04	2 x 4	1-03-08	1-04-13 11-08	133.21 84.33		
	1	T10 Hip	8 /12	**30-05-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	132.01 84.17		
	1	T11 Hip	8 /12	**30-05-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.36 86.00		
	1	T12 Hip	8 /12	**30-05-00**	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.42 86.67		
	1	T13X Hip	8 /12	**31-01-00**	10-01-04	2 x 4	1-03-08	1-04-13 11-08	151.48 94.67		
	3	T14X Hip	8 /12	**31-01-00**	7-01-04	2 x 4	1-03-08	1-04-13 11-08	395.65 252.50		

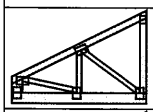
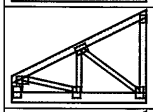
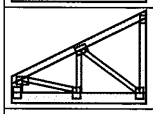
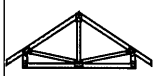
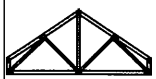
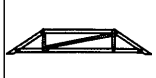
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	VILLA 10
Lot #:	
Elevation:	1

Job Track:	53256
PlanLog:	207153
Layout ID:	432495
Ref #	
Page:	2 of 3
Date:	06-14-2023
Designer:	Leo Chen
Sales Rep:	Rick DiCiano

Roof Trusses

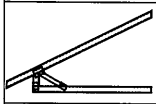
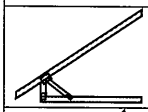
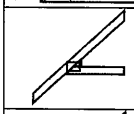
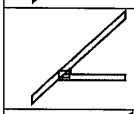
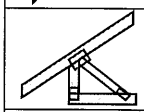
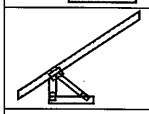
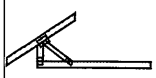
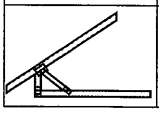
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	T15 Common	8 /12	13-07-00	5-11-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	235.77 148.00		
	1	T15G GABLE	8 /12	13-07-00	5-11-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	57.71 37.00		
	1 2-ply	T16 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T16Z Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1 2-ply	T16Z2 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	3	T17 Common	8 /12	7-04-00	3-08-02	2 x 4	1-00-08 1-00-08	1-02-13 1-02-13	94.37 62.00		
	3	T17G GABLE	8 /12	7-04-00	3-08-02	2 x 4	1-00-08 1-00-08	1-02-13 1-02-13	86.6 56.00		
	2	T18 Common	8 /12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	113.51 72.67		
	2	T18S Roof Special	8 /12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	121.35 79.00		
	1	T19 Hip Girder	8 /12	12-03-00	4-11-10	2 x 4		1-04-13 1-04-13	52.24 34.17		
	1	T19Z Hip	8 /12	12-03-00	4-11-10	2 x 4		1-04-13 1-04-13	52.24 34.17		
	1	T20 Common	8 /12	12-03-00	5-05-13	2 x 4		1-04-13 1-04-13	50.4 32.17		
	1	PB1 Piggyback	8 /12	11-11-11	2-00-00	2 x 4			33.68 21.67		
	1	PB2 Piggyback	8 /12	11-11-11	3-00-00	2 x 4			34.15 22.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207153
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432495
Location:	RICHMOND HILL	Ref #	
Model:	VILLA 10	Page:	3 of 3
Lot #:		Date:	06-14-2023
Elevation:	1	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	PB3 Piggyback	8 /12	**11-11-11**	3-11-14	2 x 4			123.73 80.00		
	9	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	162.7 105.00		
	2	J2 Jack-Open	8 /12	**5-04-04**	4-11-10	2 x 4	1-03-08	1-04-13 4-11-10	35.77 23.33		
	7	J3 Jack-Open	11 /12	**2-03-08**	3-08-03	2 x 4	1-03-08	5-00 2-06-03	61.77 46.67		
	3	J4 Jack-Open	11 /12	**3-03-08**	4-07-03	2 x 4	1-03-08	5-00 3-05-03	34.53 24.00		
	2	C1 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	18.05 11.33		
	2	C2 Jack-Open	8 /12	**1-10-08**	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.53 15.33		
	2	C3 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-05-00 3-06-13	1-04-13 2-07-02	26.33 18.00		
	2	C4 Jack-Open	8 /12	**5-04-04**	3-11-02	2 x 4	1-03-08	1-04-13 4-11-10	31.47 20.67		

TOTAL # TRUSS= **79** TOTAL BFT OF ALL TRUSSES= **2753.7** BFT. TOTAL WEIGHT OF ALL TRSSES **4314.04** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
9	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **12**

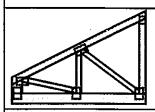
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	VILLA 10
Lot #:	
Elevation:	2

Job Track:	53256
PlanLog:	207153
Layout ID:	432499
Ref #	
Page:	1 of 3
Date:	06-14-2023
Designer:	Leo Chen
Sales Rep:	Rick DiCiano

Roof Trusses

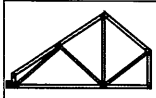
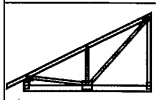
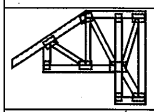
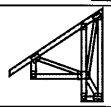
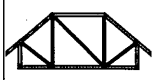
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	8 /12	29-01-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	271.08 168.67		
	1	T2 Hip	8 /12	29-01-00	5-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	116.29 73.33		
	1	T3 Hip	8 /12	29-01-00	6-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	129.44 81.67		
	1	T4 Hip	8 /12	29-01-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	128.23 81.17		
	1	T10 Hip	8 /12	30-05-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	132.01 84.17		
	1	T11 Hip	8 /12	30-05-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.36 86.00		
	1	T12 Hip	8 /12	30-05-00	9-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.42 86.67		
	4	T15 Common	8 /12	13-07-00	5-11-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	235.77 148.00		
	1	T15G GABLE	8 /12	13-07-00	5-11-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	57.71 37.00		
	1 2-ply	T16 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	T31S Roof Special	8 /12	31-01-00	8-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-07-11	152.25 98.33		
	3	T32S Piggyback Base	8 /12	31-01-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-07-11	441.53 283.00		
	3	T33S Piggyback Base	8 /12	31-01-00	7-01-04	2 x 4	1-03-08 1-03-08	1-04-13 1-07-11	474.18 303.50		
	1	T34S Roof Special	8 /12	31-01-00	10-01-04	2 x 4	1-03-08	1-04-13 1-07-11	157.5 100.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207153
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432499
Location:	RICHMOND HILL	Ref #	
Model:	VILLA 10	Page:	2 of 3
Lot #:		Date:	06-14-2023
Elevation:	2	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

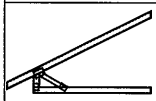
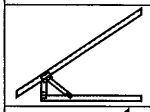
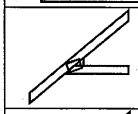
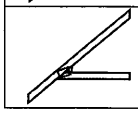
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T35 Hip Girder	8 /12	**30-05-00**	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	339.44 210.67		
	1	T36 Hip Girder	10 /12	**12-03-00**	4-11-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	56.09 37.33		
	2	T36Z Hip	10 /12	**12-03-00**	4-11-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	112.17 74.67		
	1	T37 Hip	10 /12	**12-03-00**	3-07-10	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	55.62 38.67		
	1	T38 Roof Special	8 /12	**11-06-08**	6-04-11	2 x 4		1-03-02 3-02-03	51.23 32.50		
	1 2-ply	T39 Monopitch Girder	0 /12	**9-10-08**	1-04-00	2 x 6 2 x 4		1-04-00 1-04-00	78.16 48.33		
	3	T40 Monopitch	6 /12	**9-10-08**	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	130.25 82.50		
	1 2-ply	T41 Monopitch Girder	6 /12	**9-10-08**	6-01-04	2 x 4 2 x 6	1-03-08	1-02-00 6-01-04	101.66 62.33		
	2	T42 Half Hip	8 /12	**4-03-04**	3-11-00	2 x 4	1-03-08	1-04-13 3-11-00	58.66 40.67		
	5	T43 Monopitch	8 /12	**4-03-04**	5-07-00	2 x 4	1-03-08	1-04-13 5-07-00	157.2 100.83		
	1	T44 Hip Girder	10 /12	**13-00-00**	5-07-00	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	69.96 42.83		
	2	T45 Hip	10 /12	**13-00-00**	6-11-00	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	134.99 88.67		
	1	T46 Hip	10 /12	**13-00-00**	5-11-10	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	64.09 41.00		
	1	PB1 Piggyback	8 /12	**11-11-11**	2-00-00	2 x 4			33.68 21.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207153
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432499
Location:	RICHMOND HILL	Ref #	
Model:	VILLA 10	Page:	3 of 3
Lot #:		Date:	06-14-2023
Elevation:	2	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	PB2 Piggyback	8 /12	**11-11-11**	3-00-00	2 x 4			34.15 22.33		
	1	PB3 Piggyback	8 /12	**11-11-11**	3-11-14	2 x 4			30.93 20.00		
	1	PB11 Piggyback	8 /12	**13-03-11**	1-04-00	2 x 4			38.17 25.17		
	1	PB12 Piggyback	8 /12	**13-03-11**	2-00-14	2 x 4			38.12 24.67		
	1	PB13 Piggyback	8 /12	**13-03-11**	3-04-14	2 x 4			38.43 25.00		
	7	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	126.54 81.67		
	6	J2 Jack-Open	8 /12	**5-04-04**	4-11-10	2 x 4	1-03-08	1-04-13 4-11-10	107.3 70.00		
	7	J11 Jack-Open	10 /12	**2-03-08**	3-04-06	2 x 4	1-03-08	4-13 2-03-12	60.26 42.00		
	3	J12 Jack-Open	10 /12	**3-03-08**	4-02-06	2 x 4	1-03-08	4-13 3-01-12	33.7 24.00		

TOTAL # TRUSS= **77** TOTAL BFT OF ALL TRUSSES= **2927.19** BFT. TOTAL WEIGHT OF ALL TRSSES **4581.97** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
7	Hardware	LJS26DS	
11	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **21**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207153
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432501
Location:	RICHMOND HILL	Ref #	
Model:	VILLA 10	Page:	1 of 3
Lot #:		Date:	06-14-2023
Elevation:	3	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

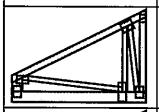
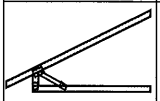
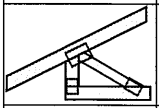
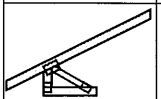
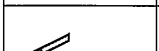
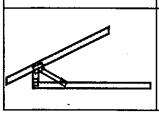
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T61 Hip Girder	6 /12	**29-01-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	268.83 170.00		
	1	T62 Hip	6 /12	**29-01-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	115.4 72.50		
	1	T63 Hip	6 /12	**29-01-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	121.11 77.33		
	1	T64 Hip	6 /12	**29-01-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.21 73.83		
	1	T65S Roof Special	6 /12	**31-01-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	150.93 97.00		
	3	T66S Roof Special	6 /12	**31-01-00**	8-05-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	424.34 268.50		
	2	T67S Hip	6 /12	**31-01-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	280.11 175.33		
	2	T68S Hip	6 /12	**31-01-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	296.56 188.00		
	1 2-ply	T69 Hip Girder	6 /12	**31-01-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	278.2 177.33		
	1	T70 Hip	6 /12	**31-01-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.3 74.50		
	1	T71 Hip	6 /12	**31-01-00**	6-01-04	2 x 4	1-03-08	1-02-00 1-02-00	125.1 79.33		
	1	T72S Roof Special	6 /12	**32-01-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	141.05 90.00		
	1	T73S Roof Special	6 /12	**32-01-00**	8-01-04	2 x 4	1-03-08	1-02-00 8-00	147.56 93.17		
	1	T74S Roof Special	6 /12	**32-01-00**	8-09-12	2 x 4	1-03-08	1-02-00 8-00	143.99 91.83		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207153
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432501
Location:	RICHMOND HILL	Ref #	
Model:	VILLA 10	Page:	2 of 3
Lot #:		Date:	06-14-2023
Elevation:	3	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T75 Hip Girder	6 /12	13-07-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	57.93 38.00		
	1	T76 Common	6 /12	13-07-00	4-06-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	54.08 34.00		
	1 2-ply	T77 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	62.98 40.67		
	1 2-ply	T77Z Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	62.98 40.67		
	1 2-ply	T77Z2 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	62.98 40.67		
	1	T78 Common Girder	6 /12	12-03-00	4-02-12	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	56.3 37.00		
	1	T78Z Common	6 /12	12-03-00	4-02-12	2 x 4	1-03-08	1-02-00 1-02-00	49.35 32.67		
	1	T79 Hip Girder	6 /12	18-09-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	89.27 56.33		
	14	J31 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	253.08 163.33		
	7	C31 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	58.13 39.67		
	7	C32 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	75.98 49.00		
	7	C33 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	90.06 58.33		
	7	C34 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	107.9 67.67		

TOTAL # TRUSS= **73** TOTAL BFT OF ALL TRUSSES= **2426.66** BFT. TOTAL WEIGHT OF ALL TRSSES **3810.7** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	VILLA 10
Lot #:	
Elevation:	3

Job Track:	53256
PlanLog:	207153
Layout ID:	432501
Ref #	
Page:	3 of 3
Date:	06-14-2023
Designer:	Leo Chen
Sales Rep:	Rick DiCiano

HARDWARE

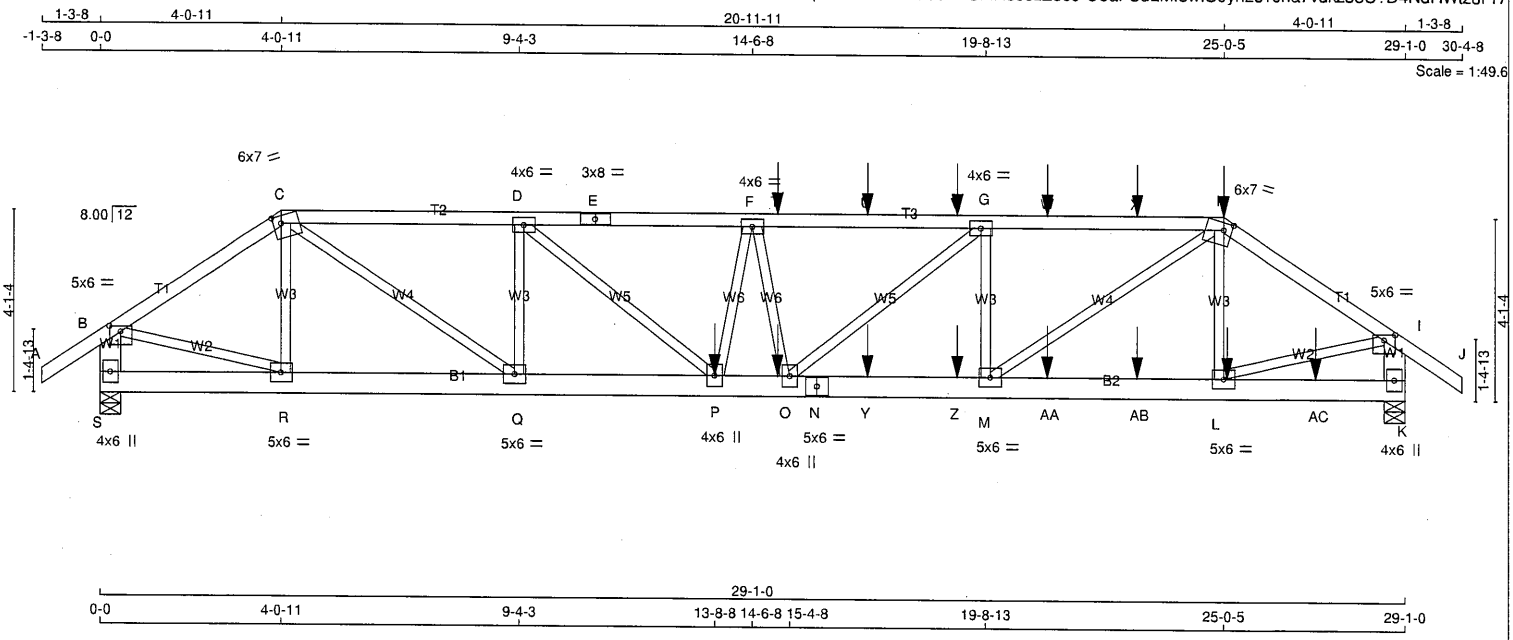
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
3	Hardware	HGUS26-2	
5	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **11**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:27:34 2023 Page 1
ID:quZDzNLEe2lG33FPSMH8sozZ3c0-O8aFSuLMI5wIU9yn23TJha7vdkz33U?D4NdHWtz8Pr7



TOTAL WEIGHT = 2 X 136 = 271 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(6.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2307583

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3028	0	3028	0	0	5-8	1-10
K	3689	0	3689	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2135	1440 / 0	0 / 0	0 / 0	0 / 0	694 / 0	0 / 0
K	2601	1749 / 0	0 / 0	0 / 0	0 / 0	852 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
B- C	-3560 / 0	-91.8	-91.8	0.20 (1)	4.79	
C- D	-6033 / 0	-91.8	-91.8	0.37 (1)	3.72	
D- E	-7860 / 0	-91.8	-91.8	0.49 (1)	3.20	
E- F	-7860 / 0	-91.8	-91.8	0.49 (1)	3.20	
F- T	-7727 / 0	-91.8	-91.8	0.76 (1)	2.87	
T- U	-7727 / 0	-91.8	-91.8	0.76 (1)	2.87	
U- V	-7727 / 0	-91.8	-91.8	0.76 (1)	2.87	
V- G	-7727 / 0	-91.8	-91.8	0.76 (1)	2.87	
G- W	-6673 / 0	-91.8	-91.8	0.68 (1)	3.17	
W- X	-6673 / 0	-91.8	-91.8	0.68 (1)	3.17	
X- H	-6673 / 0	-91.8	-91.8	0.68 (1)	3.17	
H- I	-4416 / 0	-91.8	-91.8	0.24 (1)	4.36	
I- J	0 / 35	-91.8	-91.8	0.07 (1)	10.00	
S- B	-3003 / 0	0.0	0.0	0.11 (1)	7.81	
K- I	-3650 / 0	0.0	0.0	0.13 (1)	7.35	
S- R	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
R- Q	0 / 2940	-18.5	-18.5	0.21 (1)	10.00	
Q- P	0 / 6033	-18.5	-18.5	0.49 (1)	10.00	
P- O	0 / 7865	-18.5	-18.5	0.61 (1)	10.00	
O- N	0 / 6673	-18.5	-18.5	0.50 (1)	10.00	
N- Y	0 / 6673	-18.5	-18.5	0.50 (1)	10.00	
Y- Z	0 / 6673	-18.5	-18.5	0.50 (1)	10.00	
Z- M	0 / 6673	-18.5	-18.5	0.50 (1)	10.00	
M-AA	0 / 3650	-18.5	-18.5	0.28 (1)	10.00	
AA-AB	0 / 3650	-18.5	-18.5	0.28 (1)	10.00	
AB- L	0 / 3650	-18.5	-18.5	0.28 (1)	10.00	
L-AC	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
AC- K	0 / 0	-18.5	-18.5	0.05 (4)	10.00	

WEBS MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
R- C	-592 / 0	0.08 (1)
C- Q	0 / 3778	0.47 (1)
Q- D	-2046 / 0	0.26 (1)
M- G	-1968 / 0	0.25 (1)
M- H	0 / 3693	0.46 (1)
L- H	-662 / 0	0.08 (1)
B- R	0 / 3041	0.38 (1)
L- I	0 / 3772	0.47 (1)
D- P	0 / 2390	0.30 (1)
P- F	-22 / 2	0.00 (1)
F- O	-681 / 0	0.09 (1)
O- G	0 / 1379	0.17 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	25-0-6	-40	-40	---	FRONT	VERT	DEAD	---	C1
H	25-0-5	-128	-128	---	FRONT	VERT	TOTAL	---	C1
H	25-0-6	-171	-171	---	FRONT	VERT	SNOW	---	C1
L	25-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
O	15-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
P	13-8-8	-1143	-1143	---	FRONT	VERT	TOTAL	---	C1
T	15-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
U	17-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
V	19-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.76/1.00 (F-G:1) , BC=0.61/1.00 (O-P:1) , WB=0.47/1.00 (C-Q:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	550	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-O8aFSuLMl5wIU9yn23TJha7vdkz33U?D4NdHWtz8Pr7

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	6.0	7.0	2.00	2.25
D, F, G						
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
H	TTWW-m	MT20	6.0	7.0	2.00	2.25
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	4.0	6.0		
L, M, Q, R						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
S	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	21-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
X	23-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
Y	17-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Z	19-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AA	21-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	23-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AC	27-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307583

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T2	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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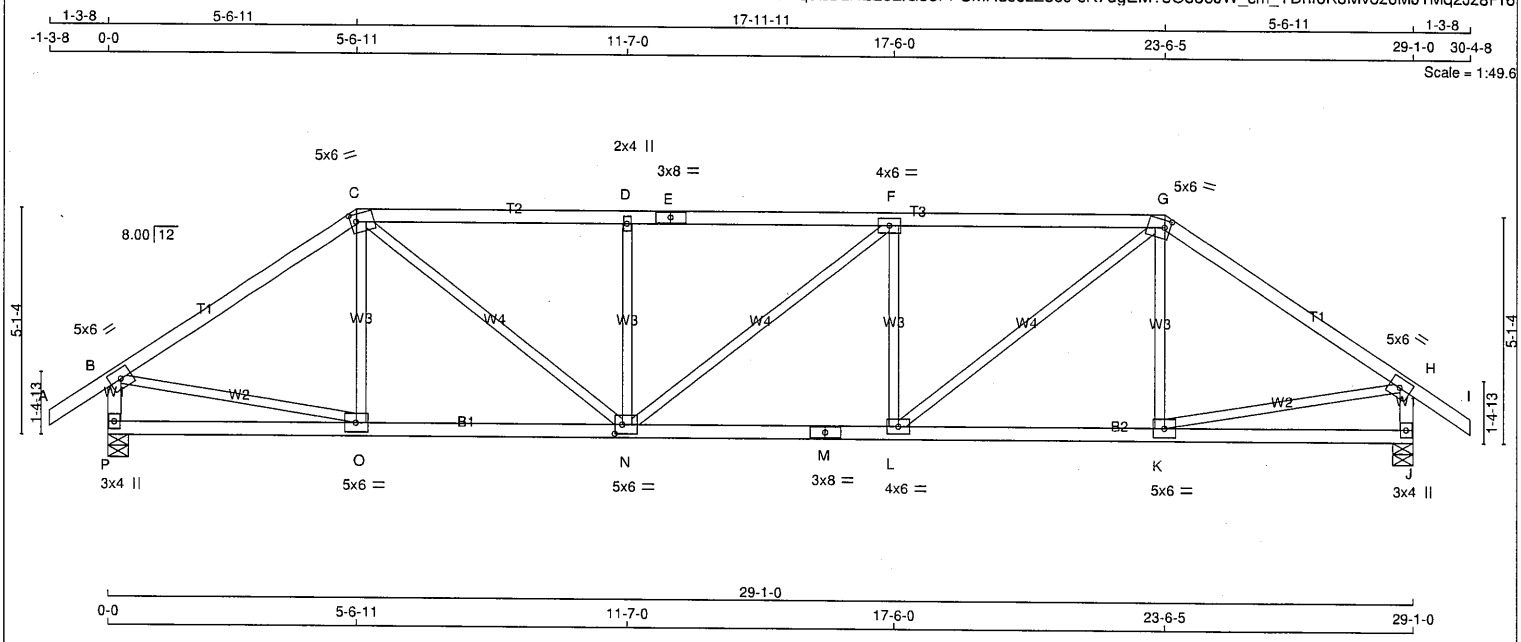

TOTAL WEIGHT = 116 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.00	1.50
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1729	0	1729	0	0	5-8	2-2
J	1729	0	1729	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
J	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C	-1828 / 0	-91.8	-91.8	0.64 (1)		4.22
C- D	-2394 / 0	-91.8	-91.8	0.61 (1)		3.81
D- E	-2395 / 0	-91.8	-91.8	0.61 (1)		3.81
E- F	-2395 / 0	-91.8	-91.8	0.61 (1)		3.81
F- G	-2396 / 0	-91.8	-91.8	0.62 (1)		3.81
G- H	-1828 / 0	-91.8	-91.8	0.64 (1)		4.22
H- I	0 / 35	-91.8	-91.8	0.12 (1)		10.00
P- B	-1688 / 0	0.0	0.0	0.17 (1)		6.39
J- H	-1687 / 0	0.0	0.0	0.17 (1)		6.39
P- O	0 / 0	-18.5	-18.5	0.15 (4)		10.00
O- N	0 / 1516	-18.5	-18.5	0.33 (1)		10.00
N- M	0 / 2396	-18.5	-18.5	0.44 (1)		10.00
M- L	0 / 2396	-18.5	-18.5	0.44 (1)		10.00
L- K	0 / 1515	-18.5	-18.5	0.32 (1)		10.00
K- J	0 / 0	-18.5	-18.5	0.15 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-168 / 35	0.06 (1)
C- N	0 / 1124	0.25 (1)
N- D	-594 / 0	0.23 (1)
N- F	-2 / 0	0.00 (1)
L- F	-594 / 0	0.23 (1)
L- G	0 / 1126	0.25 (1)
K- G	-169 / 34	0.07 (1)
B- O	0 / 1547	0.35 (1)
K- H	0 / 1546	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.44/1.00 (L-N:1) , WB=0.35/1.00 (B-O:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.73 (M) (INPUT = 0.95)

Structural component only
DWG# T-2307584

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T3	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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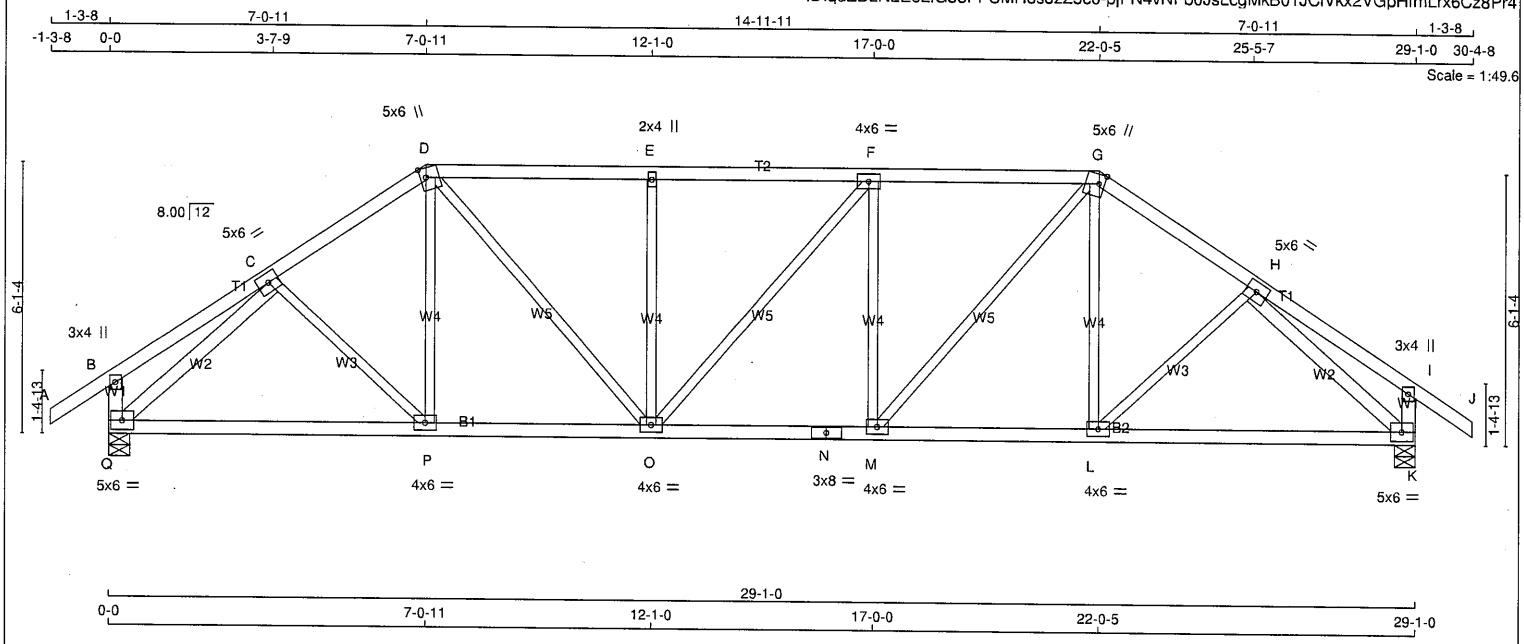

TOTAL WEIGHT = 129 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.50	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, M, P						
L	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	4.0	6.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1729	0	1729	0	0	5-8	1-14
K	1729	0	1729	0	0	5-8	1-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
K	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- P	0 / 62	0.02 (4)
B- C	0 / 19	-91.8	-91.8	0.17 (1)	10.00	P- D	0 / 110	0.04 (4)
C- D	-1814 / 0	-91.8	-91.8	0.22 (1)	4.76	L- G	0 / 109	0.04 (4)
D- E	-1988 / 0	-91.8	-91.8	0.38 (1)	4.40	L- H	0 / 62	0.02 (4)
E- F	-1988 / 0	-91.8	-91.8	0.38 (1)	4.39	Q- C	-2038 / 0	0.56 (1)
F- G	-1989 / 0	-91.8	-91.8	0.39 (1)	4.39	H- K	-2038 / 0	0.56 (1)
G- H	-1814 / 0	-91.8	-91.8	0.22 (1)	4.76	M- G	0 / 759	0.17 (1)
H- I	0 / 19	-91.8	-91.8	0.17 (1)	10.00	D- O	0 / 756	0.17 (1)
I- J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	M- F	-493 / 0	0.29 (1)
Q- B	-253 / 0	0.0	0.0	0.03 (1)	7.81	O- E	-492 / 0	0.29 (1)
K- I	-253 / 0	0.0	0.0	0.03 (1)	7.81	O- F	-3 / 0	0.00 (1)
Q- P	0 / 1460	-18.5	-18.5	0.35 (1)	10.00			
P- O	0 / 1493	-18.5	-18.5	0.35 (1)	10.00			
O- N	0 / 1989	-18.5	-18.5	0.37 (1)	10.00			
N- M	0 / 1989	-18.5	-18.5	0.37 (1)	10.00			
M- L	0 / 1493	-18.5	-18.5	0.35 (1)	10.00			
L- K	0 / 1460	-18.5	-18.5	0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.39/1.00 (F-G:1) , BC=0.37/1.00 (M-O:1) , WB=0.56/1.00 (C-Q:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.60 (N) (INPUT = 0.95)

Structural component only
DWG# T-2307585

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T4	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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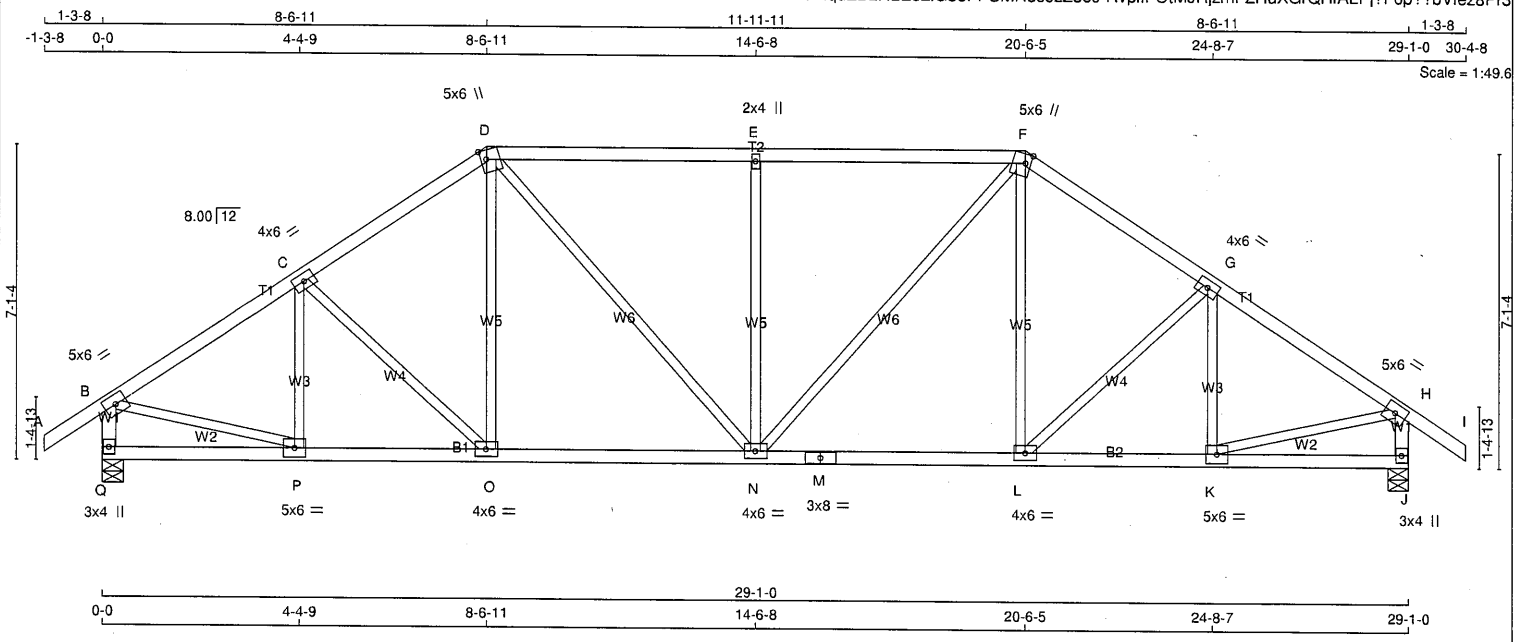

TOTAL WEIGHT = 128 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1729	0	1729	0	0	5-8	2-2
J	1729	0	1729	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
J	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
	A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
	B- C	-1830 / 0	-91.8	-91.8	0.25 (1)	4.72
	C- D	-1745 / 0	-91.8	-91.8	0.25 (1)	4.82
	D- E	-1776 / 0	-91.8	-91.8	0.47 (1)	4.49
	E- F	-1776 / 0	-91.8	-91.8	0.47 (1)	4.49
	F- G	-1745 / 0	-91.8	-91.8	0.25 (1)	4.82
	G- H	-1830 / 0	-91.8	-91.8	0.25 (1)	4.72
	H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00
	Q- B	-1692 / 0	0.0	0.0	0.17 (1)	6.39
	J- H	-1692 / 0	0.0	0.0	0.17 (1)	6.39
	Q- P	0 / 0	-18.5	-18.5	0.07 (4)	10.00
	P- O	0 / 1543	-18.5	-18.5	0.31 (1)	10.00
	O- N	0 / 1431	-18.5	-18.5	0.30 (1)	10.00
	N- M	0 / 1431	-18.5	-18.5	0.30 (1)	10.00
	M- L	0 / 1431	-18.5	-18.5	0.30 (1)	10.00
	L- K	0 / 1543	-18.5	-18.5	0.31 (1)	10.00
	K- J	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
	P- C	-303 / 0	0.08 (1)
	C- O	-158 / 0	0.09 (1)
	O- D	0 / 207	0.05 (4)
	D- N	0 / 522	0.12 (1)
	N- E	-673 / 0	0.59 (1)
	N- F	0 / 522	0.12 (1)
	L- F	0 / 207	0.05 (4)
	L- G	-158 / 0	0.09 (1)
	K- G	-303 / 0	0.08 (1)
	B- P	0 / 1590	0.36 (1)
	K- H	0 / 1590	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.31/1.00 (K-L:1) , WB=0.59/1.00 (E-N:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 0.95)

Structural component only
DWG# T-2307586

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T5X	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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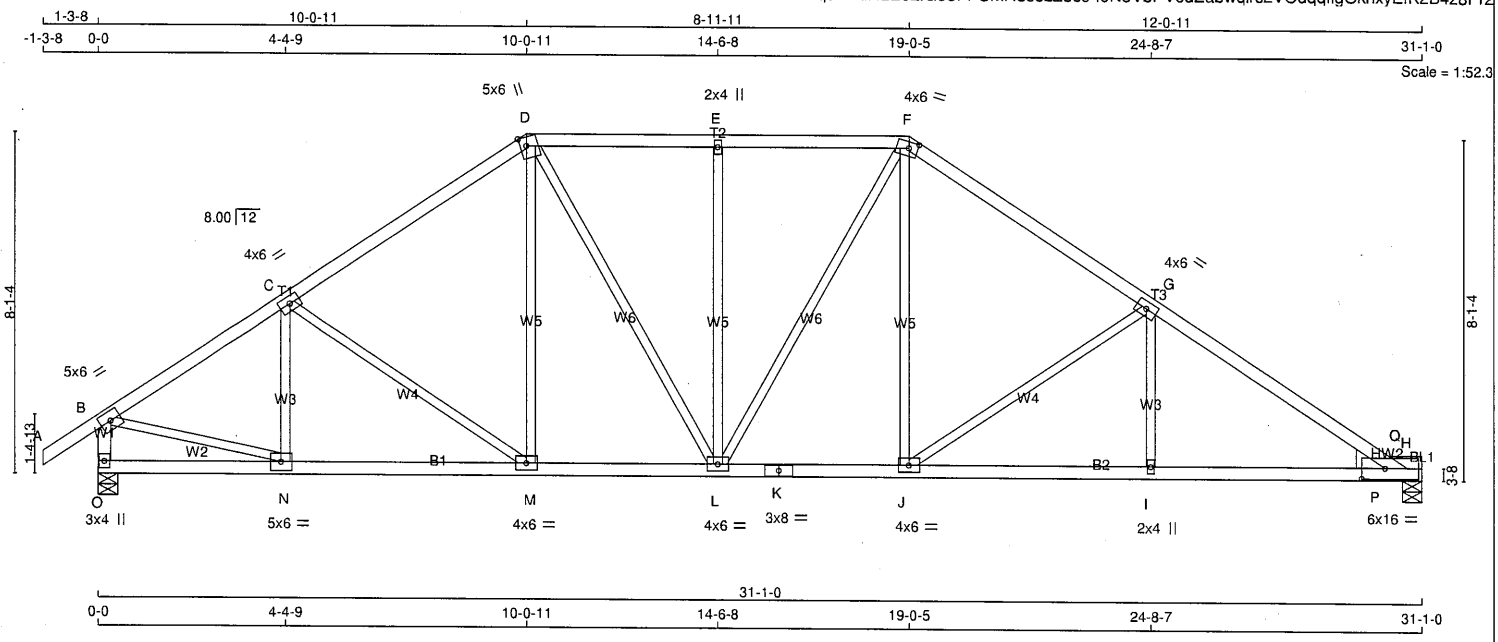

TOTAL WEIGHT = 133 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TMWW-t	MT20	4.0	6.0		
H	TBMHK1*m	MT20	6.0	16.0	3.00	6.75
I	BMW+w	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307587

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
H	1695	0	1695	0	0	5-8	1-13	2x6 R
O	1821	0	1821	0	0	5-8	2-7	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1199	787 / 0	0 / 0	0 / 0	0 / 0	412 / 0	0 / 0
O	1285	857 / 0	0 / 0	0 / 0	0 / 0	428 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-1972 / 0	-91.8	-91.8	0.36 (1)	4.45	
C- D	-1802 / 0	-91.8	-91.8	0.41 (1)	4.58	
D- E	-1693 / 0	-91.8	-91.8	0.26 (1)	4.85	
E- F	-1693 / 0	-91.8	-91.8	0.26 (1)	4.85	
F- G	-1927 / 0	-91.8	-91.8	0.44 (1)	4.41	
G- Q	-2565 / 0	-91.8	-91.8	0.49 (1)	3.87	
Q- H	-2580 / 0	-91.8	-91.8	0.10 (1)	4.25	
O- B	-1788 / 0	0.0	0.0	0.18 (1)	6.25	
O- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00	
N- M	0 / 1671	-18.5	-18.5	0.33 (1)	10.00	
M- L	0 / 1477	-18.5	-18.5	0.30 (1)	10.00	
L- K	0 / 1580	-18.5	-18.5	0.29 (1)	10.00	
K- J	0 / 1580	-18.5	-18.5	0.29 (1)	10.00	
J- I	0 / 2167	-18.5	-18.5	0.48 (1)	10.00	
I- P	0 / 2167	-18.5	-18.5	0.60 (1)	10.00	
P- H	0 / 2167	-18.5	-18.5	0.60 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-313 / 0	0.09 (1)
M- C	-240 / 0	0.23 (1)
M- D	0 / 239	0.05 (1)
D- L	0 / 433	0.10 (1)
L- E	-497 / 0	0.63 (1)
L- F	0 / 226	0.05 (1)
J- F	0 / 494	0.11 (1)
J- G	-721 / 0	0.68 (1)
I- G	0 / 166	0.05 (4)
B- N	0 / 1722	0.39 (1)
P- Q	-251 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.49/1.00 (G-Q:1) , BC=0.60/1.00 (I-P:1) , WB=0.68/1.00 (G-J:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T6	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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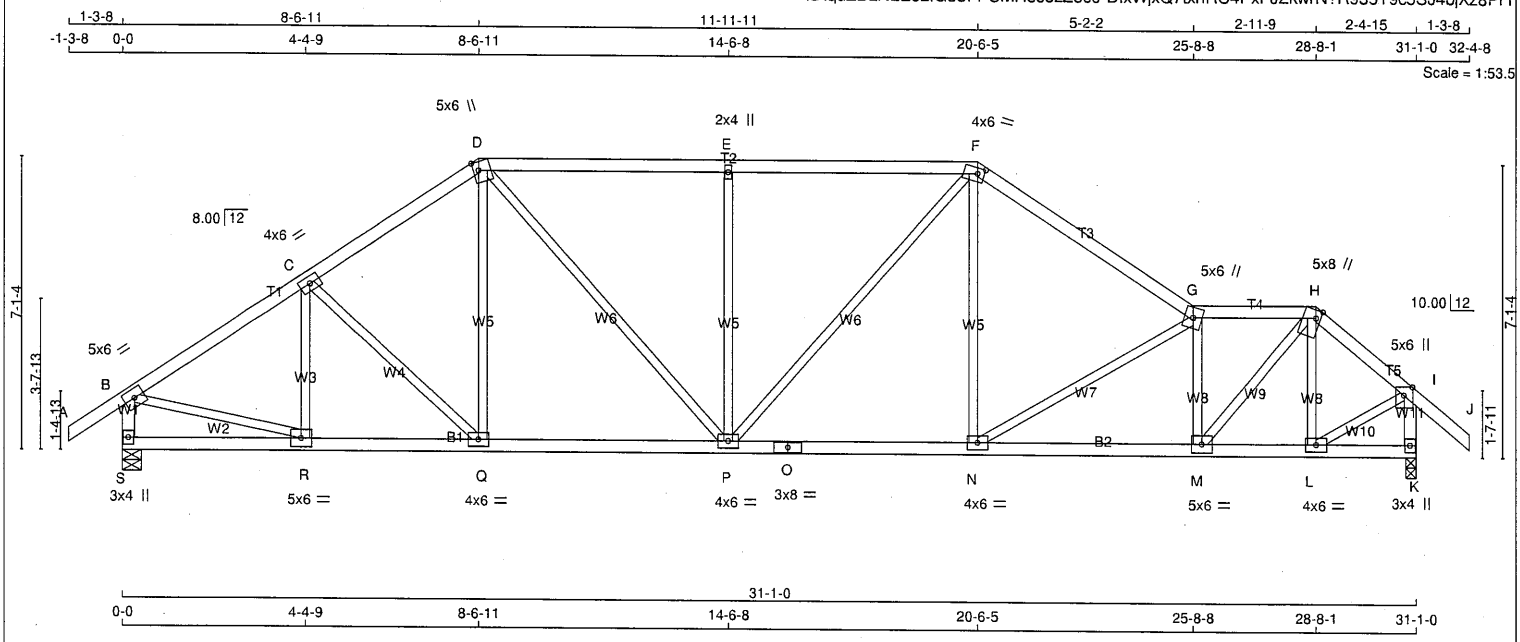

TOTAL WEIGHT = 3 X 138 = 413 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.00
G	TTWW+m	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	1.25
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L, N, Q						
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307588

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1839	0	1839	0	0	5-8	2-8
K	1841	0	1841	0	0	3-0	2-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1298	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
K	1299	866 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-1975 / 0	-91.8	-91.8	0.26 (1)	4.58	
C- D	-1912 / 0	-91.8	-91.8	0.25 (1)	4.64	
D- E	-2011 / 0	-91.8	-91.8	0.48 (1)	4.25	
E- F	-2011 / 0	-91.8	-91.8	0.48 (1)	4.25	
F- G	-2114 / 0	-91.8	-91.8	0.40 (1)	4.30	
G- H	-2238 / 0	-91.8	-91.8	0.16 (1)	4.45	
H- I	-1526 / 0	-91.8	-91.8	0.09 (1)	5.25	
I- J	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
S- B	-1802 / 0	0.0	0.0	0.19 (1)	6.23	
K- I	-1826 / 0	0.0	0.0	0.19 (1)	6.20	
S- R	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
R- Q	0 / 1663	-18.5	-18.5	0.33 (1)	10.00	
Q- P	0 / 1570	-18.5	-18.5	0.31 (1)	10.00	
P- O	0 / 1766	-18.5	-18.5	0.36 (1)	10.00	
O- N	0 / 1766	-18.5	-18.5	0.36 (1)	10.00	
N- M	0 / 2275	-18.5	-18.5	0.44 (1)	10.00	
M- L	0 / 1150	-18.5	-18.5	0.21 (1)	10.00	
L- K	0 / 0	-18.5	-18.5	0.03 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-334 / 0	0.09 (1)
Q- C	-132 / 0	0.08 (1)
Q- D	0 / 191	0.05 (4)
P- D	0 / 667	0.15 (1)
P- E	-672 / 0	0.59 (1)
P- F	0 / 371	0.08 (1)
N- F	0 / 443	0.10 (1)
N- G	-611 / 0	0.43 (1)
M- G	-1215 / 0	0.26 (1)
M- H	0 / 1672	0.38 (1)
L- H	-539 / 0	0.12 (1)
B- R	0 / 1713	0.39 (1)
L- I	0 / 1314	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.44/1.00 (M-N:1) , WB=0.59/1.00 (E-P:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.56 (I) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T7X	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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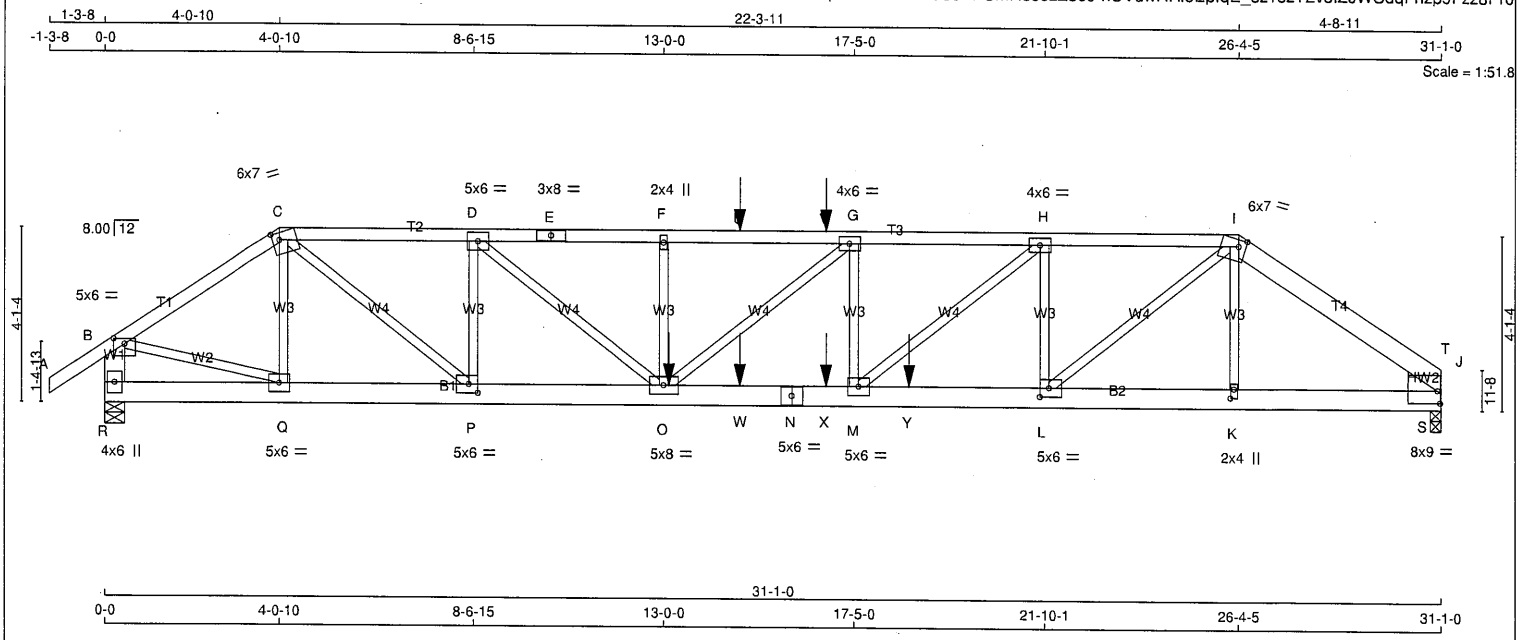

