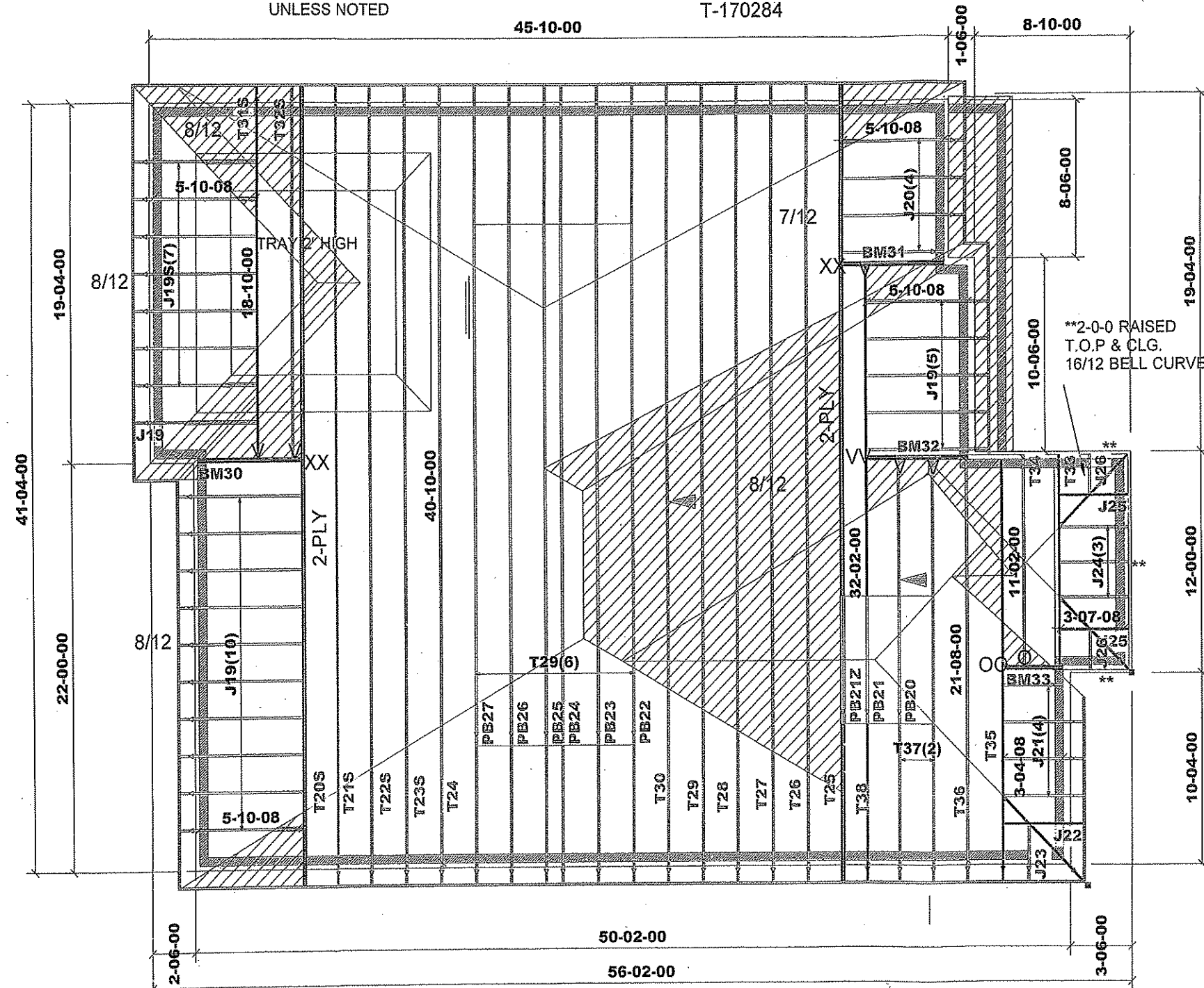


14/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170284



12" FINISH O.H
R.T.M.C
3" STUCCO
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF
FRAMING TO CONFORM TO
PART9 OF THE OBC.
LATEST EDITION ROOF
RAFTERS THAT MEET OR
CROSS OVER TRUSSES
ARE TO BE 2"x4"SPF@
24"o.c. WITH A 2"x4"SPF
VERTICAL POST TO THE
TRUSS UNDER AT EACH
CROSS POINT.POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS
AND BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

DESIGN CONFORMS WITH
THE RELEVANT SECTION
OF THE LATEST EDITION
OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
S_g= 1.3 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
LUS26-2(VV)
HGUS26-2(XX)

DATE: 4/27/17
BCIN:26064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED
ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING
DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING
DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO
NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A
QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE
BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR
PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS
PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE
ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
INDIVIDUAL LOT.

DWG #TAM THROUGH DWG #TAM, INCLUSIVE, DATED; 12

STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH
REFERENCED TRUSS COMPONENT DRAWINGS ANDSEALED HARDWARE DRAWINGS.
HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING
REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS
SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3-1/4"
COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING"
ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME
NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB
MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-
1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF
NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING
FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS
COMPONENTS AND AT LOCATIONS MARKED BY "X".
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X
4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY.

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE
TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH
STUDS AT24" O/C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE
(CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG #TAM 19714-17
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY



APRIL 27, 2017
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS
COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:44080/281390)
DWG #TAM19684-17 THROUGH DWG #TAM19712-17, INCLUSIVE, AND ALL DATED 4-27-17;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)



Job Track: 44080
Layout ID: 281390
Plan Log: 91851

Builder: GREENYORK HOMES /
Project: DEGREY DRIVE
Date: 4/26/2017 Designer: JG

Location: BRAMPTON

Model: YORK 2 / B

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR
UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF
TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

MITek ver 7.5.0



Delivery Shiplist

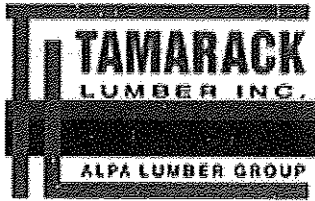
DATE	04/27/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281389 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 2 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1 2 Ply	T1S HIP GIRDER	14.00 12.00	40-10-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	468.66 290.66		
	1 2 Ply	T1Z HIP GIRDER	14.00 0.00	40-10-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	457.58 278.00		
	1	T2 HIP	14.00 0.00	40-10-00	06-07-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	206.48 125.17		
	1	T2S HIP	14.00 12.00	40-10-00	06-07-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	220.89 135.33		
	1	T3 HIP	14.00 0.00	40-10-00	07-11-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	222.16 135.67		
	1	T3S HIP	14.00 12.00	40-10-00	07-11-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	224.30 139.83		
	9	T4 HIP	14.00 0.00	40-10-00	09-03-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	2179.98 1297.53		
	1	T4S HIP	14.00 12.00	40-10-00	09-03-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	261.98 161.17		
	2	T5 HIP	14.00 0.00	40-10-00	10-07-13	2 X 4	2 X 6	01-03-08 01-03-08	02-01-05 02-01-05	510.62 305.34		
	1	T6S HIP GIRDER	8.00 12.00	18-10-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	85.71 58.67		
	1	T7S HIP	8.00 12.00	18-10-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	85.72 58.50		
	5	T8 PIGGYBACK	14.00 0.00	21-08-00	10-00-00	2 X 4	2 X 4	01-03-08 01-03-08	02-01-05 02-01-05	629.65 396.65		
	1	T8S PIGGYBACK	14.00 6.00	21-08-00	10-00-00	2 X 4	2 X 4	01-03-08 01-03-08	02-01-05 02-01-05	133.83 85.33		
	2	T9S SCISSOR	14.00 6.00	11-02-00	08-07-08	2 X 4	2 X 4	01-03-08 01-05-00	02-01-05 02-01-05	117.92 76.66		
	2	PB1 PIGGYBACK	14.00 0.00	27-05-08	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	184.68 112.00		
	2	PB2 PIGGYBACK	14.00 0.00	27-05-08	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	194.90 122.66		
	2	PB3 PIGGYBACK	14.00 0.00	27-05-08	05-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	206.56 129.34		
	2	PB4 PIGGYBACK	14.00 0.00	27-05-08	06-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	219.70 136.34		



Delivery Shiplist

DATE	04/27/17
SALES REP	Rick

JOB TRACK:44080 LAYOUT ID: 281389 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 2 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	PB5 PIGGYBACK	14.00 0.00	07-01-07	04-07-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	153.30 100.02		
	21	J1 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	471.24 287.07		
	7	J1S JACK-OPEN	8.00 12.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	182.00 127.19		

TOTAL # TRUSS= 72.00

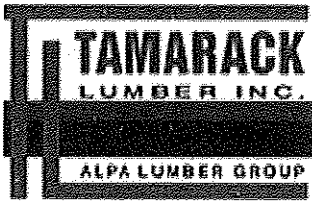
TOTAL BFT OF ALL TRUSSES=

4559.13 BFT. TOTAL WEIGHT OF ALL TRUSSES= 7417.86 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



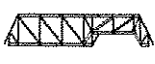
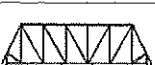
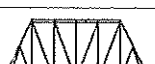
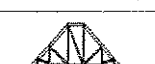
Delivery Shiplist

DATE	04/27/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281390 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T20S HIP GIRDER	14.00	40-10-00	05-02-08	2 X 4	2 X 6	01-01-08	01-11-00	463.40		
	2 Ply		12.00							290.66		
	1	T21S HIP	14.00	40-10-00	06-06-08	2 X 4	2 X 6	01-01-08	01-11-00	218.38		
			12.00					01-01-08	01-11-00	133.67		
	1	T22S HIP	14.00	40-10-00	07-10-08	2 X 4	2 X 6	01-01-08	01-11-00	230.87		
			12.00					01-01-08	01-11-00	143.00		
	1	T23S HIP	14.00	40-10-00	09-02-08	2 X 4	2 X 6	01-01-08	01-11-00	260.67		
			12.00					01-01-08	01-11-00	162.17		
	1	T24 HIP	14.00	40-10-00	10-06-08	2 X 4	2 X 6	01-01-08	01-11-00	252.79		
			0.00					01-01-08	01-11-00	152.67		
	1	T25 PIGGYBACK	14.00	40-10-00	10-06-04	2 X 4	2 X 6	01-01-08	01-11-00	465.24		
	2 Ply		0.00							282.34		
	1	T26 HIP	14.00	40-10-00	05-09-06	2 X 4	2 X 6	01-01-08	01-11-00	218.77		
			0.00					01-01-08	01-11-00	133.17		
	1	T27 HIP	14.00	40-10-00	06-11-06	2 X 4	2 X 6	01-01-08	01-11-00	208.00		
			0.00					01-01-08	01-11-00	127.50		
	1	T28 HIP	14.00	40-10-00	08-01-06	2 X 4	2 X 6	01-01-08	01-11-00	222.06		
			0.00					01-01-08	01-11-00	135.67		
	7	T29 PIGGYBACK	14.00	40-10-00	09-03-06	2 X 4	2 X 6	01-01-08	01-11-00	1683.15		
			0.00					01-01-08	01-11-00	1009.19		
	1	T30 HIP	14.00	40-10-00	10-05-06	2 X 4	2 X 6	01-01-08	01-11-00	251.88		
			0.00					01-01-08	01-11-00	152.67		
	1	T31S HIP GIRDER	8.00	18-10-00	05-02-08	2 X 4	2 X 4	01-01-08	01-03-08	83.67		
			12.00					01-01-08	01-03-08	58.17		
	1	T32S HIP	8.00	18-10-00	06-06-08	2 X 4	2 X 4	01-01-08	01-03-08	84.21		
			12.00					01-01-08	01-03-08	57.50		
	1	T33 HIP GIRDER	16.00	11-02-00	05-07-08	2 X 4	2 X 4	00-03-00	00-03-08	52.92		
			0.00					00-00-00	00-09-08	35.50		
	1	T34 HIP	16.00	11-02-00	10-00-03	2 X 4	2 X 4	00-03-00	02-09-08	58.40		
			0.00					00-00-00	02-09-08	37.67		
	1	T35 HIP GIRDER	14.00	21-08-00	05-10-04	2 X 4	2 X 4	01-01-08	01-11-00	99.76		
			0.00					00-00-00	01-11-00	64.67		
	1	T36 HIP	14.00	21-08-00	08-02-04	2 X 4	2 X 4	01-01-08	01-11-00	106.35		
			0.00					01-01-08	01-11-00	67.33		
	2	T37 HIP	14.00	21-08-00	10-06-04	2 X 4	2 X 4	01-01-08	01-11-00	252.26		
			0.00					00-00-00	01-11-00	156.00		



