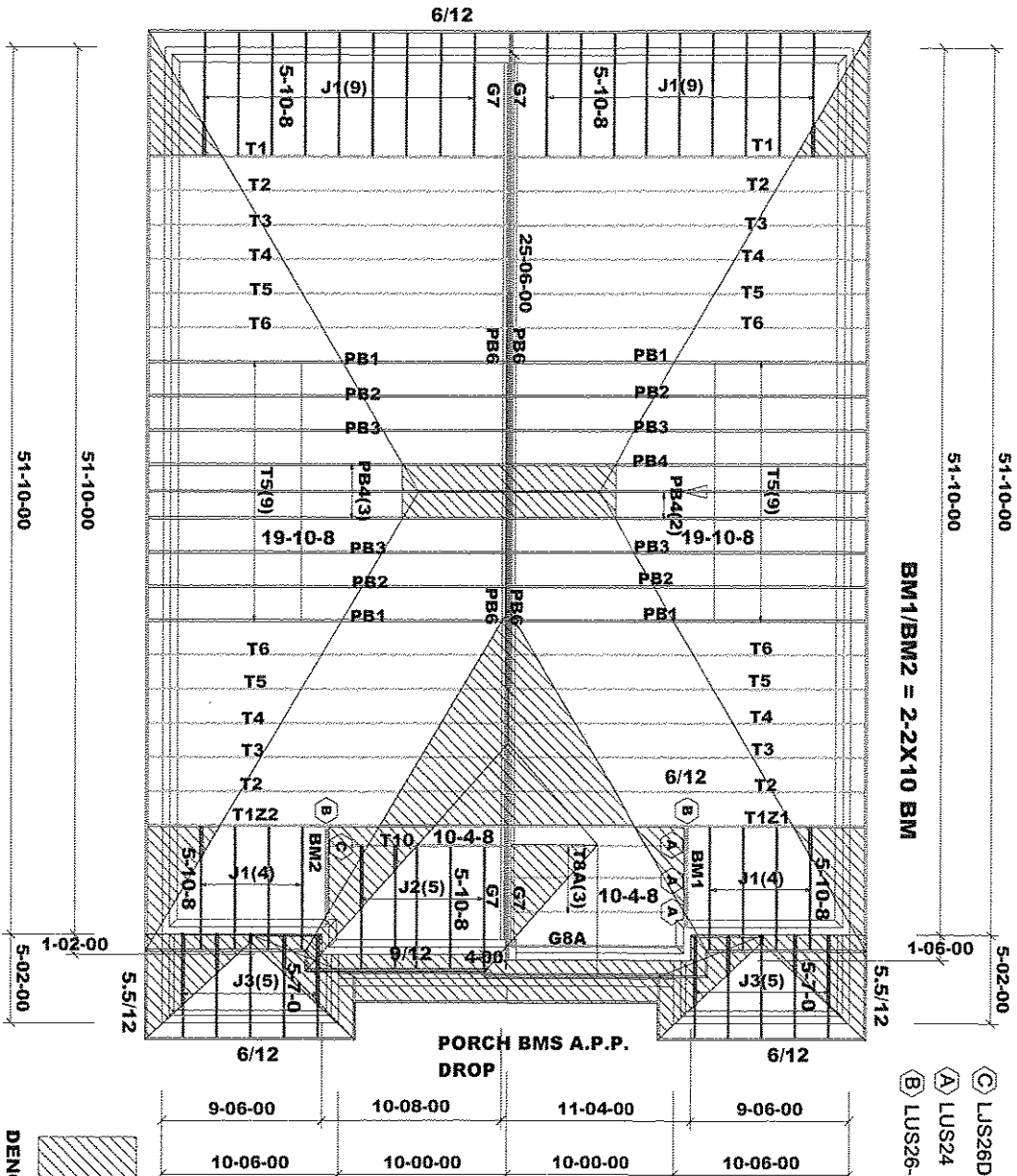
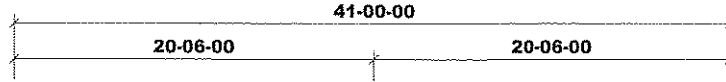


**SLOPE 10/12 TYP.
UNLESS NOTED**

**ELMBROOK 4B
ELEV. 1B
267476**

**ELMBROOK 4B
ELEV. 1A
267473**

**ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 FASCIA BOARD
2X6 EXTERIOR WALLS**



BM1/BM2 = 2-2X10 BM

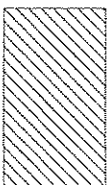
(C) LUS26DS
 (A) LUS24
 (B) LUS26-2

NOTES:
1- TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

**DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
GROUND SNOW 1.5 KPA
RAIN 8.4 PSF
TC DEAD 3.0 PSF
BC LIVE 10.5 PSF
BC DEAD 7. PSF**

**CONVENTIONAL
FRAMING**



117767

TAMARACK LUMBER, INC. 10000 100th Ave. S.E. Bellevue, WA 98004 (206) 835-1000	
Job Track:	42181
Layout ID:	267472
Plan Log:	86179
Builder / Location:	GREENPARK HOMES / VAUGHAN
Project:	STAR TIME HOME LTD
Date:	5/25/2016
Designer:	FV/JG
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
ELMBROOK 4B / 1A & 1B PAIRED	

**SLOPE 10/12 TYP.
UNLESS NOTED**

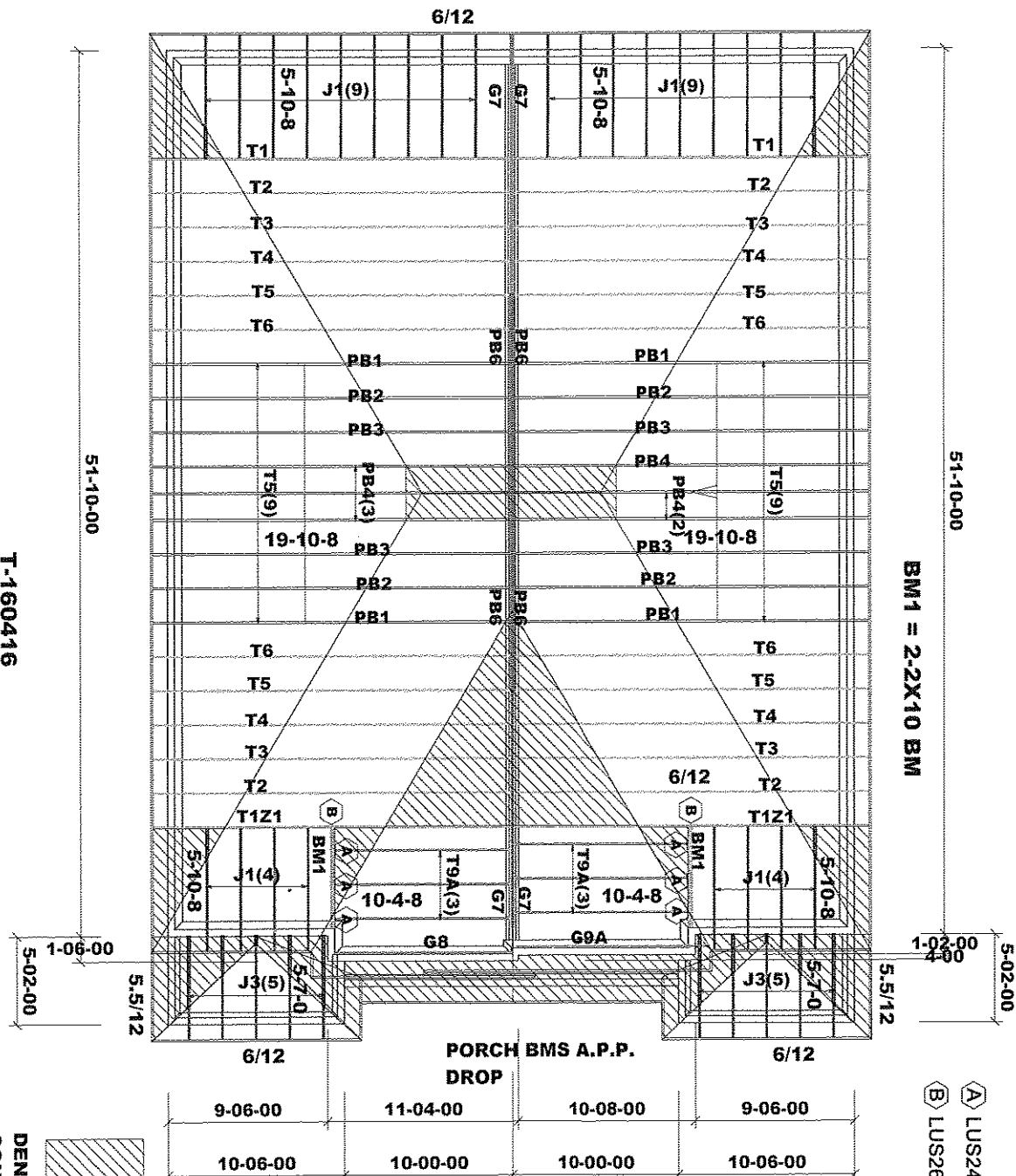
**ELMBROOK 4B
ELEV. 2A
267478**

**ELMBROOK 4B
ELEV. 2B
267479**

**ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 FASCIA BOARD
2X6 EXTERIOR WALLS**

119769

41-00-00
20-06-00 20-06-00



**DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
GROUND SNOW 1.5 KPA
RAIN 8.4 PSF
TC DEAD 3.0 PSF
BC LIVE 10.5 PSF
BC DEAD 7. PSF**

NOTES:
1- TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

**DENOTES:
CONVENTIONAL
FRAMING**



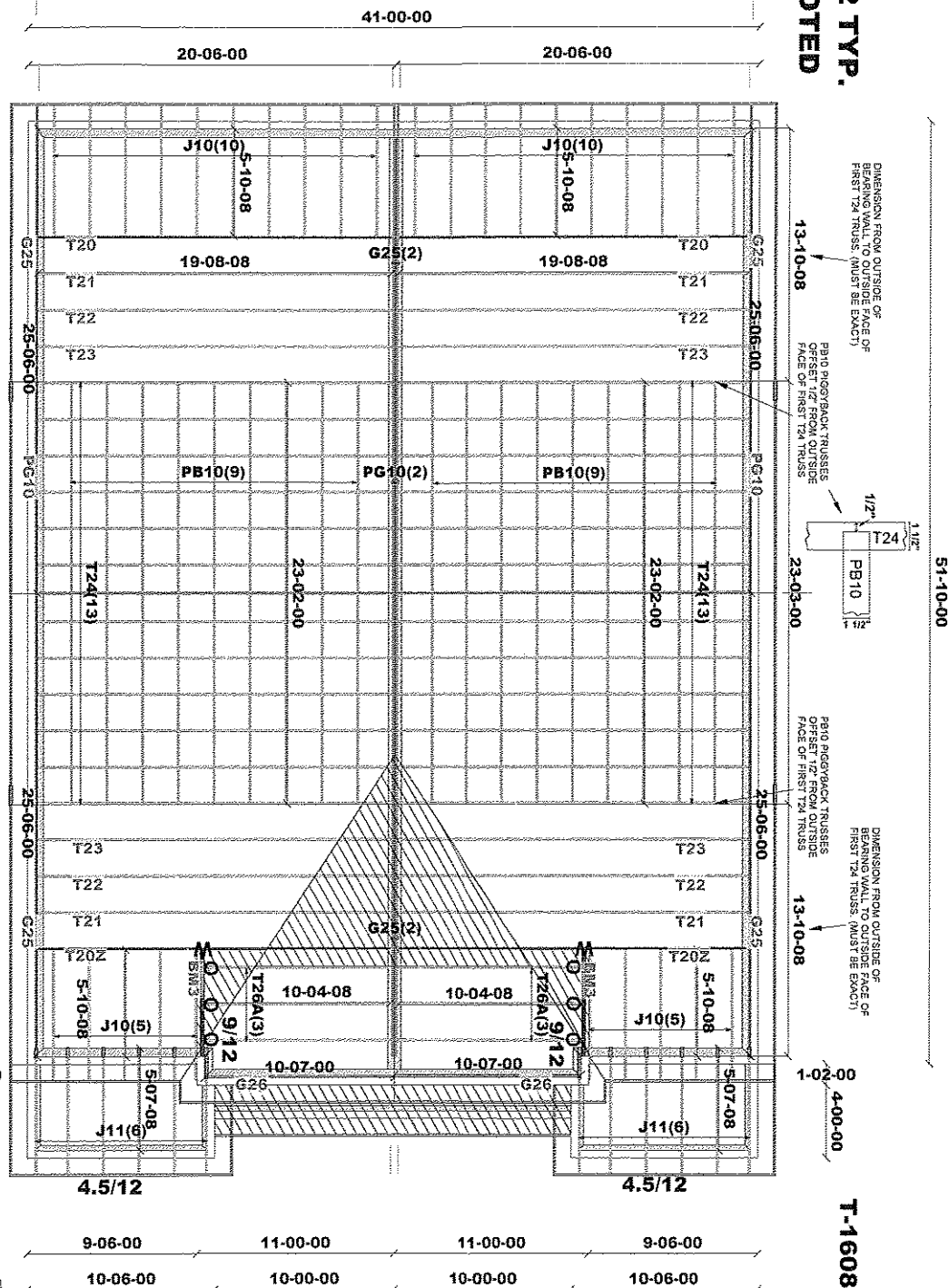
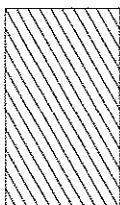
TAMARACK Job Track: 42181 Layout ID: 267477 Plan Log: 86179		Builder / Location: GREENPARK HOMES / VAUGHAN		Model / Elevation: ELMBROOK 4B / 2A & 2B PAIRED	
Project: STAR TIME HOME LTD Date: 5/25/2016 Designer: FV/JG		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 7.5.0			

T-160849

ELMBROOK 4B
ELEV. 3
274597

BM3 = 2-2X10 SPF #2
LUS24 - (O)
LUS26-2 - (W)

**DENOTES:
CONVENTIONAL
FRAMING**



1- TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH O.B.C. 2012 PART 9 DESIGN LOADS: GROUND SNOW 1.5 KPA RAIN 8.4 PSF	TC DEAD 3.0 PSF BC LIVE 10.5 PSF BC DEAD 7. PSF
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Hot Track: 42481

Layout ID: **274585**

Plan Log: **89264**

Builder / Location

GREENPARK HOMES / VAUGHAN

Project: **STARTIME HOME LTD**

Date: 10/27/2016 Designer: colinf

Model / Elevation:

ELMBROOK 4B / 3 - PAIRED

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Model ver 7.3.0

Mitek ver 7.5.0



Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK:42181 LAYOUT ID: 267473 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 1A

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	1	T1Z2 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	2	T2 HALF HIP	10.00 0.00	19-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	172.38 109.00		
	2	T3 HALF HIP	10.00 0.00	19-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	169.36 105.34		
	2	T4 HALF HIP	10.00 0.00	19-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-04	183.84 116.00		
	11	T5 HALF HIP	10.00 0.00	19-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-04	1066.45 674.63		
	2	T6 HALF HIP	10.00 0.00	19-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-04	209.12 129.66		
	2	G7 PIGGYBACK	6.00 0.00	25-06-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	255.88 161.66		
	1	T10 HALF HIP	10.00 0.00	10-04-08	05-11-02	2 X 4	2 X 6	00-00-00 00-00-00	01-07-11 05-11-02	53.54 35.50		
	2	PB1 PIGGYBACK	10.00 0.00	11-05-14	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	77.90 51.34		
	2	PB2 PIGGYBACK	10.00 0.00	11-05-14	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	83.76 54.66		
	2	PB3 PIGGYBACK	10.00 0.00	11-05-14	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-10-08	85.46 55.34		
	3	PB4 PIGGYBACK	10.00 0.00	11-05-14	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 04-10-08	129.39 83.49		
	2	PB6 PIGGYBACK	6.00 0.00	10-08-03	05-08-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 05-08-04	78.52 49.00		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	5	J2 JACK-OPEN	9.00 0.00	05-10-08	05-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 05-11-02	115.15 75.00		
	5	J3 JACK-OPEN	6.00 0.00	05-07-00	03-01-07	2 X 4	2 X 4	01-03-08 -00-01-12	00-04-13 00-03-08	75.55 46.65		

TOTAL # TRUSS= 58.00

TOTAL BFT OF ALL TRUSSES=

2011.64 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3173.73 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267473 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 1A

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LJS26DS	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 2.00



Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267476 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 1B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	1	T1Z1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	2	T2 HALF HIP	10.00 0.00	19-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	172.38 109.00		
	2	T3 HALF HIP	10.00 0.00	19-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	169.36 105.34		
	2	T4 HALF HIP	10.00 0.00	19-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-04	183.84 116.00		
	11	T5 HALF HIP	10.00 0.00	19-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-04	1066.45 674.63		
	2	T6 HALF HIP	10.00 0.00	19-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-04	209.12 129.66		
	2	G7 PIGGYBACK	6.00 0.00	25-06-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	255.88 161.66		
	3	T8A COMMON	10.00 0.00	10-04-08	06-00-03	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-08-15	129.30 84.00		
	1	G8A COMMON	10.00 0.00	10-04-08	06-00-03	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-08-15	47.04 32.00		
	2	PB1 PIGGYBACK	10.00 0.00	11-05-14	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	77.90 51.34		
	2	PB2 PIGGYBACK	10.00 0.00	11-05-14	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	83.76 54.66		
	2	PB3 PIGGYBACK	10.00 0.00	11-05-14	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-10-08	85.46 55.34		
	3	PB4 PIGGYBACK	10.00 0.00	11-05-14	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 04-10-08	129.39 83.49		
	2	PB6 PIGGYBACK	6.00 0.00	10-08-03	05-08-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 05-08-04	78.52 49.00		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	5	J3 JACK-OPEN	6.00 0.00	05-07-00	03-01-07	2 X 4	2 X 4	01-03-08 -00-01-12	00-04-13 00-03-08	75.55 46.65		

TOTAL # TRUSS= 56.00

TOTAL BFT OF ALL TRUSSES=

2017.14 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3181.38 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267476 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGHAN SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 1B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 4.00



Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK:42181 LAYOUT ID: 267478 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 2A

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	1	T1Z1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	2	T2 HALF HIP	10.00 0.00	19-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	172.38 109.00		
	2	T3 HALF HIP	10.00 0.00	19-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	169.36 105.34		
	2	T4 HALF HIP	10.00 0.00	19-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-04	183.84 116.00		
	11	T5 HALF HIP	10.00 0.00	19-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-04	1066.45 674.63		
	2	T6 HALF HIP	10.00 0.00	19-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-04	209.12 129.66		
	2	G7 PIGGYBACK	6.00 0.00	25-06-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	255.88 161.66		
	1	G8 COMMON	10.00 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	51.05 34.00		
	3	T9A JACK-CLOSED	10.00 0.00	10-04-08	10-03-07	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 10-03-07	162.39 102.51		
	2	PB1 PIGGYBACK	10.00 0.00	11-05-14	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	77.90 51.34		
	2	PB2 PIGGYBACK	10.00 0.00	11-05-14	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	83.76 54.66		
	2	PB3 PIGGYBACK	10.00 0.00	11-05-14	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-10-08	85.46 55.34		
	3	PB4 PIGGYBACK	10.00 0.00	11-05-14	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 04-10-08	129.39 83.49		
	2	PB6 PIGGYBACK	6.00 0.00	10-08-03	05-08-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 05-08-04	78.52 49.00		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	5	J3 JACK-OPEN	6.00 0.00	05-07-00	03-01-07	2 X 4	2 X 4	01-03-08 -00-01-12	00-04-13 00-03-08	75.55 46.65		

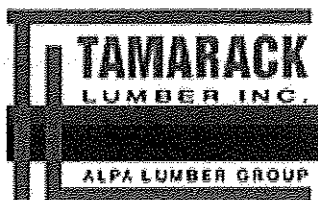
TOTAL # TRUSS= 56.00

TOTAL BFT OF ALL TRUSSES=

2037.65 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3218.48 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267478 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 2A

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 4.00



Delivery Shiplist

DATE	05/25/16
SALES REP	Mario

JOB TRACK:42181 LAYOUT ID: 267479 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 2B

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	1	T1Z1 HALF HIP	10.00 0.00	19-10-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-07-11 04-01-04	99.58 62.83		
	2	T2 HALF HIP	10.00 0.00	19-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-01-04	172.38 109.00		
	2	T3 HALF HIP	10.00 0.00	19-10-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	169.36 105.34		
	2	T4 HALF HIP	10.00 0.00	19-10-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-01-04	183.84 116.00		
	11	T5 HALF HIP	10.00 0.00	19-10-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-01-04	1066.45 674.63		
	2	T6 HALF HIP	10.00 0.00	19-10-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-01-04	209.12 129.66		
	2	G7 PIGGYBACK	6.00 0.00	25-06-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	255.88 161.66		
	3	T9A JACK-CLOSED	10.00 0.00	10-04-08	10-03-07	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 10-03-07	162.39 102.51		
	1	G9A MONOPITCH	10.00 0.00	10-04-08	10-03-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-03-07	61.70 40.00		
	2	PB1 PIGGYBACK	10.00 0.00	11-05-14	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 01-10-08	77.90 51.34		
	2	PB2 PIGGYBACK	10.00 0.00	11-05-14	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 02-10-08	83.76 54.66		
	2	PB3 PIGGYBACK	10.00 0.00	11-05-14	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 03-10-08	85.46 55.34		
	3	PB4 PIGGYBACK	10.00 0.00	11-05-14	04-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 04-10-08	129.39 83.49		
	2	PB6 PIGGYBACK	6.00 0.00	10-08-03	05-08-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 05-08-04	78.52 49.00		
	13	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	218.27 138.71		
	5	J3 JACK-OPEN	6.00 0.00	05-07-00	03-01-07	2 X 4	2 X 4	01-03-08 -00-01-12	00-04-13 00-03-08	75.55 46.65		

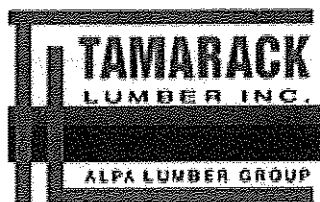
TOTAL # TRUSS= 56.00

TOTAL BFT OF ALL TRUSSES=

2043.65 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3229.13 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

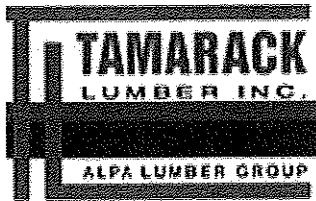
DATE	05/25/16
SALES REP	Mario

JOB TRACK: 42181 LAYOUT ID: 267479 LOCATION: VAUGHAN
 BUILDER: GREENPARK/STARTIME HOMES/VAUGH SUB-BUILDER:
 MODEL: ELMBROOK 4B ELEVATION: 2B

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 4.00



Delivery Shiplist

DATE	10/28/16
SALES REP	Mario

JOB TRACK:42181	LAYOUT ID: 274597	LOCATION: VAUGHAN
BUILDER: GREENPARK/STARTIME HOMES/VAUGH	SUB-BUILDER:	
MODEL: ELMBROOK 4B	ELEVATION: 3	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T20 FLAT GIRDER	0.00 0.00	19-08-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	98.43 61.33		
	1	T20Z FLAT GIRDER	0.00 0.00	19-08-08	04-01-04	2 X 4	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	98.43 61.33		
	2	T21 FLAT	0.00 0.00	19-08-08	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	05-01-04 05-01-04	162.88 100.66		
	2	T22 FLAT	0.00 0.00	19-08-08	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	06-01-04 06-01-04	173.76 108.34		
	2	T23 FLAT	0.00 0.00	19-08-08	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	07-01-04 07-01-04	184.96 116.00		
	13	T24 FLAT	0.00 0.00	19-08-08	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	08-01-04 08-01-04	1401.14 849.29		
	4	G25 GABLE	6.00 0.00	25-06-00	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-01-04	511.76 323.32		
	1	G26 MONOPITCH	9.00 0.00	10-07-00	09-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 09-05-08	58.35 37.33		
	3	T26A MONOPITCH	9.00 0.00	10-04-08	09-03-10	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 09-03-10	159.87 99.51		
	9	PB10 COMMON	6.00 0.00	23-02-00	05-09-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-04 00-00-04	628.56 398.97		
	2	PG10 COMMON	6.00 0.00	23-02-00	05-09-12	2 X 4	2 X 4	00-00-00 00-00-00	00-00-04 00-00-04	153.36 100.66		
	15	J10 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	251.85 160.05		
	6	J11 JACK-OPEN	4.50 0.00	05-07-08	02-05-05	2 X 4	2 X 4	01-03-08 00-00-00	00-04-00 00-04-05	89.76 55.98		