TOTAL WEIGHT = 2 X 144 = 287 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- I	1	12	SIDE(0.0)
I- J	2	12	TOP
R- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- J	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
O- F	1	6	SIDE(238.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2307589

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
R	3514	0	3514	0	0	5-8	1-14	
J	3398	0	3398	0	0	3-0	1-13	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2478	1663 / 0	0 / 0	0 / 0	0 / 0	815 / 0	0 / 0
J	2399	1598 / 0	0 / 0	0 / 0	0 / 0	800 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	Q- C	-740 / 0		0.09 (1)
B- C	-4194 / 0	-91.8	-91.8	0.23 (1)	4.46	K- I	-256 / 0		0.03 (1)
C- D	-6742 / 0	-91.8	-91.8	0.34 (1)	3.55	B- Q	0 / 3583		0.44 (1)
D- E	-9264 / 0	-91.8	-91.8	0.51 (1)	2.91	L- I	0 / 4245		0.53 (1)
E- F	-9264 / 0	-91.8	-91.8	0.51 (1)	2.91	C- P	0 / 4242		0.53 (1)
F- U	-9264 / 0	-91.8	-91.8	0.61 (1)	2.80	L- H	-2103 / 0		0.27 (1)
U- V	-9264 / 0	-91.8	-91.8	0.61 (1)	2.80	P- D	-2544 / 0		0.32 (1)
V- G	-9264 / 0	-91.8	-91.8	0.61 (1)	2.80	M- H	0 / 2763		0.34 (1)
G- H	-9378 / 0	-91.8	-91.8	0.55 (1)	2.85	D- O	0 / 3300		0.41 (1)
H- I	-7267 / 0	-91.8	-91.8	0.38 (1)	3.40	M- G	-554 / 0		0.07 (1)
I- T	-4916 / 0	-91.8	-91.8	0.34 (1)	4.86	O- F	-535 / 0		0.07 (1)
T- J	-6461 / 0	-91.8	-91.8	0.35 (1)	4.31	O- G	-150 / 0		0.04 (1)
R- B	-3482 / 0	0.0	0.0	0.12 (1)	7.49	S- T	0 / 2078		0.00 (1)
R- Q	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
Q- P	0 / 3463	-18.5	-18.5	0.24 (1)	10.00				
P- O	0 / 6742	-18.5	-18.5	0.56 (1)	10.00				
O- W	0 / 9378	-18.5	-18.5	0.73 (1)	10.00				
W- N	0 / 9378	-18.5	-18.5	0.73 (1)	10.00				
N- X	0 / 9378	-18.5	-18.5	0.73 (1)	10.00				
X- M	0 / 9378	-18.5	-18.5	0.73 (1)	10.00				
M- Y	0 / 7267	-18.5	-18.5	0.81 (1)	10.00				
Y- L	0 / 7267	-18.5	-18.5	0.81 (1)	10.00				
L- K	0 / 3968	-18.5	-18.5	0.36 (1)	10.00				
K- S	0 / 3975	-18.5	-18.5	0.52 (1)	10.00				
S- J	0 / 3975	-18.5	-18.5	0.52 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	13-1-8	-1138	-1138	---	BACK	VERT	TOTAL	---	C1
U	14-9-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
V	16-9-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
W	14-9-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
X	16-9-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
Y	18-8-8	-927	-927	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 901 (0.41")

CSI: TC=0.61/1.00 (F-G:1) , BC=0.81/1.00 (L-M:1) , WB=0.53/1.00 (I-L:1) , SSI=0.60/1.00 (J-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.81 (N) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T7X	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2IG33FPSMH8sozZ3c0-hUVuwHRleEplqE_8z15zT2v8IZJWCdqFhzp9Fzz8Pr0

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	6.0	7.0	2.00	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW-m	MT20	6.0	7.0	2.00	2.00
J	TMBH1-l	MT20	8.0	9.0	3.50	0.75
K	BMW+w	MT20	2.0	4.0	2.50	1.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307589

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T8X	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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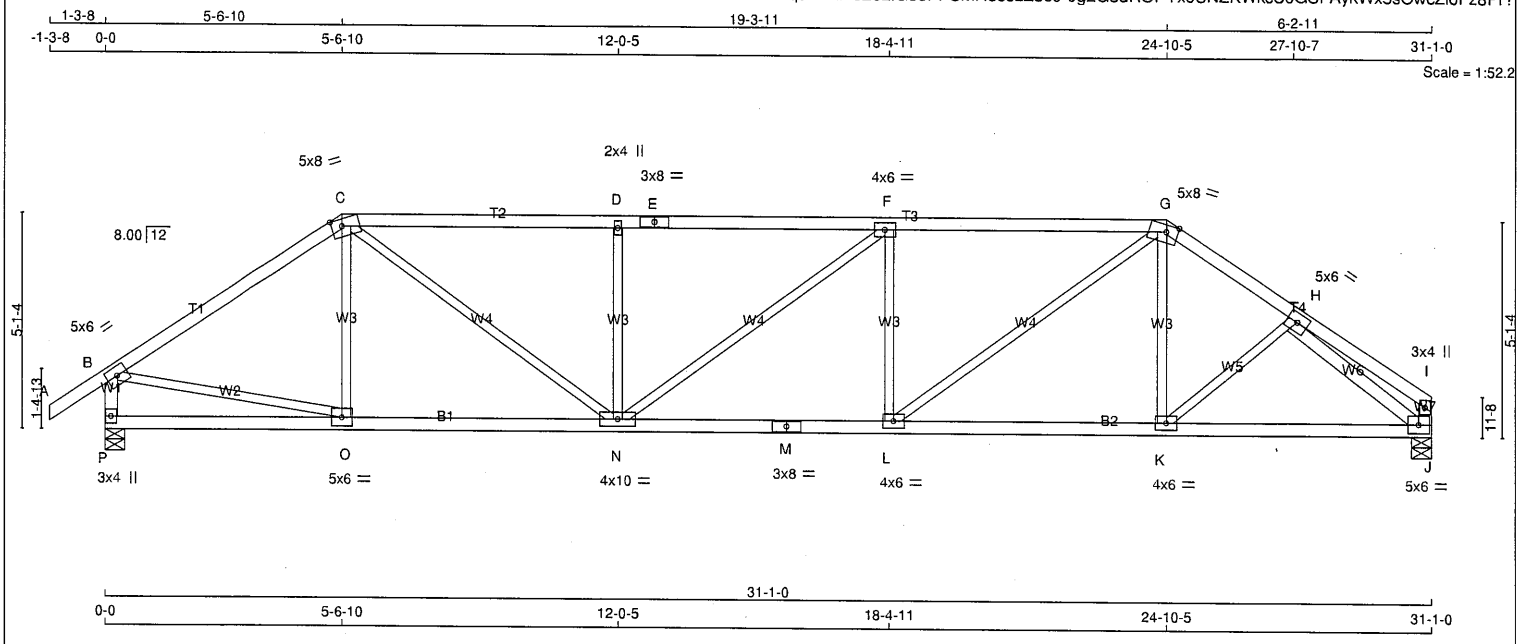

TOTAL WEIGHT = 123 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TS-I	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.25
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	10.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1839	0	1839	0	0	5-8	2-8
J	1714	0	1714	0	0	5-8	1-14

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1298	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
J	1212	795 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	O- C	-189 / 30		0.07 (1)
B- C	-1983 / 0	-91.8	-91.8	0.66 (1)	4.06	C- N	0 / 1327		0.30 (1)
C- D	-2710 / 0	-91.8	-91.8	0.78 (1)	3.45	N- D	-638 / 0		0.25 (1)
D- E	-2710 / 0	-91.8	-91.8	0.77 (1)	3.44	N- F	-62 / 0		0.08 (1)
E- F	-2710 / 0	-91.8	-91.8	0.77 (1)	3.44	L- F	-602 / 0		0.23 (1)
F- G	-2759 / 0	-91.8	-91.8	0.79 (1)	3.41	L- G	0 / 1202		0.27 (1)
G- H	-2172 / 0	-91.8	-91.8	0.17 (1)	4.48	K- G	0 / 123		0.04 (4)
H- I	0 / 14	-91.8	-91.8	0.11 (1)	10.00	K- H	0 / 50		0.02 (4)
P- B	-1799 / 0	0.0	0.0	0.19 (1)	6.23	B- O	0 / 1678		0.38 (1)
J- I	-116 / 0	0.0	0.0	0.01 (1)	7.81	H- J	-2360 / 0		0.48 (1)
P- O	0 / 0	-18.5	-18.5	0.16 (4)	10.00				
O- N	0 / 1644	-18.5	-18.5	0.35 (1)	10.00				
N- M	0 / 2759	-18.5	-18.5	0.51 (1)	10.00				
M- L	0 / 2759	-18.5	-18.5	0.51 (1)	10.00				
L- K	0 / 1794	-18.5	-18.5	0.39 (1)	10.00				
K- J	0 / 1779	-18.5	-18.5	0.38 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.79/1.00 (F-G:1) , BC=0.51/1.00 (L-N:1) , WB=0.48/1.00 (H-J:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.88 (M) (INPUT = 0.95)

Structural component only
DWG# T-2307590

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T9X	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:27:44 2023 Page 1
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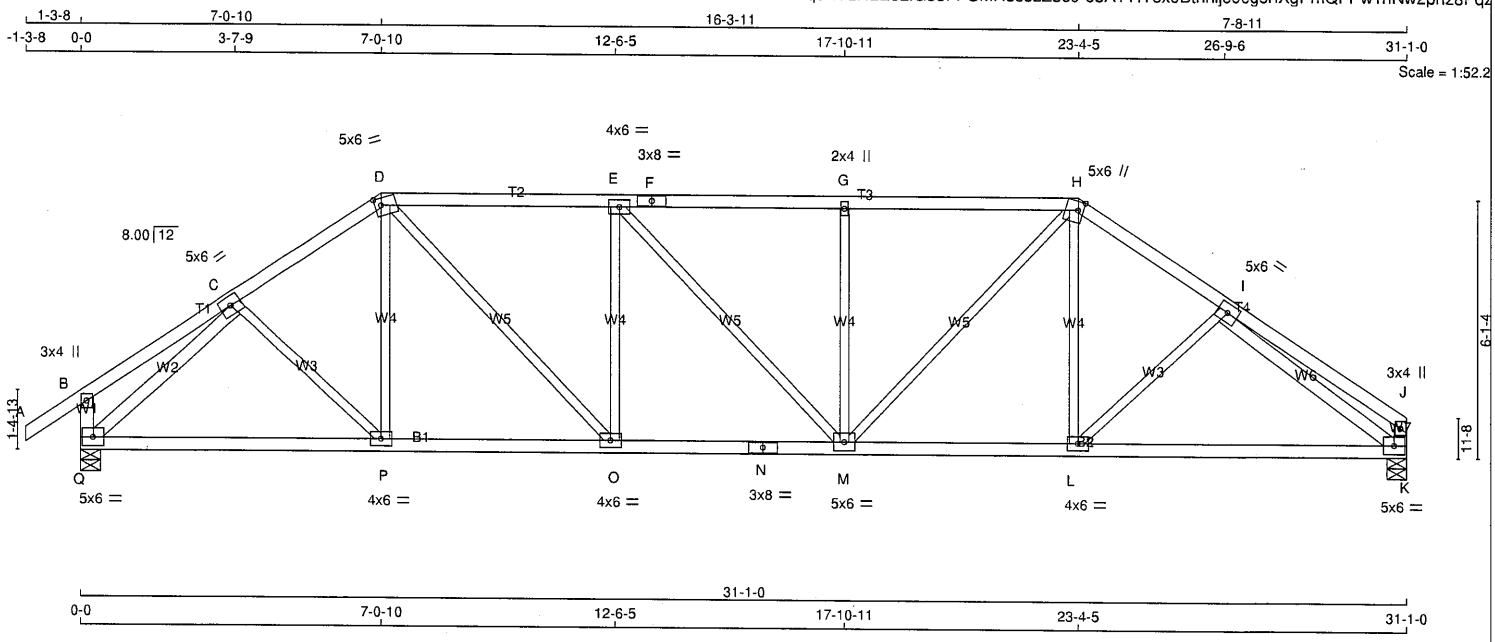

TOTAL WEIGHT = 133 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.50	1.50
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, O, P						
L	BMWW-t	MT20	4.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307591

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1839	0	1839	0	0	5-8	2-0
K	1714	0	1714	0	0	5-8	1-14

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1298	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
K	1212	795 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- P	0 / 73	0.03 (4)
B- C	0 / 19	-91.8	-91.8	0.17 (1)	10.00	P- D	0 / 104	0.04 (4)
C- D	-1976 / 0	-91.8	-91.8	0.22 (1)	4.60	D- O	0 / 914	0.21 (1)
D- E	-2254 / 0	-91.8	-91.8	0.49 (1)	4.06	O- E	-574 / 0	0.34 (1)
E- F	-2286 / 0	-91.8	-91.8	0.48 (1)	4.03	E- M	0 / 47	0.01 (1)
F- G	-2286 / 0	-91.8	-91.8	0.48 (1)	4.03	M- G	-537 / 0	0.31 (1)
G- H	-2286 / 0	-91.8	-91.8	0.48 (1)	4.04	M- H	0 / 806	0.18 (1)
H- I	-2105 / 0	-91.8	-91.8	0.27 (1)	4.42	L- H	0 / 243	0.06 (4)
I- J	0 / 20	-91.8	-91.8	0.21 (1)	10.00	L- I	-138 / 9	0.06 (1)
Q- B	-253 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2195 / 0	0.61 (1)
K- J	-157 / 0	0.0	0.0	0.02 (1)	7.81	I- K	-2363 / 0	0.82 (1)
Q- P	0 / 1573	-18.5	-18.5	0.36 (1)	10.00			
P- O	0 / 1627	-18.5	-18.5	0.37 (1)	10.00			
O- N	0 / 2254	-18.5	-18.5	0.42 (1)	10.00			
N- M	0 / 2254	-18.5	-18.5	0.42 (1)	10.00			
M- L	0 / 1733	-18.5	-18.5	0.41 (1)	10.00			
L- K	0 / 1829	-18.5	-18.5	0.43 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.43/1.00 (K-L:1) , WB=0.82/1.00 (I-K:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.73 (N) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T10	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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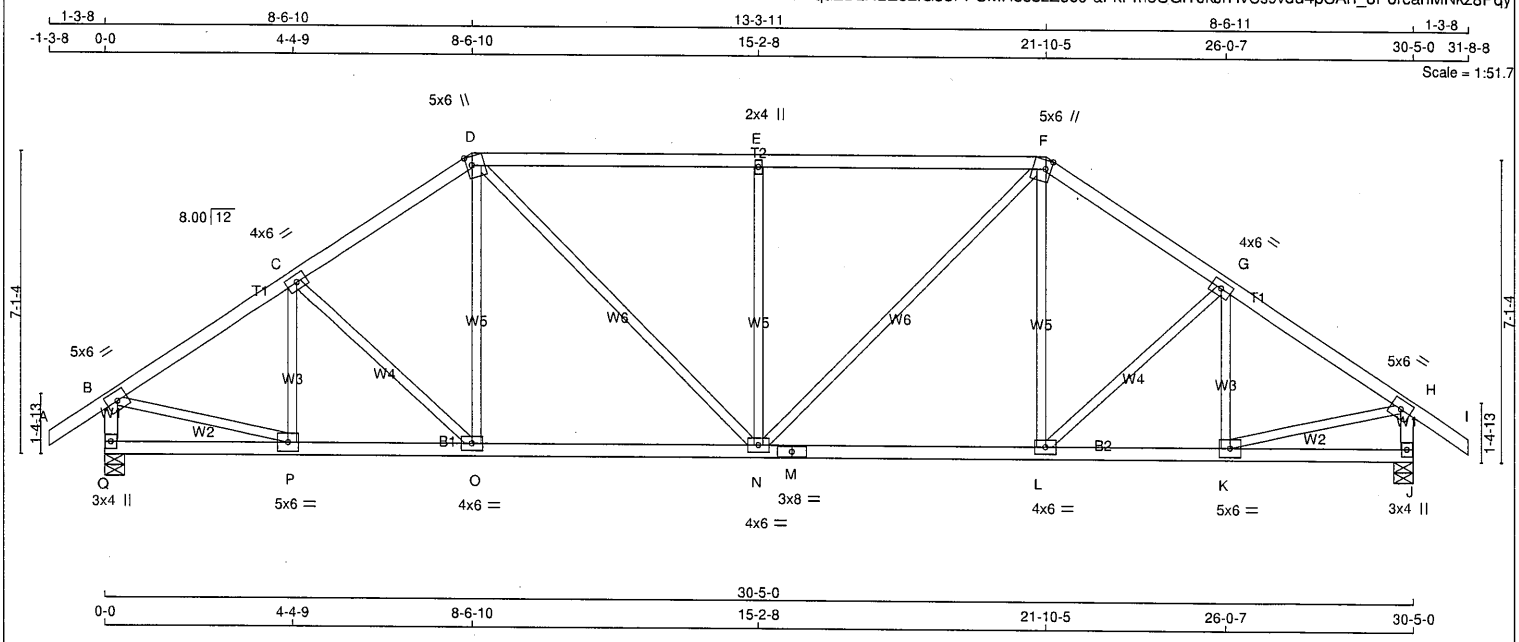

TOTAL WEIGHT = 132 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1803	0	1803	0	0	5-8	2-6
J	1803	0	1803	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1272	848 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0
J	1272	848 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 35	-91.8	-91.8	0.12 (1)		10.00
B- C		-1926 / 0	-91.8	-91.8	0.26 (1)		4.63
C- D		-1858 / 0	-91.8	-91.8	0.25 (1)		4.70
D- E		-1953 / 0	-91.8	-91.8	0.59 (1)		4.13
E- F		-1953 / 0	-91.8	-91.8	0.59 (1)		4.13
F- G		-1858 / 0	-91.8	-91.8	0.25 (1)		4.70
G- H		-1926 / 0	-91.8	-91.8	0.26 (1)		4.63
H- I		0 / 35	-91.8	-91.8	0.12 (1)		10.00
Q- B		-1765 / 0	0.0	0.0	0.18 (1)		6.28
J- H		-1765 / 0	0.0	0.0	0.18 (1)		6.28
Q- P		0 / 0	-18.5	-18.5	0.08 (4)		10.00
P- O		0 / 1623	-18.5	-18.5	0.33 (1)		10.00
O- N		0 / 1525	-18.5	-18.5	0.33 (1)		10.00
N- M		0 / 1525	-18.5	-18.5	0.33 (1)		10.00
M- L		0 / 1525	-18.5	-18.5	0.33 (1)		10.00
L- K		0 / 1623	-18.5	-18.5	0.33 (1)		10.00
K- J		0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C		-326 / 0	0.09 (1)
C- O		-138 / 0	0.08 (1)
O- D		0 / 202	0.05 (4)
D- N		0 / 612	0.14 (1)
N- E		-750 / 0	0.66 (1)
N- F		0 / 612	0.14 (1)
L- F		0 / 202	0.05 (4)
L- G		-138 / 0	0.08 (1)
K- G		-326 / 0	0.09 (1)
B- P		0 / 1672	0.38 (1)
K- H		0 / 1672	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.33/1.00 (K-L:1) , WB=0.66/1.00 (E-N:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.48 (M) (INPUT = 0.95)

Structural component only
DWG# T-2307592

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T11	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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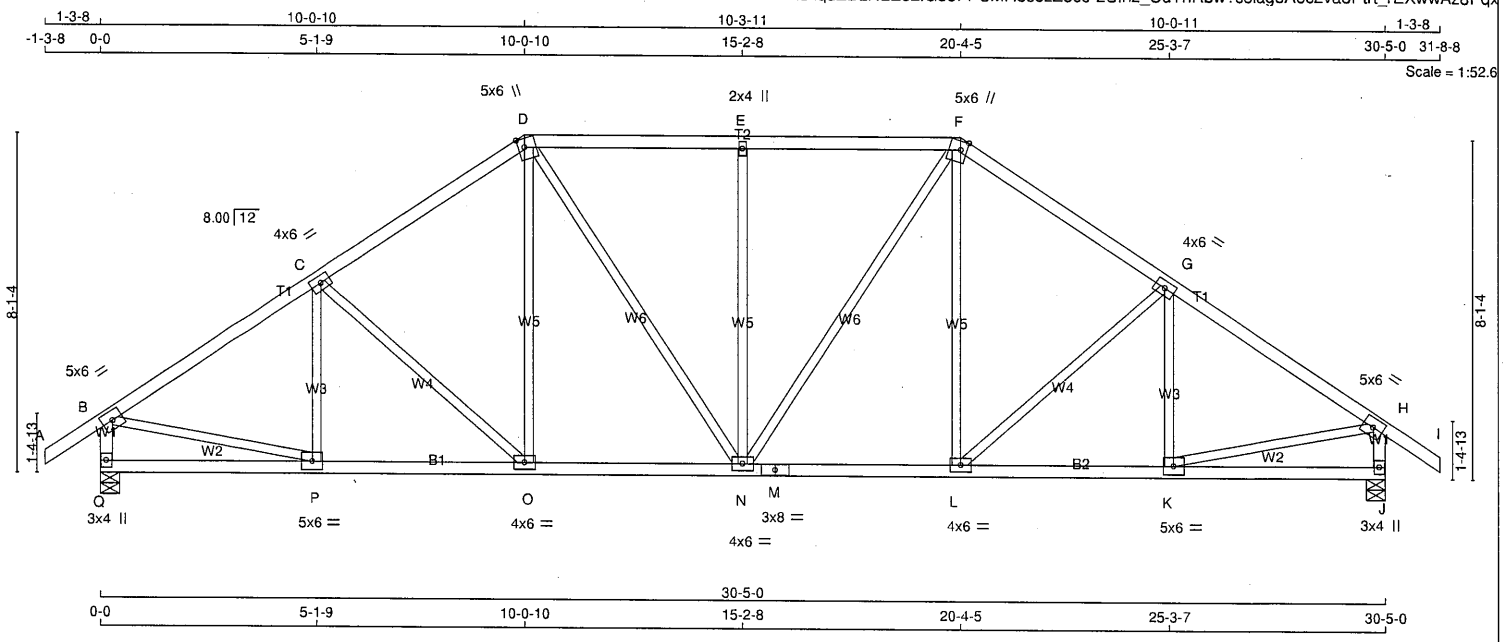

TOTAL WEIGHT = 138 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1803	0	1803	0	0	5-8	2-6
J	1803	0	1803	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1272	848 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0
J	1272	848 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00	P- C	-250 / 0	0.09 (1)
B- C	-1962 / 0	-91.8	-91.8	0.36 (1)		4.48	C- O	-279 / 0	0.24 (1)
C- D	-1775 / 0	-91.8	-91.8	0.34 (1)		4.68	O- D	0 / 282	0.06 (1)
D- E	-1673 / 0	-91.8	-91.8	0.34 (1)		4.77	D- N	0 / 400	0.09 (1)
E- F	-1673 / 0	-91.8	-91.8	0.34 (1)		4.77	N- E	-576 / 0	0.73 (1)
F- G	-1775 / 0	-91.8	-91.8	0.34 (1)		4.68	N- F	0 / 400	0.09 (1)
G- H	-1962 / 0	-91.8	-91.8	0.36 (1)		4.48	L- F	0 / 282	0.06 (1)
H- I	0 / 35	-91.8	-91.8	0.12 (1)		10.00	L- G	-279 / 0	0.24 (1)
Q- B	-1762 / 0	0.0	0.0	0.18 (1)		6.29	K- G	-250 / 0	0.09 (1)
J- H	-1762 / 0	0.0	0.0	0.18 (1)		6.29	B- P	0 / 1694	0.38 (1)
							K- H	0 / 1694	0.38 (1)
Q- P	0 / 0	-18.5	-18.5	0.11 (4)		10.00			
P- O	0 / 1657	-18.5	-18.5	0.32 (1)		10.00			
O- N	0 / 1453	-18.5	-18.5	0.29 (1)		10.00			
N- M	0 / 1453	-18.5	-18.5	0.29 (1)		10.00			
M- L	0 / 1453	-18.5	-18.5	0.29 (1)		10.00			
L- K	0 / 1657	-18.5	-18.5	0.32 (1)		10.00			
K- J	0 / 0	-18.5	-18.5	0.11 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.36/1.00 (G-H:1) , BC=0.32/1.00 (O-P:1) , WB=0.73/1.00 (E-N:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.49 (H) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307593

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T12	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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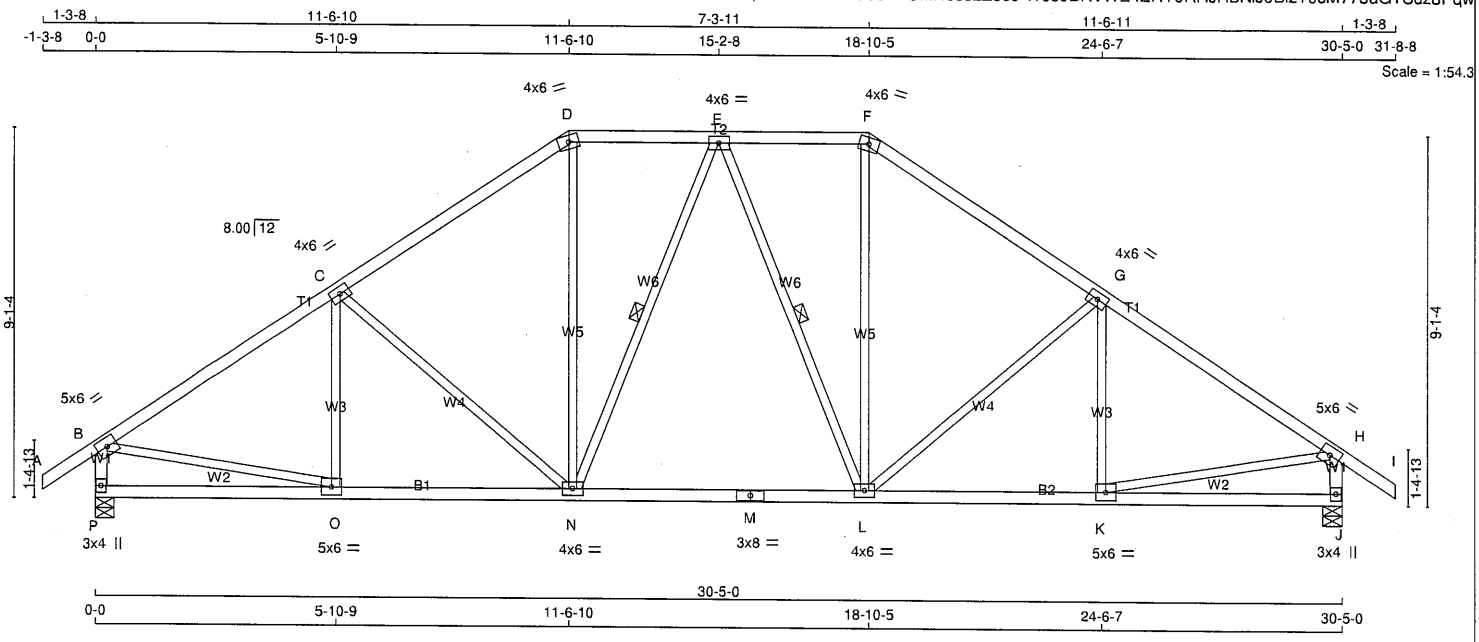

TOTAL WEIGHT = 138 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C, E, G						
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
P	1803	0	1803	0	0	IN-SX	IN-SX
J	1803	0	1803	0	0	5-8	2-6
						5-8	2-6

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1272	848 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0
J	1272	848 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	O- C	-195 / 26	0.08 (1)
B- C	-1976 / 0	-91.8	-91.8	0.48 (1)	4.32	C- N	-402 / 0	0.47 (1)
C- D	-1684 / 0	-91.8	-91.8	0.45 (1)	4.64	N- D	0 / 584	0.13 (1)
D- E	-1376 / 0	-91.8	-91.8	0.16 (1)	5.38	L- F	0 / 584	0.13 (1)
E- F	-1376 / 0	-91.8	-91.8	0.16 (1)	5.38	L- G	-402 / 0	0.47 (1)
F- G	-1684 / 0	-91.8	-91.8	0.45 (1)	4.64	K- G	-195 / 26	0.08 (1)
G- H	-1976 / 0	-91.8	-91.8	0.48 (1)	4.32	B- O	0 / 1701	0.38 (1)
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00	K- H	0 / 1701	0.38 (1)
P- B	-1756 / 0	0.0	0.0	0.18 (1)	6.29	N- E	-214 / 0	0.14 (1)
J- H	-1756 / 0	0.0	0.0	0.18 (1)	6.29	E- L	-214 / 0	0.14 (1)
P- O	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
O- N	0 / 1673	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 1456	-18.5	-18.5	0.32 (1)	10.00			
M- L	0 / 1456	-18.5	-18.5	0.32 (1)	10.00			
L- K	0 / 1673	-18.5	-18.5	0.36 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.48/1.00 (G-H:1) , BC=0.36/1.00 (K-L:1) , WB=0.47/1.00 (G-L:1) , SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.53 (M) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307594

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T13X	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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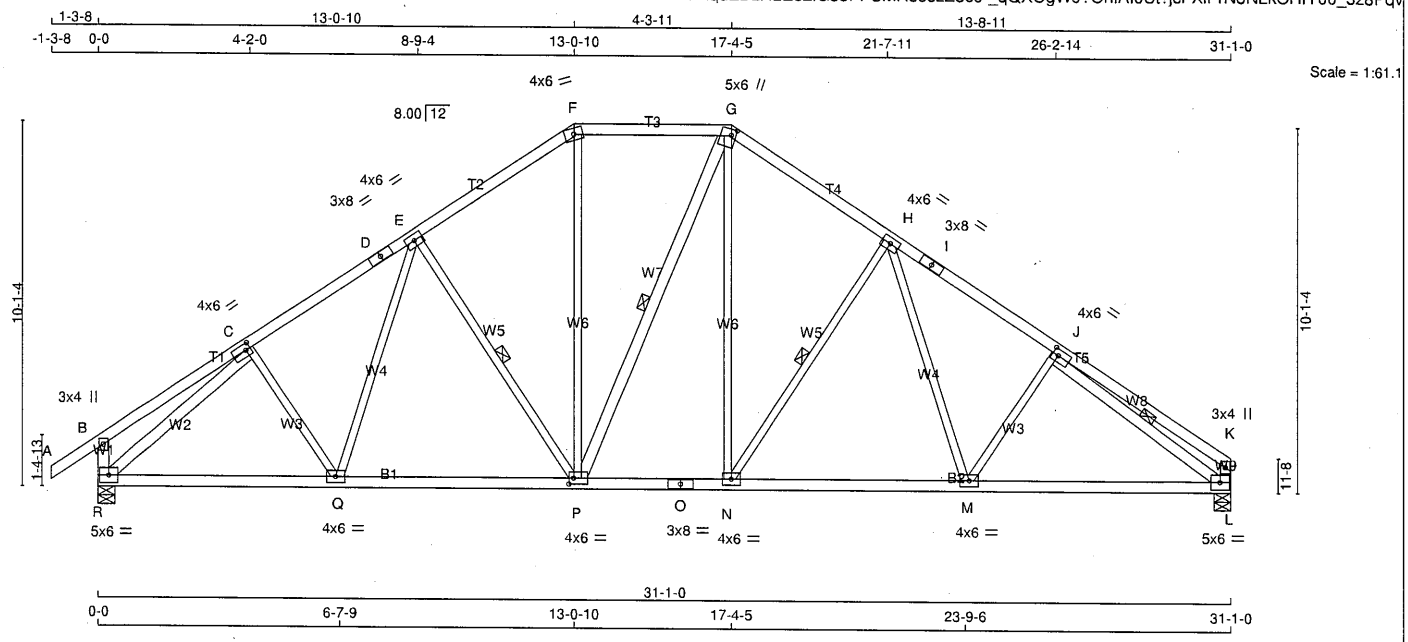

TOTAL WEIGHT = 151 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW-t	MT20	4.0	6.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	4.0	6.0	2.00	2.00
K	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M, N, Q						
M	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	6.0	2.00	1.50
R	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307595

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1839	0	1839	0	0	5-8	2-0
L	1714	0	1714	0	0	5-8	1-14

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1298	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
L	1212	795 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, G-P, H-N, J-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- Q	0 / 55	0.02 (4)
B- C	0 / 21	-91.8	-91.8	0.21 (1)	10.00	Q- E	0 / 139	0.04 (4)
C- D	-1949 / 0	-91.8	-91.8	0.26 (1)	4.61	E- P	-477 / 0	0.22 (1)
D- E	-1949 / 0	-91.8	-91.8	0.26 (1)	4.61	P- F	0 / 532	0.12 (1)
E- F	-1618 / 0	-91.8	-91.8	0.24 (1)	4.98	P- G	-36 / 0	0.02 (1)
F- G	-1330 / 0	-91.8	-91.8	0.24 (1)	5.36	N- G	0 / 573	0.13 (1)
G- H	-1633 / 0	-91.8	-91.8	0.23 (1)	4.97	N- H	-579 / 0	0.26 (1)
H- I	-2098 / 0	-91.8	-91.8	0.27 (1)	4.47	H- M	0 / 314	0.07 (1)
I- J	-2098 / 0	-91.8	-91.8	0.27 (1)	4.47	M- J	-184 / 0	0.06 (1)
J- K	0 / 22	-91.8	-91.8	0.25 (1)	10.00	R- C	-2220 / 0	0.79 (1)
R- B	-273 / 0	0.0	0.0	0.03 (1)	7.81	J- L	-2369 / 0	0.44 (1)
L- K	-177 / 0	0.0	0.0	0.02 (1)	7.81			
R- Q	0 / 1622	-18.5	-18.5	0.36 (1)	10.00			
Q- P	0 / 1584	-18.5	-18.5	0.35 (1)	10.00			
P- O	0 / 1344	-18.5	-18.5	0.28 (1)	10.00			
O- N	0 / 1344	-18.5	-18.5	0.28 (1)	10.00			
N- M	0 / 1653	-18.5	-18.5	0.38 (1)	10.00			
M- L	0 / 1848	-18.5	-18.5	0.41 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.27/1.00 (H-J:1) , BC=0.41/1.00 (L-M:1) , WB=0.79/1.00 (C-R:1) , SSI=0.17/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.54 (J) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T14X	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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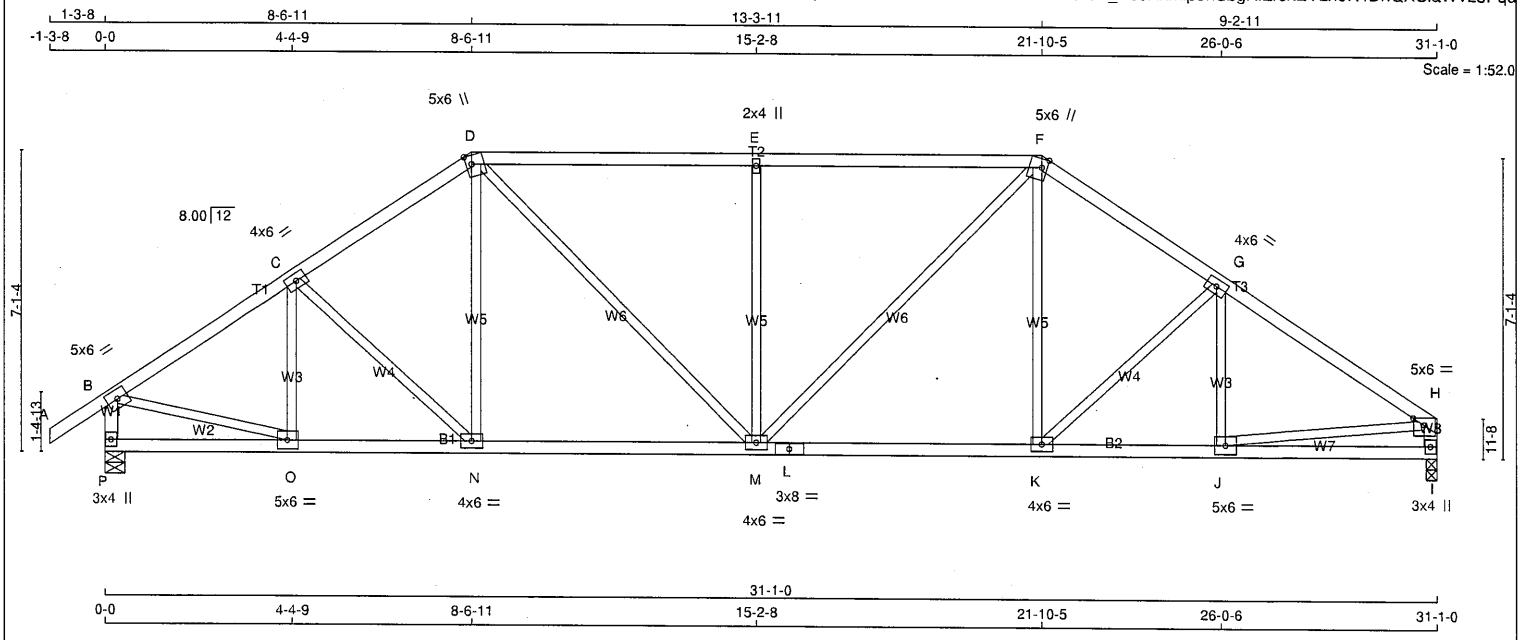

TOTAL WEIGHT = 3 X 132 = 396 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1839	0	1839	0	0	5-8	2-8
I	1714	0	1714	0	0	3-0	2-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1298	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
I	1212	795 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C		-1974 / 0	-91.8	-91.8	0.26 (1)	4.58
C- D		-1914 / 0	-91.8	-91.8	0.25 (1)	4.64
D- E		-2035 / 0	-91.8	-91.8	0.60 (1)	4.05
E- F		-2035 / 0	-91.8	-91.8	0.60 (1)	4.05
F- G		-2003 / 0	-91.8	-91.8	0.30 (1)	4.50
G- H		-2225 / 0	-91.8	-91.8	0.35 (1)	4.27
P- B		-1802 / 0	0.0	0.0	0.19 (1)	6.23
I- H		-1670 / 0	0.0	0.0	0.17 (1)	6.42
P- O		0 / 0	-18.5	-18.5	0.08 (4)	10.00
O- N		0 / 1663	-18.5	-18.5	0.34 (1)	10.00
N- M		0 / 1571	-18.5	-18.5	0.34 (1)	10.00
M- L		0 / 1643	-18.5	-18.5	0.35 (1)	10.00
L- K		0 / 1643	-18.5	-18.5	0.35 (1)	10.00
K- J		0 / 1872	-18.5	-18.5	0.37 (1)	10.00
J- I		0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C		-336 / 0	0.09 (1)
C- N		-130 / 0	0.08 (1)
N- D		0 / 196	0.05 (4)
D- M		0 / 663	0.15 (1)
M- E		-750 / 0	0.66 (1)
M- F		0 / 561	0.13 (1)
K- F		0 / 325	0.07 (1)
K- G		-321 / 0	0.19 (1)
J- G		-154 / 21	0.04 (1)
B- O		0 / 1713	0.39 (1)
J- H		0 / 1887	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.37/1.00 (J-K:1) , WB=0.66/1.00 (E-M:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.51 (L) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307596

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T15	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:27:50 2023 Page 1
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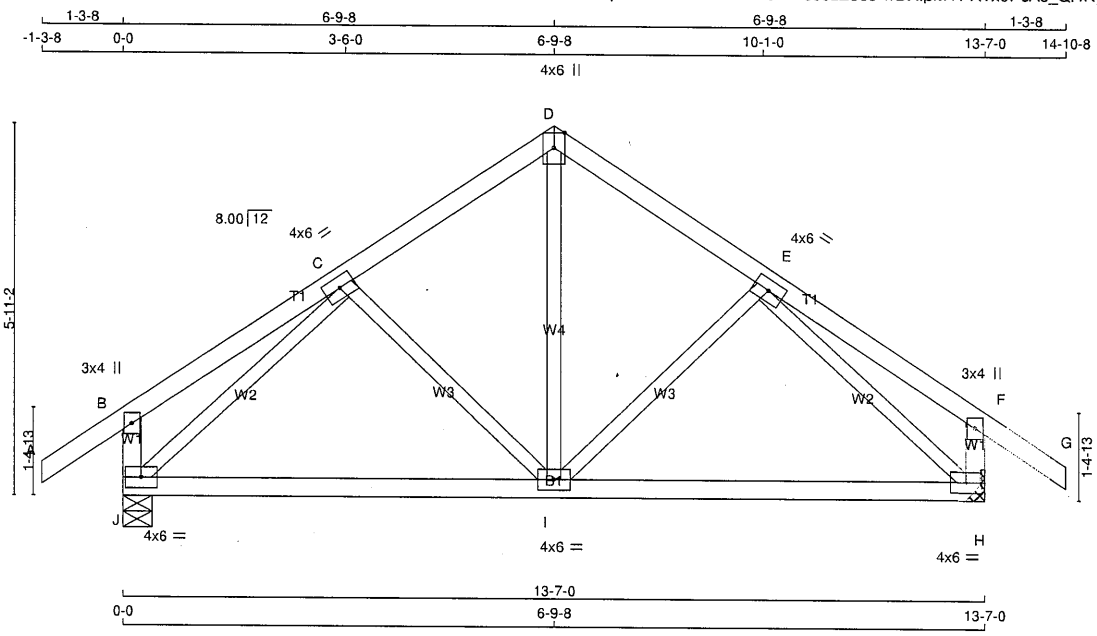

TOTAL WEIGHT = 4 X 59 = 236 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	875	0	875	0	0	5-8	1-8
H	875	0	875	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	616	418 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	616	418 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12	(1)	10.00	I- D	0 / 385	0.09 (1)
B- C	0 / 19	-91.8	-91.8	0.16	(1)	10.00	I- E	-165 / 0	0.06 (1)
C- D	-585 / 0	-91.8	-91.8	0.13	(1)	6.25	C- I	-165 / 0	0.06 (1)
D- E	-585 / 0	-91.8	-91.8	0.13	(1)	6.25	J- C	-827 / 0	0.30 (1)
E- F	0 / 19	-91.8	-91.8	0.16	(1)	10.00	E- H	-827 / 0	0.30 (1)
F- G	0 / 35	-91.8	-91.8	0.12	(1)	10.00			
J- B	-246 / 0	0.0	0.0	0.03	(1)	7.81			
H- F	-246 / 0	0.0	0.0	0.03	(1)	7.81			
J- I	0 / 589	-18.5	-18.5	0.29	(4)	10.00			
I- H	0 / 589	-18.5	-18.5	0.29	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (E-F:1) , BC=0.29/1.00 (H-I:4) , WB=0.30/1.00 (C-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (J) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 0.95)

Structural component only
DWG# T-2307597

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T15G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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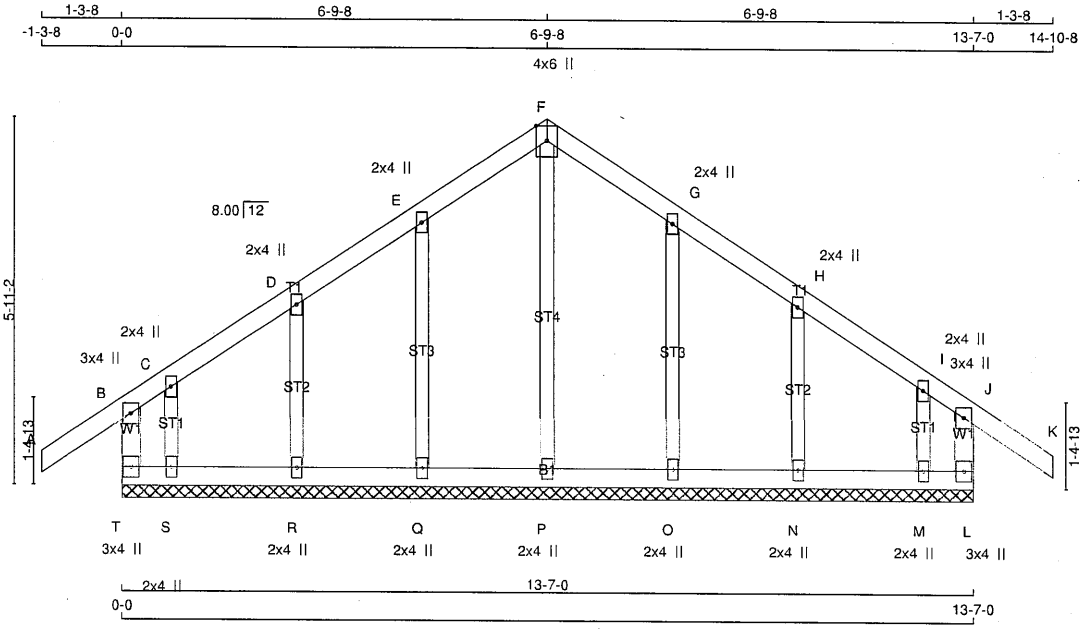

TOTAL WEIGHT = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, N, O, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-212 / 0	0.0	0.0	0.03 (1)	7.81	P- F	-224 / 0	0.12 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	Q- E	-186 / 0	0.06 (1)
B- C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25	R- D	-188 / 0	0.04 (1)
C- D	0 / 25	-91.8	-91.8	0.05 (1)	10.00	S- C	-52 / 0	0.01 (1)
D- E	0 / 28	-91.8	-91.8	0.05 (1)	10.00	O- G	-186 / 0	0.06 (1)
E- F	0 / 31	-91.8	-91.8	0.05 (1)	10.00	N- H	-188 / 0	0.04 (1)
F- G	0 / 31	-91.8	-91.8	0.05 (1)	10.00	M- I	-52 / 0	0.01 (1)
G- H	0 / 28	-91.8	-91.8	0.05 (1)	10.00			
H- I	0 / 25	-91.8	-91.8	0.05 (1)	10.00			
I- J	-17 / 0	-91.8	-91.8	0.09 (1)	6.25			
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
L- J	-212 / 0	0.0	0.0	0.03 (1)	7.81			
T- S	-15 / 0	-18.5	-18.5	0.01 (4)	6.25			
S- R	-18 / 0	-18.5	-18.5	0.02 (4)	6.25			
R- Q	-23 / 0	-18.5	-18.5	0.02 (4)	6.25			
Q- P	-27 / 0	-18.5	-18.5	0.01 (4)	6.25			
P- O	-27 / 0	-18.5	-18.5	0.01 (4)	6.25			
O- N	-23 / 0	-18.5	-18.5	0.02 (4)	6.25			
N- M	-18 / 0	-18.5	-18.5	0.02 (4)	6.25			
M- L	-15 / 0	-18.5	-18.5	0.01 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (J-K:1) , BC=0.02/1.00 (N-O:4) , WB=0.12/1.00 (F-P:1) , SSI=0.08/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (H) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 0.95)

Structural component only
DWG# T-2307598

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T16	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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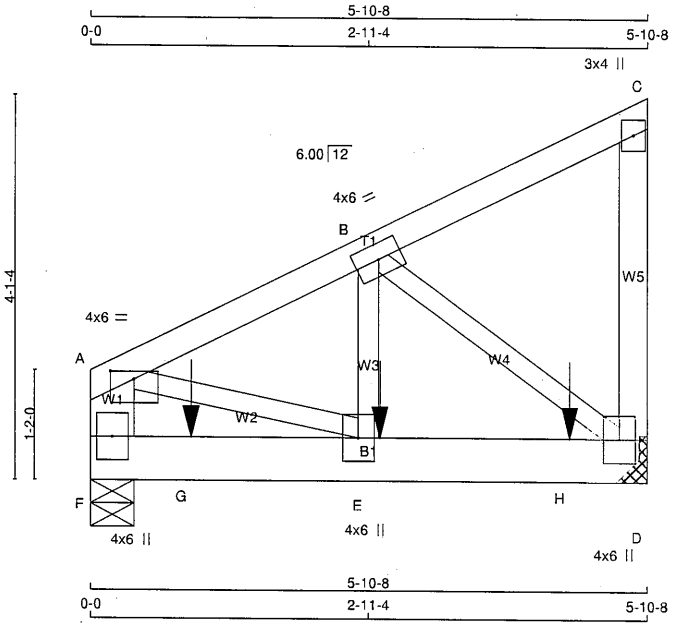

Scale = 1:23.5

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(40.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2307599

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1646	0	1646	0	0	MECHANICAL	
F	1572	0	1572	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1158	795 / 0	0 / 0	0 / 0	0 / 0	363 / 0	0 / 0
F	1106	759 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1387 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	0 / 1263	0.16 (1)
B- C	-11 / 0	-91.8	-91.8	0.05 (1)	6.25	B- D	-1573 / 0	0.19 (1)
D- C	-111 / 0	0.0	0.0	0.01 (1)	7.81	A- E	0 / 1293	0.16 (1)
F- A	-1066 / 0	0.0	0.0	0.04 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E- H	0 / 1250	-18.5	-18.5	0.25 (1)	10.00			
H- D	0 / 1250	-18.5	-18.5	0.25 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-601	-601	---	BACK	VERT	TOTAL	---	C1
G	1-0-12	-601	-601	---	BACK	VERT	TOTAL	---	C1
H	5-0-12	-603	-603	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.25/1.00 (D-E:1) , WB=0.19/1.00 (B-D:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T16	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-tcf2E2Zf3dCkewKF6qnYPNs92_CqHiqsDA_E7qz8Pqr