Delivery Shiplist

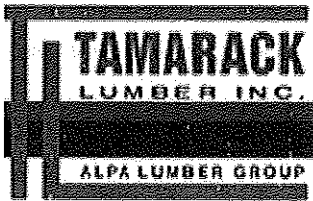
DATE	04/27/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281390 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T38 PIGGYBACK	14.00 0.00	32-02-00	10-06-04	2 X 6	2 X 6	01-01-08 00-00-00	01-11-00 01-11-00	211.83 130.67		
	1	PB20 PIGGYBACK	14.00 0.00	05-10-12	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	22.13 14.83		
	1	PB21 PIGGYBACK	14.00 0.00	05-10-12	03-10-15	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	21.42 14.00		
	1 2 Ply	PB21Z PIGGYBACK	14.00 0.00	05-10-12	03-10-15	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	42.84 28.00		
	1	PB22 PIGGYBACK	14.00 0.00	27-02-04	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	91.81 59.67		
	1	PB23 PIGGYBACK	14.00 0.00	27-02-04	03-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	94.63 58.67		
	1	PB24 PIGGYBACK	14.00 0.00	27-02-04	04-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	99.46 62.00		
	1	PB25 PIGGYBACK	14.00 0.00	27-02-04	05-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	101.97 63.33		
	1	PB26 PIGGYBACK	14.00 0.00	27-02-04	03-09-10	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	96.34 61.33		
	1	PB27 PIGGYBACK	14.00 0.00	27-02-04	02-05-10	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	91.26 56.00		
	16	J19 JACK-OPEN	8.00 0.00	05-10-08	05-02-08	2 X 4	2 X 4	01-01-08 00-00-00	01-03-08 05-02-08	279.68 170.72		
	7	J19S JACK-OPEN	8.00 12.00	05-10-08	05-02-08	2 X 4	2 X 4	01-01-08 00-00-00	01-03-08 03-02-08	177.94 127.19		
	4	J20 JACK-OPEN	7.00 0.00	05-10-08	04-07-06	2 X 4	2 X 4	01-01-08 00-00-00	01-02-04 04-07-06	67.88 42.68		
	4	J21 JACK-OPEN	14.00 0.00	03-04-08	05-10-04	2 X 4	2 X 4	01-01-08 00-00-00	01-11-00 05-10-04	66.60 45.32		
	1	J22 JACK-OPEN	14.00 0.00	03-04-08	04-00-00	2 X 4	2 X 4	01-01-08 -01-07-01	01-11-00 00-03-08	13.86 9.33		
	1	J23 JACK-OPEN	14.00 0.00	01-10-08	04-00-00	2 X 4	2 X 4	01-01-08 -00-01-01	01-11-00 00-03-08	11.14 7.50		
	3	J24 JACK-OPEN	16.00 0.00	03-07-08	05-07-08	2 X 4	2 X 4	00-03-00 00-00-00	00-03-08 05-07-08	40.17 26.01		
	2	J25 JACK-OPEN	16.00 0.00	03-07-08	03-02-01	2 X 4	2 X 4	00-03-00 -01-10-01	00-03-08 00-03-08	19.78 13.34		



Delivery Shiplist

DATE	04/27/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281390 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 2 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J26 JACK-OPEN	16.00 0.00	01-10-08	03-02-01	2 X 4	2 X 4	00-03-00 -00-01-01	00-03-08 00-03-08	15.80 10.66		

TOTAL # TRUSS= 78.00

TOTAL BFT OF ALL TRUSSES=

4200.80 BFT. TOTAL WEIGHT OF ALL TRUSSES= 6779.32 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
1	Hangers	LUS24	
1	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 10.00

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:													
Building number, street name			Unit no.	Lot/con.												
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description														
B. Individual who reviews and takes responsibility for design activities																
Name SAM KATSOUKAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.														
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.												
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail													
Telephone number (519) 287-2242 Business		Fax number (519) 287-5750	Cell number													
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1, of Division C]																
<table style="width: 100%; border: none;"> <tr> <td>☐ House</td> <td>☐ HVAC – House</td> <td>☒ Building Structural</td> </tr> <tr> <td>☐ Small Buildings</td> <td>☐ Building Services</td> <td>☐ Plumbing – House</td> </tr> <tr> <td>☐ Large Buildings</td> <td>☐ Detection, Lighting and Power</td> <td>☐ Plumbing – All Buildings</td> </tr> <tr> <td>☐ Complex Buildings</td> <td>☐ Fire Protection</td> <td>☐ On-site Sewage Systems</td> </tr> </table>					☐ House	☐ HVAC – House	☒ Building Structural	☐ Small Buildings	☐ Building Services	☐ Plumbing – House	☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings	☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems
☐ House	☐ HVAC – House	☒ Building Structural														
☐ Small Buildings	☐ Building Services	☐ Plumbing – House														
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings														
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems														
Description of designer's work GREENYORK HOMES – DEGREY DRIVE – MODEL: YORK 2 – ELEV. A (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM19713-17 DATED 4-27-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.																
D. Declaration of Designer																
I, <u>SAM KATSOUKAKOS, P. ENG</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____																
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.																
Date		Signature of Designer														

NOTE:

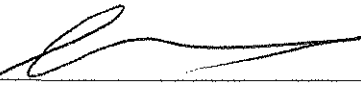
- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM19713-17-S
 DWG#TAM19715-17-S

4-27-17

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:													
Building number, street name		Unit no.	Lot/con.												
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description													
B. Individual who reviews and takes responsibility for design activities															
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.													
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.												
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail												
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number													
C. Design activities undertaken by individual identified in Section B. (Building Code Table 3.5.2.1. of Division C)															
<table style="width: 100%; border: none;"> <tr> <td>☐ House</td> <td>☐ HVAC – House</td> <td>☒ Building Structural</td> </tr> <tr> <td>☐ Small Buildings</td> <td>☐ Building Services</td> <td>☐ Plumbing – House</td> </tr> <tr> <td>☐ Large Buildings</td> <td>☐ Detection, Lighting and Power</td> <td>☐ Plumbing – All Buildings</td> </tr> <tr> <td>☐ Complex Buildings</td> <td>☐ Fire Protection</td> <td>☐ On-site Sewage Systems</td> </tr> </table>				☐ House	☐ HVAC – House	☒ Building Structural	☐ Small Buildings	☐ Building Services	☐ Plumbing – House	☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings	☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems
☐ House	☐ HVAC – House	☒ Building Structural													
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Description of designer's work GREENYORK HOMES – DEGREY DRIVE – MODEL: YORK 2 – ELEV. B (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM19714-17 DATED 4-27-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.															
D. Declaration of Designer															
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name)															
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.															
Individual BCIN: <u>26064</u>															
Firm BCIN: <u>29991</u>															
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.															
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Basis for exemption from registration: _____															
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Basis for exemption from registration and qualification: _____															
I certify that:															
1. The information contained in this schedule is true to the best of my knowledge.															
2. I have submitted this application with the knowledge and consent of the firm.															
Date		Signature of Designer													
4-27-17															

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
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DWG#TAM 19714-17-S
DWG#TAM 19716-17-S

42737


CONFIDENTIAL

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TMWW+I	MT20	5.0	8.0	3.00	1.50
H	TMW+u	MT20	3.0	6.0		
I	TS-I	MT20	5.0	8.0		
J	TMWW-I	MT20	5.0	8.0	2.50	3.50
K	TMWW-I	MT20	5.0	8.0	2.50	2.25
L	TTWW-h	MT20	8.0	9.0	2.50	4.50
M	TMW-I	MT20	5.0	8.0	2.25	2.50
O	BMV1+p	MT20	3.0	6.0		
P	BBWW+m	MT20	8.0	9.0	Edge	
Q	BBWW+m	MT20	5.0	12.0	5.00	2.50
R	BMWW-I	MT20	5.0	8.0	2.50	2.25
S	BMWWWW-I	MT16	12.0	22.5	5.75	6.50
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWWW-I	MT20	8.0	12.0	4.00	4.00
V	BMWW-I	MT20	5.0	6.0		
W	BS-I	MT20	5.0	6.0		
X	BMWW-I	MT20	6.0	9.0		
Y	BMWW-I	MT20	5.0	6.0		
Z	BMV1+p	MT20	3.0	6.0		
AA	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 122.7 lbs FACTORED DOWN AT 2-9-0, 149.9 lbs FACTORED DOWN AT 3-11-4, 149.9 lbs FACTORED DOWN AT 5-11-4, 149.9 lbs FACTORED DOWN AT 7-11-4, 149.9 lbs FACTORED DOWN AT 9-11-4, 149.9 lbs FACTORED DOWN AT 11-11-4, 149.9 lbs FACTORED DOWN AT 13-11-4, 149.9 lbs FACTORED DOWN AT 15-11-4, AND 149.9 lbs FACTORED DOWN AT 17-11-4, AND 149.9 lbs FACTORED DOWN AT 19-11-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 87.4 lbs FACTORED DOWN AT 5-11-4, 87.4 lbs FACTORED DOWN AT 7-11-4, 87.4 lbs FACTORED DOWN AT 9-11-4, 87.4 lbs FACTORED DOWN AT 11-11-4, 87.4 lbs FACTORED DOWN AT 13-11-4, 87.4 lbs FACTORED DOWN AT 15-11-4, 87.4 lbs FACTORED DOWN AT 17-11-4, AND 87.4 lbs FACTORED DOWN AT 19-11-4, AND 2404.5 lbs FACTORED DOWN AT 21-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
Y-AL	0 / 2625	-28.0	-28.0 0.26 (1)	10.00			
AL-AM	0 / 2625	-28.0	-28.0 0.26 (1)	10.00			
AM-AN	0 / 2625	-28.0	-28.0 0.26 (1)	10.00			
AN-X	0 / 2625	-28.0	-28.0 0.26 (1)	10.00			
X-AO	0 / 7469	-28.0	-28.0 0.56 (1)	10.00			
AO-AP	0 / 7469	-28.0	-28.0 0.56 (1)	10.00			
AP-W	0 / 7469	-28.0	-28.0 0.56 (1)	10.00			
W-V	0 / 7469	-28.0	-28.0 0.56 (1)	10.00			
V-AQ	0 / 10426	-28.0	-28.0 0.74 (1)	10.00			
AQ-AR	0 / 10426	-28.0	-28.0 0.74 (1)	10.00			
AR-AS	0 / 10426	-28.0	-28.0 0.74 (1)	10.00			
AS-U	0 / 10426	-28.0	-28.0 0.74 (1)	10.00			
U-T	0 / 145	-28.0	-28.0 0.10 (2)	10.00			
S-R	0 / 12592	-28.0	-28.0 0.89 (1)	10.00			
R-Q	0 / 6087	-28.0	-28.0 0.47 (1)	10.00			
Q-P	0 / 3291	-28.0	-28.0 0.22 (1)	10.00			
P-O	0 / 0	-28.0	-28.0 0.01 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-9-0	-123	-123	--	FRONT	VERT	TOTAL
U	21-10-8	-2405	-2405	--	BACK	VERT	TOTAL
W	13-11-4	-50	-87	--	BACK	VERT	TOTAL
AB	3-11-4	-150	-150	--	BACK	VERT	TOTAL
AC	5-11-4	-150	-150	--	BACK	VERT	TOTAL
AD	7-11-4	-150	-150	--	BACK	VERT	TOTAL
AE	9-11-4	-150	-150	--	BACK	VERT	TOTAL
AF	11-11-4	-150	-150	--	BACK	VERT	TOTAL
AG	13-11-4	-150	-150	--	BACK	VERT	TOTAL
AH	15-11-4	-150	-150	--	BACK	VERT	TOTAL
AI	17-11-4	-150	-150	--	BACK	VERT	TOTAL
AJ	19-11-4	-150	-150	--	BACK	VERT	TOTAL
AK	1-11-4	-50	-87	--	BACK	VERT	TOTAL
AL	3-11-4	-50	-87	--	BACK	VERT	TOTAL
AM	5-11-4	-50	-87	--	BACK	VERT	TOTAL
AN	7-11-4	-50	-87	--	BACK	VERT	TOTAL
AO	9-11-4	-50	-87	--	BACK	VERT	TOTAL
AP	11-11-4	-50	-87	--	BACK	VERT	TOTAL
AQ	13-11-4	-50	-87	--	BACK	VERT	TOTAL
AR	15-11-4	-50	-87	--	BACK	VERT	TOTAL
AS	17-11-4	-50	-87	--	BACK	VERT	TOTAL



