

M9964



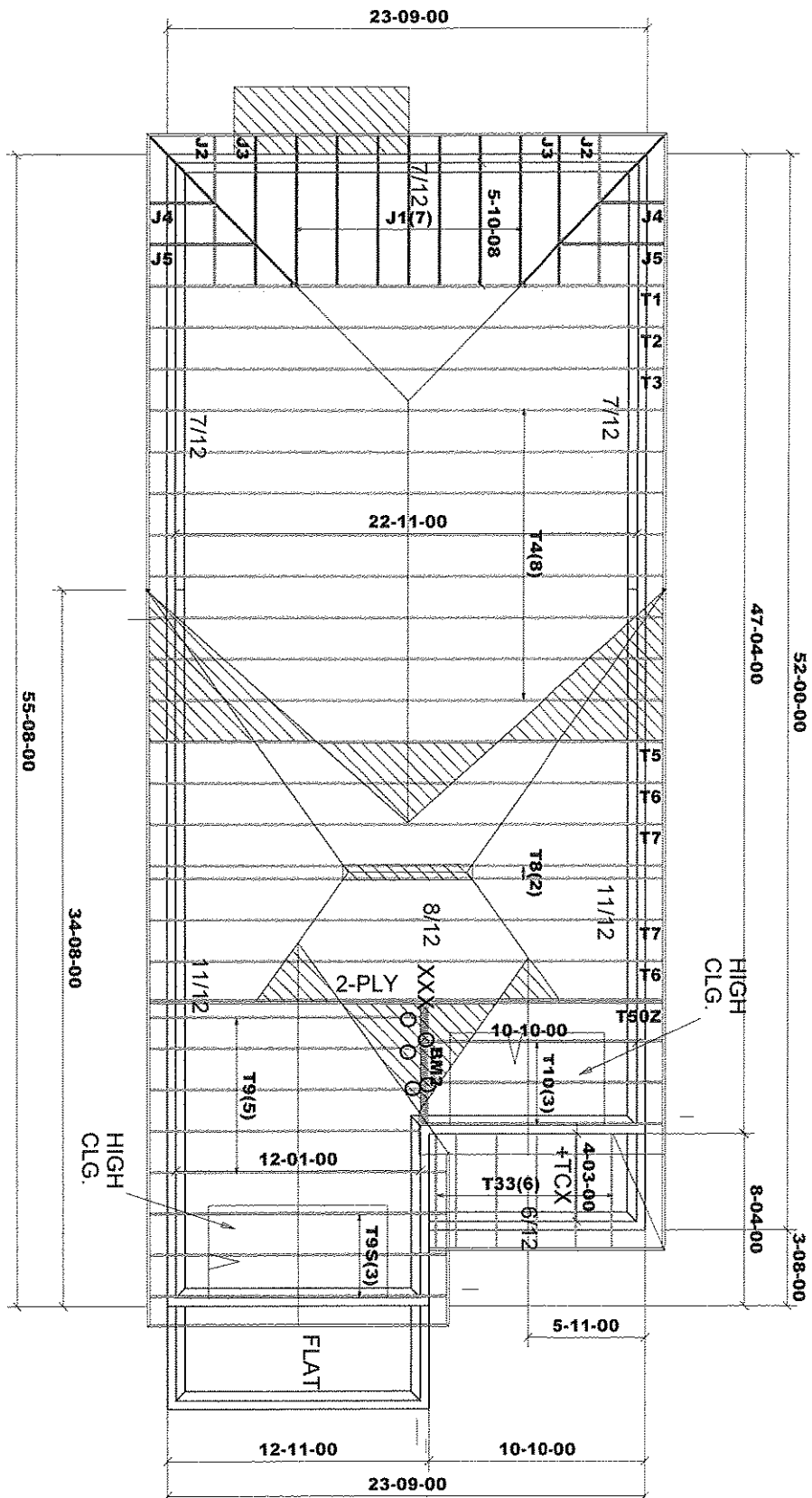
Job Track **42181**
Layout ID: **267349**
Plan Log: **86179**

Builder / Location:
GREENPARK HOMES / VAUGHAN
Project: **STARTIME HOME LTD**
Date: **8/9/2016** Designer: **JG**

Model / Elevation:
BRIDGEFORD 1 / 2

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mark ver 7.5.0



T-160338

- 12" FINISH O.H
- R.T.M.C
- 2X6 EXTERIOR WALLS
- ASPHALT SHINGLES
- 2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF @24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

- DESIGN LOADS:
- GROUND SNOW LOAD $S_s = 1.5 \text{ kPa}$
 - TC DEAD 3 PSF
 - BC LIVE 10.5 PSF
 - BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

- HARDWARE
- LUS24(O)
 - HGUS26-3(XXX)
 - BM2: 3-2X10

52-00-00

47-04-00

HIGH
CLG.

8-04-00

FASCIA
RAISED BY 2"

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC LATEST EDITION ROOF RATERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF @24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced So That The Distance Between End Points And Between Rows Of Bracing Does Not Exceed 6'.

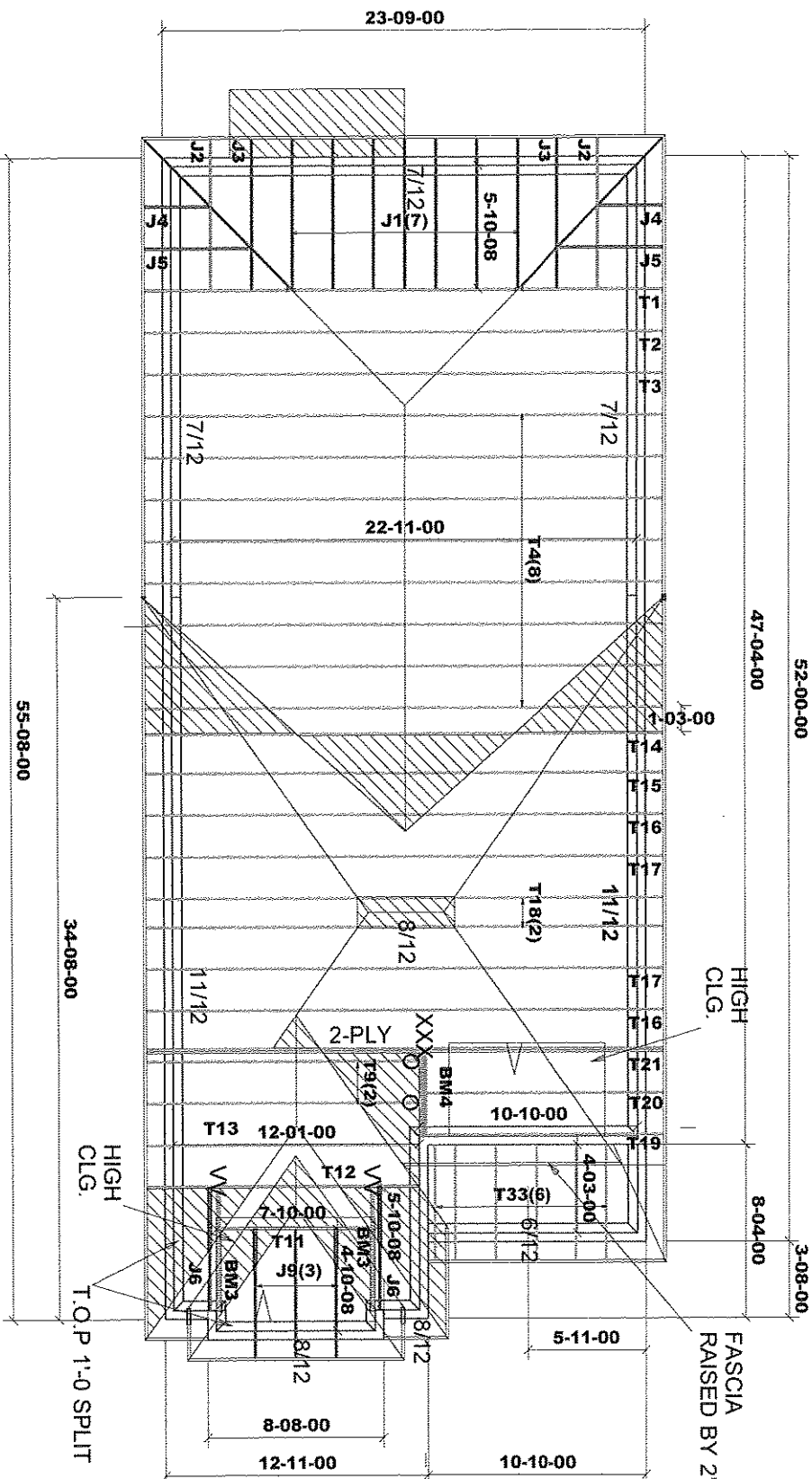
DESIGN CONFORMS WITH
THE RELEVANT SECTION
OF THE LATEST EDITION
OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
 CONVENTIONAL
 FRAMING

HARDWARE
LUS24(O)
LUS26-2(V)
HGUS26-3(XXX)

BM3 : 2-2X10
BM4: 3-2X10
BUILD UP 1' WALL



5095



Job Track: 42181
Layout ID: 267350
Plan Log: 86179

Builder / Location:		GREENPARK HOMES / VAUGHAN	
Project:		STARTIME HOME LTD	
Date:	8/10/2016	Designer:	JG
		THE OR R TAM PLICE	

Model / Elevation:

BRIDGEFORD 1 / 3

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMMACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMMACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMMACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE

Mitek ver 7.3.0

Milek ver 7.5.0



Delivery Shiplist

DATE	08/09/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267348 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGHAN SUB-BUILDER:
 MODEL: BRIDGEFORD 1 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	96.90 61.83		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	8	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	750.64 461.36		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1 2 Ply	T50 HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	246.18 153.66		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	202.54 131.00		
	2	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	229.72 147.34		
	2	T8 HIP	11.00 0.00	22-11-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	236.34 151.34		
	5	T9 COMMON	11.00 0.00	12-01-00	07-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	305.60 197.50		
	3	T9S ROOF	11.00 0.00	12-01-00	07-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	189.24 127.98		
	6	T33 MONOPITCH	6.00 0.00	04-03-00	02-10-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-05-11	102.00 67.98		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	5	J6 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	112.20 68.35		



Delivery Shiplist

DATE	08/09/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267348 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 1 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	J7 JACK-OPEN	6.00 0.00	05-04-08	03-00-07	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 00-04-11	88.74 55.98		

TOTAL # TRUSS= 60.00

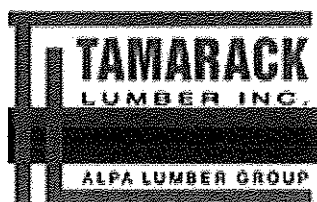
TOTAL BFT OF ALL TRUSSES=

1942.51 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3066.42 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LUS24	

TOTAL # ITEMS= 4.00



Delivery Shiplist

DATE	08/09/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267349 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 1 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	96.90 61.83		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	8	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	750.64 461.36		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1 2 Ply	T50Z HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	238.12 148.00		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	202.54 131.00		
	2	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	229.72 147.34		
	2	T8 HIP	11.00 0.00	22-11-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	236.34 151.34		
	5	T9 COMMON	11.00 0.00	12-01-00	07-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	305.60 197.50		
	3	T9S ROOF	11.00 0.00	12-01-00	07-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	189.24 127.98		
	3	T10 ROOF	11.00 0.00	10-10-00	06-09-09	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-10-14	175.65 120.51		
	6	T33 MONOPITCH	6.00 0.00	04-03-00	02-10-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-05-11	102.00 67.98		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		

TOTAL # TRUSS= 52.00

TOTAL BFT OF ALL TRUSSES=

1933.03 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3033.07 LBS.



Delivery Shiplist

DATE	08/09/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267349 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 1 ELEVATION: 2

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
5	Hangers	LUS24	

TOTAL # ITEMS= 6.00



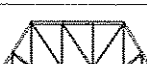
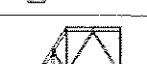
Delivery Shiplist

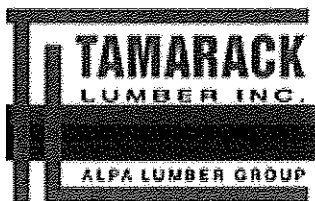
DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267350 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORT 1 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	105.17 64.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	8	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	750.64 466.64		
	2	T9 COMMON	11.00 0.00	12-01-00	07-03-08	2 X 4	2 X 4	00-00-00 01-03-08	00-00-00 00-00-00	118.24 76.34		
	1	T11 HIP GIRDER	11.00 0.00	07-10-00	04-07-13	2 X 4	2 X 4	00-00-00 00-00-00	01-09-01 01-09-01	39.29 27.50		
	1	T12 HIP GIRDER	11.00 0.00	12-01-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	57.32 37.83		
	1	T13 HIP	11.00 0.00	12-01-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	62.95 42.50		
	1	T14 HIP	11.00 0.00	22-11-00	04-09-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	98.11 63.17		
	1	T15 HIP	11.00 0.00	22-11-00	06-00-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	98.42 62.33		
	2	T16 HIP	11.00 0.00	22-11-00	07-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	222.64 139.00		
	2	T17 HIP	11.00 0.00	22-11-00	08-08-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	226.10 141.34		
	2	T18 HIP	11.00 0.00	22-11-00	10-00-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	244.02 153.34		
	1	T19 HALF HIP	11.00 0.00	10-10-00	03-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-09-01 02-04-13	47.27 32.00		
	1	T20 HALF HIP	11.00 0.00	10-10-00	04-08-13	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	51.06 34.83		
	1 2 Ply	T21 HIP GIRDER	11.00 0.00	22-11-00	06-00-13	2 X 4	2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	263.72 170.00		
	6	T33 MONOPITCH	6.00 0.00	04-03-00	02-10-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-05-11	102.00 67.98		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		



Delivery Shiplist

DATE	08/10/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267350 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORT 1 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	2	J6 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	44.16 27.34		
	3	J9 JACK-OPEN	8.00 0.00	04-10-08	04-07-13	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	57.39 35.49		

TOTAL # TRUSS= 54

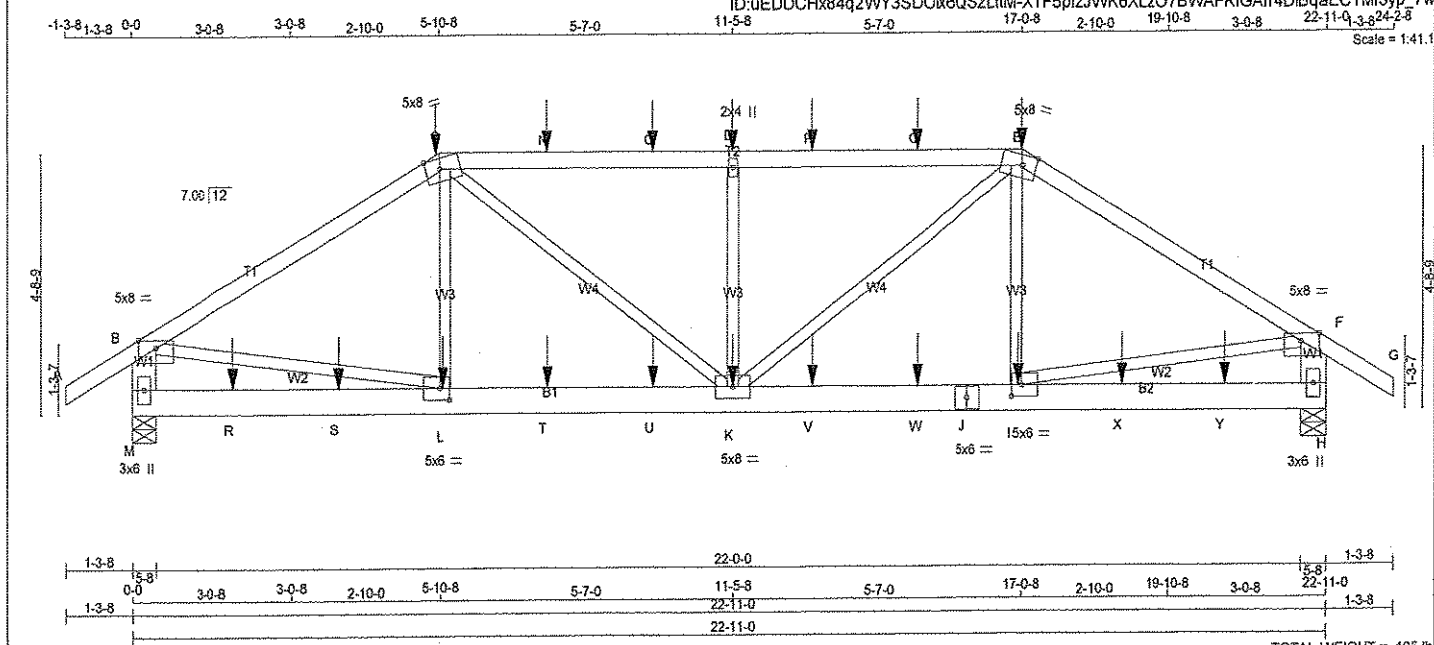
TOTAL BFT OF ALL TRUSSES=

1895.99 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2993.66 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
2	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= 5