TOTAL # TRUSS= 61

TOTAL BFT OF ALL TRUSSES=

2472.77 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3973.11 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 4

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
S	9-11-4	-101	-101	—	BACK	VERT
T	11-11-4	-101	-101	—	BACK	VERT
U	15-11-4	-101	-101	—	BACK	VERT
V	17-11-4	-101	-101	—	BACK	VERT
W	1-11-4	-40	-70	—	BACK	VERT
X	3-11-4	-40	-70	—	BACK	VERT
Y	5-11-4	-40	-70	—	BACK	VERT
Z	7-11-4	-40	-70	—	BACK	VERT
AA	11-11-4	-40	-70	—	BACK	VERT
AB	13-11-4	-40	-70	—	BACK	VERT
AC	15-11-4	-40	-70	—	BACK	VERT
AD	17-11-4	-40	-70	—	BACK	VERT
						TOTAL



1624
DWG NO. TAN24323-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267478	T1Z1	1	1	TRUSS DESC.		

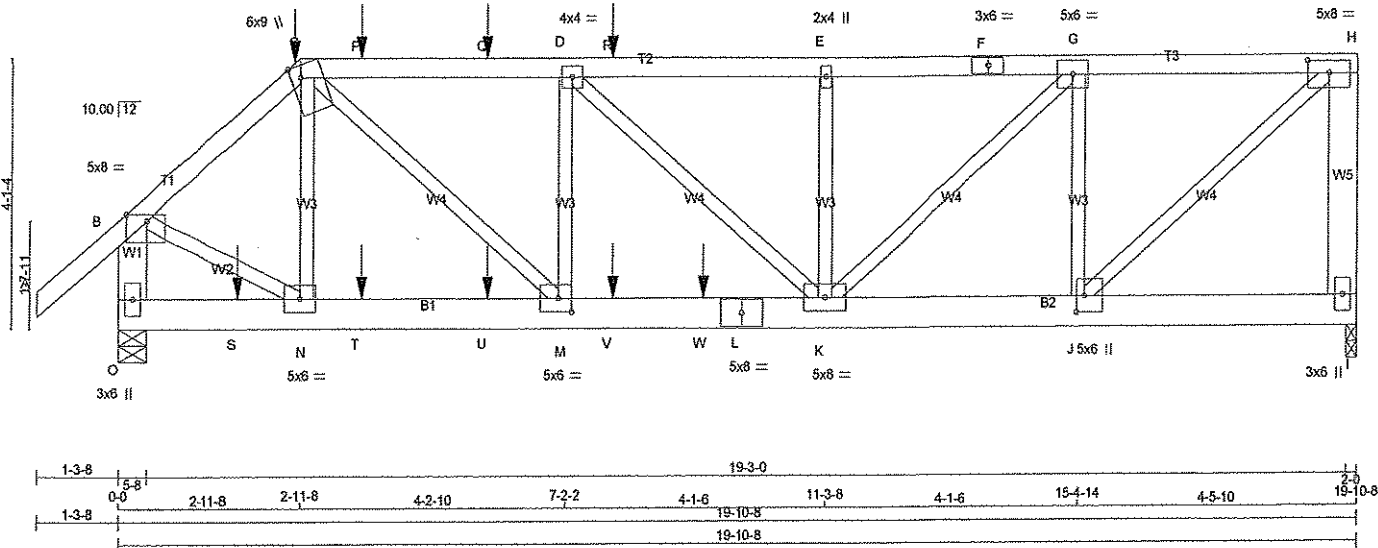
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 2-11-8 2-11-8 4-2-10 7-2-2 4-1-6 11-3-8 4-1-6 15-4-14 4-5-10 19-10-8

Scale = 1:34.4



TOTAL WEIGHT = 100 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	2100F 1.8E	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMW-p	MT20	5.0	8.0	Edge
C TTWW+m	MT20	6.0	9.0	Edge 1.75
D TMWW-t	MT20	4.0	4.0	
E TMW+w	MT20	2.0	4.0	
F TS-t	MT20	3.0	6.0	
G TMWW-t	MT20	5.0	6.0	
H TMWW-t	MT20	5.0	8.0	2.25 4.00
I BMW1+p	MT20	3.0	6.0	
J BMWW-t	MT20	5.0	6.0	3.00 1.50
K BMWW-t	MT20	5.0	8.0	
L BS-t	MT20	5.0	8.0	
M BMWW-t	MT20	5.0	6.0	2.50 2.50
N BMWW-t	MT20	5.0	6.0	
O BMW1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.8 lbs FACTORED DOWN AT 2-11-8, 102.2 lbs FACTORED DOWN AT 3-11-4, AND 101.4 lbs FACTORED DOWN AT 5-11-4, AND 101.4 lbs FACTORED DOWN AT 7-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, AND 69.9 lbs FACTORED DOWN AT 5-11-4, AND 69.9 lbs FACTORED DOWN AT 7-11-4, AND 102.7 lbs FACTORED DOWN AT 9-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1754	0	1754	0	2-0	1-14	
O	2237	0	2237	0	0	5-8	2-7

UNFACTORED REACTIONS							
	1ST LCASE	MAX	MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD
I	1431	809 / 0	316 / 0	0 / 0	0 / 0	306 / 0	0 / 0
O	1827	1031 / 0	405 / 0	0 / 0	0 / 0	391 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	
FR-TO		FROM	TO		FR-TO				
A-B	0 / 37	-84.3	-84.3	0.13 (1)	10.00	N-C	-514 / 0	0.13 (1)	
B-C	-2048 / 0	-84.3	-84.3	0.20 (1)	4.52	B-N	0 / 1688	0.42 (1)	
C-P	-3044 / 0	-84.3	-84.3	0.52 (1)	3.44	J-H	0 / 2385	0.59 (1)	
P-Q	-3044 / 0	-84.3	-84.3	0.52 (1)	3.44	C-M	0 / 1965	0.49 (1)	
Q-D	-3044 / 0	-84.3	-84.3	0.52 (1)	3.44	J-G	-1488 / 0	0.38 (1)	
D-R	-3009 / 0	-84.3	-84.3	0.51 (1)	3.46	M-D	-575 / 0	0.15 (1)	
R-E	-3009 / 0	-84.3	-84.3	0.51 (1)	3.46	K-G	0 / 1590	0.39 (1)	
E-F	-3009 / 0	-84.3	-84.3	0.40 (1)	3.62	D-K	-47 / 0	0.02 (1)	
F-G	-3009 / 0	-84.3	-84.3	0.40 (1)	3.62	K-E	-305 / 0	0.06 (1)	
G-H	-1831 / 0	-84.3	-84.3	0.31 (1)	4.60				
I-H	-1680 / 0	0.0	0.0	0.27 (1)	7.59				
O-B	-2185 / 0	0.0	0.0	0.16 (1)	6.87				
O-S	0 / 0	-28.0	-28.0	0.03 (3)	10.00				
S-N	0 / 0	-28.0	-28.0	0.03 (3)	10.00				
N-T	0 / 1553	-28.0	-28.0	0.19 (1)	10.00				
T-U	0 / 1553	-28.0	-28.0	0.19 (1)	10.00				
U-M	0 / 1553	-28.0	-28.0	0.19 (1)	10.00				
M-V	0 / 3044	-28.0	-28.0	0.51 (1)	10.00				
V-W	0 / 3044	-28.0	-28.0	0.51 (1)	10.00				
W-L	0 / 3044	-28.0	-28.0	0.51 (1)	10.00				
L-K	0 / 3044	-28.0	-28.0	0.51 (1)	10.00				
K-J	0 / 1831	-28.0	-28.0	0.19 (1)	10.00				
J-I	0 / 0	-28.0	-28.0	0.04 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	TOTAL	
C	2-11-8	-150	-150	-	FRONT	VERT	TOTAL		
P	3-11-4	-102	-102	-	FRONT	VERT	TOTAL		
Q	5-11-4	-101	-101	-	FRONT	VERT	TOTAL		
R	7-11-4	-101	-101	-	FRONT	VERT	TOTAL		
S	1-11-4	-40	-70	-	FRONT	VERT	TOTAL		
T	3-11-4	-40	-70	-	FRONT	VERT	TOTAL		
U	5-11-4	-40	-70	-	FRONT	VERT	TOTAL		
V	7-11-4	-40	-70	-	FRONT	VERT	TOTAL		
W	9-4-8	-1028	-1028	-	FRONT	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.66")
CALCULATED VERT. DEFL (LL) = $L/999$ (0.12")
ALLOWABLE DEFL (TL) = $L/360$ (0.66")
CALCULATED VERT. DEFL (TL) = $L/999$ (0.20")

CSI: TC=0.52 (C-D:1), BC=0.51 (K-M:1), WB=0.59 (H-J:1), SSI=0.40 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

OTOSOLVE RIGHT HEEL ONLY

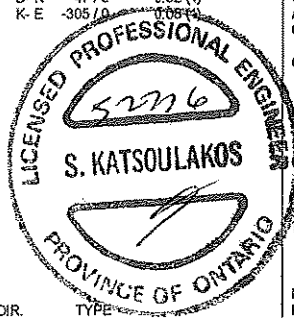
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.90 (L) (INPUT = 1.00)



DWG NO. TAM 2734/-16

STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267473	T1Z2	1	1	TRUSS DESC.		

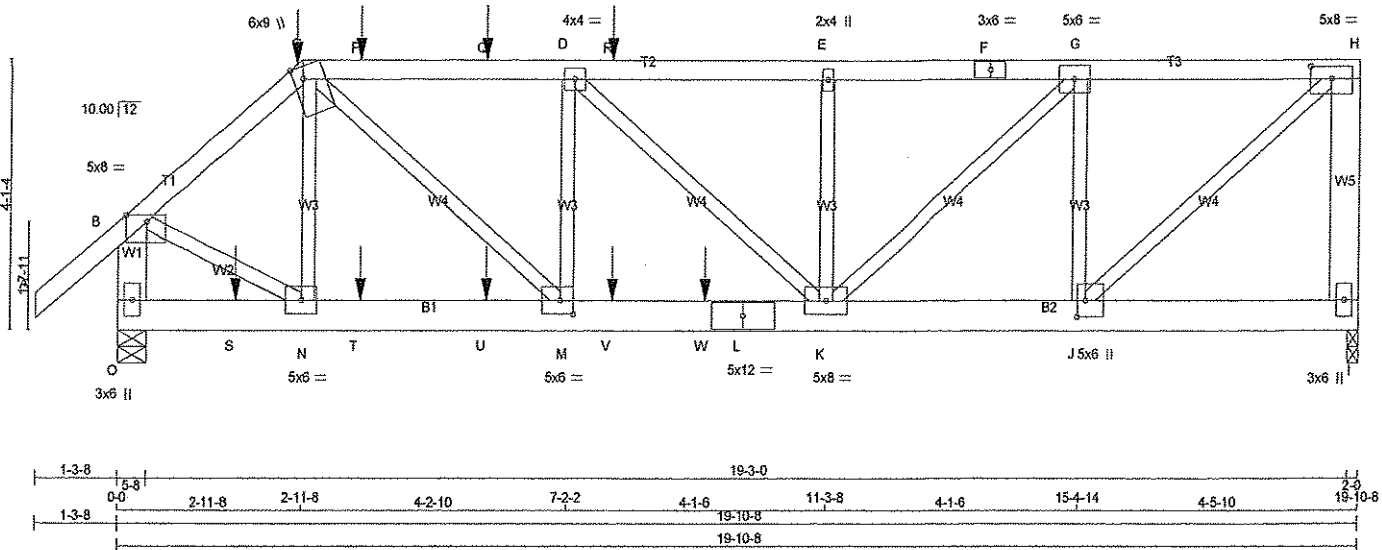
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 2-11-8 2-11-8 4-2-10 7-2-2 4-1-6 11-3-8 4-1-6 15-4-14 4-5-10 19-10-8

Scale = 1:34.4



TOTAL WEIGHT = 100 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
I - H	2x6	DRY No.2	SPF
O - B	2x6	DRY No.2	SPF
O - L	2x6	DRY 2100F 1.8E	SPF
L - I	2x6	DRY 2100F 1.8E	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge 1.75	
D	TMWW-I	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-I	MT20	5.0	6.0		
H	TMVW-I	MT20	5.0	8.0	2.25	4.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-I	MT20	5.0	6.0	3.00	1.50
K	BMWW-I	MT20	5.0	8.0		
L	BS-t	MT20	5.0	12.0		
M	BMWW-I	MT20	5.0	6.0	2.50	2.50
N	BMWW-I	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.8 lbs FACTORED DOWN AT 2-11-8, 104.0 lbs FACTORED DOWN AT 3-10-12, AND 101.4 lbs FACTORED DOWN AT 4-10-12, AND 101.4 lbs FACTORED DOWN AT 7-10-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-10-12, 69.9 lbs FACTORED DOWN AT 3-10-12, 69.9 lbs FACTORED DOWN AT 5-10-12, AND 69.9 lbs FACTORED DOWN AT 7-10-12, AND 1078.9 lbs FACTORED DOWN AT 9-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
I	1777	0	1777	0
O	2267	0	2267	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD
I	1484	805/0	339/0	0/0	0/0	0/0	321/0
O	1866	1027/0	431/0	0/0	0/0	0/0	408/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1) (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1) (LBS)	MAX. FACTORED VERT. LOAD (LC1) (LBS)
FR-TO				FR-TO			
A-B	0/37	-84.3	-84.3 0.13 (1)	10.00	N-C	-528/0	0.13 (1)
B-C	-2077/0	-84.3	-84.3 0.21 (1)	4.49	B-N	0/1712	0.42 (1)
C-P	-3098/0	-84.3	-84.3 0.53 (1)	3.41	J-H	0/2421	0.60 (1)
P-Q	-3098/0	-84.3	-84.3 0.53 (1)	3.41	C-M	0/2028	0.50 (1)
Q-D	-3098/0	-84.3	-84.3 0.53 (1)	3.41	J-G	-1515/0	0.38 (1)
D-R	-3067/0	-84.3	-84.3 0.52 (1)	3.42	M-D	-578/0	0.15 (1)
R-E	-3067/0	-84.3	-84.3 0.52 (1)	3.42	K-G	0/1630	0.40 (1)
E-F	-3067/0	-84.3	-84.3 0.41 (1)	3.58	D-K	-43/0	0.02 (1)
F-G	-3067/0	-84.3	-84.3 0.41 (1)	3.58	K-E	-303/0	0.08 (1)
G-H	-1859/0	-84.3	-84.3 0.31 (1)	4.57			
I-H	-1703/0	0.0	0.0 0.27 (1)	7.55			
O-B	-2213/0	0.0	0.0 0.16 (1)	6.93			
O-S	0/0	-28.0	-28.0 0.03 (3)	10.00			
S-N	0/0	-28.0	-28.0 0.03 (3)	10.00			
N-T	0/1575	-28.0	-28.0 0.20 (1)	10.00			
T-U	0/1575	-28.0	-28.0 0.20 (1)	10.00			
U-M	0/1575	-28.0	-28.0 0.20 (1)	10.00			
M-V	0/3098	-28.0	-28.0 0.53 (1)	10.00			
V-W	0/3098	-28.0	-28.0 0.53 (1)	10.00			
W-L	0/3098	-28.0	-28.0 0.53 (1)	10.00			
L-K	0/3098	-28.0	-28.0 0.53 (1)	10.00			
K-J	0/1859	-28.0	-28.0 0.20 (1)	10.00			
J-I	0/0	-28.0	-28.0 0.05 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
C	2-11-8	-150	-150	-	FRONT	VERT	TOTAL
P	3-10-12	-104	-104	-	FRONT	VERT	TOTAL
Q	5-10-12	-101	-101	-	FRONT	VERT	TOTAL
R	7-10-12	-101	-101	-	FRONT	VERT	TOTAL
S	1-10-12	-40	-70	-	FRONT	VERT	TOTAL
T	3-10-12	-40	-70	-	FRONT	VERT	TOTAL
U	5-10-12	-40	-70	-	FRONT	VERT	TOTAL
V	7-10-12	-40	-70	-	FRONT	VERT	TOTAL
W	9-4-8	-1079	-1079	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.66")
CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.66")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CSI: TC=0.53 (C-D-1), BC=0.53 (K-M-1), WB=0.80 (H-J-1), SSI=0.42 (K-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	618	354
	1867	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

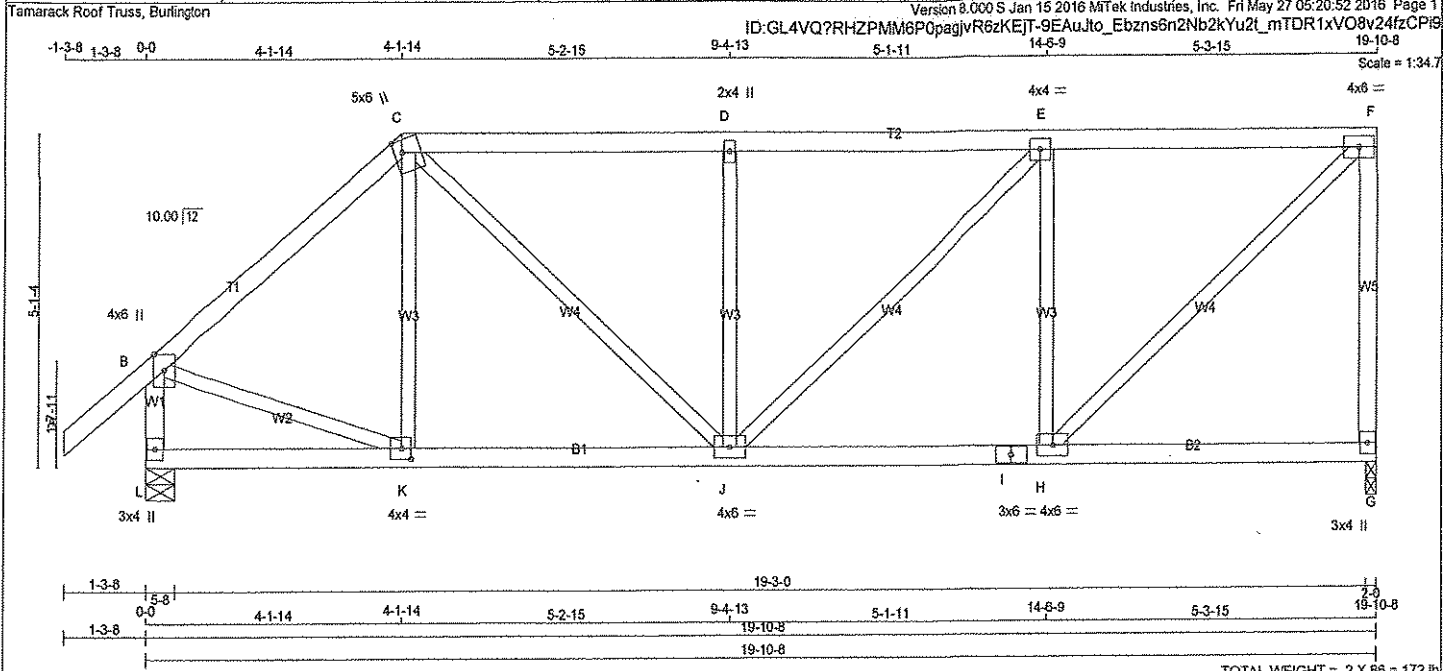
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.93 (L) (INPUT = 1.00)

DWG NO. TAM 2432416

STRUCTURAL
COMPONENT ONLY

JOB NAME 267473	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. 42181	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Fri May 27 05:20:52 2016 Page 1	



TOTAL WEIGHT = 2 X 88 = 172 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTVW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWw-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMVt+p	MT20	3.0	4.0		
H	BMWw-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWw-t	MT20	4.0	6.0		
K	BMWw-t	MT20	4.0	4.0	2.00	1.75
L	BMVt+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
G	1116	0	1116	0	2-0	1-8
L	1232	0	1232	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
G	916	509 / 0	209 / 0	0 / 0	0 / 0	199 / 0	0 / 0
L	995	580 / 0	209 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRACED LENGTH	MEMB.	FACTORED		
		VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. FACTORED CSI (LC)					
FR-TO		FROM	TO			FR-TO			
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	K-C	-90 / 134	0.03 (1)	
B-C	-1021 / 0	-84.3	-84.3	0.28 (1)	5.85	B-K	0 / 818	0.18 (1)	
C-D	-1203 / 0	-84.3	-84.3	0.33 (1)	5.41	H-F	0 / 1303	0.29 (1)	
D-E	-1203 / 0	-84.3	-84.3	0.35 (1)	5.39	C-J	0 / 572	0.13 (1)	
E-F	-987 / 0	-84.3	-84.3	0.34 (1)	5.87	H-E	-707 / 0	0.27 (1)	
F-G	-1055 / 0	0.0	0.0	0.47 (1)	7.68	J-D	-472 / 0	0.18 (1)	
L-B	-1188 / 0	0.0	0.0	0.13 (1)	7.32	J-E	0 / 323	0.07 (1)	
L-K	0 / 0	-28.0	-28.0	0.15 (3)	10.00				
K-J	0 / 780	-28.0	-28.0	0.24 (2)	10.00				
J-I	0 / 967	-28.0	-28.0	0.30 (2)	10.00				
I-H	0 / 967	-28.0	-28.0	0.30 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.19 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.47 (F-G:1), BC=0.30 (H-J:2), WB=0.29 (F-H:1), SSI=0.21 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	PL
MT20	618	354	1667	822
	MAX	MIN	MAX	MIN
	618	354	1667	822

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)