1622

DWG NO. TAM19664-17
STRUCTURAL
COMPONENT ONLY

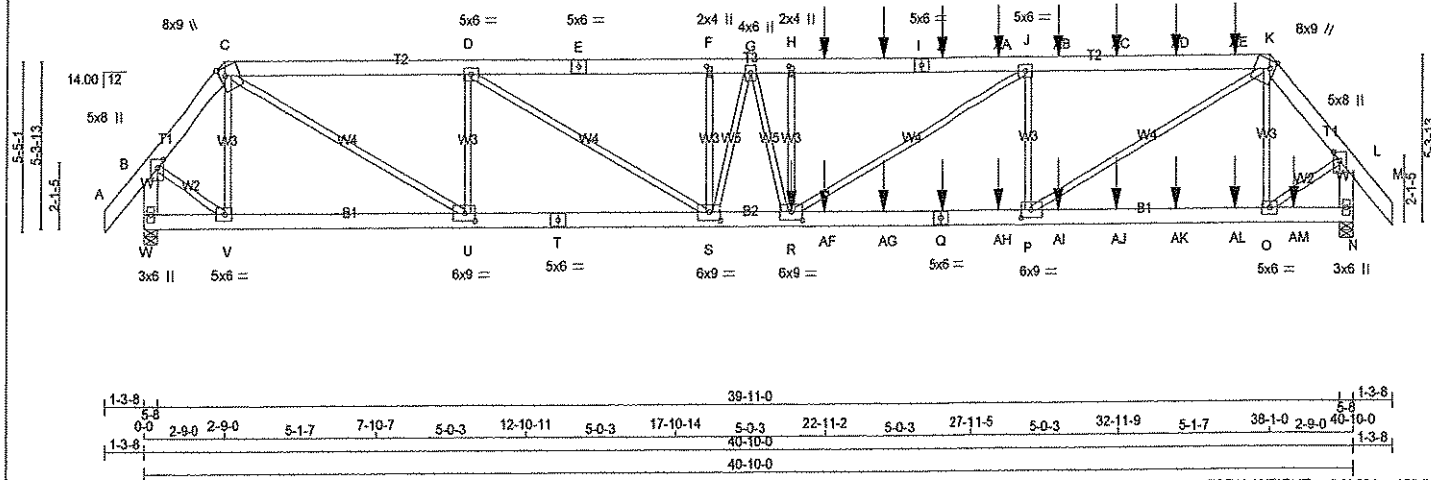
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 5 Oct 5 2016 Mitek Industries, Inc. Thu Apr 27 15:51:06 2017 Page 1

ID:jjDnzujgMGNFCVaO_fHsDzMo73-bsgRCuO4ySyA6enYdMBXLhzS6V9BgWELZ1rDm1zMUNJ

1-3-8 0-0 2-9-0 5-1-7 7-10-7 5-0-3 12-10-11 5-0-3 17-10-14 2-6-2 20-5-0 2-6-22-11-2 5-0-3 27-11-5 5-0-3 32-11-9 5-1-7 38-1-0 2-9-0 40-10-0 1-3-8
Scale = 1:72.0



TOTAL WEIGHT = 2 X 229 = 458 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - I	2x6	DRY No.2	SPF
I - K	2x6	DRY No.2	SPF
K - M	2x6	DRY No.2	SPF
W - B	2x6	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
W - T	2x6	DRY No.2	SPF
T - Q	2x6	DRY No.2	SPF
Q - N	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - E	2	12
E - I	2	12
I - K	2	12
K - M	2	12
W - B	2	12
N - L	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
W - T	2	12
T - Q	2	12
Q - N	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
H - R	1	6
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	8.0	3.25	2.25
C TTWW+m	MT20	8.0	9.0	Edge	2.00
D TMVW+l	MT20	5.0	6.0		
E TS-l	MT20	5.0	6.0		
F TMVW+w	MT20	2.0	4.0	2.50	1.00
G TMVW+l	MT20	4.0	6.0		
H TMVW+w	MT20	2.0	4.0	2.50	1.00
I TS-l	MT20	5.0	6.0		
J TMVW+l	MT20	5.0	6.0		
K TTWW+m	MT20	8.0	9.0	Edge	2.00
L TMVW+p	MT20	5.0	8.0	3.25	2.25
N BMVl+p	MT20	3.0	6.0		
O BMVW+l	MT20	5.0	6.0		
P BMVW+l	MT20	6.0	9.0	3.00	4.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
W	3641	0	5-8	5-8
N	4565	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2982	1656 / 0	688 / 0	0 / 0	0 / 0	658 / 0	0 / 0
N	3759	2055 / 0	863 / 0	0 / 0	0 / 0	840 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 43	-77.4 -77.4 0.03 (1)	V-C	-819 / 0
B-C	-3121 / 0	-77.4 -77.4 0.04 (1)	C-U	0 / 5530
C-D	-6763 / 0	-77.4 -77.4 0.31 (1)	U-D	-2577 / 0
D-E	-9926 / 0	-77.4 -77.4 0.44 (1)	D-S	0 / 3688
E-F	-9926 / 0	-77.4 -77.4 0.44 (1)	S-F	-441 / 0
F-G	-9926 / 0	-77.4 -77.4 0.23 (1)	R-H	-803 / 0
G-H	-10736 / 0	-77.4 -77.4 0.32 (1)	P-K	-2896 / 0
H-X	-10736 / 0	-77.4 -77.4 0.73 (1)	P-K	0 / 6599
X-Y	-10736 / 0	-77.4 -77.4 0.73 (1)	Q-K	-940 / 0
Y-I	-10736 / 0	-77.4 -77.4 0.73 (1)	B-V	0 / 2279
I-Z	-10736 / 0	-77.4 -77.4 0.73 (1)	O-L	0 / 2904
Z-AA	-10736 / 0	-77.4 -77.4 0.73 (1)	R-J	0 / 2917
AA-J	-10736 / 0	-77.4 -77.4 0.73 (1)	G-R	0 / 1612
J-AB	-8234 / 0	-77.4 -77.4 0.60 (1)	S-G	-1364 / 0
AB-AC	-8234 / 0	-77.4 -77.4 0.60 (1)		
AC-AD	-8234 / 0	-77.4 -77.4 0.60 (1)		
AD-AE	-8234 / 0	-77.4 -77.4 0.60 (1)		
AE-K	-8234 / 0	-77.4 -77.4 0.60 (1)		
K-L	-3978 / 0	-77.4 -77.4 0.05 (1)		
L-M	0 / 43	-77.4 -77.4 0.03 (1)		
W-B	-3684 / 0	0.0 0.0 0.14 (1)		
N-L	-4635 / 0	0.0 0.0 0.18 (1)		
W-V	0 / 0	-28.0 -28.0 0.10 (2)		
V-U	0 / 1991	-28.0 -28.0 0.22 (1)		
U-T	0 / 6764	-28.0 -28.0 0.51 (1)		
T-S	0 / 6764	-28.0 -28.0 0.51 (1)		
S-R	0 / 10297	-28.0 -28.0 0.68 (1)		
R-AF	0 / 8235	-28.0 -28.0 0.66 (1)		
AF-AG	0 / 8235	-28.0 -28.0 0.66 (1)		
AG-Q	0 / 8235	-28.0 -28.0 0.66 (1)		
Q-AH	0 / 8235	-28.0 -28.0 0.66 (1)		
AH-P	0 / 8235	-28.0 -28.0 0.66 (1)		
P-AI	0 / 2540	-28.0 -28.0 0.31 (2)		
AI-AJ	0 / 2540	-28.0 -28.0 0.31 (2)		
AJ-AK	0 / 2540	-28.0 -28.0 0.31 (2)		
AK-AL	0 / 2540	-28.0 -28.0 0.31 (2)		
AL-O	0 / 2540	-28.0 -28.0 0.31 (2)		
O-AM	0 / 0	-28.0 -28.0 0.17 (2)		
AM-N	0 / 0	-28.0 -28.0 0.17 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.38")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.34")
ALLOWABLE DEFL.(TL) = $L/360$ (1.38")
CALCULATED VERT. DEFL.(TL) = $L/880$ (0.56")

CSI: TC=0.73 (H-J:1), BC=0.68 (R-S:1), WB=0.82 (K-P:1), SSI=0.25 (J-K:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

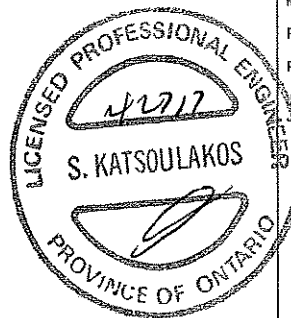
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP= 0.60 (J) (INPUT = 0.50)
ISI METAL= 0.73 (I) (INPUT = 1.00)



DRWG NO. TAM 19665-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:jjDnzujgMGNFCVaO fhJsDzMo73-bsgRCuO4ySyA6enYdMBXLhzS6V9BgWELZ1rDm1zMUNJ

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
Q	BS-1	MT20	5.0	6.0	
R	BMWWW-t	MT20	6.0	9.0	3.25 4.50
S	BMWWW-t	MT20	6.0	9.0	3.25 4.50
T	BS-1	MT20	5.0	6.0	
U	BMWWW-t	MT20	6.0	9.0	3.00 4.25
V	BMWWW-t	MT20	5.0	6.0	
W	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.9 lbs FACTORED DOWN AT 22-10-12, 149.9 lbs FACTORED DOWN AT 24-10-12, 149.9 lbs FACTORED DOWN AT 26-10-12, 149.9 lbs FACTORED DOWN AT 28-10-12, 149.9 lbs FACTORED DOWN AT 30-10-12, 149.9 lbs FACTORED DOWN AT 32-10-12, AND 149.9 lbs FACTORED DOWN AT 34-10-12, AND 149.9 lbs FACTORED DOWN AT 36-10-12 ON TOP CHORD, AND 2028.2 lbs FACTORED DOWN AT 21-9-8, 87.4 lbs FACTORED DOWN AT 22-10-12, 87.4 lbs FACTORED DOWN AT 24-10-12, 87.4 lbs FACTORED DOWN AT 26-10-12, 87.4 lbs FACTORED DOWN AT 28-10-12, 87.4 lbs FACTORED DOWN AT 30-10-12, 87.4 lbs FACTORED DOWN AT 32-10-12, 87.4 lbs FACTORED DOWN AT 34-10-12, AND 87.4 lbs FACTORED DOWN AT 36-10-12, AND 87.4 lbs FACTORED DOWN AT 38-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE		
Q	26-10-12	-50	-87	---	FRONT	VERT	TOTAL		
R	21-9-8	-2028	-2028	---	FRONT	VERT	TOTAL		
X	22-10-12	-150	-150	---	FRONT	VERT	TOTAL		
Y	24-10-12	-150	-150	---	FRONT	VERT	TOTAL		
Z	26-10-12	-150	-150	---	FRONT	VERT	TOTAL		
AA	28-10-12	-150	-150	---	FRONT	VERT	TOTAL		
AB	30-10-12	-150	-150	---	FRONT	VERT	TOTAL		
AC	32-10-12	-150	-150	---	FRONT	VERT	TOTAL		
AD	34-10-12	-150	-150	---	FRONT	VERT	TOTAL		
AE	36-10-12	-150	-150	---	FRONT	VERT	TOTAL		
AF	22-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AG	24-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AH	28-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AI	30-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AJ	32-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AK	34-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AL	36-10-12	-50	-87	---	FRONT	VERT	TOTAL		
AM	38-10-12	-50	-87	---	FRONT	VERT	TOTAL		



