

9M-9903

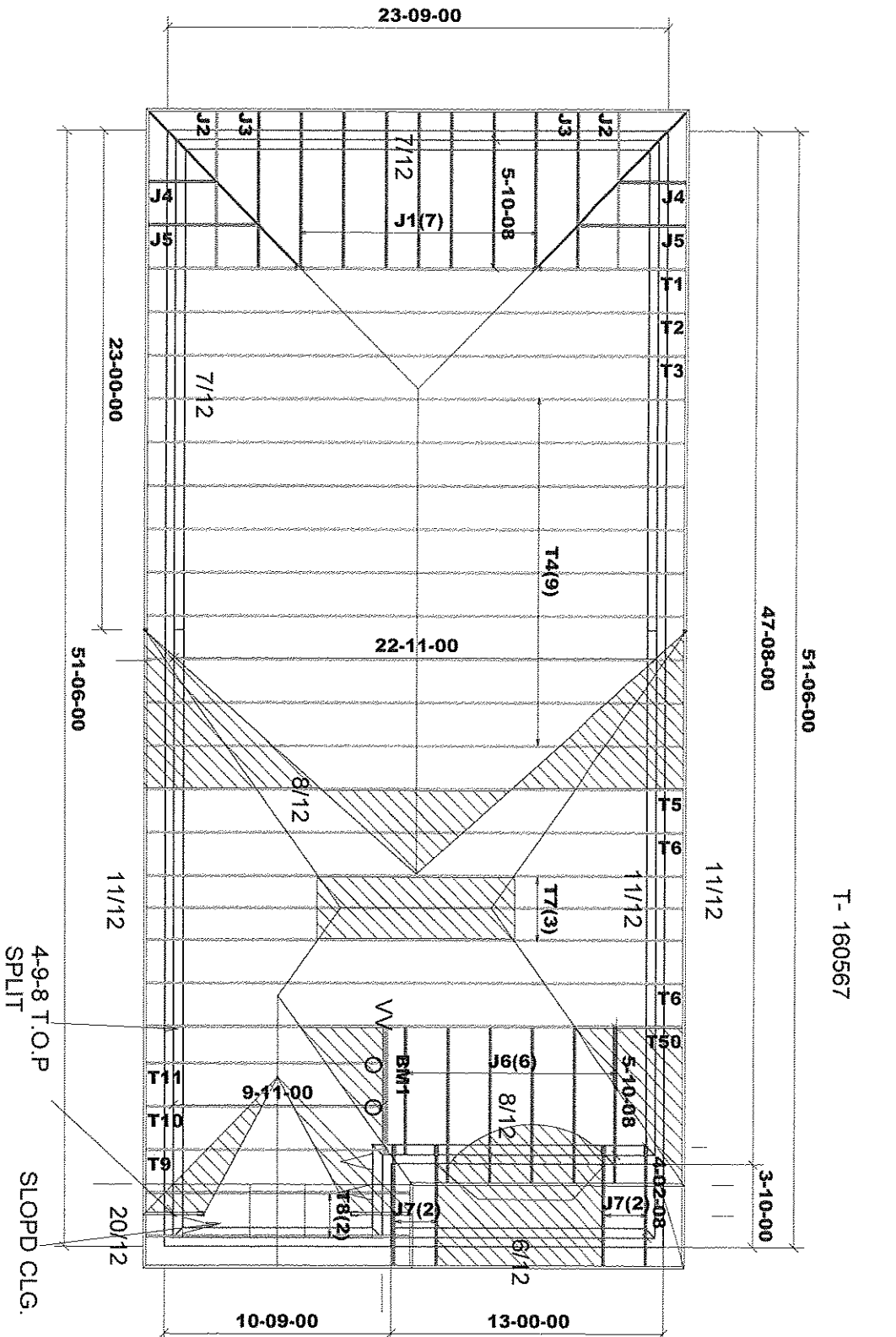


Job Track: 42181
Layout ID: 267388
Plan Log: 86179

Builder / Location: GREENPARK HOMES / VAUGHAN
Project: STARTIME HOME LTD
Date: 7/22/2016 Designer: JG

Model / Elevation: BRIDGEFORD 2 / 1

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12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa
T.C DEAD 3 PSF
B.C LIVE 10.5 PSF
B.C DEAD 7 PSF

NOTES
CONVENTIONAL ☒ FRAMING

HARDWARE
LUS24(O)
LUS26-2 (VV)
BM1:2-2X10



Delivery Shiplist

DATE	07/22/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267388 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGHAN SUB-BUILDER:
 MODEL: BRIDGEFORD 2 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	110.11 68.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	9	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	844.47 519.03		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1	T50 HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	117.82 74.00		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	202.54 131.00		
	3	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	344.58 221.01		
	2	T8 ROOF	20.00 18.00	09-11-00	11-01-00	2 X 4	2 X 4	01-03-08 01-03-08	02-09-13 02-09-13	156.56 103.34		
	1	T9 HIP	11.00 0.00	09-11-00	03-02-00	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	45.41 30.67		
	1	T10 HIP	11.00 0.00	09-11-00	05-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	50.16 32.67		
	1	T11 COMMON	11.00 0.00	09-11-00	06-03-10	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	46.61 29.33		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	6	J6 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	106.98 64.02		



Delivery Shiplist

DATE	07/22/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267388 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGHAN SUB-BUILDER:
 MODEL: BRIDGEFORD 2 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	J7 JACK-OPEN	6.00 0.00	04-02-08	02-05-07	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 00-04-11	47.92 32.00		

TOTAL # TRUSS= 49.00

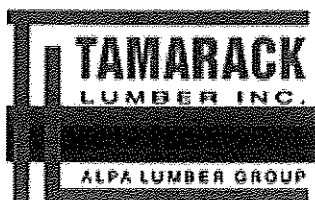
TOTAL BFT OF ALL TRUSSES=

1623.26 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2579.48 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 3.00



Delivery Shiplist

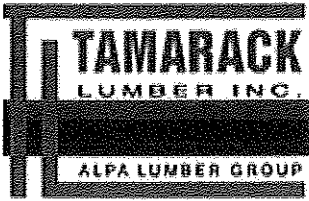
DATE	07/22/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267390 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 2 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	110.11 68.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	9	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	844.47 519.03		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-05-00 01-03-08	01-09-01 01-09-01	202.92 131.00		
	3	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	344.58 221.01		
	2	T11 COMMON	11.00 0.00	09-11-00	06-03-10	2 X 4	2 X 4	01-03-08 00-00-00	00-00-00 00-00-00	89.24 56.00		
	1	T12 HIP GIRDER	11.00 0.00	09-11-00	05-03-11	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	51.44 34.00		
	2	T13 COMMON	11.00 0.00	12-07-08	08-06-00	2 X 4	2 X 4	01-03-08 00-00-00	02-11-01 02-06-00	133.32 85.34		
	1	G13 GABLE	11.00 0.00	12-07-08	08-06-00	2 X 4	2 X 4	01-03-08 00-00-00	02-11-01 02-06-00	69.82 46.17		
	1	T14 HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	02-11-01 01-09-01	116.00 73.00		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	4	J9 JACK-OPEN	5.00 0.00	04-02-08	02-01-01	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 00-04-07	46.92 32.00		



Delivery Shiplist

DATE	07/22/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267390 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: BRIDGEFORD 2 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J10 JACK-OPEN	11.00 0.00	03-10-08	05-03-11	2 X 4	2 X 4	01-03-08 00-00-00	01-09-01 05-03-11	34.70 22.66		
	2	J11 JACK-OPEN	11.00 0.00	03-10-08	03-04-11	2 X 4	2 X 4	01-03-08 -02-01-01	01-09-01 00-03-08	28.24 18.66		
	2	J12 JACK-OPEN	11.00 0.00	01-10-08	03-04-11	2 X 4	2 X 4	01-03-08 -00-01-01	01-09-01 00-03-08	20.82 15.00		

TOTAL # TRUSS= 50.00

TOTAL BFT OF ALL TRUSSES=

1640.06 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2598.90 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
4	Hangers	LUS24	

TOTAL # ITEMS= 5.00



Delivery Shiplist

DATE	07/22/16
SALES REP	Mario

JOB TRACK: 42181	LAYOUT ID: 267396	LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH	SUB-BUILDER:	
MODEL: BRIDGEFORD 2	ELEVATION: 3	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	7.00 0.00	22-11-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	110.11 68.00		
	1	T2 HIP	7.00 0.00	22-11-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	95.39 59.67		
	1	T3 HIP	7.00 0.00	22-11-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	101.15 64.00		
	9	T4 COMMON	7.00 0.00	22-11-00	07-11-10	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	844.47 519.03		
	1	T5 HIP	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	101.16 63.83		
	1	T50Z HIP GIRDER	11.00 0.00	22-11-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-09-01 01-09-01	119.72 75.33		
	2	T6 HIP	11.00 0.00	22-11-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	202.54 131.00		
	3	T7 HIP	11.00 0.00	22-11-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	344.58 221.01		
	2	T11 COMMON	11.00 0.00	09-11-00	06-03-10	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	93.22 58.66		
	1	T12 HIP GIRDER	11.00 0.00	09-11-00	05-03-11	2 X 4	2 X 4	01-03-08 01-03-08	01-09-01 01-09-01	51.44 34.00		
	1	T20 ROOF	20.00 15.00	09-02-00	10-05-08	2 X 4	2 X 4	01-03-08 01-03-08	02-09-13 02-09-13	75.38 49.83		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	2	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	23.80 14.66		
	2	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	29.08 18.66		
	2	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	14.68 9.34		
	2	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	19.96 13.34		
	1	J6 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	22.44 13.67		
	5	J6A JACK-OPEN	8.00 0.00	05-05-08	05-03-13	2 X 4	2 X 4	00-00-00 00-00-00	01-08-02 05-03-13	96.45 62.50		



Delivery Shiplist

DATE	07/22/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267396 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGHAN SUB-BUILDER:
 MODEL: BRIDGEFORD 2 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	J7 JACK-OPEN	6.00 0.00	04-02-08	02-05-07	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 00-04-11	47.92 32.00		
	2	J10 JACK-OPEN	11.00 0.00	03-10-08	05-03-11	2 X 4	2 X 4	01-03-08 00-00-00	01-09-01 05-03-11	34.70 22.66		
	2	J11 JACK-OPEN	11.00 0.00	03-10-08	03-04-11	2 X 4	2 X 4	01-03-08 -02-01-01	01-09-01 00-03-08	28.24 18.66		
	2	J12 JACK-OPEN	11.00 0.00	01-10-08	03-04-11	2 X 4	2 X 4	01-03-08 -00-01-01	01-09-01 00-03-08	20.82 15.00		

TOTAL # TRUSS= 54.00

TOTAL BFT OF ALL TRUSSES=

1639.54 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2598.35 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 8.00

267388

T1

QUANTITY
1PLY
1JOB DESC. 42181
TRUSS DESC.

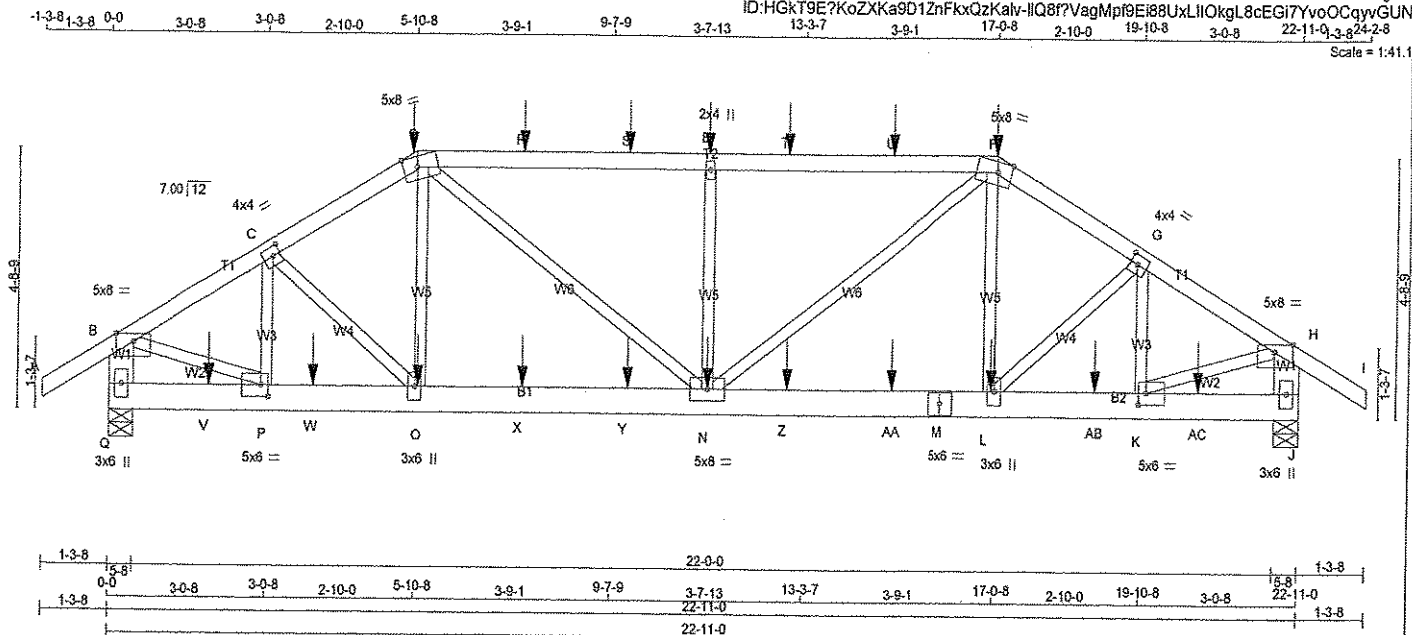
DRWG NO.