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307599

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T16Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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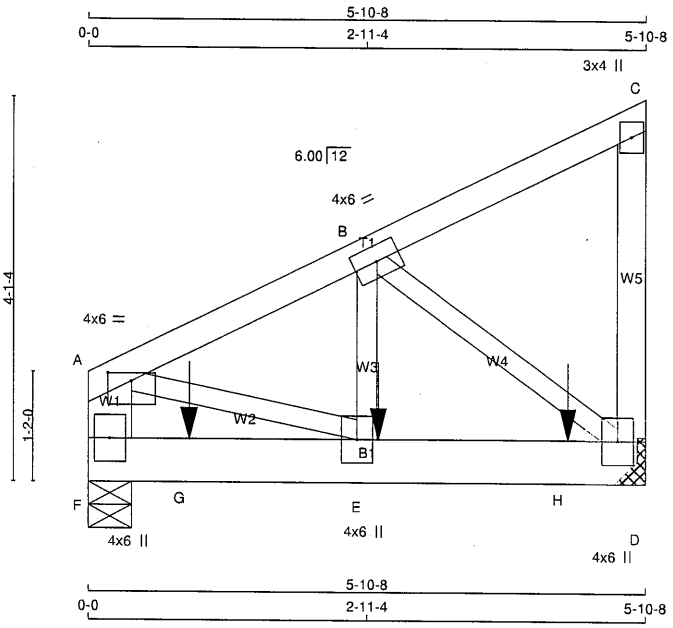

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(31.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		

Structural component only
DWG# T-2307600

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1633	0	1633	0	0	MECHANICAL	
F	1691	0	1691	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1152	772 / 0	0 / 0	0 / 0	0 / 0	381 / 0	0 / 0
F	1206	737 / 0	0 / 0	0 / 0	0 / 0	468 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1417 / 0	-91.8	-91.8	1	0.06	6.25	E- B	0 / 1297	0.16 (1)
B- C	-10 / 0	-91.8	-91.8	1	0.05	6.25	B- D	-1606 / 0	0.19 (1)
D- C	-111 / 0	0.0	0.0	1	0.01	7.81	A- E	0 / 1320	0.16 (1)
F- A	-1086 / 0	0.0	0.0	1	0.04	7.81			
F- G	0 / 0	-18.5	-18.5	1	0.23	10.00			
G- E	0 / 0	-18.5	-18.5	1	0.23	10.00			
E- H	0 / 1277	-18.5	-18.5	1	0.24	10.00			
H- D	0 / 1277	-18.5	-18.5	1	0.24	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-0-12	-579	-579	---	FRONT	VERT	TOTAL	---	C1
G	1-0-12	-741	-741	---	TOP	VERT	TOTAL	---	C1
H	5-0-12	-580	-580	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.24/1.00 (D-E:1) , WB=0.19/1.00 (B-D:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (D) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T16Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-p_npfibvaESSuDTeDFq0VoyVZouQlcG9gUTLCjz8Pqp

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307600

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T16Z2	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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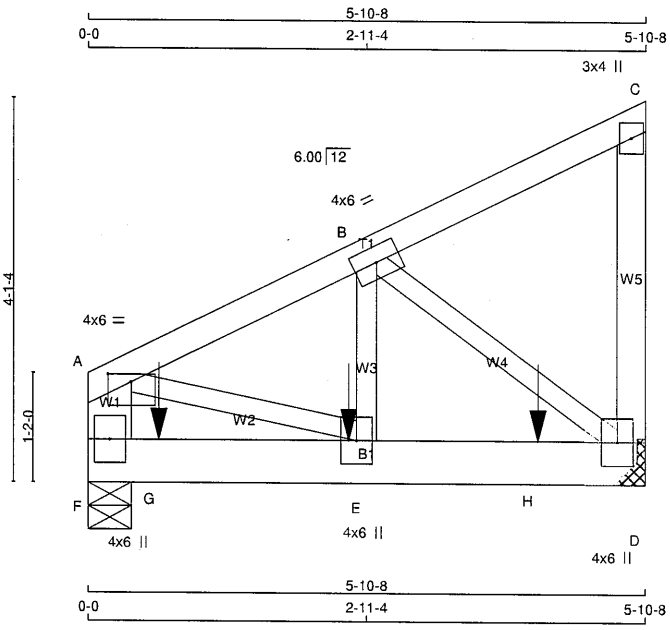

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2307601

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1335	0	1335	0	0	MECHANICAL	
F	1813	0	1813	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	942	631 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0
F	1281	848 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1260 / 0	-91.8	-91.8	0.06	(1)	6.25	E- B	0 / 1118	0.14 (1)
B- C	-11 / 0	-91.8	-91.8	0.05	(1)	6.25	B- D	-1430 / 0	0.17 (1)
D- C	-110 / 0	0.0	0.0	0.01	(1)	7.81	A- E	0 / 1175	0.15 (1)
F- A	-978 / 0	0.0	0.0	0.03	(1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.23	(1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.23	(1)	10.00			
E- H	0 / 1137	-18.5	-18.5	0.22	(1)	10.00			
H- D	0 / 1137	-18.5	-18.5	0.22	(1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-9-0	-463	-463	---	BACK	VERT	TOTAL	---	C1
G	9-0	-839	-839	---	BACK	VERT	TOTAL	---	C1
H	4-9-0	-463	-463	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.23/1.00 (E-F:1) , WB=0.17/1.00 (B-D:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (D) (INPUT = 0.90)
JSI METAL= 0.26 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	T16Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-HBLBs3bYLYaJVN2qnzLF1?UgICEvU2rJv8Cuk9z8Pqo

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307601

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T17	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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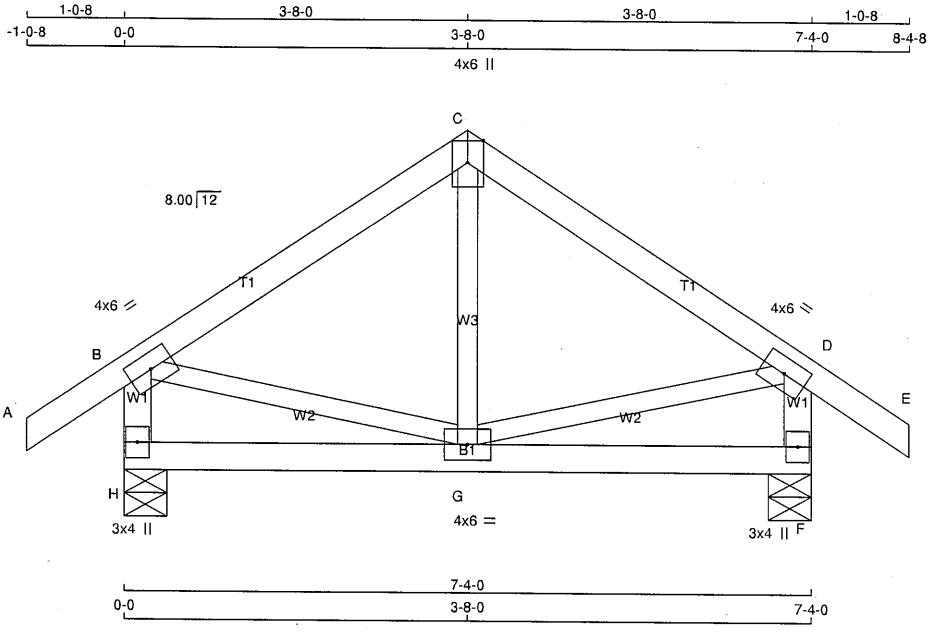

TOTAL WEIGHT = 3 X 31 = 94 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1).
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	507	0	507	0	0	5-8	1-8
F	507	0	507	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	357	245 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	357	245 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 29		-91.8	-91.8	0.08 (1)	10.00	G- C	-32 / 52		0.02 (4)
B- C	-275 / 0		-91.8	-91.8	0.16 (1)	6.25	B- G	0 / 235		0.05 (1)
C- D	-275 / 0		-91.8	-91.8	0.16 (1)	6.25	G- D	0 / 235		0.05 (1)
D- E	0 / 29		-91.8	-91.8	0.08 (1)	10.00				
H- B	-480 / 0		0.0	0.0	0.05 (1)	7.81				
F- D	-480 / 0		0.0	0.0	0.05 (1)	7.81				
H- G	0 / 0		-18.5	-18.5	0.07 (4)	10.00				
G- F	0 / 0		-18.5	-18.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.07/1.00 (G-H:4) , WB=0.05/1.00 (D-G:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (G) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307602

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T17G	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:27:57 2023 Page 1
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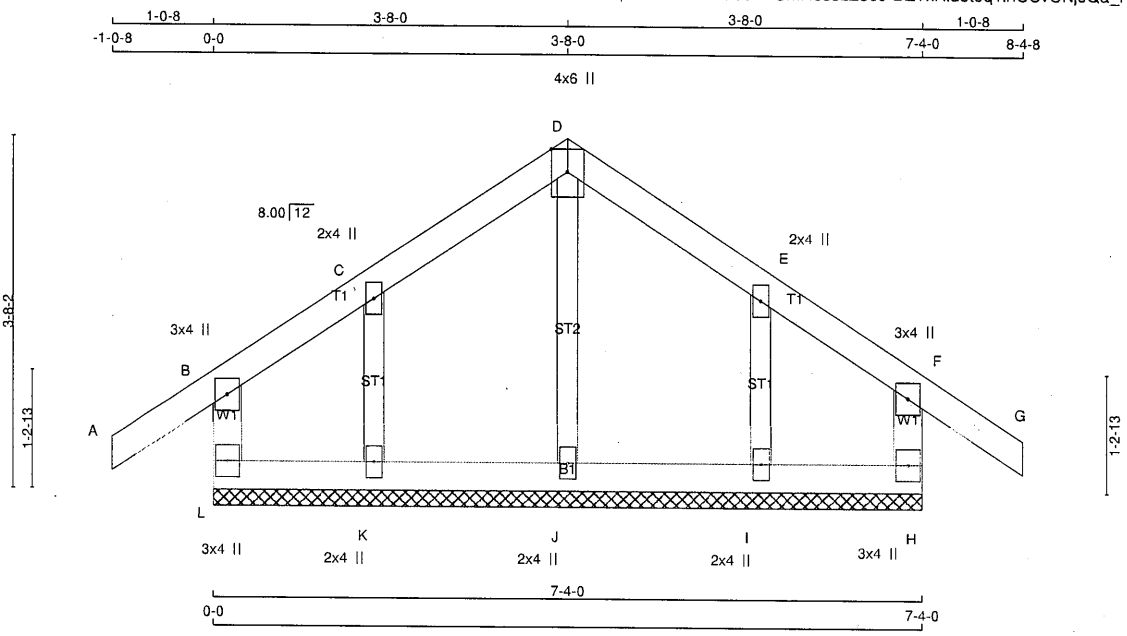

TOTAL WEIGHT = 3 X 29 = 87 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-183 / 0	0.0	0.0	0.02	(1)	7.81	J- D	-180 / 0	0.04 (1)
A- B	0 / 29	-91.8	-91.8	0.08	(1)	10.00	K- C	-166 / 0	0.03 (1)
B- C	-21 / 0	-91.8	-91.8	0.04	(1)	6.25	I- E	-166 / 0	0.03 (1)
C- D	-14 / 0	-91.8	-91.8	0.05	(1)	6.25			
D- E	-14 / 0	-91.8	-91.8	0.05	(1)	6.25			
E- F	-21 / 0	-91.8	-91.8	0.04	(1)	6.25			
F- G	0 / 29	-91.8	-91.8	0.08	(1)	10.00			
H- F	-183 / 0	0.0	0.0	0.02	(1)	7.81			
L- K	0 / 21	-18.5	-18.5	0.01	(4)	10.00			
K- J	0 / 14	-18.5	-18.5	0.02	(4)	10.00			
J- I	0 / 14	-18.5	-18.5	0.02	(4)	10.00			
I- H	0 / 21	-18.5	-18.5	0.01	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1) , BC=0.02/1.00 (I-J:4) , WB=0.04/1.00 (D-J:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307603

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T18	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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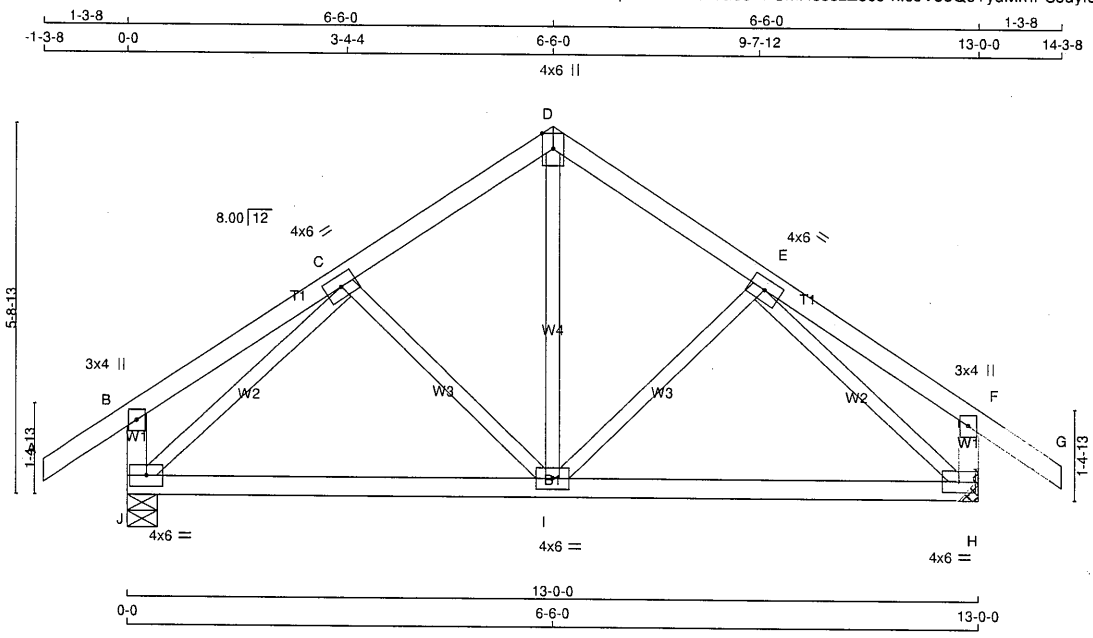

TOTAL WEIGHT = 2 X 57 = 114 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	843	0	843	0	0	IN-SX	IN-SX
H	843	0	843	0	0	5-8	1-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	594	403 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
H	594	403 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)		10.00	I- D	0 / 363	0.08 (1)
B- C	0 / 18	-91.8	-91.8	0.15 (1)		10.00	I- E	-152 / 0	0.05 (1)
C- D	-555 / 0	-91.8	-91.8	0.12 (1)		6.25	C- I	-152 / 0	0.05 (1)
D- E	-555 / 0	-91.8	-91.8	0.12 (1)		6.25	J- C	-785 / 0	0.26 (1)
E- F	0 / 18	-91.8	-91.8	0.15 (1)		10.00	E- H	-785 / 0	0.26 (1)
F- G	0 / 35	-91.8	-91.8	0.12 (1)		10.00			
J- B	-241 / 0	0.0	0.0	0.02 (1)		7.81			
H- F	-241 / 0	0.0	0.0	0.02 (1)		7.81			
J- I	0 / 555	-18.5	-18.5	0.27 (4)		10.00			
I- H	0 / 555	-18.5	-18.5	0.27 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.27/1.00 (I-J:4) , WB=0.26/1.00 (E-H:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307604

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T18S	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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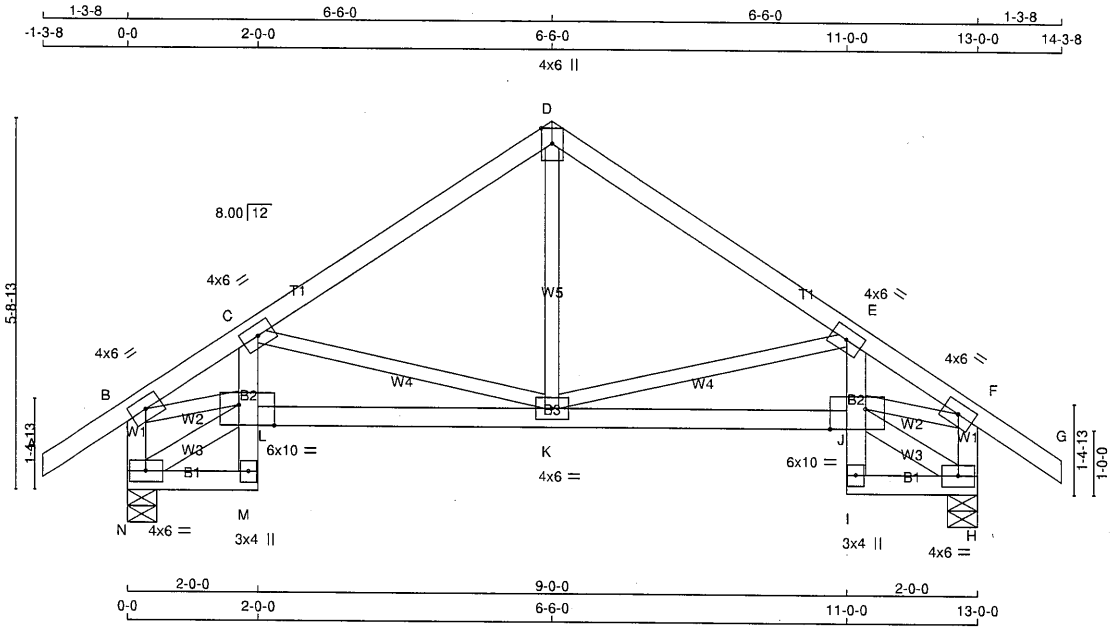

TOTAL WEIGHT = 2 X 61 = 121 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, C, E, F						
B	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	10.0	3.75	6.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	6.0	10.0	3.75	6.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1046	0	1046	0	0	5-8	1-8
H	1046	0	1046	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	756	403 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0
H	756	403 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	K- D	0 / 626	0.20 (4)
B- C	-1364 / 0	-91.8	-91.8	0.24 (1)	5.24	K- E	-461 / 0	0.18 (1)
C- D	-910 / 0	-91.8	-91.8	0.30 (1)	6.04	C- K	-461 / 0	0.18 (1)
D- E	-910 / 0	-91.8	-91.8	0.30 (1)	6.04	N- L	-57 / 0	0.01 (1)
E- F	-1364 / 0	-91.8	-91.8	0.24 (1)	5.24	B- L	0 / 1150	0.28 (1)
F- G	0 / 35	-91.8	-91.8	0.14 (1)	10.00	J- H	-57 / 0	0.01 (1)
N- B	-996 / 0	0.0	0.0	0.11 (1)	7.80	J- F	0 / 1150	0.28 (1)
H- F	-996 / 0	0.0	0.0	0.11 (1)	7.80			
N- M	0 / 50	-18.5	-36.3	0.04 (4)	10.00			
M- L	0 / 32	0.0	0.0	0.08 (1)	10.00			
L- C	0 / 87	0.0	0.0	0.08 (4)	10.00			
L- K	0 / 1187	-36.3	-81.0	0.57 (4)	10.00			
K- J	0 / 1187	-81.0	-36.3	0.57 (4)	10.00			
I- J	0 / 32	0.0	0.0	0.08 (1)	10.00			
J- E	0 / 87	0.0	0.0	0.08 (4)	10.00			
I- H	0 / 50	-36.3	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.57/1.00 (K-L:4) , WB=0.28/1.00 (B-L:1) , SSI=0.28/1.00 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL.= 0.250 Inches

PLATE ROTATION TOL.= 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 0.95)

Structural component only
DWG# T-2307605

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T19	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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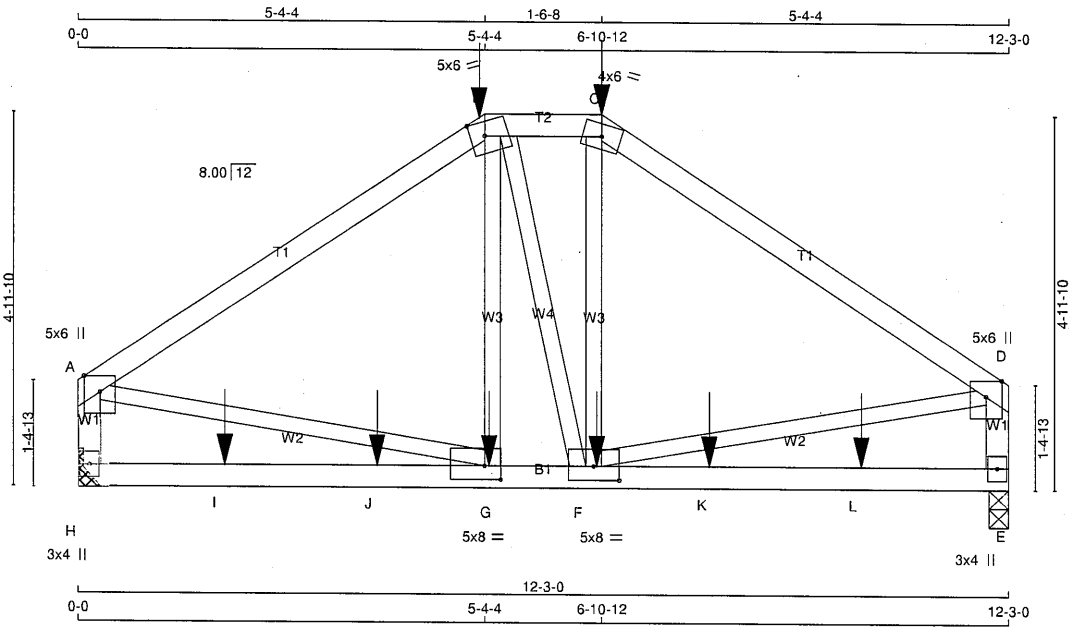

TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.25	2.25
C	TTW-m	MT20	4.0	6.0		
D	TMVW+p	MT20	5.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	5.0	8.0	2.25	4.00
G	BMWW-t	MT20	5.0	8.0	2.25	2.50
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307606

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1203	0	1203	0	0	MECHANICAL	
E	1203	0	1203	0	0	3-0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	852	552 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
E	852	552 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1240 / 0	-91.8	-91.8	0.58 (1)	4.93	G- B	-64 / 84	0.03 (4)
B- C	-1030 / 0	-91.8	-91.8	0.05 (1)	6.11	B- F	0 / 8	0.00 (4)
C- D	-1242 / 0	-91.8	-91.8	0.58 (1)	4.92	F- C	-60 / 91	0.03 (4)
H- A	-1139 / 0	0.0	0.0	0.13 (1)	7.42	A- G	0 / 1050	0.26 (1)
E- D	-1137 / 0	0.0	0.0	0.13 (1)	7.42	F- D	0 / 1051	0.26 (1)
H- I	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
I- J	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
J- G	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
G- F	0 / 1029	-18.5	-18.5	0.34 (4)	10.00			
F- K	0 / 0	-18.5	-18.5	0.24 (4)	10.00			
K- L	0 / 0	-18.5	-18.5	0.24 (4)	10.00			
L- E	0 / 0	-18.5	-18.5	0.24 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-4-4	-300	-300	---	BACK	VERT	TOTAL	---	C1
C	6-10-12	-300	-300	---	BACK	VERT	TOTAL	---	C1
F	6-10-0	-25	-25	---	BACK	VERT	TOTAL	---	C1
G	5-5-0	-25	-25	---	BACK	VERT	TOTAL	---	C1
I	1-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
J	3-11-4	-25	-25	---	BACK	VERT	TOTAL	---	C1
K	8-3-12	-25	-25	---	BACK	VERT	TOTAL	---	C1
L	10-3-12	-25	-25	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.58/1.00 (C-D:1) , BC=0.34/1.00 (F-G:4) , WB=0.26/1.00 (D-F:1) , SSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.51 (D) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T19Z	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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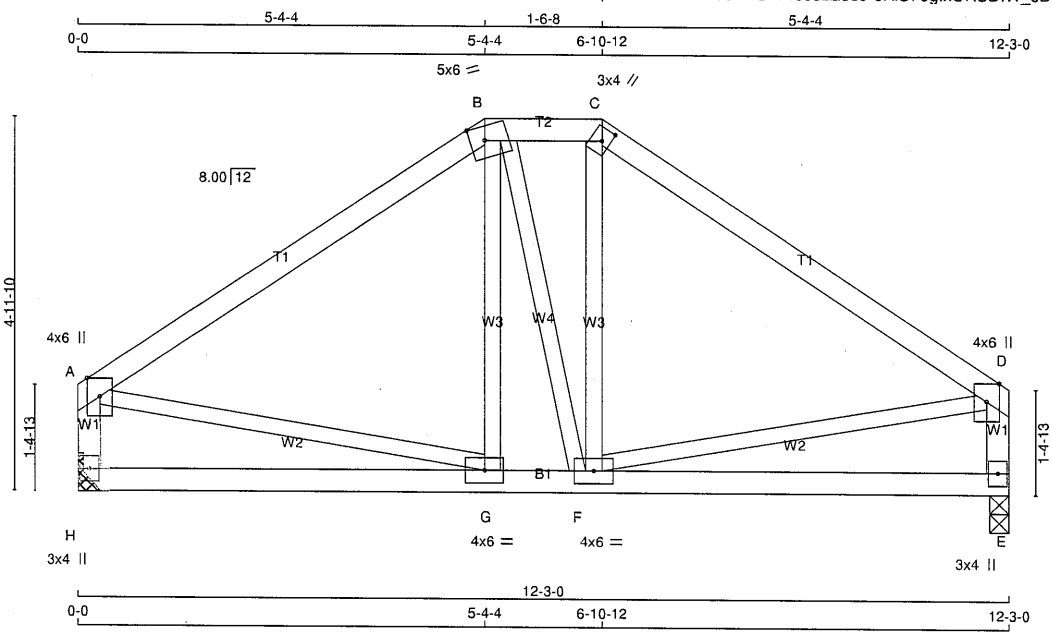

TOTAL WEIGHT = 52 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.25	2.25
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	675	0	675	0	0	MECHANICAL	
E	675	0	675	0	0	3-0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	478	313 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
E	478	313 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-538 / 0	-91.8	-91.8	0.34 (1)	6.25	G- B	-20 / 44	0.02 (4)
B- C	-447 / 0	-91.8	-91.8	0.03 (1)	6.25	B- F	0 / 5	0.00 (4)
C- D	-539 / 0	-91.8	-91.8	0.34 (1)	6.25	F- C	-16 / 49	0.02 (4)
H- A	-633 / 0	0.0	0.0	0.07 (1)	7.81	A- G	0 / 456	0.10 (1)
E- D	-633 / 0	0.0	0.0	0.07 (1)	7.81	F- D	0 / 457	0.10 (1)
H- G	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
G- F	0 / 447	-18.5	-18.5	0.16 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.34/1.00 (C-D:1) , BC=0.16/1.00 (F-G:4) , WB=0.10/1.00 (D-F:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (F) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307607

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	T20	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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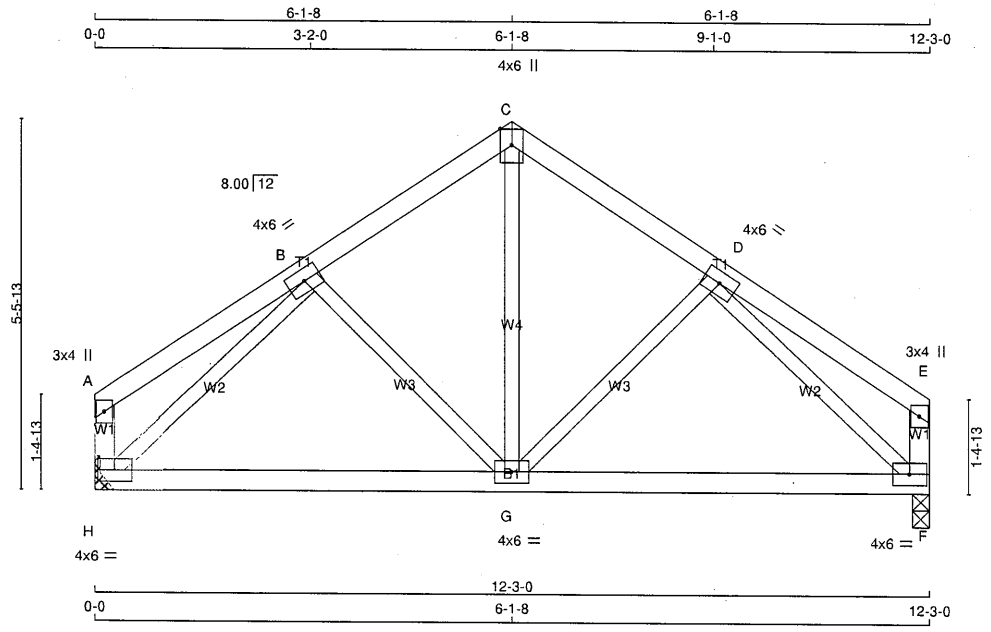

TOTAL WEIGHT = 50 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	675	0	675	0	0	MECHANICAL	
F	675	0	675	0	0	3-0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	478	313 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
F	478	313 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-91.8	-91.8	0.13	(1)	10.00	G- C	0 / 334	0.08 (1)
B- C	-517 / 0	-91.8	-91.8	0.10	(1)	6.25	G- D	-137 / 0	0.04 (1)
C- D	-517 / 0	-91.8	-91.8	0.10	(1)	6.25	B- G	-137 / 0	0.04 (1)
D- E	0 / 17	-91.8	-91.8	0.13	(1)	10.00	H- B	-732 / 0	0.22 (1)
H- A	-109 / 0	0.0	0.0	0.01	(1)	7.81	D- F	-732 / 0	0.22 (1)
F- E	-109 / 0	0.0	0.0	0.01	(1)	7.81			
H- G	0 / 512	-18.5	-18.5	0.24	(4)	10.00			
G- F	0 / 512	-18.5	-18.5	0.24	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.24/1.00 (F-G:4) , WB=0.22/1.00 (B-H:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307608

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T31S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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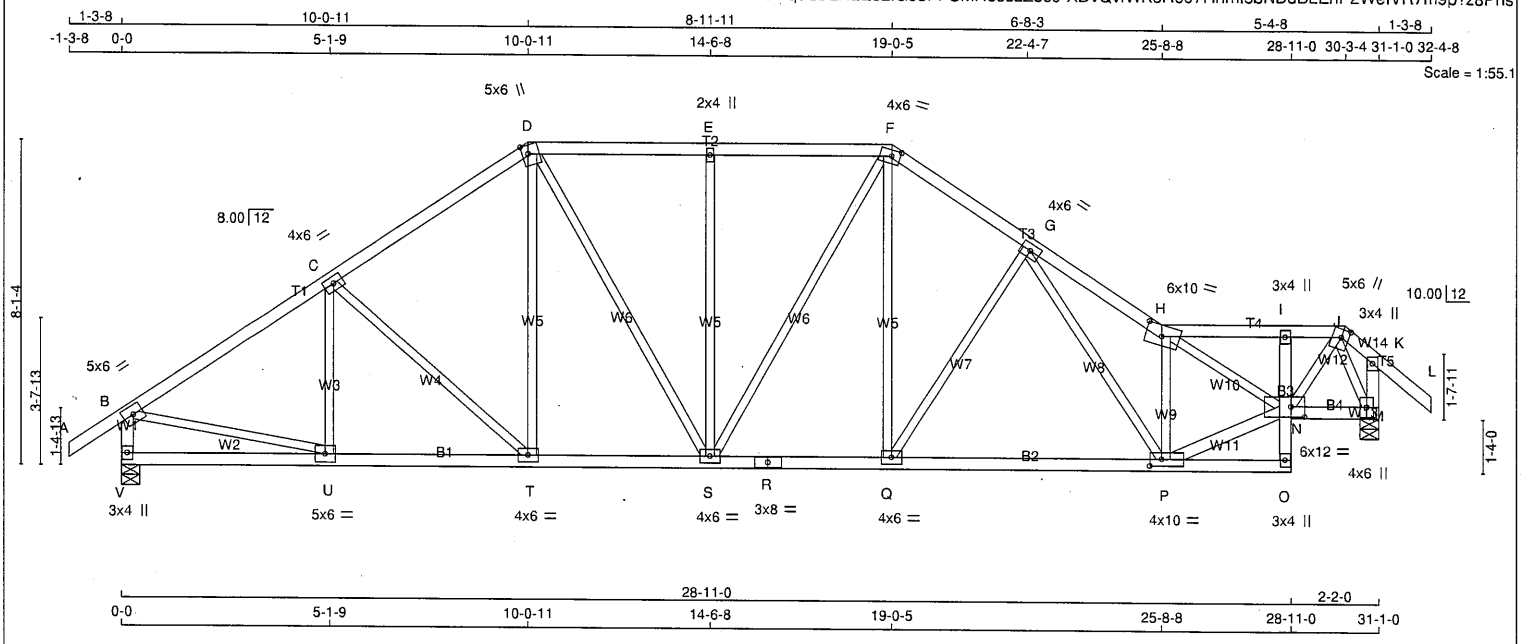

TOTAL WEIGHT = 152 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	6.0	10.0	3.25	4.75
I	TMV+p	MT20	3.0	4.0		
J	TTWW+m	MT20	5.0	6.0	2.50	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		
N	BVMWWW-t	MT20	6.0	12.0	3.00	4.00
O	BMV+p	MT20	3.0	4.0		
P	BMWWW-t	MT20	4.0	10.0	2.00	3.50
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
S	BMWWW-t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2307614

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	1837	0	1837	0	0	5-8	2-8
M	1843	0	1843	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1297	864 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
M	1301	868 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-2009 / 0	-91.8	-91.8	0.36 (1)	4.44
C- D	-1827 / 0	-91.8	-91.8	0.34 (1)	4.62
D- E	-1721 / 0	-91.8	-91.8	0.26 (1)	4.82
E- F	-1721 / 0	-91.8	-91.8	0.26 (1)	4.82
F- G	-1965 / 0	-91.8	-91.8	0.18 (1)	4.67
G- H	-2715 / 0	-91.8	-91.8	0.22 (1)	4.05
H- I	-1761 / 0	-91.8	-91.8	0.14 (1)	4.91
I- J	-1790 / 0	-91.8	-91.8	0.08 (1)	4.95
J- K	-65 / 0	-91.8	-91.8	0.09 (1)	6.25
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00
V- B	-1796 / 0	0.0	0.0	0.18 (1)	6.24
M- K	-268 / 0	0.0	0.0	0.03 (1)	7.81
V- U	0 / 0	-18.5	-18.5	0.11 (4)	10.00
U- T	0 / 1696	-18.5	-18.5	0.32 (1)	10.00
T- S	0 / 1496	-18.5	-18.5	0.29 (1)	10.00
S- R	0 / 1624	-18.5	-18.5	0.35 (1)	10.00
R- Q	0 / 1624	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 1946	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 45	-18.5	-18.5	0.13 (4)	10.00
O- N	0 / 16	0.0	0.0	0.08 (1)	10.00
N- I	-240 / 0	0.0	0.0	0.06 (1)	7.81
N- M	0 / 664	-18.5	-18.5	0.12 (1)	10.00

WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
U- C	-257 / 0	0.09 (1)
T- D	-273 / 0	0.23 (1)
T- D	0 / 274	0.06 (1)
D- S	0 / 452	0.10 (1)
S- E	-496 / 0	0.63 (1)
S- F	0 / 196	0.04 (1)
Q- F	0 / 627	0.14 (1)
Q- G	-606 / 0	0.47 (1)
G- P	0 / 646	0.15 (1)
P- H	-1443 / 0	0.31 (1)
N- J	0 / 1939	0.44 (1)
B- U	0 / 1733	0.39 (1)
P- N	0 / 2404	0.39 (1)
H- N	-569 / 0	0.13 (1)
J- M	-1689 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.40/1.00 (P-Q:1) , WB=0.63/1.00 (E-S:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.60 (J) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T31S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2IG33FPSMH8sozZ3c0-XDvQvfWKeHe97Hnml5bNDdBLEnP2WeIVR7m9p?z8Phs

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307614

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T32S	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:37:28 2023 Page 1
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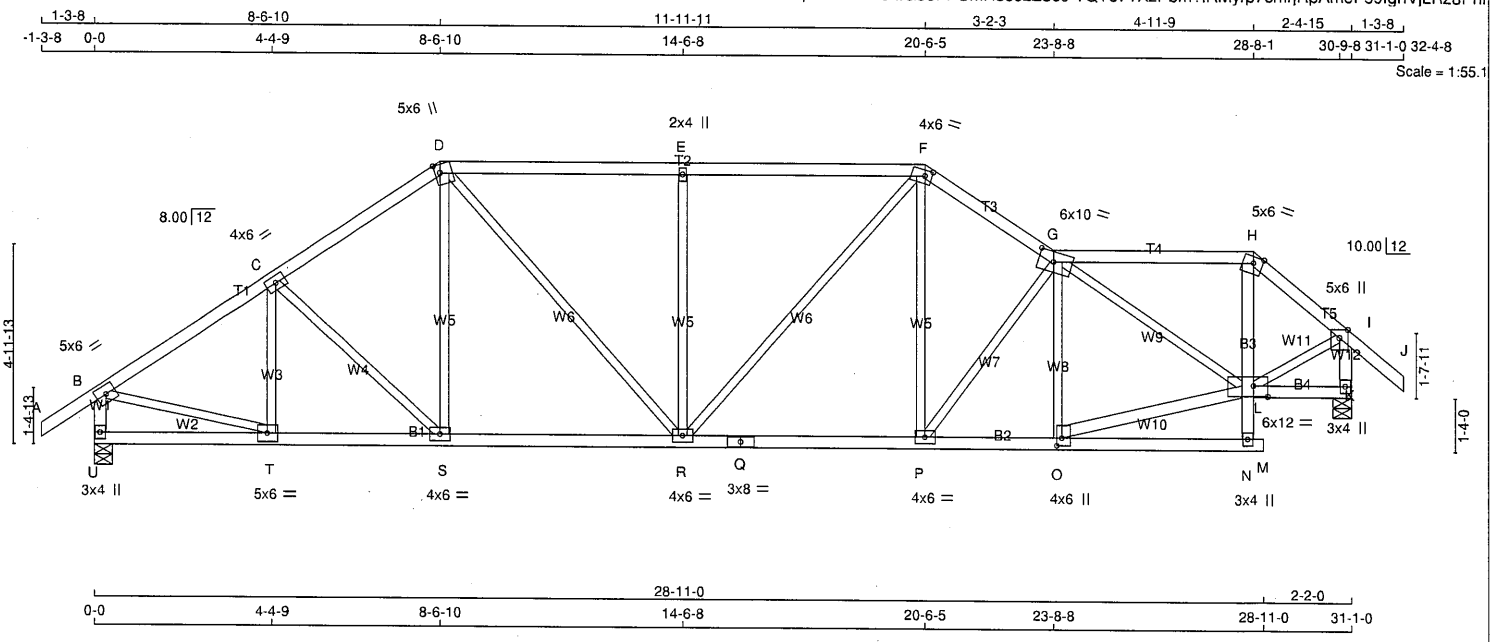

TOTAL WEIGHT = 3 X 147 = 442 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.00
G	TTWWW-m	MT20	6.0	10.0	3.00	4.50
H	TTV-m	MT20	5.0	6.0	Edge	2.75
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BVMWWW-l	MT20	6.0	12.0	3.25	4.25
N	BMV+p	MT20	3.0	4.0		
O	BMWW+t	MT20	4.0	6.0	2.25	1.50
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)
1)

Structural component only
DWG# T-2307615

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	1840	0	1840	0	0	IN-SX	IN-SX
K	1847	0	1847	0	0	5-8	2-8
						5-8	2-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1299	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
K	1305	866 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1976 / 0	-91.8	-91.8	0.35 (1)	4.46
C- D	-1913 / 0	-91.8	-91.8	0.34 (1)	4.52
D- E	-2013 / 0	-91.8	-91.8	0.69 (1)	3.91
E- F	-2013 / 0	-91.8	-91.8	0.69 (1)	3.91
F- G	-2111 / 0	-91.8	-91.8	0.22 (1)	4.47
G- H	-1191 / 0	-91.8	-91.8	0.43 (1)	5.27
H- I	-1533 / 0	-91.8	-91.8	0.12 (1)	5.20
I- J	0 / 41	-91.8	-91.8	0.13 (1)	10.00
U- B	-1803 / 0	0.0	0.0	0.19 (1)	6.23
K- I	-1824 / 0	0.0	0.0	0.19 (1)	6.20
U- T	0 / 0	-18.5	-18.5	0.07 (4)	10.00
T- S	0 / 1664	-18.5	-18.5	0.33 (1)	10.00
S- R	0 / 1571	-18.5	-18.5	0.32 (1)	10.00
R- Q	0 / 1765	-18.5	-18.5	0.35 (1)	10.00
Q- P	0 / 1765	-18.5	-18.5	0.35 (1)	10.00
P- O	0 / 2067	-18.5	-18.5	0.38 (1)	10.00
O- N	0 / 25	-18.5	-18.5	0.09 (4)	10.00
N- M	0 / 0	-18.5	-18.5	0.00 (4)	10.00
N- L	0 / 50	0.0	0.0	0.05 (1)	10.00
L- H	0 / 648	0.0	0.0	0.15 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-334 / 0	0.09 (1)
C- S	-132 / 0	0.08 (1)
S- D	0 / 189	0.05 (4)
D- R	0 / 670	0.15 (1)
R- E	-672 / 0	0.59 (1)
R- F	0 / 376	0.08 (1)
P- F	0 / 529	0.12 (1)
P- G	-540 / 0	0.30 (1)
O- G	-501 / 0	0.18 (1)
B- T	0 / 1714	0.39 (1)
L- I	0 / 1316	0.30 (1)
O- L	0 / 2111	0.34 (1)
G- L	-1053 / 0	0.59 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.69/1.00 (E-F:1) , BC=0.38/1.00 (O-P:1) , WB=0.59/1.00 (G-L:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.90 (O) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T33S	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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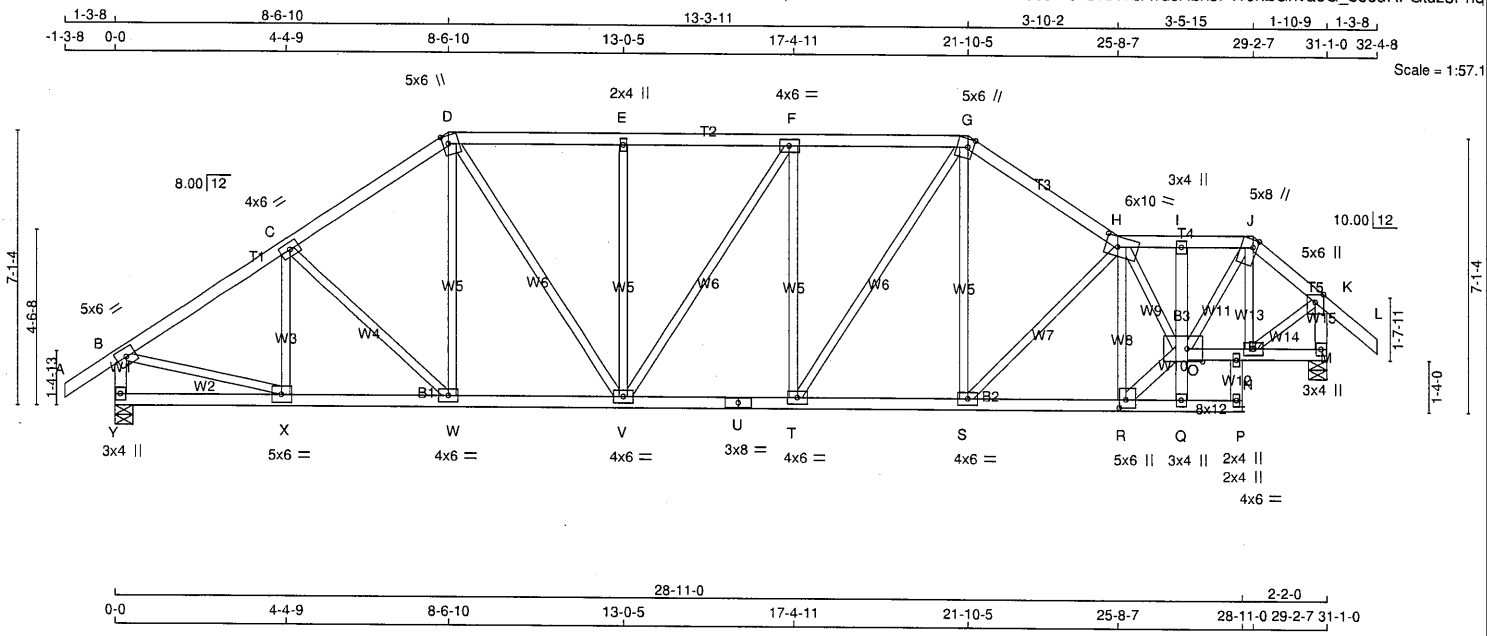

TOTAL WEIGHT = 3 X 158 = 474 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
Y - U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
Q - I	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	Edge	1.75
H	TTWWW-m	MT20	6.0	10.0	3.25	3.75
I	TMV+p	MT20	3.0	4.0		
J	TTWW+m	MT20	5.0	8.0	Edge	1.25
K	TMVW+p	MT20	5.0	6.0	Edge	
M	BMV1+p	MT20	3.0	4.0		
N, S, T, W						
N	BMWW-t	MT20	4.0	6.0		
O	BVMWWW-t	MT20	8.0	12.0	4.00	4.75
P	NP+w	MT20	2.0	4.0		
Q	BMV+p	MT20	3.0	4.0		
R	BMWW+t	MT20	5.0	6.0	2.50	2.00
U	BS-t	MT20	3.0	8.0		
V	BMWWW-t	MT20	4.0	6.0		
X	BMWW-t	MT20	5.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		
Z	NP+w	MT20	2.0	4.0		

Structural component only
DWG# T-2307616

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Y	1839	0	1839	0	0	IN-SX	2-8
M	1870	0	1870	0	0	IN-SX	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1299	864 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
M	1322	868 / 0	0 / 0	0 / 0	0 / 0	454 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	LC1 TO	MAX (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	
B- C	-1976 / 0	-91.8	-91.8	0.35 (1)	4.46	
C- D	-1910 / 0	-91.8	-91.8	0.34 (1)	4.52	
D- E	-1930 / 0	-91.8	-91.8	0.29 (1)	4.55	
E- F	-1930 / 0	-91.8	-91.8	0.30 (1)	4.54	
F- G	-1956 / 0	-91.8	-91.8	0.30 (1)	4.52	
G- H	-1964 / 0	-91.8	-91.8	0.30 (1)	4.52	
H- I	-1909 / 0	-91.8	-91.8	0.09 (1)	4.80	
I- J	-1929 / 0	-91.8	-91.8	0.10 (1)	4.77	
J- K	-1427 / 0	-91.8	-91.8	0.14 (1)	5.32	
K- L	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
Y- B	-1803 / 0	0.0	0.0	0.19 (1)	6.23	
M- K	-1862 / 0	0.0	0.0	0.20 (1)	6.14	
Y- X	0 / 0	-18.5	-18.5	0.07 (4)	10.00	
X- W	0 / 1664	-18.5	-18.5	0.31 (1)	10.00	
W- V	0 / 1568	-18.5	-18.5	0.29 (1)	10.00	
V- U	0 / 1956	-18.5	-18.5	0.35 (1)	10.00	
U- T	0 / 1956	-18.5	-18.5	0.35 (1)	10.00	
T- S	0 / 1638	-18.5	-18.5	0.30 (1)	10.00	
S- R	0 / 1805	-18.5	-18.5	0.33 (1)	10.00	
R- Q	0 / 45	-18.5	-18.5	0.06 (4)	10.00	
Q- P	0 / 0	-18.5	-18.5	0.05 (4)	10.00	
Q- O	0 / 51	0.0	0.0	0.09 (1)	10.00	
O- I	-101 / 0	0.0	0.0	0.08 (1)	7.81	
O- N	0 / 1043	-18.5	-18.5	0.20 (1)	10.00	
N- M	0 / 0	-18.5	-18.5	0.03 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-330 / 0	0.09 (1)
C- W	-135 / 0	0.08 (1)
W- D	0 / 179	0.04 (4)
D- G	0 / 256	0.06 (1)
S- H	-250 / 0	0.14 (1)
R- H	-1431 / 0	0.44 (1)
B- X	0 / 1714	0.39 (1)
R- O	0 / 2279	0.37 (1)
H- O	0 / 282	0.06 (1)
O- J	0 / 1646	0.37 (1)
N- J	-643 / 0	0.12 (1)
N- K	0 / 1276	0.29 (1)
T- G	0 / 579	0.13 (1)
D- V	0 / 659	0.15 (1)
T- F	-399 / 0	0.35 (1)
V- E	-435 / 0	0.38 (1)
V- F	-48 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.35/1.00 (T-V:1) , WB=0.44/1.00 (H-R:1) , SSI=0.19/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.62 (R) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T33S	3	1	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307616