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.25
D	TMVW-r	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.25
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMVW-l	MT20	5.0	6.0	2.50	2.25
J	BS-l	MT20	5.0	6.0		
K	BMVW-w	MT20	5.0	6.0		
L	BMVW-l	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 429.0 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, 101.4 lbs FACTORED DOWN AT 11-5-8, 101.4 lbs FACTORED DOWN AT 12-11-12, AND 101.4 lbs FACTORED DOWN AT 14-11-12, AND 429.0 lbs FACTORED DOWN AT 17-0-8 ON TOP CHORD, AND 75.4 lbs FACTORED DOWN AT 1-11-4, 70.9 lbs FACTORED DOWN AT 3-11-4, 70.9 lbs FACTORED DOWN AT 5-11-4, 70.9 lbs FACTORED DOWN AT 7-11-4, 70.9 lbs FACTORED DOWN AT 9-11-4, 70.9 lbs FACTORED DOWN AT 11-5-8, 70.9 lbs FACTORED DOWN AT 12-11-12, 70.9 lbs FACTORED DOWN AT 14-11-12, 70.9 lbs FACTORED DOWN AT 16-11-12, AND 70.9 lbs FACTORED DOWN AT 18-11-12, AND 75.4 lbs FACTORED DOWN AT 20-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	2307	0	2307	0	0	5-8	2-8
H	2307	0	2307	0	0	5-8	2-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1887	1059 / 0	422 / 0	0 / 0	0 / 0	408 / 0	0 / 0
H	1887	1059 / 0	422 / 0	0 / 0	0 / 0	408 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH
A-B	0 / 29	-84.3	-84.3	0.08 (1)	10.00	B-L	0 / 2535	0.63 (1)	
B-C	-2906 / 0	-84.3	-84.3	0.50 (1)	4.43	L-C	-55 / 366	0.09 (3)	
C-N	-3278 / 0	-84.3	-84.3	0.61 (1)	3.97	I-E	-55 / 366	0.09 (3)	
N-O	-3278 / 0	-84.3	-84.3	0.61 (1)	3.97	I-F	0 / 2535	0.63 (1)	
C-D	-3278 / 0	-84.3	-84.3	0.61 (1)	3.97	K-E	0 / 977	0.24 (1)	
D-P	-3278 / 0	-84.3	-84.3	0.61 (1)	3.97	C-K	0 / 977	0.24 (1)	
P-Q	-3278 / 0	-84.3	-84.3	0.61 (1)	3.97	K-D	-953 / 0	0.30 (1)	
Q-E	-3278 / 0	-84.3	-84.3	0.61 (1)	3.97				
E-F	-2906 / 0	-84.3	-84.3	0.50 (1)	4.43				
F-G	0 / 29	-84.3	-84.3	0.08 (1)	10.00				
M-B	-2203 / 0	0.0	0.0	0.16 (1)	6.84				
H-F	-2203 / 0	0.0	0.0	0.16 (1)	6.84				
M-R	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
R-S	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
S-L	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
L-T	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
T-U	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
U-K	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
K-V	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
V-W	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
W-J	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
J-I	0 / 2506	-28.0	-28.0	0.43 (2)	10.00				
I-X	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
X-Y	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
Y-H	0 / 0	-28.0	-28.0	0.20 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	5-10-8	-429	-429	-	FRONT	VERT	TOTAL
D	11-5-8	-101	-101	-	FRONT	VERT	TOTAL
E	17-0-8	-429	-429	-	FRONT	VERT	TOTAL
I	16-11-12	-41	-71	-	FRONT	VERT	TOTAL
K	11-5-8	-41	-71	-	FRONT	VERT	TOTAL
L	5-11-4	-41	-71	-	FRONT	VERT	TOTAL
N	7-11-4	-101	-101	-	FRONT	VERT	TOTAL
O	9-11-4	-101	-101	-	FRONT	VERT	TOTAL
P	12-11-12	-101	-101	-	FRONT	VERT	TOTAL
Q	14-11-12	-101	-101	-	FRONT	VERT	TOTAL
R	1-11-4	-40	-75	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.0X12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.61 (C-D:1), BC=0.43 (K-L:2), WB=0.63 (B-L:1), SSI=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687

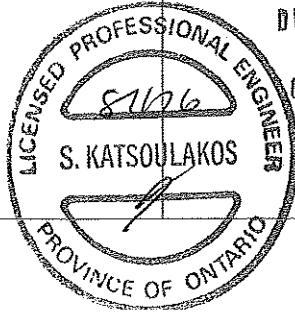
MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (I) (INPUT = 0.90)
JSI METAL=0.63 (L) (INPUT = 1.00)



DRWG NO. TAM36766-16
STRUCTURAL
COMPONENT ONLY
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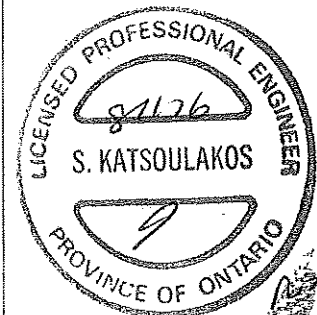
JOB NAME 267348	TRUSS NAME T1	QUANTITY 1	PLY 1	JOB DESC. 42181 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: uEDDCHx84q2WY3SDOx6QSzLtiM-XTF5plzJWVK6XLzO7BWAFKIGAr4DiBqaECTMf3yp_7w

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC	LC1	MAX-	MAX+	FACE	DIR.
S	3-11-4	-41	-71	---	FRONT	VERT
T	7-11-4	-41	-71	---	FRONT	VERT
U	9-11-4	-41	-71	---	FRONT	VERT
V	12-11-12	-41	-71	---	FRONT	VERT
W	14-11-12	-41	-71	---	FRONT	VERT
X	18-11-12	-41	-71	---	FRONT	VERT
Y	20-11-12	-40	-75	---	FRONT	VERT
						TYPE
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL
						TOTAL



DWG NO. TAM36766-16
STRUCTURAL
COMPONENT ONLY

	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1857 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (J) (INPUT = 0.88)
 JSI METAL = 0.55 (L) (INPUT = 1.00)

DWG NO. TAM36767-16

STRUCTURAL
 COMPONENT ONLY



DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O. I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.76")
CALCULATED VERT. DEFL(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL)= $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.29 (B-C:1), BC=0.33 (M-N:2), WB=0.31 (B-N:1), SS=0.18 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

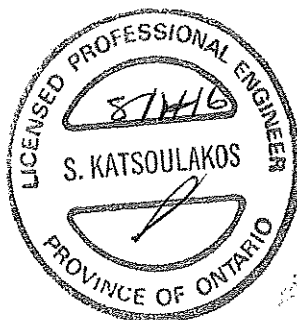
NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

DWG NO. FAM 3676B-16
STRUCTURAL
COMPONENT ONLY



Tamarack Roof Truss, Burlington

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ID: uEDDCxH84q2WY3SD0x6QSZLttM-7ipT1e_xHeEOz6zKkEhUsVoN4FOuRfwkTsDvBWyp_7v

11-5-8 5-7-8 17-1-0 5-10-0 22-11-0.3.2-8

Scale = 1:47.4

1-3-8 3-3 0-0 5-10-0 5-10-0 5-7-8 4x6 ||

7-11-10

1-3-7

3x4 ||

7x8

5x6 ||

7x8

5x6 ||

3x4 ||

1-3-7

4x6 ||

4x4 ||

3x6 ||

4x4 ||

4x6 ||

1-3-5 5-6 22-0-0 1-3-8

0-0 7-8-8 7-8-8 7-6-0 15-2-8 7-8-8 1-3-8

1-3-3 22-11-0 22-11-0 22-11-0 1-3-8

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/29	-84.3	-84.3 0.11 (1)	10.00	D-I	0/582	0.13 (1)		
B-C	0/29	-84.3	-84.3 0.45 (1)	10.00	I-E	-319/50	0.12 (1)		
C-D	-1433/0	-84.3	-84.3 0.39 (1)	5.02	K-D	0/582	0.13 (1)		
D-E	-1433/0	-84.3	-84.3 0.39 (1)	5.02	C-K	-319/50	0.12 (1)		
E-F	0/29	-84.3	-84.3 0.45 (1)	10.00	L-C	-1685/0	0.57 (1)		
F-G	0/29	-84.3	-84.3 0.11 (1)	10.00	E-H	-1685/0	0.57 (1)		
L-B	-299/0	0.0	0.0 0.03 (1)	7.81					
H-F	-299/0	0.0	0.0 0.03 (1)	7.81					
L-K	0/1347	-28.0	-28.0 0.53 (2)	10.00					
K-J	0/957	-28.0	-28.0 0.49 (2)	10.00					
J-I	0/957	-28.0	-28.0 0.49 (2)	10.00					
I-H	0/1347	-28.0	-28.0 0.53 (2)	10.00					

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)



DWG NO. FAM 36769-16
STRUCTURAL
COMPONENT ONLY

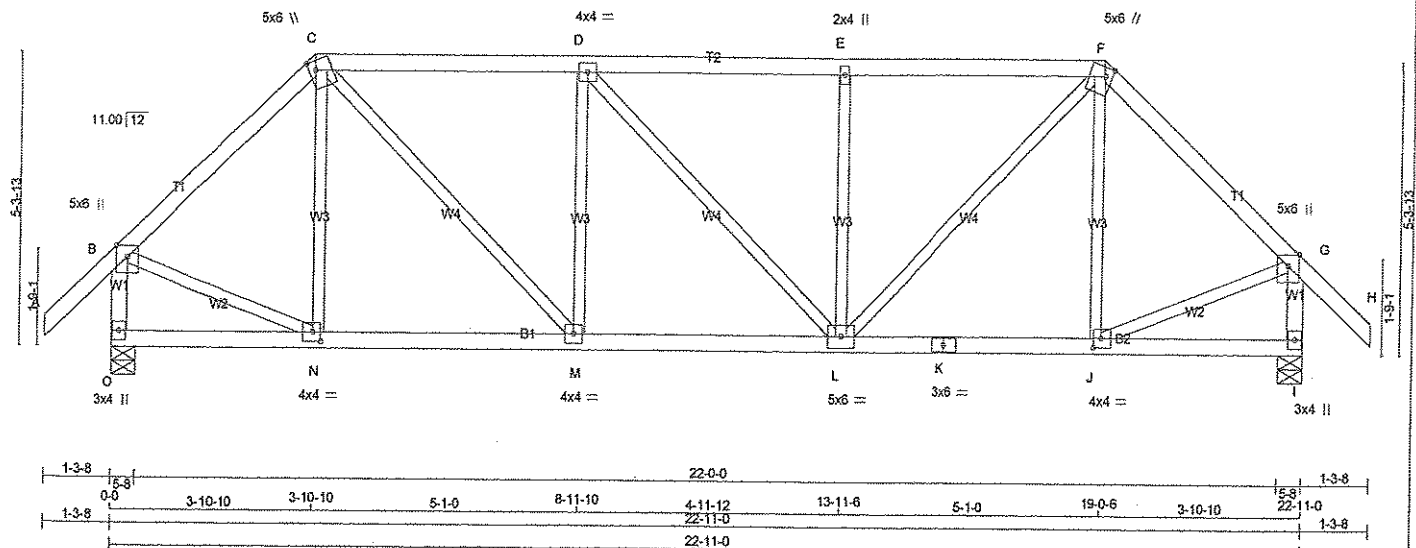
JOB NAME 267348	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. 42181	DRWG NO.
Tamarack Roof Truss, Burlington					

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Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Aug 11 09:09:24 2016 Page 1

-1-3-8 1-3-8 0-0 3-10-10 3-10-10 5-1-0 8-11-10 4-11-12 13-11-6 5-1-0 19-0-6 3-10-10 22-11-0 3-8 24-2-8

Scale = 1:41.1



TOTAL WEIGHT = 101 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1404	0	1404	0
I	1404	0	1404	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	N-C	-159 / 89	0.07 (1)
B-C	-1152 / 0	-84.3	-84.3 0.25 (1)	5.63	C-M	0 / 849	0.19 (1)
C-D	-1448 / 0	-84.3	-84.3 0.33 (1)	5.05	M-D	-456 / 0	0.19 (1)
D-E	-1447 / 0	-84.3	-84.3 0.33 (1)	5.05	D-L	-2 / 0	0.00 (1)
E-F	-1447 / 0	-84.3	-84.3 0.33 (1)	5.05	L-E	-456 / 0	0.19 (1)
F-G	-1152 / 0	-84.3	-84.3 0.25 (1)	5.63	L-F	0 / 849	0.19 (1)
G-H	0 / 40	-84.3	-84.3 0.12 (1)	10.00	J-F	-158 / 89	0.07 (1)
O-B	-1362 / 0	0.0	0.0 0.15 (1)	6.96	B-N	0 / 898	0.20 (1)
I-G	-1362 / 0	0.0	0.0 0.15 (1)	6.96	J-G	0 / 898	0.20 (1)
O-N	0 / 0	-28.0	-28.0 0.13 (2)	10.00			
N-M	0 / 843	-28.0	-28.0 0.23 (2)	10.00			
M-L	0 / 1448	-28.0	-28.0 0.30 (1)	10.00			
L-K	0 / 843	-28.0	-28.0 0.23 (2)	10.00			
K-J	0 / 843	-28.0	-28.0 0.23 (2)	10.00			
J-I	0 / 0	-28.0	-28.0 0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.09")

CSI: TC=0.33 (C-D:1), BC=0.30 (L-M:1), WB=0.20 (G-J:1), SS=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