Tamarack Roof Truss, Burlington

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Scale = 1:41.1



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - F	2x4	DRY	2100F 1.8E	SPF
F - I	2x4	DRY	2100F 1.8E	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.25
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.25
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	1.75
L	BMVW-t	MT20	3.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	8.0		
O	BMVW-t	MT20	3.0	6.0		
P	BMVW-t	MT20	5.0	6.0	2.50	1.75
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 429.0 lbs FACTORED DOWN AT 5'-10"-8, 101.4 lbs FACTORED DOWN AT 7'-11"-4, 101.4 lbs FACTORED DOWN AT 9'-11"-4, 101.4 lbs FACTORED DOWN AT 11'-5"-8, 101.4 lbs FACTORED DOWN AT 12'-11"-12, AND 101.4 lbs FACTORED DOWN AT 14'-11"-12, AND 429.0 lbs FACTORED DOWN AT 17'-0"-8 ON TOP CHORD, AND 75.4 lbs FACTORED DOWN AT 1'-11"-4, 70.9 lbs FACTORED DOWN AT 3'-11"-4, 70.9 lbs FACTORED DOWN AT 5'-11"-4, 70.9 lbs FACTORED DOWN AT 7'-11"-4, 70.9 lbs FACTORED DOWN AT 9'-11"-4, 70.9 lbs FACTORED DOWN AT 11'-5"-8, 70.9 lbs FACTORED DOWN AT 12'-11"-12, 70.9 lbs FACTORED DOWN AT 14'-11"-12, 70.9 lbs FACTORED DOWN AT 16'-11"-12, AND 70.9 lbs FACTORED DOWN AT 18'-11"-12, AND 75.4 lbs FACTORED DOWN AT 20'-11"-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
Q	2307	0	2307	0
J	2307	0	2307	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
Q	1887	1059 / 0	422 / 0	0 / 0
J	1887	1059 / 0	422 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0 / 29	-84.3	-84.3	0.08 (1)	10.00	P-C	-615 / 0
B-C	-2637 / 0	-84.3	-84.3	0.10 (1)	5.04	C-D	0 / 288
C-D	-2609 / 0	-84.3	-84.3	0.10 (1)	4.85	D-E	0 / 329
D-E	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	E-F	0 / 986
E-F	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	F-G	0 / 986
F-G	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	G-H	0 / 986
G-H	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	H-I	0 / 986
H-I	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	I-J	0 / 986
I-J	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	J-K	0 / 986
J-K	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	K-L	0 / 986
K-L	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	L-M	0 / 986
L-M	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	M-N	0 / 986
M-N	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	N-O	0 / 986
N-O	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	O-P	0 / 986
O-P	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	P-Q	0 / 986
P-Q	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	Q-R	0 / 986
Q-R	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	R-S	0 / 986
R-S	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	S-T	0 / 986
S-T	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	T-U	0 / 986
T-U	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	U-V	0 / 986
U-V	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	V-W	0 / 986
V-W	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	W-X	0 / 986
W-X	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	X-Y	0 / 986
X-Y	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	Y-Z	0 / 986
Y-Z	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	Z-AA	0 / 986
Z-AA	-3281 / 0	-84.3	-84.3	0.61 (1)	3.97	AA-M	0 / 2502
AA-M	0 / 2502	-28.0	-28.0	0.09 (2)	10.00	M-L	0 / 2502
M-L	0 / 2502	-28.0	-28.0	0.09 (2)	10.00	L-AB	0 / 2286
L-AB	0 / 2286	-28.0	-28.0	0.38 (1)	10.00	AB-K	0 / 2286
AB-K	0 / 2286	-28.0	-28.0	0.38 (1)	10.00	K-AC	0 / 0
K-AC	0 / 0	-28.0	-28.0	0.09 (2)	10.00	AC-J	0 / 0
AC-J	0 / 0	-28.0	-28.0	0.09 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX	MAX+	FACE	DIR.	TYPE
D	5'-10"-8	-429	-429	FRONT	VERT	TOTAL
E	11'-5"-8	-101	-101	FRONT	VERT	TOTAL
F	17'-0"-8	-429	-429	FRONT	VERT	TOTAL
L	16'-11"-12	-41	-71	FRONT	VERT	TOTAL
N	11'-5"-8	-41	-71	FRONT	VERT	TOTAL
O	5'-11"-4	-41	-71	FRONT	VERT	TOTAL
R	7'-11"-4	-101	-101	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.61 (D-E:1), BC=0.40 (N-O:1), WB=0.59 (B-P:1), SS=0.37 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

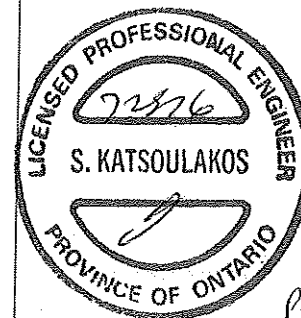
PLATE ROTATION TOL. = 5.0 Deg.

JSH GRIP= 0.90 (P) (INPUT = 0.90)
JSH METAL= 0.57 (K) (INPUT = 1.00)

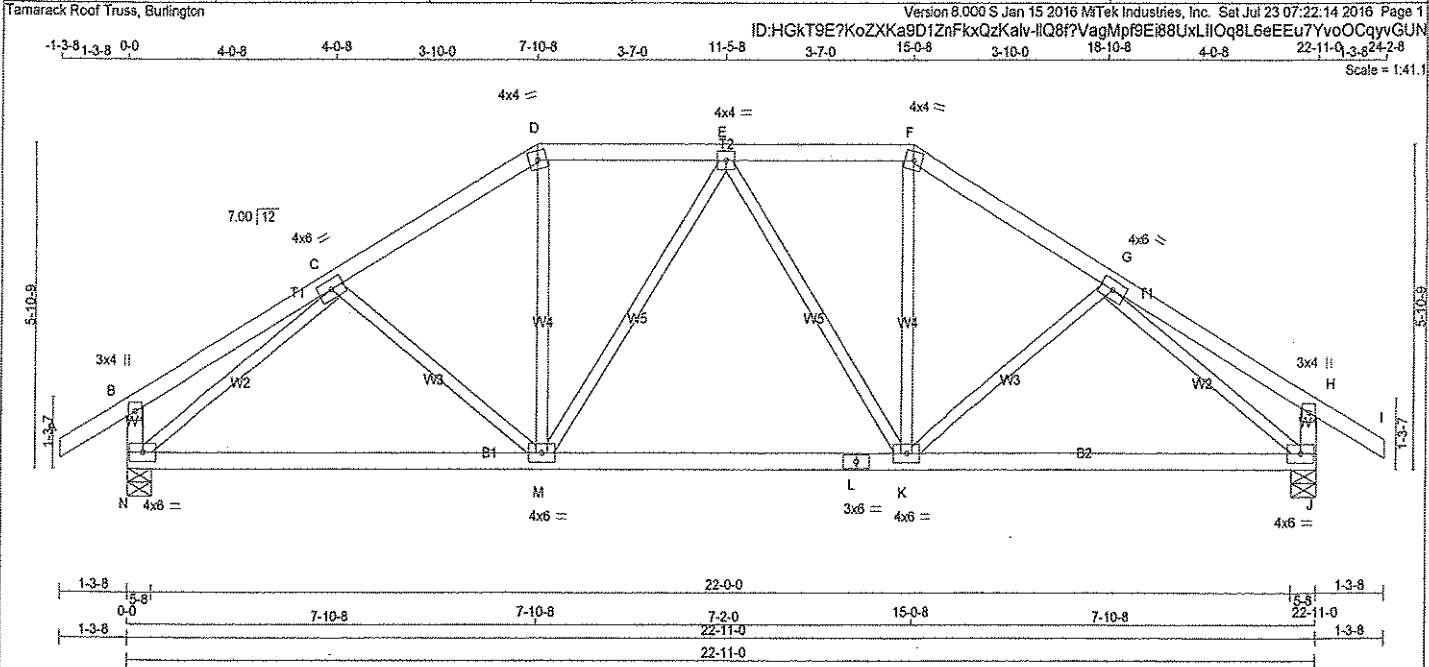
DRWG NO. TAM34656-16
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
S	9-11-4	-101	-101	---	FRONT	VERT
T	12-11-12	-101	-101	---	FRONT	VERT
U	14-11-12	-101	-101	---	FRONT	VERT
V	1-11-4	-40	-75	---	FRONT	VERT
W	3-11-4	-41	-71	---	FRONT	VERT
X	7-11-4	-41	-71	---	FRONT	VERT
Y	9-11-4	-41	-71	---	FRONT	VERT
Z	12-11-12	-41	-71	---	FRONT	VERT
AA	14-11-12	-41	-71	---	FRONT	VERT
AB	18-11-12	-41	-71	---	FRONT	VERT
AC	20-11-12	-40	-75	---	FRONT	VERT
						TOTAL



DWG NO. TAM 34656-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP		
N	1402	0	1402	0	5-8	1-8
J	1402	0	1402	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
N	1134	656 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0
J	1134	656 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING										
TOTAL LOAD CASES: (4)										
CHORDS					WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO			FR-TO				
A-B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	C-M	-84 / 99	0.04 (1)		
B-C	0 / 18	-84.3	-84.3	0.19 (1)	10.00	M-D	0 / 462	0.10 (1)		
C-D	-1428 / 0	-84.3	-84.3	0.17 (1)	5.30	M-E	-213 / 0	0.17 (1)		
D-E	-1225 / 0	-84.3	-84.3	0.14 (1)	5.65	E-K	-213 / 0	0.17 (1)		
E-F	-1225 / 0	-84.3	-84.3	0.14 (1)	5.65	K-F	0 / 462	0.10 (1)		
F-G	-1428 / 0	-84.3	-84.3	0.17 (1)	5.30	K-G	-84 / 99	0.04 (1)		
G-H	0 / 18	-84.3	-84.3	0.19 (1)	10.00	N-C	-1665 / 0	0.71 (1)		
H-I	0 / 29	-84.3	-84.3	0.11 (1)	10.00	G-J	-1665 / 0	0.71 (1)		
N-B	-245 / 0	0.0	0.0	0.03 (1)	7.81					
J-H	-245 / 0	0.0	0.0	0.03 (1)	7.81					
N-M	0 / 1281	-28.0	-28.0	0.51 (2)	10.00					
M-L	0 / 1337	-28.0	-28.0	0.53 (2)	10.00					
L-K	0 / 1337	-28.0	-28.0	0.53 (2)	10.00					
K-J	0 / 1281	-28.0	-28.0	0.51 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.6	PSF
BOT CH.	LL	PSF
LL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/972 (0.28")

CSI: TC=0.19 (G-J-1), BC=0.53 (K-M-2), WB=0.71 (G-J-1), SSI=0.18 (J-K-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 822 2284 1655

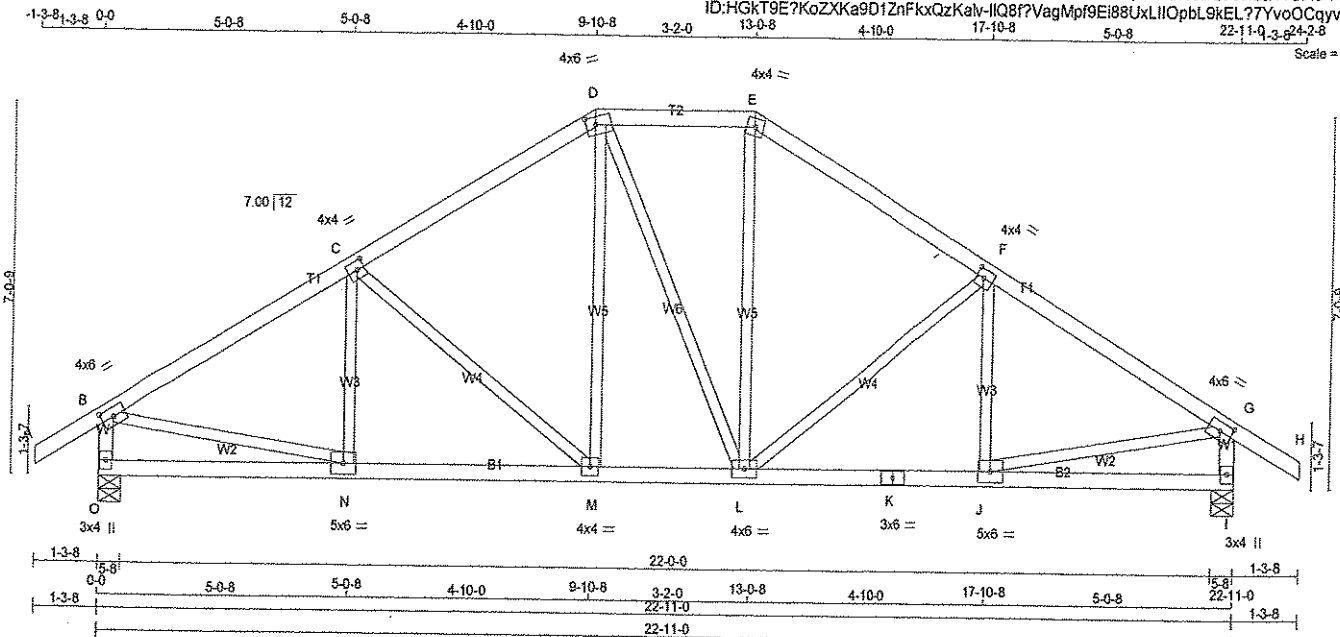
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.55 (L) (INPUT = 1.00)



DWG NO. TAM 34657-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 101 lb
 (M/L)

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF

A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	6.0	2.00	3.00
C	TMVW-I	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	4.0	6.0	1.75	2.25
E	TTVW-m	MT20	4.0	4.0		
F	TMVW-I	MT20	4.0	4.0	2.00	1.75
G	TMVW-I	MT20	4.0	6.0	2.00	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-I	MT20	5.0	6.0		
K	BS-I	MT20	3.0	6.0		
L	BMVW-I	MT20	4.0	6.0		
M	BMVW-I	MT20	4.0	4.0		
N	BMVW-I	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
O	1402	0	1402	0	5-8	1-8
I	1402	0	1402	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX MIN COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1134	658 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0
I	1134	658 / 0	241 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 29	-84.3 -84.3	0.11 (1)	10.00	N-C	-106 / 152	0.03 (3)
B-C	-1562 / 0	-84.3 -84.3	0.29 (1)	4.99	C-M	-381 / 0	0.27 (1)
C-D	-1268 / 0	-84.3 -84.3	0.28 (1)	5.41	M-D	0 / 345	0.08 (2)
D-E	-1079 / 0	-84.3 -84.3	0.12 (1)	5.95	D-L	0 / 3	0.00 (2)
E-F	-1269 / 0	-84.3 -84.3	0.28 (1)	5.41	L-E	0 / 348	0.08 (2)
F-G	-1562 / 0	-84.3 -84.3	0.29 (1)	4.98	L-F	-379 / 0	0.27 (1)
G-H	0 / 29	-84.3 -84.3	0.11 (1)	10.00	J-F	-108 / 150	0.03 (3)
O-B	-1343 / 0	0.0 0.0	0.14 (1)	6.99	B-N	0 / 1396	0.31 (1)
I-G	-1343 / 0	0.0 0.0	0.14 (1)	6.99	J-G	0 / 1395	0.31 (1)
O-N	0 / 0	-28.0 -28.0	0.18 (3)	10.00			
N-M	0 / 1370	-28.0 -28.0	0.33 (2)	10.00			
M-L	0 / 1077	-28.0 -28.0	0.23 (1)	10.00			
L-K	0 / 1370	-28.0 -28.0	0.33 (2)	10.00			
K-J	0 / 1370	-28.0 -28.0	0.33 (2)	10.00			
J-I	0 / 0	-28.0 -28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.76")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.29 (B-C:1), BC=0.33 (M-N:2), WB=0.31 (B-N:1), SSI=0.18 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 622 2284 1656