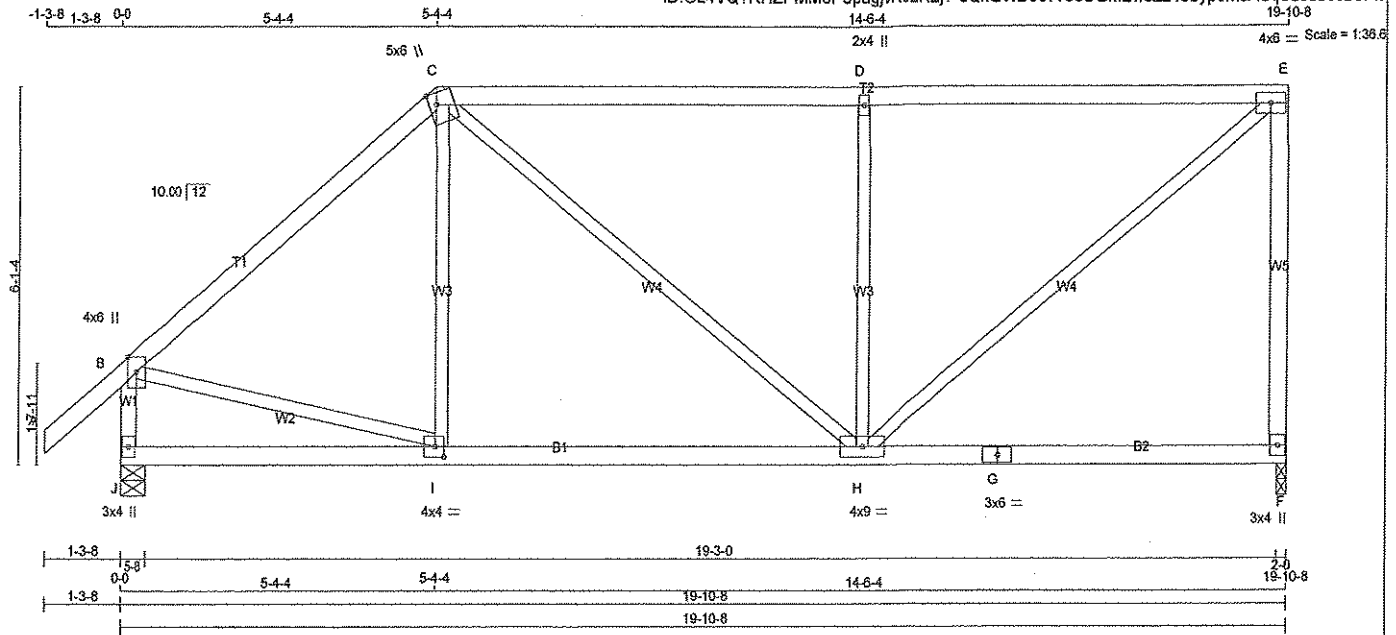
DRWG NO. **YAM 24325-16**
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
267473	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 85 = 169 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMVW+w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BS-t	MT20	3.0	6.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BMVWW-t	MT20	4.0	4.0	2.00 1.75
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
F	1116	0	1116	0	0	2-0	1-8		
J	1232	0	1232	0	0	5-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT	REACTIONS	WIND	DEAD	SOIL
F	918	508 / 0	209 / 0	PERM	LIVE	0 / 0	0 / 0	199 / 0
J	995	580 / 0	209 / 0	PERM	LIVE	0 / 0	0 / 0	207 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-C	0 / 229	0.05 (3)	
B-C	-1008 / 0	-84.3	-84.3	0.48 (1)	5.57	C-H	0 / 298	0.07 (1)	
C-D	-1005 / 0	-84.3	-84.3	0.84 (1)	4.64	H-D	-758 / 0	0.44 (1)	
D-E	-1005 / 0	-84.3	-84.3	0.84 (1)	4.63	H-E	0 / 1285	0.29 (1)	
F-E	-1034 / 0	0.0	0.0	0.75 (1)	7.72	B-I	0 / 794	0.18 (1)	
J-B	-1174 / 0	0.0	0.0	0.12 (1)	7.36				
J-I	0 / 0	-28.0	-28.0	0.24 (3)	10.00				
I-H	0 / 773	-28.0	-28.0	0.46 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.37 (3)	10.00				
G-F	0 / 0	-28.0	-28.0	0.37 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.66")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.66")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.84 (D-E:1), BC=0.46 (H-I:2), WB=0.44 (D-H:1), SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)

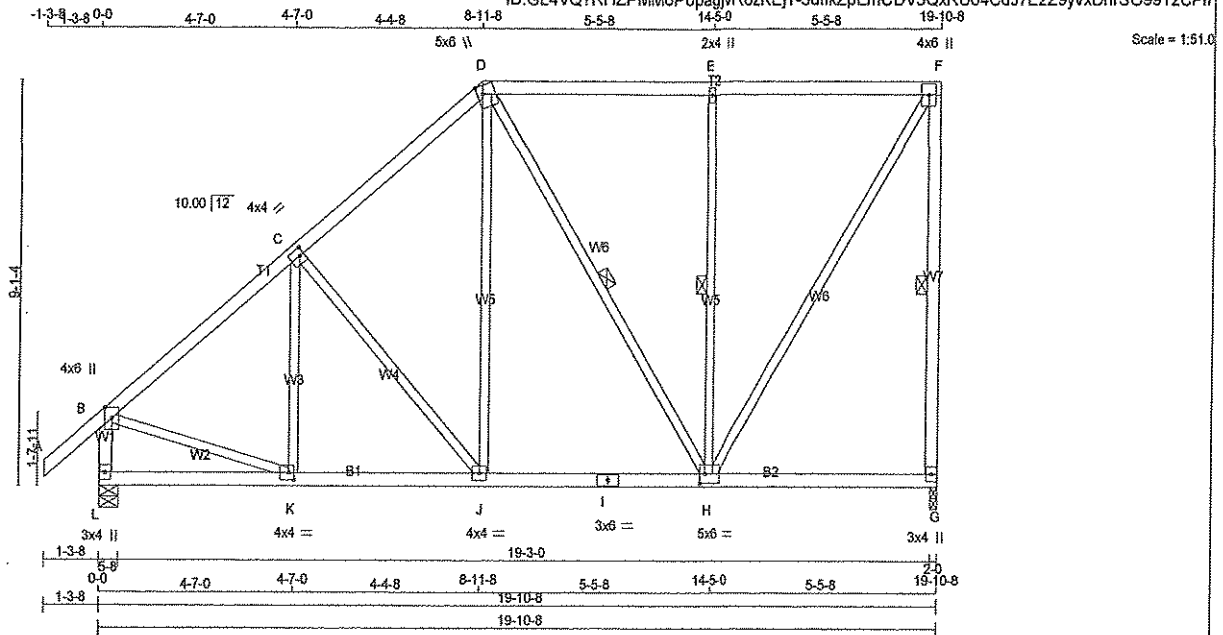
JSI METAL= 0.29 (I) (INPUT = 1.00)



DWG NO. TAN 24326-10
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 24328-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 105 = 209 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWVW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1116	0	1116	0	2-0	1-8
L	1232	0	1232	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
G	916	509 / 0	209 / 0	0 / 0	199 / 0	0 / 0
L	995	580 / 0	209 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	K-C	-94 / 131	0.04 (1)	
B-C	-1035 / 0	-84.3	-84.3	0.31 (1)	5.77	C-J	-290 / 0	0.25 (1)	
C-D	-847 / 0	-84.3	-84.3	0.30 (1)	6.22	J-D	0 / 374	0.08 (2)	
D-E	-547 / 0	-84.3	-84.3	0.43 (1)	6.25	D-H	-159 / 0	0.12 (1)	
E-F	-547 / 0	-84.3	-84.3	0.43 (1)	6.25	H-E	-569 / 0	0.31 (1)	
G-F	-1054 / 0	0.0	0.0	0.41 (1)	6.13	H-F	0 / 1034	0.23 (1)	
L-B	-1179 / 0	0.0	0.0	0.13 (1)	7.35	B-K	0 / 850	0.19 (1)	
L-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00				
K-J	0 / 817	-28.0	-28.0	0.23 (2)	10.00				
J-I	0 / 631	-28.0	-28.0	0.28 (2)	10.00				
I-H	0 / 631	-28.0	-28.0	0.28 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.66")$
CALCULATED VERT. DEFL.(LL) = $L/999 (0.04")$
ALLOWABLE DEFL.(TL) = $L/360 (0.66")$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.07")$

CSI: TC=0.43 (E-F:1), BC=0.28 (H-J:2), WB=0.31 (E-H:1), SSI=0.22 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

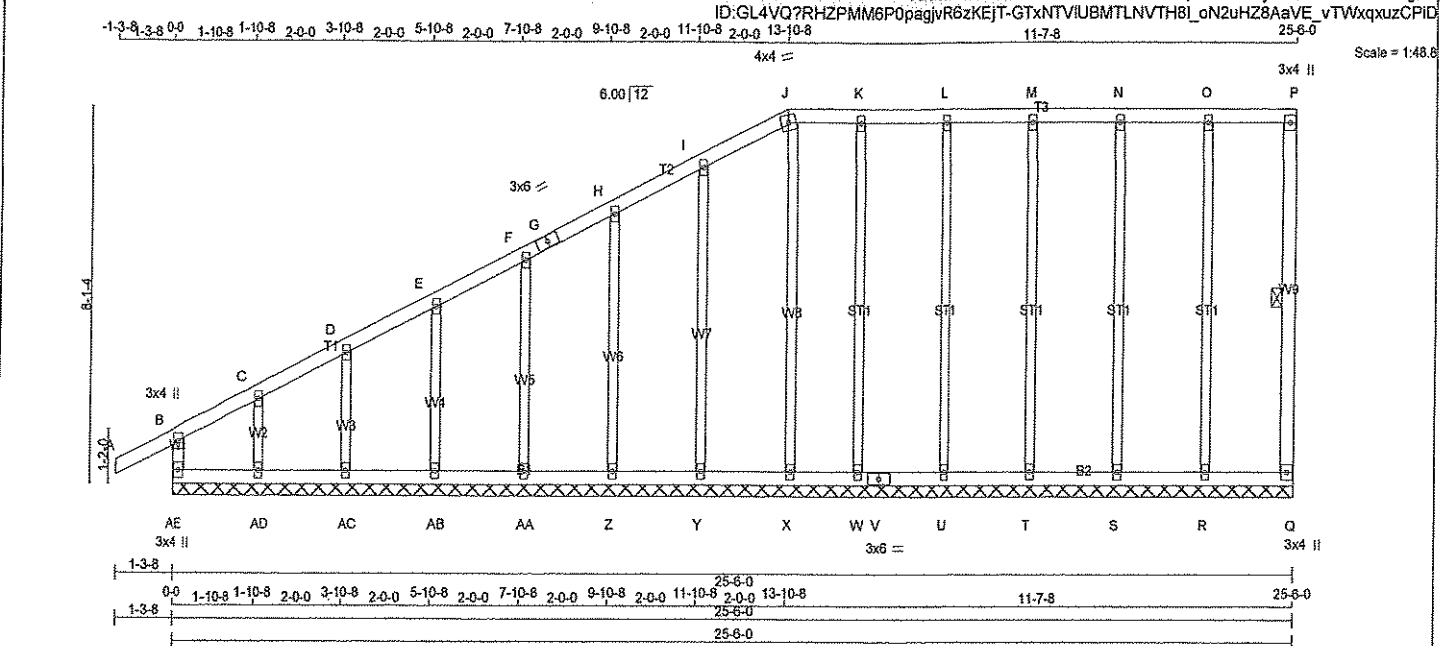
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

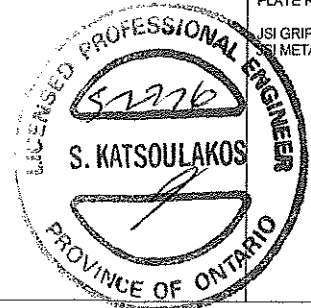
JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)



DRWG NO. TAM 24324-16
STRUCTURAL
COMPONENT ONLY



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF AE- B 2x4 DRY No.2 SPF A - G 2x4 DRY No.2 SPF G - J 2x4 DRY No.2 SPF J - P 2x4 DRY No.2 SPF Q - P 2x4 DRY No.2 SPF AE- V 2x4 DRY No.2 SPF V - Q 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-Q. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4)				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 46.1 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.11 (A-B:1), BC=0.03 (R-S:3), WB=0.23 (O-R:1), SSI=0.08 (A-B:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLJ) (PLJ) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.55 (J) (INPUT = 0.90) JSI METAL= 0.04 (J) (INPUT = 1.00)																																																																																																																																																																																																																																																																																			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 3.0 4.0 C, D, E, F, H, I, K, L, M, N, O C TMW+w MT20 2.0 4.0 G TS-t MT20 3.0 6.0 J TTW-m MT20 4.0 4.0 P TMV+p MT20 3.0 4.0 Q BMV1+p MT20 3.0 4.0 R, S, T, U, V, W, X, Y, Z, AA, AB, AC, AD R BMV1+w MT20 2.0 4.0 V BS-t MT20 3.0 6.0 AE BMV1+p MT20 3.0 4.0				<table> <tr> <th colspan="2">CHORDS</th><th colspan="2">FACTORED</th><th colspan="2">WEBS</th><th colspan="2">FACTORED</th></tr> <tr> <th>MEMB.</th><th>FORCE (LBS)</th><th>VERT. LOAD (PLF)</th><th>MAX. CSI (LC)</th><th>MEMB.</th><th>FORCE (LBS)</th><th>MAX. CSI (LC)</th><th></th></tr> <tr> <td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr> <tr> <td>AE-B</td><td>-208 / 0</td><td>0.0 0.0 0.01 (1)</td><td>7.81</td><td>R-O</td><td>-179 / 0</td><td>0.23 (1)</td><td></td></tr> <tr> <td>A-B</td><td>0 / 26</td><td>-84.3 -84.3 0.11 (1)</td><td>10.00</td><td>S-N</td><td>-168 / 0</td><td>0.21 (1)</td><td></td></tr> <tr> <td>B-C</td><td>-25 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>6.25</td><td>T-M</td><td>-168 / 0</td><td>0.21 (1)</td><td></td></tr> <tr> <td>C-D</td><td>-18 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>6.25</td><td>U-L</td><td>-172 / 0</td><td>0.22 (1)</td><td></td></tr> <tr> <td>D-E</td><td>-14 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>6.25</td><td>W-K</td><td>-151 / 0</td><td>0.19 (1)</td><td></td></tr> <tr> <td>E-F</td><td>-11 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>6.25</td><td>X-J</td><td>-148 / 0</td><td>0.18 (1)</td><td></td></tr> <tr> <td>F-G</td><td>-8 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td>Y-I</td><td>-169 / 0</td><td>0.15 (1)</td><td></td></tr> <tr> <td>G-H</td><td>-8 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td>Z-H</td><td>-168 / 0</td><td>0.10 (1)</td><td></td></tr> <tr> <td>H-I</td><td>-6 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td>AA-F</td><td>-167 / 0</td><td>0.07 (1)</td><td></td></tr> <tr> <td>I-J</td><td>-6 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td>AB-E</td><td>-167 / 0</td><td>0.04 (1)</td><td></td></tr> <tr> <td>J-K</td><td>-2 / 0</td><td>-84.3 -84.3 0.03 (1)</td><td>10.00</td><td>AC-D</td><td>-168 / 0</td><td>0.03 (1)</td><td></td></tr> <tr> <td>K-L</td><td>-2 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td>AD-C</td><td>-161 / 0</td><td>0.02 (1)</td><td></td></tr> <tr> <td>L-M</td><td>-2 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>M-N</td><td>-2 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>N-O</td><td>-2 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>O-P</td><td>-2 / 0</td><td>-84.3 -84.3 0.04 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Q-P</td><td>-75 / 0</td><td>0.0 0.0 0.01 (1)</td><td>6.25</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AE-AD</td><td>0 / 22</td><td>-28.0 -28.0 0.03 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AD-AC</td><td>0 / 16</td><td>-28.0 -28.0 0.03 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AC-AB</td><td>0 / 12</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AB-AA</td><td>0 / 10</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AA-Z</td><td>0 / 7</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Z-Y</td><td>0 / 5</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Y-X</td><td>0 / 4</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>X-W</td><td>0 / 2</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>W-V</td><td>0 / 2</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>V-U</td><td>0 / 2</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>U-T</td><td>0 / 2</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>T-S</td><td>0 / 2</td><td>-28.0 -28.0 0.02 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>S-R</td><td>0 / 2</td><td>-28.0 -28.0 0.03 (3)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>R-Q</td><td>0 / 2</td><td>-28.0 -28.0 0.03 (3)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> </table>				CHORDS		FACTORED		WEBS		FACTORED		MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)		FR-TO		FROM TO		FR-TO				AE-B	-208 / 0	0.0 0.0 0.01 (1)	7.81	R-O	-179 / 0	0.23 (1)		A-B	0 / 26	-84.3 -84.3 0.11 (1)	10.00	S-N	-168 / 0	0.21 (1)		B-C	-25 / 0	-84.3 -84.3 0.04 (1)	6.25	T-M	-168 / 0	0.21 (1)		C-D	-18 / 0	-84.3 -84.3 0.04 (1)	6.25	U-L	-172 / 0	0.22 (1)		D-E	-14 / 0	-84.3 -84.3 0.04 (1)	6.25	W-K	-151 / 0	0.19 (1)		E-F	-11 / 0	-84.3 -84.3 0.04 (1)	6.25	X-J	-148 / 0	0.18 (1)		F-G	-8 / 0	-84.3 -84.3 0.04 (1)	10.00	Y-I	-169 / 0	0.15 (1)		G-H	-8 / 0	-84.3 -84.3 0.04 (1)	10.00	Z-H	-168 / 0	0.10 (1)		H-I	-6 / 0	-84.3 -84.3 0.04 (1)	10.00	AA-F	-167 / 0	0.07 (1)		I-J	-6 / 0	-84.3 -84.3 0.04 (1)	10.00	AB-E	-167 / 0	0.04 (1)		J-K	-2 / 0	-84.3 -84.3 0.03 (1)	10.00	AC-D	-168 / 0	0.03 (1)		K-L	-2 / 0	-84.3 -84.3 0.04 (1)	10.00	AD-C	-161 / 0	0.02 (1)		L-M	-2 / 0	-84.3 -84.3 0.04 (1)	10.00					M-N	-2 / 0	-84.3 -84.3 0.04 (1)	10.00					N-O	-2 / 0	-84.3 -84.3 0.04 (1)	10.00					O-P	-2 / 0	-84.3 -84.3 0.04 (1)	10.00					Q-P	-75 / 0	0.0 0.0 0.01 (1)	6.25					AE-AD	0 / 22	-28.0 -28.0 0.03 (2)	10.00					AD-AC	0 / 16	-28.0 -28.0 0.03 (2)	10.00					AC-AB	0 / 12	-28.0 -28.0 0.02 (2)	10.00					AB-AA	0 / 10	-28.0 -28.0 0.02 (2)	10.00					AA-Z	0 / 7	-28.0 -28.0 0.02 (2)	10.00					Z-Y	0 / 5	-28.0 -28.0 0.02 (2)	10.00					Y-X	0 / 4	-28.0 -28.0 0.02 (2)	10.00					X-W	0 / 2	-28.0 -28.0 0.02 (2)	10.00					W-V	0 / 2	-28.0 -28.0 0.02 (2)	10.00					V-U	0 / 2	-28.0 -28.0 0.02 (2)	10.00					U-T	0 / 2	-28.0 -28.0 0.02 (2)	10.00					T-S	0 / 2	-28.0 -28.0 0.02 (2)	10.00					S-R	0 / 2	-28.0 -28.0 0.03 (3)	10.00					R-Q	0 / 2	-28.0 -28.0 0.03 (3)	10.00					 DWG NO. TAM24330 STRUCTURAL COMPONENT ONLY			
CHORDS		FACTORED		WEBS		FACTORED																																																																																																																																																																																																																																																																																					
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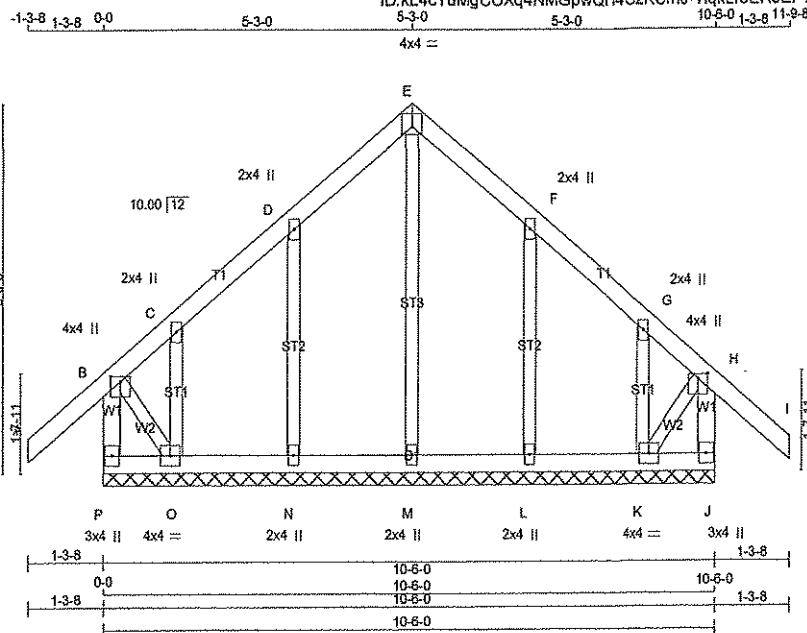
DWG NO. TAM24330
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267478	G8	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:KL4cYUMgCUXq4NMGpwQH4GzKCMJ-YqkLr8ERoEFtIoQbkvmqh0vey0AJDW7vFvAvzCPW5



Scale = 1:36.9

TOTAL WEIGHT = 51 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-250 / 0	0.0	0.0 0.03 (1)	M-E	-131 / 0	0.07 (1)	
A-B	0 / 37	-84.3	-84.3 0.12 (1)	N-D	-204 / 0	0.06 (1)	
B-C	-55 / 0	-84.3	-84.3 0.11 (1)	O-C	-52 / 0	0.01 (1)	
C-D	-1 / 0	-84.3	-84.3 0.05 (1)	L-F	-204 / 0	0.06 (1)	
D-E	-18 / 0	-84.3	-84.3 0.05 (1)	K-G	-52 / 0	0.01 (1)	
E-F	-18 / 0	-84.3	-84.3 0.05 (1)	S-O	0 / 16	0.00 (1)	
F-G	-1 / 0	-84.3	-84.3 0.05 (1)	K-H	0 / 16	0.00 (1)	
G-H	-65 / 0	-84.3	-84.3 0.11 (1)				
H-I	0 / 37	-84.3	-84.3 0.12 (1)				
J-H	-250 / 0	0.0	0.0 0.03 (1)				
P-O	0 / 0	-28.0	-28.0 0.02 (3)				
O-N	0 / 9	-28.0	-28.0 0.02 (2)				
N-M	0 / 4	-28.0	-28.0 0.02 (2)				
M-L	0 / 4	-28.0	-28.0 0.02 (2)				
L-K	0 / 9	-28.0	-28.0 0.02 (2)				
K-J	0 / 0	-28.0	-28.0 0.02 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, GCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (A-B:1), BC=0.02 (K-L:2), WB=0.07 (E-M:1), SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (FLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (E) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



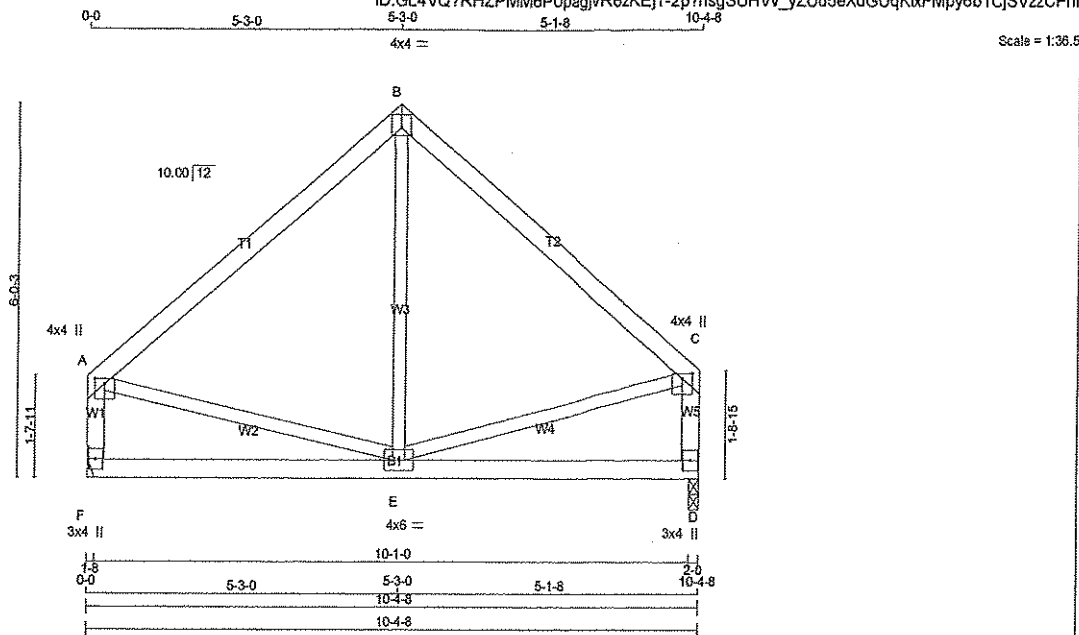
DRWG NO. TAM 24348 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267476	T8A	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:GL4VQ?RHZPMM6P0pagivR6zKEJT-2p?hsgSUHVv_yzUd5eXdGUqKbFmpy6b1CjSVzzCPHl