DWG NO. TAM 19665-17
 STRUCTURAL
 COMPONENT ONLY

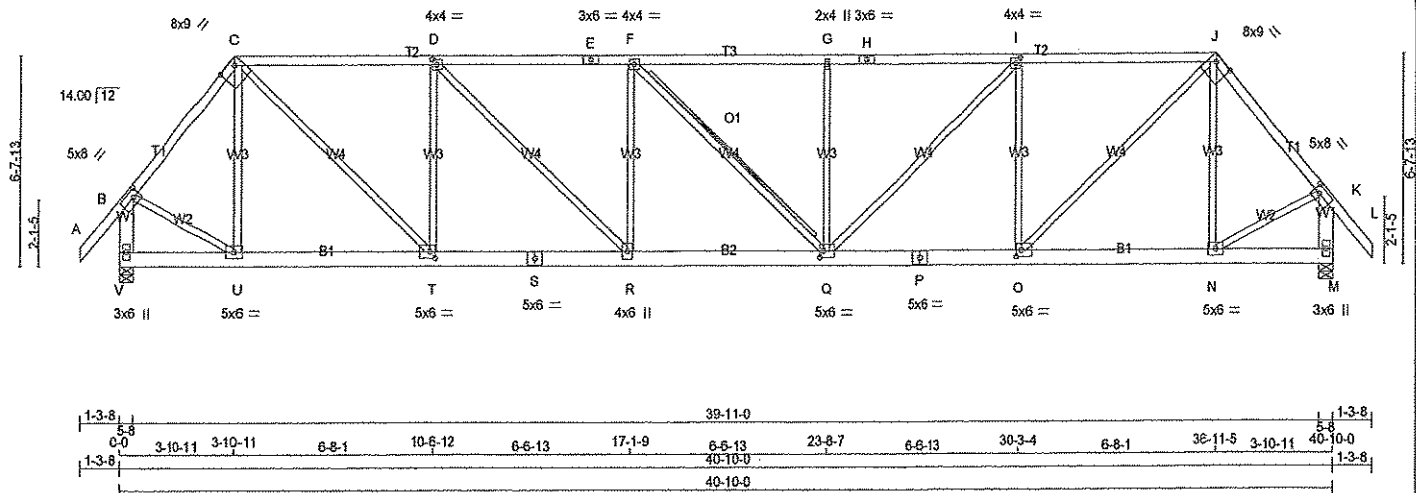
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281389	T2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:jjDnzujgMGNFCVaO_fHsDzMo73-32EqPEPjim51koMkB3mtuWaaVYdPy7UohbmITzMUNl

-1-3-8 0 3-10-11 3-10-11 6-8-1 10-6-12 6-6-13 17-1-9 6-6-13 23-8-7 6-6-13 30-3-4 6-8-1 36-11-5 3-10-11 40-10-0 1-3-8 1-8
Scale = 1:71.7



TOTAL WEIGHT = 206 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
V	2260	0	2260	0	0
M	2260	0	2260	0	0

UNFACTORED REACTIONS							
	1ST LCASE	MAX	MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
V	1862	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
M	1862	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 80% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-77.4 -77.4 0.11 (1)	10.00	U-C	-334 / 13	0.23 (1)	
B-C	-1913 / 0	-77.4 -77.4 0.29 (1)	4.61	C-T	0 / 2077	0.47 (1)	
C-D	-2750 / 0	-77.4 -77.4 0.79 (1)	3.47	T-D	-1239 / 0	0.85 (1)	
D-E	-3459 / 0	-77.4 -77.4 0.94 (1)	3.02	D-R	0 / 980	0.22 (1)	
E-F	-3459 / 0	-77.4 -77.4 0.94 (1)	3.02	R-F	-490 / 0	0.34 (1)	
F-G	-3454 / 0	-77.4 -77.4 0.76 (1)	3.26	F-Q	-6 / 0	0.00 (1)	
G-H	-3454 / 0	-77.4 -77.4 0.94 (1)	3.02	Q-G	-492 / 0	0.34 (1)	
H-I	-3454 / 0	-77.4 -77.4 0.94 (1)	3.02	I-Q	0 / 972	0.22 (1)	
I-J	-2751 / 0	-77.4 -77.4 0.80 (1)	3.46	Q-I	-1235 / 0	0.84 (1)	
J-K	-1912 / 0	-77.4 -77.4 0.29 (1)	4.61	O-J	0 / 2079	0.47 (1)	
K-L	0 / 41	-77.4 -77.4 0.11 (1)	10.00	N-J	-335 / 12	0.23 (1)	
V-B	-2230 / 0	0.0 0.0 0.17 (1)	6.85	B-U	0 / 1341	0.30 (1)	
M-K	-2229 / 0	0.0 0.0 0.17 (1)	6.85	N-K	0 / 1340	0.30 (1)	
V-U	0 / 0	-28.0 -28.0 0.10 (2)	10.00				
U-T	0 / 1233	-28.0 -28.0 0.22 (2)	10.00				
T-S	0 / 2750	-28.0 -28.0 0.38 (1)	10.00				
S-R	0 / 2750	-28.0 -28.0 0.38 (1)	10.00				
R-Q	0 / 3459	-28.0 -28.0 0.48 (1)	10.00				
Q-P	0 / 2751	-28.0 -28.0 0.39 (1)	10.00				
P-O	0 / 2751	-28.0 -28.0 0.39 (1)	10.00				
O-N	0 / 1233	-28.0 -28.0 0.22 (2)	10.00				
N-M	0 / 0	-28.0 -28.0 0.10 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.38")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.24")
ALLOWABLE DEFL. (TL) = $L/360$ (1.38")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.39")

CSI: TC=0.94 (G-I), BC=0.48 (Q-R), WB=0.85 (D-T), SSI=0.24 (I-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

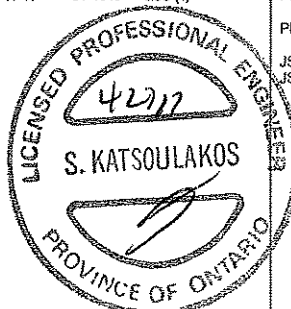
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.49 (P) (INPUT = 1.00)



DWG NO. TAM 19666-17
STRUCTURAL
COMPONENT ONLY

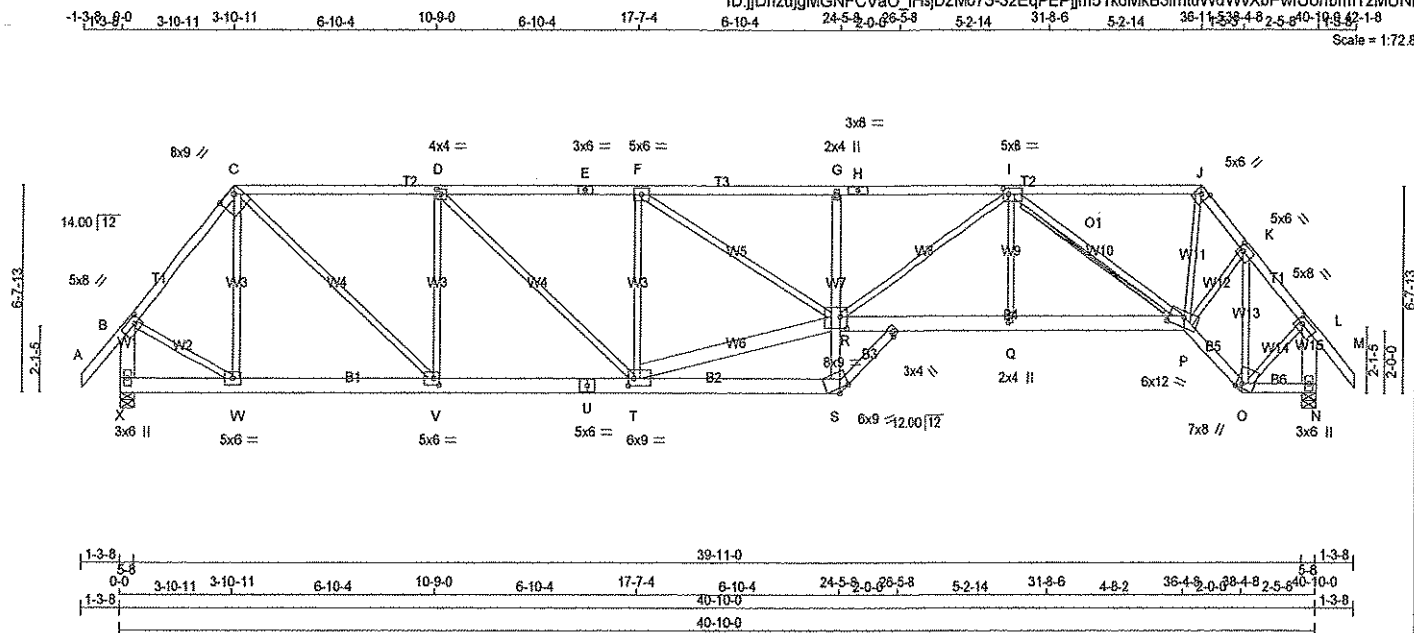
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281389	T2S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:jjDnzujgMGNFCVaO_1hsDzMo73-32EqPEPjm51koMk83mtuWdWvXbPwUohbmITzMUN