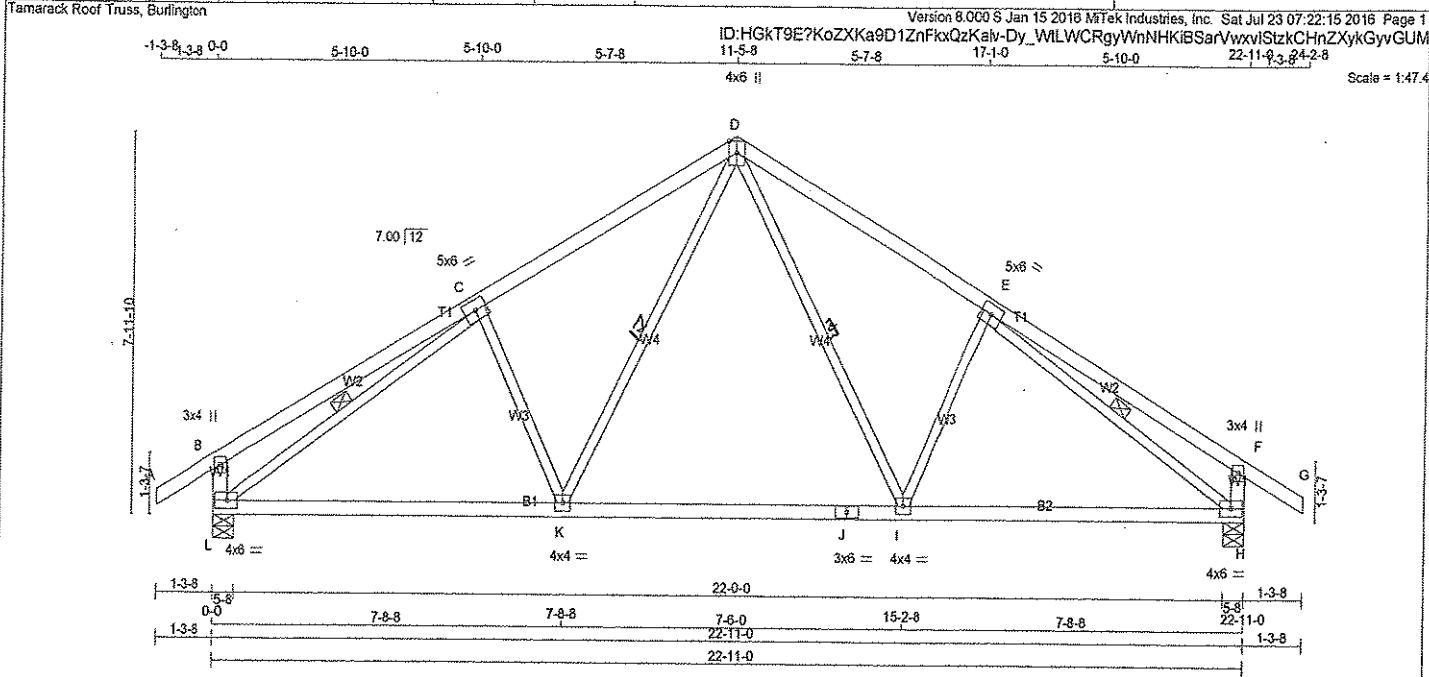
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
 JSI METAL= 0.43 (B) (INPUT = 1.00)



DRWG NO. TAW34658-10
STRUCTURAL COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE
LUMBER
DESCR.
SPF

A - D 2x4 DRY No.2 SPF
D - G 2x4 DRY No.2 SPF
L - B 2x4 DRY No.2 SPF
H - F 2x4 DRY No.2 SPF
L - J 2x4 DRY No.2 SPF
J - H 2x4 DRY No.2 SPF
ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X

B TMV+p MT20 3.0 4.0
C TMWW-t MT20 5.0 6.0
D TTWW+p MT20 4.0 6.0
E TMWW-t MT20 5.0 6.0
F TMV+p MT20 3.0 4.0
H BMWW-t MT20 4.0 6.0
I BMWW-t MT20 4.0 4.0
J BS-t MT20 3.0 6.0
K BMWW-t MT20 4.0 4.0
L BMWW-t MT20 4.0 6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
FACTORED GROSS REACTION
MAXIMUM FACTORED GROSS REACTION
INPUT BRG
REQRD BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX
L 1402 0 1402 0 0 5-8 1-8
H 1402 0 1402 0 0 5-8 1-8

UNFACTORED REACTIONS
1ST LCASE MAX/MIN COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
L 1134 656 / 0 241 / 0 0 / 0 0 / 0 237 / 0 0 / 0
H 1134 656 / 0 241 / 0 0 / 0 0 / 0 237 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW
LOADING
TOTAL LOAD CASES: (4)

CHORDS
MEMB. MAX. FACTORED FORCE (LBS)
FACTORED VERT. LOAD (PLF)
LC1 MAX
MAX. UNBRACED LENGTH

WEBS
MEMB. MAX. FACTORED FORCE (LBS)
MAX. FACTORED CSI (LC)

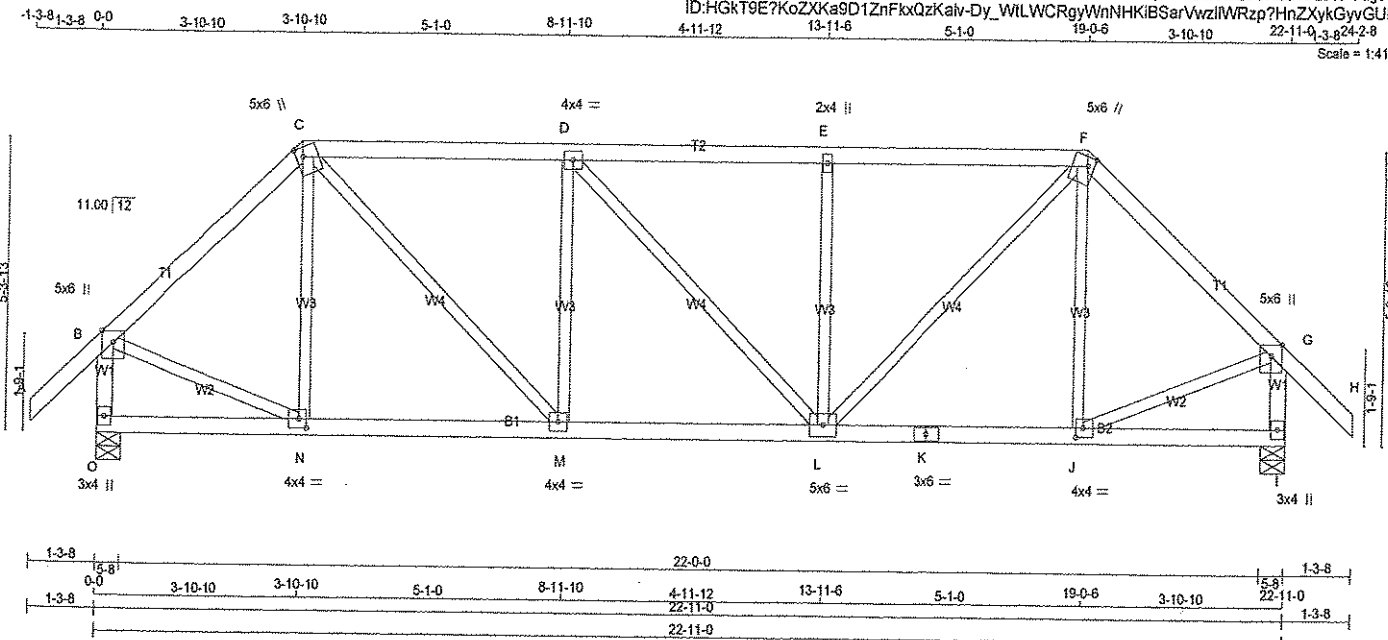
FR-TO FROM TO
A-B 0 / 29 -84.3 -84.3 0.11 (1) 10.00
B-C 0 / 29 -84.3 -84.3 0.45 (1) 10.00
C-D -1433 / 0 -84.3 -84.3 0.39 (1) 5.02
D-E -1433 / 0 -84.3 -84.3 0.39 (1) 5.02
E-F 0 / 29 -84.3 -84.3 0.45 (1) 10.00
F-G 0 / 29 -84.3 -84.3 0.11 (1) 10.00
L-B -299 / 0 0.0 0.0 0.03 (1) 7.81
H-F -299 / 0 0.0 0.0 0.03 (1) 7.81

L-K 0 / 1347 -28.0 -28.0 0.53 (2) 10.00
K-J 0 / 957 -28.0 -28.0 0.49 (2) 10.00
J-I 0 / 957 -28.0 -28.0 0.49 (2) 10.00
I-H 0 / 1347 -28.0 -28.0 0.53 (2) 10.00

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF
SPACING = 24.0 IN./C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010
THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011
(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")
CSI: TC=0.45 (E-F:1), BC=0.53 (H-I:2), WB=0.57 (E-H:1), SSI=0.20 (D-E:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10
COMPANION LIVE LOAD FACTOR = 0.50
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAW 34659-10
STRUCTURAL COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMVW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	4.0	2.00	1.75
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1404	0	1404	0	0	5-8	1-8
I	1404	0	1404	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	N-C	-159 / 89	0.07 (1)
B-C	-1152 / 0	-84.3	-84.3	0.25 (1)	5.63	C-M	0 / 849	0.19 (1)
C-D	-1448 / 0	-84.3	-84.3	0.33 (1)	5.05	M-D	-456 / 0	0.19 (1)
D-E	-1447 / 0	-84.3	-84.3	0.33 (1)	5.05	D-L	-2 / 0	0.00 (1)
E-F	-1447 / 0	-84.3	-84.3	0.33 (1)	5.06	L-E	-456 / 0	0.19 (1)
F-G	-1152 / 0	-84.3	-84.3	0.25 (1)	5.63	L-F	0 / 846	0.19 (1)
G-H	0 / 40	-84.3	-84.3	0.12 (1)	10.00	J-F	-158 / 89	0.07 (1)
O-B	-1362 / 0	0.0	0.0	0.15 (1)	6.96	N	0 / 898	0.20 (1)
I-G	-1362 / 0	0.0	0.0	0.15 (1)	6.96	J-G	0 / 898	0.20 (1)
O-N	0 / 0	-28.0	-28.0	0.13 (2)	10.00			
N-M	0 / 843	-28.0	-28.0	0.23 (2)	10.00			
M-L	0 / 1448	-28.0	-28.0	0.30 (1)	10.00			
L-K	0 / 843	-28.0	-28.0	0.23 (2)	10.00			
K-J	0 / 843	-28.0	-28.0	0.23 (2)	10.00			
J-I	0 / 0	-28.0	-28.0	0.13 (2)	10.00			

TOTAL WEIGHT = 101 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.76")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.33 (C-D:1), BC=0.30 (L-M:1), WB=0.20 (G-J:1), SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

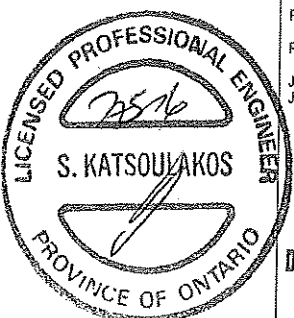
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 622 2284 1656

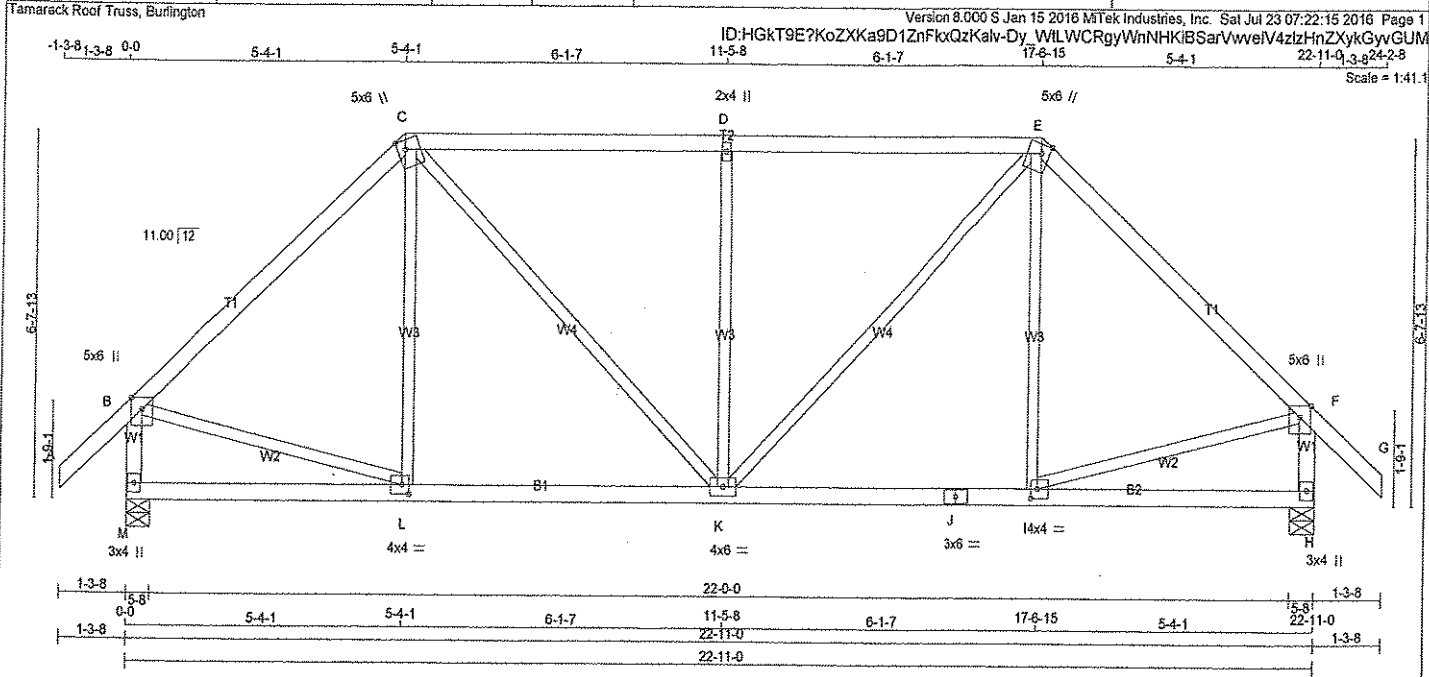
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
 JSI METAL= 0.32 (J) (INPUT = 1.00)