TOTAL WEIGHT = 3 X 43 = 129 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
F - A	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
F	582	0	582	0
D	582	0	582	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
COMBINED	SNOW						
F	478	268 / 0	109 / 0	0 / 0	0 / 0	104 / 0	0 / 0
D	478	268 / 0	109 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
MEMB.	FORCE	VERT. LOAD	LC1	LC2	LC3	LC4	LC5	LC6	LC7	LC8	LC9	LC10
FR-TO		FROM	TO									
A-B	-367 / 0	-84.3	-84.3	0.30 (1)	6.25	E-B	0 / 231	0.05 (3)				
B-C	-367 / 0	-84.3	-84.3	0.29 (1)	6.25	A-E	0 / 260	0.07 (1)				
F-A	-526 / 0	0.0	0.0	0.06 (1)	7.81	E-C	0 / 292	0.07 (1)				
D-C	-526 / 0	0.0	0.0	0.06 (1)	7.81							
F-E	0 / 0	-28.0	-28.0	0.22 (3)	10.00							
E-D	0 / 0	-28.0	-28.0	0.22 (3)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.30 (A-B:1), BC=0.22 (E-F:3), WB=0.07 (C-E:1), SSI=0.13 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

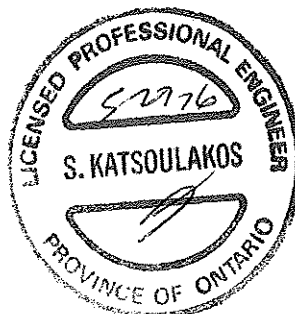
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.13 (A) (INPUT = 1.00)



DRWG NO. TAM 2433216
STRUCTURAL
COMPONENT ONLY

JOB NAME 267476	TRUSS NAME G8A	QUANTITY 1	PLY 1	JOB DESC. 42181	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

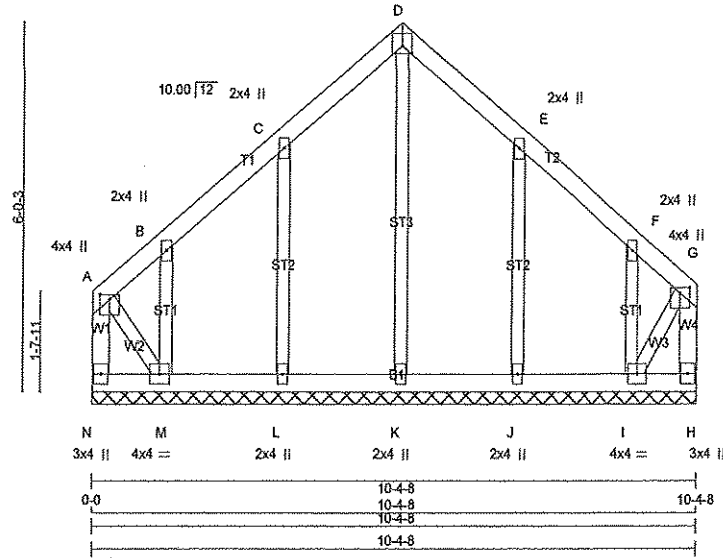
Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Fri May 27 05:21:42 2016 Page 1

ID:GL4VQ?RHZPMM6P0pagjvR6zKEJT-hsCpzOLTyGhsobfi5ySZR7XrwbE8izsuw0hqmzCPhN

0-0 5-3-0 5-1-8 10-4-8

4x4 =

Scale = 1:36.0



TOTAL WEIGHT = 47 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
N - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A TMVW+p	MT20	4.0	4.0	1.00 2.00
B, C, E, F				
B TMW+w	MT20	2.0	4.0	
D TTW-p	MT20	4.0	4.0	1.50 2.00
G TMVW+p	MT20	4.0	4.0	1.00 2.00
H BMV1+p	MT20	3.0	4.0	
I BMVW1-t	MT20	4.0	4.0	
J, K, L				
J BMV1+w	MT20	2.0	4.0	
M BMVW1-t	MT20	4.0	4.0	
N BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
N-A	-85/0	0.0	0.0 0.01 (1)	K-D	-110/0	0.06 (1)	
A-B	-26/0	-84.3	-84.3 0.03 (1)	L-C	-195/0	0.05 (1)	
B-C	-24/0	-84.3	-84.3 0.05 (1)	M-B	-137/0	0.02 (1)	
C-D	-35/0	-84.3	-84.3 0.05 (1)	J-E	-196/0	0.06 (1)	
D-E	-35/0	-84.3	-84.3 0.05 (1)	I-F	-133/0	0.02 (1)	
E-F	-24/0	-84.3	-84.3 0.05 (1)	A-M	0/41	0.01 (1)	
F-G	-25/0	-84.3	-84.3 0.03 (1)	I-G	0/45	0.01 (1)	
H-G	-84/0	0.0	0.0 0.01 (1)				
N-M	0/0	-28.0	-28.0 0.02 (2)				
M-L	0/23	-28.0	-28.0 0.02 (2)				
L-K	0/18	-28.0	-28.0 0.02 (2)				
K-J	0/18	-28.0	-28.0 0.02 (2)				
J-I	0/23	-28.0	-28.0 0.02 (2)				
I-H	0/0	-28.0	-28.0 0.02 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (D-E:1), BC=0.02 (I-J:2), WB=0.06 (D-K:1), SSI=0.06 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.18 (D) (INPUT = 0.90)

JSI METAL= 0.05 (E) (INPUT = 1.00)



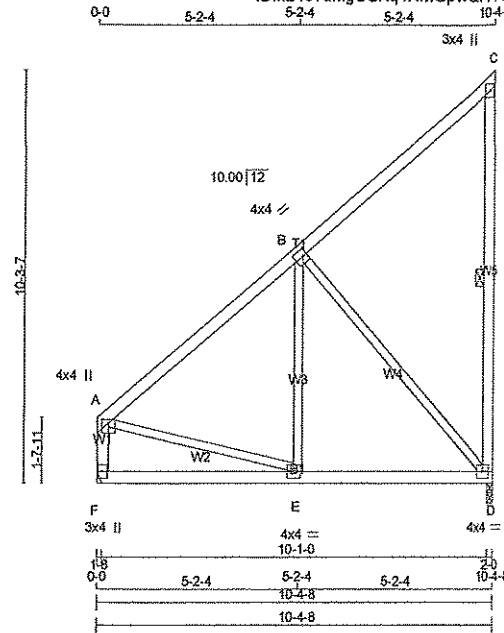
DRWG NO. YAM 2433616
STRUCTURAL
COMPONENT ONLY

JOB NAME 267478	TRUSS NAME T9A	QUANTITY 3	PLY 1	JOB DESC. 42181 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:56.5

TOTAL WEIGHT = 3 X 54 = 162 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	BMVW-t	MT20	4.0	4.0	
E	BMVW-t	MT20	4.0	4.0	
F	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	BRG	IN-SX	IN-SX
F	582	0	582	0	0	2-0	1-8		
D	582	0	582	0	0	2-0	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	478	268 / 0	109 / 0	0 / 0	0 / 0	104 / 0	0 / 0
D	478	268 / 0	109 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. (LC)	UNBRAC LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX. (LC)
F-A	-527 / 0	0.0	0.0	0.08 (1)	7.81	A-E	0 / 344	0.08 (1)	
A-B	-400 / 0	-84.3	-84.3	0.30 (1)	6.25	E-B	0 / 274	0.08 (3)	
B-C	-35 / 0	-84.3	-84.3	0.29 (1)	6.25	B-D	-492 / 0	0.56 (1)	
D-C	-165 / 0	0.0	0.0	0.08 (1)	6.25				
F-E	0 / 0	-28.0	-28.0	0.22 (3)	10.00				
E-D	0 / 334	-28.0	-28.0	0.26 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.30 (A-B:1), BC=0.26 (D-E:2), WB=0.56 (B-D:1), SSI=0.16 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

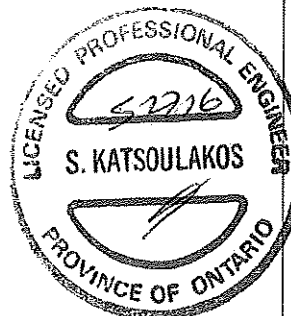
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (A) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)



DWG NO. TAM 24349-16
STRUCTURAL
COMPONENT ONLY

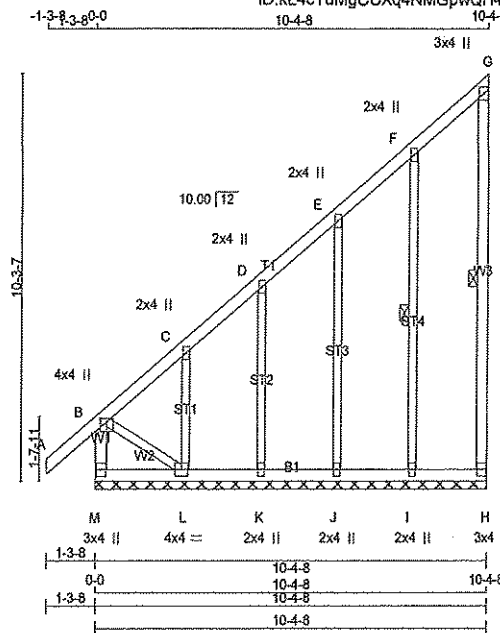
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267479	G9A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:kL4cYuMgCUXq4NMGpwQH4GzKcmJ-5Nreqxbf9X9TRc1WEoVsE5S064g1qtUP10Yw0zCPVJ

Scale = 1:57.2



TOTAL WEIGHT = 62 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
M - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C, D, E, F					
C	TMV+w	MT20	2.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMV1+p	MT20	3.0	4.0	
I, J, K					
I	BMV1+w	MT20	2.0	4.0	
L	BMVW1-t	MT20	4.0	4.0	
M	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO			FR-TO			
M-B	-213/0	0.0	0.0	0.02 (1)	7.81	I-F	-189/0	0.09 (1)
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	J-E	-165/0	0.14 (1)
B-C	-6/0	-84.3	-84.3	0.06 (1)	10.00	K-D	-156/0	0.06 (1)
C-D	-15/0	-84.3	-84.3	0.06 (1)	6.25	L-C	-203/0	0.04 (1)
D-E	-5/0	-84.3	-84.3	0.04 (1)	10.00	B-L	0/15	0.00 (1)
E-F	-1/0	-84.3	-84.3	0.05 (1)	10.00			
F-G	-10/0	-84.3	-84.3	0.05 (1)	6.25			
H-G	-72/0	0.0	0.0	0.04 (1)	6.25			
M-L	0/0	-28.0	-28.0	0.04 (2)	10.00			
L-K	0/8	-28.0	-28.0	0.04 (2)	10.00			
K-J	0/5	-28.0	-28.0	0.02 (2)	10.00			
J-I	0/2	-28.0	-28.0	0.03 (2)	10.00			
I-H	0/0	-28.0	-28.0	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (A-B:1), BC=0.04 (K-L:2), WB=0.14 (E-J:1), SS=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

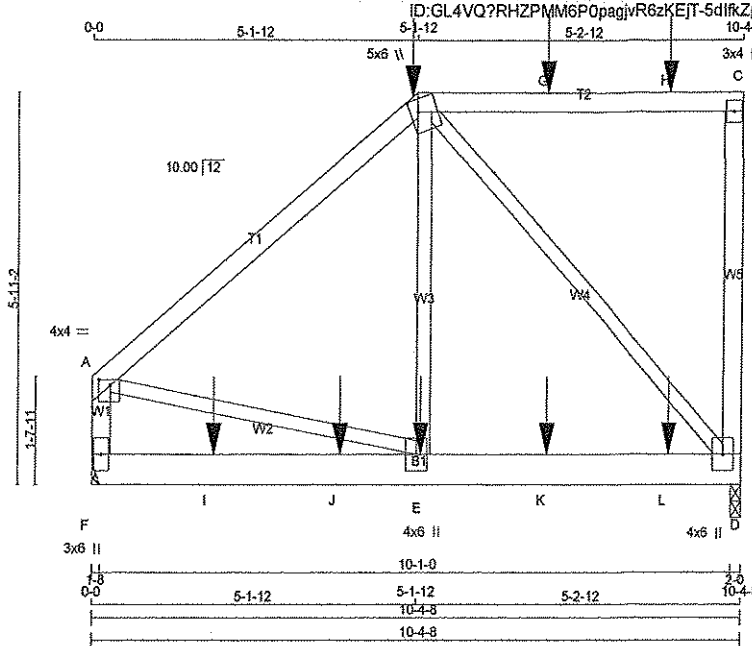


DWG NO. TAM 24355-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
267473	T10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:34.3

TOTAL WEIGHT = 54 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 428.7 lbs FACTORED DOWN AT 5-1-12, AND 163.3 lbs FACTORED DOWN AT 7-2-8, AND 163.3 lbs FACTORED DOWN AT 9-2-8 ON TOP CHORD, AND 88.2 lbs FACTORED DOWN AT 1-11-4, 88.2 lbs FACTORED DOWN AT 3-11-4, 88.2 lbs FACTORED DOWN AT 5-2-8, AND 88.2 lbs FACTORED DOWN AT 7-2-8, AND 88.2 lbs FACTORED DOWN AT 9-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	1187	0	1187	0
F	985	0	985	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	984	531 / 0	234 / 0	0 / 0	0 / 0	219 / 0	0 / 0
F	838	417 / 0	222 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	-822 / 0	-84.3 -84.3 0.46 (1)	5.99	E-B	0 / 532	0.13 (3)	
B-G	0 / 0	-84.3 -84.3 0.83 (1)	10.00	B-D	-823 / 0	0.99 (1)	
G-H	0 / 0	-84.3 -84.3 0.83 (1)	10.00	A-E	0 / 647	0.16 (1)	
H-C	0 / 0	-84.3 -84.3 0.83 (1)	10.00				
D-C	-412 / 0	0.0 0.0 0.26 (1)	7.81				
F-A	-892 / 0	0.0 0.0 0.10 (1)	7.81				
F-I	0 / 0	-28.0 -28.0 0.20 (3)	10.00				
I-J	0 / 0	-28.0 -28.0 0.20 (3)	10.00				
J-E	0 / 0	-28.0 -28.0 0.20 (3)	10.00				
E-K	0 / 634	-28.0 -28.0 0.26 (2)	10.00				
K-L	0 / 634	-28.0 -28.0 0.26 (2)	10.00				
L-D	0 / 634	-28.0 -28.0 0.26 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-1-12	-429	-429	—	FRONT	VERT	TOTAL
E	5-2-8	-50	-88	—	FRONT	VERT	TOTAL
G	7-2-8	-163	-163	—	FRONT	VERT	TOTAL
H	9-2-8	-163	-163	—	FRONT	VERT	TOTAL
I	1-11-4	-50	-88	—	FRONT	VERT	TOTAL
J	3-11-4	-50	-88	—	FRONT	VERT	TOTAL
K	7-2-8	-50	-88	—	FRONT	VERT	TOTAL
L	9-2-8	-50	-88	—	FRONT	VERT	TOTAL



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(65 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.83 (B-C:1), BC=0.26 (D-E:2), WB=0.99 (B-D:1), SSI=0.35 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

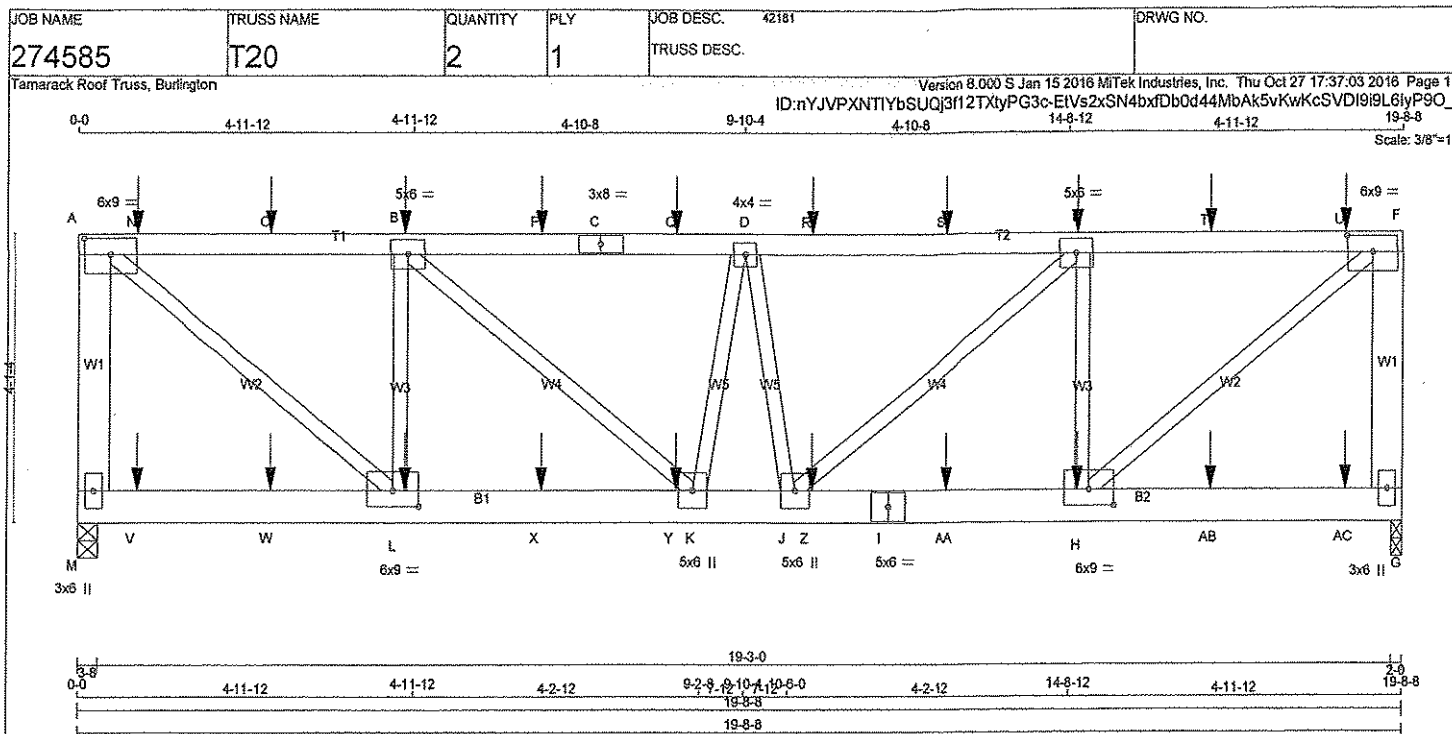
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.78 (A) (INPUT = 0.90)
JSI METAL= 0.24 (A) (INPUT = 1.00)

DRWG NO. TAM 24331-16
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
M - A	2x6	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x6	DRY	No.2
M - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.75	4.50
B	TMVW-t	MT20	5.0	6.0		
C	TS-t	MT20	3.0	8.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	TMVW-t	MT20	6.0	9.0	2.75	4.50
G	BMV1+p	MT20	3.0	6.0		
H	BMVW-t	MT20	6.0	9.0	2.75	4.50
I	BS-t	MT20	5.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	6.0	9.0	2.75	4.50
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 107.6 lbs FACTORED DOWN AT 10-4, 101.4 lbs FACTORED DOWN AT 2-10-4, 101.4 lbs FACTORED DOWN AT 4-10-4, 101.4 lbs FACTORED DOWN AT 6-10-4, 101.4 lbs FACTORED DOWN AT 8-10-4, 101.4 lbs FACTORED DOWN AT 10-10-4, 101.4 lbs FACTORED DOWN AT 12-10-4, 101.4 lbs FACTORED DOWN AT 14-10-4, 101.4 lbs FACTORED DOWN AT 16-10-4, 101.4 lbs FACTORED DOWN AT 18-10-4 ON TOP CHORD, AND 73.5 lbs FACTORED DOWN AT 10-4, 69.9 lbs FACTORED DOWN AT 2-10-4, 69.9 lbs FACTORED DOWN AT 4-10-4, 69.9 lbs FACTORED DOWN AT 6-10-4, 69.9 lbs FACTORED DOWN AT 8-10-4, 69.9 lbs FACTORED DOWN AT 10-10-4, 69.9 lbs FACTORED DOWN AT 12-10-4, 69.9 lbs FACTORED DOWN AT 14-10-4, AND 69.9 lbs FACTORED DOWN AT 16-10-4, AND 73.5 lbs FACTORED DOWN AT 18-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
M	1822	0	1822	0
G	1822	0	1822	0

UNFACTORED REACTIONS

	1ST LCASE	MAX MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
M	1509	816 / 0	358 / 0
G	1509	816 / 0	358 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
M-A	-1716 / 0	0.0	0.0 0.27 (1)	A-L	0 / 2360	0.58 (1)	
A-N	-1861 / 0	-84.3	-84.3 0.58 (1)	L-B	-1197 / 0	0.30 (1)	
N-O	-1861 / 0	-84.3	-84.3 0.58 (1)	B-K	0 / 673	0.17 (1)	
O-B	-1861 / 0	-84.3	-84.3 0.58 (1)	J-E	0 / 694	0.17 (1)	
B-P	-2374 / 0	-84.3	-84.3 0.64 (1)	H-E	-1194 / 0	0.30 (1)	
P-C	-2374 / 0	-84.3	-84.3 0.64 (1)	H-F	0 / 2356	0.58 (1)	
C-Q	-2374 / 0	-84.3	-84.3 0.64 (1)	K-D	-326 / 0	0.08 (1)	
Q-D	-2374 / 0	-84.3	-84.3 0.64 (1)	D-J	-337 / 0	0.09 (1)	
D-R	-2386 / 0	-84.3	-84.3 0.61 (1)				
R-S	-2386 / 0	-84.3	-84.3 0.61 (1)				
S-E	-2386 / 0	-84.3	-84.3 0.61 (1)				
E-T	-1857 / 0	-84.3	-84.3 0.55 (1)				
T-U	-1857 / 0	-84.3	-84.3 0.55 (1)				
U-F	-1857 / 0	-84.3	-84.3 0.55 (1)				
F-G	-1716 / 0	0.0	0.0 0.27 (1)				
M-V	0 / 0	-28.0	-28.0 0.12 (2)				
V-W	0 / 0	-28.0	-28.0 0.12 (2)				
W-L	0 / 0	-28.0	-28.0 0.12 (2)				
L-X	0 / 1861	-28.0	-28.0 0.32 (1)				
X-Y	0 / 1861	-28.0	-28.0 0.32 (1)				
Y-K	0 / 1861	-28.0	-28.0 0.32 (1)				
K-J	0 / 2440	-28.0	-28.0 0.34 (1)				
J-Z	0 / 1857	-28.0	-28.0 0.32 (1)				
Z-I	0 / 1857	-28.0	-28.0 0.32 (1)				
I-AA	0 / 1857	-28.0	-28.0 0.32 (1)				
AA-H	0 / 1857	-28.0	-28.0 0.32 (1)				
H-AB	0 / 0	-28.0	-28.0 0.12 (2)				
AB-AC	0 / 0	-28.0	-28.0 0.12 (2)				
AC-G	0 / 0	-28.0	-28.0 0.12 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
B	4-10-4	-101	-101		BACK	VERT	TOTAL
E	14-10-4	-101	-101		BACK	VERT	TOTAL
H	14-10-4	-40	-70		BACK	VERT	TOTAL
L	4-10-4	-40	-70		BACK	VERT	TOTAL
N	10-4	-108	-108		BACK	VERT	TOTAL
O	2-10-4	-101	-101		BACK	VERT	TOTAL
P	6-10-4	-101	-101		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.64 (B-D:1), BC=0.34 (J-K:1), WB=0.58 (A-L:1), SS=0.30 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.65 (L) (INPUT = 0.90)
JSI METAL=0.35 (I) (INPUT = 1.00)

DWG NO. TAM47156-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	T20	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:nYJVPXNTIYbSUQj3f12TXlyPG3c-EtVs2xSN4bxfDb0d44MbAk5vKwKcSVDI9I9L6lyP9O