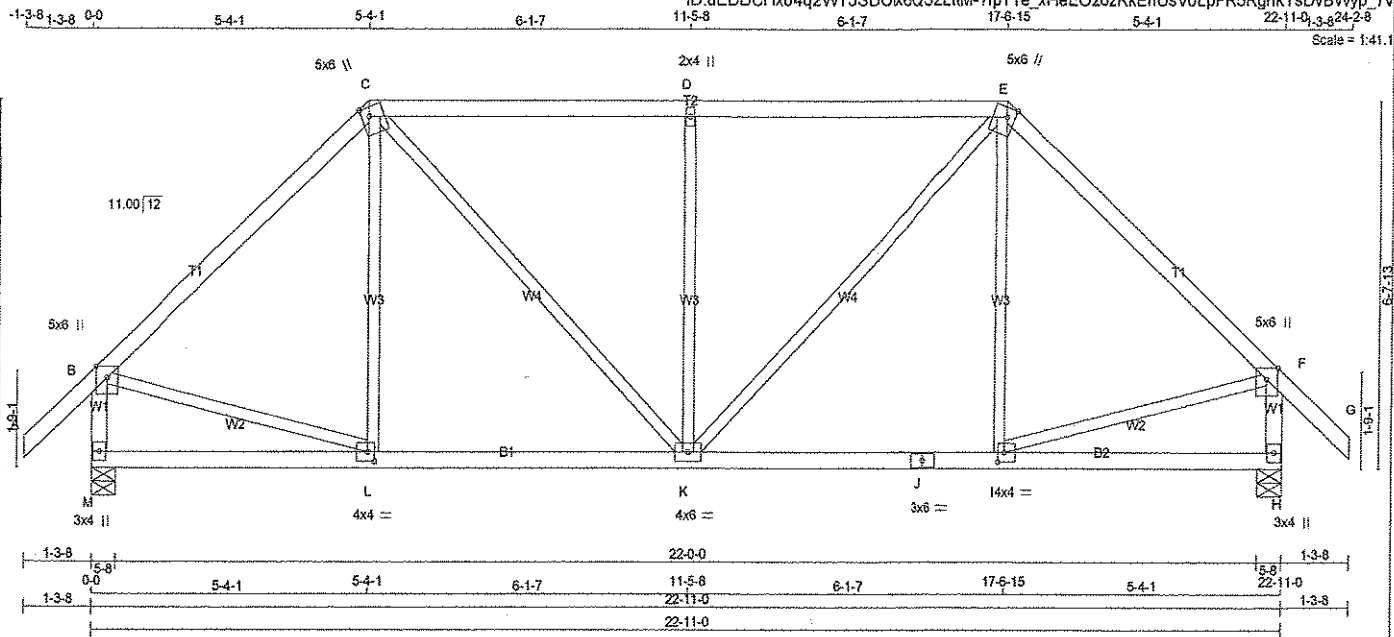


DWG NO. TAM36770-10
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T6	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	CHORDS - SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 Edge
C	TTVW+m	MT20	5.0	6.0 2.00 1.75
D	TMVW+w	MT20	2.0	4.0
E	TTVW+m	MT20	5.0	6.0 2.00 1.75
F	TMVW+p	MT20	5.0	6.0 Edge
H	BMV1+p	MT20	3.0	4.0
I	BMVW+t	MT20	4.0	4.0 2.00 1.50
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	4.0 2.00 1.50
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1404	0	1404	0	0	5-8	1-8	1-8	1-8
H	1404	0	1404	0	0	5-8	1-8	1-8	1-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
M	1136	658 / 0	241 / 0	0 / 0	0 / 0
H	1136	658 / 0	241 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MEMB.		MAX. FACTORED		FACTORED		VERT. LOAD LC1		MAX.		MAX.		UNBRAC		MEMB.		MAX. FACTORED	
FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)	FR-TO	FORCE (LBS)
A-B	0 / 40	A-B	-84.3	-84.3	0.12 (1)	10.00	L-C	-39 / 200	0.05 (3)										
B-C	-1154 / 0	B-C	-84.3	-84.3	0.49 (1)	5.28	C-K	0 / 550	0.12 (1)										
C-D	-1230 / 0	C-D	-84.3	-84.3	0.59 (1)	4.92	K-D	-630 / 0	0.46 (1)										
D-E	-1230 / 0	D-E	-84.3	-84.3	0.59 (1)	4.92	K-E	0 / 550	0.12 (1)										
E-F	-1154 / 0	E-F	-84.3	-84.3	0.49 (1)	5.28	J-E	-39 / 200	0.05 (3)										
F-G	0 / 40	F-G	-84.3	-84.3	0.12 (1)	10.00	B-L	0 / 677	0.20 (1)										
M-B	-1345 / 0	M-B	0.0	0.0	0.15 (1)	6.99	I-F	0 / 677	0.20 (1)										
H-F	-1345 / 0	H-F	0.0	0.0	0.15 (1)	6.99													
M-L	0 / 0	M-L	-28.0	-28.0	0.22 (3)	10.00													
L-K	0 / 848	L-K	-28.0	-28.0	0.32 (2)	10.00													
K-J	0 / 848	K-J	-28.0	-28.0	0.32 (2)	10.00													
J-I	0 / 848	J-I	-28.0	-28.0	0.32 (2)	10.00													
I-H	0 / 0	I-H	-28.0	-28.0	0.22 (3)	10.00													

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = U/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = U/999 (0.08")

CSI: TC=0.59 (D-E:1), BC=0.32 (K-L:2), WB=0.46 (D-K:1), SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.50)
JSI METAL= 0.32 (L) (INPUT = 1.00)



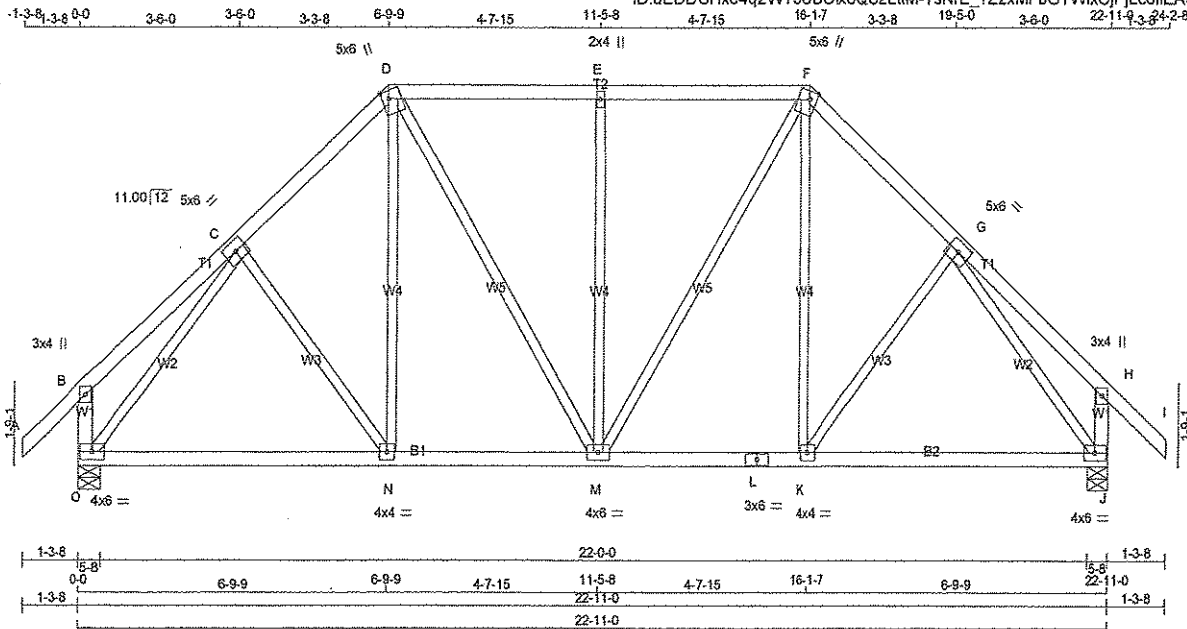
DRWG NO. TAM 36771-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T7	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 115 = 230 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.75
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
O	1404	0	1404	0	0	5-8	1-8	5-8	1-8
J	1404	0	1404	0	0	5-8	1-8	5-8	1-8

UNFACTORED REACTIONS

1ST CASE	MAX	MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	C-N	-7 / 53	0.02 (3)
B-C	0 / 21	-84.3	-84.3 0.15 (1)	10.00	N-D	0 / 298	0.07 (2)
C-D	-1133 / 0	-84.3	-84.3 0.13 (1)	5.84	D-M	0 / 323	0.07 (1)
D-E	-987 / 0	-84.3	-84.3 0.24 (1)	5.99	M-E	-475 / 0	0.59 (1)
E-F	-987 / 0	-84.3	-84.3 0.24 (1)	5.99	M-F	0 / 323	0.07 (1)
F-G	-1133 / 0	-84.3	-84.3 0.13 (1)	5.84	K-F	0 / 298	0.07 (2)
G-H	0 / 21	-84.3	-84.3 0.15 (1)	10.00	K-G	-7 / 53	0.02 (3)
H-I	0 / 40	-84.3	-84.3 0.12 (1)	10.00	O-C	-1370 / 0	0.72 (1)
O-B	-227 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1370 / 0	0.72 (1)
J-H	-227 / 0	0.0	0.0 0.02 (1)	7.81			
O-N	0 / 821	-28.0	-28.0 0.39 (2)	10.00			
N-M	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
M-L	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
L-K	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
K-J	0 / 821	-28.0	-28.0 0.39 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.24 (D-E-1), EC=0.39 (M-N-2), WB=0.72 (C-O-1), SSI=0.19 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.34 (L) (INPUT = 1.00)



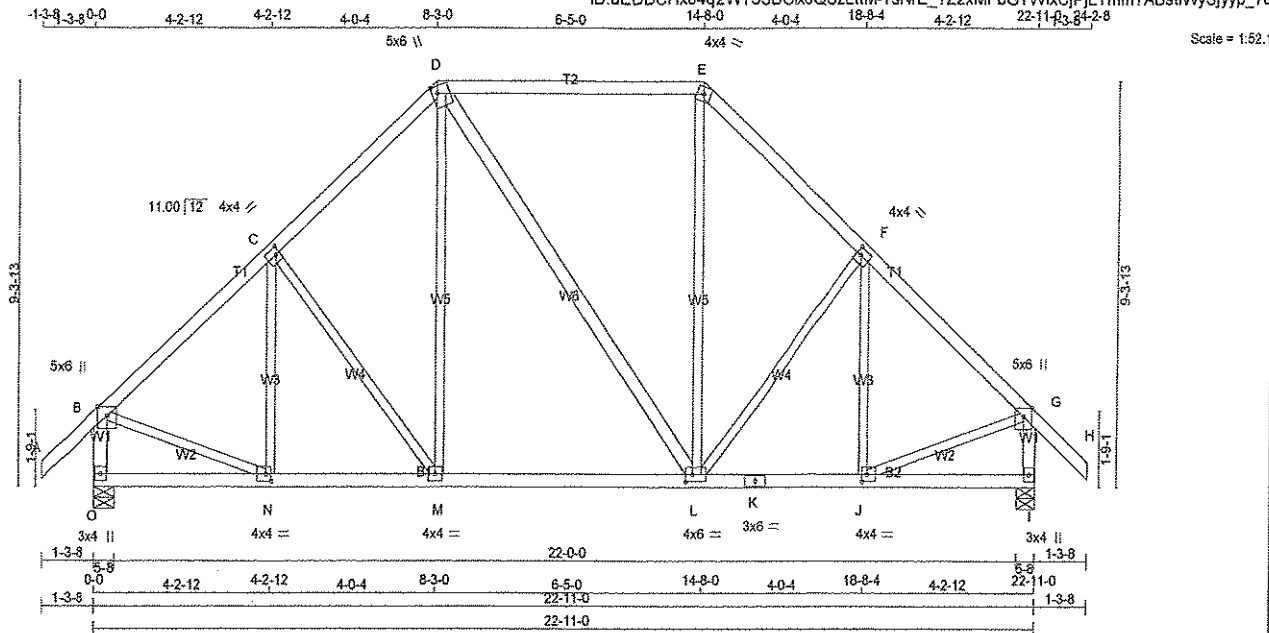
DRWG NO. TAM3672-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T8	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 118 = 236 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0	2.00	1.50
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT
O	1404	0	1404	0	5-8
I	1404	0	1404	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0	
I	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 40	N-C	-177 / 61
B-C	-1163 / 0	C-M	-191 / 0
C-D	-1063 / 0	M-D	0 / 362
D-E	-764 / 0	D-L	0 / 0
E-F	-1064 / 0	L-E	0 / 364
F-G	-1163 / 0	L-F	-190 / 0
G-H	0 / 40	J-F	-178 / 60
O-B	-1352 / 0	B-N	0 / 925
I-G	-1352 / 0	J-G	0 / 925
O-N	0 / 0		
N-M	0 / 877		
M-L	0 / 764		
L-K	0 / 877		
K-J	0 / 877		
J-I	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360} (0.76')$
CALCULATED VERT. DEFL.(LL) = $\frac{1}{999} (0.06')$
ALLOWABLE DEFL.(TL) = $\frac{1}{360} (0.76')$
CALCULATED VERT. DEFL.(TL) = $\frac{1}{999} (0.09')$

CSI: TC=0.45 (D-E-1), BC=0.28 (J-L-2), WB=0.21 (B-N-1), SS=0.21 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 L5 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.76 (N) (INPUT = 0.90)
JSI METAL= 0.33 (N) (INPUT = 1.00)



DWG NO. TAM 36773-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = $5 \times 61 = 305 \text{ lb}$

SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	46.1 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NCC 2010

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/40	-84.3	-84.3 0.12 (1)	10.00	I-D	0/371	0.08 (2)		
B-C	0/19	-84.3	-84.3 0.12 (1)	10.00	I-E	-100/37	0.04 (1)		
C-D	-442/0	-84.3	-84.3 0.09 (1)	6.25	C-I	-100/37	0.04 (1)		
D-E	-442/0	-84.3	-84.3 0.09 (1)	6.25	J-C	-636/0	0.27 (1)		
E-F	0/19	-84.3	-84.3 0.12 (1)	10.00	E-H	-636/0	0.27 (1)		
F-G	0/40	-84.3	-84.3 0.12 (1)	10.00					
J-B	-214/0	0.0	0.0 0.02 (1)	7.81					
H-F	-214/0	0.0	0.0 0.02 (1)	7.81					
J-I	0/372	-28.0	-28.0 0.34 (2)	10.00					
I-H	0/372	-28.0	-28.0 0.34 (2)	10.00					

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

SSIMETAL= 0.24 (E) (INPUT = 1.00)



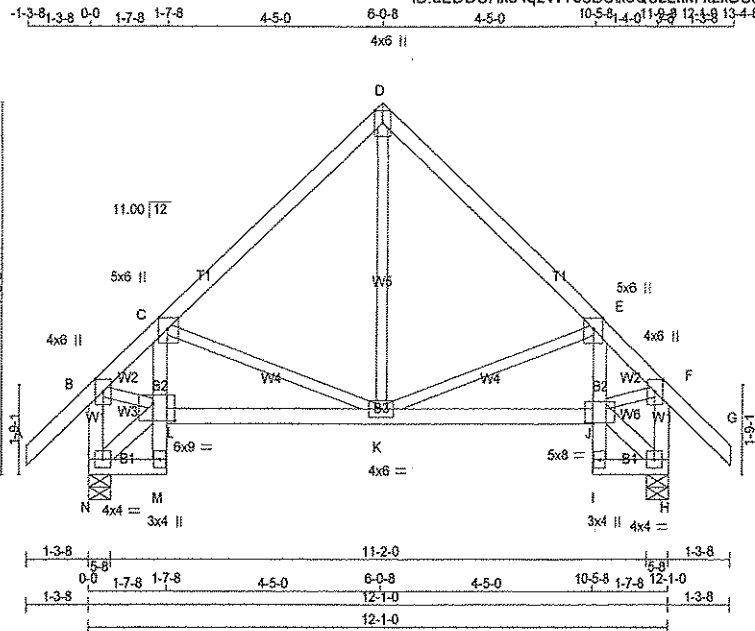
DWG NO. TAM 36774-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T9S	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267349	T10	3	1	TRUSS DESC.		