DWG NO. FAM34660-10
 STRUCTURAL
 COMPONENT ONLY



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS			
A - C	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQD
C - E	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG
E - G	2x4	DRY	No.2	JT VERT	DOWN	HORZ	UPLIFT
M - B	2x4	DRY	No.2	M 1404	0	1404	0
H - F	2x4	DRY	No.2	H 1404	0	1404	0
M - J	2x4	DRY	No.2				
J - H	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2	UNFACTORED REACTIONS			
EXCEPT				1ST LCASE	MAX/MIN	COMPONENT REACTIONS	
DRY: SEASONED LUMBER.				JT COMBINED	SNOW	LIVE	PERM.LIVE
				M 1136	658 / 0	241 / 0	0 / 0
				H 1136	658 / 0	241 / 0	0 / 0

TOTAL WEIGHT = 2 X 101 = 203 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

PLATES (table is in inches)				CHORDS				WEBS			
JT	TYPE	PLATES	W	LEN	Y	X	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
B	TMVW+p	MT20	5.0	6.0	Edge		A-B	0 / 40	-84.3	-84.3	0.12 (1)
C	TTWW+m	MT20	5.0	6.0	2.00	1.75	B-C	-1154 / 0	-84.3	-84.3	0.49 (1)
D	TMVW+w	MT20	2.0	4.0			C-D	-1230 / 0	-84.3	-84.3	0.59 (1)
E	TTWW+m	MT20	5.0	6.0	2.00	1.75	D-E	-1230 / 0	-84.3	-84.3	0.59 (1)
F	TMVW+p	MT20	5.0	6.0	Edge		E-F	-1154 / 0	-84.3	-84.3	0.49 (1)
H	BMV1+p	MT20	3.0	4.0			F-G	0 / 40	-84.3	-84.3	0.12 (1)
I	BMVW-t	MT20	4.0	4.0	2.00	1.50	M-B	-1345 / 0	0.0	0.0	0.15 (1)
J	BS-t	MT20	3.0	8.0			H-F	-1345 / 0	0.0	0.0	0.15 (1)
K	BMVW-t	MT20	4.0	4.0	2.00	1.50	M-L	0 / 0	-28.0	-28.0	0.22 (3)
L	BMVW-t	MT20	4.0	4.0	2.00	1.50	L-K	0 / 848	-28.0	-28.0	0.32 (2)
M	BMV1+p	MT20	3.0	4.0			K-J	0 / 848	-28.0	-28.0	0.32 (2)
							J-I	0 / 848	-28.0	-28.0	0.32 (2)
							I-H	0 / 0	-28.0	-28.0	0.22 (3)

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TOTAL LOAD CASES: (4)

CHORDS

WEBS

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.08")

CSI: TC=0.59 (D-E:1), BC=0.32 (I-K:2), WB=0.46 (D-K:1), SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

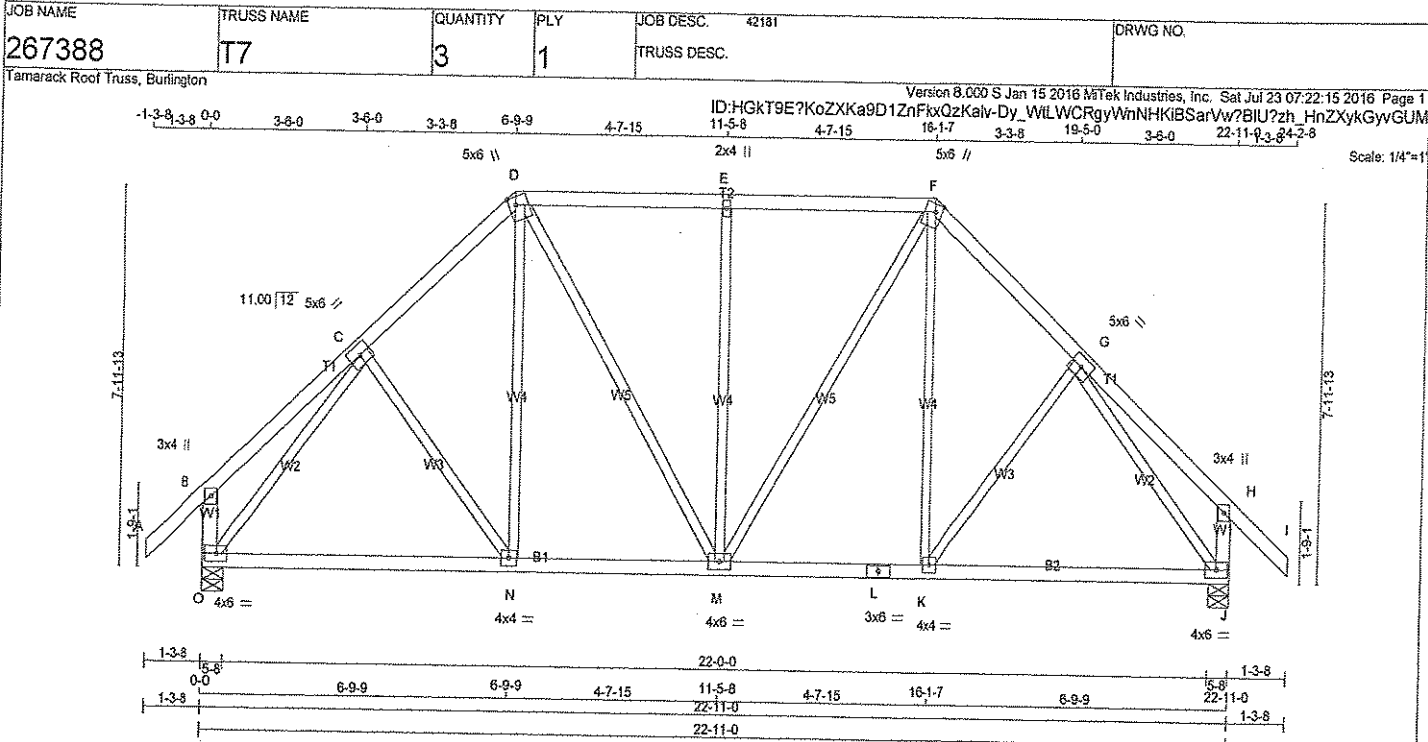
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.32 (L) (INPUT = 1.00)

DWG NO. TAM 34661 -16
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQ'D BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
O	1404	0	1404	0	0	5-8	1-8	
J	1404	0	1404	0	0	5-8	1-8	

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1136	658 / 0	241 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	C-N	-7 / 93	0.02 (3)
B-C	0 / 21	-84.3	-84.3 0.15 (1)	10.00	N-D	0 / 298	0.07 (2)
C-D	-1133 / 0	-84.3	-84.3 0.13 (1)	5.84	D-M	0 / 323	0.07 (1)
D-E	-987 / 0	-84.3	-84.3 0.24 (1)	5.99	M-E	-475 / 0	0.59 (1)
E-F	-987 / 0	-84.3	-84.3 0.24 (1)	5.99	M-F	0 / 323	0.07 (1)
F-G	-1133 / 0	-84.3	-84.3 0.13 (1)	5.84	K-F	0 / 298	0.07 (2)
G-H	0 / 21	-84.3	-84.3 0.15 (1)	10.00	K-G	-7 / 93	0.02 (3)
H-I	0 / 40	-84.3	-84.3 0.12 (1)	10.00	O-C	-1370 / 0	0.72 (1)
O-B	-227 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1370 / 0	0.72 (1)
J-H	-227 / 0	0.0	0.0 0.02 (1)	7.81			
O-N	0 / 821	-28.0	-28.0 0.39 (2)	10.00			
N-M	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
M-L	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
L-K	0 / 819	-28.0	-28.0 0.39 (2)	10.00			
K-J	0 / 821	-28.0	-28.0 0.39 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.24 (D-E-1), BC=0.39 (K-M-2), WB=0.72 (G-J-1), SSI=0.19 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

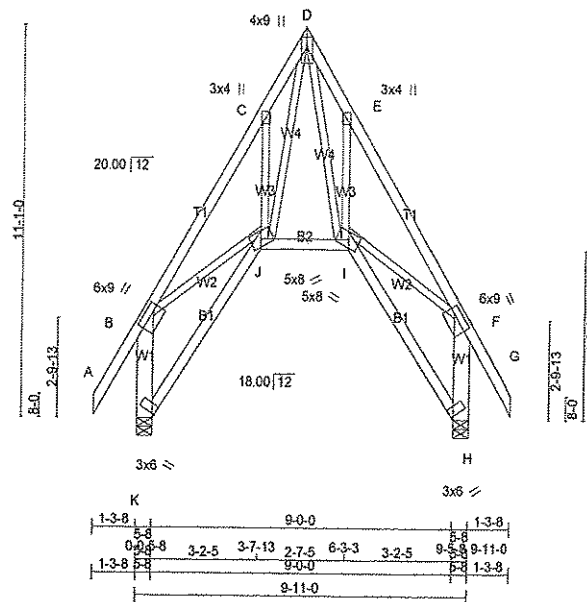
JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.34 (L) (INPUT = 1.00)



DWG NO. TAM34662-16
STRUCTURAL
COMPONENT ONLY

ID: HGKT9E?KoZXKa9D1ZnFkxQzKalv-h8Xu4hXqC_4NPXrXFv_gNjTBG9vdiH9Q0DHVGjyvGUL
 -1-3-8-0-0 3-7-13 3-7-13 4-1-8-6-3-3 3-7-13 9-11-0-3-2-5

Scale: 3/16"=1'



TOTAL WEIGHT = 2 X 78 = 157 lb

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
K	676	0	676	0	0	5-8	1-8
H	676	0	676	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
K	538	326 / 0	104 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	538	326 / 0	104 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. LENGTH	MEMB.	WEBS		MAX. LENGTH
	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD	CS1 (LC)			MAX. FORCE (LBS)	CS1 (LC)	
FR-TO						FR-TO			
A-B	0 / 51	-84.3	-84.3	0.12 (1)	10.00	D-I	0 / 644	0.14 (1)	
B-C	-714 / 0	-84.3	-84.3	0.15 (1)	6.25	I-E	-287 / 0	0.08 (1)	
C-D	-780 / 0	-84.3	-84.3	0.09 (1)	6.25	J-D	0 / 644	0.14 (1)	
D-E	-780 / 0	-84.3	-84.3	0.09 (1)	6.25	J-C	-287 / 0	0.08 (1)	
E-F	-714 / 0	-84.3	-84.3	0.15 (1)	6.25	B-J	0 / 454	0.10 (1)	
F-G	0 / 51	-84.3	-84.3	0.12 (1)	10.00	I-F	0 / 454	0.10 (1)	
K-B	-625 / 0	0.0	0.0	0.08 (1)	7.81				
H-F	-625 / 0	0.0	0.0	0.08 (1)	7.81				
K-J	0 / 0	-28.0	-28.0	0.11 (3)	10.00				
J-I	0 / 234	-28.0	-28.0	0.08 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.11 (3)	10.00				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	6.0	9.0	2.25	2.25
C	TMW-w	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	9.0	3.50	Edge
E	TMW-w	MT20	3.0	4.0		
F	TMVW-I	MT20	6.0	9.0	2.25	2.25
H	BVM1-H	MT20	3.0	6.0	Edge	
I	BBWWW-m	MT20	5.0	8.0	2.50	3.75
J	BBWWW-m	MT20	5.0	8.0	2.50	3.75
K	BVM1-H	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.15 (E-F:1), BC=0.11 (H-I:3), WB=0.14 (D-J:1), SS=0.07 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLJ)	(PLI)
MT20	618	354	1667	822

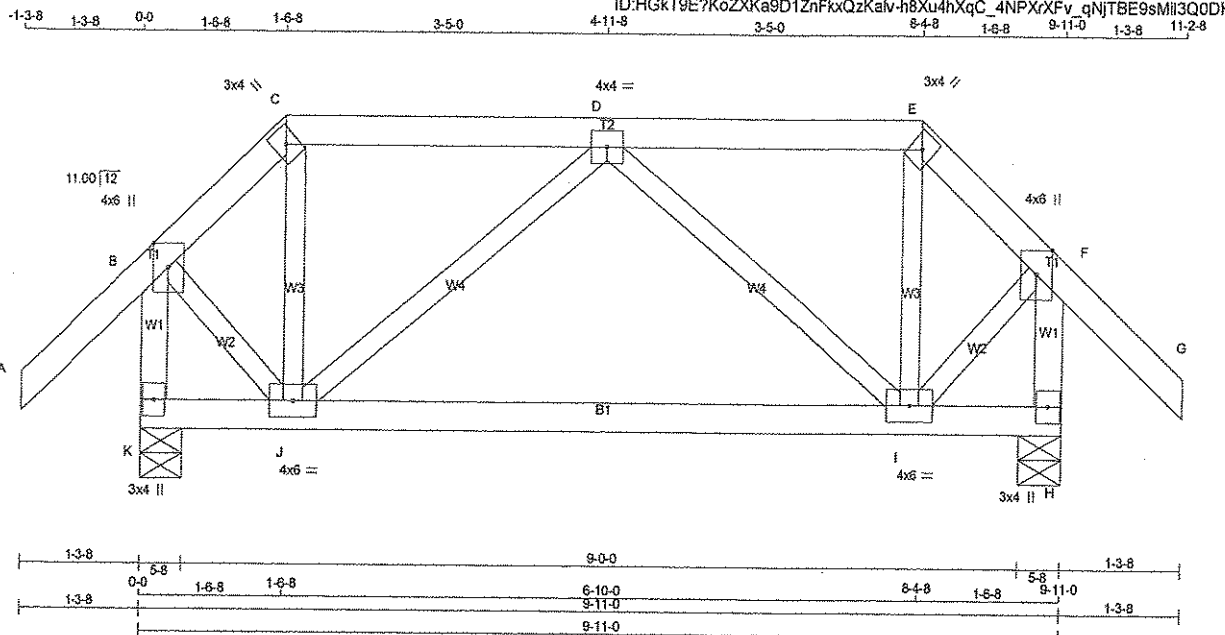
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
 JSI METAL= 0.16 (I) (INPUT = 1.00)