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	8-10-4	-101	-101	---	BACK	VERT	TOTAL
R	10-10-4	-101	-101	---	BACK	VERT	TOTAL
S	12-10-4	-101	-101	---	BACK	VERT	TOTAL
T	16-10-4	-101	-101	---	BACK	VERT	TOTAL
U	18-10-4	-108	-108	---	BACK	VERT	TOTAL
V	10-4	-42	-73	---	BACK	VERT	TOTAL
W	2-10-4	-40	-70	---	BACK	VERT	TOTAL
X	6-10-4	-40	-70	---	BACK	VERT	TOTAL
Y	8-10-4	-40	-70	---	BACK	VERT	TOTAL
Z	10-10-4	-40	-70	---	BACK	VERT	TOTAL
AA	12-10-4	-40	-70	---	BACK	VERT	TOTAL
AB	16-10-4	-40	-70	---	BACK	VERT	TOTAL
AC	18-10-4	-42	-73	---	BACK	VERT	TOTAL



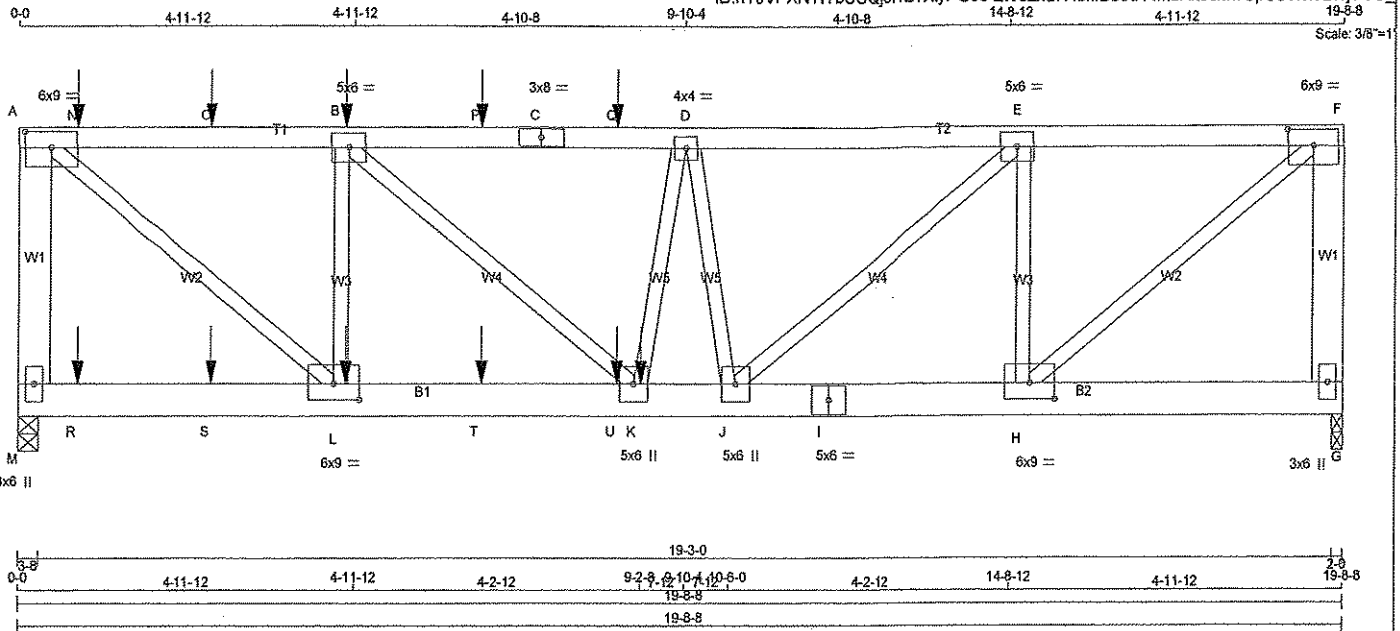
DWG NO. TAM47156-10
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	T20Z	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:nYJVPXNTIYbSUQ3f12TXlyPG3c-Elvs2xSN4bxD0d44MbAk5sMwGpSSvI9L6lyP90



TOTAL WEIGHT = 2 X 98 = 197 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
M - A	2x6	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x6	DRY	No.2
M - I	2x6	DRY	No.2
I - G	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	6.0	9.0	2.75 4.50
B	TMVW-t	MT20	5.0	8.0	
C	TS-t	MT20	3.0	8.0	
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	5.0	6.0	
F	TMVW-t	MT20	6.0	9.0	2.75 4.50
G	BMV1+p	MT20	3.0	6.0	
H	BMVW-t	MT20	6.0	9.0	2.75 4.50
I	BS-t	MT20	5.0	6.0	
J	BMVW-t	MT20	5.0	6.0	
K	BMVW-t	MT20	5.0	6.0	
L	BMVW-t	MT20	6.0	9.0	2.75 4.50
M	BMV1+p	MT20	3.0	6.0	

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 107.6 lbs FACTORED DOWN AT 10-4, 101.4 lbs FACTORED DOWN AT 2-10-4, 101.4 lbs FACTORED DOWN AT 4-10-4, AND 101.4 lbs FACTORED DOWN AT 8-10-4 ON TOP CHORD, AND 73.5 lbs FACTORED DOWN AT 10-4, 69.9 lbs FACTORED DOWN AT 2-10-4, 69.9 lbs FACTORED DOWN AT 4-10-4, 69.9 lbs FACTORED DOWN AT 6-10-4, AND 69.9 lbs FACTORED DOWN AT 8-10-4, AND 1299.5 lbs FACTORED DOWN AT 9-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	2342	0	2342	0	0
G	1885	0	1885	0	0

UNFACTORED REACTIONS					
1ST LCASE	MAX/MIN	COMPONENT	REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
M	1804	1088 / 0	413 / 0	0 / 0	0 / 0
G	1526	883 / 0	324 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	LC1 MAX
	(LBS)	(PLF)	CS1 (LC)		(LBS)	CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
M-A	-2241 / 0	0.0	0.0 0.36 (1)	6.79	A-L	0 / 3217	0.80 (1)
A-N	-2537 / 0	-84.3	-84.3 0.66 (1)	3.52	L-B	-1682 / 0	0.43 (1)
N-O	-2537 / 0	-84.3	-84.3 0.66 (1)	3.52	B-K	0 / 1398	0.35 (1)
O-B	-2537 / 0	-84.3	-84.3 0.66 (1)	3.52	J-E	0 / 1648	0.41 (1)
B-P	-3602 / 0	-84.3	-84.3 0.83 (1)	2.80	H-E	-1515 / 0	0.38 (1)
P-C	-3602 / 0	-84.3	-84.3 0.83 (1)	2.80	H-F	0 / 2698	0.67 (1)
C-Q	-3602 / 0	-84.3	-84.3 0.83 (1)	2.80	K-D	0 / 374	0.09 (1)
Q-D	-3602 / 0	-84.3	-84.3 0.83 (1)	2.80	D-J	-902 / 0	0.24 (1)
D-E	-3384 / 0	-84.3	-84.3 0.54 (1)	3.31			
E-F	-2128 / 0	-84.3	-84.3 0.38 (1)	4.25			
G-F	-1823 / 0	0.0	0.0 0.29 (1)	7.36			
M-R	0 / 0	-28.0	-28.0 0.13 (3)	10.00			
R-S	0 / 0	-28.0	-28.0 0.13 (3)	10.00			
S-L	0 / 0	-28.0	-28.0 0.13 (3)	10.00			
L-T	0 / 2537	-28.0	-28.0 0.47 (1)	10.00			
T-U	0 / 2537	-28.0	-28.0 0.47 (1)	10.00			
U-K	0 / 2537	-28.0	-28.0 0.47 (1)	10.00			
K-J	0 / 3527	-28.0	-28.0 0.59 (1)	10.00			
J-I	0 / 2128	-28.0	-28.0 0.33 (1)	10.00			
I-H	0 / 2128	-28.0	-28.0 0.33 (1)	10.00			
H-G	0 / 0	-28.0	-28.0 0.07 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	MAX+	FACE DIR.
B	4-10-4	-101	-101	-	BACK VERT
K	9-2-8	-1300	-1300	-	BACK VERT
L	4-10-4	-40	-70	-	BACK VERT
N	10-4	-108	-108	-	BACK VERT
O	2-10-4	-101	-101	-	BACK VERT
P	6-10-4	-101	-101	-	BACK VERT
Q	8-10-4	-101	-101	-	BACK VERT
R	10-4	-42	-73	-	BACK VERT
S	2-10-4	-40	-70	-	BACK VERT
T	6-10-4	-40	-70	-	BACK VERT
U	8-10-4	-40	-70	-	BACK VERT

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
 ALLOWABLE DEFL.(TL)= L/360 (0.66")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.83 (B-D:1), BC=0.59 (J-K:1), WB=0.80 (A-L:1), SSI=0.30 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

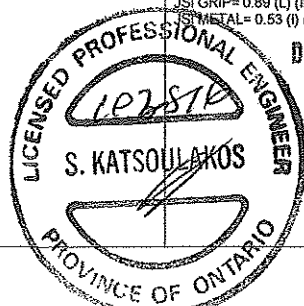
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP=0.89 (L) (INPUT = 0.90)
 ISI METAL=0.53 (I) (INPUT = 1.00)



DRWG NO. TAM47157-16
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	T21	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

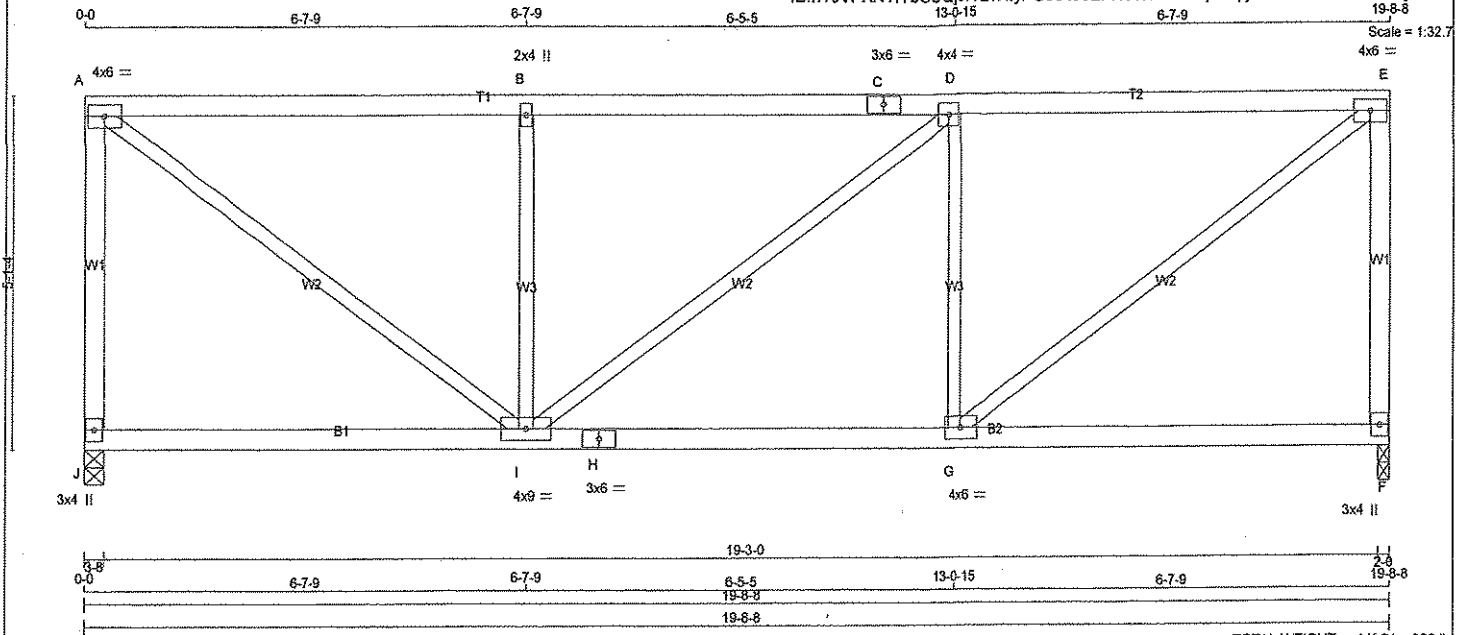
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13-0-15

19-8-8

Scale = 1:32.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0			
B	TMVW-1	MT20	2.0	4.0			
C	TS-1	MT20	3.0	6.0			
D	TMVW-1	MT20	4.0	4.0			
E	TMVW-1	MT20	4.0	6.0			
F	BMV1+p	MT20	3.0	4.0			
G	BMVW-1	MT20	4.0	6.0			
H	BS-1	MT20	3.0	6.0			
I	BMVW-1	MT20	4.0	9.0			
J	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
JT	1106	0	1106	0
F	1106	0	1106	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
JT	908	504/0	207/0
F	908	504/0	207/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC	MEMB.	MAX. FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC
FR-TO														
J-A	-1030/0	0.0	0.0	0.45	(1)	7.74	G-E	-0/1369	0.31	(1)				
A-B	-1107/0	-84.3	-84.3	0.55	(1)	5.23	A-I	0/1369	0.31	(1)				
B-C	-1107/0	-84.3	-84.3	0.55	(1)	5.22	G-D	-804/0	0.23	(1)				
C-D	-1107/0	-84.3	-84.3	0.55	(1)	5.22	I-B	-603/0	0.23	(1)				
D-E	-1106/0	-84.3	-84.3	0.55	(1)	5.22	I-D	-1/0	0.00	(1)				
F-E	-1030/0	0.0	0.0	0.45	(1)	7.73								
J-I	0/0	-28.0	-28.0	0.28	(3)	10.00								
I-H	0/1108	-28.0	-28.0	0.41	(2)	10.00								
H-G	0/1108	-28.0	-28.0	0.41	(2)	10.00								
G-F	0/0	-28.0	-28.0	0.28	(3)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
 ALLOWABLE DEFL.(TL)= L/360 (0.66")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.55 (D-E-1), BC=0.41 (G-I-2), WB=0.31 (E-G-1), SSI=0.26 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1658

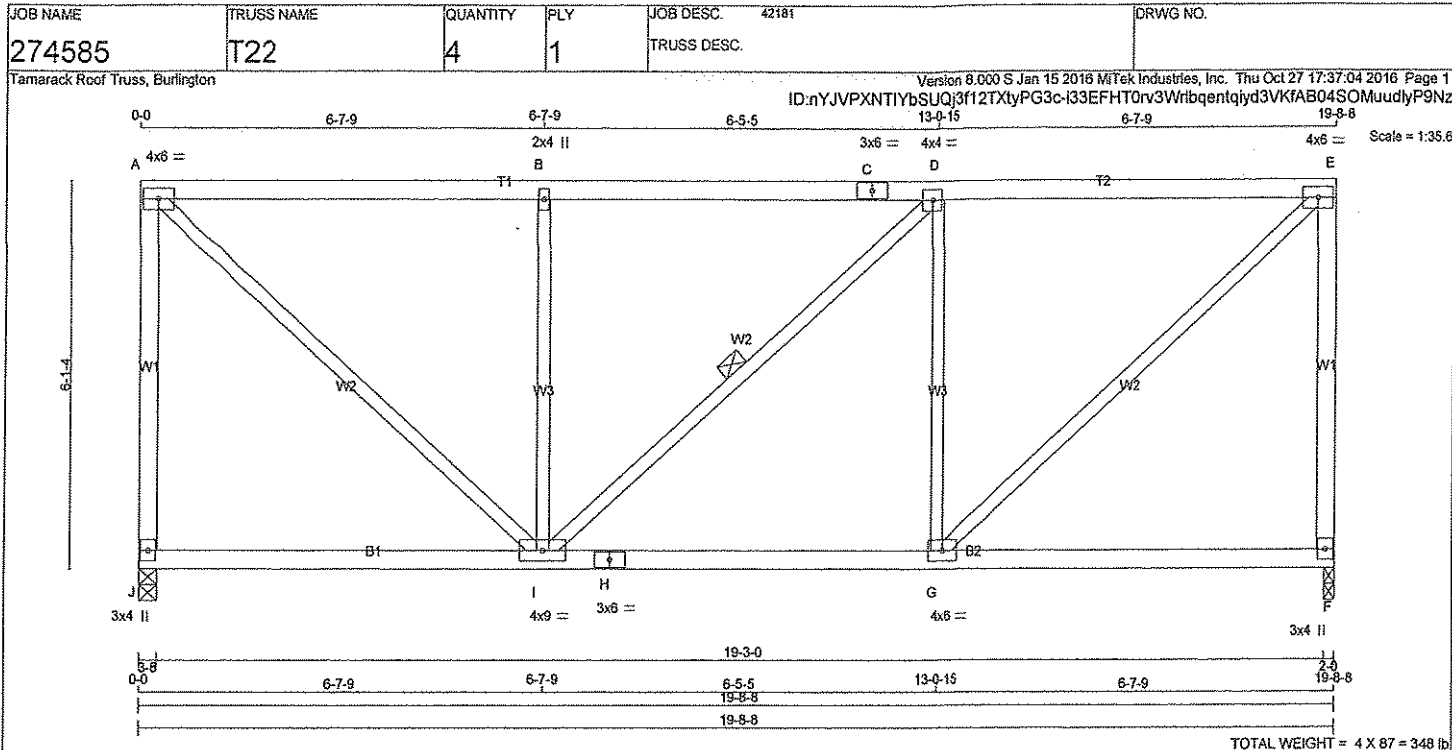
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (G) (INPUT = 0.90)
 JSI METAL= 0.34 (H) (INPUT = 1.00)



DWG NO. TAM47158-10
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
J - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMWV-t	MT20	4.0	6.0	
B	TMWV-w	MT20	2.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMWV-t	MT20	4.0	4.0	
E	TMWV-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWV-t	MT20	4.0	6.0	
H	BS-t	MT20	3.0	6.0	
I	BMWV-t	MT20	4.0	9.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
J	1106 0	1106 0	0	3-8	1-8
F	1106 0	1106 0	0	2-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
J	908	504 / 0	207 / 0	0 / 0	197 / 0
F	908	504 / 0	207 / 0	0 / 0	197 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
J-A	-1030 / 0	0.0 0.0 0.74 (1)	7.73	G-E	0 / 1221
A-B	-917 / 0	-84.3 -84.3 0.54 (1)	5.63	A-I	0 / 1219
B-C	-917 / 0	-84.3 -84.3 0.54 (1)	5.62	G-D	-804 / 0
C-D	-917 / 0	-84.3 -84.3 0.54 (1)	5.62	I-B	-804 / 0
D-E	-918 / 0	-84.3 -84.3 0.54 (1)	5.62	I-D	-2 / 0
F-E	-1031 / 0	0.0 0.0 0.74 (1)	7.73		
J-I	0 / 0	-28.0 -28.0 0.28 (3)	10.00		
I-H	0 / 918	-28.0 -28.0 0.39 (2)	10.00		
H-G	0 / 918	-28.0 -28.0 0.39 (2)	10.00		
G-F	0 / 0	-28.0 -28.0 0.28 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.74 (E-F:1), BC=0.39 (G-I:2), WB=0.35 (D-G:1), SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.80)
JSI METAL= 0.31 (H) (INPUT = 1.00)

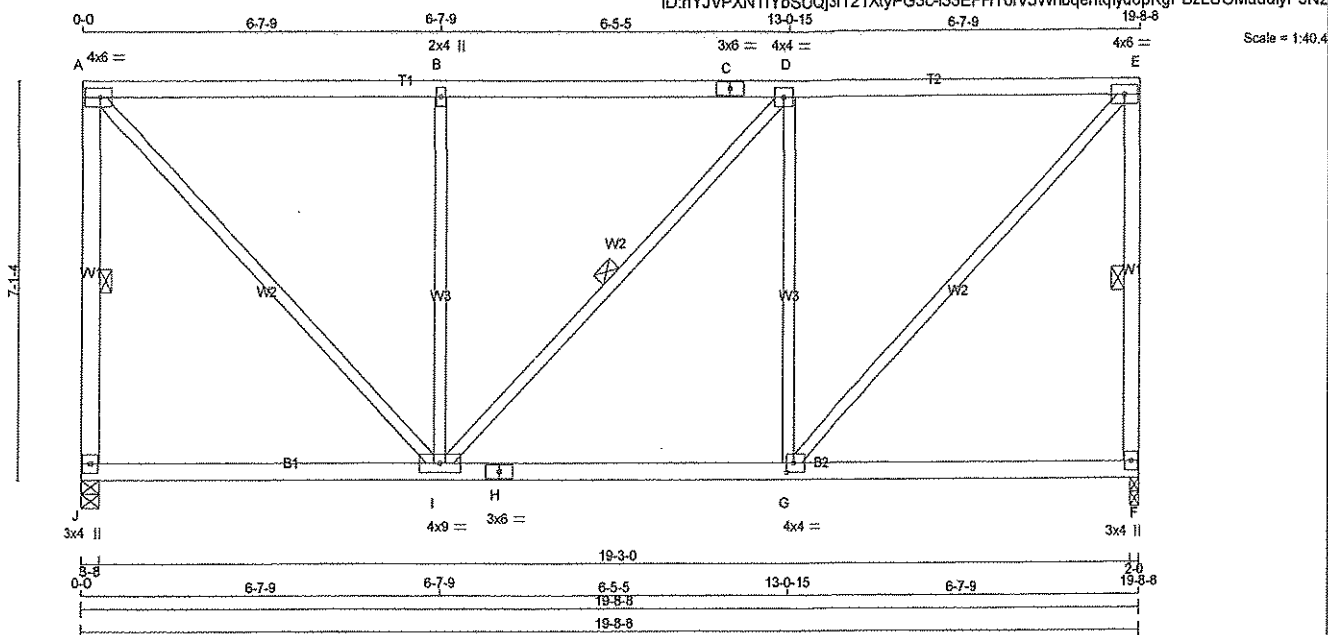


DRWG NO. FAM 47159 - 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	T23	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 92 = 370 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0			
B	TMVW-w	MT20	2.0	4.0			
C	TS-t	MT20	3.0	6.0			
D	TMVW-t	MT20	4.0	4.0			
E	TMVW-t	MT20	4.0	6.0			
F	BMV1+p	MT20	3.0	4.0			
G	BMVW-t	MT20	4.0	4.0	2.00	1.50	
H	BS-t	MT20	3.0	6.0			
I	BMVW-t	MT20	4.0	9.0			
J	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
J	1108	0	1108	0
F	1108	0	1108	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	908	504 / 0	207 / 0
F	908	504 / 0	207 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MEMB.	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX	MEMB.
FR-TO	(LBS)	(PLF)	(LC)	(LC)	FR-TO	(LBS)	(LC)	FR-TO
J-A	-1030 / 0	0.0	0.0	0.23 (1)	6.18	G-E	0 / 1124	0.25 (1)
A-B	-783 / 0	-84.3	-84.3	0.53 (1)	5.98	A-I	0 / 1122	0.25 (1)
B-C	-783 / 0	-84.3	-84.3	0.53 (1)	5.97	G-D	-604 / 0	0.53 (1)
C-D	-783 / 0	-84.3	-84.3	0.53 (1)	5.97	I-B	-604 / 0	0.53 (1)
D-E	-784 / 0	-84.3	-84.3	0.53 (1)	5.96	I-D	-2 / 0	0.00 (1)
F-E	-1031 / 0	0.0	0.0	0.23 (1)	6.18			
J-I	0 / 0	-28.0	-28.0	0.28 (3)	10.00			
I-H	0 / 784	-28.0	-28.0	0.37 (2)	10.00			
H-G	0 / 784	-28.0	-28.0	0.37 (2)	10.00			
G-F	0 / 0	-28.0	-28.0	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.53 (D-E:1), BC=0.37 (G-I:2), WB=0.53 (D-G:1), SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.29 (G) (INPUT = 1.00)