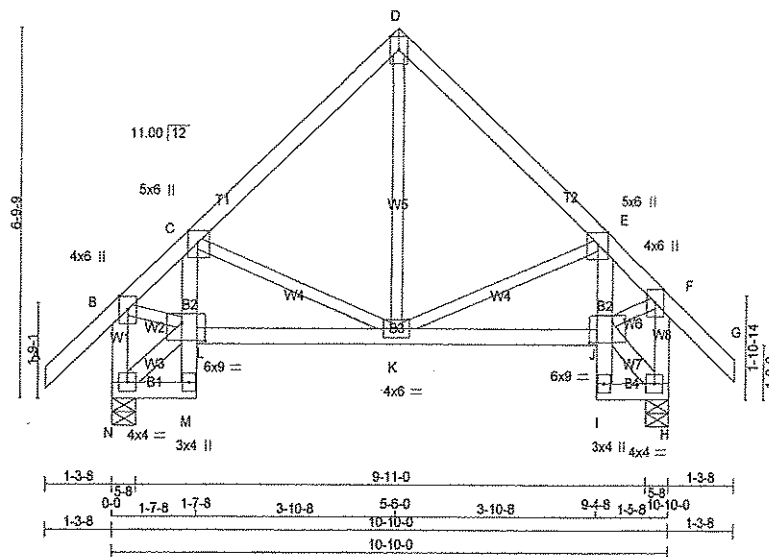
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 1-7-8 1-7-8 3-10-8 5-6-0 3-10-8 9-4-8 1-5-8 10-10-0 3-8 12-1-8

Scale = 1:41.6



TOTAL WEIGHT = 3 X 59 = 176 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+p	MT20	5.0	6.0	Edge 2.25	
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge 2.25	
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW-1	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVW-1	MT20	6.0	9.0	4.50	5.50
K	BMVW-1	MT20	4.0	6.0		
L	BMVW-1	MT20	6.0	9.0	4.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	725	0	725	0
H	725	0	725	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
N	579	348 / 0	114 / 0
H	579	348 / 0	114 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO				FR-TO			
A-B	0 / 40	-84.3	-84.3	C-K	-226 / 0	0.07 (1)	0.07 (1)
B-C	-656 / 0	-84.3	-84.3	K-D	0 / 312	0.07 (2)	0.07 (2)
C-D	-421 / 0	-84.3	-84.3	D-E	-192 / 0	0.08 (1)	0.08 (1)
D-E	-421 / 0	-84.3	-84.3	E-F	-20 / 0	0.00 (1)	0.00 (1)
E-F	-610 / 0	-84.3	-84.3	F-G	0 / 498	0.11 (1)	0.11 (1)
F-G	0 / 40	-84.3	-84.3	G-H	-20 / 0	0.00 (1)	0.00 (1)
G-H	-692 / 0	0.0	0.0	H-I	0 / 477	0.11 (1)	0.11 (1)
H-I	-694 / 0	0.0	0.0				
N-M	0 / 16	-28.0	-28.0				
M-L	0 / 36	0.0	0.0				
L-C	-6 / 77	0.0	0.0				
L-K	0 / 503	-28.0	-28.0				
K-J	0 / 472	-28.0	-28.0				
I-J	0 / 32	0.0	0.0				
J-E	-47 / 54	0.0	0.0				
I-H	0 / 14	-28.0	-28.0				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.16 (C-D:1), BC=0.19 (K-L:2), WB=0.11 (B-L:1), SS=0.11 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

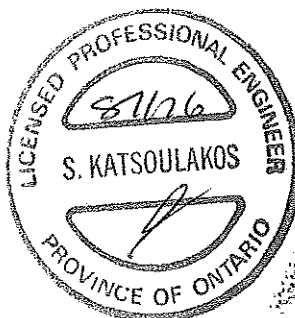
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.60)
JSI METAL= 0.16 (B) (INPUT = 1.00)



DRWG NO. TAM36778-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 39 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	#	25.6	PSF
	DL	#	3.0	PSF
BOT CH.	LL	#	10.5	PSF
	DL	#	7.0	PSF
TOTAL LOAD			#	46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/360$ (0.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.16 (A-B:1), BC=0.16 (F-G:2), WB=0.10 (A-G:1), SS=0.12 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
	PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

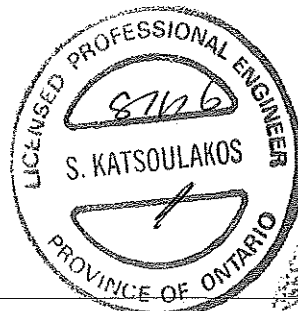
PLATE PLACEMENT TOL. = 0.250 inches

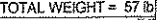
PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.14 (G) (INPUT = 1.00)

DWG NO. TAM36780 - 16

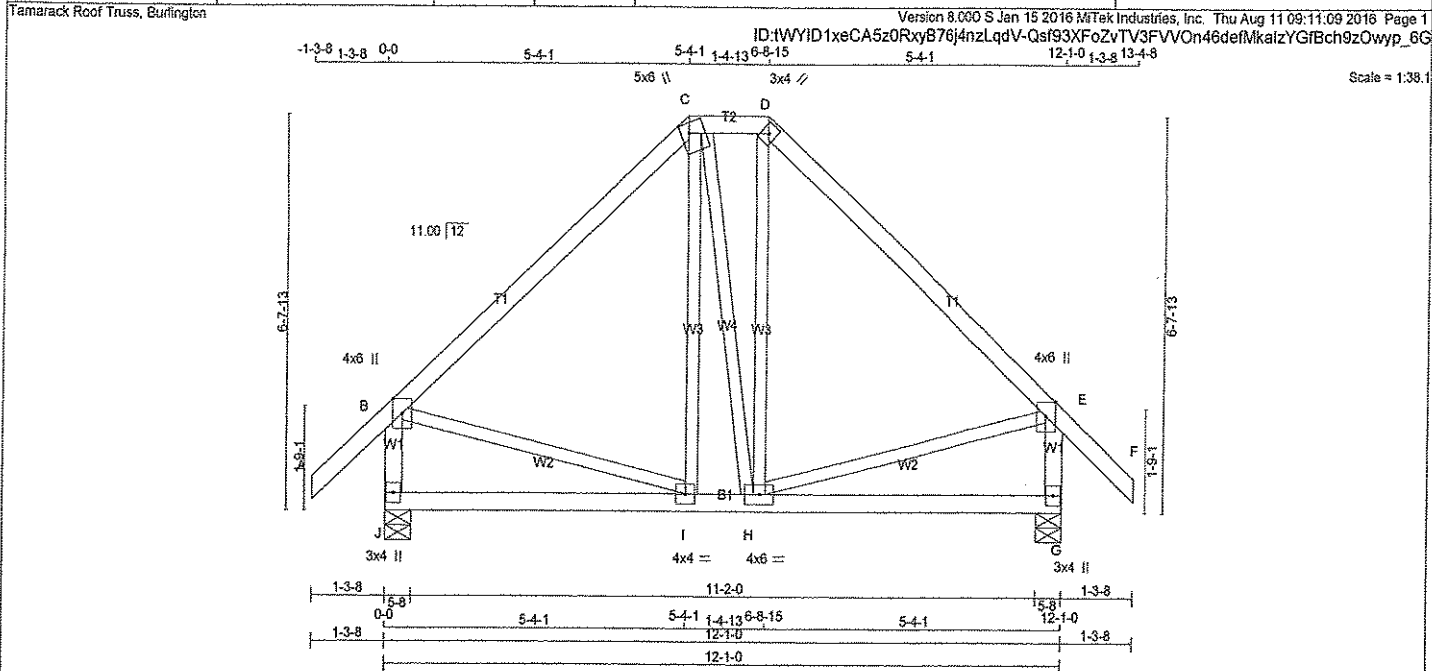
STRUCTURAL
COMPONENT ONLY





DWG NO. TAM36781-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267350	T13	1	1	TRUSS DESC.		



TOTAL WEIGHT = 63 lb

(M/F)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.75
D	TTWW+h	MT20	3.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMVf+p	MT20	3.0	4.0		
H	BMVWW+I	MT20	4.0	6.0		
I	BMVWW+I	MT20	4.0	4.0		
J	BMVf+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	796	0	796	0
G	796	0	796	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
J	COMBINED	SNOW	LIVE			
J	636	380 / 0	127 / 0	0 / 0	0 / 0	129 / 0
G	636	380 / 0	127 / 0	0 / 0	0 / 0	129 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-84.3	-84.3 0.12 (1)	I-C	0 / 126	0.03 (3)	
B-C	-450 / 0	-84.3	-84.3 0.31 (1)	C-H	0 / 15	0.00 (3)	
C-D	-332 / 0	-84.3	-84.3 0.02 (1)	H-D	0 / 141	0.03 (3)	
D-E	-451 / 0	-84.3	-84.3 0.31 (1)	B-I	0 / 342	0.08 (1)	
E-F	0 / 40	-84.3	-84.3 0.12 (1)	H-E	0 / 342	0.08 (1)	
J-B	-733 / 0	0.0	0.0 0.08 (1)				
G-E	-732 / 0	0.0	0.0 0.08 (1)				
J-I	0 / 0	-28.0	-28.0 0.18 (3)				
I-H	0 / 331	-28.0	-28.0 0.24 (2)				
H-G	0 / 0	-28.0	-28.0 0.20 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL)= L/360 (0.40")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.31 (D-E:1), BC=0.24 (H-I:2), WB=0.08 (E-H:1), SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.80)
 JSI METAL= 0.14 (E) (INPUT = 1.00)

DWG NO. TAM3678216
 STRUCTURAL
 COMPONENT ONLY



DWG NO. TAM3678216
STRUCTURAL
COMPONENT ONLY



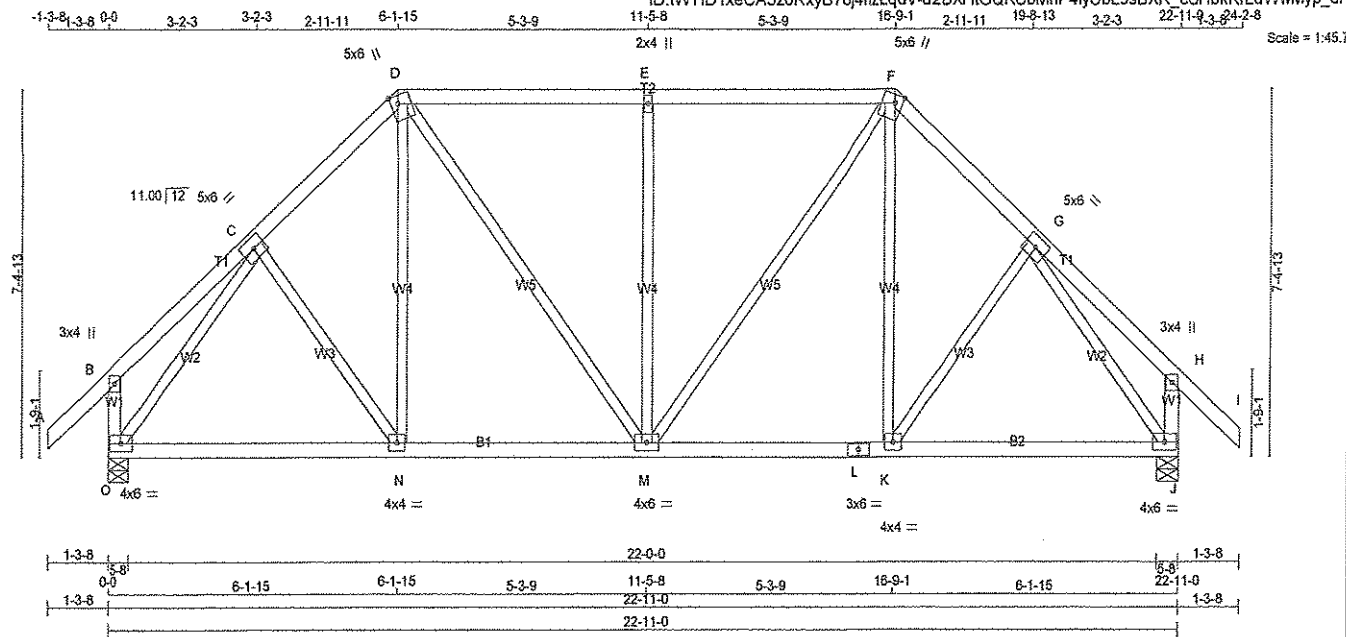
DWG NO. FAM36783.10
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267350	T16	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 111 = 223 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMW+v	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.75
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
O	1404	1404	0	1-8
J	1404	1404	0	1-8

UNFACTORED REACTIONS

JT	1ST LOSE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
O	1136	658 / 0	241 / 0	0 / 0	238 / 0
J	1136	658 / 0	241 / 0	0 / 0	238 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	C-N	0 / 108	0.02 (3)
B-C	0 / 18	-84.3	-84.3	0.12 (1)	10.00	N-D	0 / 253	0.06 (2)
C-D	-1151 / 0	-84.3	-84.3	0.10 (1)	5.83	D-M	0 / 414	0.09 (1)
D-E	-1081 / 0	-84.3	-84.3	0.31 (1)	5.69	M-E	-543 / 0	0.53 (1)
E-F	-1081 / 0	-84.3	-84.3	0.31 (1)	5.69	M-F	0 / 414	0.09 (1)
F-G	-1151 / 0	-84.3	-84.3	0.10 (1)	5.83	K-F	0 / 253	0.06 (2)
G-H	0 / 18	-84.3	-84.3	0.12 (1)	10.00	K-G	0 / 108	0.02 (3)
H-I	0 / 40	-84.3	-84.3	0.12 (1)	10.00	C-C	-1376 / 0	0.60 (1)
O-B	-218 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-1376 / 0	0.60 (1)
J-H	-218 / 0	0.0	0.0	0.02 (1)	7.81			
O-N	0 / 807	-28.0	-28.0	0.34 (2)	10.00			
N-M	0 / 834	-28.0	-28.0	0.35 (2)	10.00			
M-L	0 / 834	-28.0	-28.0	0.35 (2)	10.00			
L-K	0 / 834	-28.0	-28.0	0.35 (2)	10.00			
K-J	0 / 807	-28.0	-28.0	0.34 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/699 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/699 (0.11")

CSI: TC=0.31 (D-E:1), BC=0.35 (M-N:2), WB=0.60 (G-J:1), SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

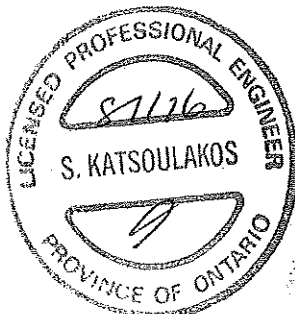
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)

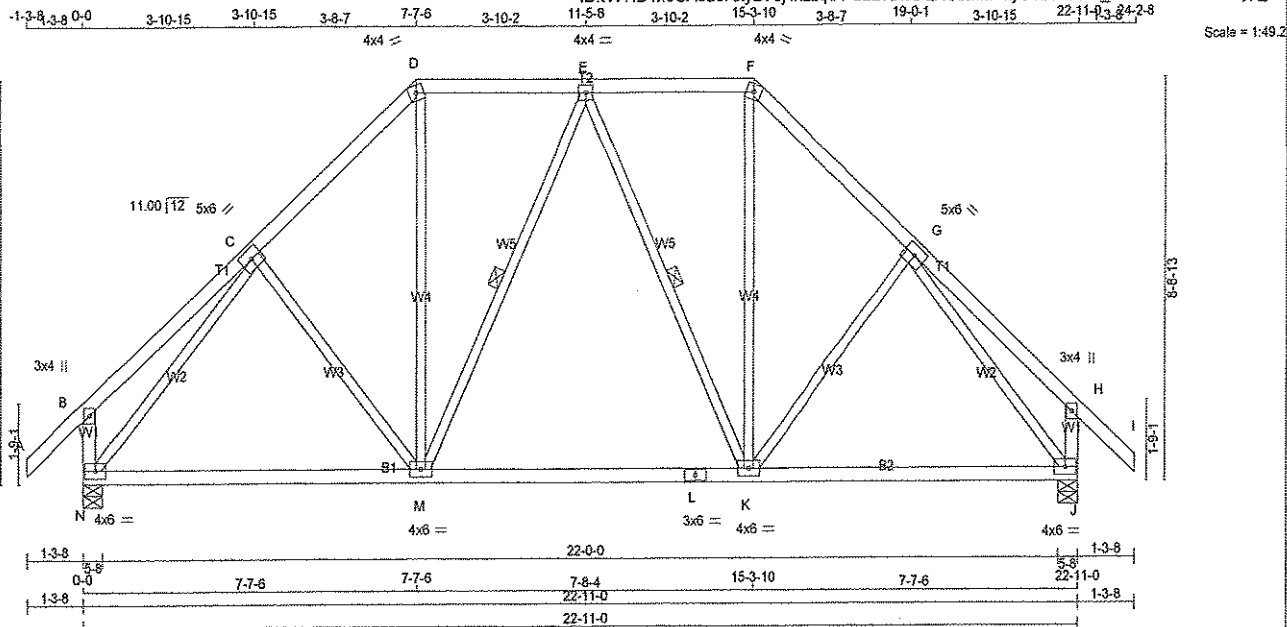


DRWG NO. TAM36705-16
STRUCTURAL
COMPONENT ONLY

JOB NAME 267350	TRUSS NAME T17	QUANTITY 2	PLY 1	JOB DESC. 42181	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 113 = 226 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
N 1404 0	1404 0	5-8	1-8
J 1404 0	1404 0	5-8	1-8