DRWG NO. TAM 34663-10
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	6.0	Edge	
C	TTW+h	MT20	3.0	4.0		
D	TMWV-t	MT20	4.0	4.0		
E	TTW+h	MT20	3.0	4.0		
F	TMWV+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWVW-t	MT20	4.0	6.0		
J	BMWVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQRD	
	VERT	HORZ	DOWN	HORZ	8RG	8RG	
K	674	0	674	0	5-8	1-8	
H	674	0	674	0	5-8	1-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
K	537	325 / 0	104 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	537	325 / 0	104 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURJIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)	
FR-TO		FROM TO	CSI (LC)	UNBRAC LENGTH	FR-TO		CSI (LC)	
A-B	0 / 40	-84.3	-84.3	0.12 (1)	10.00	J-C	0 / 165	0.04 (2)
B-C	-393 / 0	-84.3	-84.3	0.11 (1)	6.25	J-D	-267 / 0	0.09 (1)
C-D	-265 / 0	-84.3	-84.3	0.16 (1)	6.25	D-I	-267 / 0	0.09 (1)
D-E	-265 / 0	-84.3	-84.3	0.16 (1)	6.25	I-E	0 / 165	0.04 (2)
E-F	-393 / 0	-84.3	-84.3	0.11 (1)	6.25	B-J	0 / 344	0.08 (1)
F-G	0 / 40	-84.3	-84.3	0.12 (1)	10.00	I-F	0 / 344	0.08 (1)
K-B	-702 / 0	0.0	0.0	0.08 (1)	7.81			
H-F	-702 / 0	0.0	0.0	0.08 (1)	7.81			
K-J	0 / 0	-28.0	-28.0	0.21 (3)	10.00			
J-I	0 / 467	-28.0	-28.0	0.26 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00			

TOTAL WEIGHT = 45 lb (M)

DESIGN CRITERIA
 SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF
 SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.16 (C-D:1), BC=0.26 (I-J:2), WB=0.09 (D-I:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687 822 2284 1656

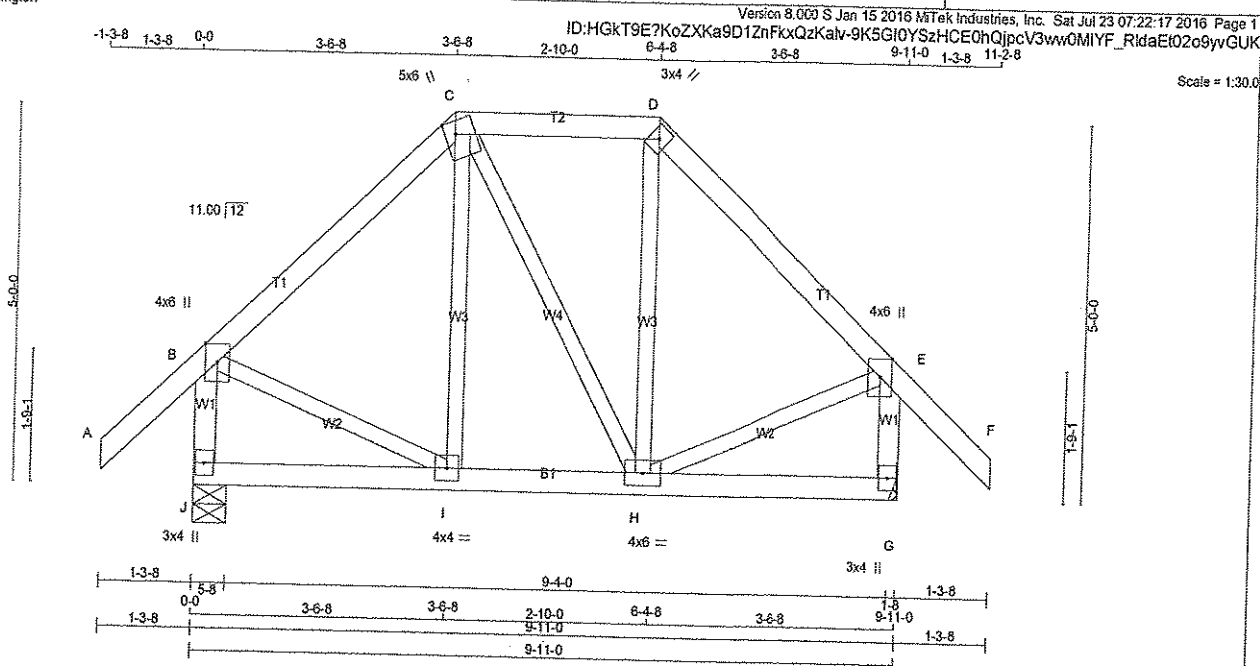
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.80)
 JSI METAL= 0.12 (I) (INPUT = 1.00)



DRWG NO. TAM 34664-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.00 1.75
D	TTW+h	MT20	3.0	4.0	
E	TMVW+p	MT20	4.0	6.0	Edge
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	6.0	
I	BMVWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	1-8	1-8
J	674	0	674	0	0	5-8	1-8	
G	674	0	674	0	0	5-8	1-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
J	537	325 / 0	104 / 0	0 / 0	0 / 0	108 / 0	0 / 0
G	537	325 / 0	104 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO				FR-TO			
A-B	0 / 40	-84.3	-84.3 0.12 (1)	10.00	I-C	-12 / 110	0.03 (3)
B-C	-378 / 0	-84.3	-84.3 0.14 (1)	6.25	C-H	0 / 1	0.00 (3)
C-D	-277 / 0	-84.3	-84.3 0.09 (1)	6.25	H-D	-12 / 111	0.03 (3)
D-E	-378 / 0	-84.3	-84.3 0.14 (1)	6.25	B-I	0 / 298	0.07 (1)
E-F	0 / 40	-84.3	-84.3 0.12 (1)	10.00	H-E	0 / 298	0.07 (1)
J-B	-630 / 0	0.0	0.0 0.07 (1)	7.81			
G-E	-630 / 0	0.0	0.0 0.07 (1)	7.81			
J-I	0 / 0	-28.0	-28.0 0.08 (3)	10.00			
I-H	0 / 277	-28.0	-28.0 0.10 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.08 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 686-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.33")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14 (B-C:1), BC=0.10 (H-I:2), WB=0.07 (B-I:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

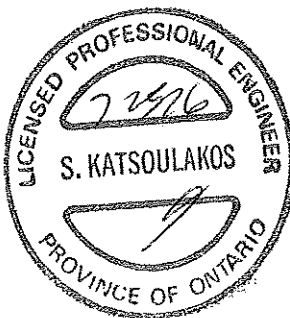
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	822
	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

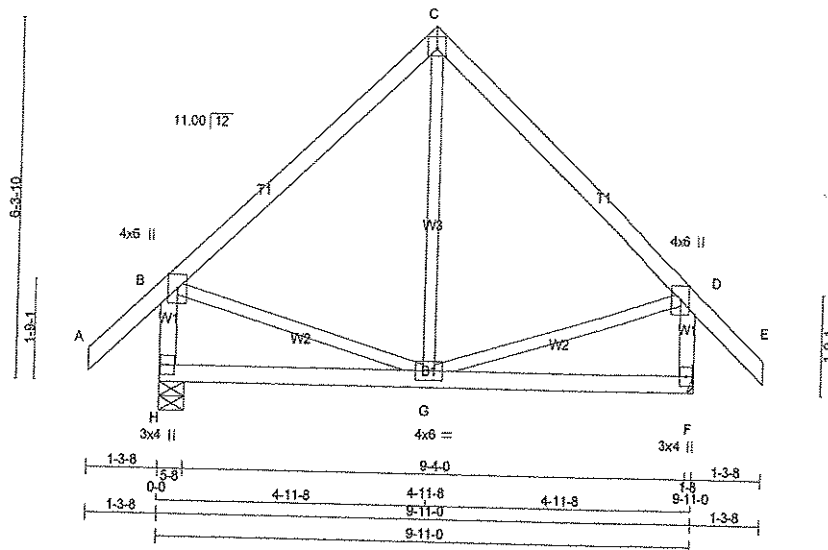
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)

JSI METAL= 0.12 (B) (INPUT = 1.00)



DRWG NO. TAM34665-16
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS --2x3-- DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW+1	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	674	0	674	0
F	674	0	674	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	
H	537	325/0	104/0	0/0	108/0
F	537	325/0	104/0	0/0	108/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0/40	-84.3	-84.3 0.12 (1)	10.00	G-C	0/222	0.05 (3)
B-C	-333/0	-84.3	-84.3 0.27 (1)	8.25	B-G	0/255	0.08 (1)
C-D	-333/0	-84.3	-84.3 0.27 (1)	8.25	G-D	0/255	0.08 (1)
D-E	0/40	-84.3	-84.3 0.12 (1)	10.00			
H-B	-621/0	0.0	0.0 0.07 (1)	7.81			
F-D	-621/0	0.0	0.0 0.07 (1)	7.81			
H-G	0/0	-28.0	-28.0 0.20 (3)	10.00			
G-F	0/0	-28.0	-28.0 0.20 (3)	10.00			

TOTAL WEIGHT = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.27 (C-D:1), BC=0.20 (G-H:3), WB=0.06 (B-G:1), SSI=0.12 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

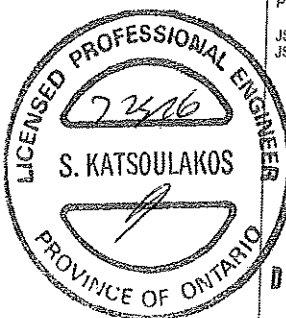
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1887 822 2284 1656

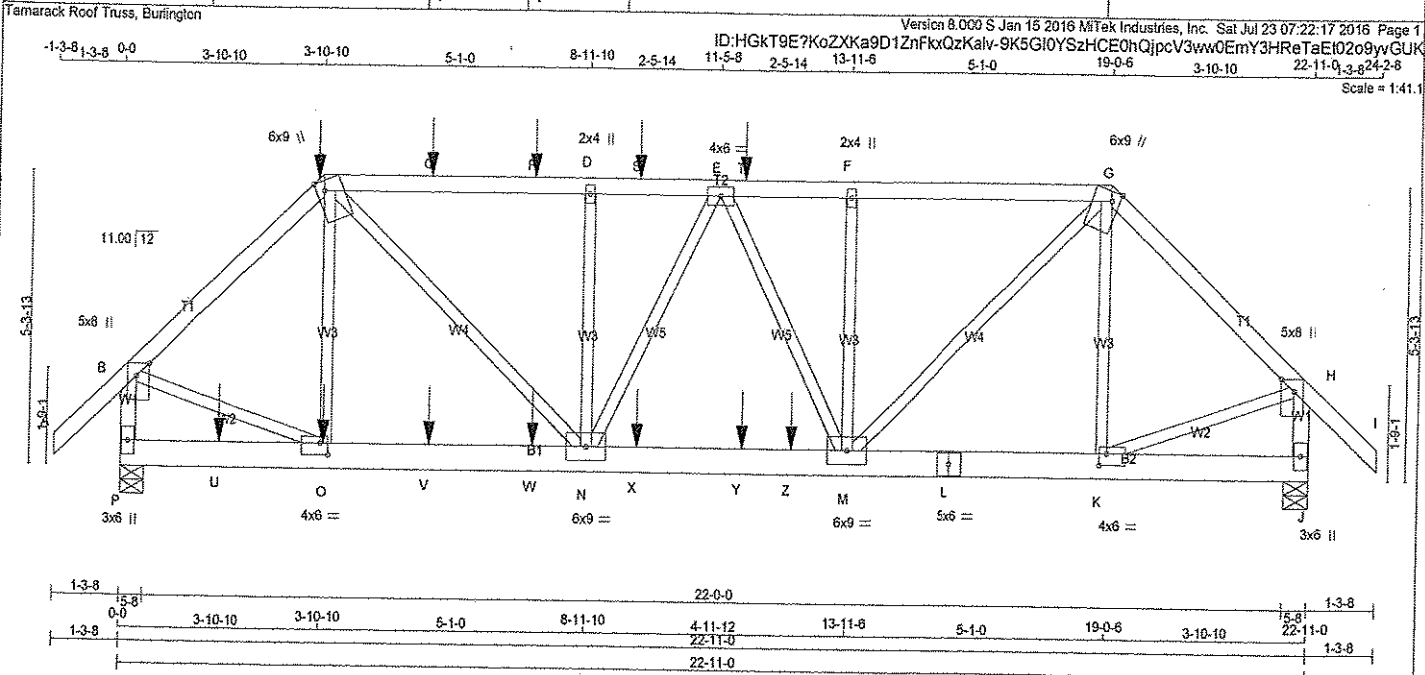
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.80)
 JSI METAL= 0.11 (B) (INPUT = 1.00)



DWG NO. TAM34666-16
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - L	2x6	DRY	No.2
L - J	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	5.0	8.0	Edge 3.00
C	TTWW+m	MT20	6.0	9.0	Edge 1.75
D	TMW+w	MT20	2.0	4.0	
E	TMWV-i	MT20	4.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TTWW+m	MT20	6.0	9.0	Edge 1.75
H	TMWV+p	MT20	5.0	8.0	Edge 3.00
J	BMV1+p	MT20	3.0	6.0	
K	BMWV-i	MT20	4.0	6.0	2.50 1.75
L	SS-i	MT20	5.0	8.0	
M	BMWVW-i	MT20	6.0	9.0	
N	BMWVW-i	MT20	6.0	9.0	
O	BMWVW-i	MT20	4.0	6.0	2.50 1.75
P	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 105.0 lbs FACTORED DOWN AT 3-10-10, 101.4 lbs FACTORED DOWN AT 5-11-4, 101.4 lbs FACTORED DOWN AT 7-11-4, AND 101.4 lbs FACTORED DOWN AT 9-11-4, AND 101.4 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 71.7 lbs FACTORED DOWN AT 1-11-4, 71.7 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-11-4, 71.7 lbs FACTORED DOWN AT 9-11-4, AND 71.7 lbs FACTORED DOWN AT 11-11-4, AND 101.8 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
P	2353	0	2353	0	0	5-8	4-2		
J	2223	0	2223	0	0	5-8	3-12		

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1822	1082/0	427/0	0/0	0/0	414/0	0/0
J	1791	1049/0	371/0	0/0	0/0	371/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
A-B	0/40	-84.3	-84.3 0.13 (1)	O-C	-334/124	0.14 (1)	10.00
B-C	-2176/0	-84.3	-84.3 0.35 (1)	C-N	0/1855	0.48 (1)	4.25
C-Q	-2924/0	-84.3	-84.3 0.68 (1)	N-D	-671/0	0.27 (1)	3.30
Q-R	-2924/0	-84.3	-84.3 0.68 (1)	M-F	-435/0	0.18 (1)	3.30
R-D	-2924/0	-84.3	-84.3 0.68 (1)	M-G	0/2120	0.52 (1)	3.30
D-S	-2924/0	-84.3	-84.3 0.57 (1)	K-G	-448/0	0.18 (1)	3.33
S-E	-2924/0	-84.3	-84.3 0.57 (1)	B-O	0/1885	0.42 (1)	3.33
E-T	-3012/0	-84.3	-84.3 0.39 (1)	K-H	0/1579	0.39 (1)	3.58
T-F	-3012/0	-84.3	-84.3 0.39 (1)	N-E	-170/0	0.09 (1)	3.58
F-G	-3012/0	-84.3	-84.3 0.49 (1)	E-M	-13/25	0.01 (3)	3.55
G-H	-2039/0	-84.3	-84.3 0.34 (1)				4.38
H-I	0/40	-84.3	-84.3 0.13 (1)				10.00
P-B	-2289/0	0.0	0.0 0.26 (1)				5.58
J-H	-2163/0	0.0	0.0 0.25 (1)				5.72
P-U	0/0	-28.0	-28.0 0.10 (3)				10.00
U-O	0/0	-28.0	-28.0 0.10 (3)				10.00
O-V	0/1591	-28.0	-28.0 0.37 (1)				10.00
V-W	0/1591	-28.0	-28.0 0.37 (1)				10.00
W-N	0/1591	-28.0	-28.0 0.37 (1)				10.00
N-X	0/3001	-28.0	-28.0 0.85 (1)				10.00
X-Y	0/3001	-28.0	-28.0 0.85 (1)				10.00
Y-Z	0/3001	-28.0	-28.0 0.85 (1)				10.00
Z-M	0/3001	-28.0	-28.0 0.85 (1)				10.00
M-L	0/1489	-28.0	-28.0 0.44 (1)				10.00
L-K	0/1489	-28.0	-28.0 0.44 (1)				10.00
K-J	0/0	-28.0	-28.0 0.06 (2)				10.00