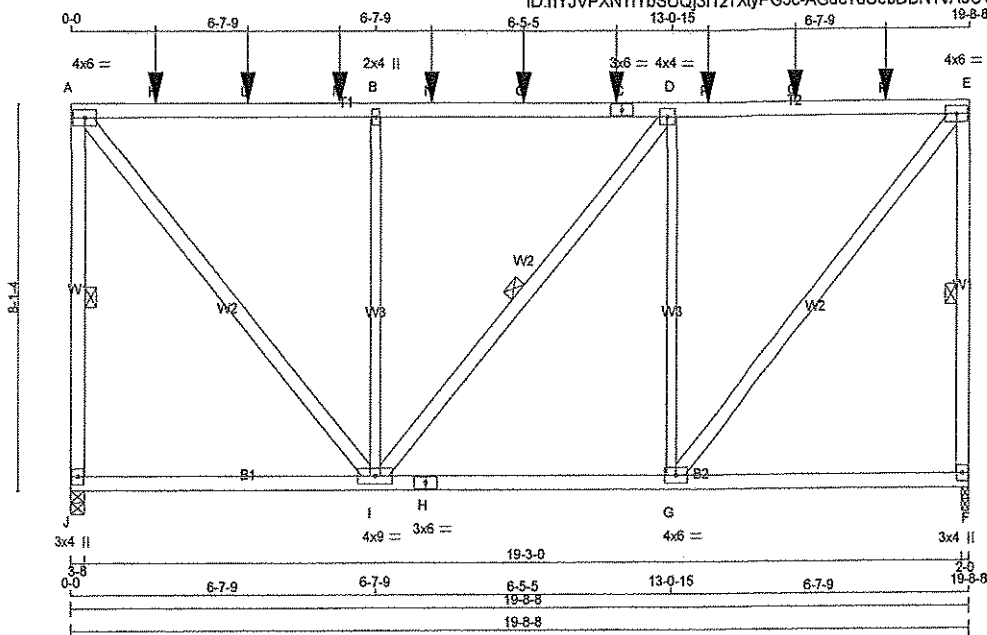


DWG NO. TAM47160-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	T24	26	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 26 X 108 = 2802 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
G - D	2x3	DRY	No.2	SPF
I - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-1	MT20	4.0	6.0			
B	TMW+V	MT20	2.0	4.0			
C	TS-1	MT20	3.0	6.0			
D	TMW-1	MT20	4.0	4.0			
E	TMW-1	MT20	4.0	6.0			
F	BMV1+p	MT20	3.0	4.0			
G	BMWV-1	MT20	4.0	6.0			
H	BS-1	MT20	3.0	6.0			
I	BMWV-1	MT20	4.0	9.0			
J	BMV1+p	MT20	3.0	4.0			

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 44.7 lbs FACTORED DOWN AT 1-10-4, 44.7 lbs FACTORED DOWN AT 3-10-4, 44.7 lbs FACTORED DOWN AT 5-10-4, 44.7 lbs FACTORED DOWN AT 7-10-4, 44.7 lbs FACTORED DOWN AT 9-10-4, 44.7 lbs FACTORED DOWN AT 11-10-4, 44.7 lbs FACTORED DOWN AT 13-10-4, AND 44.7 lbs FACTORED DOWN AT 15-10-4, AND 44.7 lbs FACTORED DOWN AT 17-10-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	1307	0	0	3-8
F	1308	0	0	2-0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
J	1071	598 / 0 241 / 0 0 / 0 0 / 0 231 / 0 0 / 0
F	1071	599 / 0 242 / 0 0 / 0 0 / 0 231 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	FACTORED	FACTORED	WEBS	MAX. FACTORED	FACTORED	FACTORED	FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (PLF)	LC2 MAX. (PLF)	MEMB.	FORCE (LBS)	MAX. (LBS)	LC1 MAX. (LBS)	LC2 MAX. (LBS)
FR-TO		FROM	TO		FR-TO				
J-A	-1231 / 0	0.0	0.0	0.36 (1)	G-E	0 / 1266	0.20 (1)		
A-K	-818 / 0	-84.3	-84.3	0.67 (1)	A-I	0 / 1263	0.20 (1)		
K-L	-818 / 0	-84.3	-84.3	0.67 (1)	G-D	-764 / 0	0.97 (1)		
L-M	-818 / 0	-84.3	-84.3	0.67 (1)	I-B	-762 / 0	0.96 (1)		
M-B	-818 / 0	-84.3	-84.3	0.67 (1)	I-D	-3 / 0	0.00 (1)		
B-N	-818 / 0	-84.3	-84.3	0.67 (1)					
N-O	-818 / 0	-84.3	-84.3	0.67 (1)					
O-C	-818 / 0	-84.3	-84.3	0.67 (1)					
C-D	-818 / 0	-84.3	-84.3	0.67 (1)					
D-P	-819 / 0	-84.3	-84.3	0.68 (1)					
P-Q	-819 / 0	-84.3	-84.3	0.68 (1)					
Q-R	-819 / 0	-84.3	-84.3	0.68 (1)					
R-E	-819 / 0	-84.3	-84.3	0.68 (1)					
F-E	-1232 / 0	0.0	0.0	0.37 (1)					
J-I	0 / 0	-28.0	-28.0	0.29 (3)					
I-H	0 / 819	-28.0	-28.0	0.38 (2)					
H-G	0 / 819	-28.0	-28.0	0.38 (2)					
G-F	0 / 0	-28.0	-28.0	0.29 (3)					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	11-10-4	-45	-45		TOP	VERT	TOTAL
K	1-10-4	-45	-45		TOP	VERT	TOTAL
L	3-10-4	-45	-45		TOP	VERT	TOTAL
M	5-10-4	-45	-45		TOP	VERT	TOTAL
N	7-10-4	-45	-45		TOP	VERT	TOTAL
O	9-10-4	-45	-45		TOP	VERT	TOTAL
P	13-10-4	-45	-45		TOP	VERT	TOTAL
Q	15-10-4	-45	-45		TOP	VERT	TOTAL
R	17-10-4	-45	-45		TOP	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
CL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.66")
CALCULATED VERT. DEFL (LL) = $L/999$ (0.06")
ALLOWABLE DEFL (TL) = $L/360$ (0.66")
CALCULATED VERT. DEFL (TL) = $L/999$ (0.13")

CSI: TC=0.68 (D-E-1), BC=0.38 (G-I-2), WB=0.97 (D-G-1), SSI=0.33 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSMETAL= 0.29 (H) (INPUT = 1.00)



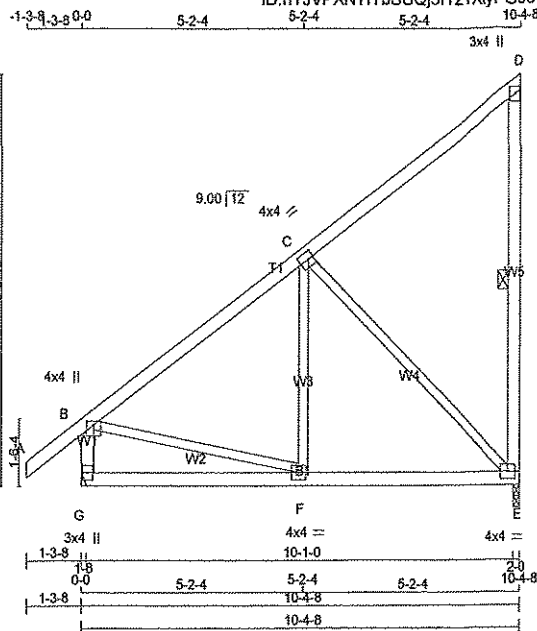
DRWG NO. TAM 47161-16
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	T26A	6	1	TRUSS DESC.		

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Scale = 1:51.2

TOTAL WEIGHT = 6 X 53 = 320 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+l	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW+l	MT20	4.0	4.0		
F	BMVW+l	MT20	4.0	4.0		
G	BMV+l	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	582	0	582	0
G	699	0	699	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
E	478	266 / 0	109 / 0	0 / 0	0 / 0
G	557	336 / 0	109 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 273
B-C	-429 / 0	-84.3	-84.3	0.30 (1)	6.25	C-E	-517 / 0
C-D	-32 / 0	-84.3	-84.3	0.29 (1)	6.25	B-F	0 / 379
E-D	-165 / 0	0.0	0.0	0.07 (1)	6.25		0.09 (1)
G-B	-643 / 0	0.0	0.0	0.07 (1)	7.81		
G-F	0 / 0	-28.0	-28.0	0.22 (3)	10.00		
F-E	0 / 389	-28.0	-28.0	0.26 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPI 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/380$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.30 (B-C:1), BC=0.26 (E-F:2), WB=0.50 (C-E:1), SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



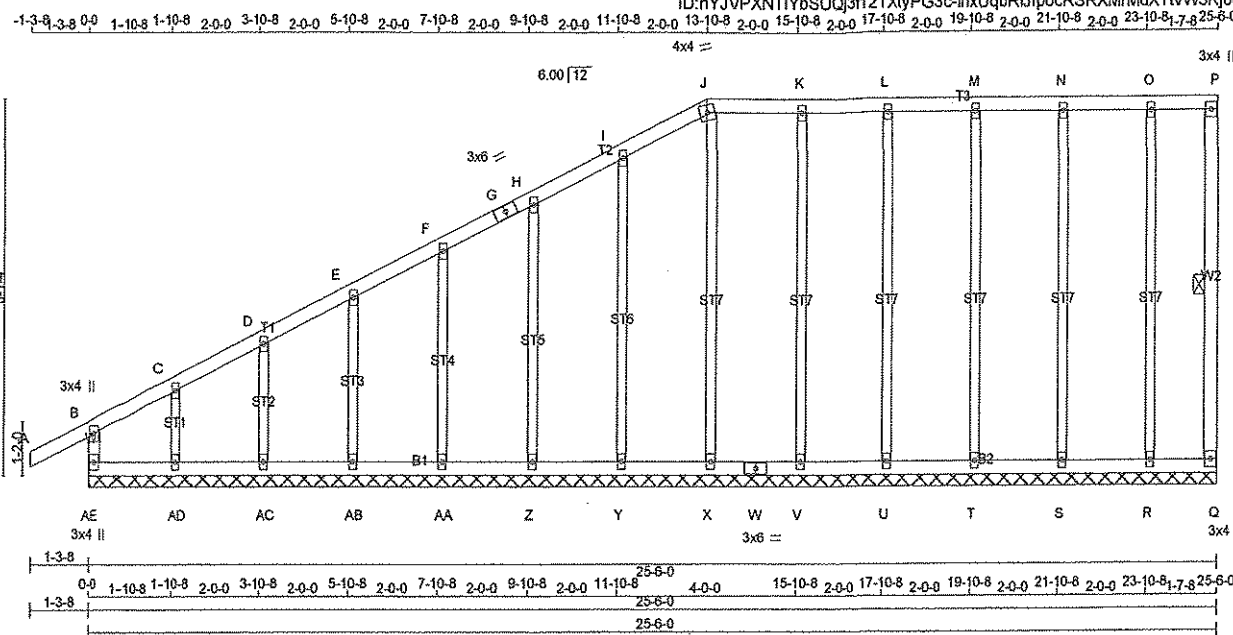
DWG NO. TAM 47162-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	G25	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
AE- B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - P	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
AE- W	2x4	DRY	No.2
W - Q	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF P-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH (FR-TO)
FR-TO		FROM	TO	FR-TO			
AE-B	-206 / 0	0.0	0.0 0.01 (1)	7.81	AD-C	-161 / 0	0.02 (1)
A-B	0 / 26	-84.3	-84.3 0.11 (1)	10.00	AC-D	-168 / 0	0.03 (1)
B-C	-24 / 0	-84.3	-84.3 0.04 (1)	6.25	AB-E	-167 / 0	0.04 (1)
C-D	-17 / 0	-84.3	-84.3 0.04 (1)	6.25	AA-F	-167 / 0	0.07 (1)
D-E	-13 / 0	-84.3	-84.3 0.04 (1)	6.25	Z-H	-168 / 0	0.10 (1)
E-F	-10 / 0	-84.3	-84.3 0.04 (1)	6.25	Y-I	-165 / 0	0.15 (1)
F-G	-8 / 0	-84.3	-84.3 0.04 (1)	10.00	X-J	-170 / 0	0.21 (1)
G-H	-8 / 0	-84.3	-84.3 0.04 (1)	10.00	V-K	-175 / 0	0.22 (1)
H-I	-6 / 0	-84.3	-84.3 0.04 (1)	10.00	U-L	-167 / 0	0.21 (1)
I-J	-3 / 0	-84.3	-84.3 0.04 (1)	10.00	T-M	-168 / 0	0.21 (1)
J-K	-1 / 0	-84.3	-84.3 0.04 (1)	10.00	S-N	-174 / 0	0.22 (1)
K-L	-1 / 0	-84.3	-84.3 0.04 (1)	10.00	R-O	-154 / 0	0.19 (1)
L-M	-1 / 0	-84.3	-84.3 0.04 (1)	10.00			
M-N	-1 / 0	-84.3	-84.3 0.04 (1)	10.00			
N-O	-1 / 0	-84.3	-84.3 0.04 (1)	10.00			
O-P	-1 / 0	-84.3	-84.3 0.03 (1)	10.00			
Q-P	-54 / 0	0.0	0.0 0.01 (1)	6.25			
AE-AD	0 / 22	-28.0	-28.0 0.03 (2)	10.00			
AD-AC	0 / 16	-28.0	-28.0 0.03 (2)	10.00			
AC-AB	0 / 12	-28.0	-28.0 0.02 (2)	10.00			
AB-AA	0 / 9	-28.0	-28.0 0.02 (2)	10.00			
AA-Z	0 / 7	-28.0	-28.0 0.02 (2)	10.00			
Z-Y	0 / 5	-28.0	-28.0 0.02 (2)	10.00			
Y-X	0 / 3	-28.0	-28.0 0.02 (2)	10.00			
X-W	0 / 1	-28.0	-28.0 0.02 (2)	10.00			
W-V	0 / 1	-28.0	-28.0 0.02 (2)	10.00			
V-U	0 / 1	-28.0	-28.0 0.02 (2)	10.00			
U-T	0 / 1	-28.0	-28.0 0.02 (2)	10.00			
T-S	0 / 1	-28.0	-28.0 0.02 (2)	10.00			
S-R	0 / 1	-28.0	-28.0 0.02 (2)	10.00			
R-Q	0 / 1	-28.0	-28.0 0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

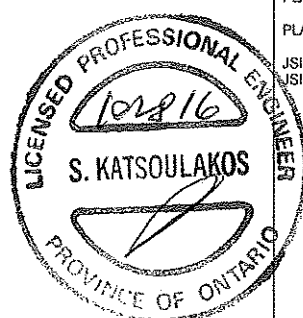
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
CSI: TC=0.11 (A-B:1), BC=0.03 (AD-AE:2), WB=0.22 (K-V:1), SSI=0.08 (A-B:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.55 (J) (INPUT = 0.90)
JSI METAL= 0.05 (J) (INPUT = 1.00)



DWG NO. TAM47163-18
STRUCTURAL
COMPONENT ONLY

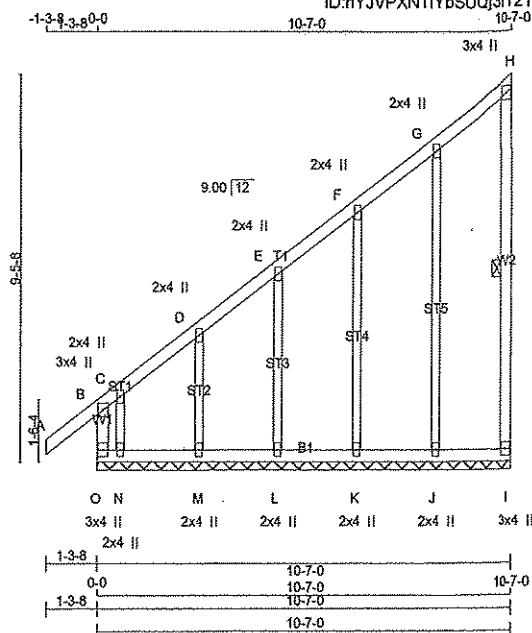
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	G26	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:55.4



TOTAL WEIGHT = 2 X 58 = 117 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, G					
H	TMV+w	MT20	2.0	4.0	
I	TMV+p	MT20	3.0	4.0	
J, K, L, M, N					
O	BMV1+p	MT20	2.0	4.0	
	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
O-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	J-G	-183 / 0	0.22 (1)
A-B	0 / 35	-84.3	-84.3	0.11 (1)	10.00	K-F	-165 / 0	0.11 (1)
B-C	-83 / 0	-84.3	-84.3	0.10 (1)	6.25	L-E	-166 / 0	0.08 (1)
C-D	-15 / 0	-84.3	-84.3	0.04 (1)	6.25	M-D	-170 / 0	0.03 (1)
D-E	-12 / 0	-84.3	-84.3	0.04 (1)	6.25	N-C	0 / 9	0.00 (1)
E-F	-8 / 0	-84.3	-84.3	0.04 (1)	10.00			
F-G	-4 / 0	-84.3	-84.3	0.04 (1)	10.00			
G-H	-8 / 0	-84.3	-84.3	0.04 (1)	10.00			
H-I	-76 / 0	0.0	0.0	0.01 (1)	6.25			
O-N	0 / 14	-28.0	-28.0	0.03 (1)	10.00			
N-M	0 / 14	-28.0	-28.0	0.02 (2)	10.00			
M-L	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
L-K	0 / 6	-28.0	-28.0	0.02 (2)	10.00			
K-J	0 / 4	-28.0	-28.0	0.03 (3)	10.00			
J-I	0 / 2	-28.0	-28.0	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B-1), BC=0.03 (N-O:1), WB=0.22 (G-J:1), SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

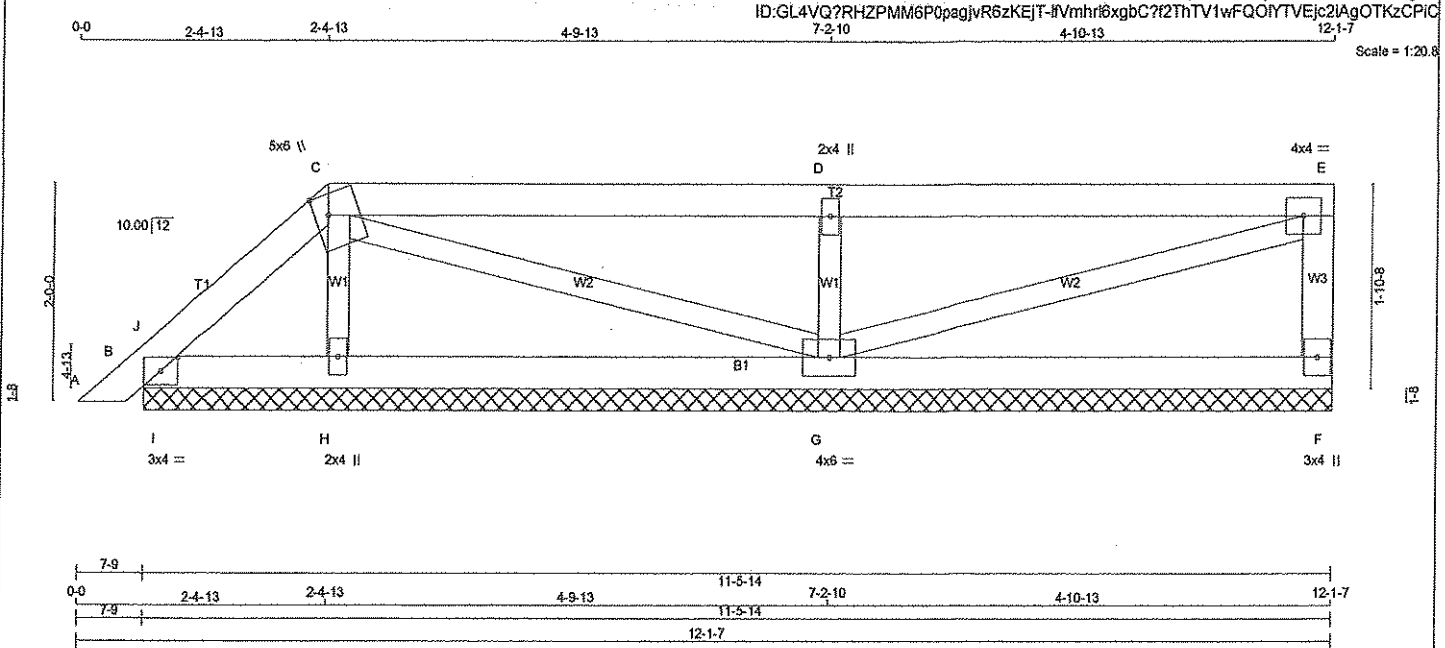
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (3) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DRWG NO. TAM 47164-16
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 2 X 39 = 78 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 F - H 2x4 DRY No.2 SPF
 B - F 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	210	0	210	0	11-5-14	1-8
B	158	0	158	0	11-5-14	1-8
H	291	0	291	0	11-5-14	1-8
G	676	0	676	0	11-5-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	173	95/0	40/0	0/0	0/0	38/0	0/0
B	112	91/0	6/0	0/0	0/0	15/0	0/0
H	258	114/0	77/0	0/0	0/0	65/0	0/0
G	549	315/0	118/0	0/0	0/0	116/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED	VERT. LOAD	FACTORED		MAX. UNBRAC	MEMB.	MAX. FACTORED	MAX	
	FORCE (LBS)		(PLF)	LC1 MAX. CSI (LC)			FORCE (LBS)		CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0/13	-84.3	-84.3	0.02 (1)	10.00	H-C	-173/0	0.03 (1)	
B-J	-25/11	-84.3	-84.3	0.01 (1)	6.25	G-E	0/0	0.00 (1)	
J-C	-50/0	-84.3	-84.3	0.03 (1)	6.25	C-G	-25/0	0.01 (1)	
C-D	0/0	-84.3	-84.3	0.34 (1)	10.00	G-D	-512/0	0.07 (1)	
D-E	0/0	-84.3	-84.3	0.34 (1)	10.00	I-J	-109/0	0.00 (1)	
F-E	-156/0	0.0	0.0	0.02 (1)	7.81				
B-I	0/35	-28.0	-28.0	0.04 (1)	10.00				
I-H	0/35	-28.0	-28.0	0.08 (2)	10.00				
H-G	0/24	-28.0	-28.0	0.17 (2)	10.00				
G-F	0/0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (D-E:1), BC=0.17 (G-H:2), WB=0.07 (D-G:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