Scale = 1:72.8



TOTAL WEIGHT = 221 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - H	2x4	DRY	1650F 1.5E	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
J - M	2x4	DRY	1650F 1.5E	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - G	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
J	T	TYPE	PLATES	W	LEN	Y X
B		TMWV-t	MT20	5.0	8.0	2.50 2.50
C		TTWW-h	MT20	8.0	9.0	2.00 6.25
D		TMWV-t	MT20	4.0	4.0	2.00 1.75
E		TS-t	MT20	3.0	6.0	
F		TMWV-t	MT20	5.0	6.0	
G		TMWV-w	MT20	2.0	4.0	
H		TS-t	MT20	3.0	8.0	
I		TMWV-t	MT20	5.0	8.0	2.25 2.25
J		TTWW-h	MT20	5.0	6.0	Edge
K		TMWV-t	MT20	5.0	6.0	2.50 2.00
L		TMWV-t	MT20	5.0	8.0	2.25 2.50
N		BMV1-p	MT20	3.0	6.0	
O		BBWV-m	MT20	7.0	8.0	Edge 2.25
P		BBWV-m	MT20	6.0	12.0	3.75 6.00
Q		BMWV-w	MT20	2.0	4.0	2.50 1.00
R		BBWV-m	MT20	8.0	9.0	4.50 2.75
S						
S						
S		BBW-m	MT20	6.0	9.0	Edge
T		BBWV-t	MT20	6.0	9.0	2.75 2.25
U		BS-t	MT20	5.0	6.0	
V		BBWV-t	MT20	5.0	6.0	2.50 2.25
W		BBWV-m	MT20	5.0	6.0	
X		BMV1-p	MT20	3.0	6.0	
Y		NP-h	MT20	3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
X	2259	0	2259	0
N	2259	0	2259	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
X	1861	1016 / 0	429 / 0
N	1861	1016 / 0	429 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX
FR-TO							FR-TO					
A-B	0 / 41		-77.4	-77.4	0.09 (1)	10.00	W-C	-326 / 24	0.22 (1)			
B-C	-1915 / 0		-77.4	-77.4	0.21 (1)	5.24	C-V	0 / 2097	0.47 (1)			
C-D	-2791 / 0		-77.4	-77.4	0.82 (1)	4.11	V-D	-1231 / 0	0.84 (1)			
D-E	-3488 / 0		-77.4	-77.4	0.69 (1)	3.72	D-T	0 / 953	0.21 (1)			
E-F	-3488 / 0		-77.4	-77.4	0.69 (1)	3.72	T-F	-1458 / 0	1.00 (1)			
F-G	-4977 / 0		-77.4	-77.4	0.75 (1)	3.25	S-R	0 / 135	0.12 (1)			
G-H	-4993 / 0		-77.4	-77.4	0.72 (1)	3.19	R-G	-467 / 0	0.12 (1)			
H-I	-4963 / 0		-77.4	-77.4	0.72 (1)	3.19	I-R	0 / 1228	0.28 (1)			
I-J	-1984 / 0		-77.4	-77.4	0.45 (1)	4.87	Q-I	0 / 358	0.08 (3)			
J-K	-2711 / 0		-77.4	-77.4	0.06 (1)	4.72	P-J	0 / 1814	0.41 (1)			
K-L	-1671 / 0		-77.4	-77.4	0.08 (1)	5.65	P-K	0 / 1067	0.24 (1)			
L-M	0 / 41		-77.4	-77.4	0.09 (1)	10.00	O-K	-1759 / 0	0.60 (1)			
X-B	-2232 / 0		0.0	0.0	0.17 (1)	6.84	B-W	0 / 1342	0.30 (1)			
N-L	-2225 / 0		0.0	0.0	0.18 (1)	6.85	O-L	0 / 1322	0.30 (1)			
X-W	0 / 0		-28.0	-28.0	0.11 (2)	10.00	T-R	0 / 3613	0.43 (1)			
W-V	0 / 1235		-28.0	-28.0	0.23 (2)	10.00	F-R	0 / 1743	0.32 (1)			
U-U	0 / 2791		-28.0	-28.0	0.40 (1)	10.00	I-P	-2468 / 0	0.20 (1)			
U-T	0 / 2791		-28.0	-28.0	0.40 (1)	10.00						
T-S	0 / 40		-28.0	-28.0	0.15 (3)	10.00						
R-Q	0 / 3988		-28.0	-28.0	0.54 (1)	10.00						
Q-P	0 / 3988		-28.0	-28.0	0.54 (1)	10.00						
P-O	0 / 1492		-28.0	-28.0	0.26 (1)	10.00						
O-N	0 / 0		-28.0	-28.0	0.05 (2)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.35")
ALLOWABLE DEFL. (TL) = L/360 (1.36")
CALCULATED VERT. DEFL. (TL) = L/857 (0.57")

CSI: TC=0.75 (F-G-1), BC=0.54 (Q-R-1), WB=1.00 (I-P-1), SSI=0.25 (C-D-1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

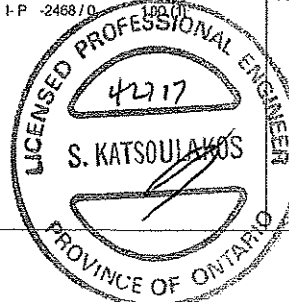
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

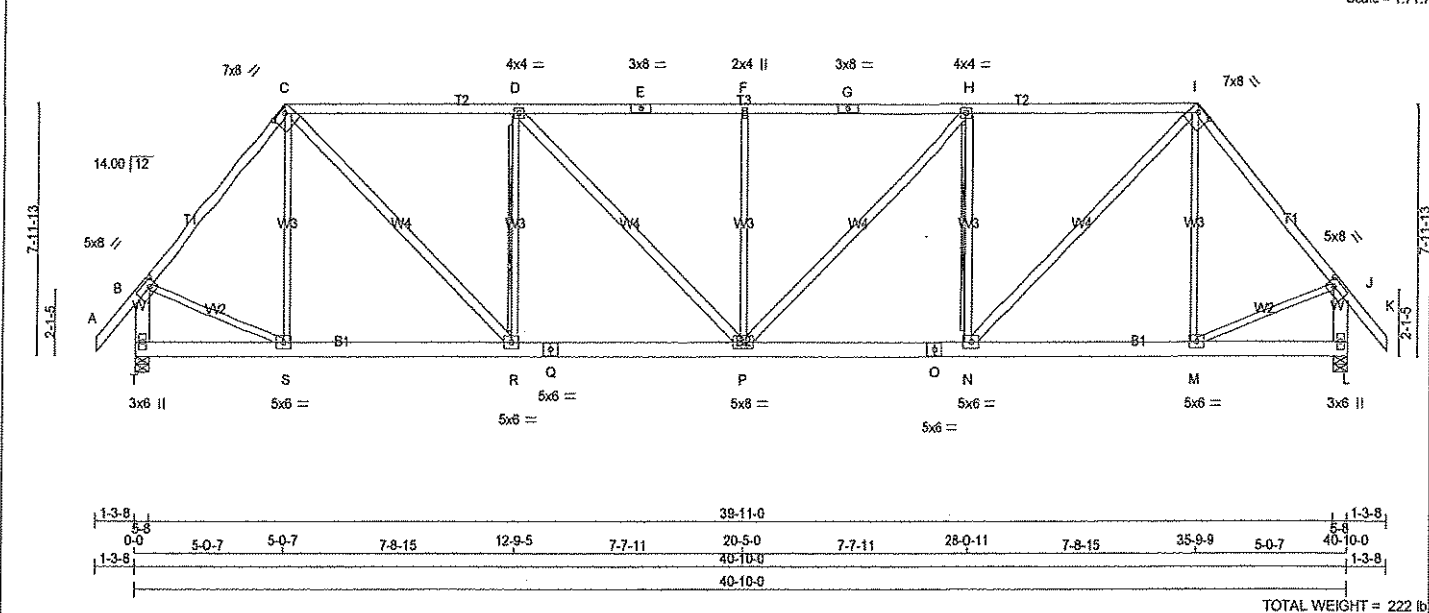
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (R) (INPUT=0.90)
JSI METAL=0.96 (I) (INPUT=1.00)



DRWG NO. TAM 19667-17
STRUCTURAL
COMPONENT ONLY



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS								SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D	TOP CH.	LL	PSF					
A - C	2x4	DRY	1650F 1.5E	SPF	GROSS REACTION	GROSS REACTION	BRG	DL	=	23.3	PSF				
C - E	2x4	DRY	1650F 1.5E	SPF	JT VERT	DOWN	IN-SX	DL	=	3.0	PSF				
E - G	2x4	DRY	1650F 1.5E	SPF	T 2260	0	5-8	BOT CH.	LL	=	10.5	PSF			
G - I	2x4	DRY	1650F 1.5E	SPF	L 2260	0	5-8	DL	=	7.0	PSF				
I - K	2x4	DRY	1650F 1.5E	SPF	UNFACTORED REACTIONS								TOTAL LOAD = 43.8 PSF		
T - B	2x6	DRY	No.2	SPF	1ST LCASE								SPACING = 24.0 IN. C/C		
L - J	2x6	DRY	No.2	SPF	JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	LOADING IN FLAT SECTION BASED ON A			
T - Q	2x6	DRY	No.2	SPF	T 1862	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0	SLOPE OF 6.00/12			
Q - O	2x6	DRY	No.2	SPF	L 1862	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
O - L	2x6	DRY	No.2	SPF	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L								THIS DESIGN COMPLIES WITH:		
ALL WEBS EXCEPT				BRACING								- PART 9 OF OBC 2012, CBCC 2012, ABC 2014			
C - R	2x4	DRY	No.2	SPF	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.								- CSA 086-09		
D - P	2x4	DRY	No.2	SPF	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.								- TPIC 2011		
P - H	2x4	DRY	No.2	SPF	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										
N - I	2x4	DRY	No.2	SPF											
DRY: SEASONED LUMBER.															

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-t	MT20	5.0	8.0	2.50 2.50
C	TTWW-h	MT20	7.0	8.0	1.75 5.00
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	8.0	
F	TMWW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	8.0	
H	TMWW-t	MT20	4.0	4.0	
I	TTWW-h	MT20	7.0	8.0	1.75 5.00
J	TMWW-t	MT20	5.0	8.0	2.50 2.50
L	BMV-t+p	MT20	3.0	6.0	
M, N, R, S					
M	BMWW-t	MT20	5.0	6.0	
O	BS-t	MT20	5.0	6.0	
P	BMWW-t	MT20	5.0	6.0	
Q	BS-t	MT20	5.0	6.0	
T	BMV-t+p	MT20	3.0	6.0	

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC (LC)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 41	-77.4	-77.4 0.09 (1)	10.00	S-C	-211 / 121	0.24 (1)
B-C	-1665 / 0	-77.4	-77.4 0.39 (1)	5.02	C-R	0 / 1808	0.29 (1)
C-D	-2562 / 0	-77.4	-77.4 0.81 (1)	4.00	R-D	-1042 / 0	0.93 (1)
D-E	-2938 / 0	-77.4	-77.4 0.86 (1)	3.76	D-P	0 / 530	0.09 (1)
E-F	-2938 / 0	-77.4	-77.4 0.86 (1)	3.76	P-F	-545 / 0	0.82 (1)
F-G	-2938 / 0	-77.4	-77.4 0.86 (1)	3.76	P-H	0 / 530	0.09 (1)
G-H	-2938 / 0	-77.4	-77.4 0.86 (1)	3.76	H-H	-1042 / 0	0.93 (1)
H-I	-2562 / 0	-77.4	-77.4 0.81 (1)	4.00	N-I	0 / 1808	0.29 (1)
I-J	-1965 / 0	-77.4	-77.4 0.39 (1)	5.02	M-I	-211 / 121	0.24 (1)
J-K	0 / 41	-77.4	-77.4 0.09 (1)	10.00	B-S	0 / 1338	0.30 (1)
T-B	-2210 / 0	0.0	0.0 0.17 (1)	6.87	M-J	0 / 1338	0.30 (1)
L-J	-2210 / 0	0.0	0.0 0.17 (1)	6.87			
T-S	0 / 0	-28.0	-28.0 0.13 (2)	10.00			
S-R	0 / 1271	-28.0	-28.0 0.27 (2)	10.00			
R-Q	0 / 2562	-28.0	-28.0 0.39 (2)	10.00			
Q-P	0 / 2562	-28.0	-28.0 0.39 (2)	10.00			
P-O	0 / 2562	-28.0	-28.0 0.39 (2)	10.00			
O-N	0 / 2562	-28.0	-28.0 0.39 (2)	10.00			
N-M	0 / 1271	-28.0	-28.0 0.27 (2)	10.00			
M-L	0 / 0	-28.0	-28.0 0.13 (2)	10.00			