UNFACTORED REACTIONS							
1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
JT COMBINED							
N 1138	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0	
J 1138	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)
FR-TO			
A-B	0 / 40	-84.3	-84.3 0.12 (1)
B-C	0 / 24	-84.3	-84.3 0.19 (1)
C-D	-1104 / 0	-84.3	-84.3 0.16 (1)
D-E	-799 / 0	-84.3	-84.3 0.16 (1)
E-F	-799 / 0	-84.3	-84.3 0.16 (1)
F-G	-1104 / 0	-84.3	-84.3 0.16 (1)
G-H	0 / 24	-84.3	-84.3 0.19 (1)
H-I	0 / 40	-84.3	-84.3 0.12 (1)
N-B	-240 / 0	0.0	0.0 0.03 (1)
J-H	-240 / 0	0.0	0.0 0.03 (1)
N-M	0 / 834	-28.0	-28.0 0.47 (2)
M-L	0 / 885	-28.0	-28.0 0.48 (2)
L-K	0 / 885	-28.0	-28.0 0.48 (2)
K-J	0 / 834	-28.0	-28.0 0.47 (2)

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)
FR-TO		
C-M	-63 / 78	0.04 (1)
M-D	0 / 476	0.11 (1)
M-E	-211 / 0	0.13 (1)
E-K	-211 / 0	0.13 (1)
K-F	0 / 476	0.11 (1)
K-G	-63 / 78	0.04 (1)
N-C	-1361 / 0	0.89 (1)
G-J	-1361 / 0	0.89 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.19 (B-C:1), BC=0.48 (K-M:2), WB=0.89 (C-N:1), SS=0.18 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1687 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (N) (INPUT = 0.90)
JSI METAL= 0.97 (L) (INPUT = 1.00)



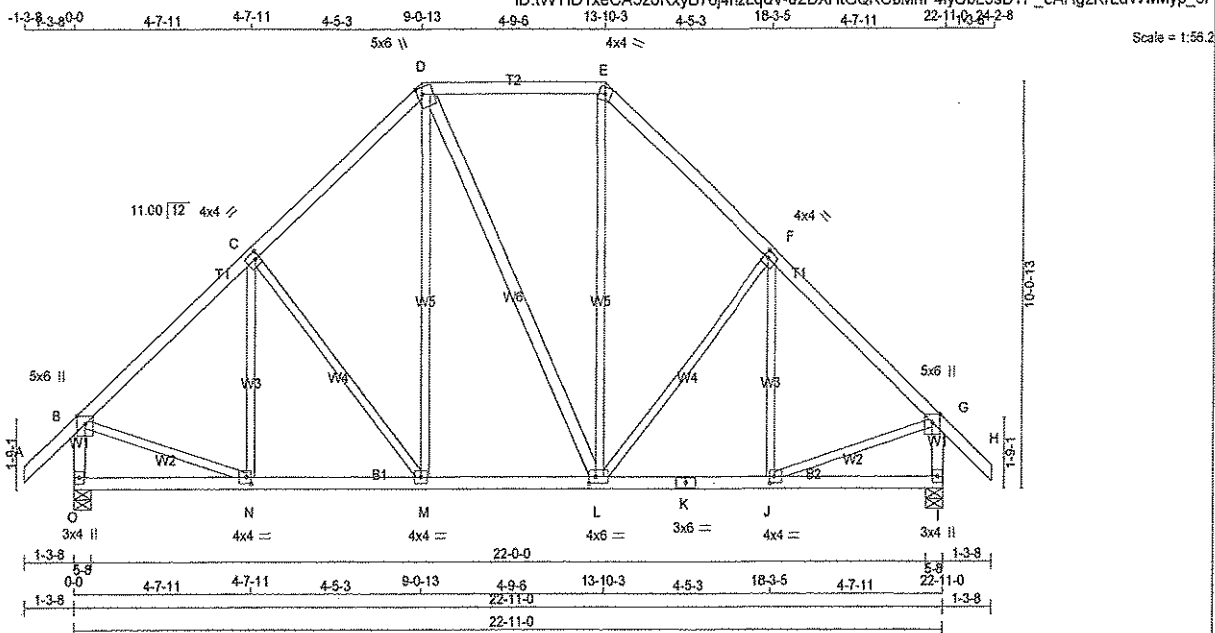
DRWG NO. TAM36786-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267350	T18	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 122 = 244 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWV+m	MT20	5.0	6.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	4.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
L	BMVWV-t	MT20	4.0	6.0	2.00	2.00
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0	2.00	1.50
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1404	0	1404	0
I	1404	0	1404	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
O	1136	658 / 0	241 / 0
I	1136	658 / 0	241 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-84.3 -84.3	0.12 (1)	10.00	N-C	-127 / 118	0.07 (1)
B-C	-1172 / 0	-84.3 -84.3	0.24 (1)	5.63	C-M	-259 / 0	0.27 (1)
C-D	-1018 / 0	-84.3 -84.3	0.24 (1)	5.94	M-D	0 / 356	0.08 (2)
D-E	-729 / 0	-84.3 -84.3	0.25 (1)	6.25	D-L	0 / 2	0.00 (2)
E-F	-1018 / 0	-84.3 -84.3	0.24 (1)	5.93	L-E	0 / 359	0.08 (2)
F-G	-1172 / 0	-84.3 -84.3	0.24 (1)	5.63	I-F	-259 / 0	0.28 (1)
G-H	0 / 40	-84.3 -84.3	0.12 (1)	10.00	J-F	-129 / 116	0.07 (1)
O-B	-1350 / 0	0.0 0.0	0.15 (1)	6.98	B-N	0 / 927	0.21 (1)
I-G	-1350 / 0	0.0 0.0	0.15 (1)	6.98	J-G	0 / 926	0.21 (1)
O-N	0 / 0	-28.0 -28.0	0.14 (3)	10.00			
N-M	0 / 886	-28.0 -28.0	0.24 (2)	10.00			
M-L	0 / 728	-28.0 -28.0	0.20 (2)	10.00			
L-K	0 / 886	-28.0 -28.0	0.24 (2)	10.00			
K-J	0 / 886	-28.0 -28.0	0.24 (2)	10.00			
J-I	0 / 0	-28.0 -28.0	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	CL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.25 (D-E-1), BC=0.24 (M-N-2), WB=0.27 (C-M-1), SS=0.16 (D-E-1)

DOL LUMBER=1.00 NAH=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (N) (INPUT = 0.90)
JSI METAL= 0.33 (N) (INPUT = 1.00)



DWG NO. TAM36787-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 47 lb

DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

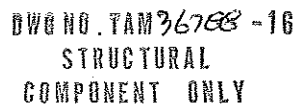
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	H-E	-463 / 0 0.07 (1)		
B-C	-563 / 0	-84.3	-84.3	0.11 (1)	6.25	H-F	0 / 950 0.21 (1)		
C-D	-497 / 0	-84.3	-84.3	0.03 (1)	6.25	I-D	-74 / 42 0.01 (1)		
D-E	-664 / 0	-84.3	-84.3	0.28 (1)	6.21	D-H	0 / 513 0.12 (1)		
E-F	-864 / 0	-84.3	-84.3	0.28 (1)	6.21	K-I	-19 / 0 0.00 (1)		
G-F	-548 / 0	0.0	0.0	0.07 (1)	7.81	B-I	0 / 392 0.09 (1)		
K-B	-702 / 0	0.0	0.0	0.08 (1)	7.81				
K-J	0 / 15	-28.0	-28.0	0.02 (2)	10.00				
J-I	0 / 36	0.0	0.0	0.02 (1)	10.00				
I-C	0 / 33	0.0	0.0	0.03 (1)	10.00				
I-H	0 / 402	-28.0	-28.0	0.21 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

GOVT. OFFICE

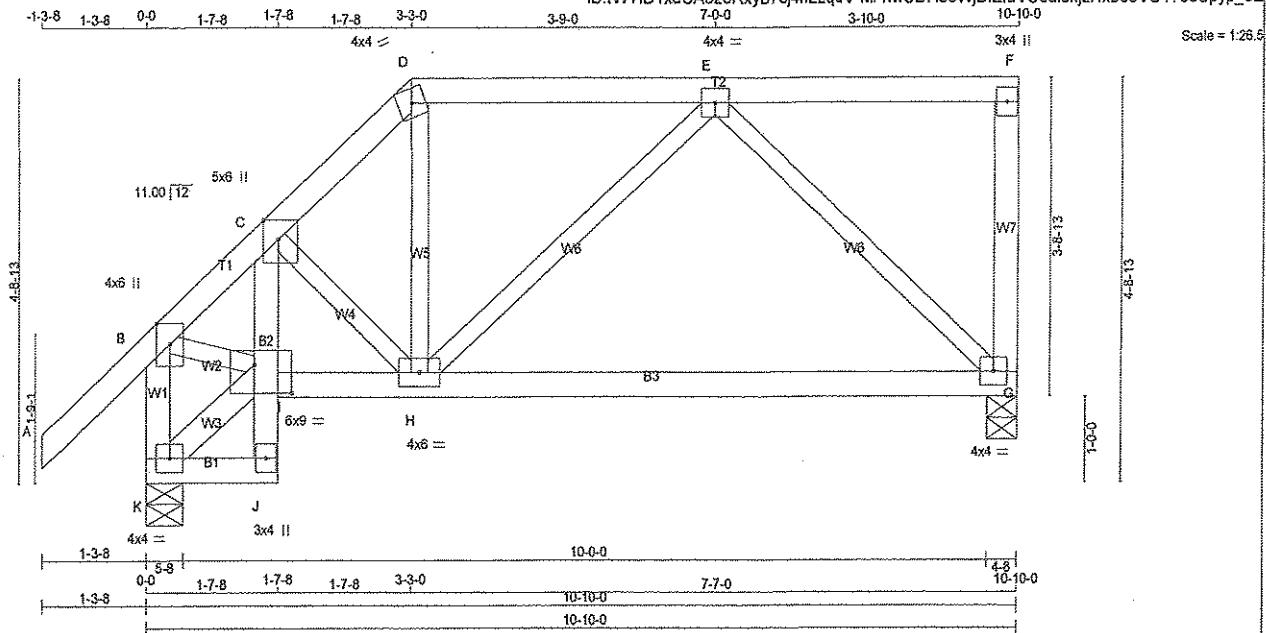


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267350	T20	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 51 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
J - C	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge
C	TMVW+p	MT20	5.0	6.0	Edge 2.25
D	TTW-m	MT20	4.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	4.0	
H	BMVWV-t	MT20	4.0	6.0	
I	BMVWV-t	MT20	6.0	9.0	4.00 5.50
J	BMV+p	MT20	3.0	4.0	
K	BMVW1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
G	601	0	0	601	0	0	4-8	1-8	
K	733	0	0	733	0	0	5-8	1-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
G	494	273 / 0	114 / 0	0 / 0	0 / 0	108 / 0	0 / 0
K	584	353 / 0	114 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 140	-84.3	-84.3 0.12 (1)	10.00	C-H	-15 / 57	0.01 (3)
B-C	-623 / 0	-84.3	-84.3 0.12 (1)	6.25	H-D	0 / 258	0.03 (2)
C-D	-587 / 0	-84.3	-84.3 0.05 (1)	6.25	K-I	-18 / 0	0.00 (1)
D-E	-437 / 0	-84.3	-84.3 0.20 (1)	6.25	B-I	0 / 436	0.10 (1)
E-F	0 / 0	-84.3	-84.3 0.20 (1)	10.00	H-E	0 / 167	0.04 (3)
G-F	-124 / 0	0.0	0.0 0.03 (1)	7.81	E-G	-587 / 0	0.25 (1)
K-B	-701 / 0	0.0	0.0 0.08 (1)	7.81			
K-J	0 / 14	-28.0	-28.0 0.02 (2)	10.00			
J-I	0 / 36	0.0	0.0 0.03 (1)	10.00			
I-C	-93 / 0	0.0	0.0 0.02 (1)	7.81			
I-H	0 / 436	-28.0	-28.0 0.42 (2)	10.00			
H-G	0 / 437	-28.0	-28.0 0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/712 (0.18")

CSI: TC=0.20 (D-E:1), BC=0.42 (H-I:2), WB=0.25 (E-G:1), SSI=0.17 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1656

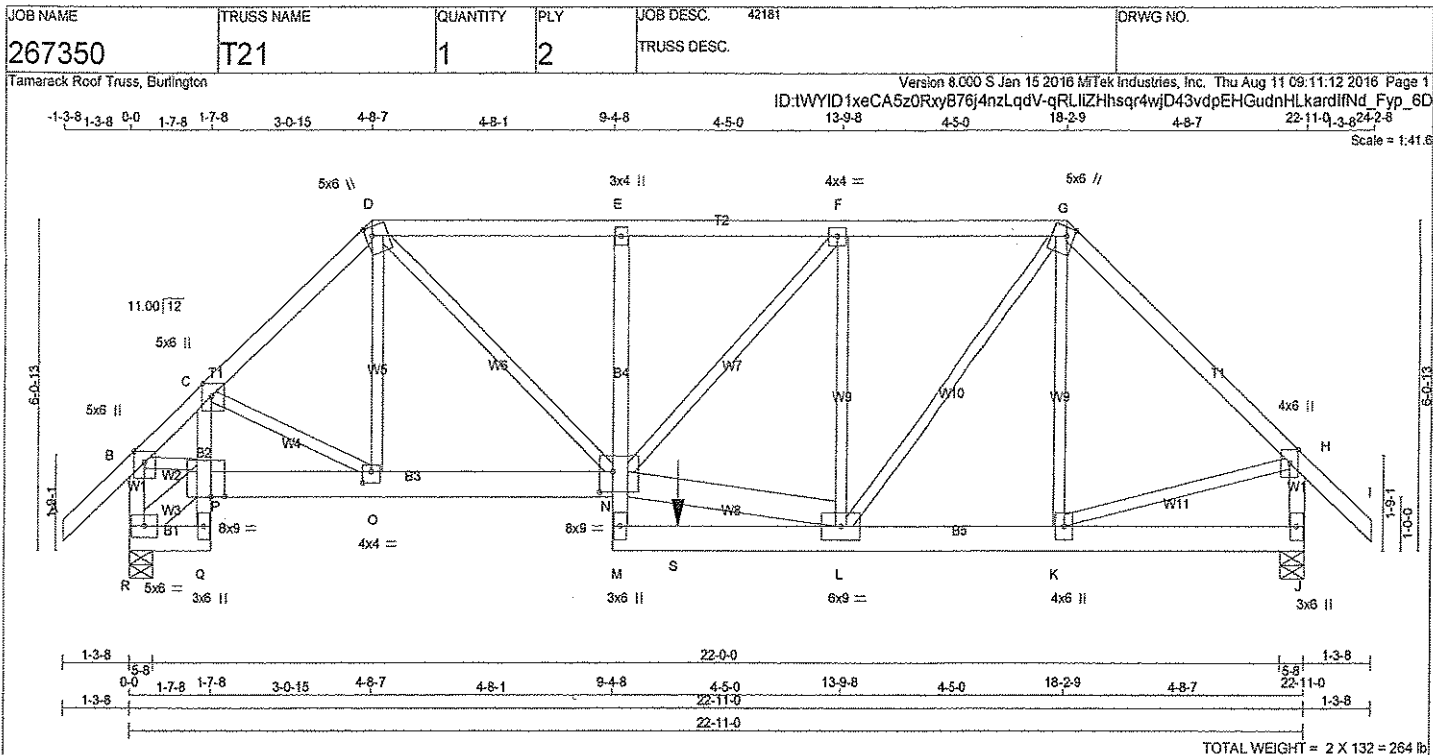
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.51 (B) (INPUT=0.90)
JSI METAL=0.16 (G) (INPUT=1.00)