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T34S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:37:30 2023 Page 1
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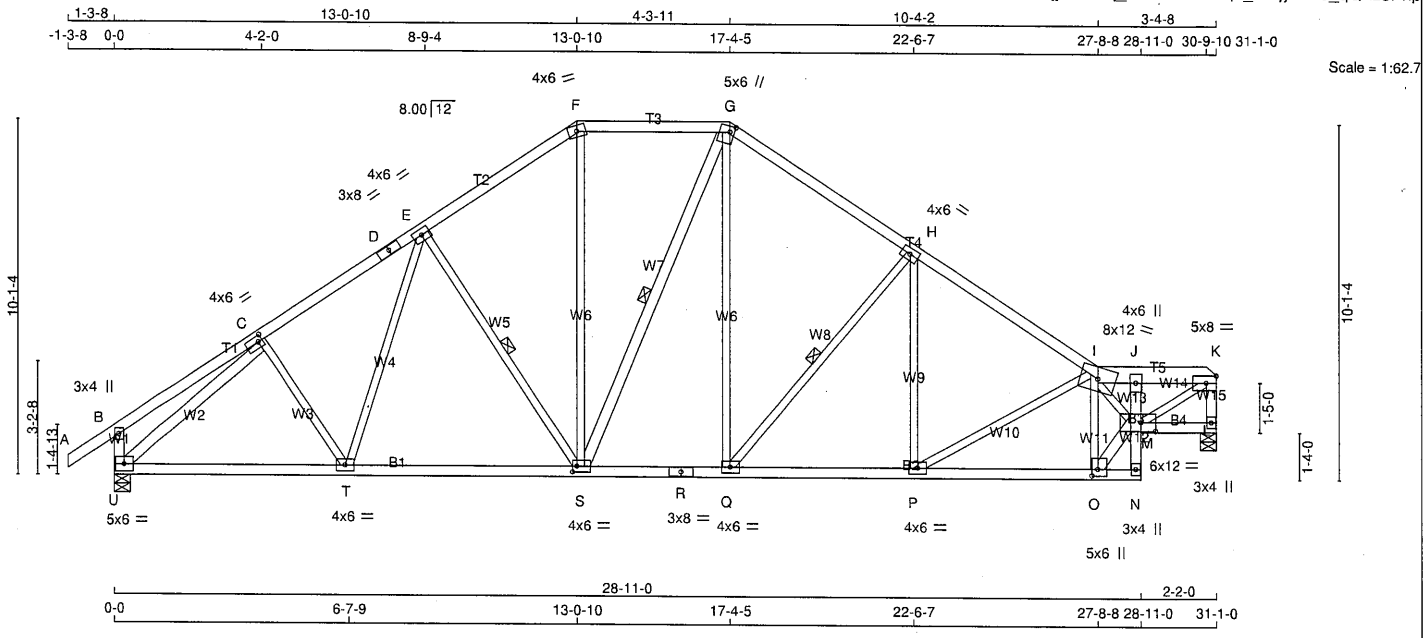

TOTAL WEIGHT = 157 lb
{M}{F}

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW-t	MT20	4.0	6.0		
I	TTWWW-m	MT20	8.0	12.0		
J	TMV+p	MT20	4.0	6.0		
K	TMVW-t	MT20	5.0	8.0		Edge
L	BMV1+p	MT20	3.0	4.0		
M	BVMWWW-t	MT20	6.0	12.0	3.00	5.00
N	BMV+p	MT20	3.0	4.0		
O	BMWW+t	MT20	5.0	6.0	2.25	2.00
P, Q, T						
P	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
S	BMWWW-t	MT20	4.0	6.0	2.00	1.50
U	BMVW1-t	MT20	5.0	6.0		

Structural component only
DWG# T-2307617

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1714	0	1714	0	0	IN-SX	5-8	2-1
U	1839	0	1839	0	0	IN-SX	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1212	795 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0
U	1298	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, G-S, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- T	0 / 55	0.02 (4)
B- C	0 / 21	-91.8	-91.8	0.21 (1)	10.00	T- E	0 / 138	0.04 (4)
C- D	-1949 / 0	-91.8	-91.8	0.26 (1)	4.61	E- S	-477 / 0	0.22 (1)
D- E	-1949 / 0	-91.8	-91.8	0.26 (1)	4.61	S- F	0 / 532	0.12 (1)
E- F	-1618 / 0	-91.8	-91.8	0.24 (1)	4.98	S- G	-34 / 0	0.02 (1)
F- G	-1330 / 0	-91.8	-91.8	0.24 (1)	5.36	Q- G	0 / 549	0.12 (1)
G- H	-1642 / 0	-91.8	-91.8	0.36 (1)	4.80	Q- H	-610 / 0	0.28 (1)
H- I	-2050 / 0	-91.8	-91.8	0.39 (1)	4.36	P- H	0 / 145	0.05 (4)
I- J	-2187 / 0	-91.8	-91.8	0.16 (1)	5.36	P- I	-103 / 0	0.06 (1)
J- K	-2269 / 0	-91.8	-91.8	0.16 (1)	5.28	O- I	-1973 / 0	0.38 (1)
U- B	-273 / 0	0.0	0.0	0.03 (1)	7.81	M- K	0 / 2704	0.61 (1)
L- K	-1692 / 0	0.0	0.0	0.18 (1)	6.39	U- C	-2220 / 0	0.79 (1)
						O- M	0 / 2739	0.44 (1)
U- T	0 / 1622	-18.5	-18.5	0.36 (1)	10.00	I- M	0 / 530	0.12 (1)
T- S	0 / 1584	-18.5	-18.5	0.35 (1)	10.00			
S- R	0 / 1344	-18.5	-18.5	0.28 (1)	10.00			
R- Q	0 / 1344	-18.5	-18.5	0.28 (1)	10.00			
Q- P	0 / 1728	-18.5	-18.5	0.33 (1)	10.00			
P- O	0 / 1818	-18.5	-18.5	0.34 (1)	10.00			
O- N	0 / 86	-18.5	-18.5	0.08 (4)	10.00			
N- M	-20 / 0	0.0	0.0	0.12 (1)	7.81			
M- J	0 / 157	0.0	0.0	0.18 (1)	10.00			
M- L	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.39/1.00 (H-I:1) , BC=0.36/1.00 (T-U:1) , WB=0.79/1.00 (C-U:1) , SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.61 (O) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T34S	1	1	TRUSS DESC.	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307617

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T35	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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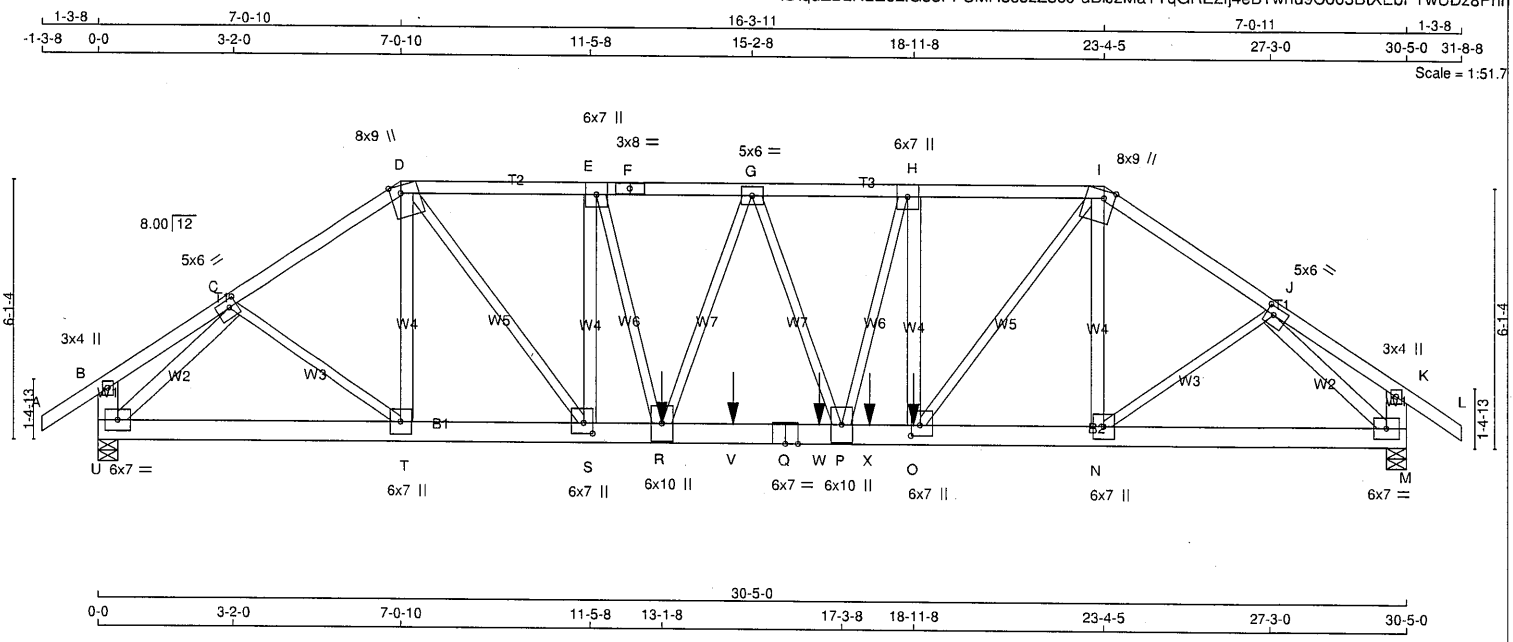

TOTAL WEIGHT = 2 X 170 = 339 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - D	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
S - E	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
I- L	1	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(0.0)
Q- M	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
H- O	1	6	SIDE(394.5)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2307618

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX.
M	5320	0	5320	0	0	5-8	2-14
U	5026	0	5026	0	0	5-8	2-12

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	3755	2503 / 0	0 / 0	0 / 0	0 / 0	1252 / 0	0 / 0
U	3546	2377 / 0	0 / 0	0 / 0	0 / 0	1168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.08 (1)	10.00
C- D	-6745 / 0	-91.8	-91.8	0.30 (1)	3.58
D- E	-8590 / 0	-91.8	-91.8	0.43 (1)	3.07
E- F	-9485 / 0	-91.8	-91.8	0.43 (1)	2.90
F- G	-9485 / 0	-91.8	-91.8	0.43 (1)	2.90
G- H	-9546 / 0	-91.8	-91.8	0.43 (1)	2.89
H- I	-9115 / 0	-91.8	-91.8	0.47 (1)	2.94
I- J	-7195 / 0	-91.8	-91.8	0.33 (1)	3.45
J- K	0 / 18	-91.8	-91.8	0.07 (1)	10.00
K- L	0 / 35	-91.8	-91.8	0.07 (1)	10.00
U- B	-234 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-235 / 0	0.0	0.0	0.01 (1)	7.81
U- T	0 / 4772	-18.5	-18.5	0.14 (1)	10.00
T- S	0 / 5578	-18.5	-18.5	0.18 (1)	10.00
S- R	0 / 8590	-18.5	-18.5	0.30 (1)	10.00
R- V	0 / 9571	-18.5	-18.5	0.41 (1)	10.00
V- Q	0 / 9571	-18.5	-18.5	0.41 (1)	10.00
Q- W	0 / 9571	-18.5	-18.5	0.41 (1)	10.00
W- P	0 / 9571	-18.5	-18.5	0.41 (1)	10.00
P- X	0 / 9115	-18.5	-18.5	0.29 (1)	10.00
X- O	0 / 9115	-18.5	-18.5	0.29 (1)	10.00
O- N	0 / 5953	-18.5	-18.5	0.19 (1)	10.00
N- M	0 / 5076	-18.5	-18.5	0.15 (1)	10.00

WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
T- D	-520 / 0	0.10 (1)
N- I	-468 / 0	0.09 (1)
S- E	-3506 / 0	0.70 (1)
D- S	0 / 4935	0.61 (1)
O- H	-1889 / 0	0.38 (1)
O- I	0 / 5180	0.64 (1)
E- R	0 / 3235	0.40 (1)
R- G	-248 / 0	0.08 (1)
G- P	-71 / 41	0.02 (1)
P- H	0 / 1559	0.19 (1)
C- T	0 / 1016	0.13 (1)
U- C	-6721 / 0	0.71 (1)
N- J	0 / 1104	0.14 (1)
J- M	-7148 / 0	0.76 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	18-11-8	-1610	-1610	---	BACK	VERT	TOTAL	---	C1
R	13-1-8	-2036	-2036	---	BACK	VERT	TOTAL	---	C1
V	14-9-4	-370	-370	---	BACK	VERT	TOTAL	---	C1
W	16-9-4	-370	-370	---	BACK	VERT	TOTAL	---	C1
X	17-11-4	-370	-370	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.47/1.00 (H-I:1) , BC=0.41/1.00 (P-R:1) , WB=0.76/1.00 (J-M:1) , SSI=0.28/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.93 (Q) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T35	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-uBiJzMaTTqGRE2fj4eBYwhu9Co63BtXEbPTwUDz8Phn

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW+m	MT20	8.0	9.0	Edge	3.00
E	TMWW+t	MT20	6.0	7.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMWW+t	MT20	6.0	7.0		
I	TTWW+m	MT20	8.0	9.0	Edge	3.00
J	TMWW-t	MT20	5.0	6.0	2.25	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	7.0		
N	BMWW+t	MT20	6.0	7.0		
O	BMWW+t	MT20	6.0	7.0	3.00	2.50
P	BMWW+t	MT20	6.0	10.0		
Q	BS-t	MT20	6.0	7.0		
R	BMWW+t	MT20	6.0	10.0		
S	BMWW+t	MT20	6.0	7.0	3.00	2.50
T	BMWW+t	MT20	6.0	7.0		
U	BMVW1-t	MT20	6.0	7.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307618

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T36	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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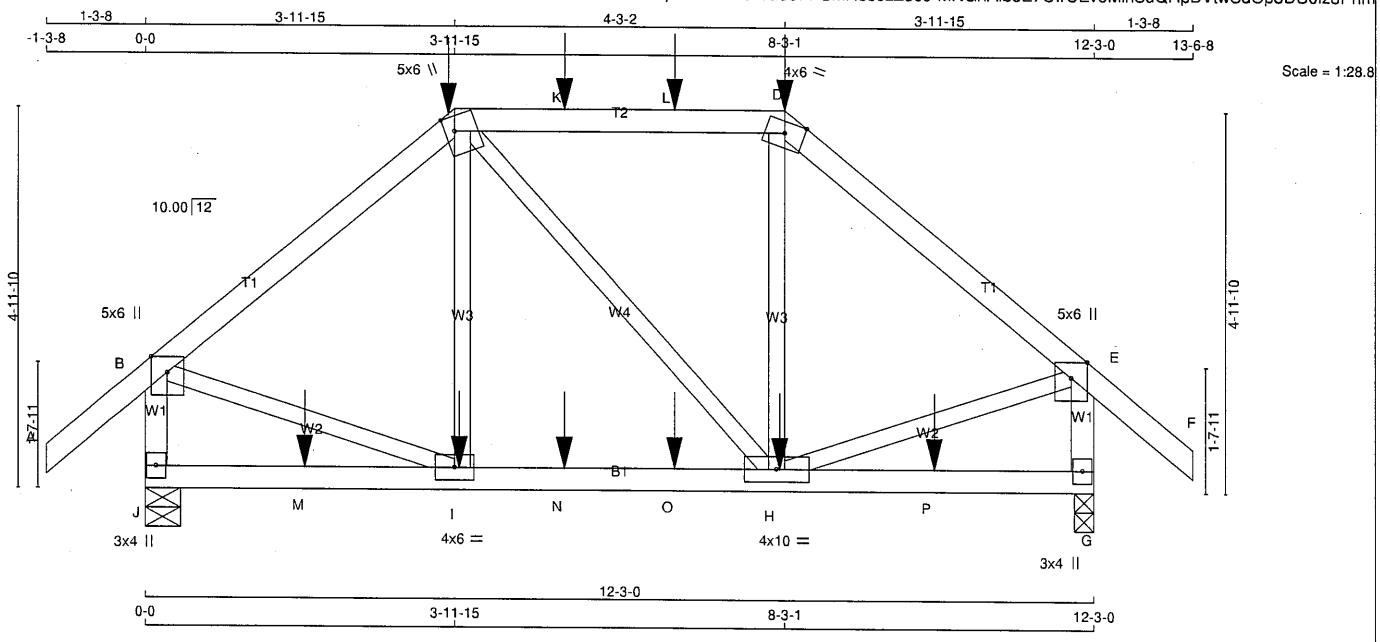

TOTAL WEIGHT = 56 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	10.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1486	0	1486	0	0	5-8	1-10
G	1486	0	1486	0	0	3-0	1-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1047	710 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0
G	1047	710 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.14 (1)	10.00
B- C	-1280 / 0	-91.8	-91.8	0.32 (1)	5.29
C- K	-977 / 0	-91.8	-91.8	0.67 (1)	5.03
K- L	-977 / 0	-91.8	-91.8	0.67 (1)	5.03
L- D	-977 / 0	-91.8	-91.8	0.67 (1)	5.03
D- E	-1279 / 0	-91.8	-91.8	0.32 (1)	5.29
E- F	0 / 41	-91.8	-91.8	0.14 (1)	10.00
J- B	-1445 / 0	0.0	0.0	0.17 (1)	6.76
G- E	-1444 / 0	0.0	0.0	0.17 (1)	6.76
J- M	0 / 0	-18.5	-18.5	0.14 (4)	10.00
M- I	0 / 0	-18.5	-18.5	0.14 (4)	10.00
I- N	0 / 977	-18.5	-18.5	0.25 (1)	10.00
N- O	0 / 977	-18.5	-18.5	0.25 (1)	10.00
O- H	0 / 977	-18.5	-18.5	0.25 (1)	10.00
H- P	0 / 0	-18.5	-18.5	0.14 (4)	10.00
P- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-147 / 84	0.06 (1)
C- H	0 / 0	0.00 (1)
H- D	-147 / 84	0.06 (1)
B- I	0 / 1029	0.25 (1)
H- E	0 / 1028	0.25 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-15	-36	-36	---	FRONT	VERT	DEAD	---	C1
C	3-11-15	-112	-112	---	FRONT	VERT	TOTAL	---	C1
C	3-11-15	-154	-154	---	FRONT	VERT	SNOW	---	C1
D	8-3-1	-36	-36	---	FRONT	VERT	DEAD	---	C1
D	8-3-1	-112	-112	---	FRONT	VERT	TOTAL	---	C1
D	8-3-1	-154	-154	---	FRONT	VERT	SNOW	---	C1
H	8-2-5	-25	-25	---	FRONT	VERT	TOTAL	---	C1
I	4-0-11	-25	-25	---	FRONT	VERT	TOTAL	---	C1
K	5-5-0	-106	-106	---	FRONT	VERT	TOTAL	---	C1
L	6-10-0	-106	-106	---	FRONT	VERT	TOTAL	---	C1
M	2-0-11	-25	-25	---	FRONT	VERT	TOTAL	---	C1
N	5-5-0	-25	-25	---	FRONT	VERT	TOTAL	---	C1
O	6-10-0	-25	-25	---	FRONT	VERT	TOTAL	---	C1
P	10-2-5	-25	-25	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.25/1.00 (H-I:1) , WB=0.25/1.00 (B-I:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL = 0.49 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307619

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T36Z	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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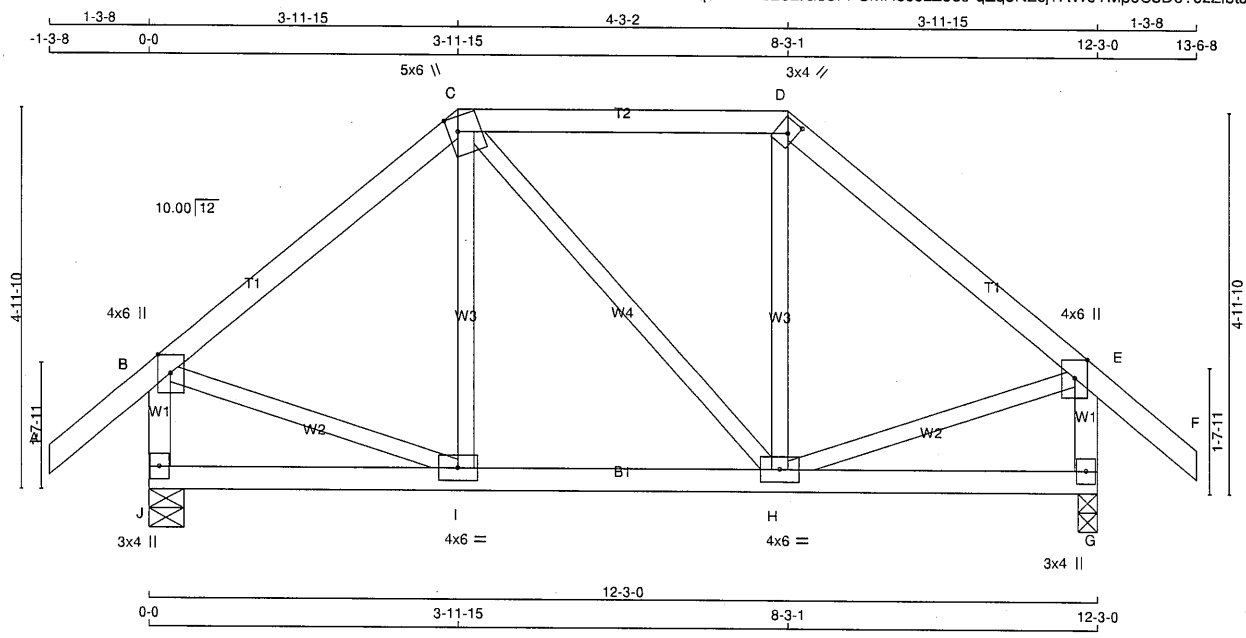

TOTAL WEIGHT = 2 X 56 = 112 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	802	0	802	0	0	5-8	1-8
G	802	0	802	0	0	3-0	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	565	384 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
G	565	384 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	I- C	-45 / 48	0.02 (4)
B- C	-520 / 0	-91.8	-91.8	0.19 (1)	6.25	C- H	0 / 0	0.00 (1)
C- D	-397 / 0	-91.8	-91.8	0.21 (1)	6.25	H- D	-45 / 48	0.02 (4)
D- E	-520 / 0	-91.8	-91.8	0.19 (1)	6.25	B- I	0 / 418	0.09 (1)
E- F	0 / 41	-91.8	-91.8	0.13 (1)	10.00	H- E	0 / 418	0.09 (1)
J- B	-771 / 0	0.0	0.0	0.08 (1)	7.81			
G- E	-771 / 0	0.0	0.0	0.08 (1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
I- H	0 / 397	-18.5	-18.5	0.10 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (C-D:1) , BC=0.10/1.00 (H-I:4) , WB=0.09/1.00 (B-I:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (H) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307620

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T37	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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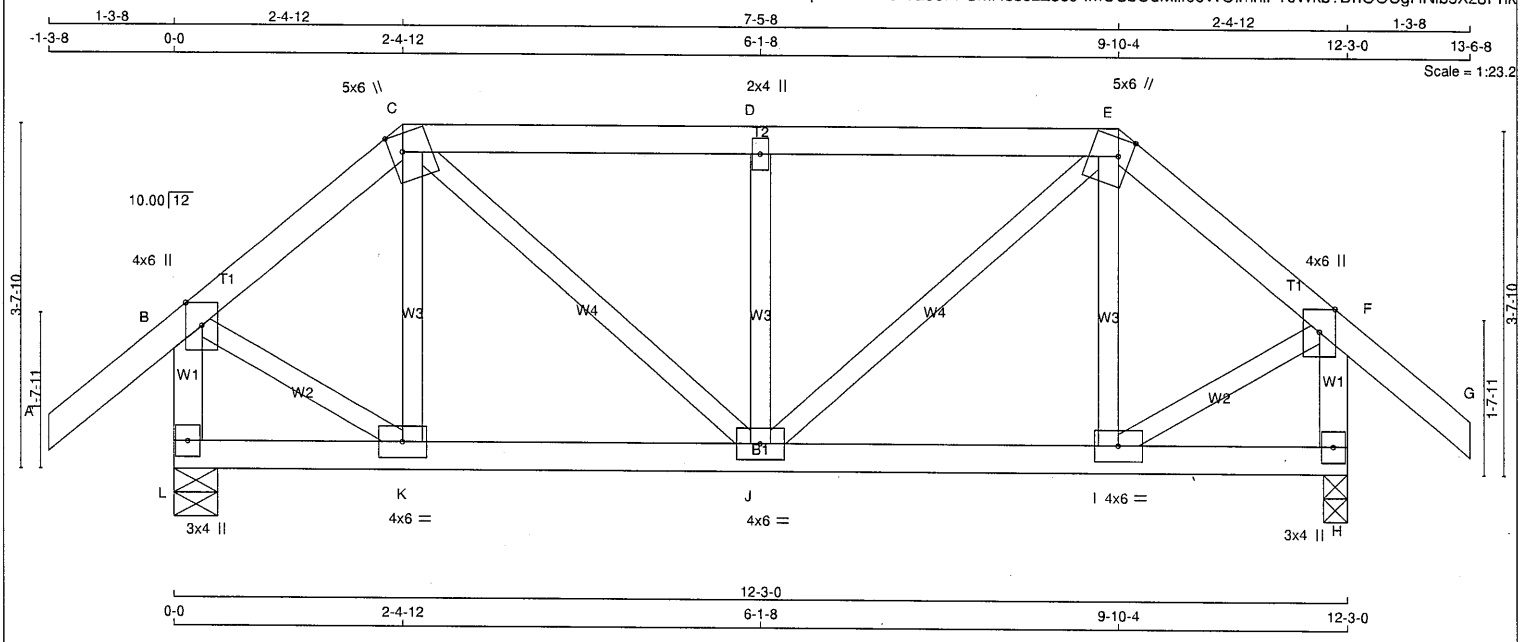

TOTAL WEIGHT = 56 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307621

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	802	0	802	0	0	5-8	1-8
H	802	0	802	0	0	3-0	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	565	384 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	565	384 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)				FR-TO		
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K- C	-144 / 0	0.03 (1)
B- C	-527 / 0	-91.8	-91.8	0.09 (1)	6.25	I- E	-144 / 0	0.03 (1)
C- D	-665 / 0	-91.8	-91.8	0.21 (1)	6.25	B- K	0 / 454	0.10 (1)
D- E	-665 / 0	-91.8	-91.8	0.21 (1)	6.25	I- F	0 / 454	0.10 (1)
E- F	-527 / 0	-91.8	-91.8	0.09 (1)	6.25	C- J	0 / 357	0.08 (1)
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	J- D	-413 / 0	0.09 (1)
L- B	-787 / 0	0.0	0.0	0.08 (1)	7.81	J- E	0 / 357	0.08 (1)
H- F	-787 / 0	0.0	0.0	0.08 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
K- J	0 / 398	-18.5	-18.5	0.09 (1)	10.00			
J- I	0 / 398	-18.5	-18.5	0.09 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.21/1.00 (C-D:1) , BC=0.09/1.00 (I-J:1) , WB=0.10/1.00 (F-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (F) (INPUT = 0.90)
JSI METAL= 0.28 (F) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T38	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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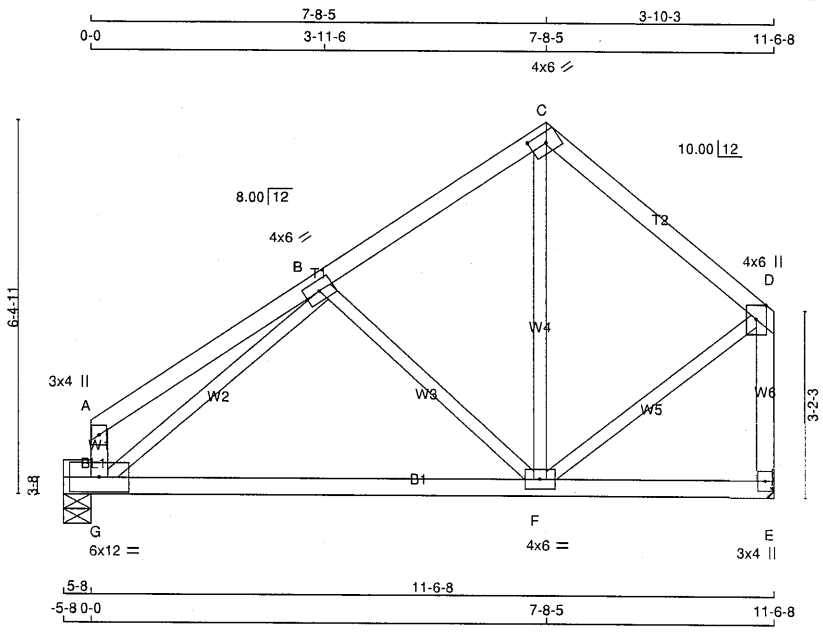

TOTAL WEIGHT = 51 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

BEARING BLOCKS

BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-I	MT20	4.0	6.0	2.00	3.25
D	TMVW+p	MT20	4.0	6.0	Edge	
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMVWK1-t	MT20	6.0	12.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	628	0	628	0	0	MECHANICAL	
G	642	0	642	0	0	5-8	5-0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	444	292 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
G	454	299 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 46	-91.8	-91.8	0.23	(1)	10.00	B- F	-255 / 0	0.11 (1)
B- C	-363 / 0	-91.8	-91.8	0.17	(1)	6.25	F- C	0 / 112	0.04 (4)
C- D	-367 / 0	-91.8	-91.8	0.18	(1)	6.25	G- B	-674 / 0	0.29 (1)
G- A	-123 / 0	0.0	0.0	0.01	(1)	7.81	F- D	0 / 347	0.08 (1)
E- D	-614 / 0	0.0	0.0	0.11	(1)	7.81			
G- F	0 / 470	-18.5	-18.5	0.25	(4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.22	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.25/1.00 (F-G:4) , WB=0.29/1.00 (B-G:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (F) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 0.95)

Structural component only
DWG# T-2307622

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T39	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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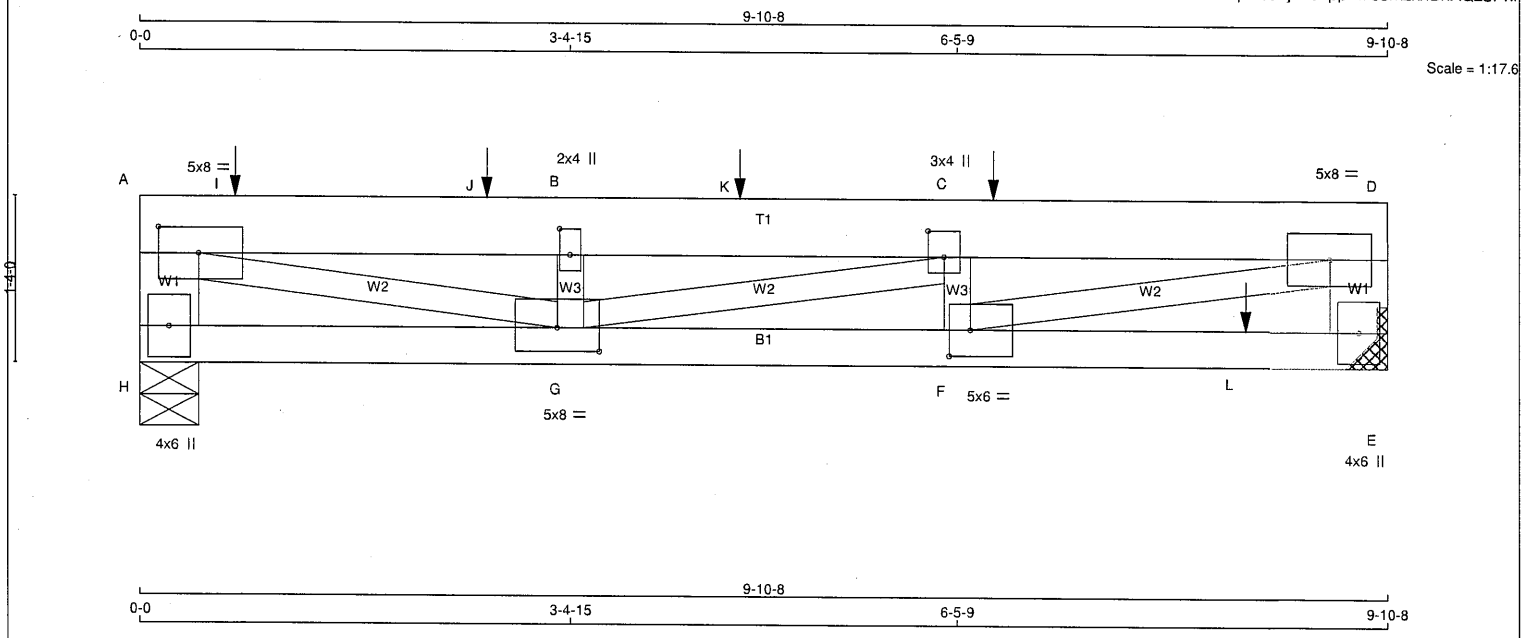

TOTAL WEIGHT = 2 X 39 = 78 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	2	12	TOP
A- D	2	12	TOP
D- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	1	12	SIDE(10.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMWW+t	MT20	3.0	4.0	2.50	1.50
D	TMVW-t	MT20	5.0	8.0		
E	BMV1+p	MT20	4.0	6.0		

Structural component only
DWG# T-2307623

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	3133	0	3133	0	0	5-8	1-11
E	2289	0	2289	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2220	1429 / 0	0 / 0	0 / 0	0 / 0	792 / 0	0 / 0
E	1625	1033 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
H- A	-3020 / 0	0.0	0.0	0.11 (1)	7.81	
A- I	-5887 / 0	-91.8	-91.8	0.42 (1)	4.39	
I- J	-5887 / 0	-91.8	-91.8	0.42 (1)	4.39	
J- B	-5887 / 0	-91.8	-91.8	0.42 (1)	4.39	
B- K	-5887 / 0	-91.8	-91.8	0.34 (1)	4.50	
K- C	-5887 / 0	-91.8	-91.8	0.34 (1)	4.50	
C- D	-5488 / 0	-91.8	-91.8	0.21 (1)	4.81	
E- D	-1834 / 0	0.0	0.0	0.06 (1)	7.81	
H- G	0 / 0	-37.3	-37.3	0.13 (1)	10.00	
G- F	0 / 5488	-37.3	-37.3	0.61 (1)	10.00	
F- L	0 / 0	-37.3	-37.3	0.37 (1)	10.00	
L- E	0 / 0	-37.3	-37.3	0.37 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- D	0 / 5701	0.71 (1)
A- G	0 / 6115	0.76 (1)
F- C	-1131 / 0	0.09 (1)
G- B	-1781 / 0	0.14 (1)
G- C	0 / 418	0.05 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	6-9-0	-502	-502	---	TOP	VERT	TOTAL	---	C1
I	9-0	-992	-992	---	TOP	VERT	TOTAL	---	C1
J	2-9-0	-502	-502	---	TOP	VERT	TOTAL	---	C1
K	4-9-0	-502	-502	---	TOP	VERT	TOTAL	---	C1
L	8-9-0	-429	-429	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 756 (0.16")

CSI: TC=0.42/1.00 (A-B:1) , BC=0.61/1.00 (F-G:1) , WB=0.76/1.00 (A-G:1) , SSI=0.45/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.62 (F) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T39	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-E8VC04ecHMvkKqYhtCnjdkb1ppm7s8mzkhBhAQz8Phi

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMWW-t	MT20	5.0	6.0	2.50	2.00
G	BMWWW-t	MT20	5.0	8.0	2.25	4.00
H	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307623

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T40	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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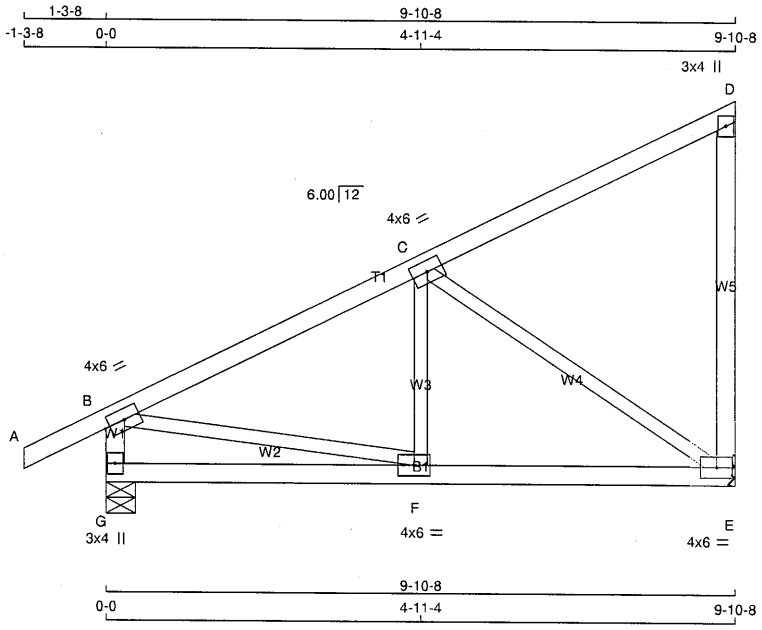

TOTAL WEIGHT = 3 X 43 = 130 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	544	0	544	0	0	MECHANICAL	
G	669	0	669	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	385	253 / 0	0 / 0	0 / 0	0 / 0	132 / 0	0 / 0
G	471	322 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	F- C	0 / 97	0.03 (4)
B- C	-534 / 0	-91.8	-91.8	0.29 (1)	6.25	C- E	-602 / 0	0.35 (1)
C- D	-25 / 0	-91.8	-91.8	0.28 (1)	6.25	B- F	0 / 507	0.11 (1)
E- D	-172 / 0	0.0	0.0	0.12 (1)	7.81			
G- B	-633 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
F- E	0 / 499	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.17/1.00 (E-F:4) , WB=0.35/1.00 (C-E:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307624

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T41	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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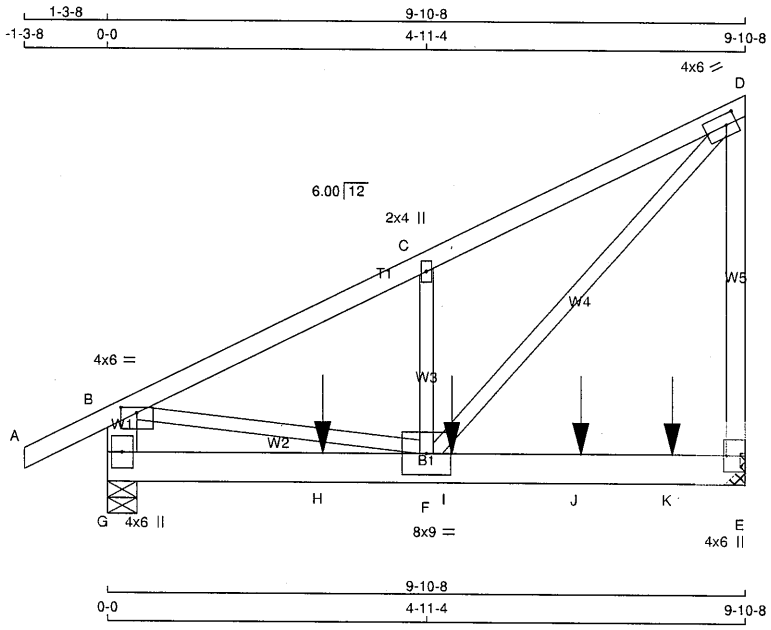

Scale = 1:34.5

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
G- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0	2.00	2.00

Structural component only
DWG# T-2307625

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	2911	0	2911	0	0	MECHANICAL	
G	2488	0	2488	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-9.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	2050	1393 / 0	0 / 0	0 / 0	0 / 0	657 / 0	0 / 0
G	1755	1179 / 0	0 / 0	0 / 0	0 / 0	575 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.07 (1)		10.00	F- C	-525 / 0	0.06 (1)
B- C	-3071 / 0	-91.8	-91.8	0.21 (1)		5.08	F- D	0 / 4227	0.52 (1)
C- D	-3083 / 0	-91.8	-91.8	0.21 (1)		5.07	B- F	0 / 2796	0.35 (1)
E- D	-2035 / 0	0.0	0.0	0.68 (1)		7.74			
G- B	-2115 / 0	0.0	0.0	0.07 (1)		7.81			
G- H	0 / 0	-18.5	-18.5	0.45 (1)		10.00			
H- F	0 / 0	-18.5	-18.5	0.45 (1)		10.00			
F- I	0 / 0	-18.5	-18.5	0.45 (1)		10.00			
I- J	0 / 0	-18.5	-18.5	0.45 (1)		10.00			
J- K	0 / 0	-18.5	-18.5	0.45 (1)		10.00			
K- E	0 / 0	-18.5	-18.5	0.45 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	3-4-0	-1211	-1211	---	FRONT	VERT	TOTAL	---	C1
I	5-4-0	-579	-579	---	FRONT	VERT	TOTAL	---	C1
J	7-4-0	-579	-579	---	FRONT	VERT	TOTAL	---	C1
K	8-9-0	-579	-579	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 51 = 102 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.45/1.00 (F-G:1) , WB=0.52/1.00 (D-F:1) , SSI=0.56/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T41	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-BXdyRlqspz9SZ7i3?cpBi9qJFcU2K4vGC?goEJz8Phq

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMV1+p	MT20	4.0	6.0	3.00	Edge
F	BMWWW-t	MT20	8.0	9.0		
G	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307625

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T42	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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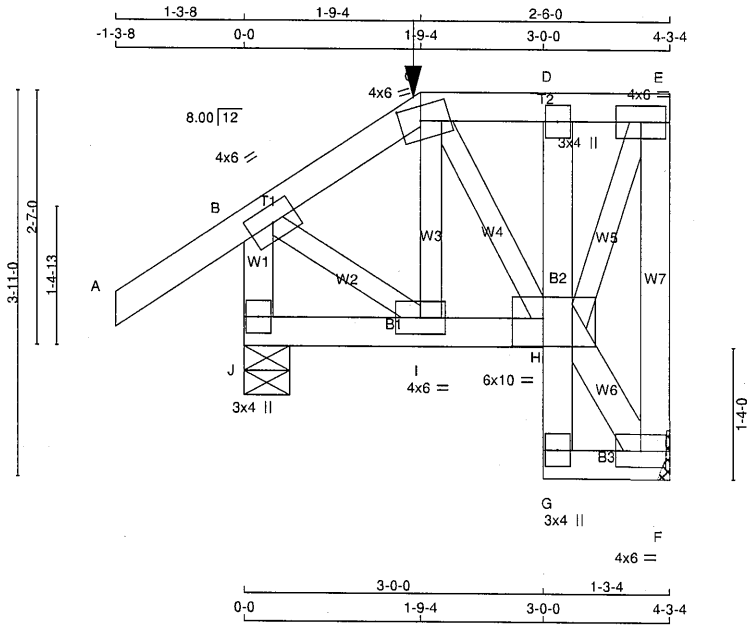

TOTAL WEIGHT = 2 X 29 = 59 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	3.50	3.75
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	228	0	228	0	0	MECHANICAL	
J	399	0	399	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	161	105 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
J	279	201 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	I- C	-27 / 18	0.01 (4)
B- C	-150 / 0	-91.8	-91.8	0.13 (1)	6.25	C- H	-29 / 0	0.01 (1)
C- D	-85 / 0	-91.8	-91.8	0.03 (1)	6.25	B- I	0 / 117	0.03 (1)
D- E	-82 / 0	-91.8	-91.8	0.02 (1)	6.25	H- F	-9 / 0	0.00 (1)
F- E	-210 / 0	0.0	0.0	0.05 (1)	7.81	H- E	0 / 186	0.05 (1)
J- B	-383 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
I- H	0 / 101	-18.5	-18.5	0.02 (1)	10.00			
G- H	0 / 12	0.0	0.0	0.01 (1)	10.00			
H- D	-128 / 0	0.0	0.0	0.01 (1)	7.81			
G- F	0 / 6	-18.5	-18.5	0.01 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-4	-4	-4	---	FRONT	VERT	DEAD	---	C1
C	1-9-4	-17	-17	---	FRONT	VERT	SNOW	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (H-I:1) , WB=0.05/1.00 (E-H:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307626

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432499	T42	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-fjBLe5gUaHHJBHGFYKKQFNDcT0x03fdQQfQMmlz8Phf

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307626

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T43	5	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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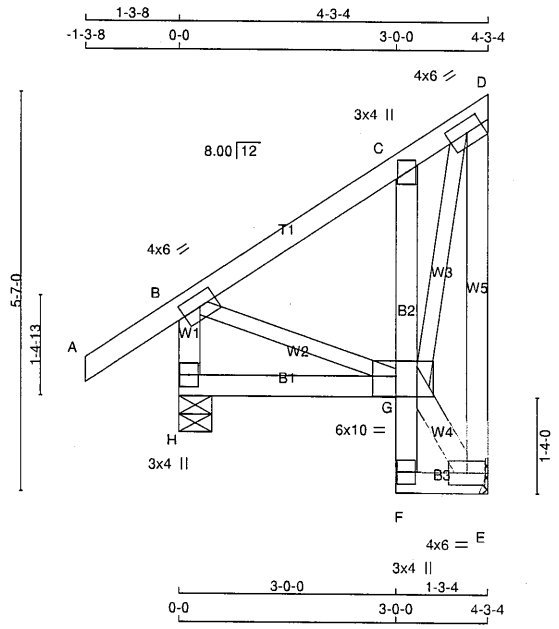

Scale = 1:30.8

TOTAL WEIGHT = 5 X 31 = 157 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	6.0	2.00	2.75
E	BMVW1-t	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-I	MT20	6.0	10.0	3.50	3.75
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	235	0	235	0	0	MECHANICAL	
H	361	0	361	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	166	109 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
H	253	179 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	B- G	0 / 93	0.02 (1)
B- C	-93 / 0	-91.8	-91.8	0.10 (1)	6.25	G- E	-17 / 0	0.00 (1)
C- D	-123 / 0	-91.8	-91.8	0.09 (1)	6.25	G- D	0 / 276	0.06 (1)
E- D	-212 / 0	0.0	0.0	0.12 (1)	7.81			
H- B	-332 / 0	0.0	0.0	0.03 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
F- G	0 / 12	0.0	0.0	0.02 (1)	10.00			
G- C	-269 / 0	0.0	0.0	0.02 (1)	7.81			
F- E	0 / 11	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.05/1.00 (G-H:4) , WB=0.06/1.00 (D-G:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 0.95)

Structural component only
DWG# T-2307627

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T44	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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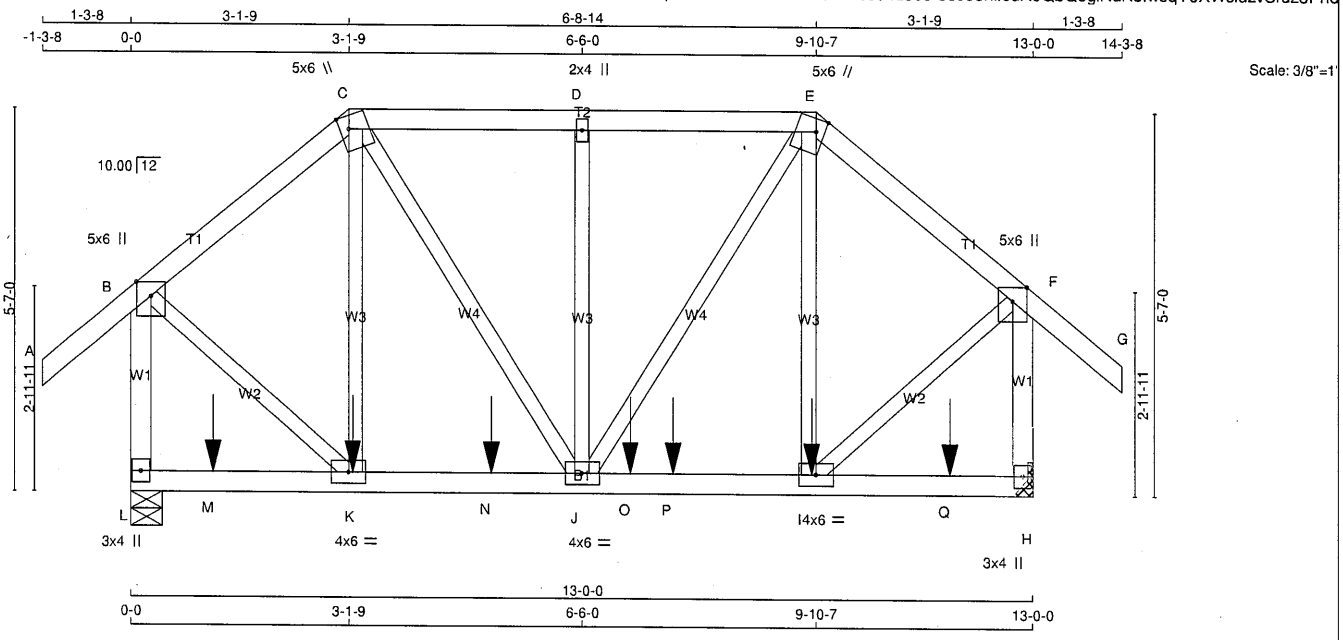