DWG NO. FAM 19668-17
STRUCTURAL COMPONENT ONLY



DRY- SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-TO		
V-C	-222/95	0.25 (1)
C-U	0/1724	0.39 (1)
U-D	-1138/0	0.49 (1)
D-S	0/767	0.17 (3)
S-E	-1252/0	0.54 (3)
R-Q	0/125	0.07 (1)
Q-G	-457/0	0.11 (3)
G-H	0/985	0.22 (3)
H-I	-1164/0	0.65 (1)
I-O	0/1393	0.43 (1)
O-J	0/930	0.09 (3)
J-N	0/1024	0.23 (1)
N-Q	-1753/0	0.60 (3)
Q-B	0/1337	0.30 (3)
B-V	0/1334	0.30 (3)
V-K	0/2993	0.35 (3)
S-Q	0/1161	0.26 (3)
E-Q		

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.57 (I) (INPUT = 1.00)



DWG NO. TAM 19669-17
STRUCTURAL
COMPONENT ONLY

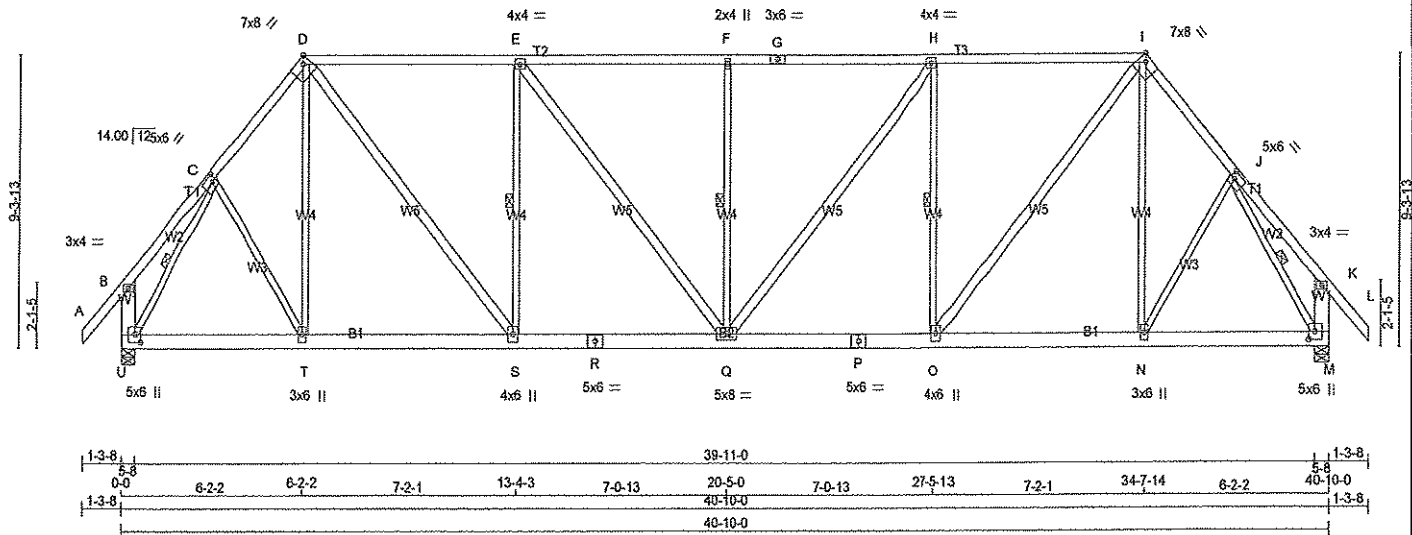
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T4	9	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: jDnzuigMGNFVCVaO_fHsjDzMo73-ORMaqrRzFNLI_6W7JUKezJbvkiGfhwonG74INMzMUNG

1-3-8 0-0 3-2-5 3-2-5 2-11-13 6-2-2 7-2-1 13-4-3 7-0-13 20-5-0 7-0-13 27-5-13 7-2-1 34-7-14 2-11-13 37-7-11 3-2-5 40-10-0 1-3-8
Scale = 1:72.0



TOTAL WEIGHT = 9 X 242 = 2180 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-h	MT20	5.0	6.0	2.50	1.75
D	TTVW-h	MT20	7.0	8.0	Edge	2.75
E	TMVW-t	MT20	4.0	4.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TTVW-h	MT20	7.0	8.0	Edge	2.75
J	TMVW-t	MT20	5.0	6.0	2.50	1.75
K	TMV-p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0	3.00	2.25
N	BMVW+h	MT20	3.0	6.0		
O	BMVW+h	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW+h	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMVW+h	MT20	4.0	6.0		
T	BMVW+h	MT20	3.0	6.0		
U	BMVW1+p	MT20	5.0	6.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
U	2260	0	2260	0	0	5-8	5-8	5-8	5-8
M	2260	0	2260	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
U	1862	1016 / 0	429 / 0
M	1862	1016 / 0	429 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-Q, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	C-T	0 / 237
B-C	0 / 18	D-S	0 / 191
C-D	-1996 / 0	E-Q	0 / 1499
D-E	-2220 / 0	F-R	-862 / 0
E-F	-2482 / 0	G-H	0 / 439
F-G	-2482 / 0	H-I	-504 / 0
G-H	-2482 / 0	I-J	0 / 439
H-I	-2220 / 0	J-K	-862 / 0
I-J	-1996 / 0	K-L	0 / 1499
J-K	0 / 18	L-M	0 / 191
K-L	0 / 41	M-N	0 / 237
L-M	-203 / 0	N-O	-2300 / 0
M-N	-203 / 0	O-P	0 / 439
		P-Q	-862 / 0
		Q-R	0 / 439
		R-S	-862 / 0
		S-T	0 / 439
		T-U	-862 / 0
		U-V	0 / 439
		V-W	-862 / 0
		W-X	0 / 439
		X-Y	-862 / 0
		Y-Z	0 / 439
		Z-A	-862 / 0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.36")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.36")
CALCULATED VERT. DEFL. (TL) = L/999 (0.23")

CSI: TC=0.90 (E-F:1), BC=0.34 (Q-S:1), WB=0.57 (J-M:1), SSI=0.26 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

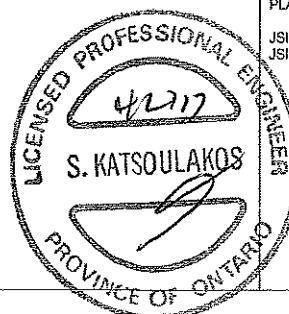
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.55 (J) (INPUT = 1.00)



DWG NO. TAM 19670-17
STRUCTURAL
COMPONENT ONLY

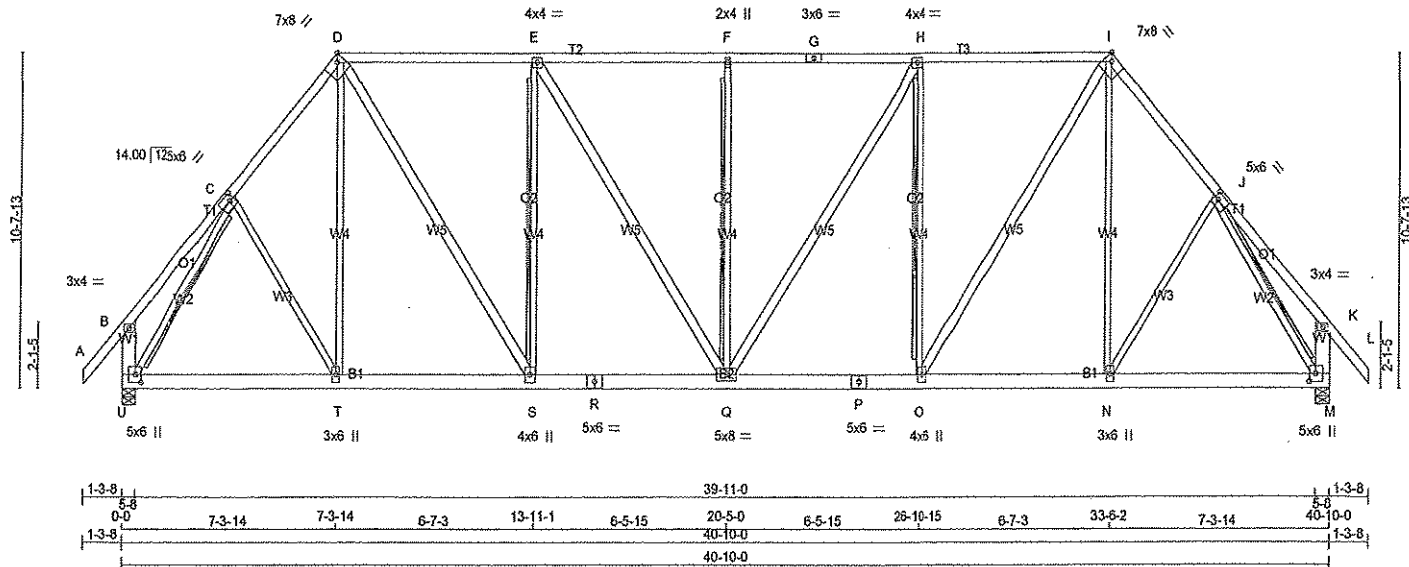
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T5	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: jDnzujgMGNFCVaO fHsDzMo73-0RMaqwRzFNLI 6W7JUkEzJbz8iHxsbng74INMzMUNG

-1-3-8 0-0 3-3-3 3-9-3 3-6-11 7-3-14 6-7-3 13-11-1 6-5-15 20-5-0 6-5-15 26-10-15 6-7-3 33-5-2 3-6-11 37-0-13 3-9-3 40-19-8 42-1-8
Scale = 1:72.0



TOTAL WEIGHT = 2 X 255 = 511 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV-p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 1.75
D	TTWW-h	MT20	7.0	8.0	Edge 2.75
E	TMWW-t	MT20	4.0	4.0	
F	TMWW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMWW-t	MT20	4.0	4.0	
I	TTWW-h	MT20	7.0	8.0	Edge 2.75
J	TMWW-t	MT20	5.0	6.0	2.50 1.75
K	TMV-p	MT20	3.0	4.0	
M	BMVW-t+p	MT20	5.0	6.0	3.00 2.25
N	BMVW-h	MT20	3.0	6.0	
O	BMVW-h	MT20	4.0	6.0	
P	BS-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	8.0	
R	BS-t	MT20	5.0	6.0	
S	BMVW-t	MT20	4.0	6.0	
T	BMVW-h	MT20	3.0	6.0	
U	BMVW-t+p	MT20	5.0	6.0	3.00 2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
U	2260	0	2260	0	0	5-8	5-8	5-8	5-8
M	2260	0	2260	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
U	1862	1018 / 0	429 / 0	0 / 0	0 / 0
M	1862	1018 / 0	429 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, F-Q, H-O, C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO		FR-TO	
A-B	0 / 41	FR-TO	0 / 178	FR-TO	0 / 178
B-C	0 / 23	FR-TO	0 / 251	FR-TO	0 / 251
C-D	-1992 / 0	FR-TO	0 / 1269	FR-TO	0 / 1269
D-E	-1964 / 0	FR-TO	-884 / 0	FR-TO	-884 / 0
E-F	-2163 / 0	FR-TO	0 / 373	FR-TO	0 / 373
F-G	-2163 / 0	FR-TO	-462 / 0	FR-TO	-462 / 0
G-H	-2163 / 0	FR-TO	0 / 373	FR-TO	0 / 373
H-I	-1564 / 0	FR-TO	-884 / 0	FR-TO	-884 / 0
I-J	-1992 / 0	FR-TO	0 / 1269	FR-TO	0 / 1269
J-K	0 / 23	FR-TO	0 / 251	FR-TO	0 / 251
K-L	0 / 41	FR-TO	0 / 178	FR-TO	0 / 178
U-B	-217 / 0	FR-TO	-2296 / 0	FR-TO	-2296 / 0
M-K	-217 / 0	FR-TO	-2296 / 0	FR-TO	-2296 / 0
U-T	0 / 1203	FR-TO	-28.0	FR-TO	-28.0
T-S	0 / 1276	FR-TO	-28.0	FR-TO	-28.0
S-R	0 / 1964	FR-TO	-28.0	FR-TO	-28.0
R-Q	0 / 1964	FR-TO	-28.0	FR-TO	-28.0
Q-P	0 / 1964	FR-TO	-28.0	FR-TO	-28.0
P-O	0 / 1964	FR-TO	-28.0	FR-TO	-28.0
O-N	0 / 1276	FR-TO	-28.0	FR-TO	-28.0
N-M	0 / 1203	FR-TO	-28.0	FR-TO	-28.0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.13")
ALLOWABLE DEFL.(TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.68 (E-F:1), BC=0.29 (Q-S:1), WB=0.78 (H-O:1), SS=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (M) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