DRWG NO. TAM 36289-16
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - Q	2x6	DRY	No.2
Q - C	2x4	DRY	No.2
P - N	2x6	DRY	No.2
M - E	2x4	DRY	No.2
M - J	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	TOP
C-G	12	TOP
G-I	12	TOP
R-B	12	TOP
J-H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-Q	2	TOP
P-N	2	TOP
M-J	2	TOP
Q-C	1	TOP
E-M	1	TOP

WEBS : (0.122"x3") SPIRAL NAILS	
2x3	6
2x6	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	5.0	6.0	Edge	2.25
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMVW+t	MT20	4.0	4.0		
G	TTVW+m	MT20	5.0	6.0	2.00	1.75
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMVW+t	MT20	4.0	6.0		
L	BMVW+t	MT20	6.0	9.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMW+t	MT20	8.0	9.0	4.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	LOC.	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
J	2012	0	2012	0	0	5-8	1-8		
R	2113	0	2113	0	0	5-8	1-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1628	943 / 0	344 / 0	0 / 0	0 / 0	340 / 0	0 / 0
R	1708	991 / 0	360 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 40	-84.3	-84.3	0.06 (1)	10.00	C-O	0 / 94	0.01 (1)
B-C	-2220 / 0	-84.3	-84.3	0.08 (1)	5.91	O-D	0 / 221	0.03 (3)
C-D	-2381 / 0	-84.3	-84.3	0.09 (1)	5.74	D-N	0 / 1721	0.21 (1)
D-E	-2979 / 0	-84.3	-84.3	0.17 (1)	5.18	N-L	0 / 2315	0.15 (1)
E-F	-2968 / 0	-84.3	-84.3	0.15 (1)	5.21	L-F	0 / 980	0.12 (1)
F-G	-2295 / 0	-84.3	-84.3	0.13 (1)	5.77	L-F	-1109 / 0	0.31 (1)
G-H	-1845 / 0	-84.3	-84.3	0.21 (1)	6.17	L-G	0 / 1552	0.19 (1)
H-I	0 / 40	-84.3	-84.3	0.06 (1)	10.00	K-G	-345 / 7	0.10 (1)
R-B	-2054 / 0	0.0	0.0	0.12 (1)	7.71	K-H	0 / 1407	0.17 (1)
J-H	-1941 / 0	0.0	0.0	0.11 (1)	7.81	R-P	-68 / 0	0.00 (1)
						B-P	0 / 1663	0.21 (1)
R-Q	0 / 56	-28.0	-28.0	0.01 (2)	10.00			
Q-P	0 / 36	0.0	0.0	0.04 (1)	10.00			
P-C	-350 / 0	0.0	0.0	0.04 (1)	7.81			
P-O	0 / 1657	-28.0	-28.0	0.13 (1)	10.00			
O-N	0 / 1743	-28.0	-28.0	0.14 (1)	10.00			
M-N	0 / 949	0.0	0.0	0.12 (1)	10.00			
N-E	-401 / 0	0.0	0.0	0.05 (1)	7.81			
M-S	0 / 53	-28.0	-28.0	0.39 (1)	10.00			
S-L	0 / 53	-28.0	-28.0	0.39 (1)	10.00			
L-K	0 / 1350	-28.0	-28.0	0.23 (1)	10.00			
K-J	0 / 0	-28.0	-28.0	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
S	10-7-12	-1317	-1317		BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.21 (G-H-1), BC=0.39 (L-M-1), WB=0.31 (F-L-1), SS=0.31 (L-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (O) (INPUT = 0.90)
JSI METAL= 0.24 (P) (INPUT = 1.00)

DWG NO. YAM36790-10
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267350	T21	1	2	TRUSS DESC.		

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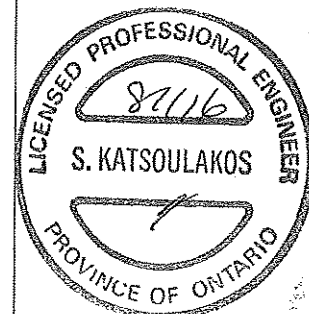
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWW-t	MT20	4.0	4.0	2.50	2.00
P	BVMWW-t	MT20	8.0	9.0		3.00
Q	BMV+p	MT20	3.0	6.0		
R	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1317.2 lbs FACTORED DOWN AT
10-7-12 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.



DWG NO. TAM 36790-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T33	6	1	TRUSS DESC.		

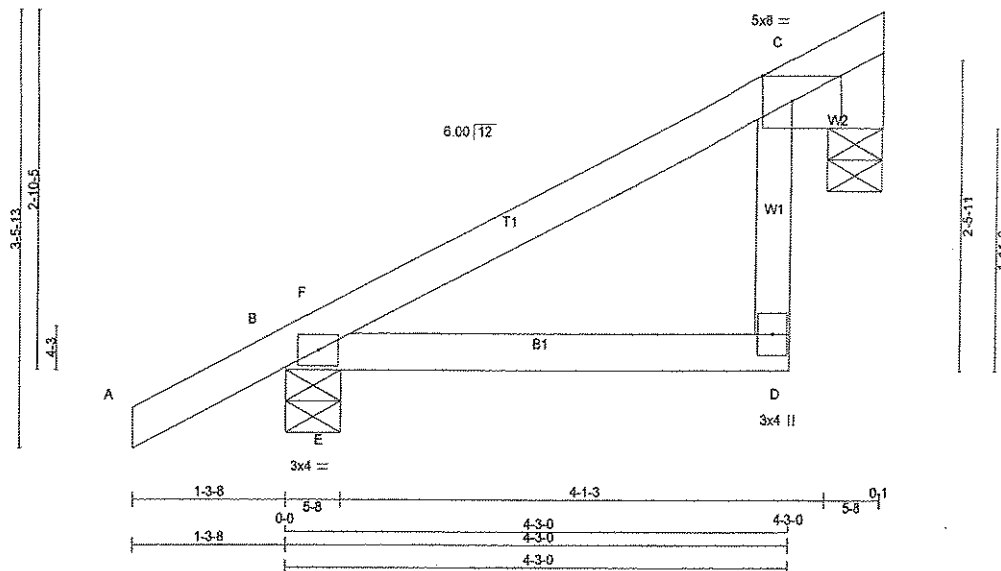
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 4-3-0 4-3-0

Scale = 1:18.0



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	D - C	2x4	DRY	No.2	SPF
	B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
	B	TMB1-I	MT20	3.0	4.0		
	C	TMVW1-p	MT20	5.0	8.0	Edge	3.00
	D	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
C	239	0	0	5-8
B	353	0	0	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
C	196	109 / 0	45 / 0
B	273	178 / 0	45 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 24	-84.3 -84.3	0.11 (1)	10.00
B-F	-65 / 13	-84.3 -84.3	0.07 (3)	6.25
F-C	-1 / 6	-84.3 -84.3	0.20 (1)	10.00
D-C	0 / 101	0.0 0.0	0.02 (2)	10.00
B-E	0 / 0	-28.0 -28.0	0.13 (1)	10.00
E-D	0 / 0	-28.0 -28.0	0.15 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL (LL) = $L/999$ (0.03")
ALLOWABLE DEFL (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL (TL) = $L/999$ (0.04")

CSI: TC=0.20 (C-F:1), BC=0.15 (D-E:1), WB=0.00 (E-F:1), SSI=0.10 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

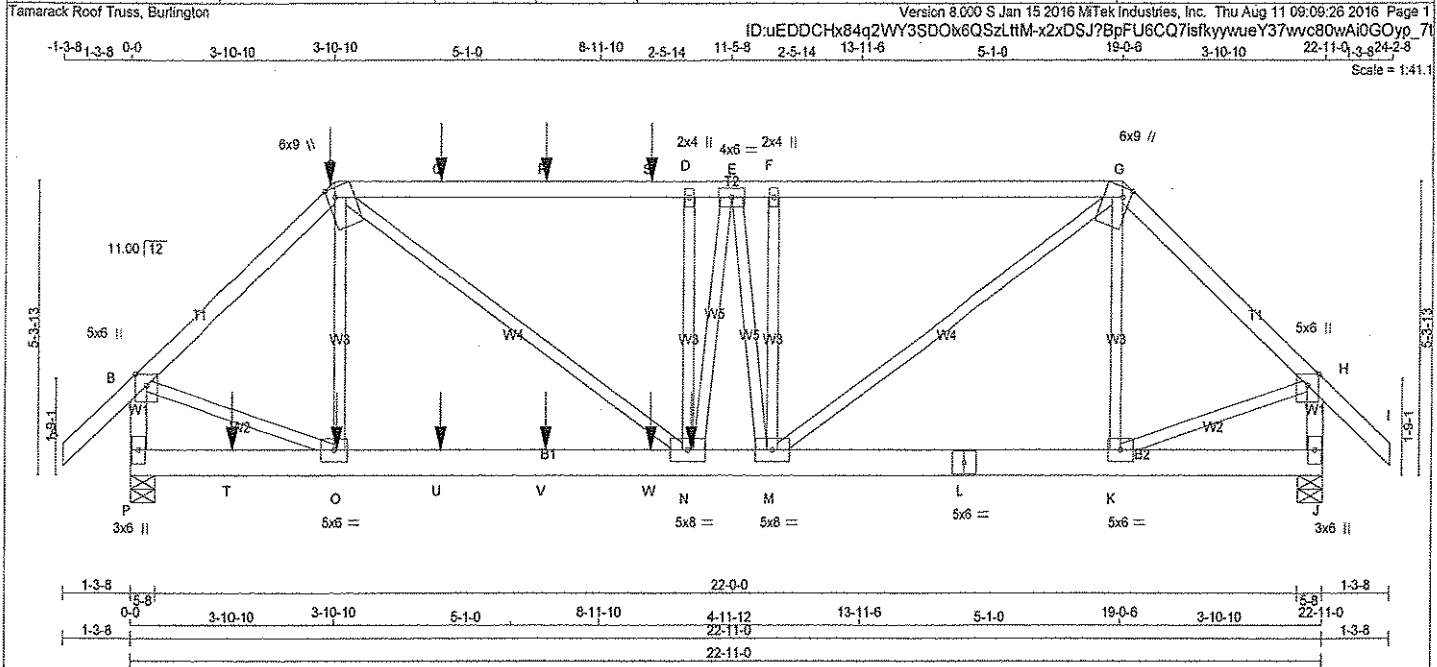
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.32 (B) (INPUT = 0.90)
JSI METAL=0.05 (B) (INPUT = 1.00)



DRWG NO. TAM 36776-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T50	1	2	TRUSS DESC.		



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - G 2x4 DRY No.2 SPF G - I 2x4 DRY No.2 SPF P - B 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF P - L 2x6 DRY No.2 SPF L - J 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE(61.0) C-G 1 12 SIDE(61.0) G-I 1 12 TOP P-B 1 12 TOP J-H 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS P-L 2 12 SIDE(183.1) L-J 2 12 TOP WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 SIDE(130.4) 2x3 1 6 SIDE(130.6) NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW+p MT20 5.0 6.0 Edge C TTWW+m MT20 6.0 9.0 Edge 1.75 D TMVW+w MT20 2.0 4.0 E TMVW+l MT20 4.0 6.0 F TMVW+w MT20 2.0 4.0 G TTWW+m MT20 6.0 9.0 Edge 1.75 H TMVW+p MT20 5.0 6.0 Edge J BMV1+p MT20 3.0 6.0 K BMVW+l MT20 5.0 6.0 L BS-l MT20 5.0 6.0 M BMVW+l MT20 5.0 8.0 N BMVW+l MT20 5.0 8.0 O BMVW+l MT20 5.0 6.0 P BMV1+p MT20 3.0 6.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQRD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX P 3123 0 3123 0 0 5-8 1-11 J 2491 0 2491 0 0 5-8 1-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL P 2528 1480 / 0 537 / 0 0 / 0 531 / 0 0 / 0 J 1999 1184 / 0 405 / 0 0 / 0 0 / 0 410 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) MAX. CSI (LC) MAX. UNBRAC LENGTH FR-TO A-B 0 / 40 -84.3 -84.3 0.06 (1) 10.00 O-C -352 / 225 0.07 (1) B-C -3054 / 0 -84.3 -84.3 0.17 (1) 5.14 C-N 0 / 2558 0.32 (1) C-Q -4305 / 0 -84.3 -84.3 0.77 (1) 3.72 N-D -1436 / 0 0.29 (1) Q-R -4305 / 0 -84.3 -84.3 0.77 (1) 3.72 M-F -808 / 0 0.18 (1) R-S -4305 / 0 -84.3 -84.3 0.77 (1) 3.72 K-G -392 / 35 0.09 (1) S-D -4305 / 0 -84.3 -84.3 0.77 (1) 3.72 B-O 0 / 2385 0.29 (1) D-E -4305 / 0 -84.3 -84.3 0.50 (1) 3.72 K-H 0 / 1830 0.23 (1) E-F -3912 / 0 -84.3 -84.3 0.34 (1) 4.39 M-G 0 / 2700 0.33 (1) F-G -3912 / 0 -84.3 -84.3 0.44 (1) 4.39 N-E 0 / 1718 0.21 (1) G-H -2364 / 0 -84.3 -84.3 0.15 (1) 5.69 E-M -585 / 0 0.12 (1) H-I 0 / 40 -84.3 -84.3 0.06 (1) 10.00 P-B -3100 / 0 0.0 0.0 0.18 (1) 6.58 J-H -2463 / 0 0.0 0.0 0.14 (1) 7.19 P-T 0 / 0 -28.0 -28.0 0.12 (2) 10.00 T-O 0 / 0 -28.0 -28.0 0.12 (2) 10.00 O-U 0 / 2237 -28.0 -28.0 0.24 (1) 10.00 U-V 0 / 2237 -28.0 -28.0 0.24 (1) 10.00 V-W 0 / 2237 -28.0 -28.0 0.24 (1) 10.00 W-N 0 / 2237 -28.0 -28.0 0.24 (1) 10.00 N-M 0 / 3897 -28.0 -28.0 0.30 (1) 10.00 M-L 0 / 1729 -28.0 -28.0 0.15 (1) 10.00 L-K 0 / 1729 -28.0 -28.0 0.15 (1) 10.00 K-J 0 / 0 -28.0 -28.0 0.05 (2) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR TYPE C 3-10-10 -197 -197 --- FRONT VERT TOTAL C 3-10-10 -167 -167 --- BACK VERT TOTAL N 10-8-8 -1702 -1702 --- BACK VERT TOTAL O 3-11-4 -50 -87 --- BACK VERT TOTAL O 5-11-4 -163 -163 --- BACK VERT TOTAL R 7-11-4 -163 -163 --- BACK VERT TOTAL S 9-11-4 -163 -163 --- BACK VERT TOTAL T 1-11-4 -50 -87 --- BACK VERT TOTAL U 5-11-4 -50 -87 --- BACK VERT TOTAL V 7-11-4 -50 -87 --- BACK VERT TOTAL W 9-11-4 -50 -87 --- BACK VERT TOTAL				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 46.1 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, ECBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.76") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08") ALLOWABLE DEFL.(TL)= L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13") CSI: TC=0.77 (C-D:1), BC=0.30 (M-N:1), WB=0.33 (G-M:1), SSI=0.33 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (P.S.I) (P.L.I) (P.L.I) MAX MIN MAX MIN MAX MIN MT20 618 354 1867 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.77 (B) (INPUT = 0.90) JSI METAL= 0.28 (C) (INPUT = 1.00) PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO DRWG NO. TAM 36772-10 STRUCTURAL COMPONENT ONLY			
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267348	T50	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 197.3 lbs FACTORED DOWN AT 3-10-10, 166.8 lbs FACTORED DOWN AT 3-10-10, 163.3 lbs FACTORED DOWN AT 5-11-4, AND 163.3 lbs FACTORED DOWN AT 7-11-4, AND 163.3 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 87.4 lbs FACTORED DOWN AT 5-11-4, 87.4 lbs FACTORED DOWN AT 7-11-4, AND 87.4 lbs FACTORED DOWN AT 9-11-4, AND 1702.4 lbs FACTORED DOWN AT 10-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 36777-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267349	T50Z	1	2	TRUSS DESC.		