TOTAL WEIGHT = 70 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307628

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1706	0	1706	0	0	5-8	2-1
H	1729	0	1729	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 2-2.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1209	776 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
H	1226	787 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.14 (1)	10.00	K- C	-103 / 27	0.05 (1)
B- C	-1009 / 0	-91.8	-91.8	0.19 (1)	5.98	C- J	0 / 569	0.14 (1)
C- D	-1077 / 0	-91.8	-91.8	0.18 (1)	5.83	J- D	-361 / 0	0.17 (1)
D- E	-1077 / 0	-91.8	-91.8	0.18 (1)	5.83	J- E	0 / 530	0.13 (1)
E- F	-1035 / 0	-91.8	-91.8	0.19 (1)	5.91	I- E	-52 / 41	0.03 (1)
F- G	0 / 41	-91.8	-91.8	0.14 (1)	10.00	B- K	0 / 997	0.25 (1)
L- B	-1550 / 0	0.0	0.0	0.26 (1)	6.58	I- F	0 / 1023	0.25 (1)
H- F	-1583 / 0	0.0	0.0	0.27 (1)	6.52			
L- M	0 / 0	-37.3	-37.3	0.24 (1)	10.00			
M- K	0 / 0	-37.3	-37.3	0.24 (1)	10.00			
K- N	0 / 770	-37.3	-37.3	0.37 (1)	10.00			
N- J	0 / 770	-37.3	-37.3	0.37 (1)	10.00			
J- O	0 / 792	-37.3	-37.3	0.37 (1)	10.00			
O- P	0 / 792	-37.3	-37.3	0.37 (1)	10.00			
P- I	0 / 792	-37.3	-37.3	0.37 (1)	10.00			
I- Q	0 / 0	-37.3	-37.3	0.23 (1)	10.00			
Q- H	0 / 0	-37.3	-37.3	0.23 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	9-9-11	-152	-152	---	BACK	VERT	TOTAL	---	C1
K	3-2-5	-152	-152	---	BACK	VERT	TOTAL	---	C1
M	1-2-5	-146	-146	---	BACK	VERT	TOTAL	---	C1
N	5-2-5	-152	-152	---	BACK	VERT	TOTAL	---	C1
O	7-2-5	-152	-152	---	BACK	VERT	TOTAL	---	C1
P	7-9-11	-152	-152	---	BACK	VERT	TOTAL	---	C1
Q	11-9-11	-146	-146	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.27/1.00 (F-H:1) , BC=0.37/1.00 (I-J:1) , WB=0.25/1.00 (F-I:1) , SSI=0.31/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.68 (J) (INPUT = 0.90)
JSI METAL=0.40 (F) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T45	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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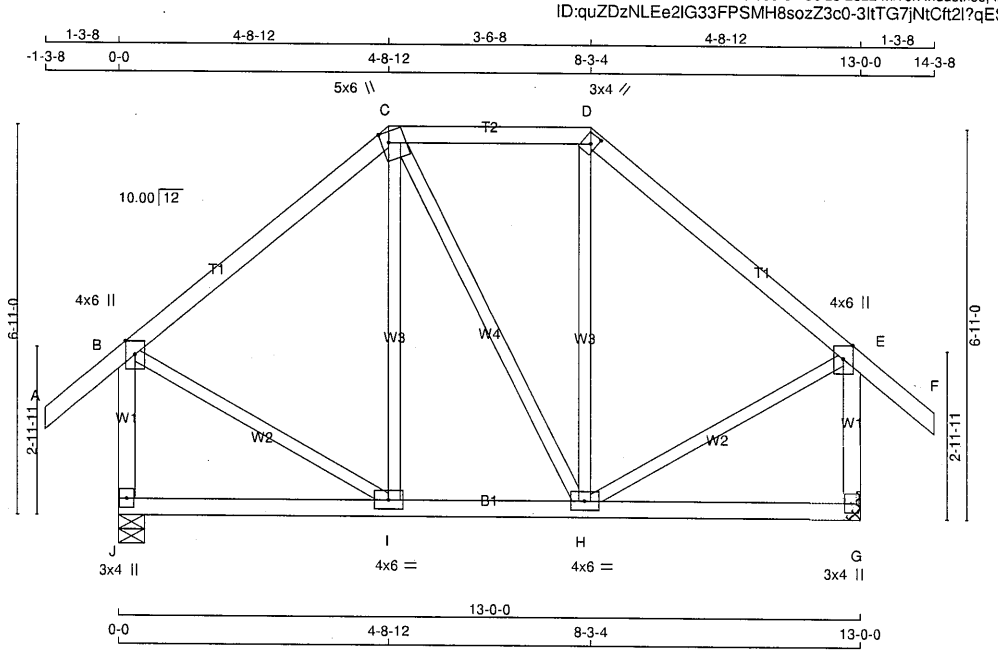

TOTAL WEIGHT = 2 X 67 = 135 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	844	0	844	0	0	5-8	1-8
G	844	0	844	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	594	403 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
G	594	403 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	-435 / 0	-91.8	-91.8	0.27 (1)	6.25
C- D	-332 / 0	-91.8	-91.8	0.15 (1)	6.25
D- E	-435 / 0	-91.8	-91.8	0.27 (1)	6.25
E- F	0 / 41	-91.8	-91.8	0.13 (1)	10.00
J- B	-806 / 0	0.0	0.0	0.13 (1)	7.81
G- E	-805 / 0	0.0	0.0	0.13 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00
I- H	0 / 332	-18.5	-18.5	0.11 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-99 / 28	0.08 (1)
C- H	0 / 0	0.00 (1)
H- D	-100 / 29	0.08 (1)
B- I	0 / 380	0.09 (1)
H- E	0 / 380	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.11/1.00 (H-I:4) , WB=0.09/1.00 (B-I:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307629

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	T46	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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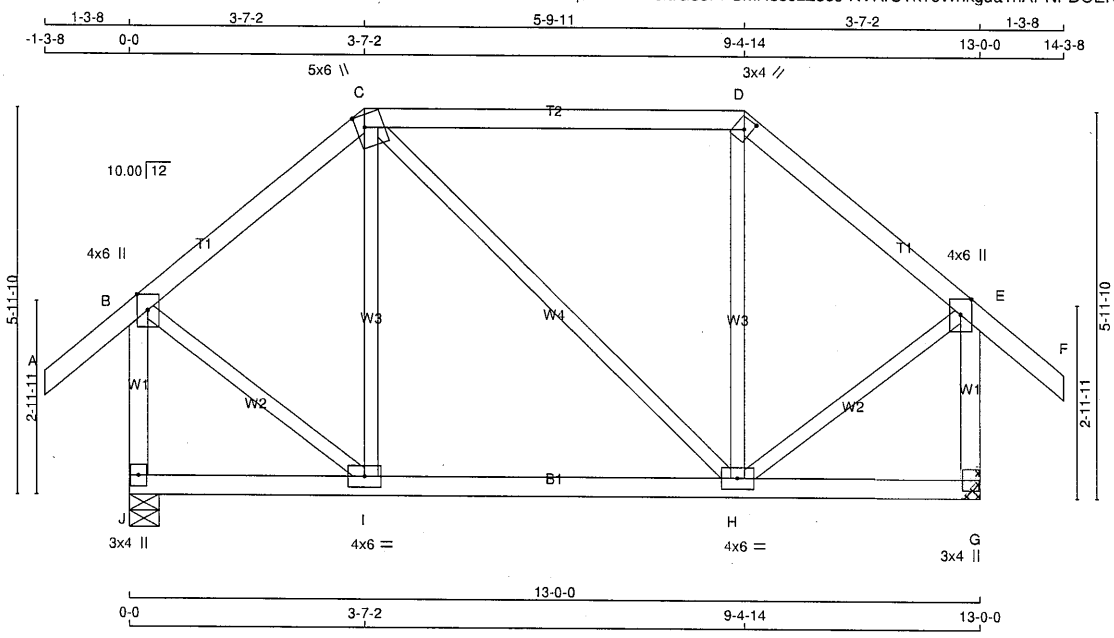

TOTAL WEIGHT = 64 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	844	0	844	0	0	5-8	1-8
G	844	0	844	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	594	403 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
G	594	403 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13	(1)	10.00	I- C	-145 / 23	0.08 (1)
B- C	-444 / 0	-91.8	-91.8	0.15	(1)	6.25	C- H	0 / 0	0.00 (1)
C- D	-337 / 0	-91.8	-91.8	0.40	(1)	6.25	H- D	-146 / 23	0.08 (1)
D- E	-444 / 0	-91.8	-91.8	0.15	(1)	6.25	B- I	0 / 418	0.09 (1)
E- F	0 / 41	-91.8	-91.8	0.13	(1)	10.00	H- E	0 / 418	0.09 (1)
J- B	-821 / 0	0.0	0.0	0.13	(1)	7.81			
G- E	-820 / 0	0.0	0.0	0.13	(1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.10	(4)	10.00			
I- H	0 / 338	-18.5	-18.5	0.13	(4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.10	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.13/1.00 (H-I:4) , WB=0.09/1.00 (B-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (D) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307630

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T61	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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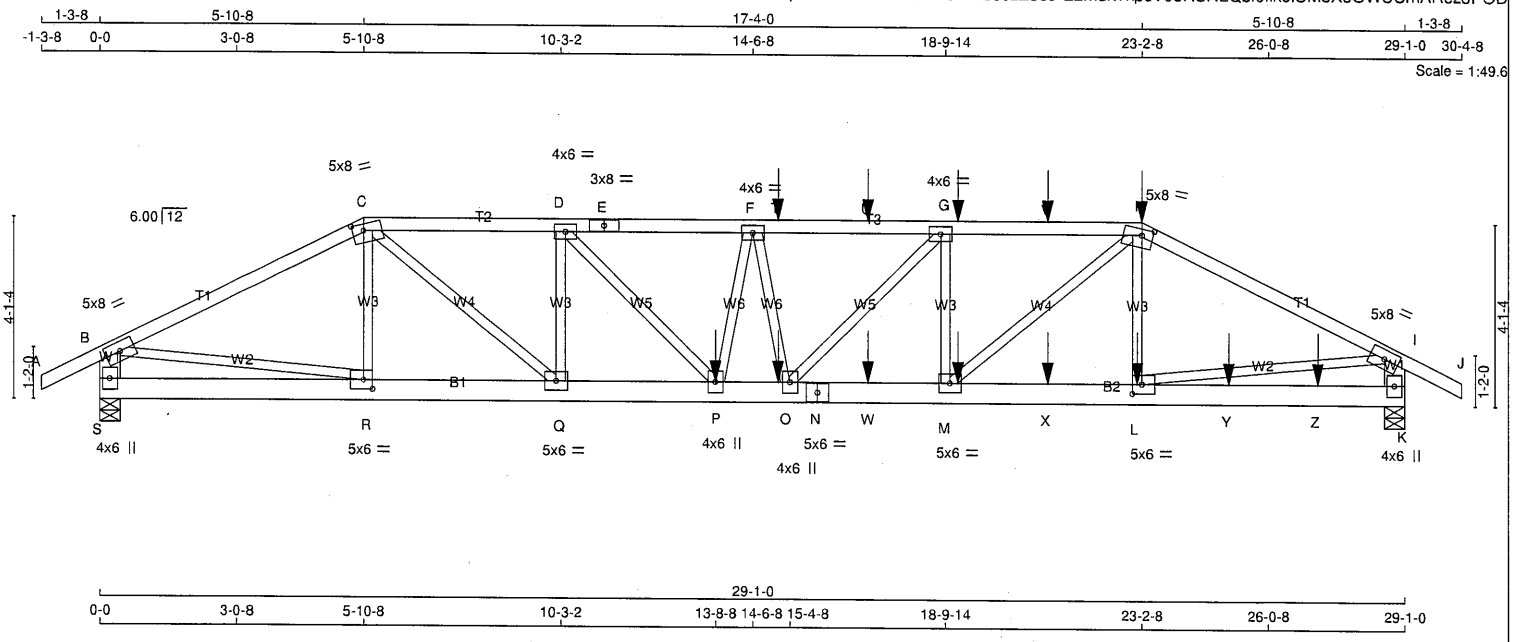

TOTAL WEIGHT = 2 X 134 = 269 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(29.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2307636

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3155	0	3155	0	0	5-8	1-11
K	3689	0	3689	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2224	1501 / 0	0 / 0	0 / 0	0 / 0	723 / 0	0 / 0
K	2603	1740 / 0	0 / 0	0 / 0	0 / 0	863 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B- C	-4790 / 0	-91.8	-91.8	0.50 (1)	3.98	
C- D	-6810 / 0	-91.8	-91.8	0.32 (1)	3.54	
D- E	-8307 / 0	-91.8	-91.8	0.39 (1)	3.16	
E- F	-8307 / 0	-91.8	-91.8	0.39 (1)	3.16	
F- T	-8152 / 0	-91.8	-91.8	0.52 (1)	3.04	
T- U	-8152 / 0	-91.8	-91.8	0.52 (1)	3.04	
U- G	-8152 / 0	-91.8	-91.8	0.52 (1)	3.04	
G- V	-7311 / 0	-91.8	-91.8	0.47 (1)	3.26	
V- H	-7311 / 0	-91.8	-91.8	0.47 (1)	3.26	
H- I	-5677 / 0	-91.8	-91.8	0.57 (1)	3.65	
I- J	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
S- B	-3096 / 0	0.0	0.0	0.11 (1)	7.81	
K- I	-3596 / 0	0.0	0.0	0.13 (1)	7.40	
S- R	0 / 0	-18.5	-18.5	0.04 (4)	10.00	
R- Q	0 / 4268	-18.5	-18.5	0.30 (1)	10.00	
Q- P	0 / 6810	-18.5	-18.5	0.56 (1)	10.00	
P- O	0 / 8290	-18.5	-18.5	0.66 (1)	10.00	
O- N	0 / 7311	-18.5	-18.5	0.53 (1)	10.00	
N- W	0 / 7311	-18.5	-18.5	0.53 (1)	10.00	
W- M	0 / 7311	-18.5	-18.5	0.53 (1)	10.00	
M- X	0 / 5062	-18.5	-18.5	0.37 (1)	10.00	
X- L	0 / 5062	-18.5	-18.5	0.37 (1)	10.00	
L- Y	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
Y- Z	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
Z- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-464 / 0	0.06 (1)
L- H	-455 / 27	0.06 (1)
B- R	0 / 4317	0.53 (1)
L- I	0 / 5117	0.63 (1)
C- Q	0 / 3333	0.41 (1)
Q- D	-2000 / 0	0.25 (1)
D- P	0 / 2183	0.27 (1)
P- F	0 / 84	0.01 (1)
F- O	-682 / 0	0.09 (1)
O- G	0 / 1228	0.15 (1)
M- G	-1774 / 0	0.22 (1)
M- H	0 / 2948	0.36 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	19-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
H	23-2-8	-402	-402	---	FRONT	VERT	TOTAL	---	C1
L	23-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
M	19-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
O	15-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
P	13-8-8	-1296	-1296	---	FRONT	VERT	TOTAL	---	C1
T	15-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
U	17-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
V	21-1-4	-122	-122	---	FRONT	VERT	TOTAL	---	C1
W	17-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
X	21-1-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Y	25-1-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 997 (0.35")

CSI: TC=0.57/1.00 (H-I:1) , BC=0.66/1.00 (O-P:1) , WB=0.63/1.00 (I-L:1) , SSI=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T61	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2IG33FPSMH8sozZ3c0-E2MdkTkpcV0oRSNLQbf9fik6IUM8X8OWUCmARez8POD

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	1.75	3.00
D, F, G						
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
H	TTWW-m	MT20	5.0	8.0	1.75	3.00
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	27-1-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307636

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T62	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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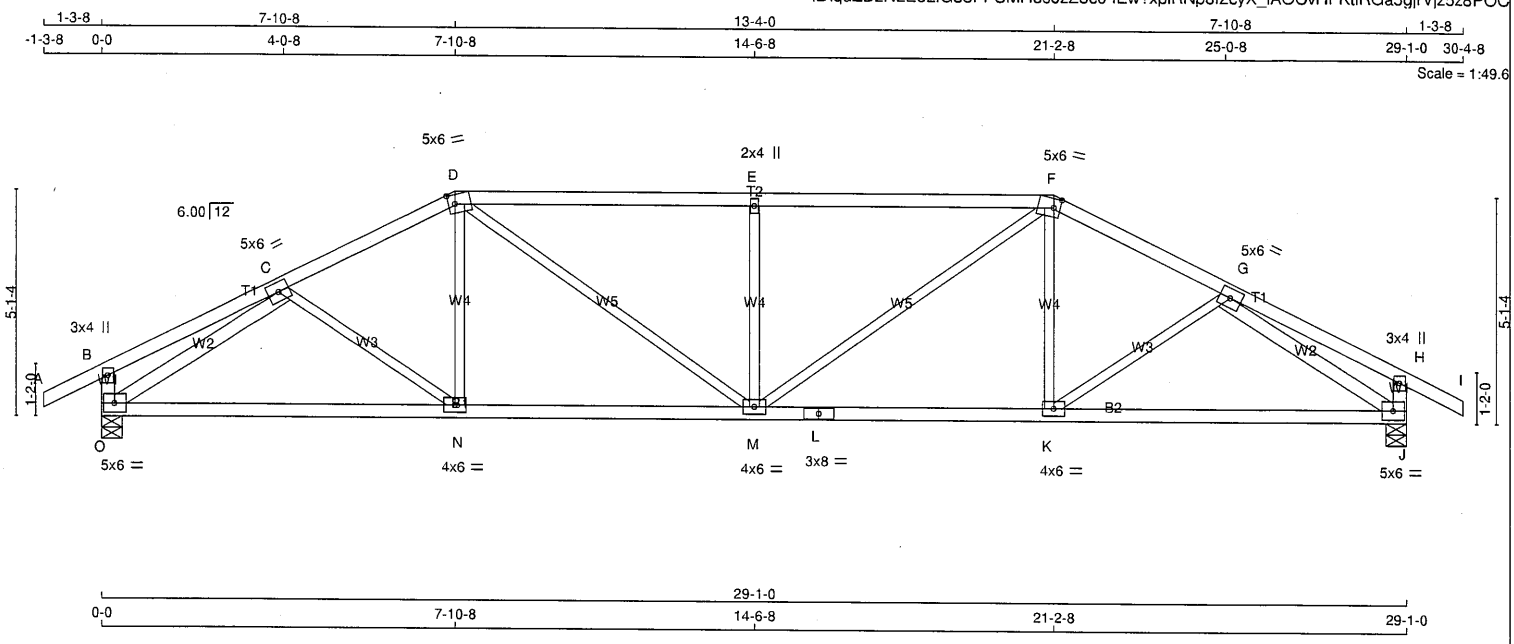

TOTAL WEIGHT = 115 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.50	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	1.75
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1728	0	1728	0	0	5-8	1-14
J	1728	0	1728	0	0	5-8	1-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
J	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- N	-19 / 58	0.02 (4)
B- C	0 / 17	-91.8	-91.8	0.21 (1)	10.00	N- D	0 / 171	0.05 (4)
C- D	-2184 / 0	-91.8	-91.8	0.22 (1)	4.44	D- M	0 / 726	0.16 (1)
D- E	-2530 / 0	-91.8	-91.8	0.64 (1)	3.64	M- E	-752 / 0	0.29 (1)
E- F	-2530 / 0	-91.8	-91.8	0.64 (1)	3.64	M- F	0 / 726	0.16 (1)
F- G	-2184 / 0	-91.8	-91.8	0.22 (1)	4.44	K- F	0 / 171	0.05 (4)
G- H	0 / 17	-91.8	-91.8	0.21 (1)	10.00	K- G	-19 / 58	0.02 (4)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	O- C	-2402 / 0	0.67 (1)
O- B	-269 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-2402 / 0	0.67 (1)
J- H	-269 / 0	0.0	0.0	0.03 (1)	7.81			
O- N	0 / 1952	-18.5	-18.5	0.46 (1)	10.00			
N- M	0 / 1941	-18.5	-18.5	0.46 (1)	10.00			
M- L	0 / 1941	-18.5	-18.5	0.46 (1)	10.00			
L- K	0 / 1941	-18.5	-18.5	0.46 (1)	10.00			
K- J	0 / 1952	-18.5	-18.5	0.46 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.46/1.00 (J-K:1) , WB=0.67/1.00 (G-J:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.60 (L) (INPUT = 0.95)

Structural component only
DWG# T-2307637

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T63	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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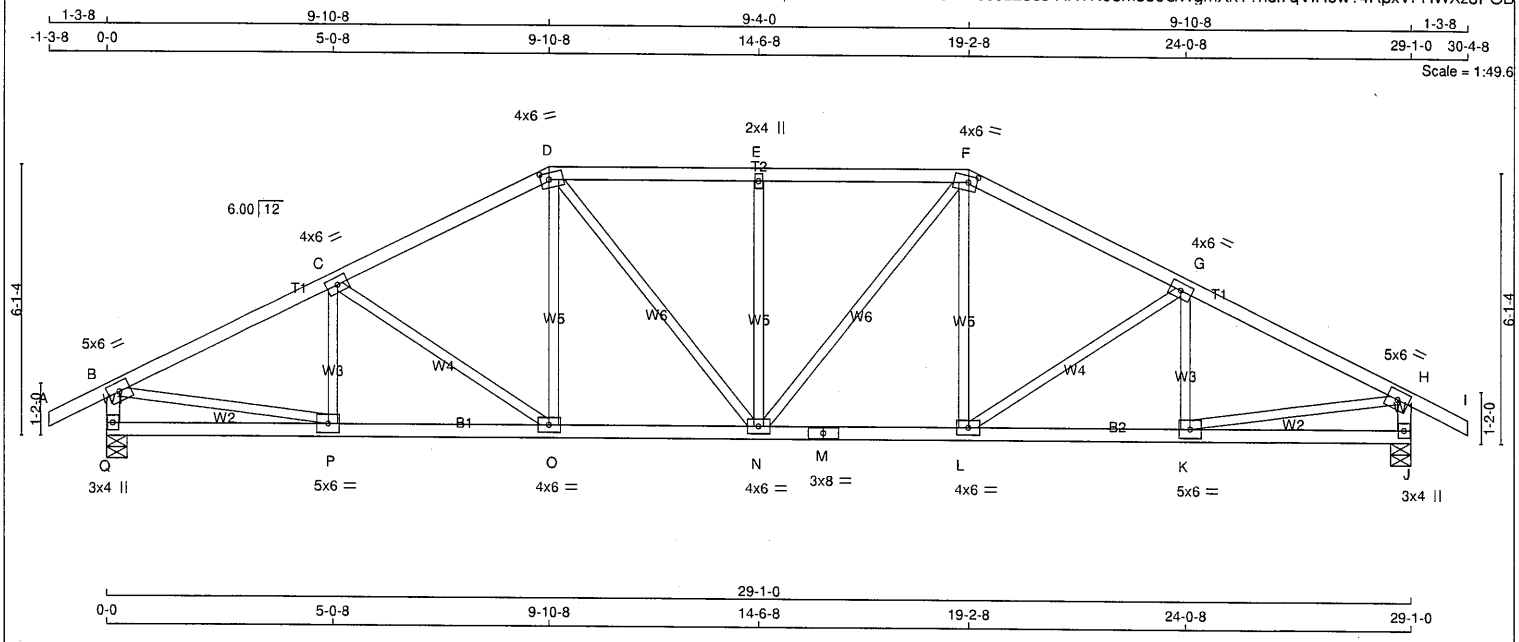

TOTAL WEIGHT = 121 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1728	0	1728	0	0	5-8	2-2
J	1728	0	1728	0	0	5-8	2-2

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
J	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P- C	-257 / 0	0.06 (1)
B- C	-2280 / 0	-91.8	-91.8	0.35 (1)	4.22	C- O	-310 / 0	0.19 (1)
C- D	-2042 / 0	-91.8	-91.8	0.33 (1)	4.43	O- D	0 / 269	0.06 (1)
D- E	-2049 / 0	-91.8	-91.8	0.30 (1)	4.46	D- N	0 / 384	0.09 (1)
E- F	-2049 / 0	-91.8	-91.8	0.30 (1)	4.46	N- E	-521 / 0	0.30 (1)
F- G	-2042 / 0	-91.8	-91.8	0.33 (1)	4.43	N- F	0 / 384	0.09 (1)
G- H	-2280 / 0	-91.8	-91.8	0.35 (1)	4.22	L- F	0 / 269	0.06 (1)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L- G	-310 / 0	0.19 (1)
Q- B	-1685 / 0	0.0	0.0	0.17 (1)	6.39	K- G	-257 / 0	0.06 (1)
J- H	-1685 / 0	0.0	0.0	0.17 (1)	6.39	B- P	0 / 2089	0.47 (1)
						K- H	0 / 2089	0.47 (1)
Q- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
P- O	0 / 2060	-18.5	-18.5	0.38 (1)	10.00			
O- N	0 / 1809	-18.5	-18.5	0.34 (1)	10.00			
N- M	0 / 1809	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 1809	-18.5	-18.5	0.34 (1)	10.00			
L- K	0 / 2060	-18.5	-18.5	0.38 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.35/1.00 (G-H:1) , BC=0.38/1.00 (K-L:1) , WB=0.47/1.00 (H-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307638

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T64	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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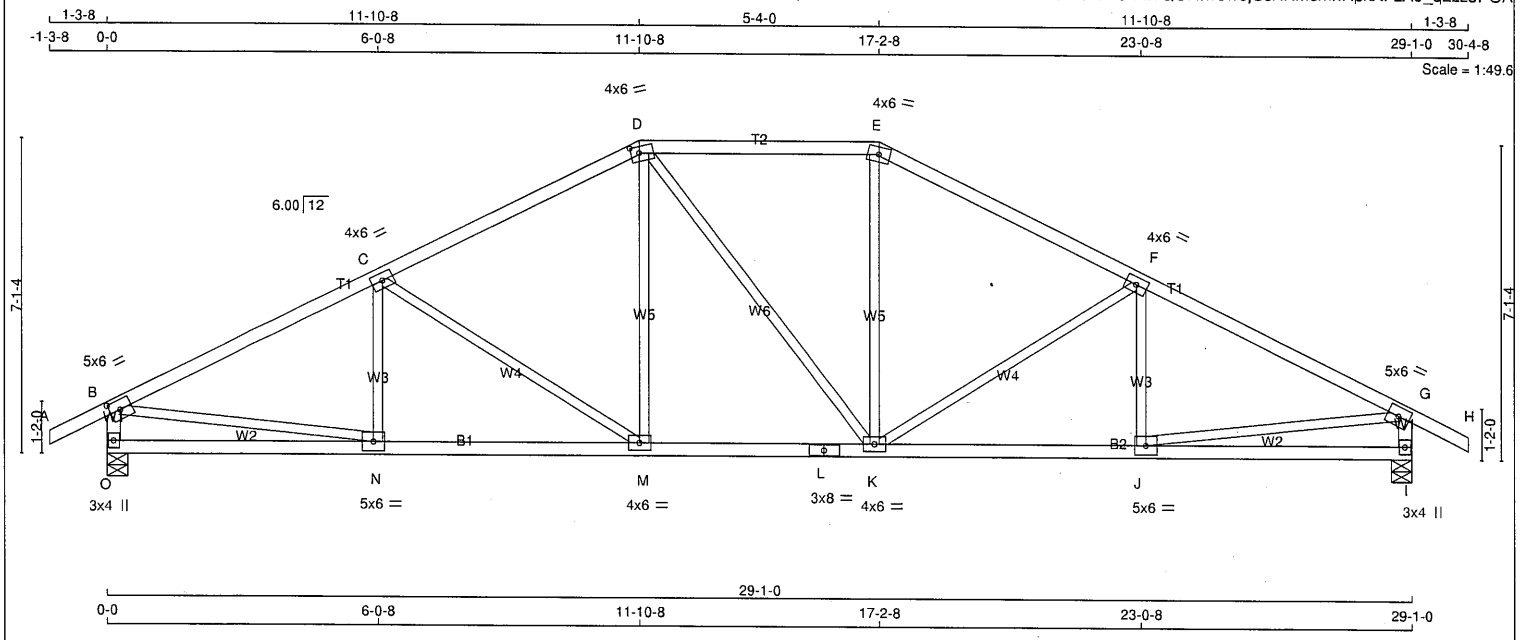

TOTAL WEIGHT = 118 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1728	0	1728	0	0	5-8	2-2
I	1728	0	1728	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
I	1220	814 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	N- C	-177 / 40	0.05 (1)
B- C	-2308 / 0	-91.8	-91.8	0.52 (1)	4.00	C- M	-517 / 0	0.50 (1)
C- D	-1883 / 0	-91.8	-91.8	0.47 (1)	4.39	M- D	0 / 383	0.09 (1)
D- E	-1663 / 0	-91.8	-91.8	0.37 (1)	4.74	D- K	0 / 0	0.00 (1)
E- F	-1883 / 0	-91.8	-91.8	0.47 (1)	4.39	K- E	0 / 384	0.09 (1)
F- G	-2307 / 0	-91.8	-91.8	0.52 (1)	4.00	K- F	-516 / 0	0.50 (1)
G- H	0 / 28	-91.8	-91.8	0.12 (1)	10.00	J- F	-178 / 39	0.05 (1)
O- B	-1680 / 0	0.0	0.0	0.17 (1)	6.41	B- N	0 / 2110	0.47 (1)
I- G	-1680 / 0	0.0	0.0	0.17 (1)	6.41	J- G	0 / 2110	0.47 (1)
O- N	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
N- M	0 / 2089	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 1663	-18.5	-18.5	0.33 (1)	10.00			
L- K	0 / 1663	-18.5	-18.5	0.33 (1)	10.00			
K- J	0 / 2089	-18.5	-18.5	0.40 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.40/1.00 (M-N:1) , WB=0.50/1.00 (C-M:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.57 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307639

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T65S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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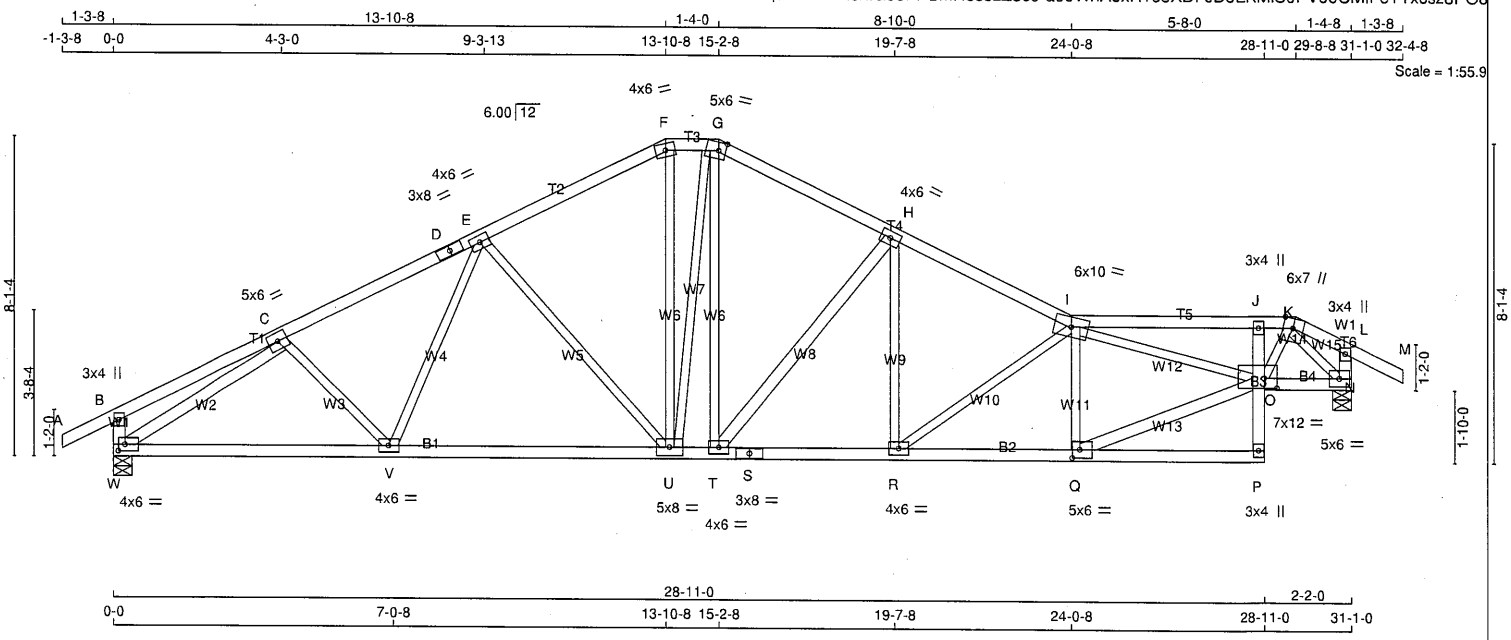

TOTAL WEIGHT = 151 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - H	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, J, L						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.50	2.25
H	TMWW-t	MT20	4.0	6.0		
I	TTWWW-m	MT20	6.0	10.0		
K	TTWW+m	MT20	6.0	7.0	Edge	
N	BMVW1-t	MT20	5.0	6.0		
O	BVMWWW-I	MT20	7.0	12.0	3.00	3.75
P	BMV+p	MT20	3.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R, T, V						
R	BMWW-t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	8.0		
U	BMWWW-t	MT20	5.0	8.0		
W	BMVW1-t	MT20	4.0	6.0	2.00	2.00

Structural component only
DWG# T-2307640

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	1835	0	1835	0	0	5-8	2-0
N	1841	0	1841	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1296	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
N	1299	866 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.23 (1)	10.00
C- D	-2369 / 0	-91.8	-91.8	0.32 (1)	4.20
D- E	-2369 / 0	-91.8	-91.8	0.32 (1)	4.20
E- F	-1906 / 0	-91.8	-91.8	0.28 (1)	4.61
F- G	-1694 / 0	-91.8	-91.8	0.05 (1)	5.08
G- H	-1912 / 0	-91.8	-91.8	0.27 (1)	4.62
H- I	-2494 / 0	-91.8	-91.8	0.31 (1)	4.11
I- J	-2345 / 0	-91.8	-91.8	0.31 (1)	4.21
J- K	-2402 / 0	-91.8	-91.8	0.23 (1)	4.24
K- L	-27 / 0	-91.8	-91.8	0.09 (1)	6.25
L- M	0 / 28	-91.8	-91.8	0.12 (1)	10.00
W- B	-275 / 0	0.0	0.0	0.03 (1)	7.81
N- L	-249 / 0	0.0	0.0	0.03 (1)	7.81
W- V	0 / 2131	-18.5	-18.5	0.45 (1)	10.00
V- U	0 / 2063	-18.5	-18.5	0.44 (1)	10.00
U- T	0 / 1699	-18.5	-18.5	0.38 (1)	10.00
T- S	0 / 2252	-18.5	-18.5	0.41 (1)	10.00
S- R	0 / 2252	-18.5	-18.5	0.41 (1)	10.00
R- Q	0 / 2741	-18.5	-18.5	0.48 (1)	10.00
Q- P	0 / 49	-18.5	-18.5	0.08 (4)	10.00
P- O	0 / 40	0.0	0.0	0.13 (1)	10.00
O- J	-470 / 0	0.0	0.0	0.10 (1)	7.81
O- N	0 / 1416	-18.5	-18.5	0.24 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	-11 / 60	0.02 (4)
V- E	0 / 159	0.05 (4)
E- U	-575 / 0	0.59 (1)
U- F	0 / 618	0.14 (1)
U- G	-33 / 11	0.04 (1)
T- G	0 / 665	0.15 (1)
T- H	-893 / 0	0.63 (1)
R- H	0 / 467	0.11 (1)
R- I	-616 / 0	0.33 (1)
Q- I	-968 / 0	0.21 (1)
O- K	0 / 1970	0.44 (1)
W- C	-2609 / 0	0.78 (1)
Q- O	0 / 2863	0.46 (1)
I- O	-386 / 0	0.15 (1)
K- N	-2115 / 0	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.32/1.00 (C-E:1) , BC=0.48/1.00 (Q-R:1) , WB=0.78/1.00 (C-W:1) , SSI=0.20/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.71 (S) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T65S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2IG33FPSMH8sozZ3c0-a09WnAoxR1e5XDFJD8EKMlS0PV66CMlFeTTx6sz8PO8

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307640

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T66S	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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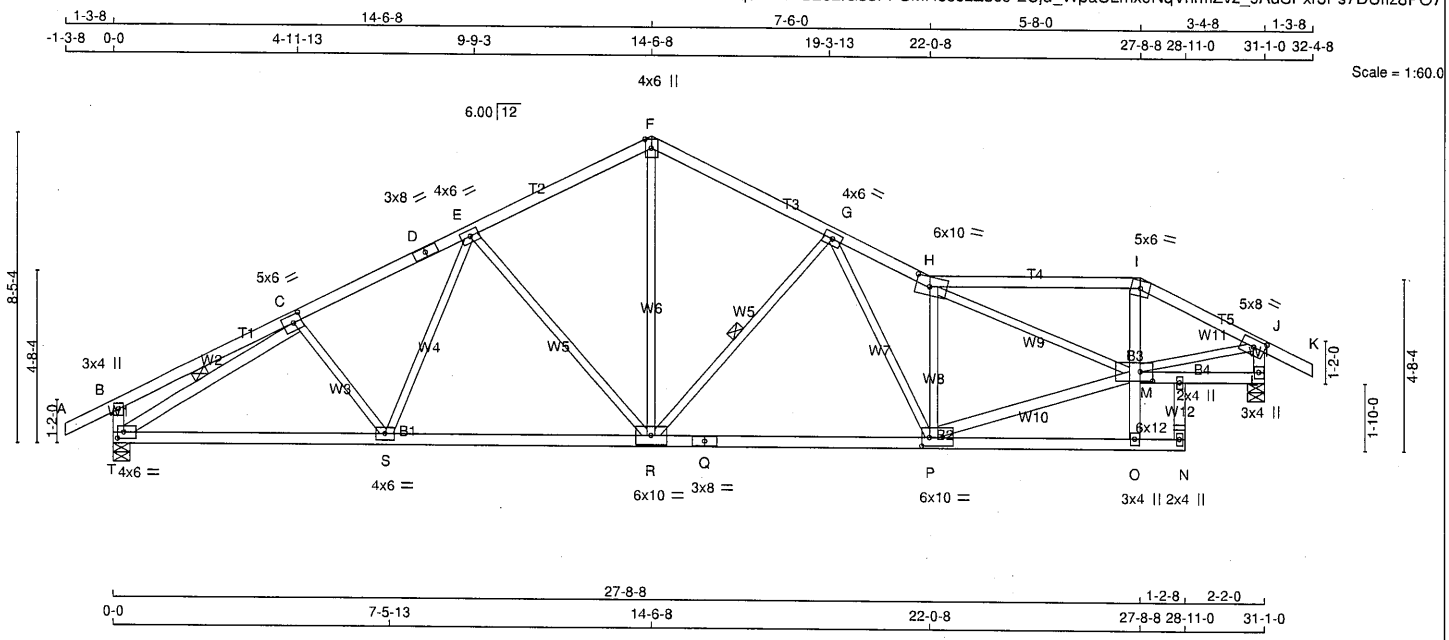

TOTAL WEIGHT = 3 X 141 = 424 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	6.0	10.0	3.25	4.50
I	TTV-m	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	8.0	2.50	3.75
L	BMV1+p	MT20	3.0	4.0		
M	BVMWWW-I	MT20	6.0	12.0	3.00	4.00
N	NP+w	MT20	2.0	4.0		
O	BMV+p	MT20	3.0	4.0		
P	BMWWW-t	MT20	6.0	10.0	2.75	2.50
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMWW-t	MT20	4.0	6.0		
T	BMVW1-t	MT20	4.0	6.0	2.00	2.00
U	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2307641

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1860	0	1860	0	0	IN-SX	IN-SX
T	1841	0	1841	0	0	5-8	2-9
						5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1315	865 / 0	0 / 0	0 / 0	0 / 0	451 / 0	0 / 0
T	1300	865 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, T

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-R, C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- S	-87 / 37	0.03 (1)
B- C	0 / 17	-91.8	-91.8	0.26 (1)	10.00	S- E	0 / 230	0.05 (1)
C- D	-2377 / 0	-91.8	-91.8	0.30 (1)	4.21	E- R	-609 / 0	0.69 (1)
D- E	-2377 / 0	-91.8	-91.8	0.30 (1)	4.21	R- F	0 / 1282	0.29 (1)
E- F	-1860 / 0	-91.8	-91.8	0.29 (1)	4.65	R- G	-897 / 0	0.34 (1)
F- G	-1859 / 0	-91.8	-91.8	0.29 (1)	4.65	G- P	0 / 727	0.16 (1)
G- H	-2801 / 0	-91.8	-91.8	0.29 (1)	3.90	P- H	-1363 / 0	0.44 (1)
H- I	-2086 / 0	-91.8	-91.8	0.45 (1)	4.24	T- C	-2632 / 0	0.47 (1)
I- J	-2322 / 0	-91.8	-91.8	0.20 (1)	4.34	M- J	0 / 2136	0.48 (1)
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P- M	0 / 2626	0.42 (1)
T- B	-309 / 0	0.0	0.0	0.03 (1)	7.81	H- M	-483 / 0	0.30 (1)
L- J	-1828 / 0	0.0	0.0	0.18 (1)	6.20			
T- S	0 / 2181	-18.5	-18.5	0.47 (1)	10.00			
S- R	0 / 2043	-18.5	-18.5	0.44 (1)	10.00			
R- Q	0 / 2230	-18.5	-18.5	0.47 (1)	10.00			
Q- P	0 / 2230	-18.5	-18.5	0.47 (1)	10.00			
P- O	0 / 34	-18.5	-18.5	0.18 (4)	10.00			
O- N	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
O- M	0 / 69	0.0	0.0	0.10 (1)	10.00			
M- I	0 / 628	0.0	0.0	0.19 (1)	10.00			
M- L	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.45/1.00 (H-I:1) , BC=0.47/1.00 (S-T:1) , WB=0.69/1.00 (E-R:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.63 (Q) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T66S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2IG33FPSMH8sozZ3c0-2Cju_WpaCLmx9NqVnrmZvz_9AuSPxr3Ps7DUflz8PO7

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307641

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T67S	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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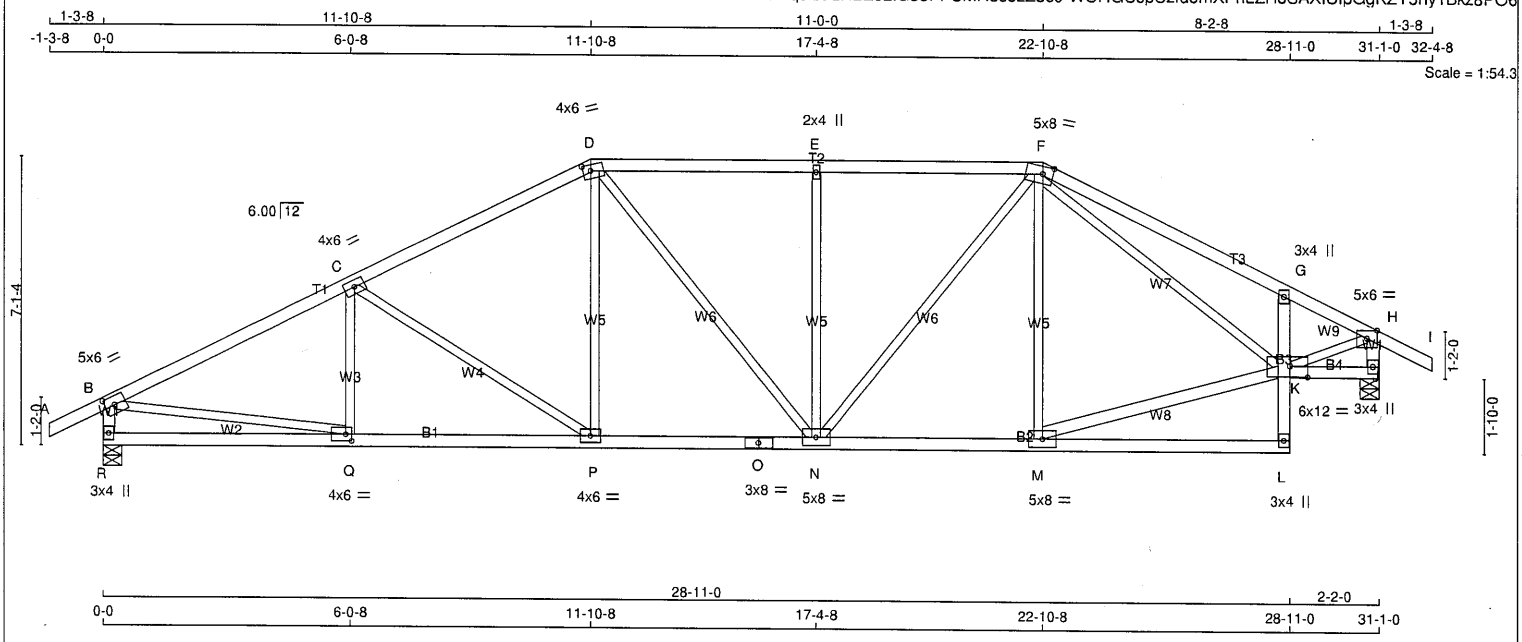

TOTAL WEIGHT = 2 X 140 = 280 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMV+w	MT20	2.0	4.0		
F	TTWWW-m	MT20	5.0	8.0	2.25	3.00
G	TMV+p	MT20	3.0	4.0		
H	TMVW-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BVMWWW-I	MT20	6.0	12.0	3.25	5.25
L	BMV+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	1.75
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307642

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1838	0	1838	0	0	5-8	2-8
J	1838	0	1838	0	0	5-8	2-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
J	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q- C	-204 / 30	0.05 (1)
B- C	-2498 / 0	-91.8	-91.8	0.54 (1)	3.85	C- P	-489 / 0	0.47 (1)
C- D	-2099 / 0	-91.8	-91.8	0.49 (1)	4.19	P- D	0 / 378	0.09 (1)
D- E	-1980 / 0	-91.8	-91.8	0.41 (1)	4.38	D- N	0 / 196	0.04 (1)
E- F	-1980 / 0	-91.8	-91.8	0.41 (1)	4.38	N- E	-619 / 0	0.54 (1)
F- G	-2141 / 0	-91.8	-91.8	0.49 (1)	4.16	N- F	0 / 711	0.16 (1)
G- H	-2140 / 0	-91.8	-91.8	0.33 (1)	4.32	M- F	-328 / 0	0.29 (1)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B- Q	0 / 2282	0.51 (1)
R- B	-1789 / 0	0.0	0.0	0.18 (1)	6.24	K- H	0 / 2092	0.47 (1)
J- H	-1817 / 0	0.0	0.0	0.18 (1)	6.21	M- K	0 / 1553	0.25 (1)
						F- K	0 / 468	0.11 (1)
R- Q	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
Q- P	0 / 2260	-18.5	-18.5	0.43 (1)	10.00			
P- O	0 / 1856	-18.5	-18.5	0.37 (1)	10.00			
O- N	0 / 1856	-18.5	-18.5	0.37 (1)	10.00			
N- M	0 / 1533	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 53	-18.5	-18.5	0.16 (4)	10.00			
L- K	0 / 50	0.0	0.0	0.14 (1)	10.00			
K- G	-513 / 0	0.0	0.0	0.11 (1)	7.81			
K- J	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.43/1.00 (P-Q:1) , WB=0.54/1.00 (E-N:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T68S	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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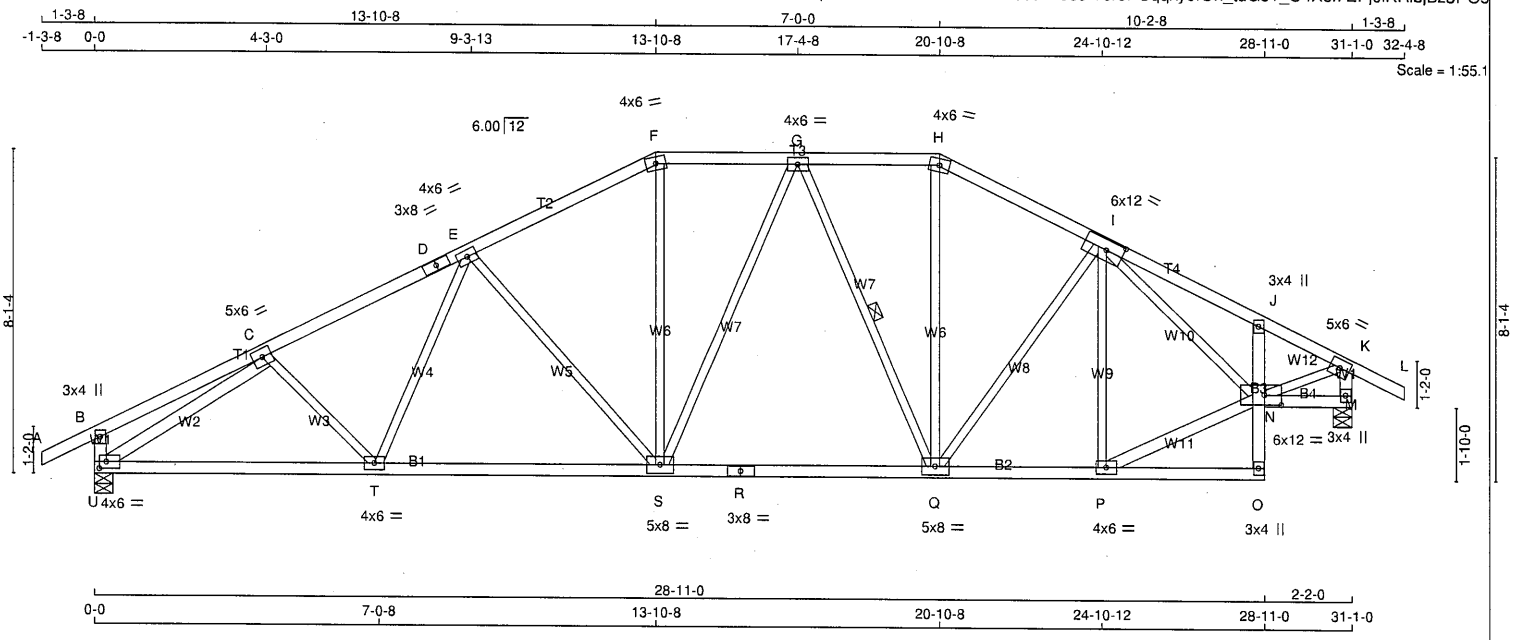