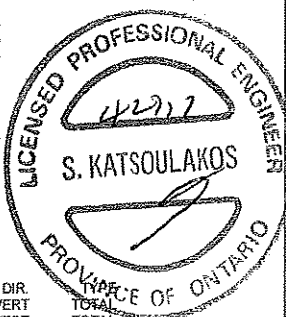
DRWG NO. TAM 19672-17
STRUCTURAL
COMPONENT ONLY

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 477.5 lbs FACTORED DOWN AT 5-10, 212 1/3 lbs FACTORED DOWN AT 7-11, 212 1/3 lbs FACTORED DOWN AT 9-5, AND 212 1/3 lbs FACTORED DOWN AT 10-12, AND 477.5 lbs FACTORED DOWN AT 12-11.8 ON TOP CHORD, AND 99.1 lbs FACTORED DOWN AT 1-11, 4, 3-11, 4, AT 5-11, 4, AT 7-11, 4, AT 9-5, 4, AT 10-12, 4, AT 12-10, 12, AND AT 14-10, 12, AND 99.1 lbs FACTORED DOWN AT 16-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/29	-77.4	-77.4 0.11 (1)	10.00	R-C	-2025/0	0.37 (1)		
B-C	-1909/0	-77.4	-77.4 0.12 (1)	4.74	C-Q	0/2022	0.50 (1)		
C-D	-4162/0	-77.4	-77.4 0.34 (1)	3.15	Q-D	0/970	0.24 (1)		
D-E	-3509/0	-77.4	-77.4 0.24 (1)	3.53	D-P	-858/0	0.16 (1)		
E-T	-2867/0	-77.4	-77.4 0.48 (1)	3.50	P-E	0/1237	0.31 (1)		
T-F	-2867/0	-77.4	-77.4 0.48 (1)	3.50	C-G	0/1237	0.31 (1)		
F-U	-2867/0	-77.4	-77.4 0.48 (1)	3.50	O-H	-858/0	0.16 (1)		
U-G	-2867/0	-77.4	-77.4 0.48 (1)	3.50	N-H	0/970	0.24 (1)		
G-H	-3509/0	-77.4	-77.4 0.24 (1)	3.53	N-I	0/2022	0.50 (1)		
H-I	-4162/0	-77.4	-77.4 0.34 (1)	3.15	M-I	-2025/0	0.37 (1)		
I-J	-1909/0	-77.4	-77.4 0.12 (1)	4.74	B-R	0/1719	0.43 (1)		
J-K	0/29	-77.4	-77.4 0.11 (1)	10.00	M-J	0/1719	0.43 (1)		
S-B	-1905/0	0.0	0.0 0.21 (1)	6.04	P-F	-632/0	0.23 (1)		
L-J	-1905/0	0.0	0.0 0.21 (1)	6.04	F-O	-632/0	0.23 (1)		
									
S-V	0/0	-28.0	-28.0 0.05 (2)	10.00					
V-R	0/0	-28.0	-28.0 0.05 (2)	10.00					
R-W	0/2112	-28.0	-28.0 0.39 (1)	10.00					
W-Q	0/2112	-28.0	-28.0 0.39 (1)	10.00					
Q-P	0/3400	-28.0	-28.0 0.73 (1)	10.00					
P-X	0/3441	-28.0	-28.0 0.73 (1)	10.00					
X-Y	0/3441	-28.0	-28.0 0.73 (1)	10.00					
Y-Z	0/3441	-28.0	-28.0 0.73 (1)	10.00					
Z-O	0/3441	-28.0	-28.0 0.73 (1)	10.00					
O-N	0/3400	-28.0	-28.0 0.73 (1)	10.00					
N-AA	0/2112	-28.0	-28.0 0.39 (1)	10.00					
AA-M	0/2112	-28.0	-28.0 0.39 (1)	10.00					
M-AB	0/0	-28.0	-28.0 0.05 (2)	10.00					
AB-I	0/0	-28.0	-28.0 0.05 (2)	10.00					

FACTORED CONCENTRATED LOADS (LBS)					FACE	DIR.	TYPE
JT	LOC.	LC1	MAX.	MAX+	FRONT	VERT	FACE OF ONTA
E	5-10-8	-478	-478	---	FRONT	VERT	TOTAL
F	9-5-0	-212	-212	---	FRONT	VERT	TOTAL
G	12-11-8	-478	-478	---	FRONT	VERT	TOTAL
O	12-10-12	---	---	---	FRONT	VERT	TOTAL
P	5-11-4	---	---	---	FRONT	VERT	TOTAL
T	7-11-4	-212	-212	---	FRONT	VERT	TOTAL
U	10-10-12	-212	-212	---	FRONT	VERT	TOTAL

ISI GRIP= 0.89 (C) (INPUT = 0.90)
ISI METAL= 0.59 (C) (INPUT = 1.00)



DWG NO. TAM 19673-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T6S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 8 Oct 5 2016 MiTek Industries, Inc. Thu Apr 27 15:51:10 2017 Page 2

ID:jjDnzujqMGNFCVaO fHsjDzMo73-Udwy2GRb0hTcbG5JsCGTVX8B36WHcOAwUfpQvozMUNF

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	1-11-4	-57	-99	---	FRONT	VERT	TOTAL
W	3-11-4	---	---	---	FRONT	VERT	TOTAL
X	7-11-4	---	---	---	FRONT	VERT	TOTAL
Y	9-5-0	---	---	---	FRONT	VERT	TOTAL
Z	10-10-12	---	---	---	FRONT	VERT	TOTAL
AA	14-10-12	---	---	---	FRONT	VERT	TOTAL
AB	16-10-12	-57	-99	---	FRONT	VERT	TOTAL



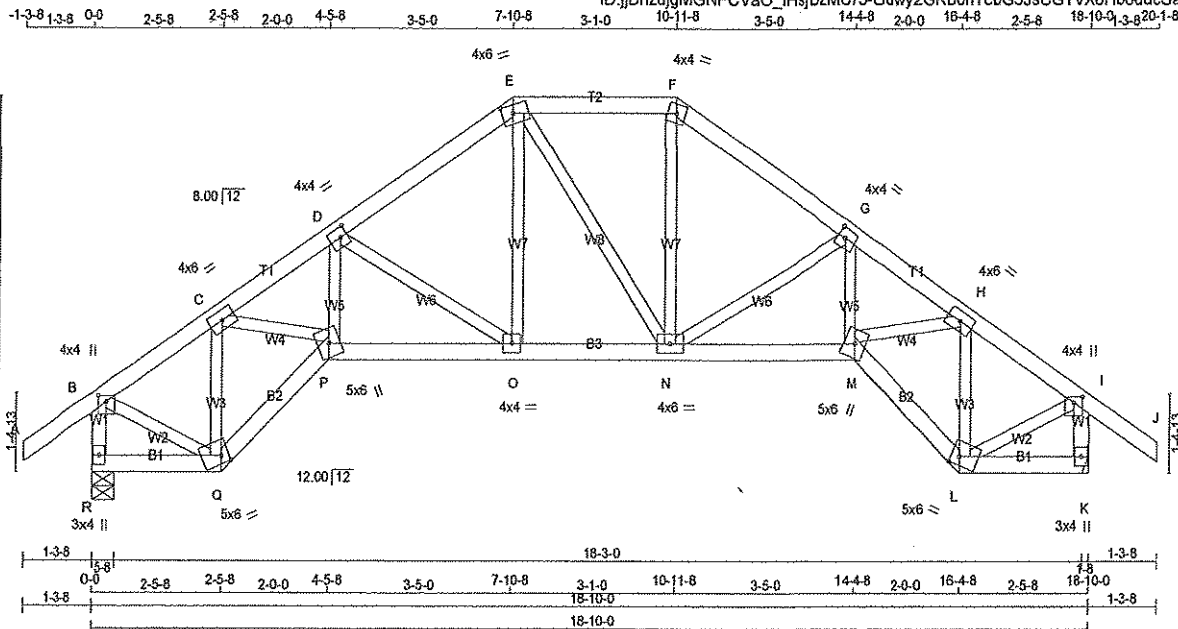
DWG NO. TAM 19673-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T7S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:jjDnzuJgMGNFCVaO_fHsjDzMo73-Udwy2GRb0hTcbG5JsCGTVX8Hb6ducSawUfpQvozMUNF



Scale = 1:40.2

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS			
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-t	MT20	4.0	6.0		
I	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BBWW-m	MT20	5.0	6.0	1.75	1.75
M	BBWW+m	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BBWW+m	MT20	5.0	6.0		
Q	BBWW-m	MT20	5.0	6.0	1.75	1.75
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	1098	0	1098	0	0	5-8	5-8		
K	1098	0	1098	0	0				

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	897	502 / 0	198 / 0	0 / 0	0 / 0	197 / 0	0 / 0
K	897	502 / 0	198 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. LC1 (LC)	MAX. UNBRAC	
FR-TO					FR-TO				
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	Q-C	-1035 / 0	0.18 (1)		
B-C	-955 / 0	-77.4	-77.4 0.07 (1)	6.25	C-P	0 / 972	0.22 (1)		
C-D	-2021 / 0	-77.4	-77.4 0.09 (1)	4.70	P-D	0 / 587	0.13 (1)		
D-E	-1263 / 0	-77.4	-77.4 0.12 (1)	5.81	D-O	-729 / 0	0.20 (1)		
E-F	-1055 / 0	-77.4	-77.4 0.11 (1)	6.02	O-E	0 / 484	0.10 (1)		
F-G	-1265 / 0	-77.4	-77.4 0.12 (1)	5.81	E-N	0 / 3	0.00 (1)		
G-H	-2021 / 0	-77.4	-77.4 0.09 (1)	4.70	N-F	0 / 488	0.11 (1)		
H-I	-955 / 0	-77.4	-77.4 0.07 (1)	6.25	F-G	-727 / 0	0.20 (1)		
I-J	0 / 29	-77.4	-77.4 0.10 (1)	10.00	M-G	0 / 585	0.13 (1)		
R-B	-1064 / 0	0.0	0.0 0.11 (1)	7.64	M-H	0 / 972	0.22 (1)		
K-I	-1064 / 0	0.0	0.0 0.11 (1)	7.64	L-H	-1035 / 0	0.18 (1)		
					B-Q	0 / 867	0.20 (1)		
					L-I	0 / 867	0.20 (1)		
R-Q	0 / 0	-28.0	-28.0 0.05 (2)	10.00					
Q-P	0 / 1069	-28.0	-28.0 0.19 (1)	10.00					
P-O	0 / 1661	-28.0	-28.0 0.31 (1)	10.00					
O-N	0 / 1053	-28.0	-28.0 0.21 (1)	10.00					
N-M	0 / 1661	-28.0	-28.0 0.31 (1)	10.00					
M-L	0 / 1069	-28.0	-28.0 0.19 (1)	10.00					
L-K	0 / 0	-28.0	-28.0 0.05 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