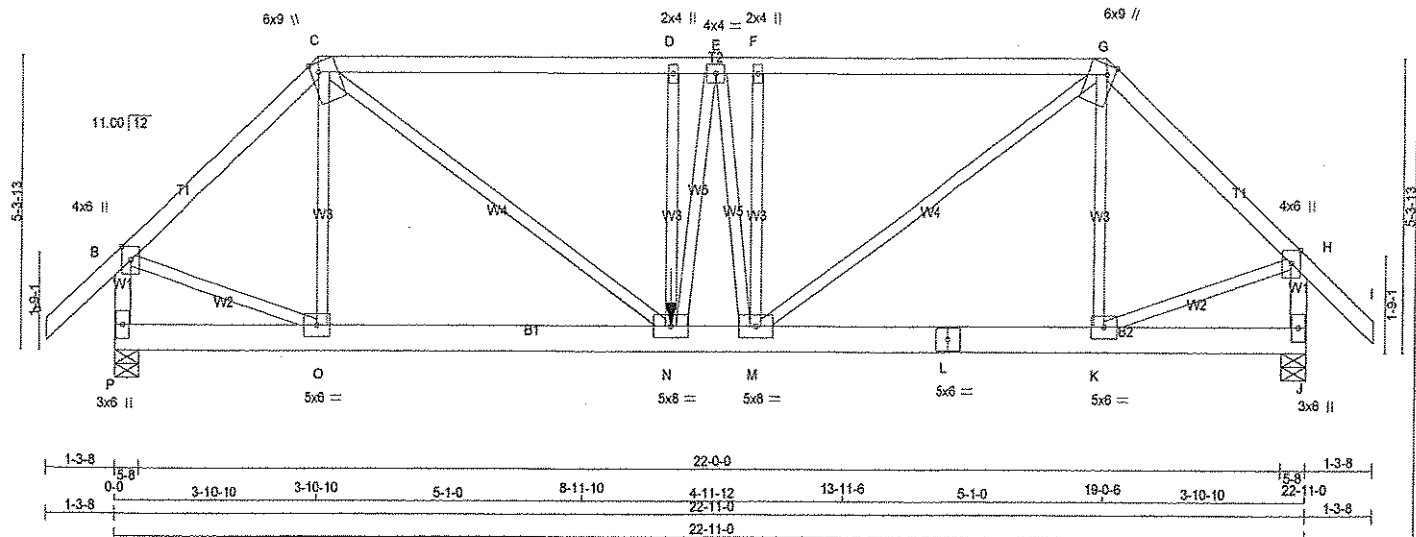
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 3-10-10 3-10-10 5-1-0 8-11-10 2-5-14 11-5-8 2-5-14 13-11-6 5-1-0 19-0-6 3-10-10 22-11-0 3-824-2-8

Scale = 1:41.1



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - G	12	TOP
G - I	12	TOP
P - B	12	TOP
J - H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - L	12	SIDE(183.1)
L - J	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
D - N	6	SIDE(175.5)
2x3	6	SIDE(175.5)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMW+p	MT20	4.0	6.0 Edge
C	TTWW+m	MT20	6.0	9.0 Edge 1.75
D	TMW+w	MT20	2.0	4.0
E	TMWV-t	MT20	4.0	4.0
F	TMWV+w	MT20	2.0	4.0
G	TTWW+m	MT20	6.0	9.0 Edge 1.75
H	TMWV+p	MT20	4.0	6.0 Edge
J	BMV1+p	MT20	3.0	6.0
K	BMWV-t	MT20	5.0	6.0
L	BS-t	MT20	5.0	6.0
M	BMWVW-t	MT20	5.0	8.0
N	BMWVW-t	MT20	5.0	8.0
O	BMWVW-t	MT20	5.0	6.0
P	BMV1+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REGRD		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2500	0	2500	0	0	5-8	1-8
J	2355	0	2355	0	0	5-8	1-8

UNFACTORED REACTIONS							
1ST LCASE	MAX	MIN	COMPONENT	REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD
P	2013	1182 / 0	415 / 0	0 / 0	0 / 0	415 / 0	0 / 0
J	1897	1113 / 0	392 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD
	(LBS)	(PLF)	LC1 (LC)		(LBS)	(PLF)	LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-84.3	-84.3 0.06 (1)	10.00	O-C	-343 / 49	0.07 (1)
B-C	-2391 / 0	-84.3	-84.3 0.15 (1)	5.66	C-N	0 / 2638	0.33 (1)
C-D	-3883 / 0	-84.3	-84.3 0.40 (1)	4.46	N-D	-681 / 0	0.14 (1)
D-E	-3883 / 0	-84.3	-84.3 0.30 (1)	4.46	M-F	-816 / 0	0.17 (1)
E-F	-3616 / 0	-84.3	-84.3 0.33 (1)	4.55	K-G	-352 / 44	0.07 (1)
F-G	-3616 / 0	-84.3	-84.3 0.42 (1)	4.55	B-O	0 / 1851	0.23 (1)
G-H	-2218 / 0	-84.3	-84.3 0.15 (1)	5.83	K-H	0 / 1717	0.21 (1)
H-I	0 / 40	-84.3	-84.3 0.06 (1)	10.00	M-G	0 / 2466	0.31 (1)
P-B	-2468 / 0	0.0	0.0 0.14 (1)	7.17	N-E	0 / 1146	0.14 (1)
J-H	-2328 / 0	0.0	0.0 0.13 (1)	7.35	E-M	-420 / 0	0.09 (1)
P-O	0 / 0	-28.0	-28.0 0.08 (2)	10.00			
O-N	0 / 1749	-28.0	-28.0 0.18 (1)	10.00			
N-M	0 / 3677	-28.0	-28.0 0.30 (1)	10.00			
M-L	0 / 1622	-28.0	-28.0 0.15 (1)	10.00			
L-K	0 / 1622	-28.0	-28.0 0.15 (1)	10.00			
K-J	0 / 0	-28.0	-28.0 0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)				
JT	LOC.	LC1	MAX-	MAX+
N	10-7-12	-2048	-2048	--



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.42 (F-G:1), BC=0.30 (M-N:1), WB=0.33 (C-N:1), SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

DWG NO. TAM36779-16
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267349	T50Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Aug 11 09:10:13 2016 Page 2
ID:uEDDCHx84q2WY3SDOk6QSzLttM-35H JOaglhPRLeDk604 LrKmA?AzQtvxk a?QAyp 78

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 2048.0 lbs FACTORED DOWN AT
10'-7-12 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

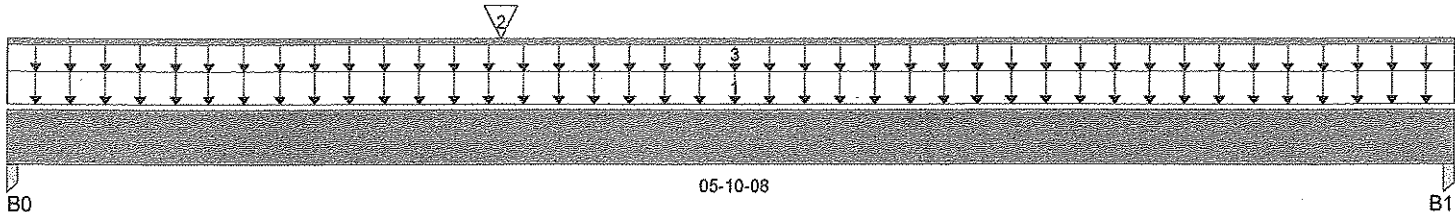


DWG NO. TAM 36779-16
STRUCTURAL
COMPONENT ONLY



Build 4516
 Job Name: 42181/GREENPARK HOMES
 Address: STARTIME HOME/BRIDGEFORD 1
 City, Province, Postal Code: VAUGHAN,
 Customer:
 Code reports: NLGA

File Name: 42181-267350
 Description: Designs\BM3
 Specifier:
 Designer: JG
 Company:
 Misc:



Total Horizontal Product Length = 05-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	164 / 0	235 / 0	355 / 0	
B1, 3-1/2"	113 / 0	184 / 0	251 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Standard Load	Unf. Area (lb/ft²)	L	00-00-00	05-10-08	11	10	26		02-00-00
2		Conc. Pt. (lbs)	L	02-00-00	02-00-00	147	147	300		n/a
3	0	Unf. Lin. (lb/ft)	L	00-00-00	05-10-08		20			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,318 ft-lbs	6,040 ft-lbs	21.8%	5	02-00-00
End Shear	751 lbs	3,985 lbs	18.9%	5	01-00-12
Total Load Defl.	L/999 (0.016")	n/a	n/a	13	02-09-08
Live Load Defl.	L/999 (0.01")	n/a	n/a	17	02-09-08
Max Defl.	0.016"	n/a	n/a	13	02-09-08
Span / Depth	7	n/a	n/a		00-00-00

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

Bearing Supports

	Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0 Post	3-1/2" x 3"	908 lbs	n/a	14.1%	Unspecified
B1 Post	3-1/2" x 3"	663 lbs	n/a	10.3%	Unspecified

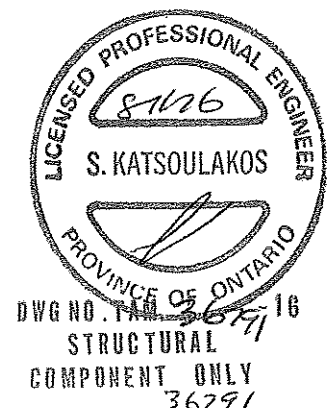
Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.

CONFORMS TO OBC 2012

2x10
 4x10
 6x10
 8x10
 10x10
 12x10
 14x10
 16x10
 18x10
 20x10
 22x10
 24x10
 26x10
 28x10
 30x10
 32x10
 34x10
 36x10
 38x10
 40x10
 42x10
 44x10
 46x10
 48x10
 50x10
 52x10
 54x10
 56x10
 58x10
 60x10
 62x10
 64x10
 66x10
 68x10
 70x10
 72x10
 74x10
 76x10
 78x10
 80x10
 82x10
 84x10
 86x10
 88x10
 90x10
 92x10
 94x10
 96x10
 98x10
 100x10

PROVIDE 4 ROWS OF 3/4" ARDOX SPIRAL NAILS @ 8" O/C FOR MULTI-PLY NAILING. MAINTAIN A MIN. 1" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS



HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

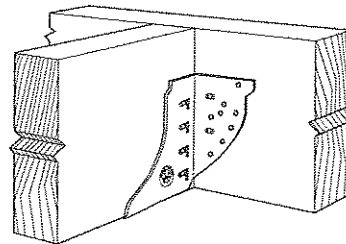
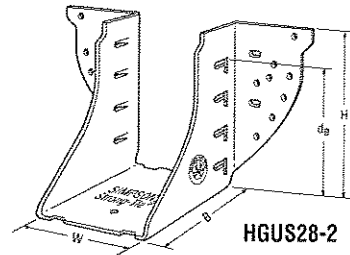
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

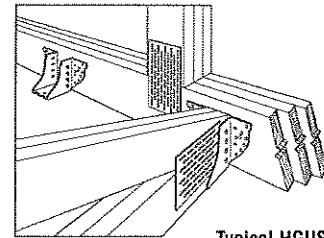
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



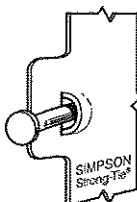
Typical HGUS Installation



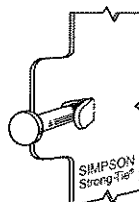
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₂ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 $\frac{1}{8}$	5 $\frac{1}{8}$	5	4 $\frac{1}{2}$	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 $\frac{1}{8}$	5 $\frac{1}{8}$	4	4 $\frac{1}{8}$	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 $\frac{1}{8}$	5 $\frac{1}{8}$	4	4 $\frac{1}{8}$	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 $\frac{1}{8}$	5 $\frac{1}{8}$	4	4 $\frac{1}{8}$	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 $\frac{1}{8}$	7 $\frac{1}{8}$	5	6 $\frac{1}{8}$	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 $\frac{1}{8}$	7 $\frac{1}{8}$	4	6 $\frac{1}{8}$	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 $\frac{1}{8}$	7 $\frac{1}{8}$	4	6 $\frac{1}{8}$	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 $\frac{1}{8}$	7 $\frac{1}{8}$	4	6 $\frac{1}{8}$	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 $\frac{1}{8}$	9 $\frac{1}{8}$	4	8 $\frac{1}{8}$	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 $\frac{1}{8}$	9 $\frac{1}{8}$	4	8 $\frac{1}{8}$	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 $\frac{1}{8}$	9 $\frac{1}{8}$	4	8 $\frac{1}{8}$	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 $\frac{1}{8}$	10 $\frac{1}{8}$	4	10 $\frac{1}{8}$	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 $\frac{1}{8}$	12 $\frac{1}{8}$	4	11 $\frac{1}{8}$	66-16d	22-16d	10130	16400	7195	11645

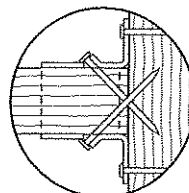
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



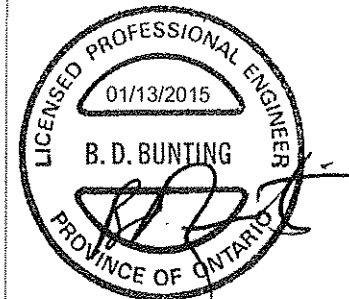
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

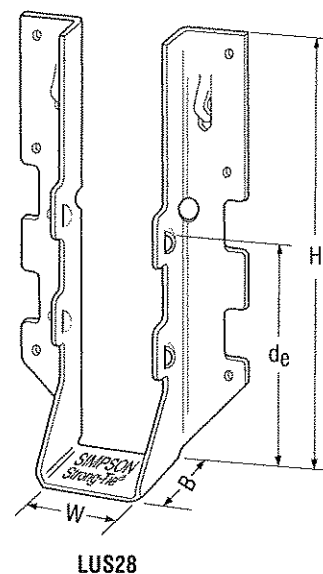
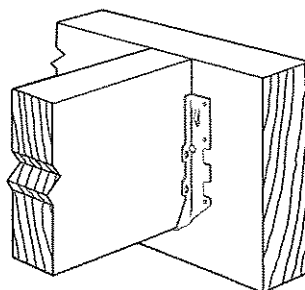
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

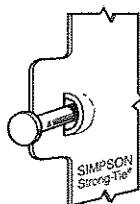
- These hangers cannot be modified.