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-10	-105	-105		BACK	VERT	TOTAL
O	3-11-4	-41	-72		BACK	VERT	TOTAL
Q	5-11-4	-101	-101		BACK	VERT	TOTAL
R	7-11-4	-101	-101		BACK	VERT	TOTAL
S	9-11-4	-101	-101		BACK	VERT	TOTAL
T	11-11-4	-101	-101		BACK	VERT	TOTAL
U	1-11-4	-41	-72		BACK	VERT	TOTAL
V	5-11-4	-41	-72		BACK	VERT	TOTAL
W	7-11-4	-41	-72		BACK	VERT	TOTAL
X	9-11-4	-41	-72		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.68 (C-D:1), BC=0.85 (M-N:1), WB=0.52 (G-M:1), SSI=0.61 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

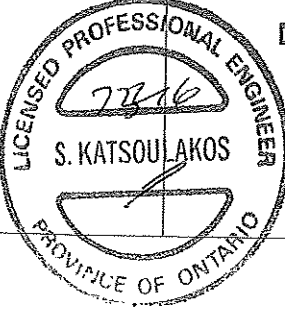
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.40 (H) (INPUT = 1.00)

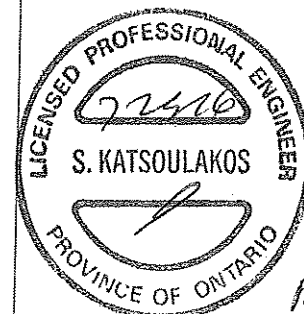


DRWG NO. TAM 34667-16

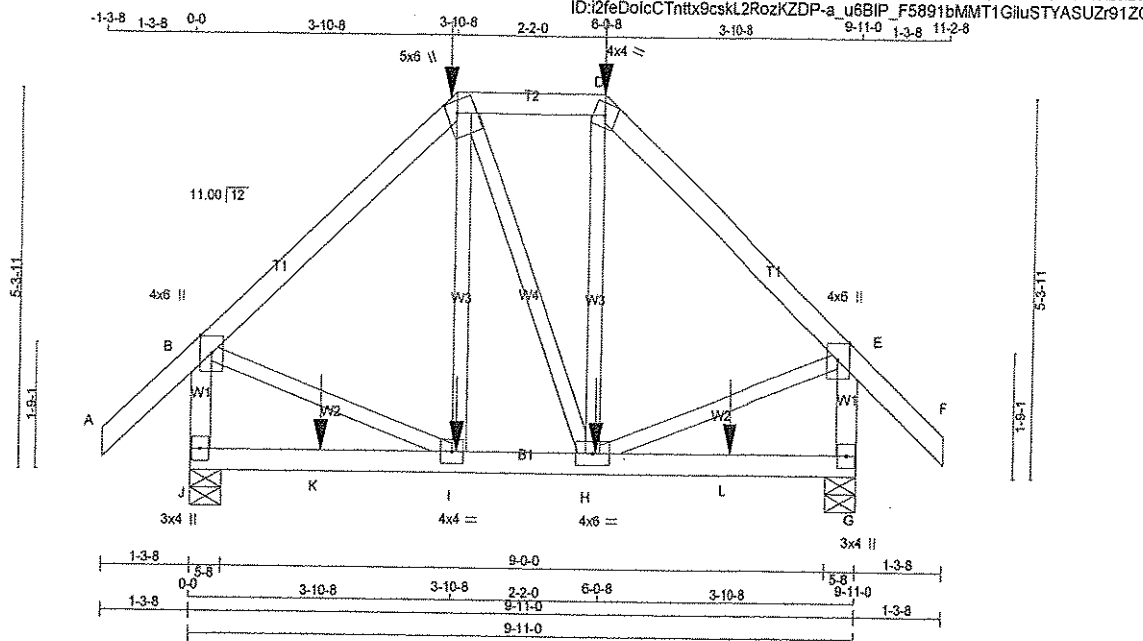
STRUCTURAL COMPONENT ONLY

pe 1/2

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
Y	11-11-4	-41	-72	--	BACK	VERT
Z	12-10-8	-1012	-1012	--	BACK	VERT
						TOTAL
						TOTAL



DWG NO. TAM34667-10
STRUCTURAL
COMPONENT ONLY



Scale: 3/8"=1'

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF
J - G	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWV+m	MT20	5.0	6.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-I	MT20	4.0	6.0		
I	BMVWV-I	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 266.0 lbs FACTORED DOWN AT 3-10-8, AND 266.0 lbs FACTORED DOWN AT 6-0-8 ON TOP CHORD, AND 42.3 lbs FACTORED DOWN AT 1-11-4, 42.3 lbs FACTORED DOWN AT 3-11-4, AND 42.3 lbs FACTORED DOWN AT 5-11-12, AND 42.3 lbs FACTORED DOWN AT 7-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION		GROSS REACTION			
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
J	988	0	988	0	5-8	1-8
G	988	0	988	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED		SNOW	LIVE			
J	778	484 / 0	140 / 0	0 / 0	0 / 0	153 / 0	0 / 0
G	778	484 / 0	140 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-84.3	-84.3 0.13 (1)	10.00	I-C	-50 / 148	0.04 (3)
B-C	-692 / 0	-84.3	-84.3 0.25 (1)	6.25	C-H	0 / 2	0.00 (3)
C-D	-507 / 0	-84.3	-84.3 0.08 (1)	6.25	H-D	-51 / 151	0.04 (3)
D-E	-692 / 0	-84.3	-84.3 0.25 (1)	6.25	B-I	0 / 539	0.13 (1)
E-F	0 / 40	-84.3	-84.3 0.13 (1)	10.00	H-E	0 / 539	0.13 (1)
J-B	-930 / 0	0.0	0.0 0.11 (1)	7.81			
G-E	-930 / 0	0.0	0.0 0.11 (1)	7.81			
J-K	0 / 0	-28.0	-28.0 0.15 (3)	10.00			
K-I	0 / 0	-28.0	-28.0 0.15 (3)	10.00			
I-H	0 / 507	-28.0	-28.0 0.18 (2)	10.00			
H-L	0 / 0	-28.0	-28.0 0.15 (2)	10.00			
L-G	0 / 0	-28.0	-28.0 0.15 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-266	-266	---	BACK	VERT	TOTAL
D	6-0-8	-266	-266	---	BACK	VERT	TOTAL
H	5-11-12	-24	-42	---	BACK	VERT	TOTAL
I	3-11-4	-24	-42	---	BACK	VERT	TOTAL
K	1-11-4	-24	-42	---	BACK	VERT	TOTAL
L	7-11-12	-24	-42	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.25 (B-C:1), BC=0.18 (H-I:2), WB=0.13 (B-I:1), SI=0.12 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.83 (H) (INPUT = 0.90)
 JSI METAL = 0.19 (I) (INPUT = 1.00)



DRWG NO. TAM34668-10
 STRUCTURAL
 COMPONENT ONLY

Tamarack Roof Truss, Burlington

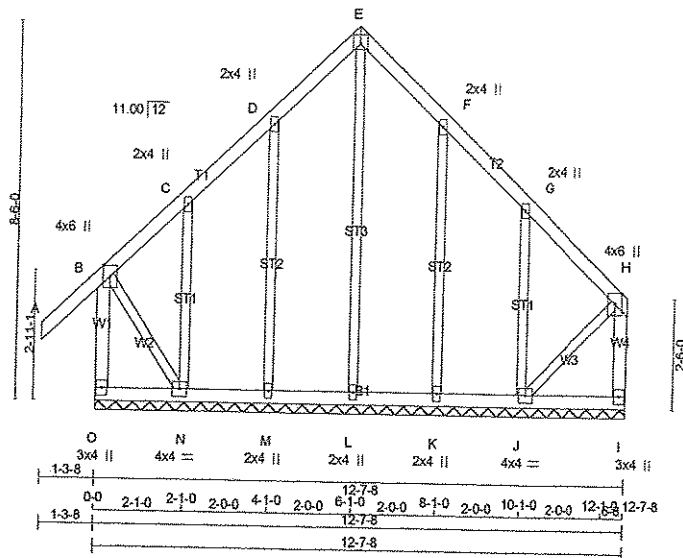
Version 8.000 S Jan 15 2016 MTek Industries, Inc. Sat Jul 23 07:23:28 2016 Page 1

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1-3-8 0-0 2-1-0 2-1-0 2-0-0 4-1-0 2-0-0 6-1-0 2-0-0 8-1-0 2-0-0 10-1-0 2-0-0 12-1-0 2-0-0 12-7-8

4x4 =

Scale = 1:50.0



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMVW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K, L, M						
K	BMV1+w	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 40	-84.3 -84.3	0.12 (1)	10.00	L-E	-124 / 0	0.17 (1)
B-C	-13 / 0	-84.3 -84.3	0.04 (1)	6.25	M-D	-188 / 0	0.14 (1)
C-D	-18 / 0	-84.3 -84.3	0.05 (1)	6.25	N-C	-180 / 0	0.06 (1)
D-E	-26 / 0	-84.3 -84.3	0.05 (1)	6.25	K-F	-179 / 0	0.13 (1)
E-F	-25 / 0	-84.3 -84.3	0.04 (1)	6.25	J-G	-208 / 0	0.07 (1)
F-G	-23 / 0	-84.3 -84.3	0.06 (1)	6.25	B-N	0 / 29	0.01 (1)
G-H	-13 / 0	-84.3 -84.3	0.06 (1)	6.25	J-H	0 / 24	0.01 (1)
O-B	-223 / 0	0.0 0.0	0.04 (1)	7.81			
I-H	-117 / 0	0.0 0.0	0.02 (1)	7.81			
O-N	0 / 0	-28.0 -28.0	0.03 (2)	10.00			
N-M	0 / 14	-28.0 -28.0	0.03 (2)	10.00			
M-L	0 / 11	-28.0 -28.0	0.02 (2)	10.00			
L-K	0 / 11	-28.0 -28.0	0.02 (2)	10.00			
K-J	0 / 14	-28.0 -28.0	0.04 (2)	10.00			
J-I	0 / 0	-28.0 -28.0	0.04 (2)	10.00			

TOTAL WEIGHT = 70 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (A-B:1), BC=0.04 (J-K:2), WB=0.17 (E-L:1), SSI=0.07 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687
	822	2284	1656

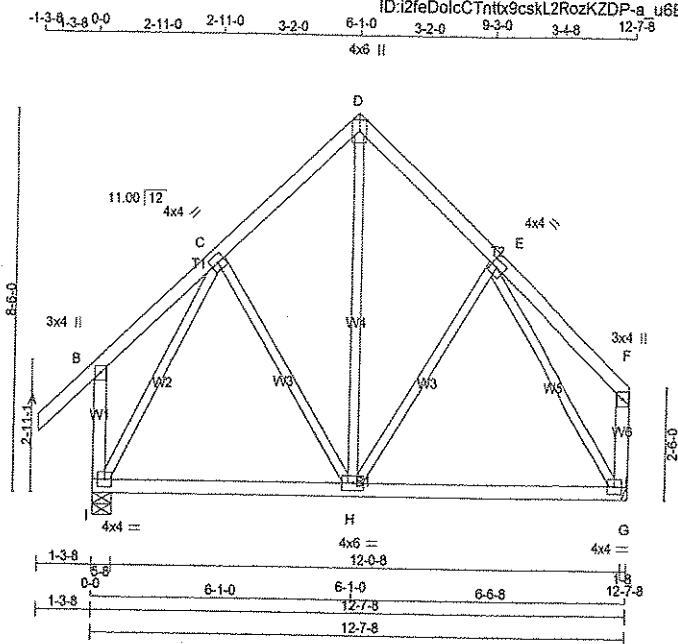
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)



DRWG NO. TAM34671-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
I - B	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
I	826	0	826	0
G	709	0	709	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
I	661	394 / 0	133 / 0	0 / 0	135 / 0	0 / 0
G	582	323 / 0	133 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-84.3	-84.3 0.12 (1)	C-H	-28 / 70	0.02 (1)	
B-C	0 / 21	-84.3	-84.3 0.12 (1)	H-D	0 / 334	0.08 (2)	
C-D	-412 / 0	-84.3	-84.3 0.10 (1)	I-E	-85 / 50	0.06 (1)	
D-E	-415 / 0	-84.3	-84.3 0.11 (1)	I-C	-635 / 0	0.36 (1)	
E-F	0 / 22	-84.3	-84.3 0.14 (1)	E-G	-631 / 0	0.40 (1)	
I-B	-205 / 0	0.0	0.0 0.03 (1)				
G-F	-104 / 0	0.0	0.0 0.01 (1)				
I-H	0 / 305	-28.0	-28.0 0.36 (2)				
H-G	0 / 337	-28.0	-28.0 0.36 (2)				

TOTAL WEIGHT = 2 X 67 = 133 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.14 (E-F:1), BC=0.36 (G-H:2), WB=0.40 (E-G:1), SSI=0.15 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

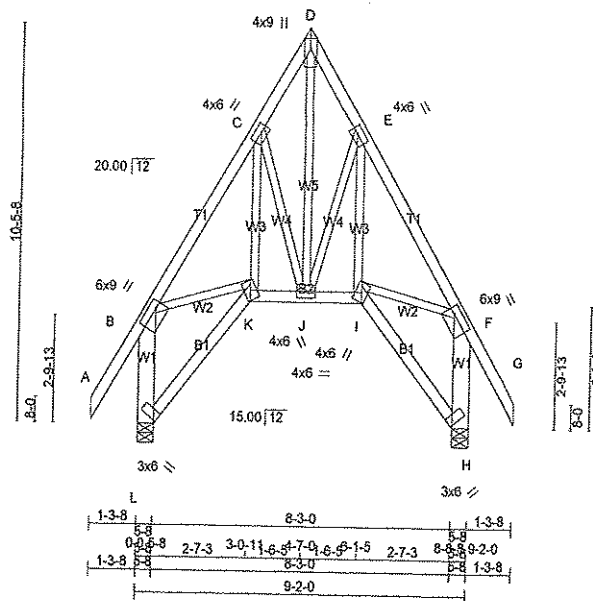
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.23 (E) (INPUT = 1.00)