TOTAL WEIGHT = 2 X 148 = 297 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTW-m	MT20	4.0	6.0		
I	TMWWW-t	MT20	6.0	12.0	3.00	5.00
J	TMV+p	MT20	3.0	4.0		
K	TMVW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		
N	BVMWWW-l	MT20	6.0	12.0	3.00	5.00
O	BMV+p	MT20	3.0	4.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	3.0	8.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BMWW-t	MT20	4.0	6.0		
U	BMVW1-t	MT20	4.0	6.0	2.00	2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307643

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1838	0	1838	0	0	5-8	2-8
U	1838	0	1838	0	0	5-8	2-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
U	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.23 (1)	10.00
C- D	-2374 / 0	-91.8	-91.8	0.32 (1)	4.19
D- E	-2374 / 0	-91.8	-91.8	0.32 (1)	4.19
E- F	-1911 / 0	-91.8	-91.8	0.28 (1)	4.61
F- G	-1697 / 0	-91.8	-91.8	0.16 (1)	4.97
G- H	-1517 / 0	-91.8	-91.8	0.15 (1)	5.19
H- I	-1709 / 0	-91.8	-91.8	0.21 (1)	4.90
I- J	-1998 / 0	-91.8	-91.8	0.22 (1)	4.59
J- K	-2054 / 0	-91.8	-91.8	0.09 (1)	4.68
K- L	0 / 28	-91.8	-91.8	0.12 (1)	10.00
U- B	-275 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-1817 / 0	0.0	0.0	0.18 (1)	6.21
U- T	0 / 2135	-18.5	-18.5	0.45 (1)	10.00
T- S	0 / 2067	-18.5	-18.5	0.43 (1)	10.00
S- R	0 / 1690	-18.5	-18.5	0.37 (1)	10.00
R- Q	0 / 1690	-18.5	-18.5	0.37 (1)	10.00
Q- P	0 / 1501	-18.5	-18.5	0.31 (1)	10.00
P- O	0 / 39	-18.5	-18.5	0.06 (4)	10.00
O- N	0 / 33	0.0	0.0	0.10 (1)	10.00
N- J	-248 / 0	0.0	0.0	0.08 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-10 / 60	0.02 (4)
T- E	0 / 159	0.04 (4)
E- S	-576 / 0	0.59 (1)
S- F	0 / 551	0.12 (1)
Q- H	0 / 486	0.11 (1)
Q- I	0 / 17	0.01 (4)
P- I	-616 / 0	0.36 (1)
U- C	-2614 / 0	0.78 (1)
N- K	0 / 1967	0.44 (1)
P- N	0 / 1618	0.26 (1)
I- N	0 / 427	0.10 (1)
S- G	0 / 42	0.01 (4)
G- Q	-433 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.32/1.00 (C-E:1) , BC=0.45/1.00 (T-U:1) , WB=0.78/1.00 (C-U:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.59 (C) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T69	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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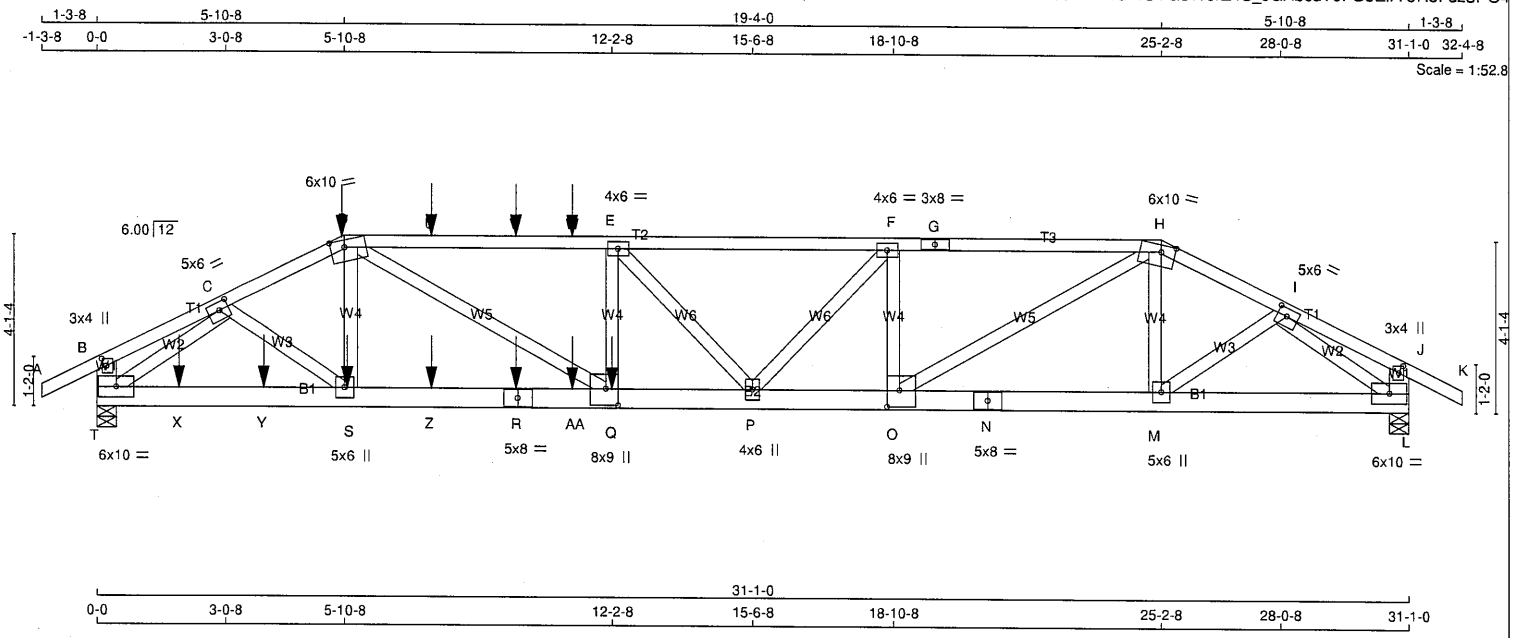

TOTAL WEIGHT = 2 X 155 = 310 lb
{M}

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - G	2x4	DRY	2100F 1.8E	SPF
G - H	2x4	DRY	2100F 1.8E	SPF
H - K	2x4	DRY	2100F 1.8E	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(65.9)
G- D	1	12	SIDE(65.9)
G- H	1	12	TOP
H- K	1	12	TOP
T- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- R	2	12	SIDE(183.1)
R- N	2	12	SIDE(183.1)
N- L	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
S- D	1	6	SIDE(20.2)
Q- E	1	4	SIDE(463.9)
2x4	1	6	
O- F	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2307644

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	3136	0	3136	0	0	5-8	1-11
T	4351	0	4351	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2212	1486 / 0	0 / 0	0 / 0	0 / 0	726 / 0	0 / 0
T	3070	2052 / 0	0 / 0	0 / 0	0 / 0	1018 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
	A- B	0 / 28	-91.8	-91.8	0.04 (1)		10.00	S- D		-350 / 67	0.03 (1)
	B- C	-2 / 0	-91.8	-91.8	0.05 (1)		10.00	M- H		-252 / 0	0.02 (1)
	C- D	-6899 / 0	-91.8	-91.8	0.10 (1)		4.53	E- P		-1720 / 0	0.26 (1)
	D- U	-10083 / 0	-91.8	-91.8	0.63 (1)		3.43	Q- F		-1962 / 0	0.18 (1)
	U- V	-10083 / 0	-91.8	-91.8	0.63 (1)		3.43	Q- E		0 / 239	0.03 (4)
	V- W	-10083 / 0	-91.8	-91.8	0.63 (1)		3.43	O- H		0 / 3989	0.35 (1)
	W- E	-10083 / 0	-91.8	-91.8	0.63 (1)		3.43	D- Q		0 / 4563	0.40 (1)
	E- F	-8937 / 0	-91.8	-91.8	0.55 (1)		3.72	P- F		0 / 1849	0.16 (1)
	F- G	-7705 / 0	-91.8	-91.8	0.32 (1)		4.17	C- S		0 / 1104	0.10 (1)
	G- H	-7705 / 0	-91.8	-91.8	0.32 (1)		4.17	T- C		-6630 / 0	0.56 (1)
	H- I	-4790 / 0	-91.8	-91.8	0.07 (1)		5.27	M- I		0 / 682	0.06 (1)
	I- J	0 / 5	-91.8	-91.8	0.04 (1)		10.00	I- L		-4686 / 0	0.40 (1)
	J- K	0 / 28	-91.8	-91.8	0.04 (1)		10.00				
	T- B	-258 / 0	0.0	0.0	0.01 (1)		7.81				
	L- J	-249 / 0	0.0	0.0	0.01 (1)		7.81				
	T- X	0 / 5270	-18.5	-18.5	0.39 (1)		10.00				
	X- Y	0 / 5270	-18.5	-18.5	0.39 (1)		10.00				
	Y- S	0 / 5270	-18.5	-18.5	0.39 (1)		10.00				
	S- Z	0 / 6149	-18.5	-18.5	0.49 (1)		10.00				
	Z- R	0 / 6149	-18.5	-18.5	0.49 (1)		10.00				
	R-AA	0 / 6149	-18.5	-18.5	0.49 (1)		10.00				
	AA- Q	0 / 6149	-18.5	-18.5	0.49 (1)		10.00				
	Q- P	0 / 10083	-18.5	-18.5	0.72 (1)		10.00				
	P- O	0 / 7705	-18.5	-18.5	0.54 (1)		10.00				
	O- N	0 / 4265	-18.5	-18.5	0.32 (1)		10.00				
	N- M	0 / 4265	-18.5	-18.5	0.32 (1)		10.00				
	M- L	0 / 3725	-18.5	-18.5	0.27 (1)		10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-402	-402	---	BACK	VERT	TOTAL	---	C1
Q	12-2-8	-1746	-1746	---	BACK	VERT	TOTAL	---	C1
R	9-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
S	5-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
U	7-11-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
V	9-11-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
W	11-3-4	-122	-122	---	BACK	VERT	TOTAL	---	C1
X	1-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
Y	3-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
Z	7-11-4	-29	-29	---	BACK	VERT	TOTAL	---	C1
AA	11-3-4	-29	-29	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.72/1.00 (P-Q:1) , WB=0.56/1.00 (C-T:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.80 (C) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T69	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.25	2.75
D	TTWW-m	MT20	6.0	10.0	2.00	4.00
E	TMWW-t	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TTWW-m	MT20	6.0	10.0	2.00	4.00
I	TMWW-t	MT20	5.0	6.0	2.25	2.75
J	TMV+p	MT20	3.0	4.0	2.00	1.25
L	BMVW1-t	MT20	6.0	10.0		
M	BMWW+t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	8.0		
O	BMWW+t	MT20	8.0	9.0	4.75	3.50
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW+t	MT20	8.0	9.0	4.75	3.50
R	BS-t	MT20	5.0	8.0		
S	BMWW+t	MT20	5.0	6.0		
T	BMVW1-t	MT20	6.0	10.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2307644

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T70	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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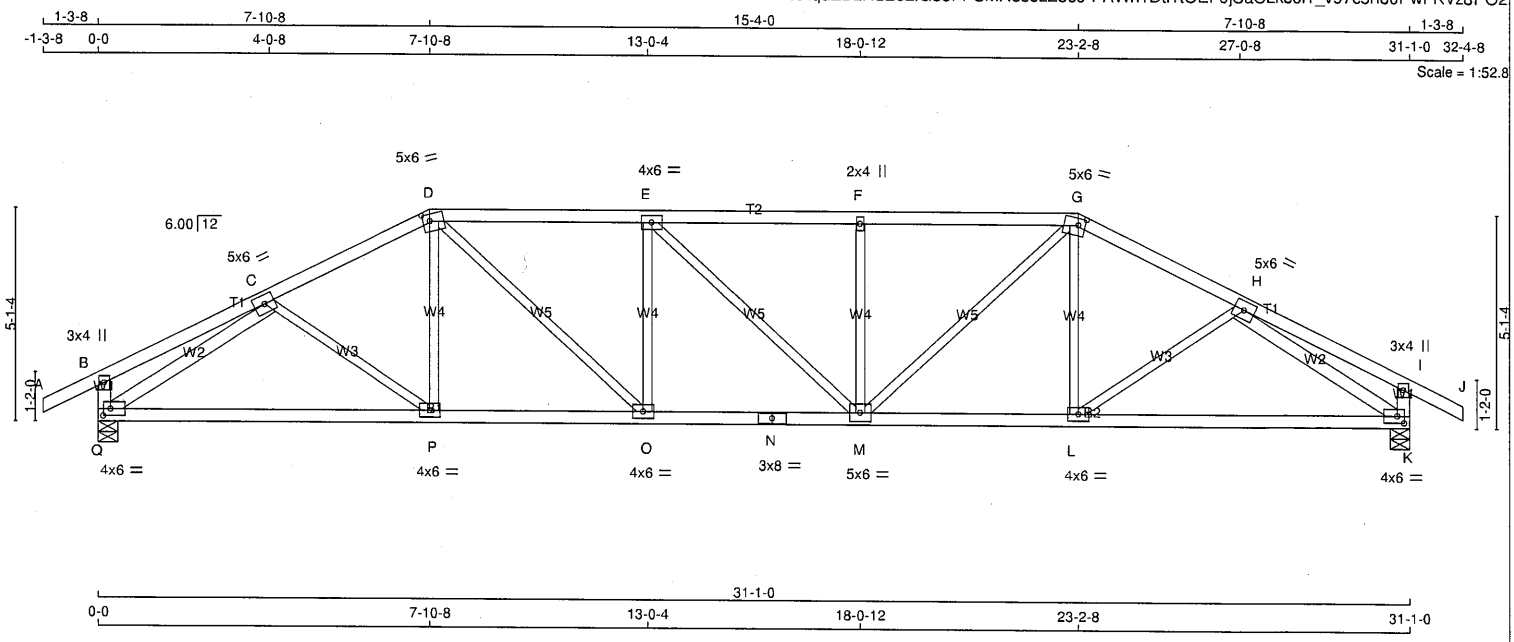

TOTAL WEIGHT = 127 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0	2.00	2.00
L, O, P						
L	BMWW-t	MT20	4.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	4.0	6.0	2.00	2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307645

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1838	0	1838	0	0	5-8	2-0
K	1838	0	1838	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
K	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- P	0 / 69	0.02 (4)
B- C	0 / 17	-91.8	-91.8	0.20 (1)	10.00	P- D	0 / 140	0.04 (4)
C- D	-2384 / 0	-91.8	-91.8	0.23 (1)	4.27	D- O	0 / 856	0.19 (1)
D- E	-2744 / 0	-91.8	-91.8	0.38 (1)	3.87	O- E	-506 / 0	0.19 (1)
E- F	-2743 / 0	-91.8	-91.8	0.34 (1)	3.91	E- M	-2 / 0	0.00 (1)
F- G	-2743 / 0	-91.8	-91.8	0.38 (1)	3.87	M- F	-505 / 0	0.19 (1)
G- H	-2385 / 0	-91.8	-91.8	0.23 (1)	4.27	M- G	0 / 853	0.19 (1)
H- I	0 / 17	-91.8	-91.8	0.20 (1)	10.00	L- G	0 / 141	0.04 (4)
I- J	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L- H	0 / 69	0.02 (4)
Q- B	-269 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2588 / 0	0.72 (1)
K- I	-269 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-2588 / 0	0.72 (1)
Q- P	0 / 2103	-18.5	-18.5	0.48 (1)	10.00			
P- O	0 / 2119	-18.5	-18.5	0.47 (1)	10.00			
O- N	0 / 2744	-18.5	-18.5	0.50 (1)	10.00			
N- M	0 / 2744	-18.5	-18.5	0.50 (1)	10.00			
M- L	0 / 2119	-18.5	-18.5	0.47 (1)	10.00			
L- K	0 / 2104	-18.5	-18.5	0.48 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
-TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.38/1.00 (F-G:1) , BC=0.50/1.00 (M-O:1) , WB=0.72/1.00 (H-K:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.88 (N) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T71	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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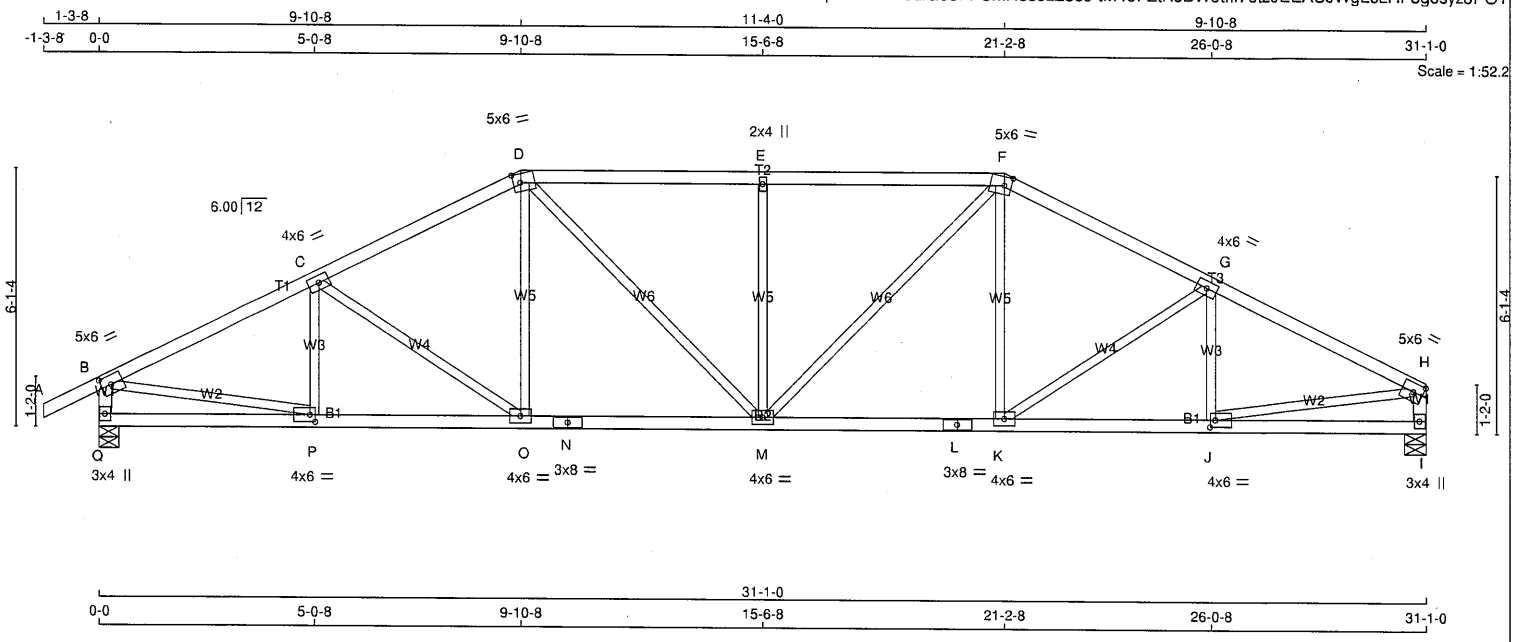

TOTAL WEIGHT = 125 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307646

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
Q	1838	0	1838	0	0	5-8	2-8
I	1714	0	1714	0	0	6-0	2-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1297	865 / 0	0 / 0	0 / 0	0 / 0	433 / 0	0 / 0
I	1212	795 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-2462 / 0	-91.8	-91.8	0.36 (1)	4.07
C- D	-2253 / 0	-91.8	-91.8	0.34 (1)	4.24
D- E	-2357 / 0	-91.8	-91.8	0.46 (1)	4.02
E- F	-2357 / 0	-91.8	-91.8	0.46 (1)	4.02
F- G	-2253 / 0	-91.8	-91.8	0.34 (1)	4.24
G- H	-2462 / 0	-91.8	-91.8	0.36 (1)	4.07
Q- B	-1795 / 0	0.0	0.0	0.18 (1)	6.24
I- H	-1671 / 0	0.0	0.0	0.17 (1)	6.42
Q- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- O	0 / 2223	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 1998	-18.5	-18.5	0.38 (1)	10.00
N- M	0 / 1998	-18.5	-18.5	0.38 (1)	10.00
M- L	0 / 1998	-18.5	-18.5	0.38 (1)	10.00
L- K	0 / 1998	-18.5	-18.5	0.38 (1)	10.00
K- J	0 / 2223	-18.5	-18.5	0.42 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-287 / 0	0.06 (1)
C- O	-278 / 0	0.17 (1)
O- D	0 / 261	0.06 (1)
D- M	0 / 515	0.12 (1)
M- E	-637 / 0	0.37 (1)
M- F	0 / 515	0.12 (1)
K- F	0 / 261	0.06 (1)
K- G	-278 / 0	0.17 (1)
J- G	-287 / 0	0.06 (1)
B- P	0 / 2255	0.51 (1)
J- H	0 / 2255	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.42/1.00 (O-P:1) , WB=0.51/1.00 (H-J:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T72S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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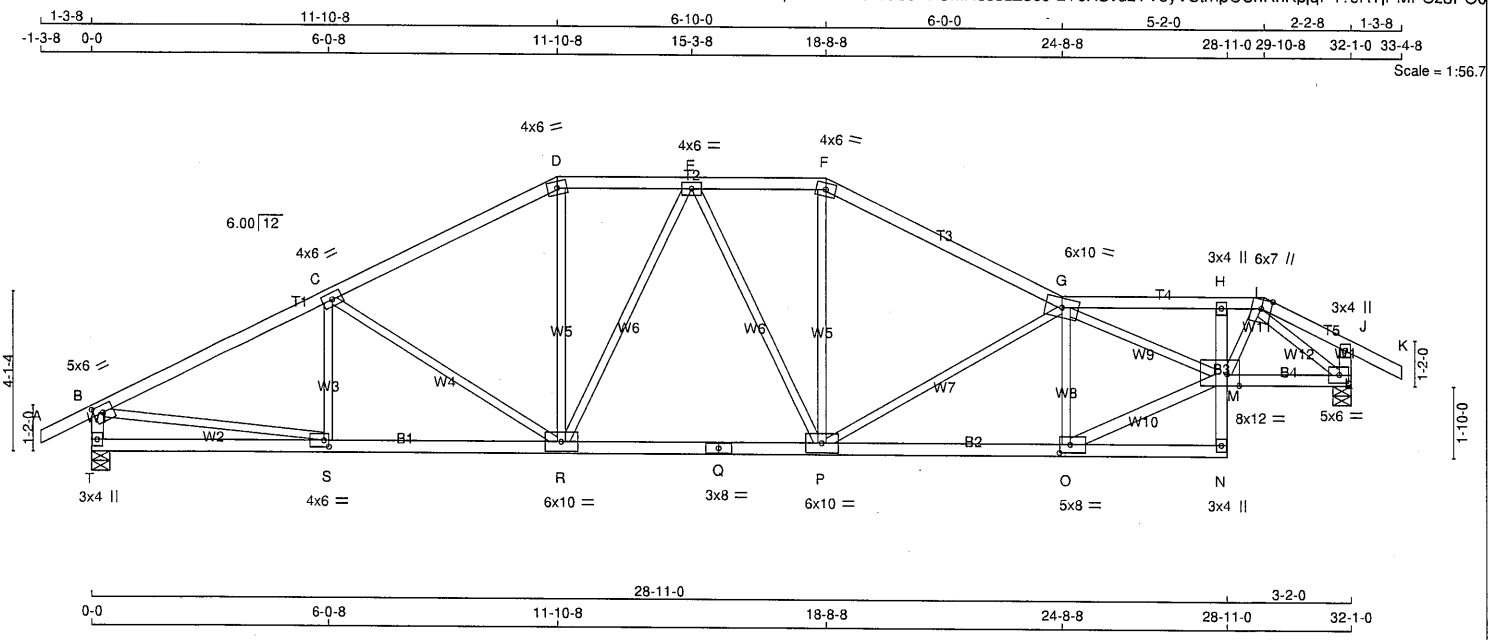

TOTAL WEIGHT = 141 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWWW-m	MT20	6.0	10.0		
H	TMV+p	MT20	3.0	4.0		
I	TTWW+m	MT20	6.0	7.0	Edge	
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BVMWWW-I	MT20	8.0	12.0	Edge	3.75
N	BMV+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BMWWW-t	MT20	6.0	10.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	6.0	10.0		
S	BMWW-t	MT20	4.0	6.0	2.00	1.50
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

~~NOTES~~ (1)
1)

Structural component only
DWG# T-2307647

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	1893	0	1893	0	0	5-8	2-10
L	1893	0	1893	0	0	5-8	2-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1336	890 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0
L	1336	890 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	S- C	-218 / 23	0.06 (1)
B- C	-2593 / 0	-91.8	-91.8	0.55 (1)	3.78	C- R	-473 / 0	0.46 (1)
C- D	-2208 / 0	-91.8	-91.8	0.49 (1)	4.10	R- D	0 / 645	0.15 (1)
D- E	-1958 / 0	-91.8	-91.8	0.16 (1)	4.69	P- F	0 / 611	0.14 (1)
E- F	-2039 / 0	-91.8	-91.8	0.16 (1)	4.61	P- G	-728 / 0	0.75 (1)
F- G	-2270 / 0	-91.8	-91.8	0.53 (1)	4.00	O- G	-1096 / 0	0.28 (1)
G- H	-2662 / 0	-91.8	-91.8	0.27 (1)	4.03	M- I	0 / 1787	0.40 (1)
H- I	-2703 / 0	-91.8	-91.8	0.17 (1)	4.09	B- S	0 / 2368	0.53 (1)
I- J	0 / 0	-91.8	-91.8	0.08 (1)	10.00	I- L	-2471 / 0	0.46 (1)
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	O- M	0 / 2837	0.46 (1)
T- B	-1844 / 0	0.0	0.0	0.19 (1)	6.16	G- M	-7 / 49	0.01 (1)
L- J	-227 / 0	0.0	0.0	0.02 (1)	7.81	R- E	-298 / 0	0.34 (1)
						E- P	-112 / 0	0.13 (1)
T- S	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
S- R	0 / 2344	-18.5	-18.5	0.46 (1)	10.00			
R- Q	0 / 2088	-18.5	-18.5	0.42 (1)	10.00			
Q- P	0 / 2088	-18.5	-18.5	0.42 (1)	10.00			
P- O	0 / 2648	-18.5	-18.5	0.51 (1)	10.00			
O- N	0 / 44	-18.5	-18.5	0.10 (4)	10.00			
N- M	0 / 29	0.0	0.0	0.11 (1)	10.00			
M- H	-342 / 0	0.0	0.0	0.09 (1)	7.81			
M- L	0 / 1853	-18.5	-18.5	0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.51/1.00 (O-P:1) , WB=0.75/1.00 (G-P:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.78 (I) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T73S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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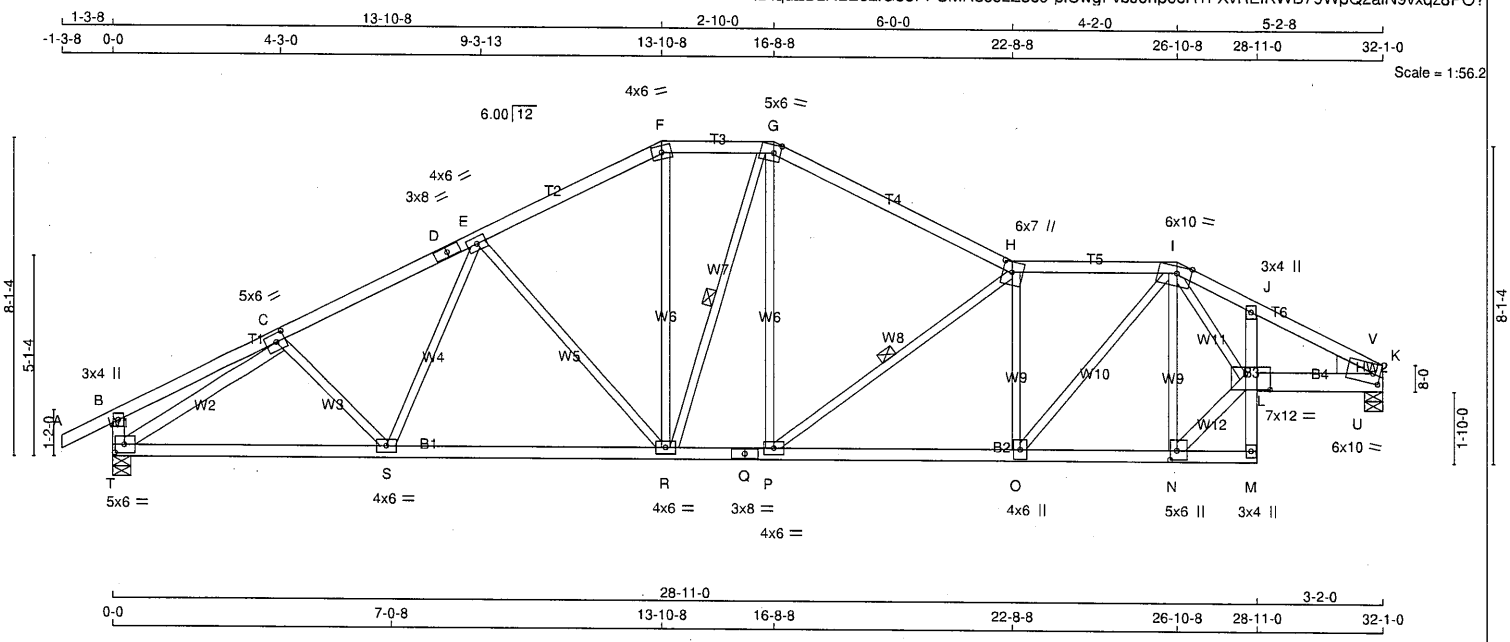

TOTAL WEIGHT = 149 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.50	2.00
H	TTWW+m	MT20	6.0	7.0	3.25	2.75
I	TTWWW-m	MT20	6.0	10.0	Edge	4.25
J	TMV+p	MT20	3.0	4.0		
K	TMBH1-m	MT20	6.0	10.0	3.00	2.00
L	BVMWW-t	MT20	7.0	12.0	5.00	7.50
M	BMV+p	MT20	3.0	4.0		
N	BMWW+t	MT20	5.0	6.0	2.75	2.00
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	6.0		
T	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2307648

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
T	1893	0	1893	0	0	5-8	2-1	
K	1769	0	1769	0	0	5-8	1-15	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1336	890 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0
K	1251	821 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-R, H-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12	(1)		10.00
B- C	0 / 18	-91.8	-91.8	0.22	(1)		10.00
C- D	-2474 / 0	-91.8	-91.8	0.33	(1)		4.11
D- E	-2474 / 0	-91.8	-91.8	0.33	(1)		4.11
E- F	-2018 / 0	-91.8	-91.8	0.29	(1)		4.50
F- G	-1793 / 0	-91.8	-91.8	0.13	(1)		4.90
G- H	-2026 / 0	-91.8	-91.8	0.51	(1)		4.22
H- I	-2427 / 0	-91.8	-91.8	0.28	(1)		4.18
I- J	-2902 / 0	-91.8	-91.8	0.22	(1)		3.91
J- V	-3036 / 0	-91.8	-91.8	0.29	(1)		3.77
V- K	-3478 / 0	-91.8	-91.8	0.31	(1)		3.51
T- B	-276 / 0	0.0	0.0	0.03	(1)		7.81
T- S	0 / 2213	-18.5	-18.5	0.46	(1)		10.00
S- R	0 / 2160	-18.5	-18.5	0.45	(1)		10.00
R- Q	0 / 1820	-18.5	-18.5	0.36	(1)		10.00
Q- P	0 / 1820	-18.5	-18.5	0.36	(1)		10.00
P- O	0 / 2446	-18.5	-18.5	0.47	(1)		10.00
O- N	0 / 1618	-18.5	-18.5	0.33	(1)		10.00
N- M	0 / 61	-18.5	-18.5	0.08	(1)		10.00
M- L	-8 / 8	0.0	0.0	0.12	(1)		10.00
L- J	-41 / 22	0.0	0.0	0.12	(1)		7.81
L- U	0 / 2673	-18.5	-18.5	0.52	(1)		10.00
U- K	0 / 2673	-18.5	-18.5	0.52	(1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
C- S	0 / 66	0.02	(4)
S- E	0 / 152	0.05	(4)
E- R	-571 / 0	0.59	(1)
R- F	0 / 599	0.13	(1)
R- G	-82 / 0	0.04	(1)
P- G	0 / 579	0.13	(1)
P- H	-806 / 0	0.33	(1)
O- H	-874 / 0	0.34	(1)
O- I	0 / 1252	0.28	(1)
N- I	-1433 / 0	0.55	(1)
T- C	-2710 / 0	0.81	(1)
N- L	0 / 2200	0.35	(1)
I- L	0 / 1751	0.39	(1)
U- V	0 / 646	0.00	(1)

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.52/1.00 (L-U:1) , WB=0.81/1.00 (C-T:1) , SSI=0.31/1.00 (K-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (T) (INPUT = 0.90)
JSI METAL= 0.61 (C) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T74S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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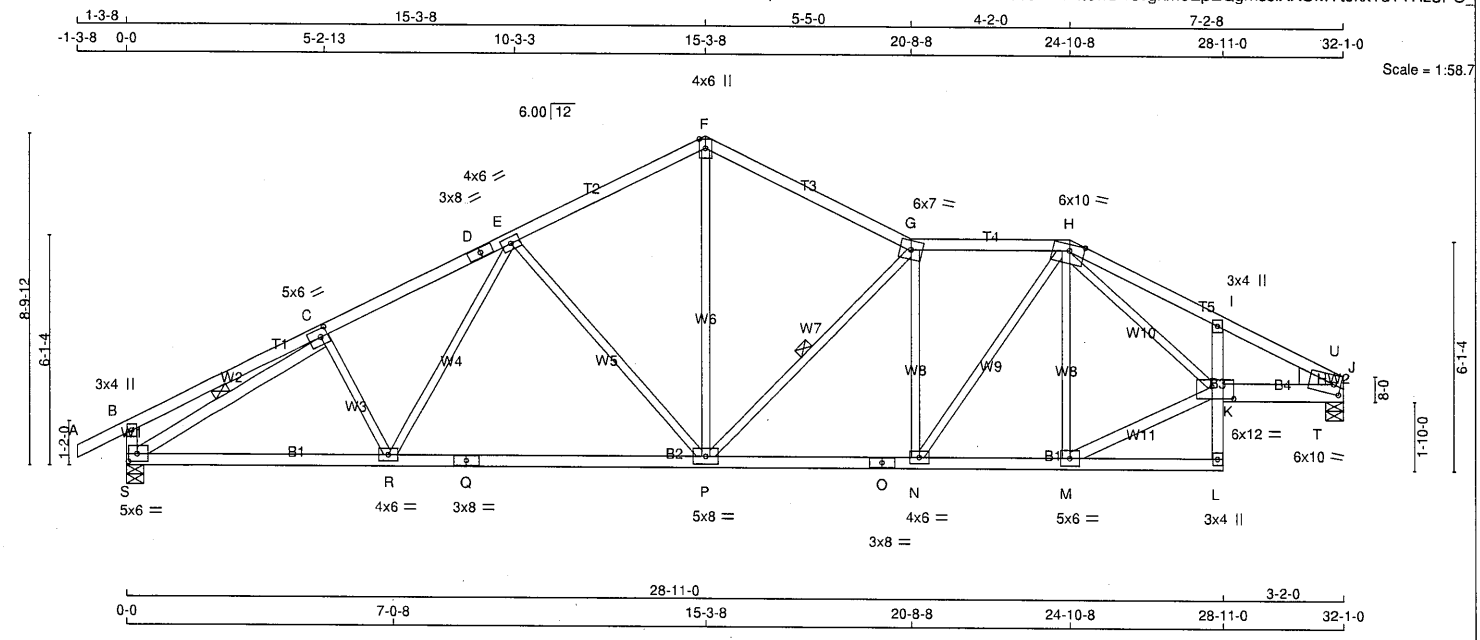

TOTAL WEIGHT = 145 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TTWW-m	MT20	6.0	7.0		
H	TTWWW-m	MT20	6.0	10.0	2.00	4.75
I	TMV+p	MT20	3.0	4.0		
J	TMBH1-m	MT20	6.0	10.0	3.00	2.25
K	BVMWW-t	MT20	6.0	12.0	3.75	7.00
L	BMV+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Structural component only
DWG# T-2307649

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
S	1893	0	1893	0	0	5-8	2-1	
J	1769	0	1769	0	0	5-8	1-15	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1336	890 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0
J	1251	821 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, C-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.29 (1)	10.00
C- D	-2493 / 0	-91.8	-91.8	0.34 (1)	4.08
D- E	-2493 / 0	-91.8	-91.8	0.34 (1)	4.08
E- F	-1893 / 0	-91.8	-91.8	0.33 (1)	4.58
F- G	-1872 / 0	-91.8	-91.8	0.41 (1)	4.49
G- H	-2224 / 0	-91.8	-91.8	0.26 (1)	4.35
H- I	-3082 / 0	-91.8	-91.8	0.34 (1)	3.70
I- U	-3190 / 0	-91.8	-91.8	0.28 (1)	3.69
U- J	-3710 / 0	-91.8	-91.8	0.31 (1)	3.41
S- B	-318 / 0	0.0	0.0	0.03 (1)	7.81
S- R	0 / 2283	-18.5	-18.5	0.49 (1)	10.00
R- Q	0 / 2096	-18.5	-18.5	0.46 (1)	10.00
Q- P	0 / 2096	-18.5	-18.5	0.46 (1)	10.00
P- O	0 / 2237	-18.5	-18.5	0.48 (1)	10.00
O- N	0 / 2237	-18.5	-18.5	0.48 (1)	10.00
N- M	0 / 1713	-18.5	-18.5	0.34 (1)	10.00
M- L	0 / 63	-18.5	-18.5	0.09 (4)	10.00
L- K	0 / 31	0.0	0.0	0.18 (1)	10.00
K- I	-216 / 0	0.0	0.0	0.13 (1)	7.81
K- T	0 / 2827	-18.5	-18.5	0.61 (1)	10.00
T- J	0 / 2830	-18.5	-18.5	0.61 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-109 / 33	0.03 (1)
R- E	0 / 285	0.06 (1)
E- P	-644 / 0	0.82 (1)
P- F	0 / 1234	0.28 (1)
P- G	-823 / 0	0.35 (1)
N- G	-673 / 0	0.39 (1)
N- H	0 / 892	0.20 (1)
M- H	-726 / 0	0.43 (1)
S- C	-2745 / 0	0.52 (1)
M- K	0 / 1856	0.30 (1)
H- K	0 / 1439	0.32 (1)
T- U	0 / 784	0.00 (1)

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.41/1.00 (F-G:1) , BC=0.61/1.00 (J-T:1) , WB=0.82/1.00 (E-P:1) , SSI=0.41/1.00 (J-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.69 (O) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T75	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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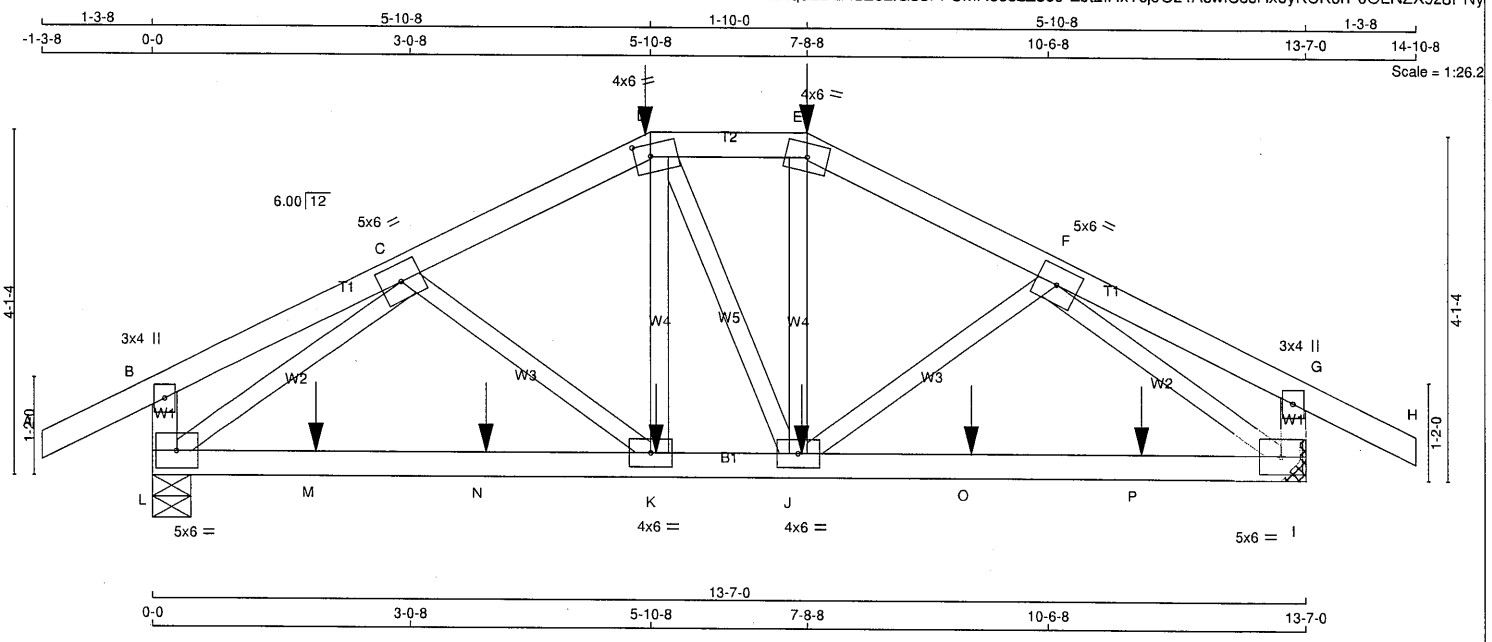

TOTAL WEIGHT = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307650

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1563	0	1563	0	0	5-8	1-11
I	1563	0	1563	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-11.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1103	738 / 0	0 / 0	0 / 0	0 / 0	365 / 0	0 / 0
I	1103	738 / 0	0 / 0	0 / 0	0 / 0	365 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 10	-91.8	-91.8	0.11 (1)	10.00
C- D	-1902 / 0	-91.8	-91.8	0.15 (1)	4.71
D- E	-1692 / 0	-91.8	-91.8	0.09 (1)	5.00
E- F	-1902 / 0	-91.8	-91.8	0.15 (1)	4.71
F- G	0 / 10	-91.8	-91.8	0.11 (1)	10.00
G- H	0 / 28	-91.8	-91.8	0.13 (1)	10.00
L- B	-238 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-238 / 0	0.0	0.0	0.03 (1)	7.81
L- M	0 / 1579	-18.5	-18.5	0.45 (1)	10.00
M- N	0 / 1579	-18.5	-18.5	0.45 (1)	10.00
N- K	0 / 1579	-18.5	-18.5	0.45 (1)	10.00
K- J	0 / 1692	-18.5	-18.5	0.44 (1)	10.00
J- O	0 / 1580	-18.5	-18.5	0.45 (1)	10.00
O- P	0 / 1580	-18.5	-18.5	0.45 (1)	10.00
P- I	0 / 1580	-18.5	-18.5	0.45 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 125	0.05 (4)
D- J	0 / 2	0.00 (4)
J- E	0 / 127	0.05 (4)
C- K	0 / 139	0.03 (1)
L- C	-2014 / 0	0.52 (1)
J- F	0 / 139	0.03 (1)
F- I	-2014 / 0	0.52 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-402	-402	---	FRONT	VERT	TOTAL	---	C1
E	7-8-8	-402	-402	---	FRONT	VERT	TOTAL	---	C1
J	7-7-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
K	5-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
M	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
N	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
O	9-7-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
P	11-7-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.45/1.00 (K-L:1) , WB=0.52/1.00 (F-I:1) , SSI=0.16/1.00 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (I) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T76	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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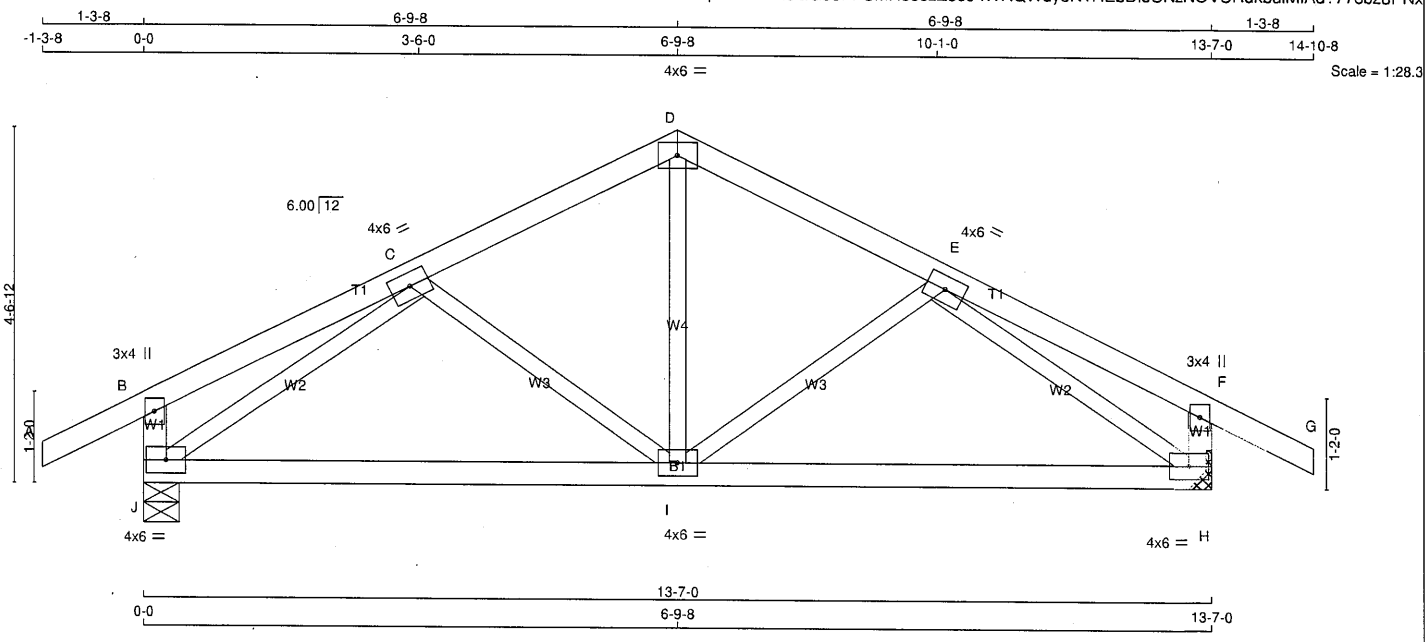

TOTAL WEIGHT = 54 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	873	0	873	0	0	5-8	1-8
H	873	0	873	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	615	417 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	615	417 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12	(1)	10.00	I- D	0 / 378	0.09 (1)
B- C	0 / 15	-91.8	-91.8	0.16	(1)	10.00	I- E	-185 / 0	0.06 (1)
C- D	-709 / 0	-91.8	-91.8	0.13	(1)	6.25	C- I	-185 / 0	0.06 (1)
D- E	-709 / 0	-91.8	-91.8	0.13	(1)	6.25	J- C	-961 / 0	0.29 (1)
E- F	0 / 15	-91.8	-91.8	0.16	(1)	10.00	E- H	-961 / 0	0.29 (1)
F- G	0 / 28	-91.8	-91.8	0.12	(1)	10.00			
J- B	-247 / 0	0.0	0.0	0.02	(1)	7.81			
H- F	-247 / 0	0.0	0.0	0.02	(1)	7.81			
J- I	0 / 768	-18.5	-18.5	0.30	(4)	10.00			
I- H	0 / 768	-18.5	-18.5	0.30	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (E-F:1) , BC=0.30/1.00 (H-I:4) , WB=0.29/1.00 (E-H:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (J) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 0.95)