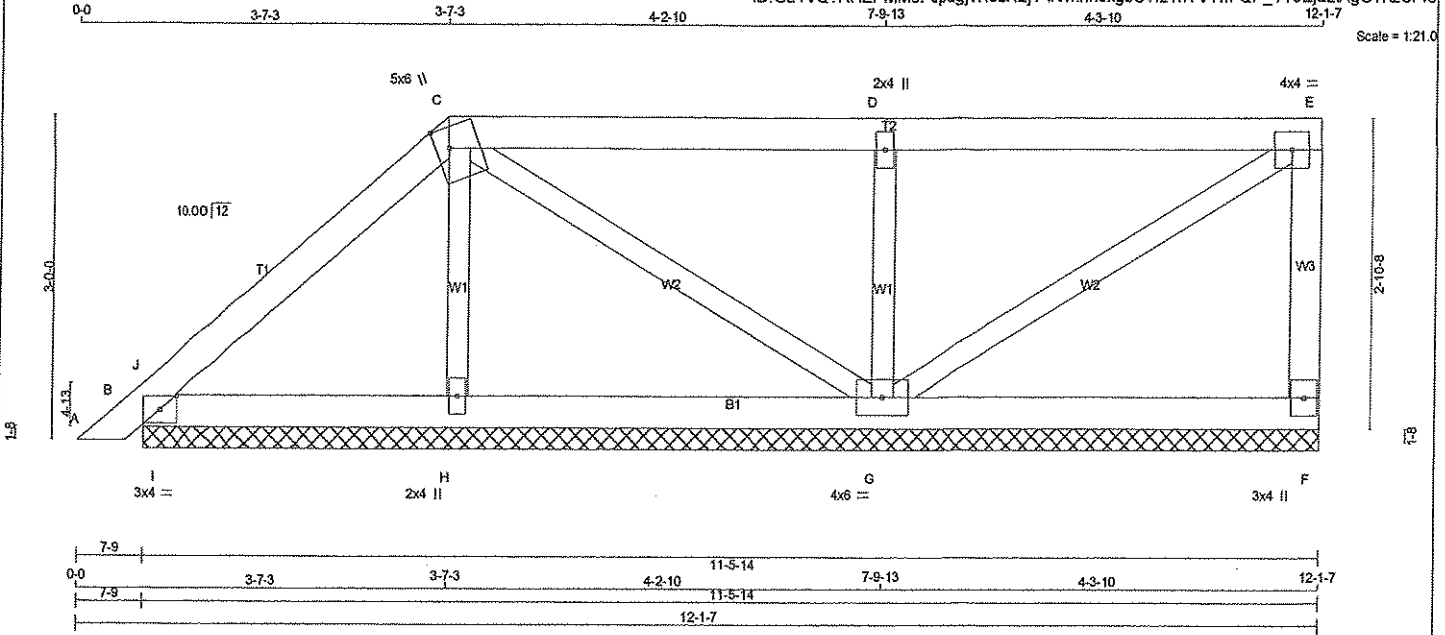
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
 JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 2433216
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-i	MT20	3.0	4.0	1.50	2.00
C TTWW+m	MT20	5.0	6.0	2.25	1.50
D TMW+w	MT20	2.0	4.0		
E TMVW-t	MT20	4.0	4.0		
F BMV1+p	MT20	3.0	4.0		
G BMVWW1-t	MT20	4.0	6.0		
H BMV1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	185	0	185	0	11-5-14	1-8
B	251	0	251	0	11-5-14	1-8
H	287	0	287	0	11-5-14	1-8
G	613	0	613	0	11-5-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	UNBRAC			
F	153	83/0	36/0	0/0	0/0	34/0	0/0
B	191	130/0	27/0	0/0	0/0	33/0	0/0
H	252	113/0	75/0	0/0	0/0	63/0	0/0
G	494	289/0	103/0	0/0	0/0	102/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	CSI (LC)		FR-TO			CSI (LC)
A-B	0 / 13	-84.3	-84.3	0.02 (1)	10.00	H-C	-156 / 0		0.03 (1)
B-J	-51 / 0	-84.3	-84.3	0.03 (1)	6.25	G-E	-2 / 3		0.00 (1)
J-C	-83 / 0	-84.3	-84.3	0.09 (1)	6.25	C-G	-63 / 0		0.02 (1)
C-D	-2 / 2	-84.3	-84.3	0.26 (1)	10.00	G-D	-448 / 0		0.08 (1)
D-E	-2 / 1	-84.3	-84.3	0.26 (1)	10.00	I-J	-215 / 42		0.00 (1)
F-E	-136 / 0	0.0	0.0	0.02 (1)	7.81				
B-I	0 / 59	-28.0	-28.0	0.08 (1)	10.00				
I-H	0 / 59	-28.0	-28.0	0.08 (2)	10.00				
H-G	0 / 53	-28.0	-28.0	0.13 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26 (C-D:1), BC=0.13 (G-H:2), WB=0.08 (D-G:1), SSI=0.18 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

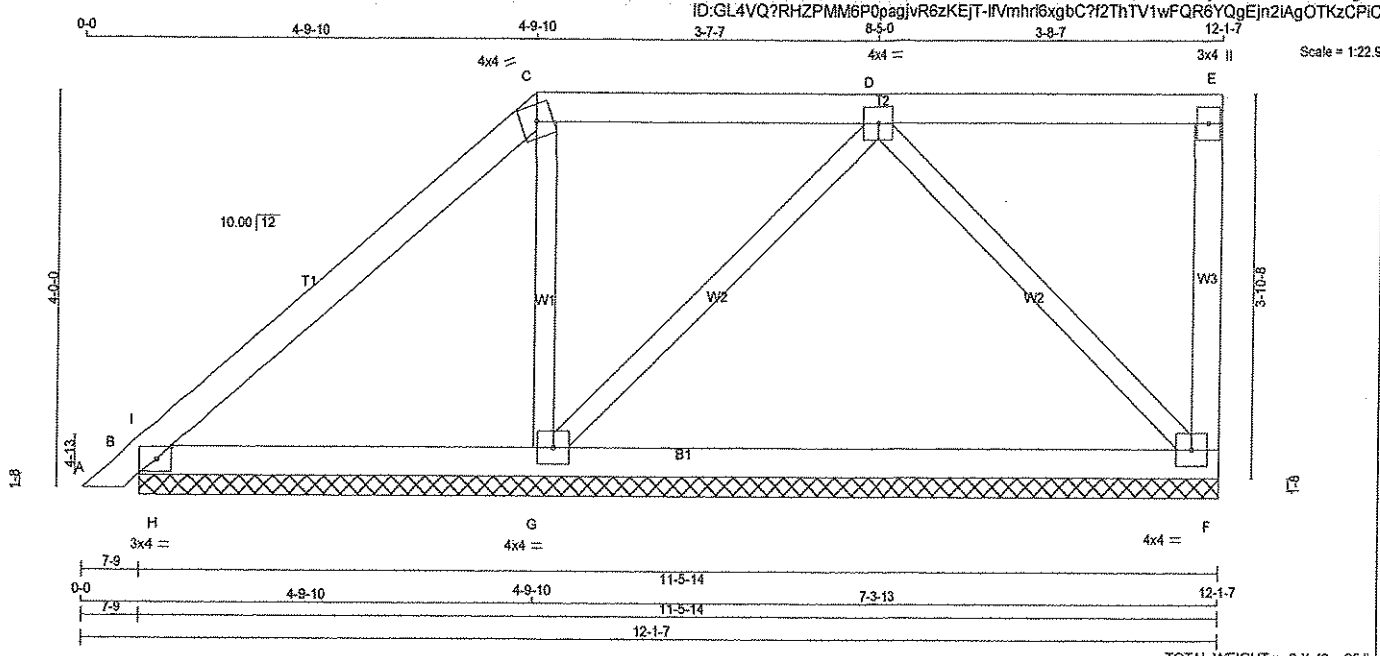
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.60)
 JSI METAL= 0.08 (D) (INPUT = 1.00)



DWG NO. TAM 24333-16
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 2 X 43 = 85 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
B - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV-p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	408	0	408	0	11-5-14	1-8
B	290	0	290	0	11-5-14	1-8
G	637	0	637	0	11-5-14	1-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	323	199 / 0	61 / 0	0 / 0	0 / 0	64 / 0	0 / 0
B	212	160 / 0	20 / 0	0 / 0	0 / 0	32 / 0	0 / 0
G	554	256 / 0	161 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)				MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO	FROM	TO				
FR-TO									
A-B	0 / 13	-84.3	-84.3	0.02 (1)	10.00	G-C	-216 / 0	0.05 (1)	
B-I	0 / 148	-84.3	-84.3	0.13 (1)	10.00	G-D	-242 / 0	0.10 (1)	
I-C	-83 / 0	-84.3	-84.3	0.17 (1)	6.25	D-F	-302 / 0	0.13 (1)	
C-D	-48 / 0	-84.3	-84.3	0.19 (1)	6.25	H-I	-490 / 0	0.00 (1)	
D-E	0 / 0	-84.3	-84.3	0.19 (1)	10.00				
F-E	-119 / 0	0.0	0.0	0.03 (1)	7.81				
B-H	0 / 54	-28.0	-28.0	0.17 (1)	10.00				
H-G	0 / 54	-28.0	-28.0	0.34 (2)	10.00				
G-F	0 / 217	-28.0	-28.0	0.36 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (C-D:1), BC=0.36 (F-G:2), WB=0.13 (D-F:1), SSI=0.38 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)



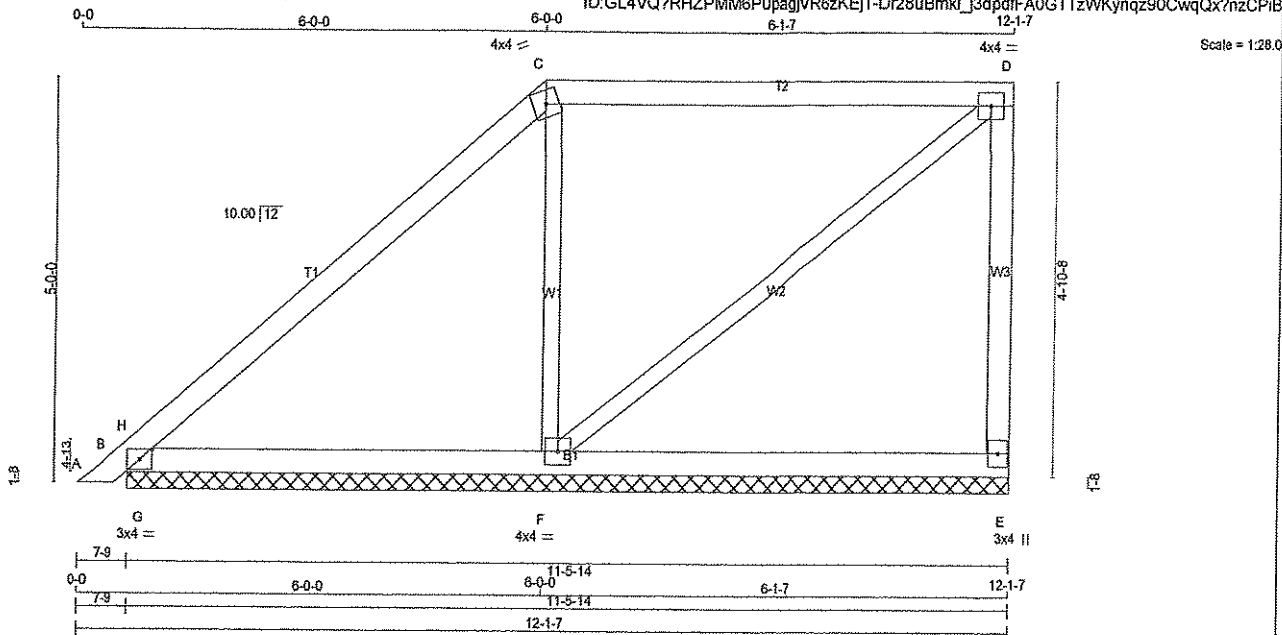
DWG NO. YAM24334-16
STRUCTURAL
COMPONENT ONLY

JOB NAME 267473	TRUSS NAME PB4	QUANTITY 3	PLY 1	JOB DESC. 42181	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 43 = 129 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	3.0	4.0	1.50	2.00	
C	TTW-m	MT20	4.0	4.0			
D	TMVW-t	MT20	4.0	4.0			
E	BMV1+p	MT20	3.0	4.0			
F	BMWW1-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	385	0	0	0
B	405	0	0	0
F	545	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	303	189 / 0	55 / 0	0 / 0	0 / 0	59 / 0	0 / 0
B	311	207 / 0	48 / 0	0 / 0	0 / 0	58 / 0	0 / 0
F	475	219 / 0	138 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX	UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX
FR-TO		FROM TO				FR-TO		
A-B	0 / 13	-84.3 -84.3	0.02 (1)	10.00		F-C	-369 / 0	0.13 (1)
B-H	-117 / 198	-84.3 -84.3	0.19 (1)	6.25		F-D	0 / 114	0.03 (1)
H-C	-144 / 0	-84.3 -84.3	0.31 (1)	6.25		G-H	-860 / 99	0.00 (1)
C-D	-91 / 0	-84.3 -84.3	0.54 (1)	6.25				
E-D	-327 / 0	0.0 0.0	0.13 (1)	7.81				
B-G	0 / 99	-28.0 -28.0	0.25 (1)	10.00				
G-F	0 / 99	-28.0 -28.0	0.30 (2)	10.00				
F-E	0 / 0	-28.0 -28.0	0.29 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.54 (C-D:1), BC=0.30 (F-G:2), WB=0.13 (C-F:1), SSI=0.55 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

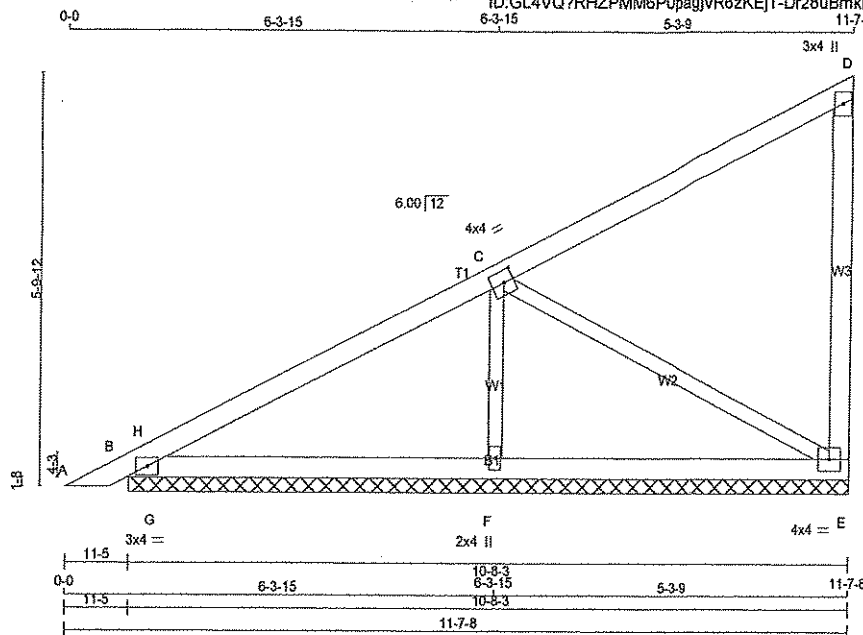
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. YAM24335-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 39 = 79 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - D 2x4 DRY No.2 SPF
 E - D 2x4 DRY No.2 SPF
 B - E 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	4.0	2.00	1.75
D	TMV+P	MT20	3.0	4.0		
E	BMVW1-I	MT20	4.0	4.0		
F	BMVW1+W	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	274	0	274	0	10-8-3	1-8
B	353	0	353	0	10-8-3	1-8
F	647	0	647	0	10-8-3	1-8

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	220	130 / 0	45 / 0	0 / 0	0 / 0	45 / 0	0 / 0
B	274	177 / 0	46 / 0	0 / 0	0 / 0	51 / 0	0 / 0
F	540	285 / 0	133 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	LC2			MEMB. FORCE (LBS)	MAX. CSI (LC)
FR-TO								
A-B	0 / 15	-84.3	-84.3	0.05 (1)	10.00	F-C	-438 / 0	0.08 (1)
B-H	-105 / 0	-84.3	-84.3	0.07 (3)	6.25	C-E	-113 / 0	0.07 (1)
H-C	-88 / 0	-84.3	-84.3	0.38 (1)	6.25	G-H	-188 / 117	0.00 (1)
C-D	-24 / 0	-84.3	-84.3	0.38 (1)	6.25			
E-D	-171 / 0	0.0	0.0	0.10 (1)	7.81			
B-G	0 / 100	-28.0	-28.0	0.14 (1)	10.00			
G-F	0 / 100	-28.0	-28.0	0.24 (2)	10.00			
F-E	0 / 100	-28.0	-28.0	0.24 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38 (C-H:1), BC=0.24 (E-F:2), WB=0.08 (C-F:1), SSI=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (C) (INPUT = 0.90)
 JSI METAL= 0.08 (C) (INPUT = 1.00)



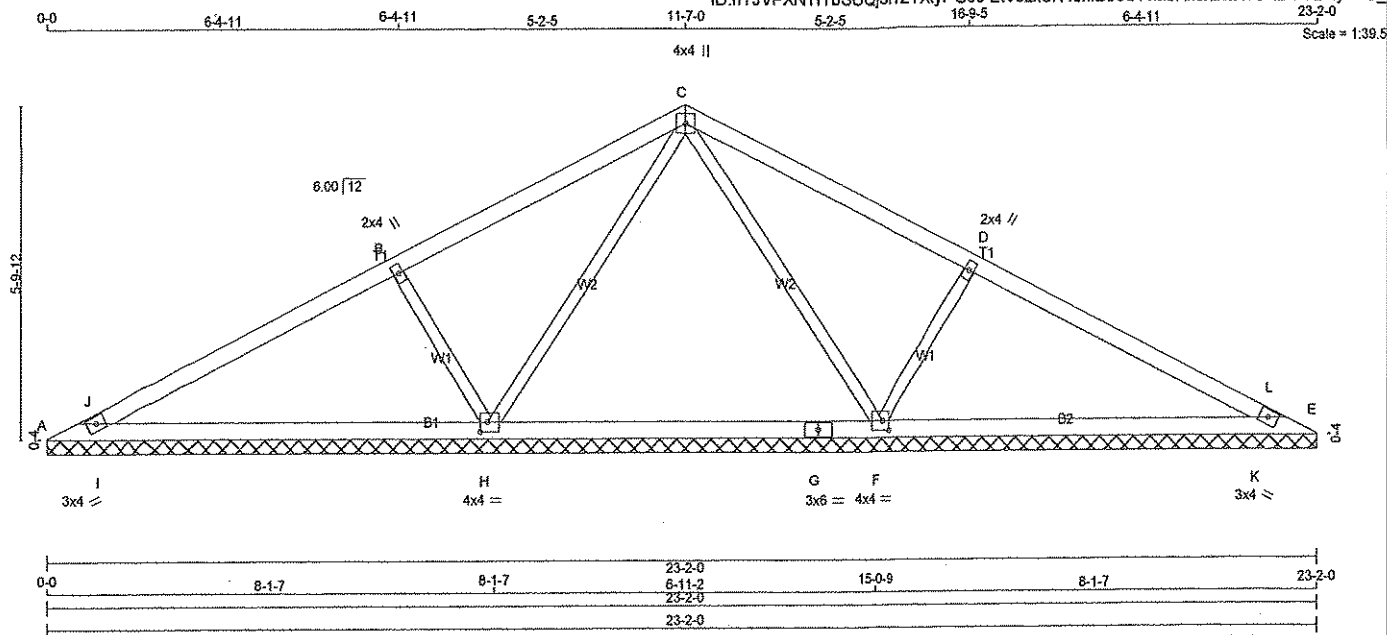
DWG NO. TAM 24336.16
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	PB10	18	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TMW+w	MT20	2.0	4.0	
C	TTWW+p	MT20	4.0	4.0	
D	TMW+w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	2.00 1.50
G	BS-t	MT20	3.0	6.0	
H	BMWW1-t	MT20	4.0	4.0	2.00 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A	129	0	129	0	0	23-2-0 (13-1148)			
E	129	0	129	0	0	23-2-0 (13-1148)			
F	1172	0	1172	0	0	23-2-0 (13-1148)			
H	1172	0	1172	0	0	23-2-0 (13-1148)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
A	93	72/0	8/0	0/0	0/0	14/0	0/0	
E	93	72/0	8/0	0/0	0/0	14/0	0/0	
F	974	521/0	236/0	0/0	0/0	218/0	0/0	
H	974	521/0	236/0	0/0	0/0	218/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	0/389	-84.3 -84.3	0.23 (2)	C-F	-581/0	0.47 (1)	
J-B	0/471	-84.3 -84.3	0.52 (1)	F-D	-524/0	0.11 (1)	
B-C	0/706	-84.3 -84.3	0.58 (1)	H-C	-581/0	0.47 (1)	
C-D	0/706	-84.3 -84.3	0.58 (1)	B-H	-524/0	0.11 (1)	
D-L	0/471	-84.3 -84.3	0.52 (1)	I-J	-60/180	0.00 (1)	
L-E	0/389	-84.3 -84.3	0.23 (2)	K-L	-60/180	0.00 (1)	
A-I	-405/0	-28.0 -28.0	0.12 (1)				
I-H	-390/0	-28.0 -28.0	0.35 (2)				
H-G	-341/0	-28.0 -28.0	0.35 (2)				
G-F	-341/0	-28.0 -28.0	0.35 (2)				
F-K	-390/0	-28.0 -28.0	0.35 (2)				
K-E	-405/0	-28.0 -28.0	0.12 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56 (B-C:1), BC=0.35 (H-I:2), WB=0.47 (C-H:1), SSI=0.21 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (C) (INPUT = 0.80)
JSI METAL= 0.23 (C) (INPUT = 1.00)



DRWG NO. TAM 47165-10
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	42181	DRWG NO.
274585	PG10	4	1	TRUSS DESC.		

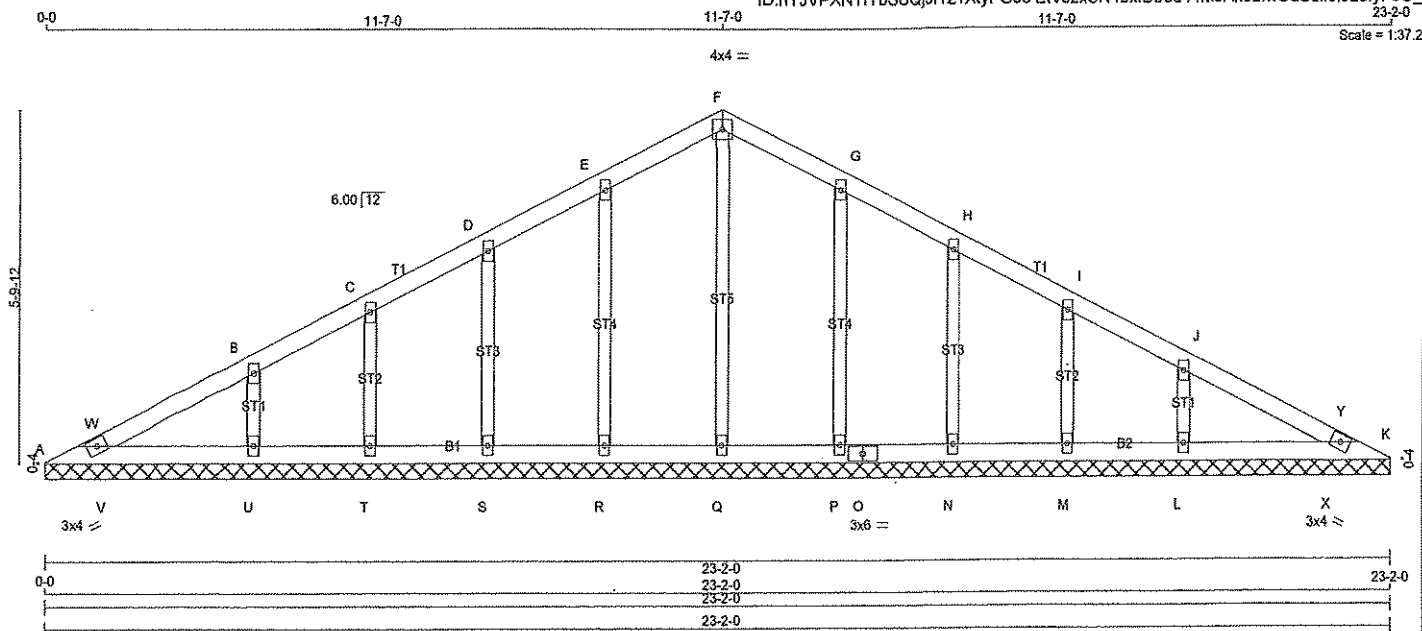
Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 Mitek Industries, Inc. Thu Oct 27 17:37:03 2016 Page 1