Typical LUS Installation



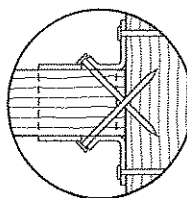
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1¾	1½	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	4-16d	2-16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

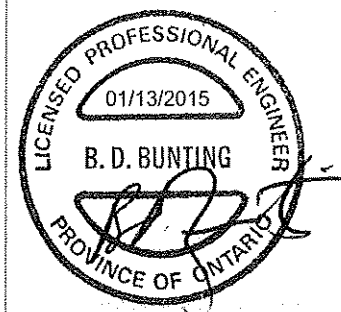


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC LUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

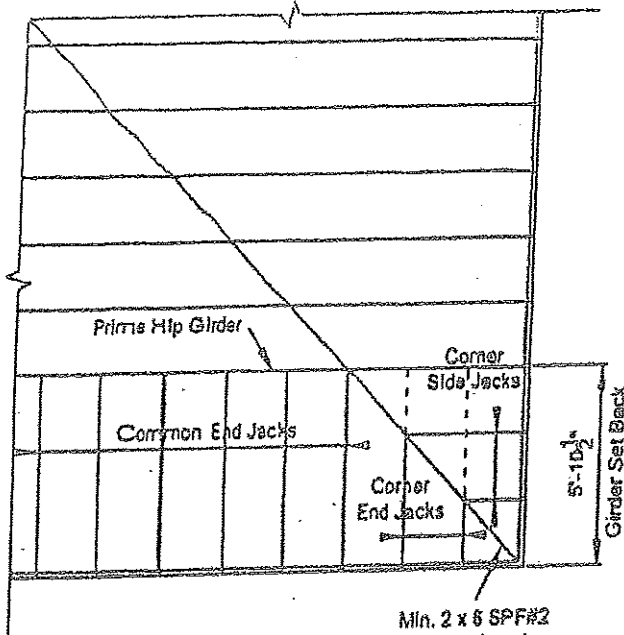
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

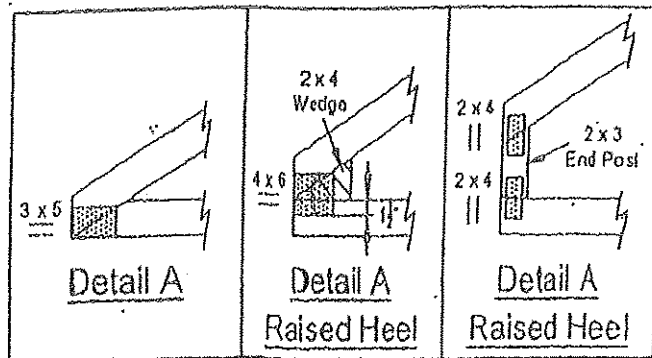
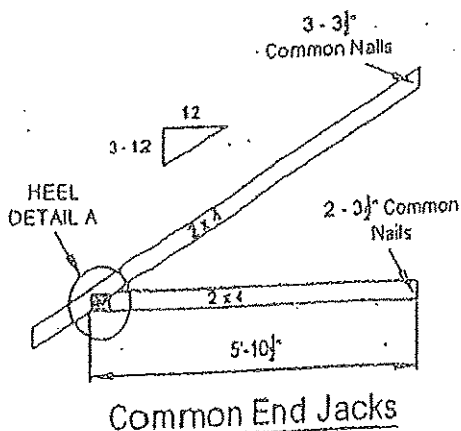
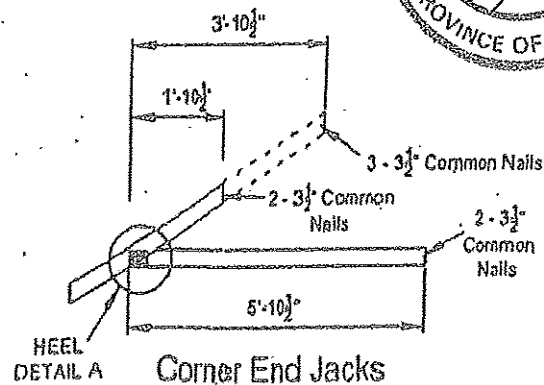
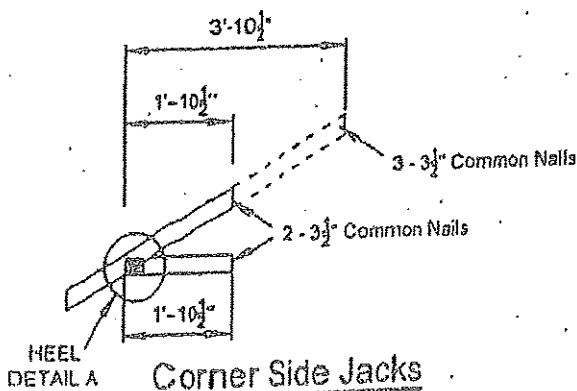
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.



45° Hip End

DWG NO T&M 3495.14
STRUCTURAL
COMPONENT ONLY



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

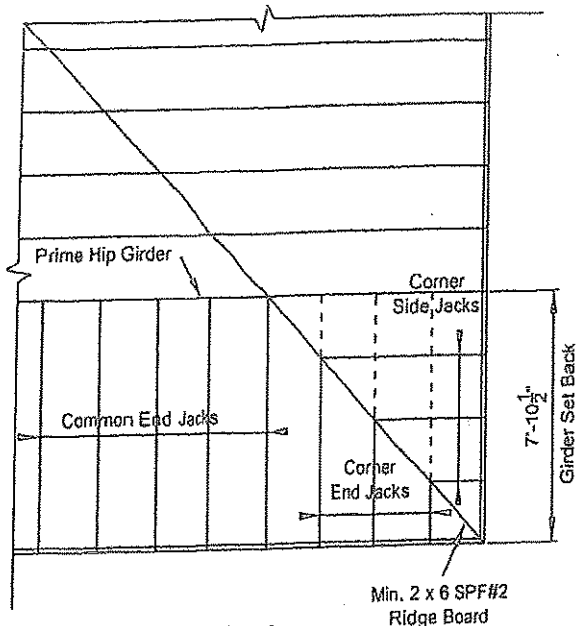
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

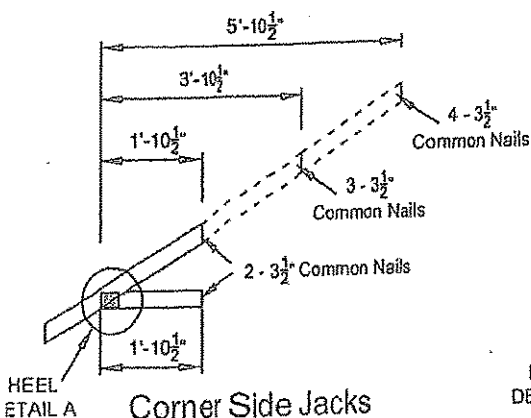
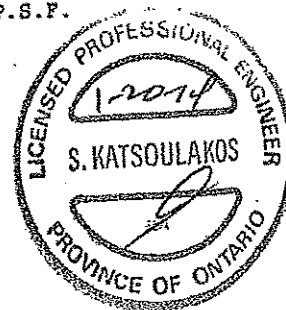
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

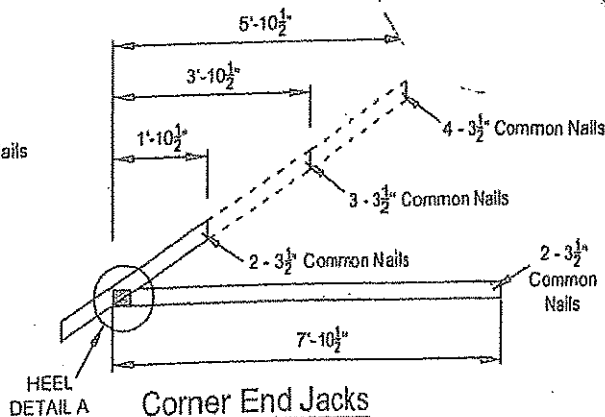


45° Hip End

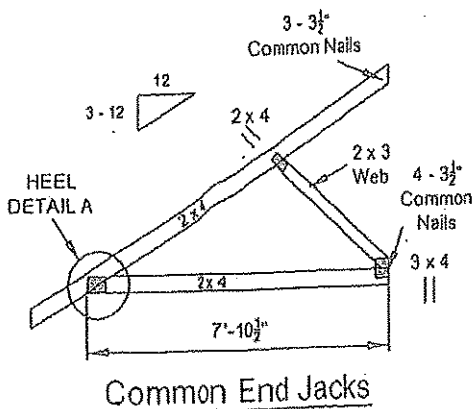
DWG NO YAM 3503.14
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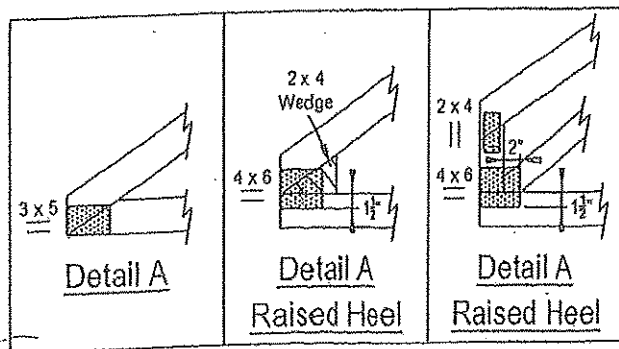
Corner Side Jacks



Corner End Jacks



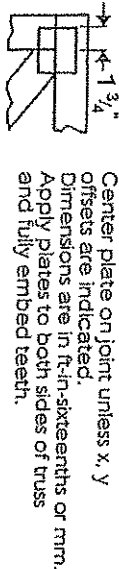
Common End Jacks



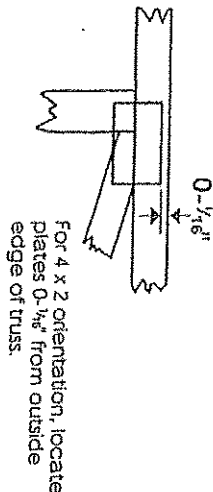
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16\" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

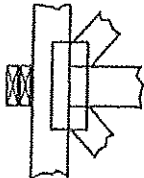
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

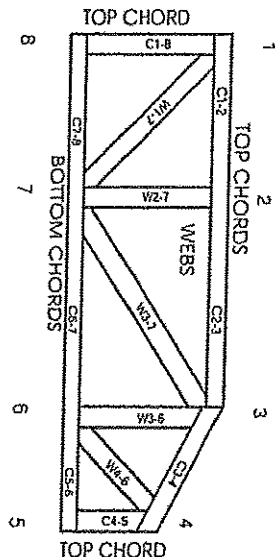
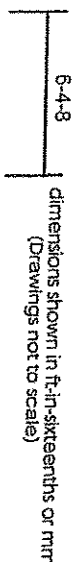
BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC:
Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89:
Design Standard for Bracing.
BCSI:
Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

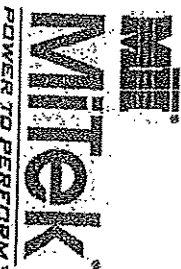
PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12891-R

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Mitek Engineering Reference Sheet MIT-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

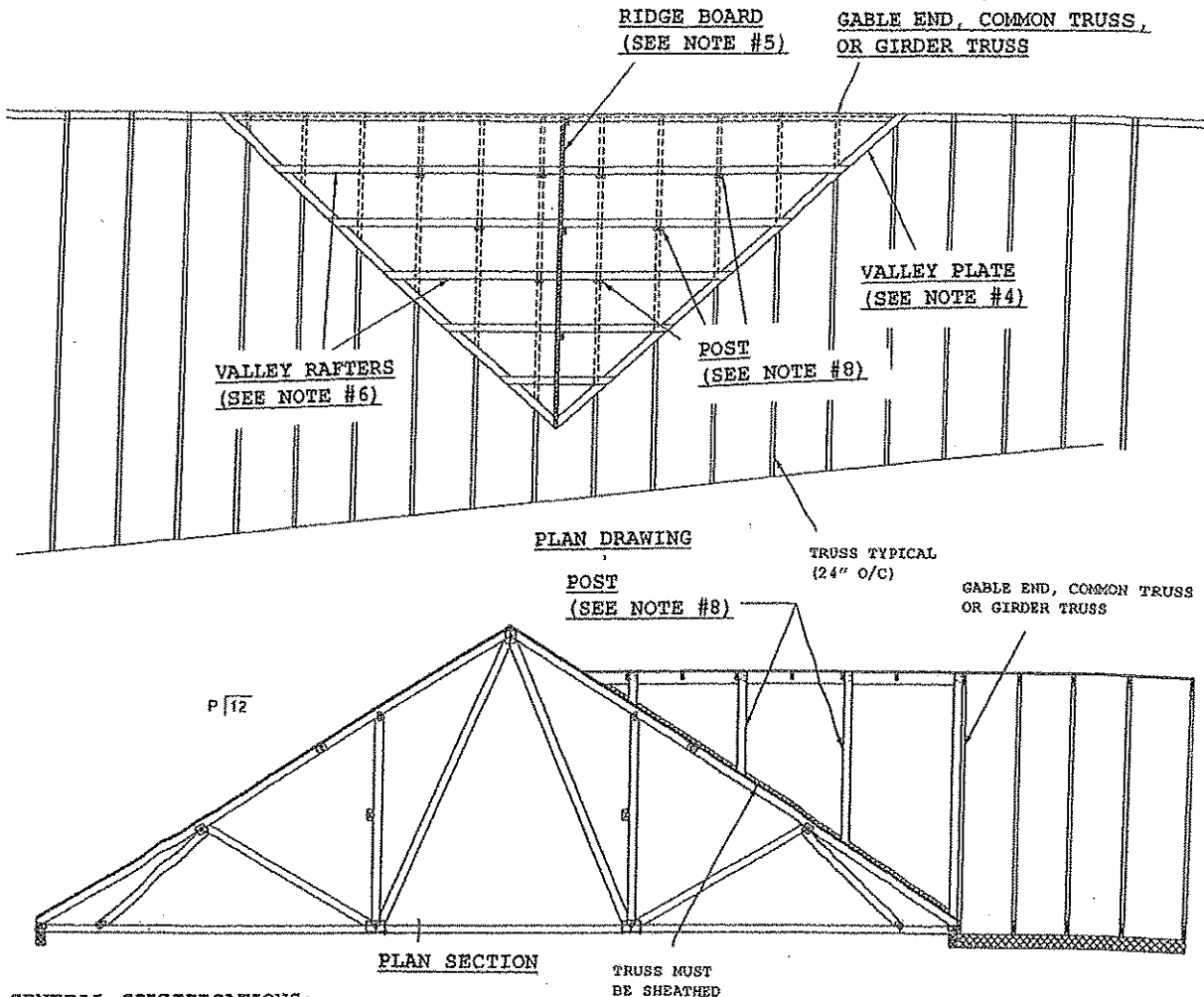
1. Additional stability bracing for truss system, e.g. diagonal or x-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pulins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

TRUSS MUST
BE SHEATHED

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.6 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

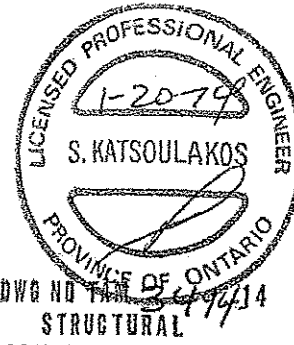


DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.