Structural component only
DWG# T-2307651

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T77	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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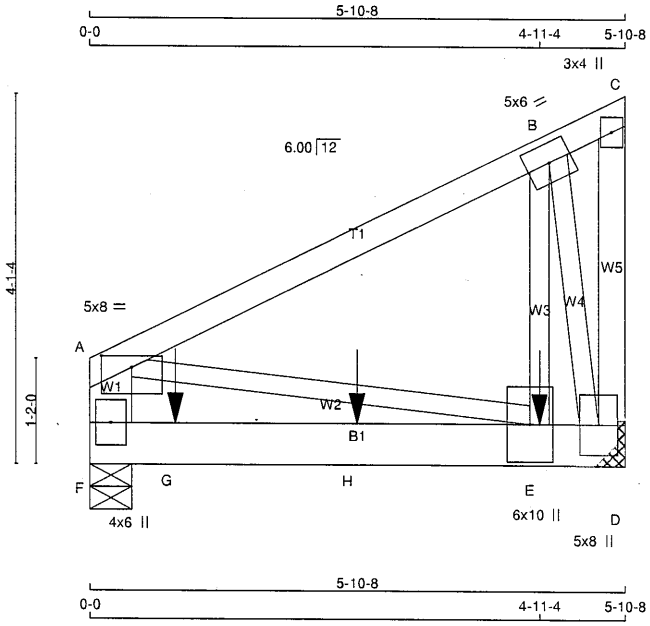

TOTAL WEIGHT = 2 X 31 = 63 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(19.5)
D- B	1	6	SIDE(20.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.50	4.00

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307652

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	1863	0	1863	0	0	MECHANICAL	
F	1483	0	1483	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1311	896 / 0	0 / 0	0 / 0	0 / 0	415 / 0	0 / 0
F	1041	726 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-920 / 0	-91.8	-91.8	1	0.17 (1)	6.25	E- B	0 / 2877	0.36 (1)
B- C	-111 / 0	-91.8	-91.8	1	0.14 (1)	6.25	B- D	-3082 / 0	0.35 (1)
D- C	0 / 213	0.0	0.0	1	0.02 (1)	10.00	A- E	0 / 848	0.10 (1)
F- A	-749 / 0	0.0	0.0	1	0.03 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	1	0.55 (1)	10.00			
G- H	0 / 0	-18.5	-18.5	1	0.55 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	1	0.55 (1)	10.00			
E- D	0 / 678	-18.5	-18.5	1	0.37 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	4-11-4	-601	-601	---	BACK	VERT	TOTAL	---	C1
G	11-4	-205	-205	---	TOP	VERT	TOTAL	---	C1
H	2-11-4	-1088	-1088	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 824 (0.09")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.55/1.00 (E-F:1) , WB=0.36/1.00 (B-E:1) , SSI=0.40/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T77	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-Ai?ojyzj8LP5DNK?24Vcxi1RC8tJUoQJsfsgb2z8PNw

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	5.0	8.0		
E	BMWW+t	MT20	6.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2307652

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T77Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:58:44 2023 Page 1
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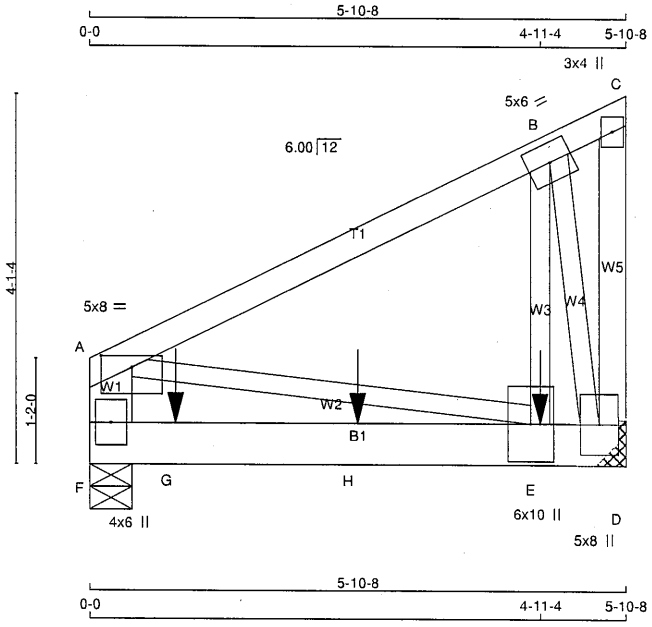

TOTAL WEIGHT = 2 X 31 = 63 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(211.0)
D- B	1	6	SIDE(222.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.50	4.00

Structural component only
DWG# T-2307653

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	2499	0	2499	0	0	MECHANICAL	
F	1063	0	1063	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1761	1190 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0
F	744	530 / 0	0 / 0	0 / 0	0 / 0	214 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-802 / 0	-91.8	-91.8	0.17 (1)	6.25	E- B	0 / 2451	0.30 (1)
B- C	-116 / 0	-91.8	-91.8	0.15 (1)	6.25	B- D	-2714 / 0	0.31 (1)
D- C	0 / 225	0.0	0.0	0.02 (1)	10.00	A- E	0 / 742	0.09 (1)
F- A	-678 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
G- H	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
E- D	0 / 597	-18.5	-18.5	0.06 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	4-11-4	-1668	-1668	---	BACK	VERT	TOTAL	---	C1
G	11-4	-174	-174	---	TOP	VERT	TOTAL	---	C1
H	2-11-4	-205	-205	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.20/1.00 (E-F:1) , WB=0.31/1.00 (B-D:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T77Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-euZBwlzMveXyqXvBbn0rTwZc0YIyDGRT4JcD7Uz8PNv

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	5.0	8.0		
E	BMWW+t	MT20	6.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307653

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T77Z2	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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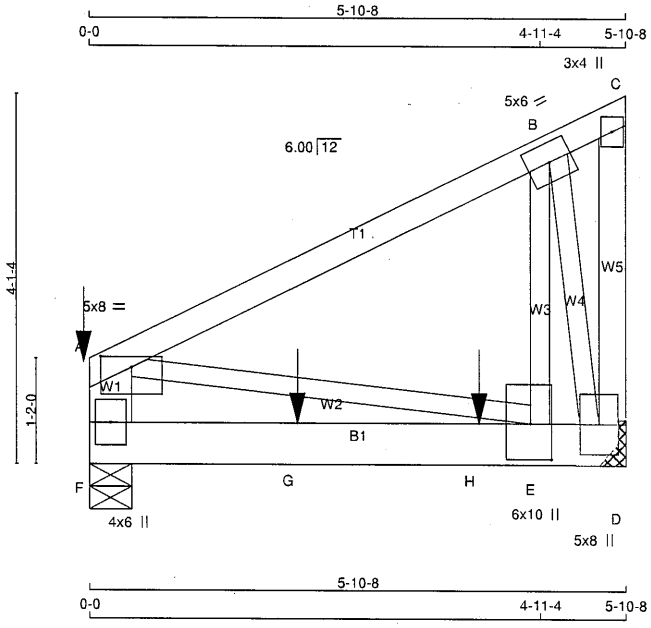

Scale = 1:24.5

TOTAL WEIGHT = 2 X 31 = 63 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP,

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.50	4.00
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		

Structural component only
DWG# T-2307654

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1325	0	1325	0	0	MECHANICAL	
F	1600	0	1600	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	935	623 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
F	1127	766 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-769 / 0	-91.8	-91.8	0.16 (1)	6.25	E- B	0 / 2330	0.29 (1)
B- C	-117 / 0	-91.8	-91.8	0.15 (1)	6.25	B- D	-2610 / 0	0.30 (1)
D- C	0 / 228	0.0	0.0	0.02 (1)	10.00	A- E	0 / 712	0.09 (1)
F- A	-924 / 0	0.0	0.0	0.03 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.55 (1)	10.00			
G- H	0 / 0	-18.5	-18.5	0.55 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	0.55 (1)	10.00			
E- D	0 / 574	-18.5	-18.5	0.38 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-185	-185	---	TOP	VERT	TOTAL	---	C1
G	2-3-4	-957	-957	---	BACK	VERT	TOTAL	---	C1
H	4-3-4	-463	-463	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.08")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.55/1.00 (E-F:1) , WB=0.30/1.00 (B-D:1) , SSI=0.47/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	T77Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:quZDzNLEe2lG33FPSMH8sozZ3c0-657Z8e_gyfpShUN9VX4076nnxYkyjtcJzLngwz8PNu

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	5.0	8.0		
E	BMWW+t	MT20	6.0	10.0	4.75	2.75
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307654

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T78	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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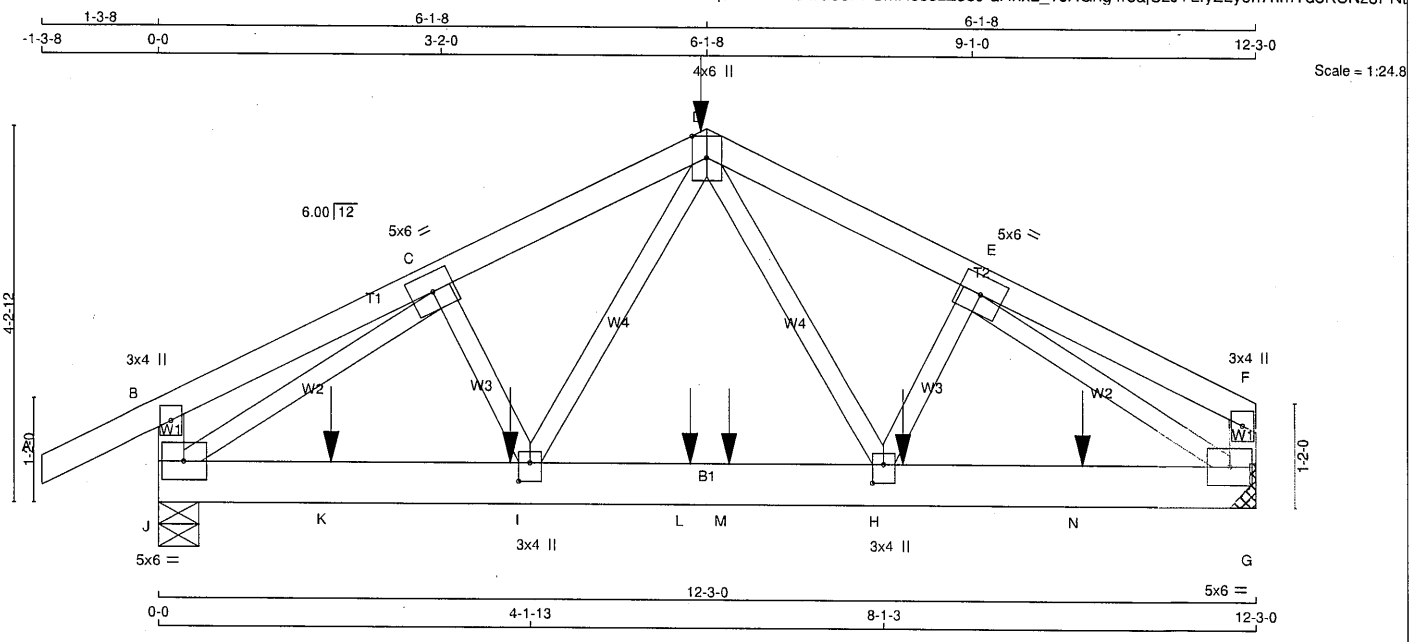

TOTAL WEIGHT = 56 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	5.0	6.0		
H	BMWW+t	MT20	3.0	4.0	2.50	1.50
I	BMWW+t	MT20	3.0	4.0	2.50	1.50
J	BMVW1-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1497	0	1497	0	0	5-8	1-10
G	1373	0	1373	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1057	704 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0
G	972	634 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 11	-91.8	-91.8	0.12 (1)	10.00
C- D	-1819 / 0	-91.8	-91.8	0.16 (1)	4.79
D- E	-1820 / 0	-91.8	-91.8	0.16 (1)	4.79
E- F	0 / 11	-91.8	-91.8	0.11 (1)	10.00
J- B	-240 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-116 / 0	0.0	0.0	0.01 (1)	7.81
J- K	0 / 1590	-18.5	-18.5	0.27 (1)	10.00
K- I	0 / 1590	-18.5	-18.5	0.27 (1)	10.00
I- L	0 / 1565	-18.5	-18.5	0.28 (1)	10.00
L- M	0 / 1565	-18.5	-18.5	0.28 (1)	10.00
M- H	0 / 1565	-18.5	-18.5	0.28 (1)	10.00
H- N	0 / 1591	-18.5	-18.5	0.27 (1)	10.00
N- G	0 / 1591	-18.5	-18.5	0.27 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- H	0 / 150	0.06 (4)
H- E	0 / 68	0.03 (4)
I- D	0 / 148	0.06 (4)
C- I	0 / 68	0.03 (4)
J- C	-1990 / 0	0.51 (1)
E- G	-1991 / 0	0.51 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-1-8	-816	-816	---	FRONT	VERT	TOTAL	---	C1
H	8-3-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
I	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
K	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
L	5-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
M	6-4-8	-29	-29	---	FRONT	VERT	TOTAL	---	C1
N	10-3-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.28/1.00 (H-I:1) , WB=0.51/1.00 (E-G:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.44 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307655

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T78Z	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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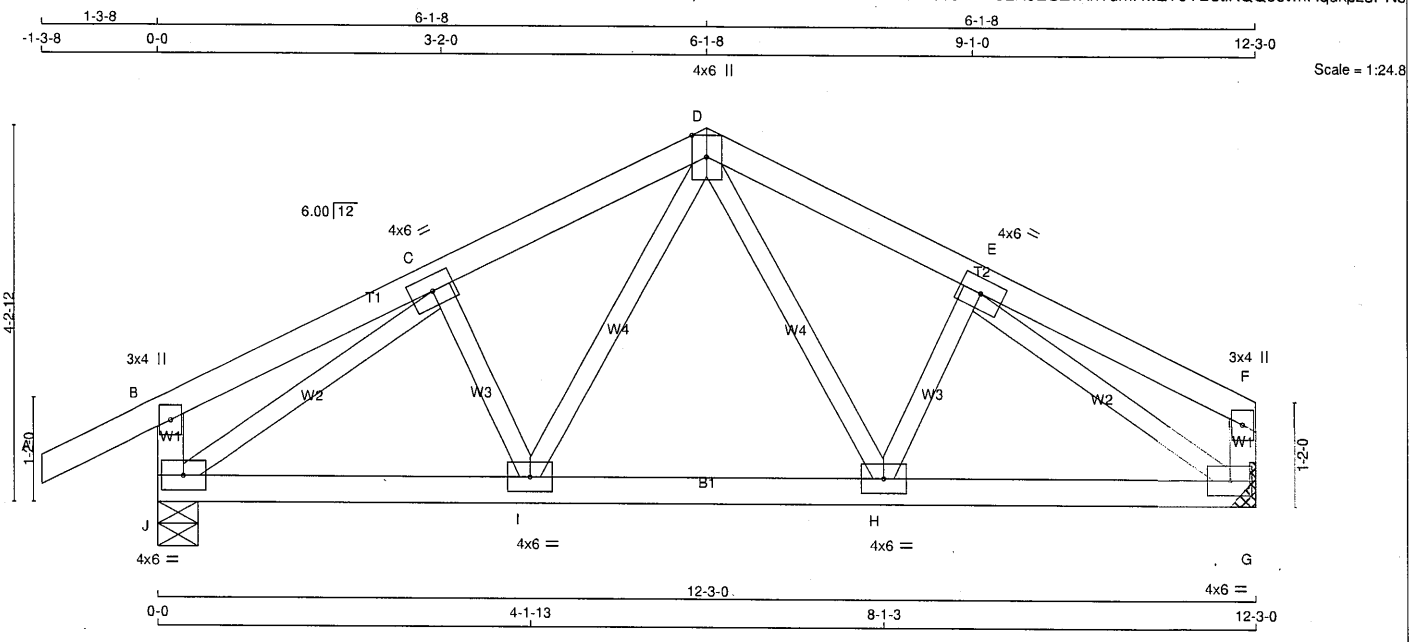

TOTAL WEIGHT = 49 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	800	0	800	0	0	IN-SX	IN-SX
G	675	0	675	0	0	5-8	1-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	563	383 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0
G	478	313 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	D- H	0 / 226	0.05 (1)
B- C	0 / 13	-91.8	-91.8	0.13 (1)	10.00	H- E	-134 / 1	0.02 (1)
C- D	-713 / 0	-91.8	-91.8	0.10 (1)	6.25	I- D	0 / 226	0.05 (1)
D- E	-712 / 0	-91.8	-91.8	0.10 (1)	6.25	C- I	-134 / 1	0.02 (1)
E- F	0 / 13	-91.8	-91.8	0.13 (1)	10.00	J- C	-864 / 0	0.22 (1)
J- B	-235 / 0	0.0	0.0	0.02 (1)	7.81	E- G	-864 / 0	0.22 (1)
G- F	-111 / 0	0.0	0.0	0.01 (1)	7.81			
J- I	0 / 681	-18.5	-18.5	0.15 (1)	10.00			
I- H	0 / 524	-18.5	-18.5	0.11 (1)	10.00			
H- G	0 / 681	-18.5	-18.5	0.15 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.15/1.00 (I-J:1) , WB=0.22/1.00 (C-J:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (E) (INPUT = 0.90)
JSI METAL= 0.19 (E) (INPUT = 0.95)

Structural component only
DWG# T-2307656

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	T79	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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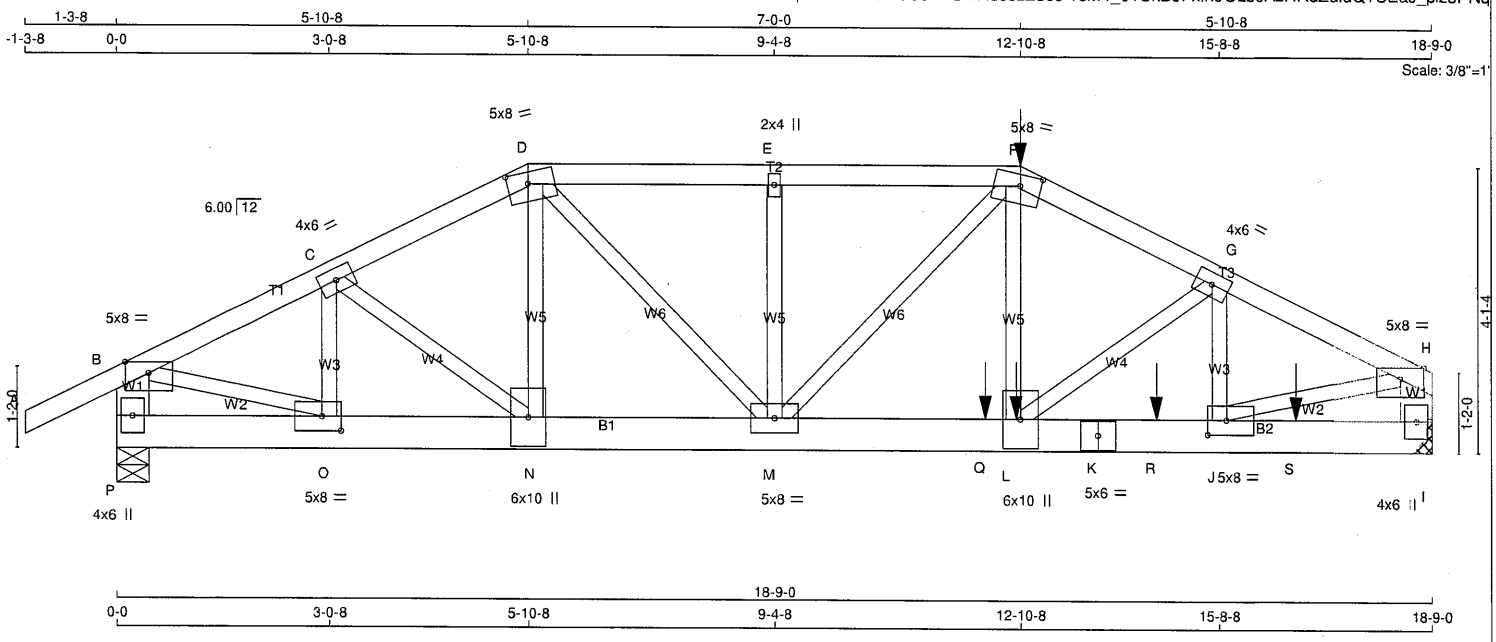

TOTAL WEIGHT = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.00	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	8.0	Edge	
I	BMV1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	5.0	8.0	2.50	3.25
K	BS-t	MT20	5.0	6.0		
L	BMWW+t	MT20	6.0	10.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW+t	MT20	6.0	10.0		
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307657

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1807	0	1807	0	0	5-8	1-15
I	2381	0	2381	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-9.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1274	862 / 0	0 / 0	0 / 0	0 / 0	412 / 0	0 / 0
I	1682	1111 / 0	0 / 0	0 / 0	0 / 0	571 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00
B- C	-2222 / 0	-91.8	-91.8	0.18 (1)	4.39
C- D	-2422 / 0	-91.8	-91.8	0.19 (1)	4.22
D- E	-2972 / 0	-91.8	-91.8	0.30 (1)	3.74
E- F	-2972 / 0	-91.8	-91.8	0.30 (1)	3.74
F- G	-3629 / 0	-91.8	-91.8	0.28 (1)	3.44
G- H	-3205 / 0	-91.8	-91.8	0.27 (1)	3.66
P- B	-1747 / 0	0.0	0.0	0.12 (1)	7.48
I- H	-2290 / 0	0.0	0.0	0.16 (1)	6.74
P- O	0 / 0	-18.5	-18.5	0.07 (1)	10.00
O- N	0 / 1996	-18.5	-18.5	0.33 (1)	10.00
N- M	0 / 2153	-18.5	-18.5	0.31 (1)	10.00
M- Q	0 / 3265	-18.5	-18.5	0.67 (1)	10.00
Q- L	0 / 3265	-18.5	-18.5	0.67 (1)	10.00
L- K	0 / 2873	-18.5	-18.5	0.48 (1)	10.00
K- R	0 / 2873	-18.5	-18.5	0.48 (1)	10.00
R- J	0 / 2873	-18.5	-18.5	0.48 (1)	10.00
J- S	0 / 0	-18.5	-18.5	0.11 (1)	10.00
S- I	0 / 0	-18.5	-18.5	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-526 / 0	0.09 (1)
N- D	0 / 200	0.05 (1)
M- E	-28 / 34	0.01 (4)
L- F	0 / 1196	0.30 (1)
K- G	-371 / 0	0.09 (1)
J- H	-427 / 0	0.18 (1)
B- O	0 / 2060	0.51 (1)
J- G	-778 / 0	0.13 (1)
L- G	0 / 458	0.11 (1)
M- F	0 / 1109	0.27 (1)
N- M	0 / 2965	0.73 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	12-10-8	-402	-402	---	FRONT	VERT	TOTAL	---	C1
L	12-9-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
Q	12-4-8	-920	-920	---	FRONT	VERT	TOTAL	---	C1
R	14-9-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
S	16-9-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.67/1.00 (L-M:1) , WB=0.73/1.00 (H-J:1) , SSI=0.81/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.61 (F) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	PB1	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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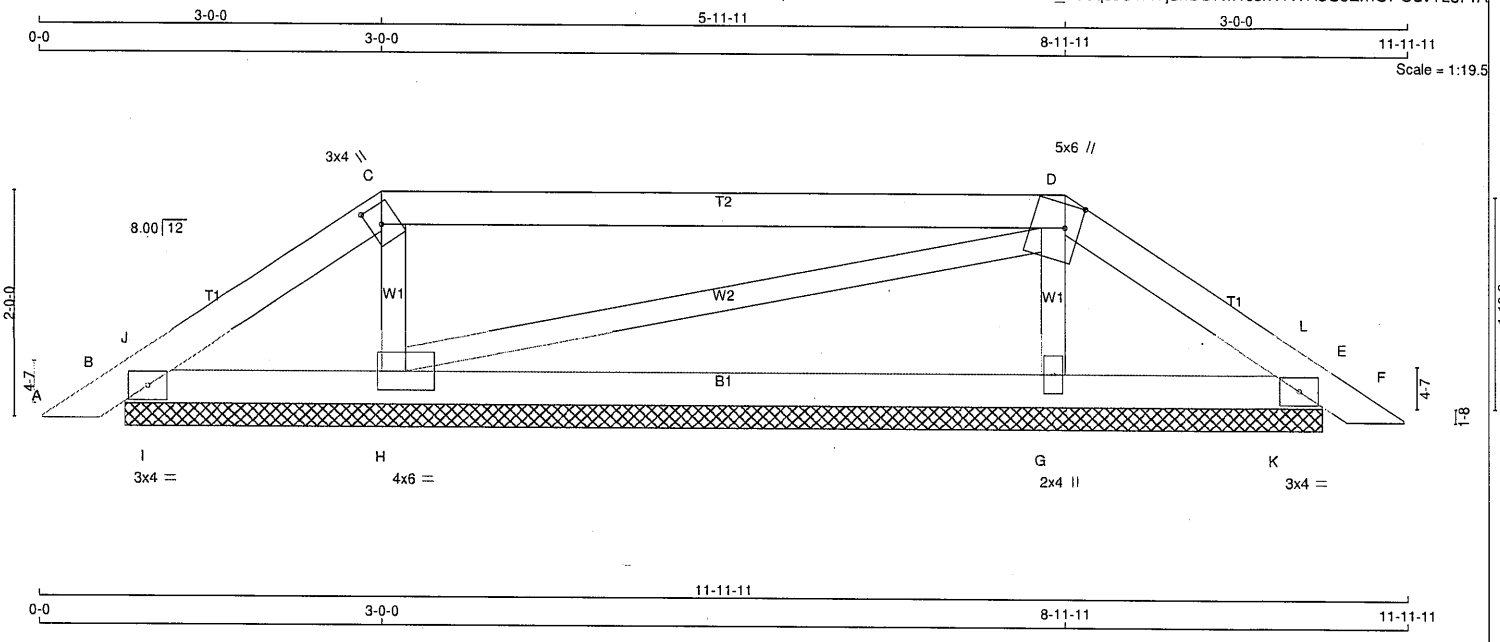

TOTAL WEIGHT = 34 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	202	0	202	0	0	10-5-13	1-8
E	209	0	209	0	0	10-5-13	1-8
H	438	0	438	0	0	10-5-13	1-8
G	424	0	424	0	0	10-5-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	139	111 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
E	144	116 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
H	312	192 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
G	303	184 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	H- C	-321 / 0	0.05 (1)
B- J	-37 / 5	-91.8	-91.8	0.02 (1)	6.25	H- D	-12 / 0	0.01 (1)
J- C	-68 / 0	-91.8	-91.8	0.05 (1)	6.25	G- D	-310 / 0	0.05 (1)
C- D	-32 / 0	-91.8	-91.8	0.56 (1)	6.25	I- J	-152 / 0	0.00 (1)
D- L	-82 / 0	-91.8	-91.8	0.05 (1)	6.25	K- L	-151 / 0	0.00 (1)
L- E	-51 / 2	-91.8	-91.8	0.02 (1)	6.25			
E- F	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B- I	0 / 53	-18.5	-18.5	0.06 (1)	10.00			
I- H	0 / 53	-18.5	-18.5	0.11 (4)	10.00			
H- G	0 / 43	-18.5	-18.5	0.11 (4)	10.00			
G- K	0 / 64	-18.5	-18.5	0.11 (4)	10.00			
K- E	0 / 64	-18.5	-18.5	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-D:1) , BC=0.11/1.00 (G-K:4) , WB=0.05/1.00 (C-H:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (C) (INPUT = 0.90)
JSI METAL= 0.07 (G) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307580

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	PB2	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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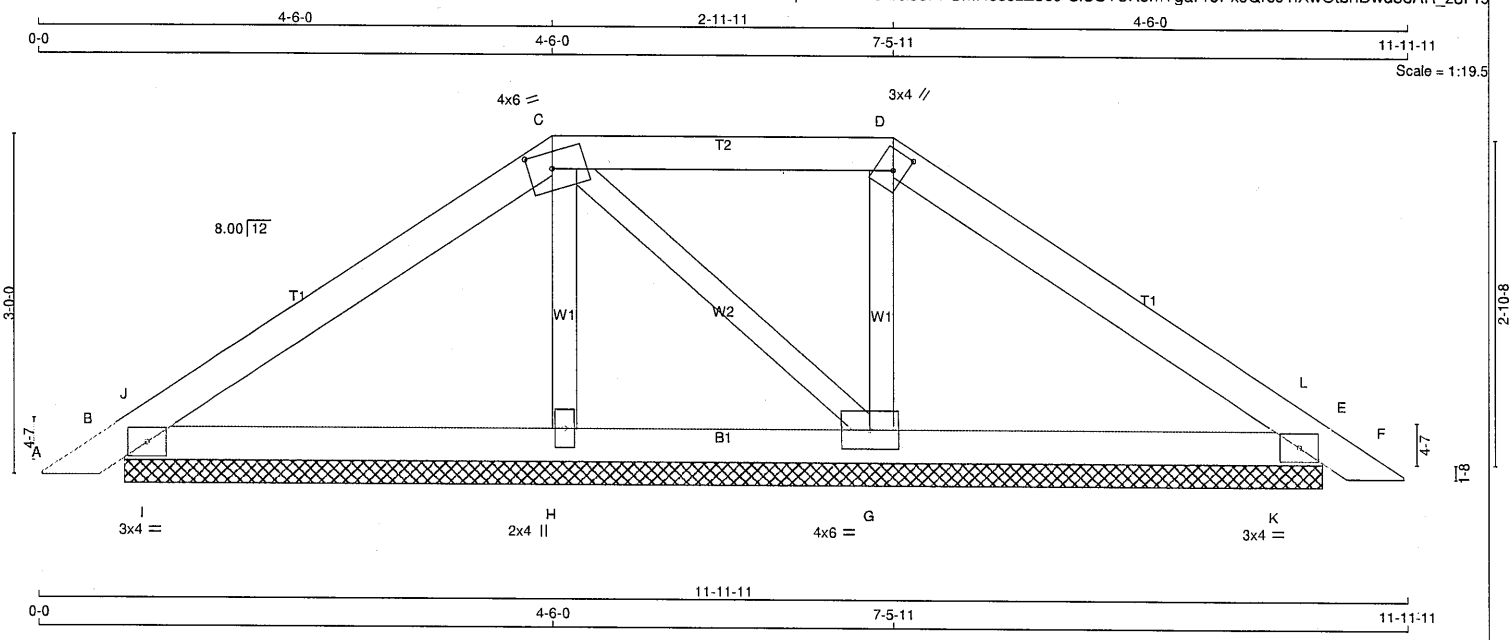

TOTAL WEIGHT = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	334	0	334	0	0	10-5-13	1-8
E	313	0	313	0	0	10-5-13	1-8
H	274	0	274	0	0	10-5-13	1-8
G	353	0	353	0	0	10-5-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	234	166 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
E	220	155 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
H	195	118 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	250	163 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03	(1)	10.00	H- C	-177 / 0	0.03 (1)
B- J	-62 / 0	-91.8	-91.8	0.06	(1)	6.25	C- G	-44 / 0	0.01 (1)
J- C	-144 / 0	-91.8	-91.8	0.16	(1)	6.25	G- D	-227 / 0	0.04 (1)
C- D	-75 / 0	-91.8	-91.8	0.14	(1)	6.25	I- J	-301 / 0	0.00 (1)
D- L	-108 / 0	-91.8	-91.8	0.16	(1)	6.25	K- L	-305 / 0	0.00 (1)
L- E	-37 / 0	-91.8	-91.8	0.06	(1)	6.25			
E- F	0 / 15	-91.8	-91.8	0.03	(1)	10.00			
B- I	0 / 114	-18.5	-18.5	0.14	(1)	10.00			
I- H	0 / 114	-18.5	-18.5	0.14	(1)	10.00			
H- G	0 / 107	-18.5	-18.5	0.08	(1)	10.00			
G- K	0 / 84	-18.5	-18.5	0.14	(1)	10.00			
K- E	0 / 84	-18.5	-18.5	0.14	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-J:1) , BC=0.14/1.00 (B-I:1) , WB=0.04/1.00 (D-G:1) , SSI=0.24/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307581

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	PB3	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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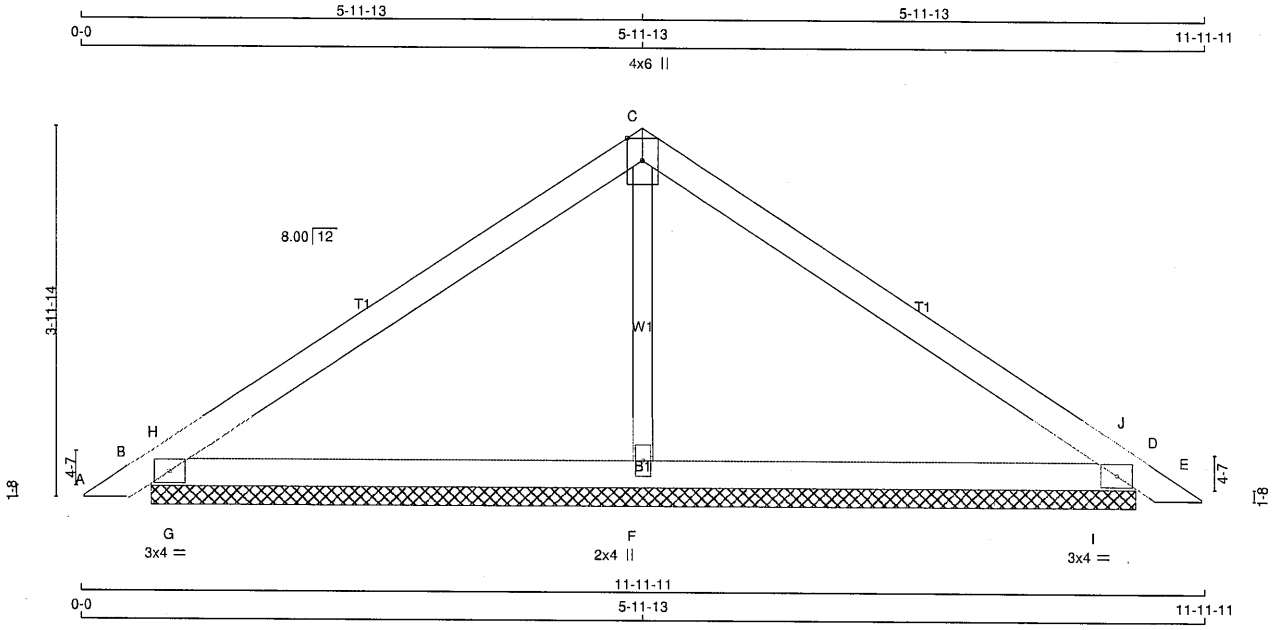

TOTAL WEIGHT = 4 X 31 = 124 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	436	0	436	0	0	10-5-13	1-8
D	436	0	436	0	0	10-5-13	1-8
F	401	0	401	0	0	10-5-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	306	217 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	306	217 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
F	287	168 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	F- C	-168 / 0	0.04 (1)
B- H	-45 / 29	-91.8	-91.8	0.17 (1)	6.25	G- H	-640 / 0	0.00 (1)
H- C	-226 / 0	-91.8	-91.8	0.31 (1)	6.25	I- J	-640 / 0	0.00 (1)
C- J	-226 / 0	-91.8	-91.8	0.31 (1)	6.25			
J- D	-45 / 29	-91.8	-91.8	0.17 (1)	6.25			
D- E	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B- G	0 / 176	-18.5	-18.5	0.28 (1)	10.00			
G- F	0 / 176	-18.5	-18.5	0.28 (1)	10.00			
F- I	0 / 176	-18.5	-18.5	0.28 (1)	10.00			
I- D	0 / 176	-18.5	-18.5	0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-J:1) , BC=0.28/1.00 (B-G:1) , WB=0.04/1.00 (C-F:1) , SSI=0.48/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.37 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 0.95)

Structural component only
DWG# T-2307582

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	PB11	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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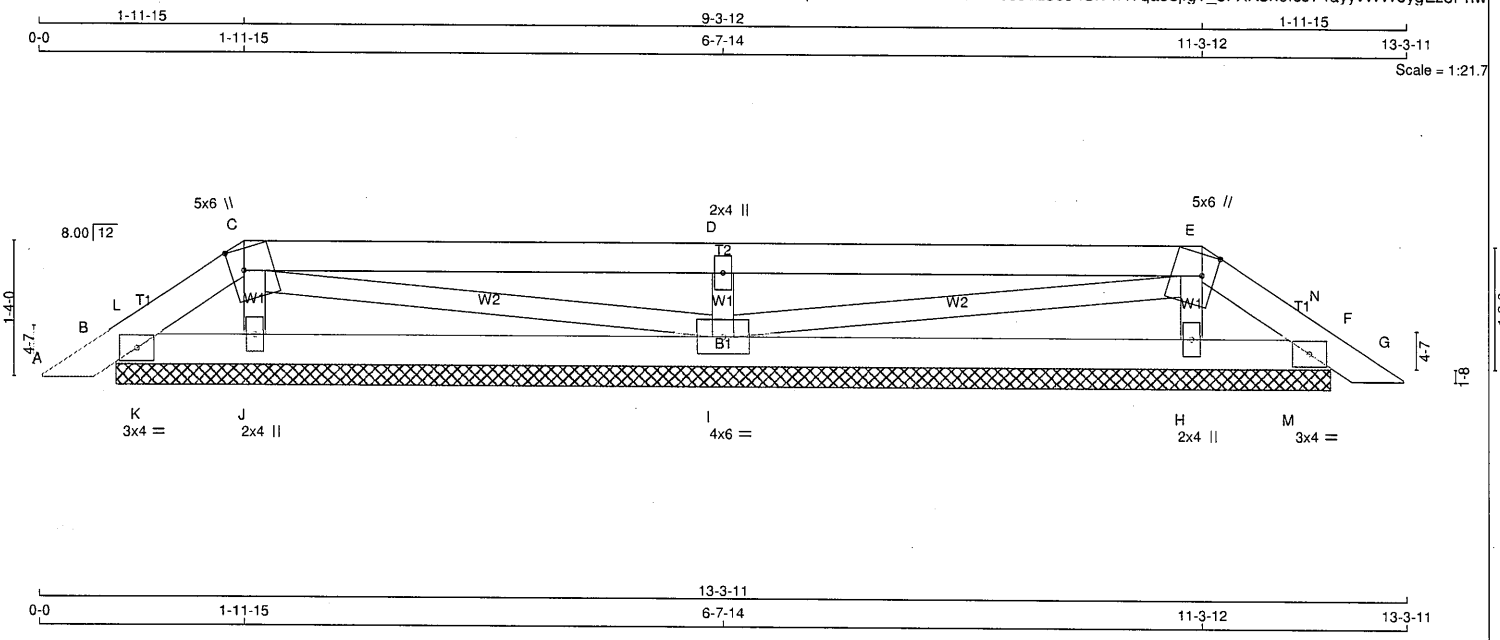

TOTAL WEIGHT = 38 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	132	0	132	0	0	11-9-13	1-8
F	132	0	132	0	0	11-9-13	1-8
J	266	0	266	0	0	11-9-13	1-8
I	624	0	624	0	0	11-9-13	1-8
H	266	0	266	0	0	11-9-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	90	77 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
F	90	77 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
J	191	109 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
I	440	299 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
H	191	109 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B- L	-30 / 5	-91.8	-91.8	0.01 (4)	6.25
L- C	-40 / 0	-91.8	-91.8	0.01 (1)	6.25
C- D	-2 / 0	-91.8	-91.8	0.34 (1)	10.00
D- E	-2 / 0	-91.8	-91.8	0.34 (1)	10.00
E- N	-40 / 0	-91.8	-91.8	0.01 (1)	6.25
N- F	-30 / 5	-91.8	-91.8	0.01 (4)	6.25
F- G	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B- K	0 / 30	-18.5	-18.5	0.02 (1)	10.00
K- J	0 / 30	-18.5	-18.5	0.06 (4)	10.00
J- I	0 / 10	-18.5	-18.5	0.08 (4)	10.00
I- H	0 / 10	-18.5	-18.5	0.08 (4)	10.00
H- M	0 / 30	-18.5	-18.5	0.06 (4)	10.00
M- F	0 / 30	-18.5	-18.5	0.02 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-178 / 0	0.02 (1)
C- I	-8 / 0	0.00 (1)
I- D	-534 / 0	0.07 (1)
I- E	-8 / 0	0.00 (1)
H- E	-178 / 0	0.02 (1)
K- L	-74 / 0	0.00 (1)
M- N	-74 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (D-E:1) , BC=0.08/1.00 (I-J:4) , WB=0.07/1.00 (D-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 0.95)

Structural component only
DWG# T-2307611

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	PB12	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:37:24 2023 Page 1
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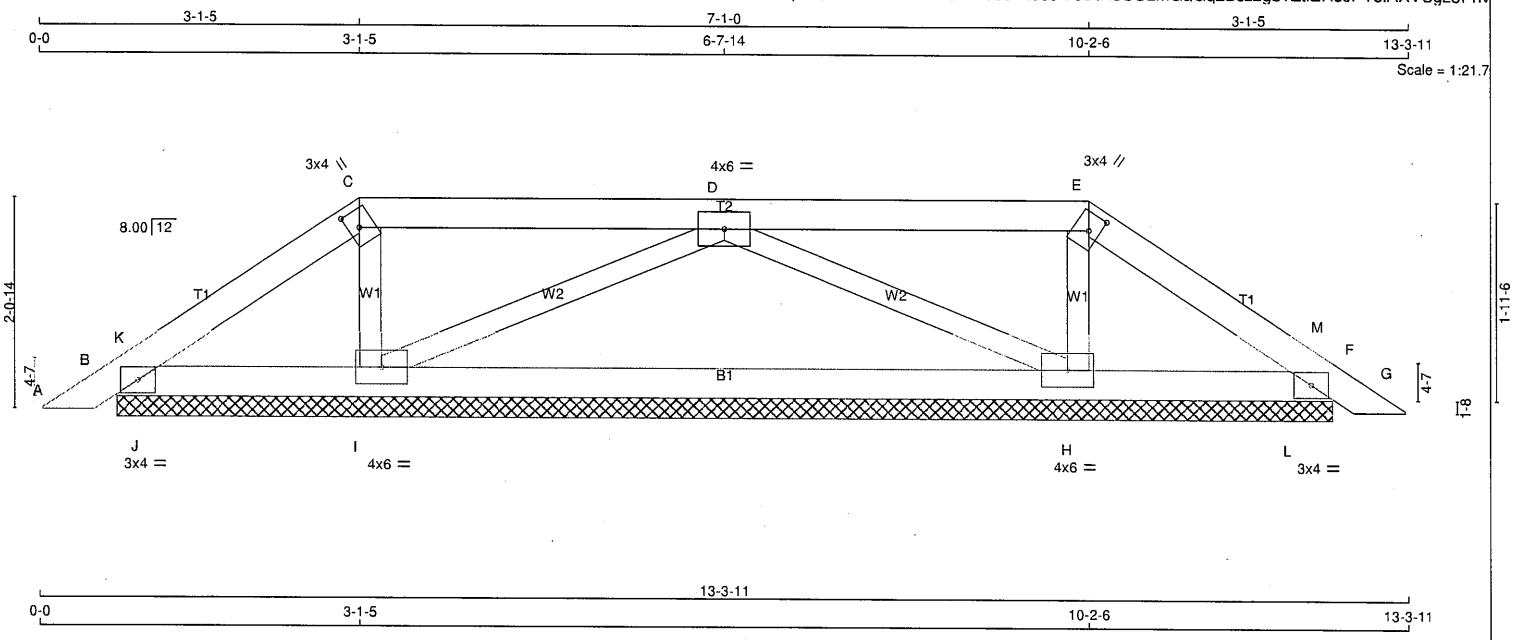

TOTAL WEIGHT = 38 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	6.0		
E	TTW+h	MT20	3.0	4.0	2.00	1.25
F	TMB1-I	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	125	0	125	0	0	11-9-13	1-8
F	125	0	125	0	0	11-9-13	1-8
I	586	0	586	0	0	11-9-13	1-8
H	586	0	586	0	0	11-9-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	85	72 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
F	85	72 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
I	416	263 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	416	263 / 0	0 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)		10.00	I- C	-250 / 0	0.04 (1)
B- K	0 / 112	-91.8	-91.8	0.04 (1)		10.00	H- E	-250 / 0	0.04 (1)
K- C	0 / 64	-91.8	-91.8	0.06 (1)		10.00	I- D	-460 / 0	0.12 (1)
C- D	0 / 73	-91.8	-91.8	0.19 (1)		10.00	D- H	-460 / 0	0.12 (1)
D- E	0 / 73	-91.8	-91.8	0.19 (1)		10.00	J- K	-187 / 0	0.00 (1)
E- M	0 / 64	-91.8	-91.8	0.06 (1)		10.00	L- M	-187 / 0	0.00 (1)
M- F	0 / 112	-91.8	-91.8	0.04 (1)		10.00			
F- G	0 / 15	-91.8	-91.8	0.03 (1)		10.00			
B- J	-57 / 0	-18.5	-18.5	0.06 (1)		6.25			
J- I	-57 / 0	-18.5	-18.5	0.15 (4)		6.25			
I- H	0 / 342	-18.5	-18.5	0.16 (4)		10.00			
H- L	-57 / 0	-18.5	-18.5	0.15 (4)		6.25			
L- F	-57 / 0	-18.5	-18.5	0.06 (1)		6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (D-E:1) , BC=0.16/1.00 (H-I:4) , WB=0.12/1.00 (D-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (H) (INPUT = 0.90)
JSI METAL= 0.10 (I) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307612

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	PB13	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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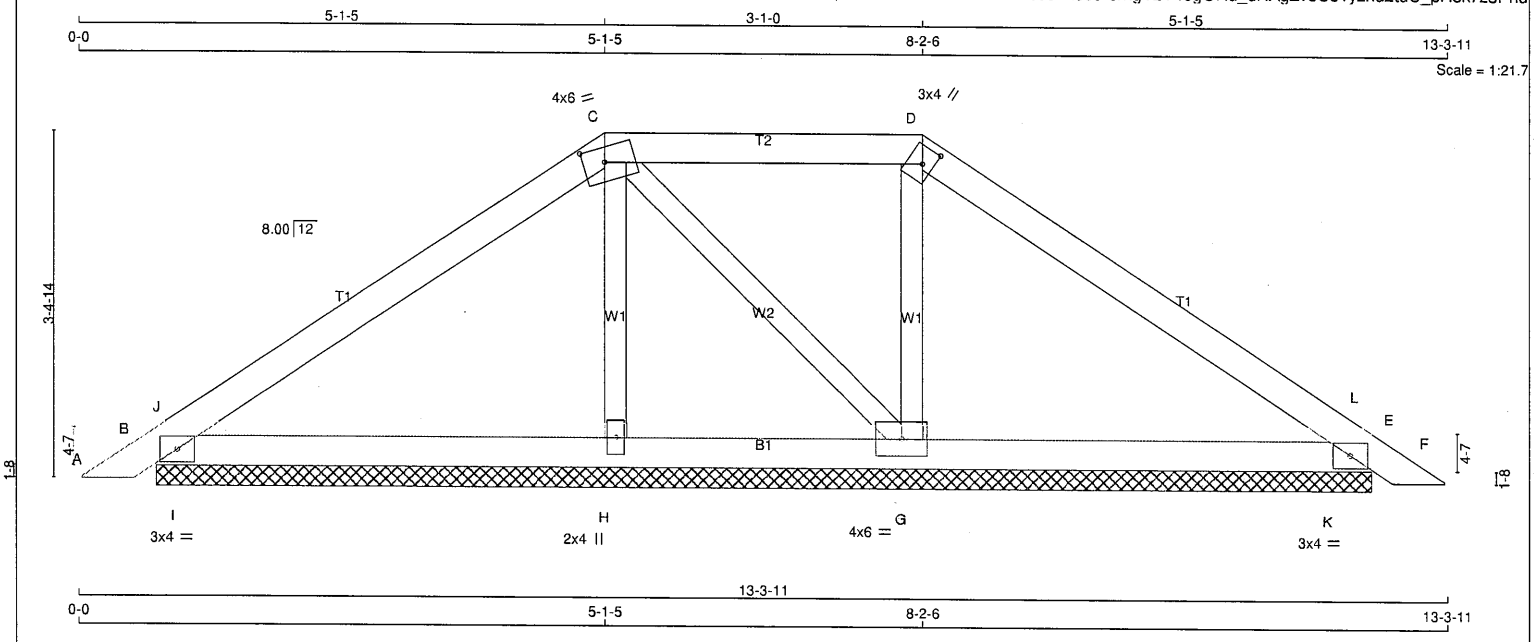