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23-2-0

Scale = 1:37.2



TOTAL WEIGHT = 4 X 77 = 307 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
A - O	2x4	DRY	No.2
O - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			
GABLE STUDS SPACED AT 2-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	MAX. FACTORED LC1 (LC)
FR-TO				FR-TO			
A-W	0/32	-84.3	-84.3 0.06 (2)	Q-F	-200/0	10.00	0.10 (1)
W-B	0/65	-84.3	-84.3 0.11 (1)	R-E	-191/0	10.00	0.07 (1)
B-C	0/46	-84.3	-84.3 0.11 (1)	S-D	-169/0	10.00	0.04 (1)
C-D	0/65	-84.3	-84.3 0.05 (1)	T-C	-134/0	10.00	0.02 (1)
D-E	0/68	-84.3	-84.3 0.06 (1)	U-B	-255/0	10.00	0.04 (1)
E-F	0/61	-84.3	-84.3 0.06 (1)	P-G	-191/0	10.00	0.07 (1)
F-G	0/61	-84.3	-84.3 0.06 (1)	N-H	-169/0	10.00	0.04 (1)
G-H	0/68	-84.3	-84.3 0.06 (1)	M-I	-134/0	10.00	0.02 (1)
H-I	0/65	-84.3	-84.3 0.05 (1)	L-J	-255/0	10.00	0.04 (1)
I-J	0/46	-84.3	-84.3 0.11 (1)	V-W	-65/43	10.00	0.00 (1)
J-Y	0/65	-84.3	-84.3 0.11 (1)	X-Y	-65/43	10.00	0.00 (1)
Y-K	0/32	-84.3	-84.3 0.06 (2)				
A-V	-50/0	-28.0	-28.0 0.09 (1)				
V-U	-41/0	-28.0	-28.0 0.09 (1)				
U-T	-52/0	-28.0	-28.0 0.07 (1)				
T-S	-56/0	-28.0	-28.0 0.03 (2)				
S-R	-59/0	-28.0	-28.0 0.03 (2)				
R-Q	-62/0	-28.0	-28.0 0.02 (2)				
Q-P	-62/0	-28.0	-28.0 0.02 (2)				
P-O	-59/0	-28.0	-28.0 0.03 (2)				
O-N	-59/0	-28.0	-28.0 0.03 (2)				
N-M	-56/0	-28.0	-28.0 0.03 (2)				
M-L	-52/0	-28.0	-28.0 0.07 (1)				
L-X	-41/0	-28.0	-28.0 0.09 (1)				
X-K	-50/0	-28.0	-28.0 0.09 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (B-W:1), BC=0.09 (U-V:1), WB=0.10 (F-Q:1), SSI=0.10 (B-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.44 (A) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DRWG NO. TAM 47166-10
STRUCTURAL
COMPONENT ONLY



Build 4516

Job Name:

GREENPARK/STAR TIME HOME

Address:

ELMBROOK 4B

City, Province, Postal Code: VAUGHAN, ON

Customer:

Code reports:

NLGA

File Name: BC CALC Project

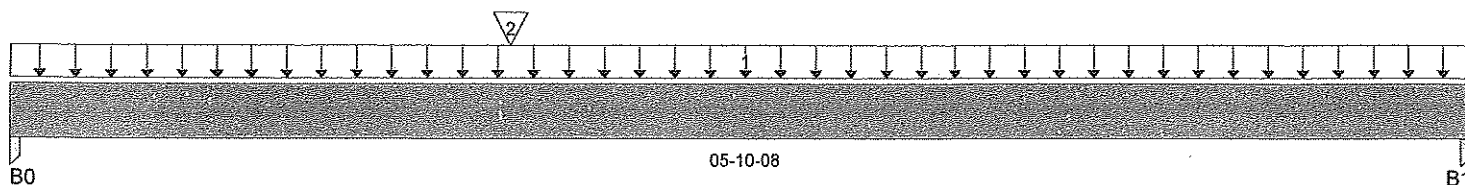
Description: Designs\BM2

Specifier:

Designer: JG

Company:

Misc:



Total Horizontal Product Length = 05-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	214 / 0	227 / 0	433 / 0	
B1, 3-1/2"	137 / 0	150 / 0	289 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live	Dead	Snow	Wind	Trib.
1	Standard Load	Unf. Area (lb/ft^2)	L	00-00-00	05-10-08	11	10	26		02-00-00
2		Conc. Pt. (lbs)	L	02-00-00	02-00-00	222	222	417		n/a

Controls Summary

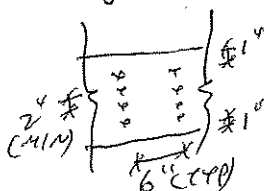
	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,602 ft-lbs	6,040 ft-lbs	26.5%	5	02-00-00
End Shear	911 lbs	3,985 lbs	22.9%	5	01-00-12
Total Load Defl.	L/999 (0.019")	n/a	n/a	13	02-08-15
Live Load Defl.	L/999 (0.013")	n/a	n/a	17	02-08-15
Max Defl.	0.019"	n/a	n/a	13	02-08-15
Span / Depth	7	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B0 Post	3-1/2" x 3"	1,041 lbs	n/a	16.1%	Unspecified
B1 Post	3-1/2" x 3"	690 lbs	n/a	10.7%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Design meets User specified (1") Maximum total load deflection criteria.
 Calculations assume Member is Fully Braced.
 Resistance Factor phi has been applied to all presented results per CSA 086.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC and CSA 086.
 Design based on Dry Service Condition.
 The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
 Importance Factor : Normal Part code : Part 4
 Deflections less than 1/8" were ignored in the results.



PROVIDE 4 ROWS OF 3/4" ARDOX SPIRAL NAILS @ 6" O/C FOR MULTI-PLY NAILING. MAINTAIN A MIN. 1" LUMBER EDGE/END DISTANCE. DO NOT USE AIR NAILS

Disclosure

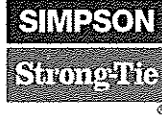
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWG NO. YAM 24339.16
 STRUCTURAL
 COMPONENT ONLY

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

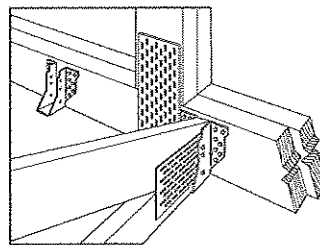
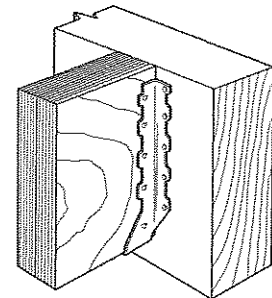
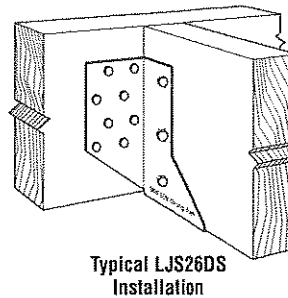
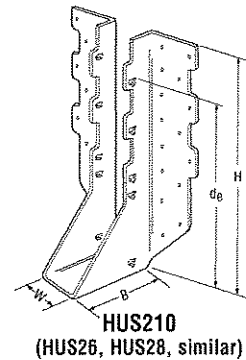
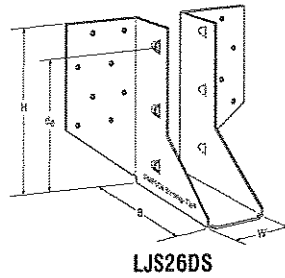
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

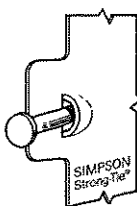
OPTIONS:

- See current catalogue for options

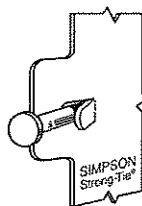


Model No.	Ga	Dimensions (in)			Fasteners			Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4½	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	30-16d	10-16d	4505	6450	4010	5200

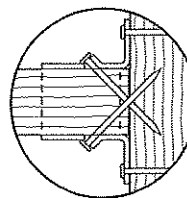
1. d_g is the distance from the seat of the hanger to the highest joist nail.



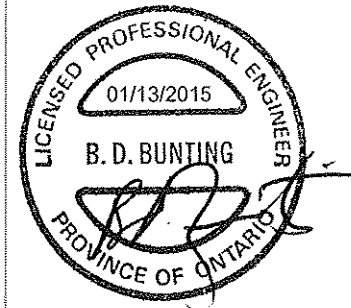
U.S. Patent
5,603,580



Double Shear Nailing Top View.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

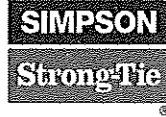
This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given.
The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

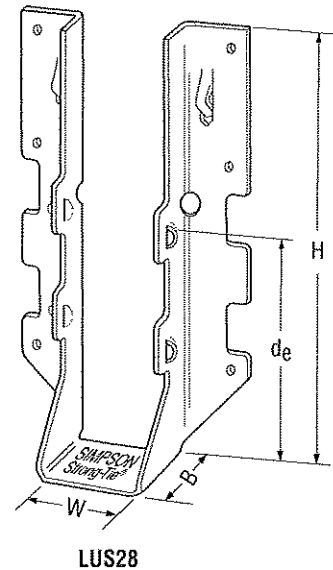
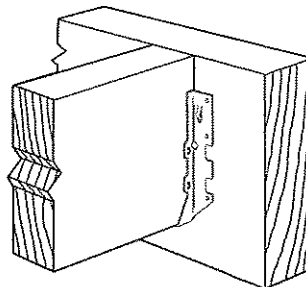
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

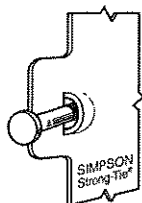
- These hangers cannot be modified.

Typical LUS Installation

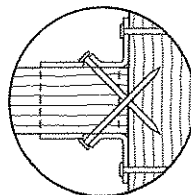


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	de ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1¾	1½	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	4-16d	2-16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¾	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¾	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4¾	8½	2	5½	8-16d	6-16d	2580	3345	2320	2375

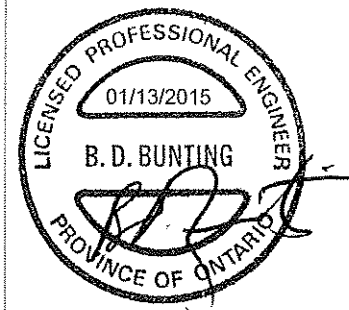
1. de is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

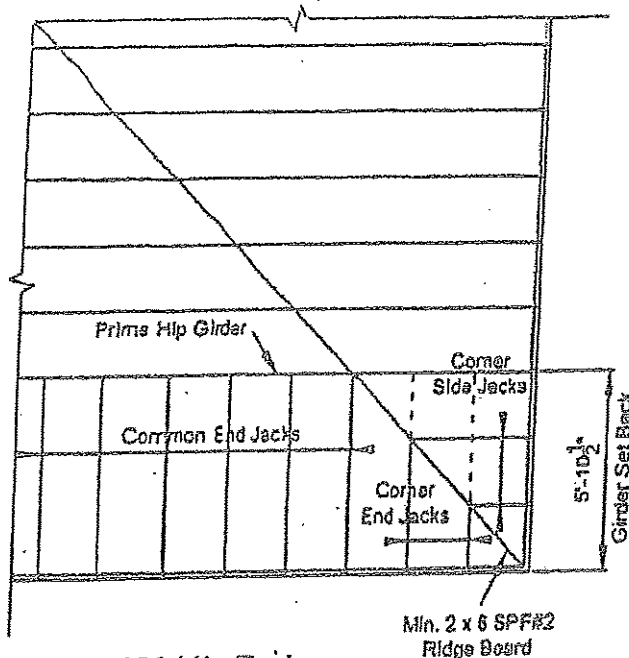
TOP CHORD LIVE LOAD : 34.8 P.S.F.

TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

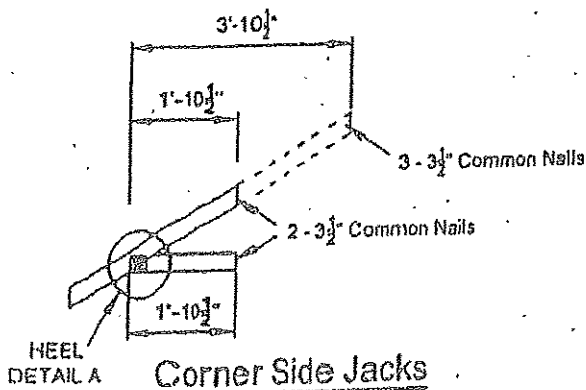
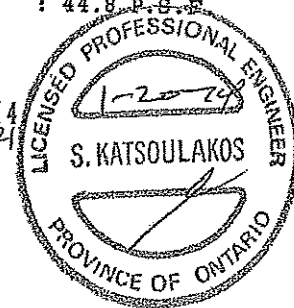
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

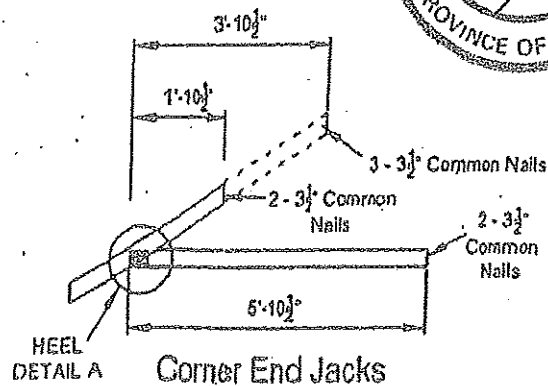


45° Hip End

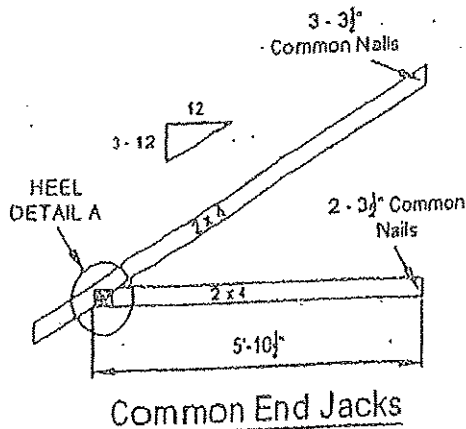
DWG NO T&M 3495.14
STRUCTURAL
COMPONENT ONLY



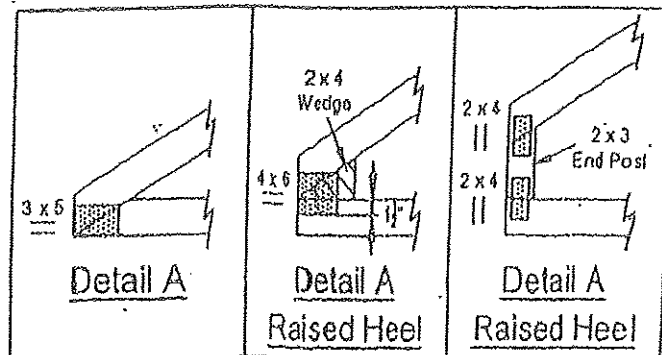
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

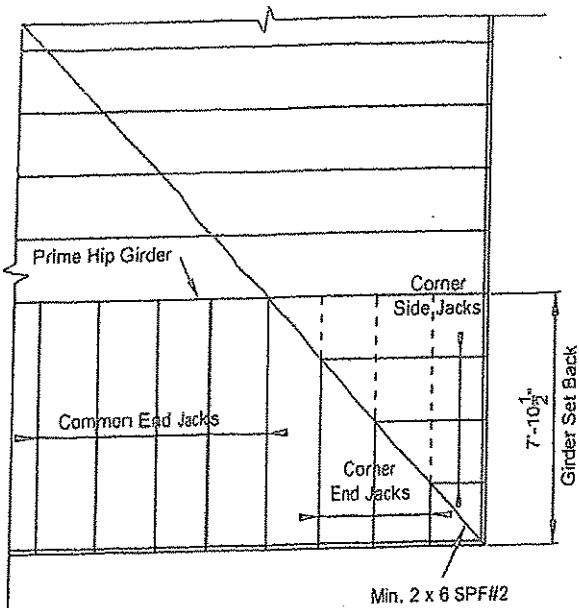
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

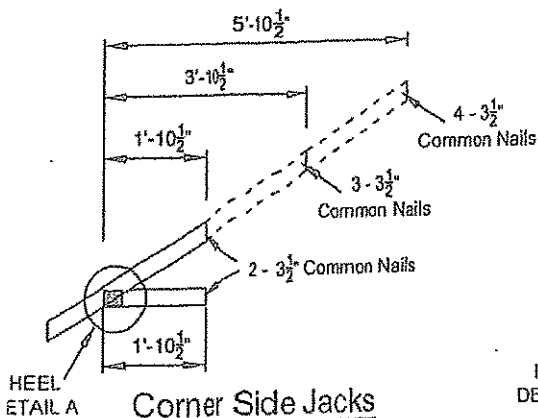
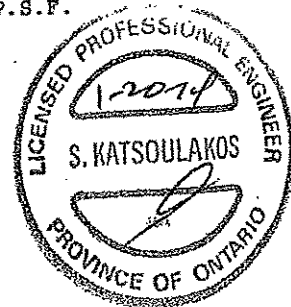
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

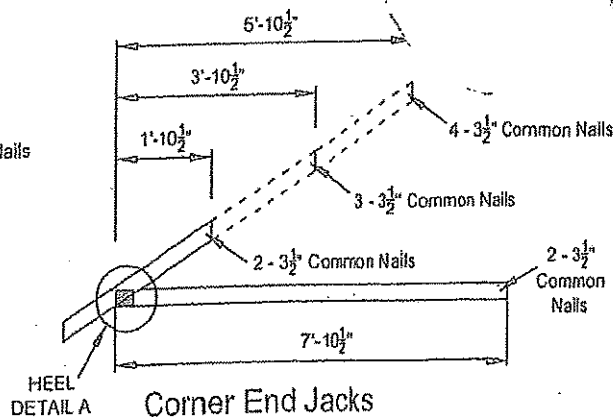


45° Hip End

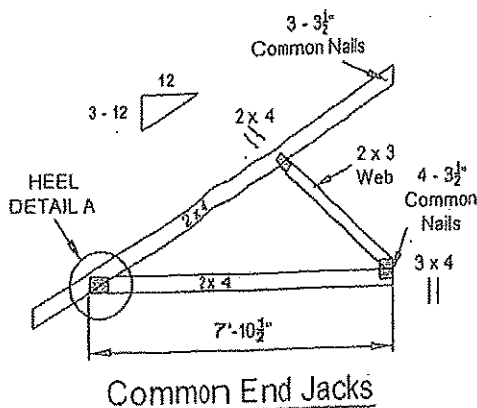
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STRUCTURAL
COMPONENT ONLY



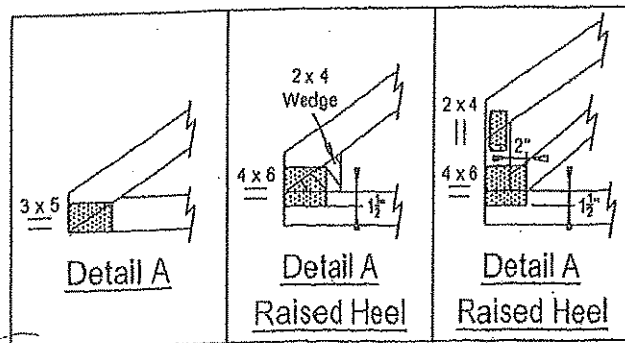
Corner Side Jacks



Corner End Jacks



Common End Jacks

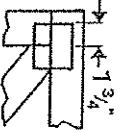


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

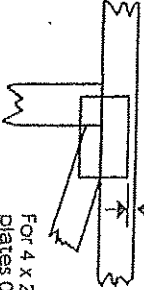
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft.-in.-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

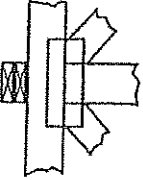
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



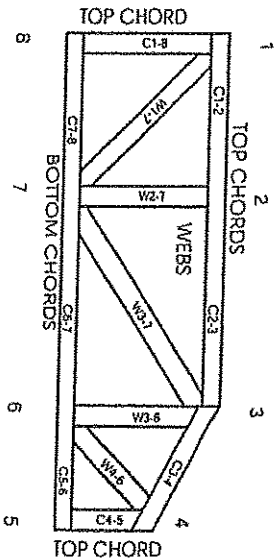
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications

DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft.-in.-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

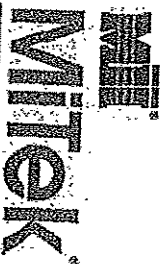
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet Mtl-7473C rev. 10-08

General Safety Notes

Failure to Follow could Cause Property Damage or Personal Injury

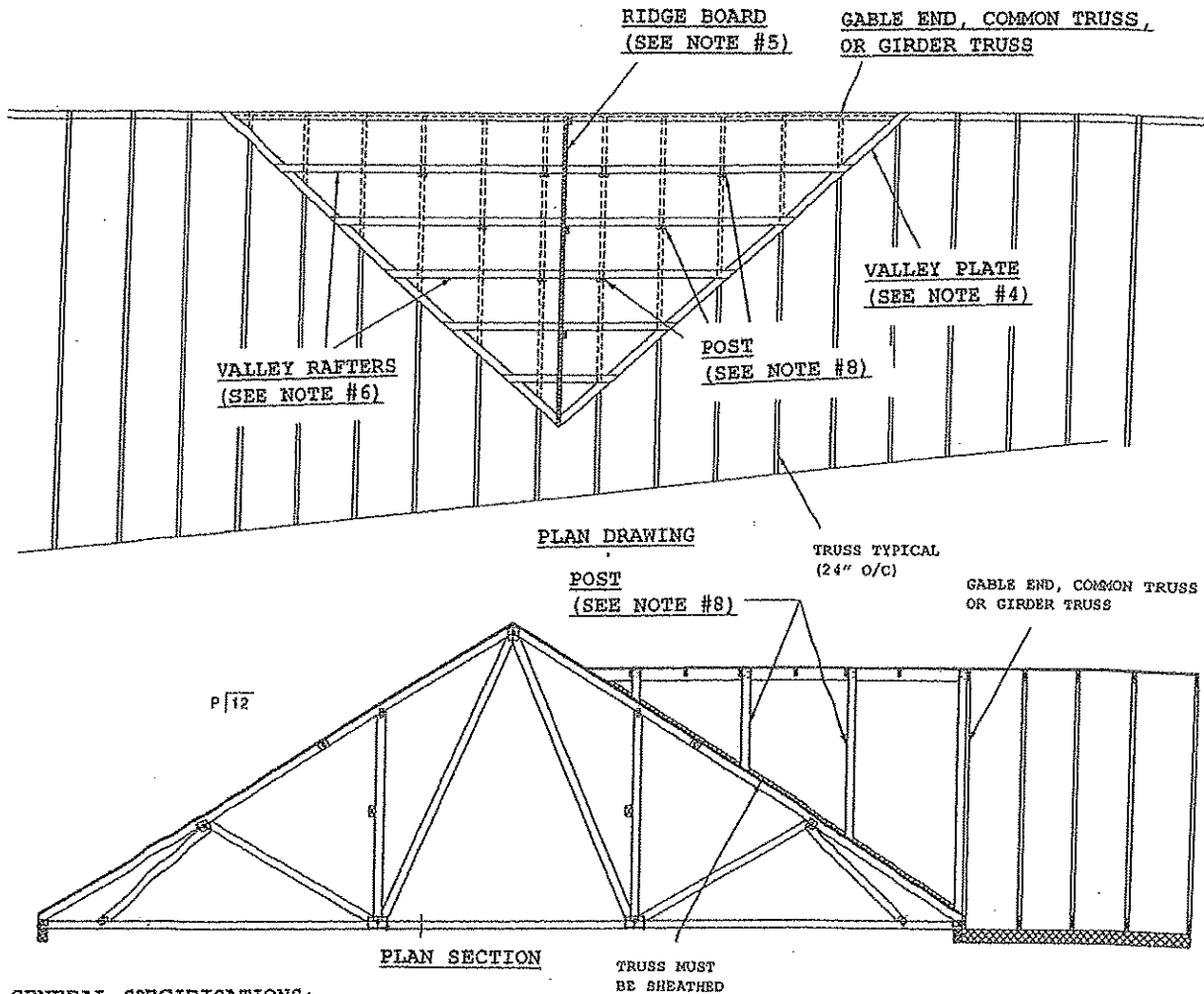
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

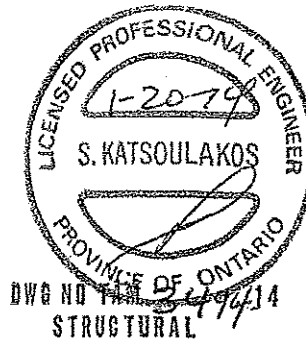
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.