DWG NO. TAM 34669-10
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.25	2.25
C	TMVW-t	MT20	4.0	6.0	2.00	2.25
D	TTV+p	MT20	4.0	9.0	3.50	Edge
E	TMVW-t	MT20	4.0	6.0	2.00	2.25
F	TMVW-t	MT20	6.0	9.0	2.25	2.25
H	BVM1-H	MT20	3.0	6.0	Edge	
J	BBVW+m	MT20	4.0	6.0		
K	BBVW+m	MT20	4.0	6.0		
L	BVM1-H	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	634	0	634	0	5-8	1-8
H	634	0	634	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	504	307/0	96/0	0/0	0/0	100/0	0/0
H	504	307/0	96/0	0/0	0/0	100/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				
A-B	0/51	-84.3	0/164	0.04 (2)
B-C	-491/0	-84.3	0/164	0.04 (2)
C-D	-378/0	-84.3	0/273	0.06 (1)
D-E	-378/0	-84.3	0/273	0.06 (1)
E-F	-491/0	-84.3	0/552	0.12 (1)
F-G	0/51	-84.3	-272/0	0.10 (1)
L-B	-591/0	0.0	-272/0	0.10 (1)
H-F	-591/0	0.0		
L-K	0/0	-28.0		
K-J	0/258	-28.0		
J-I	0/258	-28.0		
I-H	0/0	-28.0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12 (A-B:1), BC=0.08 (H:3), WB=0.12 (D-J:1), SSI=0.06 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

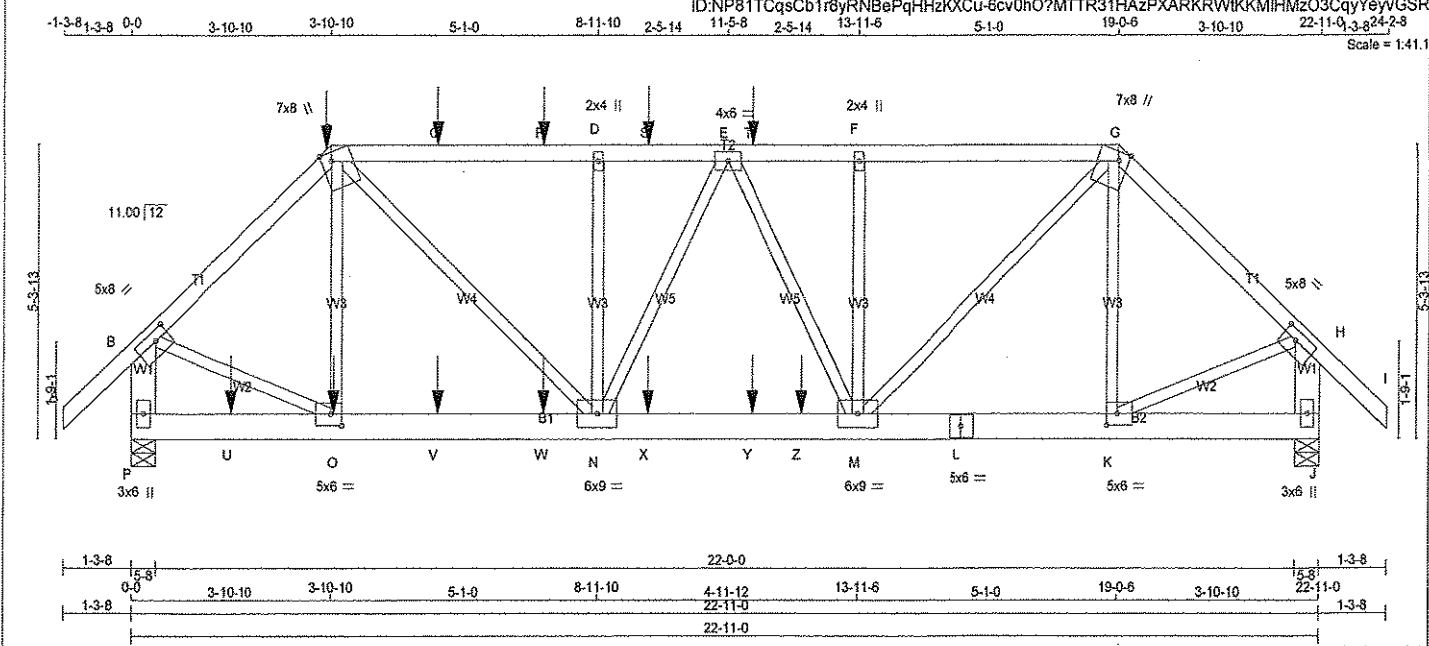
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.49 (J) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)



DWG NO. TAM34673-10
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
P - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
P - L	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00 3.25
C	TTWW+m	MT20	7.0	8.0	Edge 2.25
D	TMW+w	MT20	2.0	4.0	
E	TMW-w	MT20	4.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TTWW+m	MT20	7.0	8.0	Edge 2.25
H	TMVW-t	MT20	5.0	8.0	2.00 3.25
J	BMV1+p	MT20	3.0	6.0	
K	BMWW-t	MT20	5.0	6.0	2.50 2.50
L	BS-t	MT20	5.0	6.0	
M	BMWW-t	MT20	6.0	9.0	
N	BMWW-t	MT20	6.0	9.0	
O	BMWW-t	MT20	5.0	6.0	2.50 2.50
P	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.3 lbs FACTORED DOWN AT 3-10-10, 145.7 lbs FACTORED DOWN AT 5-11-4, 145.7 lbs FACTORED DOWN AT 7-11-4, AND 145.7 lbs FACTORED DOWN AT 9-11-4, AND 163.3 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 84.7 lbs FACTORED DOWN AT 1-11-4, 84.7 lbs FACTORED DOWN AT 3-11-4, 84.7 lbs FACTORED DOWN AT 5-11-4, 84.7 lbs FACTORED DOWN AT 7-11-4, 84.7 lbs FACTORED DOWN AT 9-11-4, AND 87.4 lbs FACTORED DOWN AT 11-11-4, AND 984.3 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	2526	0	5-8	2-12
P	2307	0	5-8	2-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	2057	1169 / 0	449 / 0	0 / 0	0 / 0	439 / 0	0 / 0
P	1855	1094 / 0	379 / 0	0 / 0	0 / 0	382 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.03 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
A-B	0 / 40	-84.3	-84.3 0.13 (1)	10.00	O-C	-357 / 140	0.15 (1)
B-C	-2362 / 0	-84.3	-84.3 0.37 (1)	4.10	C-N	0 / 1967	0.49 (1)
C-Q	-3141 / 0	-84.3	-84.3 0.80 (1)	2.97	N-D	-777 / 0	0.32 (1)
Q-R	-3141 / 0	-84.3	-84.3 0.80 (1)	2.97	M-F	-446 / 0	0.18 (1)
R-D	-3141 / 0	-84.3	-84.3 0.80 (1)	2.97	M-G	0 / 2237	0.55 (1)
D-S	-3141 / 0	-84.3	-84.3 0.67 (1)	3.02	K-G	-467 / 0	0.19 (1)
S-E	-3141 / 0	-84.3	-84.3 0.67 (1)	3.02	B-O	0 / 1829	0.45 (1)
E-T	-3165 / 0	-84.3	-84.3 0.41 (1)	3.47	K-H	0 / 1651	0.41 (1)
T-F	-3165 / 0	-84.3	-84.3 0.41 (1)	3.47	N-E	-127 / 10	0.07 (1)
F-G	-3164 / 0	-84.3	-84.3 0.50 (1)	3.45	E-M	-75 / 0	0.04 (1)
G-H	-2132 / 0	-84.3	-84.3 0.35 (1)	4.30			
H-I	0 / 40	-84.3	-84.3 0.13 (1)	10.00			
P-B	-2461 / 0	0.0	0.0 0.19 (1)	6.55			
J-H	-2249 / 0	0.0	0.0 0.17 (1)	6.79			

P-U	0 / 0	-28.0	-28.0 0.10 (3)	10.00			
U-O	0 / 0	-28.0	-28.0 0.10 (3)	10.00			
O-V	0 / 1727	-28.0	-28.0 0.38 (1)	10.00			
V-W	0 / 1727	-28.0	-28.0 0.38 (1)	10.00			
W-N	0 / 1727	-28.0	-28.0 0.38 (1)	10.00			
N-X	0 / 3199	-28.0	-28.0 0.87 (1)	10.00			
X-Y	0 / 3199	-28.0	-28.0 0.87 (1)	10.00			
Y-Z	0 / 3199	-28.0	-28.0 0.87 (1)	10.00			
Z-M	0 / 3199	-28.0	-28.0 0.87 (1)	10.00			
M-L	0 / 1557	-28.0	-28.0 0.45 (1)	10.00			
L-K	0 / 1557	-28.0	-28.0 0.45 (1)	10.00			
K-J	0 / 0	-28.0	-28.0 0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-10	-149	-149	---	BACK	VERT	TOTAL
O	3-11-4	-48	-85	---	BACK	VERT	TOTAL
Q	5-11-4	-148	-148	---	BACK	VERT	TOTAL
R	7-11-4	-148	-148	---	BACK	VERT	TOTAL
S	9-11-4	-148	-148	---	BACK	VERT	TOTAL
T	11-11-4	-163	-163	---	BACK	VERT	TOTAL
U	1-11-4	-48	-85	---	BACK	VERT	TOTAL
V	5-11-4	-48	-85	---	BACK	VERT	TOTAL
W	7-11-4	-48	-85	---	BACK	VERT	TOTAL
X	9-11-4	-48	-85	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.80 (C-D:1), BC=0.87 (M-N:1), WB=0.55 (G-M:1), SSI=0.60 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

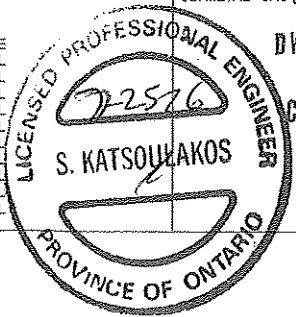
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.48 (H) (INPUT = 1.00)



DRWG NO. TAM34672-16
STRUCTURAL
COMPONENT ONLY
Pc/L

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267396	T50Z	1	1	TRUSS DESC.		

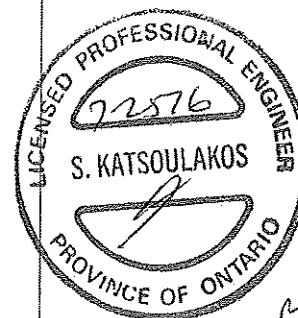
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Sat Jul 23 07:24:18 2016 Page 2

ID:NP81TCqsCb1r8yRNBePgHHzKXCu-6cv0h0?MITR31HAzPXARKRWIKKMIHMzO3CgyYeyvGSR

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	11-11-4	-80	-87	--	BACK	VERT	TOTAL
Z	12-10-8	-984	-984	--	BACK	VERT	TOTAL



DWG NO. TAM34672 -16
STRUCTURAL
COMPONENT ONLY

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

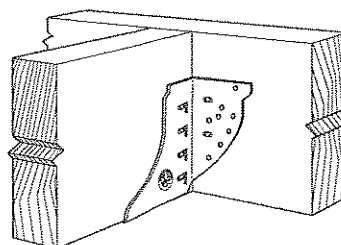
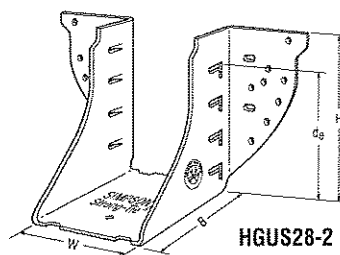
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

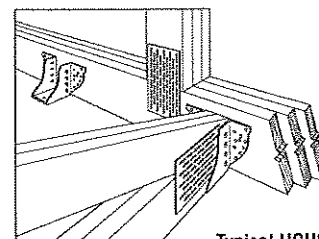
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



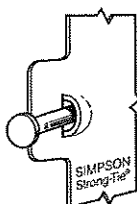
Typical HGUS
Installation



Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

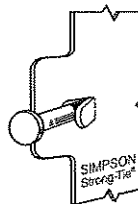
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D-Fir-L		S-P-F	
		W	H	B	d _e ¹	Face	Joist	Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
HGUS26	12	1½	5½	5	4½	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅝	5⅞	4	4¼	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5½	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅝	7⅞	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7½	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅝	9⅞	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGU210-3	12	4⅞	9¼	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGU210-4	12	6⅞	9⅞	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10½	4	10½	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12½	4	11½	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

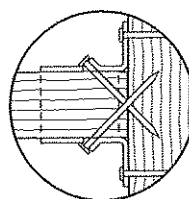


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

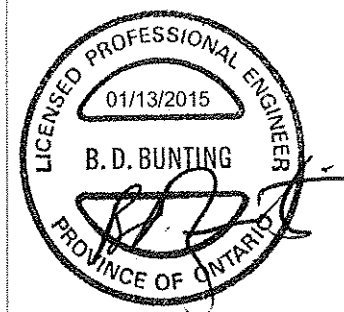
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

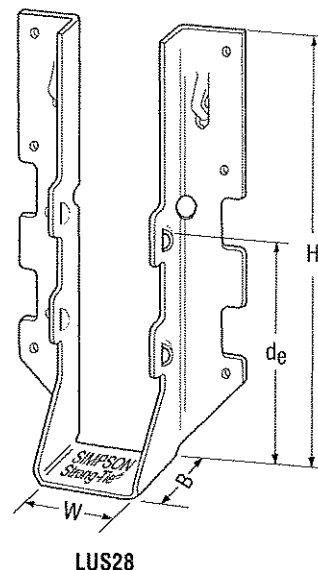
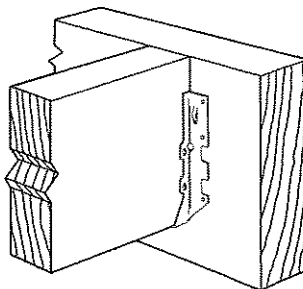
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

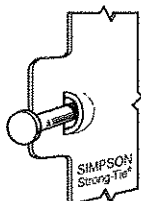
- These hangers cannot be modified.