TOTAL WEIGHT = 38 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	384	0	384	0	0	11-9-13	1-8
E	361	0	361	0	0	11-9-13	1-8
H	290	0	290	0	0	11-9-13	1-8
G	385	0	385	0	0	11-9-13	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	269	190 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
E	254	177 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
H	207	124 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
G	273	178 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03	(1)	10.00	H- C	-181 / 0	0.03 (1)
B- J	-54 / 0	-91.8	-91.8	0.09	(1)	6.25	C- G	-51 / 0	0.01 (1)
J- C	-180 / 0	-91.8	-91.8	0.22	(1)	6.25	G- D	-240 / 0	0.05 (1)
C- D	-100 / 0	-91.8	-91.8	0.15	(1)	6.25	I- J	-401 / 0	0.00 (1)
D- L	-139 / 0	-91.8	-91.8	0.22	(1)	6.25	K- L	-405 / 0	0.00 (1)
L- E	-47 / 0	-91.8	-91.8	0.09	(1)	6.25			
E- F	0 / 15	-91.8	-91.8	0.03	(1)	10.00			
B- I	0 / 142	-18.5	-18.5	0.19	(1)	10.00			
I- H	0 / 142	-18.5	-18.5	0.19	(1)	10.00			
H- G	0 / 136	-18.5	-18.5	0.10	(1)	10.00			
G- K	0 / 108	-18.5	-18.5	0.18	(1)	10.00			
K- E	0 / 108	-18.5	-18.5	0.18	(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-J:1) , BC=0.19/1.00 (B-I:1) , WB=0.05/1.00 (D-G:1) , SSI=0.32/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307613

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	J1	9	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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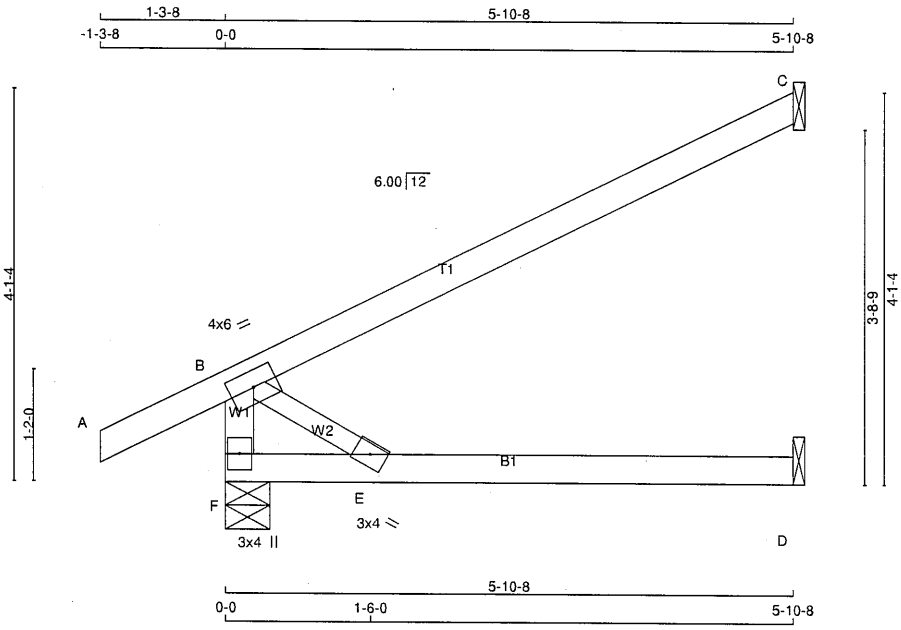

TOTAL WEIGHT = 9 X 18 = 163 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	448	0	448	0	0	5-8	1-8
C	270	0	270	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	315	220 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	186	150 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-394 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.54 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307576

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	J2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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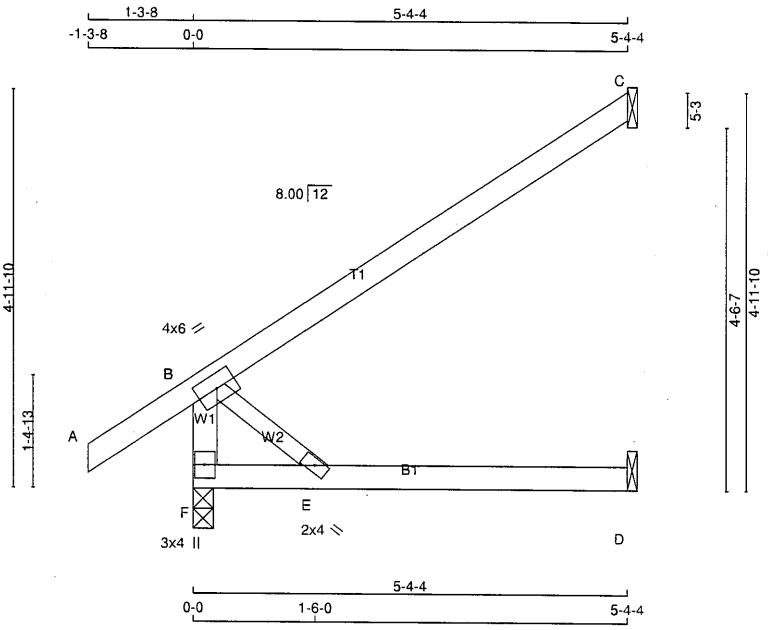

TOTAL WEIGHT = 2 X 18 = 36 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	421	0	421	0	0	3-0	1-8
C	246	0	246	0	0	1-8	1-8
D	50	0	55	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	295	207 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
C	169	137 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-372 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.45 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.16/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307577

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	J3	7	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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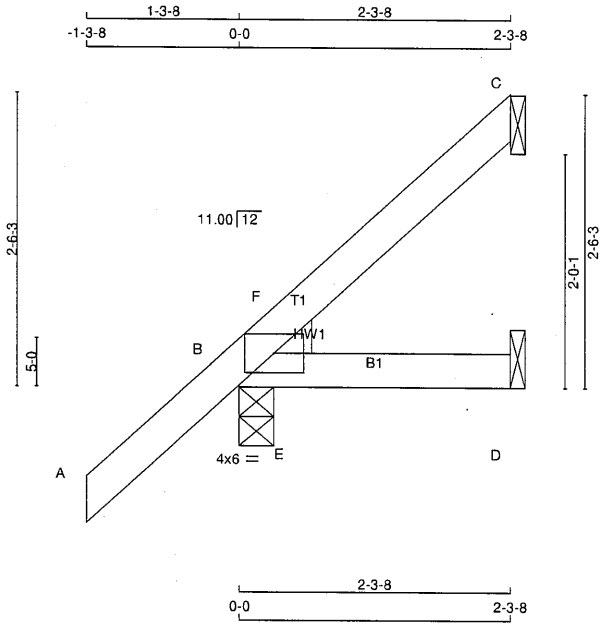

TOTAL WEIGHT = 7 X 9 = 62 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	87	0	87	0	0	1-8	1-8	
B	254	0	254	0	0	3-8	1-8	2x4 L
D	40	0	40	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	60	47 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
B	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
D	29	12 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 42	-91.8	-91.8	0.13 (5)	10.00	E- F	-92 / 0	0.00 (1)
B- F	-10 / 0	-91.8	-91.8	0.01 (4)	6.25			
F- C	0 / 2	-91.8	-91.8	0.06 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.06 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.06 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.06/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307578

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	J4	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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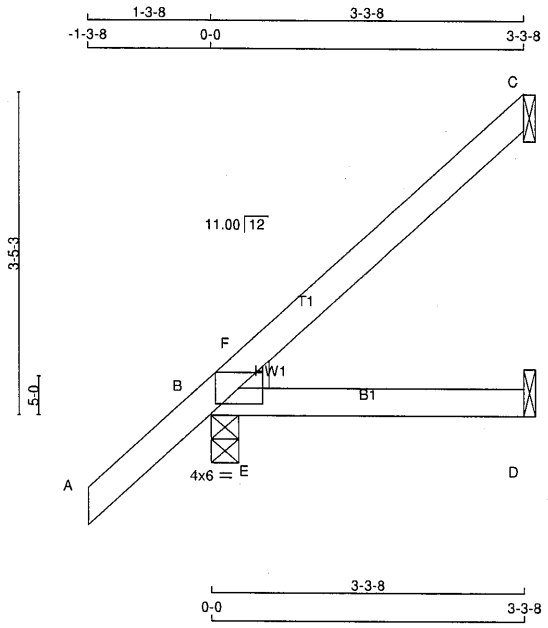

TOTAL WEIGHT = 3 X 12 = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	127	0	127	0	0	1-8	1-8	
B	309	0	309	0	0	3-8	1-8	2x4 L
D	54	0	54	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	88	69 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
B	216	155 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
D	40	16 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 42	-91.8	-91.8	0.14 (5)	10.00	E- F	-171 / 0	0.00 (1)
B- F	-15 / 12	-91.8	-91.8	0.02 (4)	6.25			
F- C	-2 / 2	-91.8	-91.8	0.12 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.11 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.11/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.14/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307579

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	J11	7	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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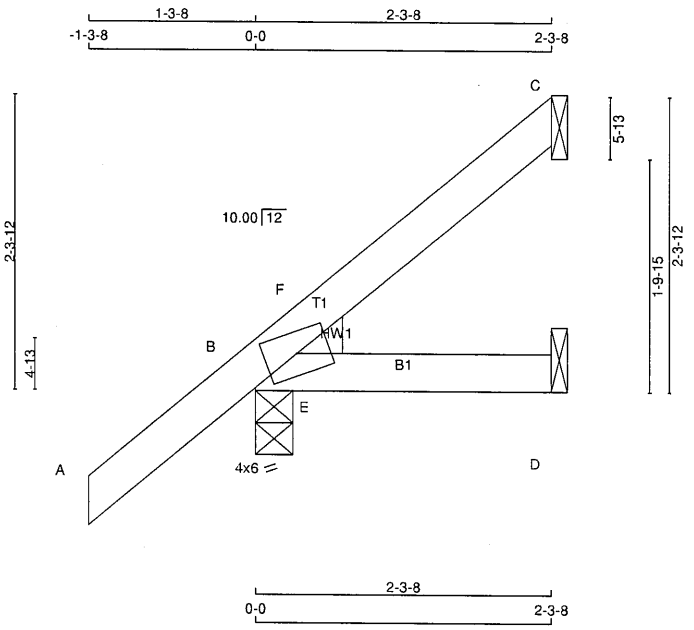

TOTAL WEIGHT = 7 X 9 = 60 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	86	0	86	0	0	1-8	1-8	
B	253	0	253	0	0	3-8	1-8	2x4 L
D	40	0	40	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	60	46 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
B	177	130 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
D	29	12 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-91.8	-91.8	0.13 (5)	10.00	E- F	-85 / 0	0.00 (1)
B- F	-11 / 0	-91.8	-91.8	0.01 (4)	6.25			
F- C	0 / 3	-91.8	-91.8	0.06 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.06 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.06 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.06/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307609

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432499	J12	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:37:22 2023 Page 1
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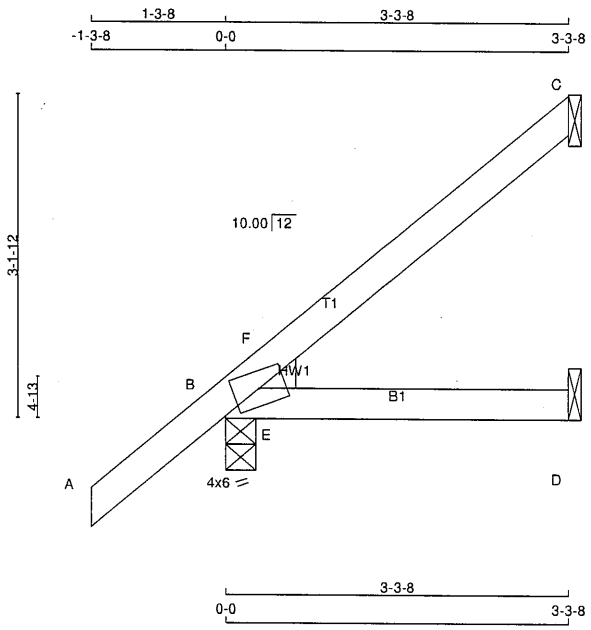

TOTAL WEIGHT = 3 X 11 = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-m	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	127	0	127	0	0	1-8	1-8	
B	309	0	309	0	0	3-8	1-8	2x4 L
D	54	0	54	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	88	68 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
B	216	155 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
D	40	16 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-91.8	-91.8	0.14 (5)	10.00	E- F	-156 / 0	0.00 (1)
B- F	-15 / 3	-91.8	-91.8	0.02 (4)	6.25			
F- C	0 / 3	-91.8	-91.8	0.12 (1)	10.00			
B- E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.11 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.11/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.13/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307610

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	J31	14	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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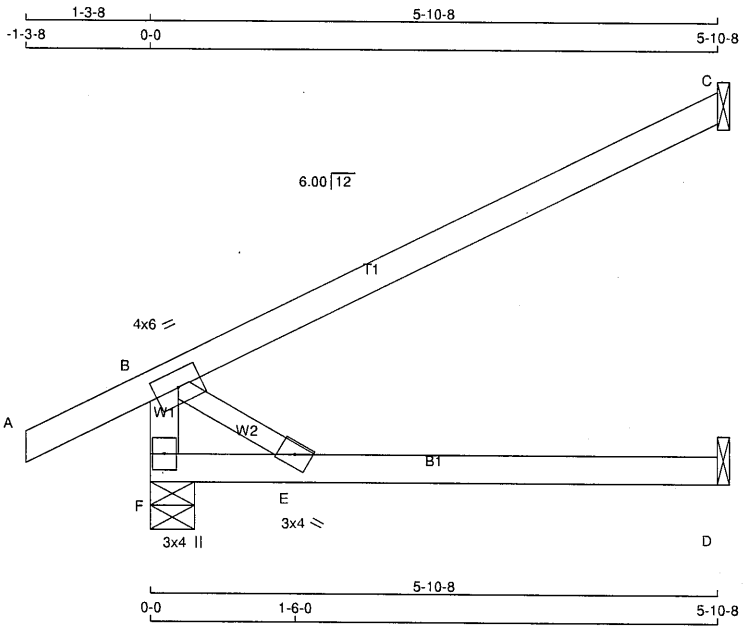

TOTAL WEIGHT = 14 X 18 = 253 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	448	0	448	0	0	5-8	1-8
C	270	0	270	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	315	220 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	186	150 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-394 / 0	0.0	0.0	0.04 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00			
B- C	0 / 0	-91.8	-91.8	0.54 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307635

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	C1	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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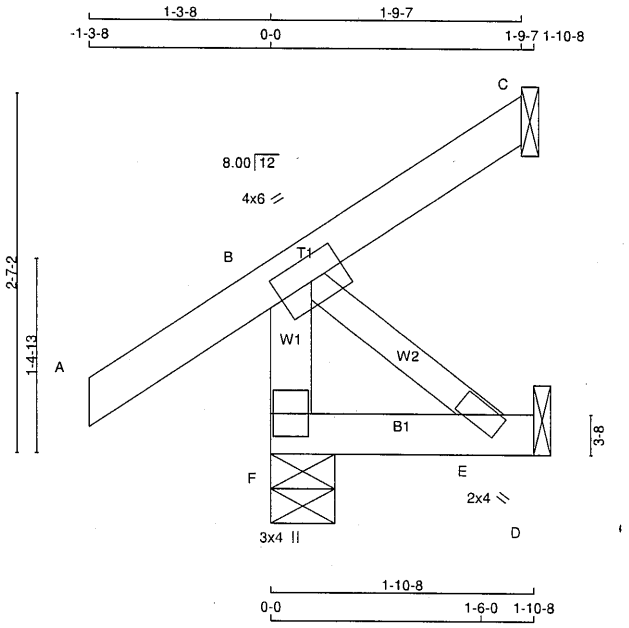

Scale: 3/4"=1'

TOTAL WEIGHT = 2 X 9 = 18 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	274	0	274	0	0	5-8	1-8
C	34	0	34	0	-39	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307572

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	C2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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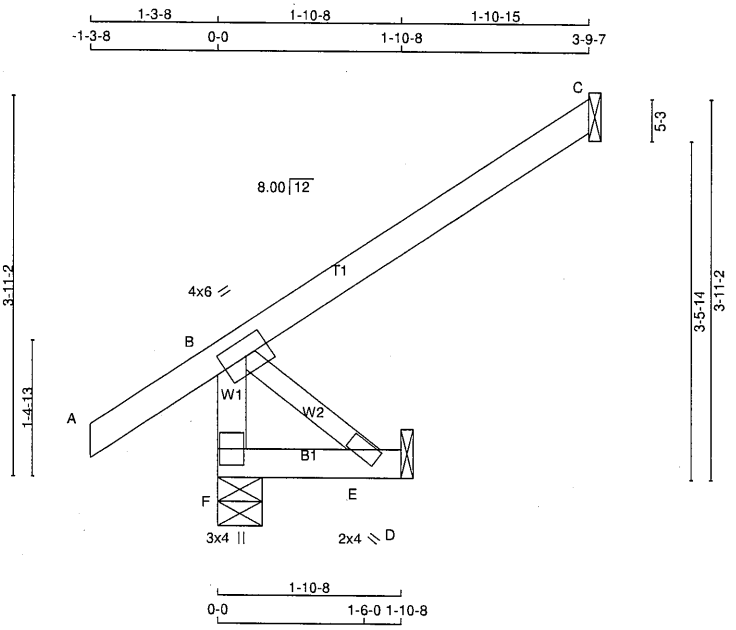

TOTAL WEIGHT = 2 X 12 = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	317	0	317	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	220	167 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-300 / 0	0.0	0.0	0.03	(1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.13	(5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22	(1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.02	(4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01	(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

Structural component only
DWG# T-2307573

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432495	C3	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:27:25 2023 Page 1
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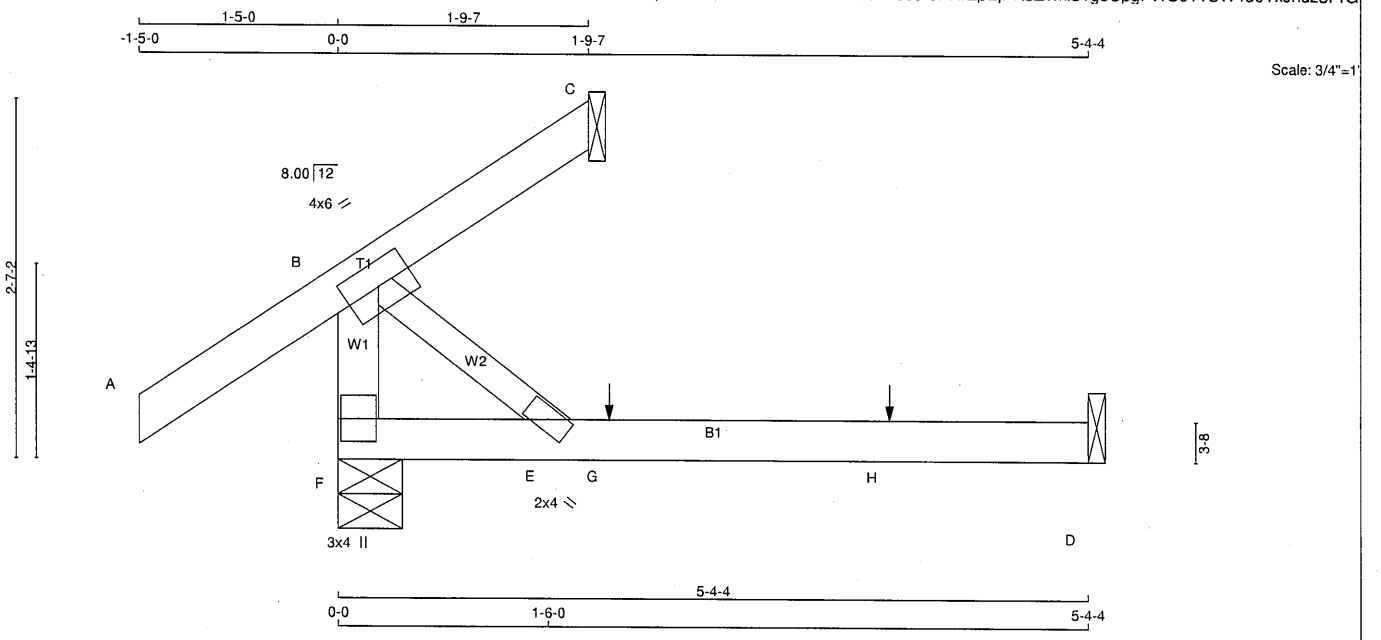

TOTAL WEIGHT = 2 X 13 = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	326	0	326	0	0	5-8	1-8
C	24	0	24	0	0	1-8	1-8
D	50	0	55	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	230	154 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
C	17	14 / 0	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-277 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 38	-91.8	-91.8	0.15 (1)	10.00			
B- C	-32 / 0	-91.8	-91.8	0.14 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.16/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307574

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432495	C4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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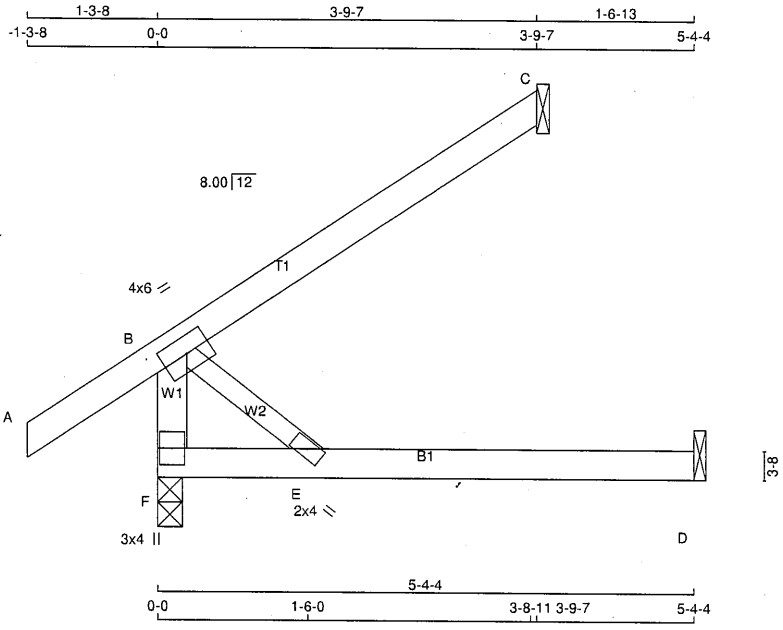

TOTAL WEIGHT = 2 X 16 = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	349	0	349	0	0	3-0	1-8
C	174	0	174	0	0	1-8	1-8
D	50	0	55	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	246	167 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-300 / 0	0.0	0.0	0.03	(1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12	(1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22	(1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.13	(4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.16	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.16/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307575

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	C31	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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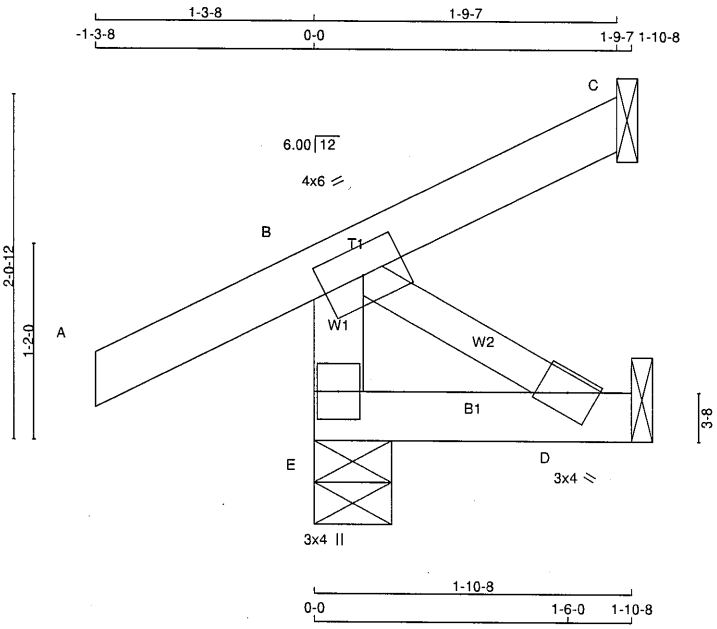

TOTAL WEIGHT = 7 X 8 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	268	0	268	0	0	5-8	1-8
C	35	0	35	0	-38	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	186	142 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
C	24	19 / -26	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-254 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-21 / 0	-91.8	-91.8	0.11 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307631

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432501	C32	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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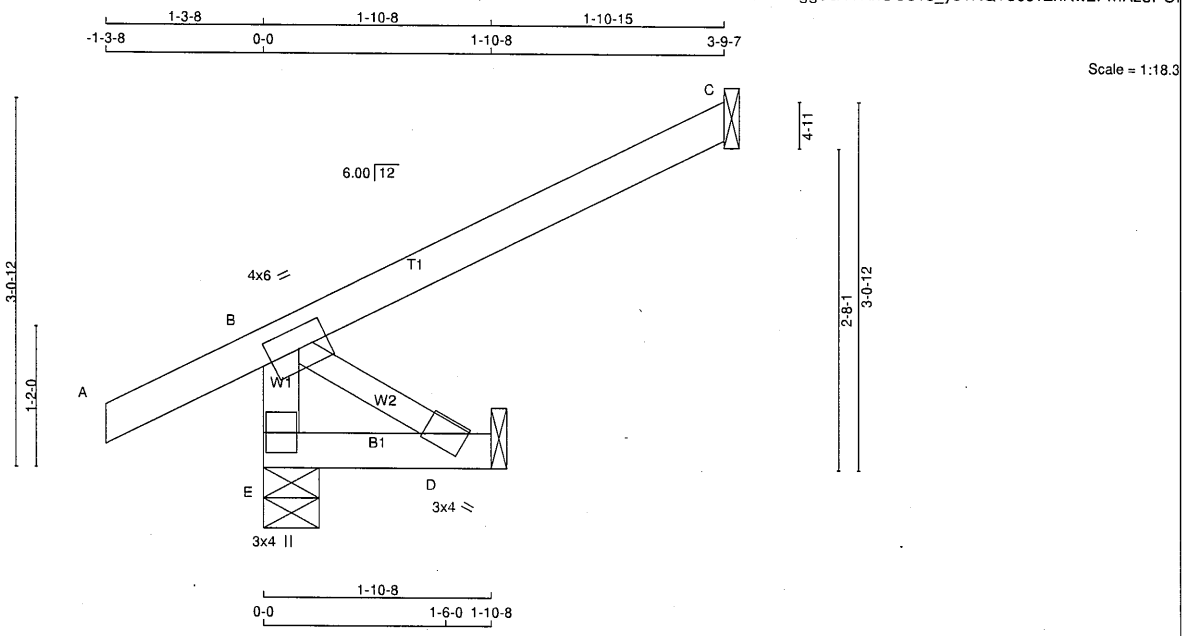

TOTAL WEIGHT = 7 X 11 = 76 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	312	0	312	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	216	166 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2307632

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	C33	7	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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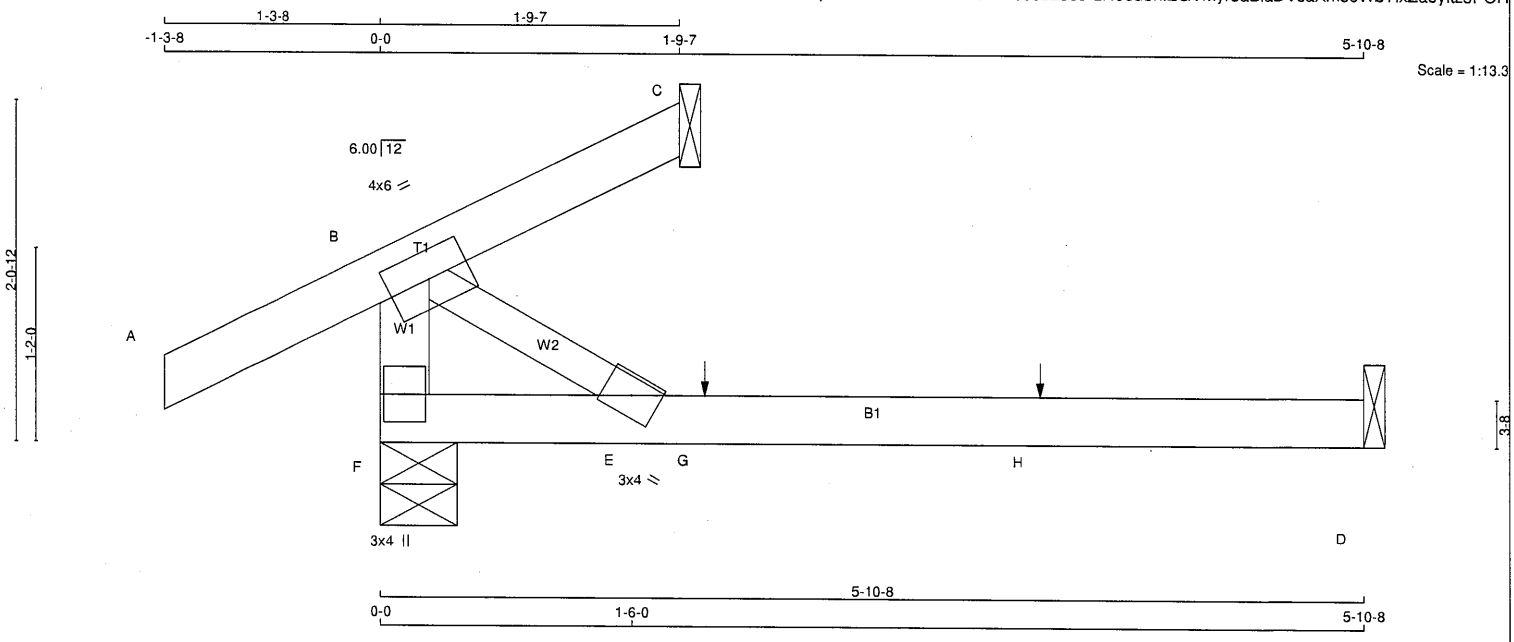


TOTAL WEIGHT = 7 X 13 = 90 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	308	0	308	0	0	5-8	1-8
C	35	0	35	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	218	142 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	24	19 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-254 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-21 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
06-08-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

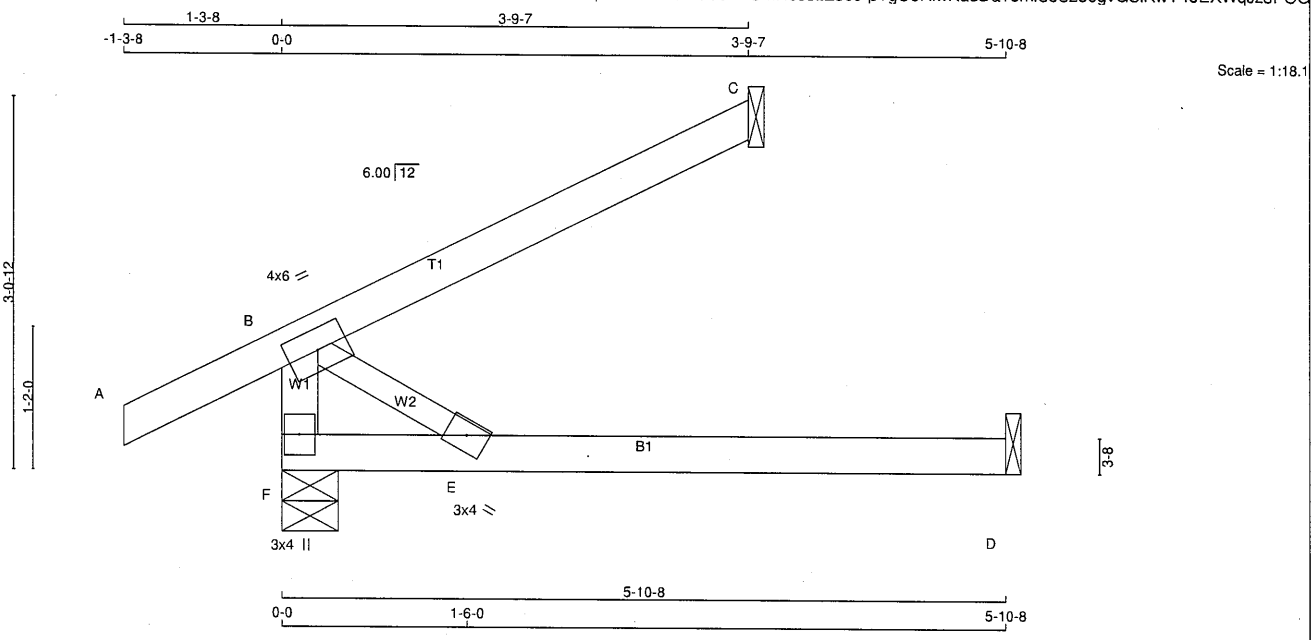
Structural component only
DWG# T-2307633

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432501	C34	7	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 8 08:58:21 2023 Page 1
ID:quZDzNLEe2lG33FPSMH8sozZ3c0-pTgU5RiwKadDa?emlS5S236gvGSIKwY4oEXWqJz8POG



TOTAL WEIGHT = 7 X 15 = 108 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	353	0	353	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	249	166 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

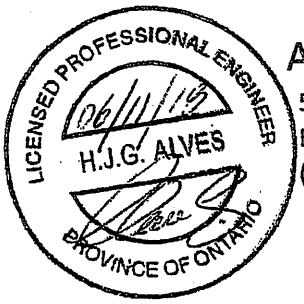
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

Structural component only
DWG# T-2307634



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

Nail Type	Length (in)	Dia. (in)	LATERAL RESISTANCE (lbs)		WITHDRAWAL RESISTANCE (lbs/in)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

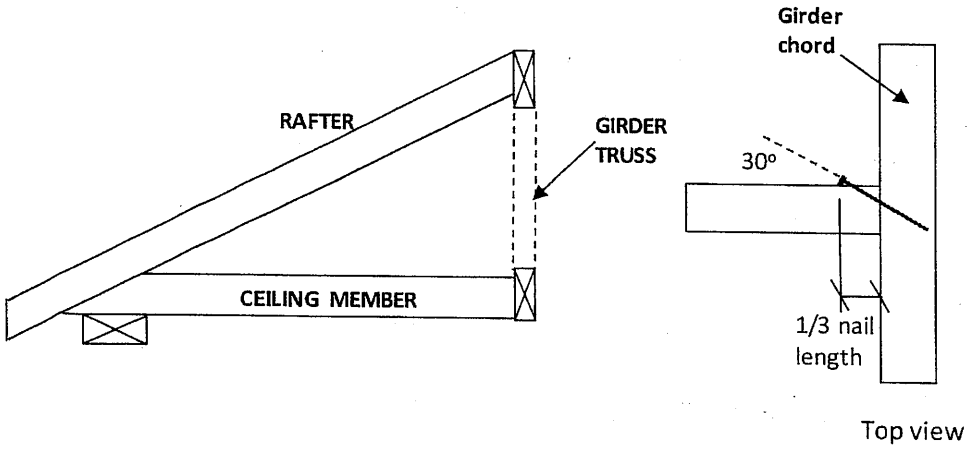


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

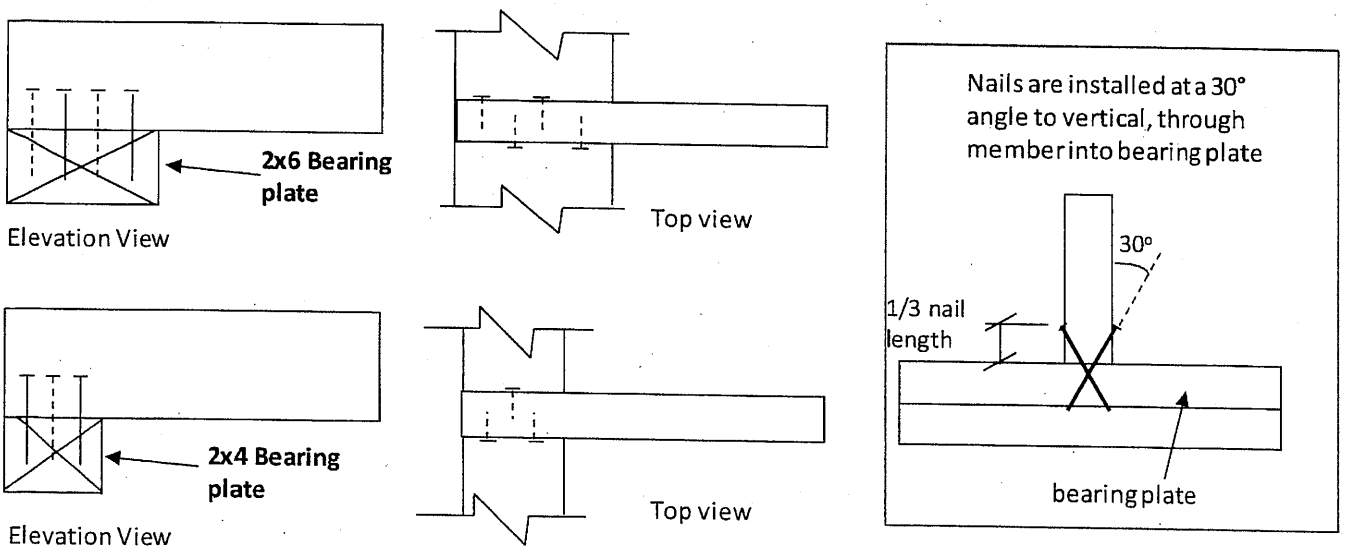
MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

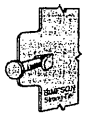
Installation:

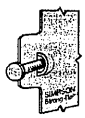
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

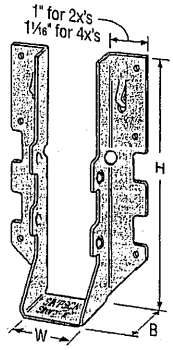
Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.

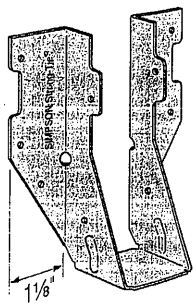

Double-Shear Nailing Top View


Double-Shear Nailing Side View; Do not bend tab

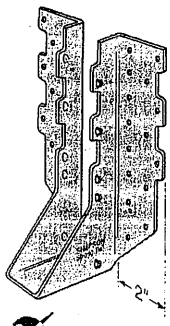

Dome Double-Shear Nailing Side View *(available on some models)*



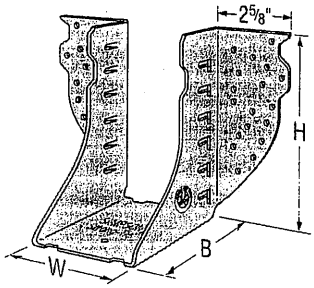
LUS28



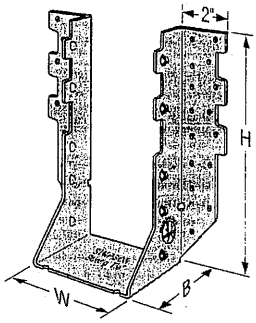
LU26L



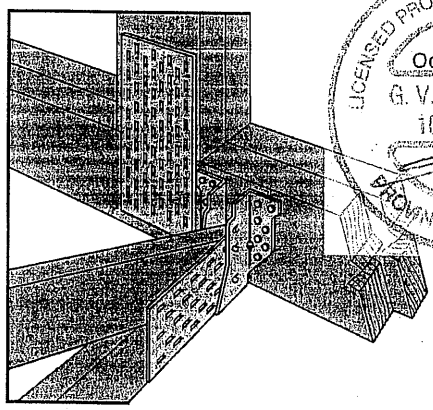
HUS210
(HUS26, HUS28, and HHUS similar)

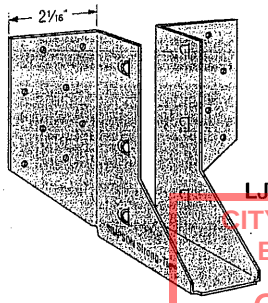


HGUS28-2



HHUS210-2


Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

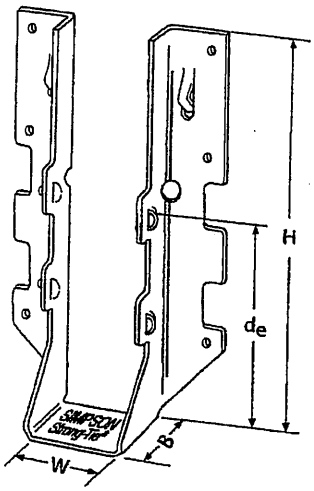
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

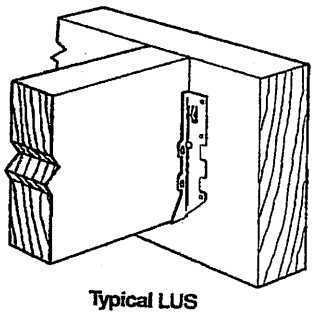
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



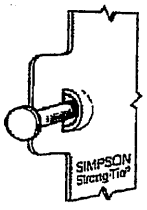
LUS28



Typical LUS Installation

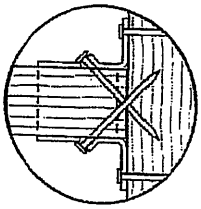
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

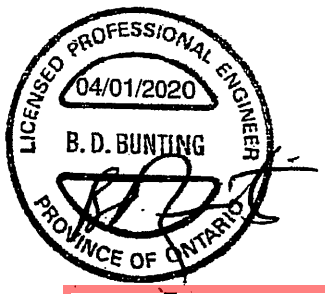


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

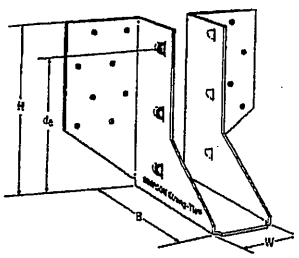
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

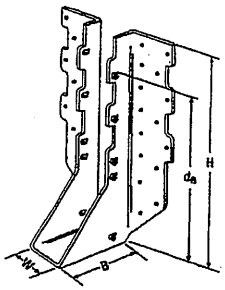
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

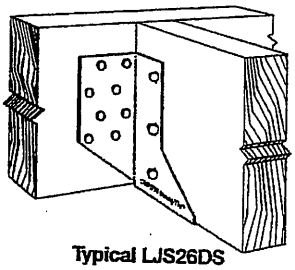
- See current catalogue for options



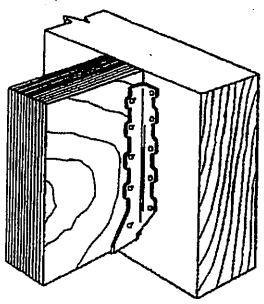
LJS26DS



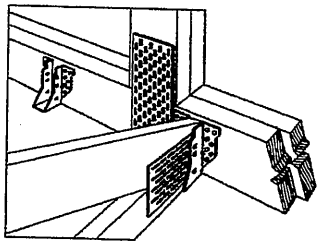
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



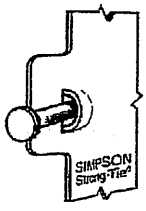
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

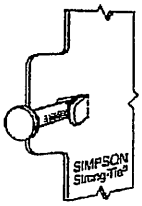
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

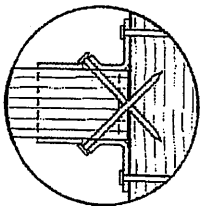


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

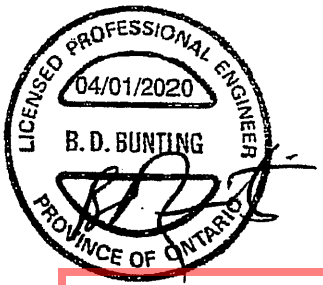
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

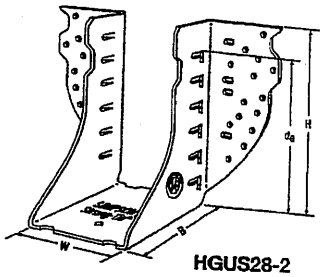
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

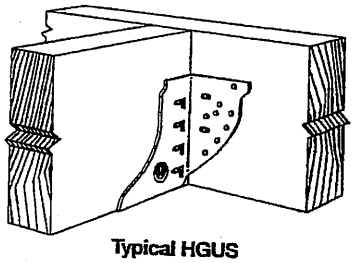
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

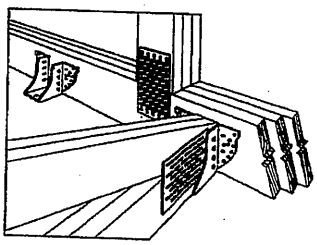

Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

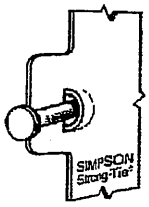
Options:

- See current catalogue for options


Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

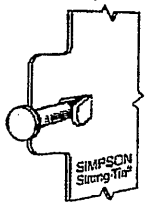
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⁵⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⁵⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

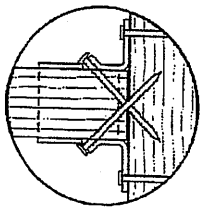


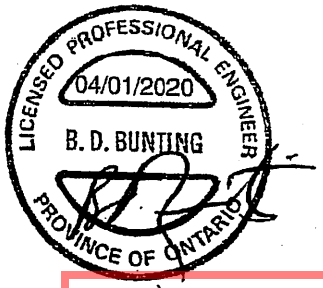
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

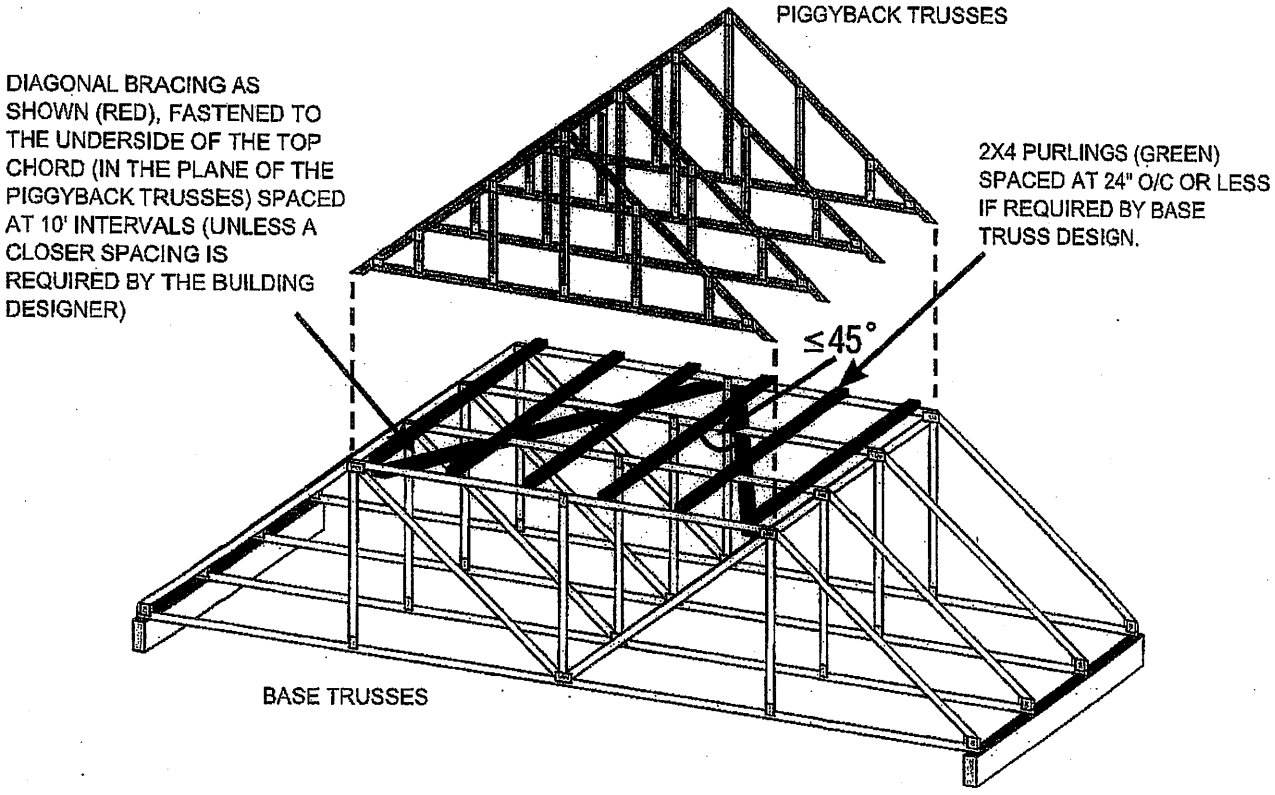
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

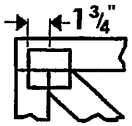


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

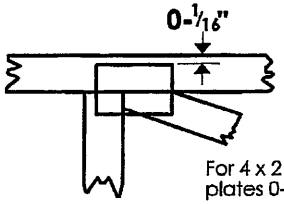
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

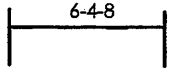
BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

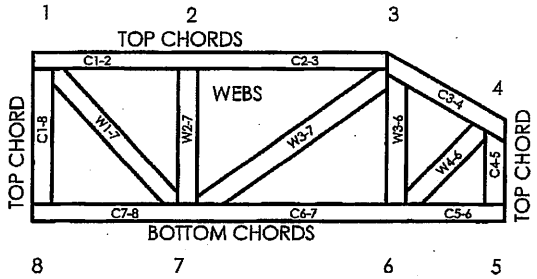
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.