Typical LUS Installation



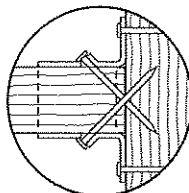
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 9/16	3 3/8	1 3/4	1 13/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 3/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 3/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

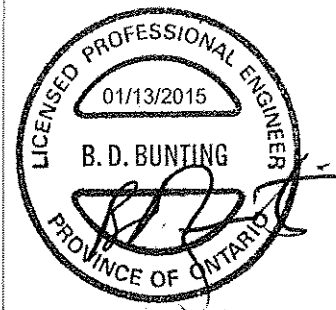


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECLUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

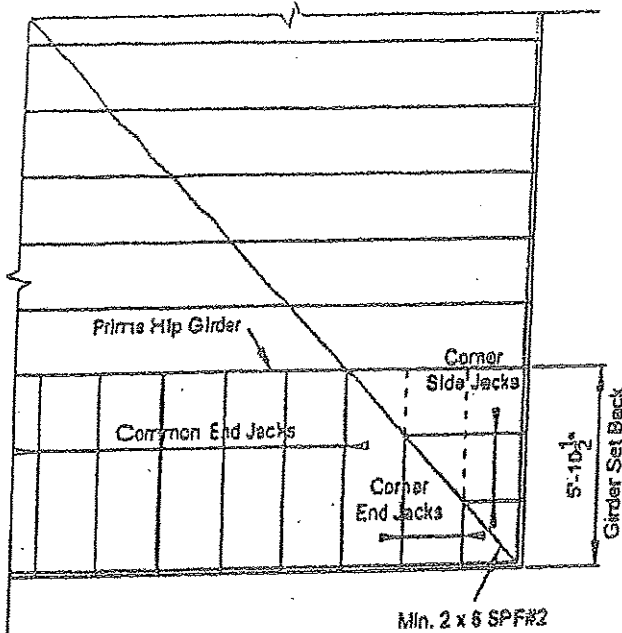
TOP CHORD LIVE LOAD : 34.8 P.S.F.

TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

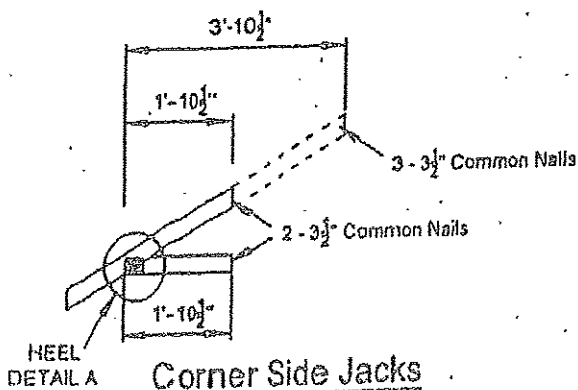
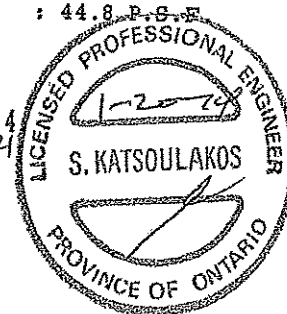
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

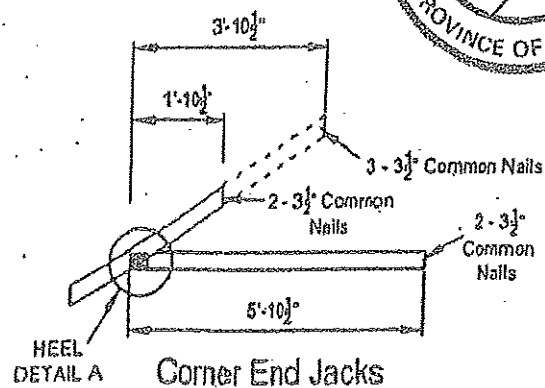


45° Hip End

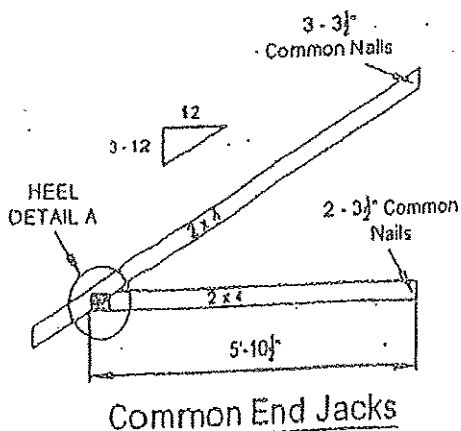
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STRUCTURAL
COMPONENT ONLY



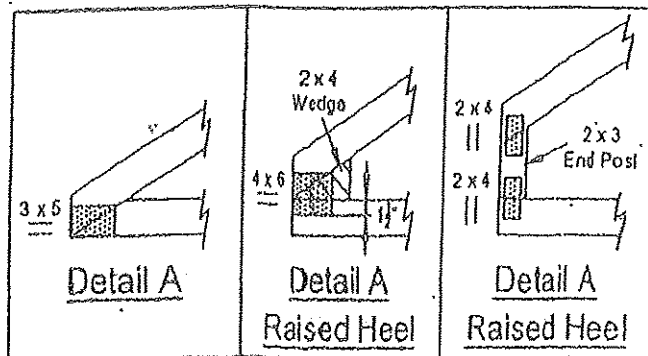
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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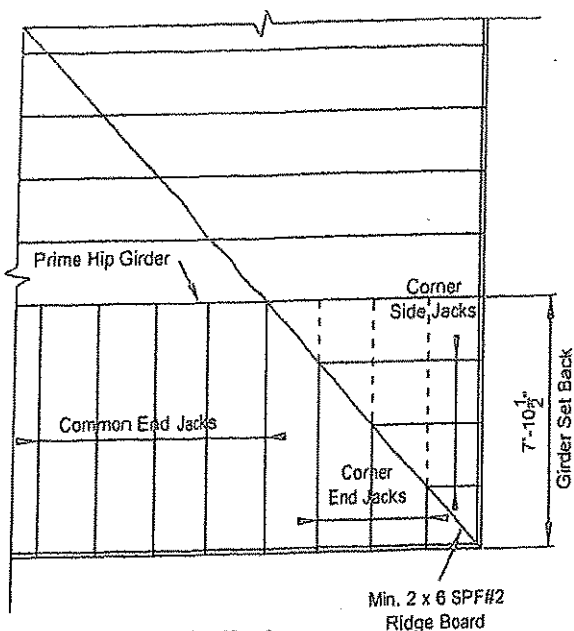
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

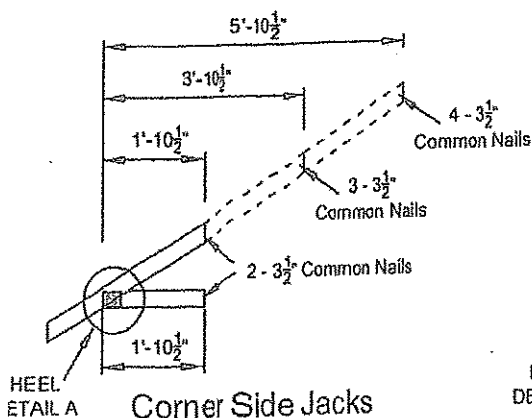
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

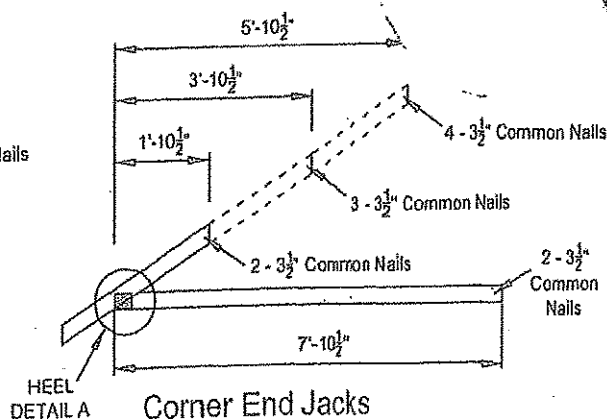
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 44.8 P.S.F.



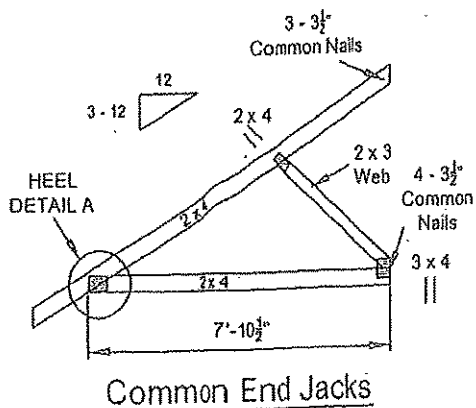
45° Hip End



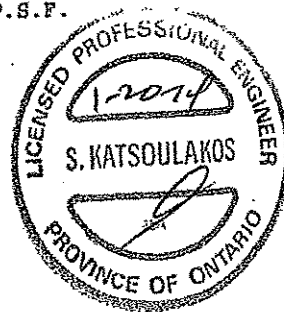
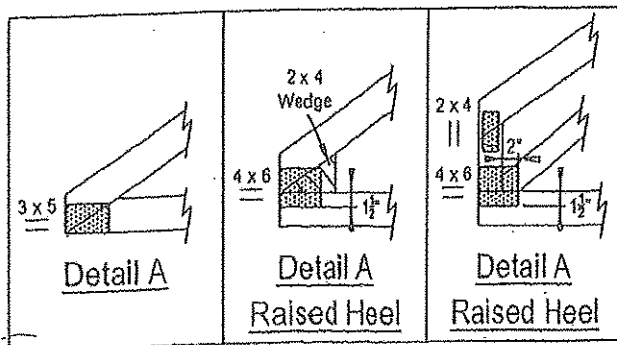
Corner Side Jacks



Corner End Jacks



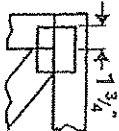
Common End Jacks



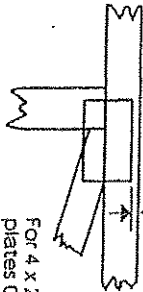
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/8" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

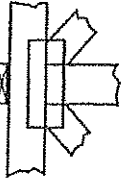
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

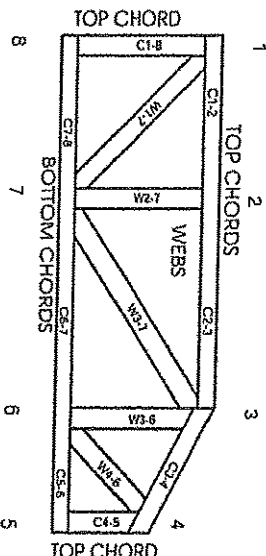


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12591-R

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Mitek Engineering Reference Sheet MIT-7473C rev. 10-08

Mitek
POWER TO PERFORM™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

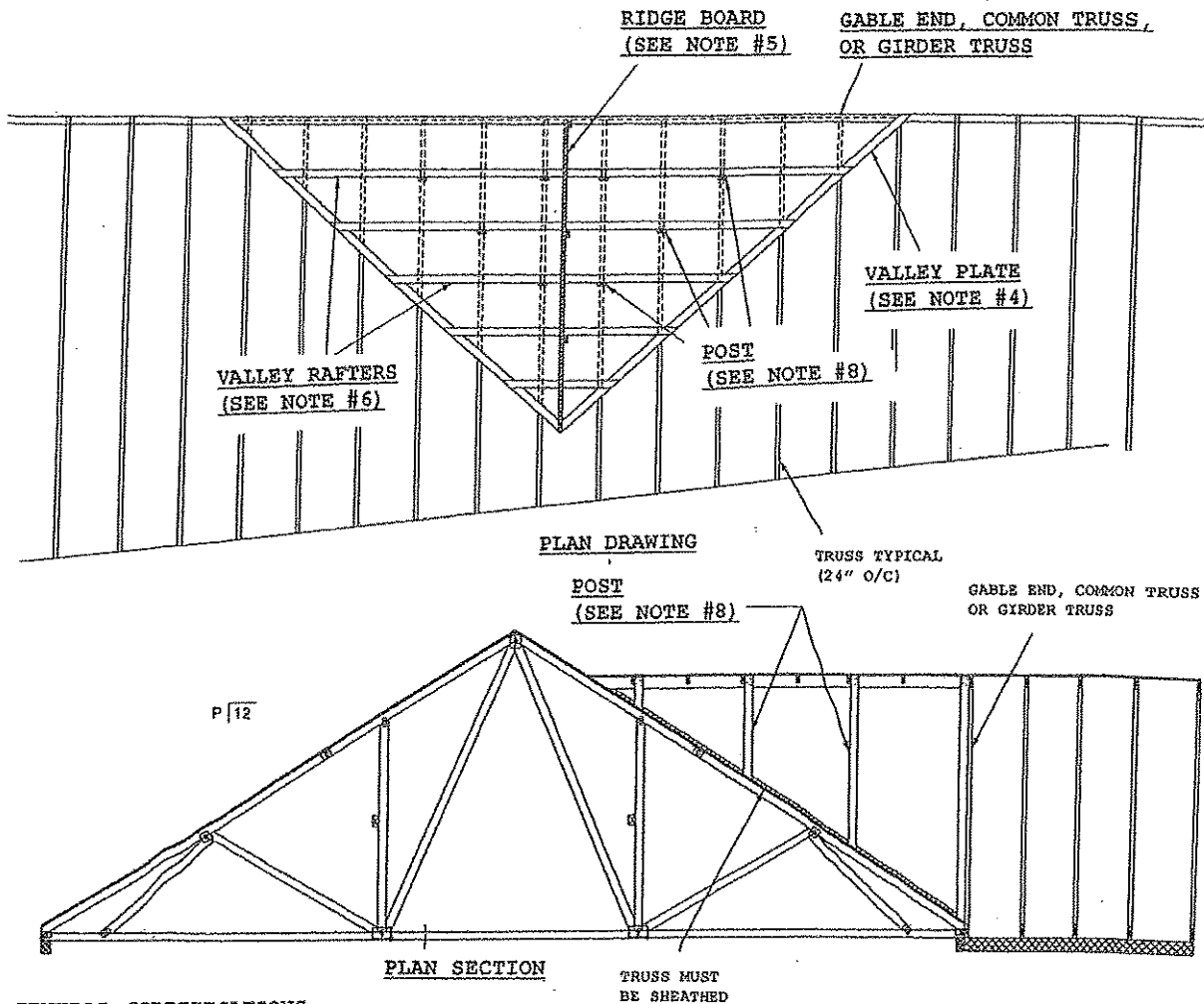
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pulins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.0 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

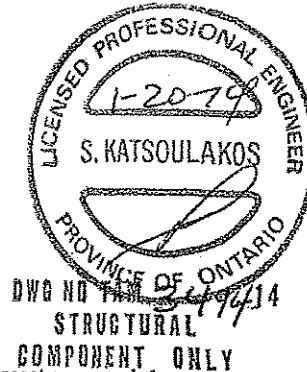
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.