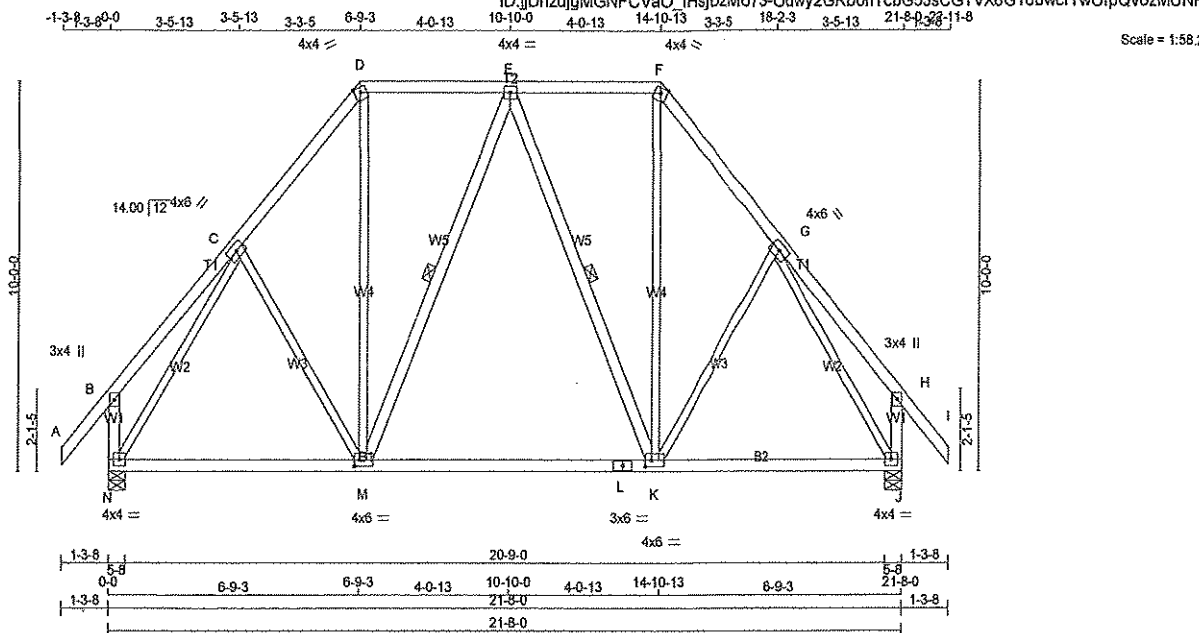
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPEC

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	T8	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Apr 27 15:51:10 2017 Page 1

ID:jjDnzujgMGNFCVaO rHsjDzMo73-Udwy2GRb0hTcbG5JsCGTVX8G16bwclTwUfpQvozMUNF



Scale = 1:58.2

TOTAL WEIGHT = 5 X 126 = 630 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - I	2x4	DRY	No.2	SPF		
N - B	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
N - L	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		

ALL WEBS	EXCEPT	SIZE	DRY	LUMBER	DESCR
M - E	2x4	DRY	No.2	SPF	
E - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWV-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMWV-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K	BMVWV-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMVWV-t	MT20	4.0	6.0	2.00	1.50
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
N	1250	0	6-8	5-8
J	1250	0	6-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
N	1022	570 / 0 228 / 0 0 / 0 0 / 0 225 / 0 0 / 0
J	1022	570 / 0 228 / 0 0 / 0 0 / 0 225 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 41	-77.4	-77.4	0.11 (1)	10.00	C-M	-17 / 78	0.02 (3)	
B-C	0 / 22	-77.4	-77.4	0.13 (1)	10.00	M-D	0 / 432	0.10 (1)	
C-D	-892 / 0	-77.4	-77.4	0.11 (1)	6.25	M-E	-204 / 0	0.12 (1)	
D-E	-567 / 0	-77.4	-77.4	0.16 (1)	6.25	E-K	-204 / 0	0.12 (1)	
E-F	-567 / 0	-77.4	-77.4	0.16 (1)	6.25	K-F	0 / 432	0.10 (1)	
F-G	-892 / 0	-77.4	-77.4	0.11 (1)	6.25	K-G	-17 / 78	0.02 (3)	
G-H	0 / 22	-77.4	-77.4	0.13 (1)	10.00	N-C	-1124 / 0	0.87 (1)	
H-I	0 / 41	-77.4	-77.4	0.11 (1)	10.00	G-J	-1124 / 0	0.87 (1)	
N-B	-208 / 0	0.0	0.0	0.02 (1)	7.81				
J-H	-208 / 0	0.0	0.0	0.02 (1)	7.81				
N-M	0 / 572	-28.0	-28.0	0.43 (2)	10.00				
M-L	0 / 645	-28.0	-28.0	0.44 (2)	10.00				
L-K	0 / 645	-28.0	-28.0	0.44 (2)	10.00				
K-J	0 / 572	-28.0	-28.0	0.43 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.16 (D-E:1), BC=0.44 (K-M:2), WB=0.87 (C-N:1), SSI=0.16 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

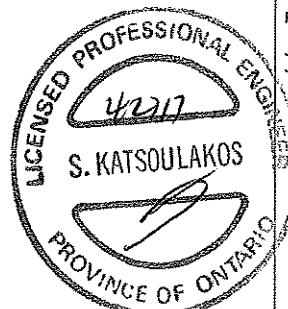
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1668

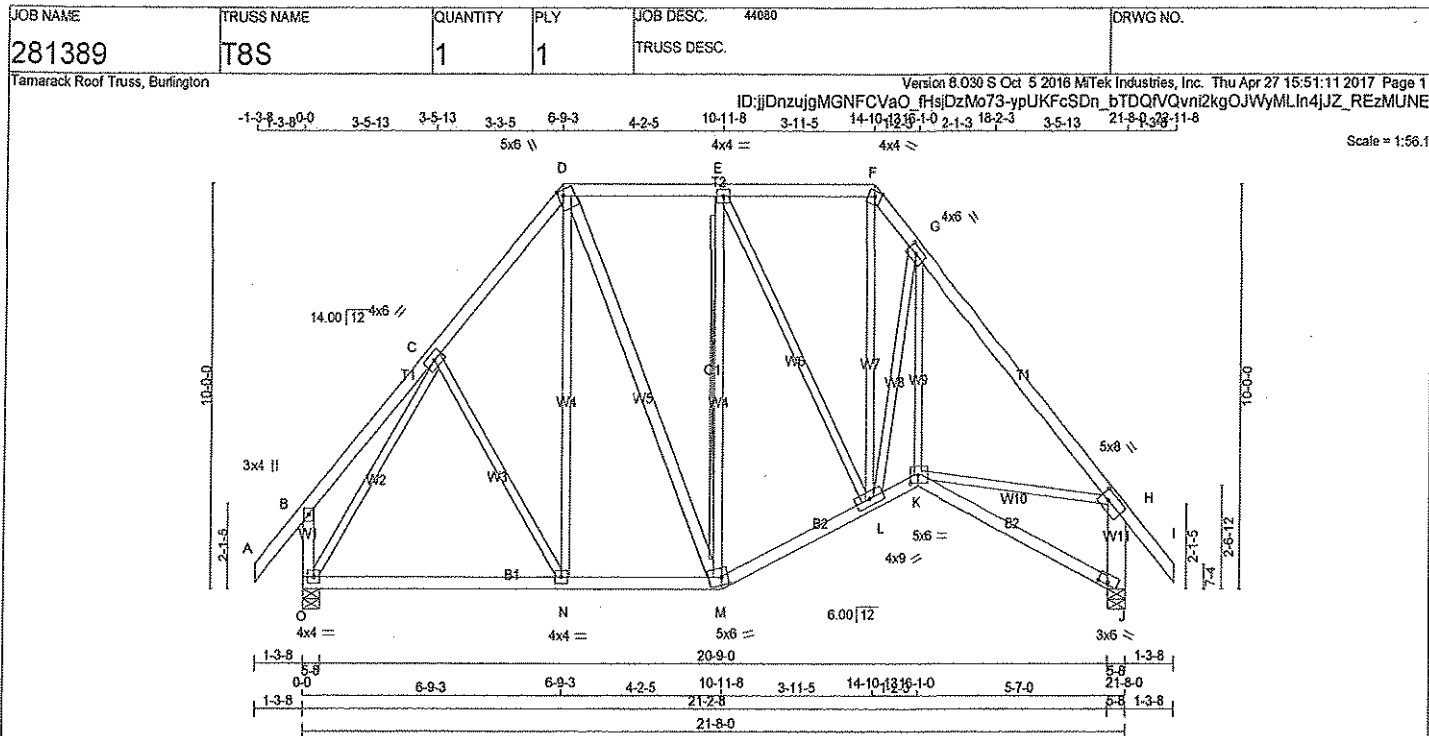
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.98 (L) (INPUT = 1.00)



DRWG NO. TAM 19675-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x6	DRY	No.2
O - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-l	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	1.75	1.50
E	TMWW-l	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMWW-l	MT20	4.0	6.0	2.00	2.75
H	TMWW-l	MT20	5.0	8.0	2.50	2.50
J	BVM-l	MT20	3.0	6.0	0.75	3.00
K	BBWW-p	MT20	5.0	6.0	2.50	2.50
L	BMWW-l	MT20	4.0	9.0		
M	BBWW-m	MT20	5.0	6.0	2.25	2.00
N	BMWW-l	MT20	4.0	4.0		
O	BMWW-l	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1250	0	1250	0	0	5-8	5-8
O	1250	0	1250	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
J	1022	570 / 0	228 / 0	0 / 0	0 / 0	225 / 0	0 / 0
O	1022	570 / 0	228 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER
90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 41	-77.4	-77.4 0.11 (1)	10.00	C-N	-19 / 77	0.02 (3)
B-C	0 / 22	-77.4	-77.4 0.14 (1)	10.00	N-D	0 / 303	0.07 (2)
C-D	-887 / 0	-77.4	-77.4 0.11 (1)	6.25	D-M	0 / 231	0.04 (1)
D-E	-654 / 0	-77.4	-77.4 0.16 (1)	6.25	M-E	-471 / 0	0.37 (1)
E-F	-698 / 0	-77.4	-77.4 0.16 (1)	6.25	E-L	0 / 108	0.02 (1)
F-G	-1246 / 0	-77.4	-77.4 0.25 (1)	5.47	L-F	0 / 882	0.20 (1)
G-H	-1301 / 0	-77.4	-77.4 0.38 (1)	5.27	O-C	-1120 / 0	0.86 (1)
H-I	0 / 41	-77.4	-77.4 0.11 (1)	10.00	K-G	0 / 605	0.14 (1)
O-B	-208 / 0	0.0	0.0 0.02 (1)	7.81	K-H	0 / 876	0.20 (1)
J-H	-1172 / 0	0.0	0.0 0.10 (1)	7.81	L-G	-958 / 0	0.76 (1)
O-N	0 / 570	-28.0	-28.0 0.35 (2)	10.00			
N-M	0 / 582	-28.0	-28.0 0.36 (2)	10.00			
M-L	0 / 734	-28.0	-28.0 0.20 (2)	10.00			
L-K	0 / 928	-28.0	-28.0 0.23 (1)	10.00			
K-J	0 / 0	-28.0	-28.0 0.26 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3 PSF
DL =	3.0 PSF	
BOT CH.	LL =	10.5 PSF
DL =	7.0 PSF	
TOTAL LOAD =	43.8 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.38 (G-H:1), BC=0.36 (M-N:2),
WB=0.66 (C-O:1), SS=0.16 (N-O:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

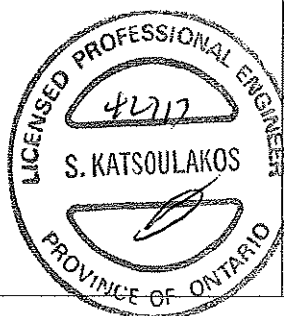
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 1.00)

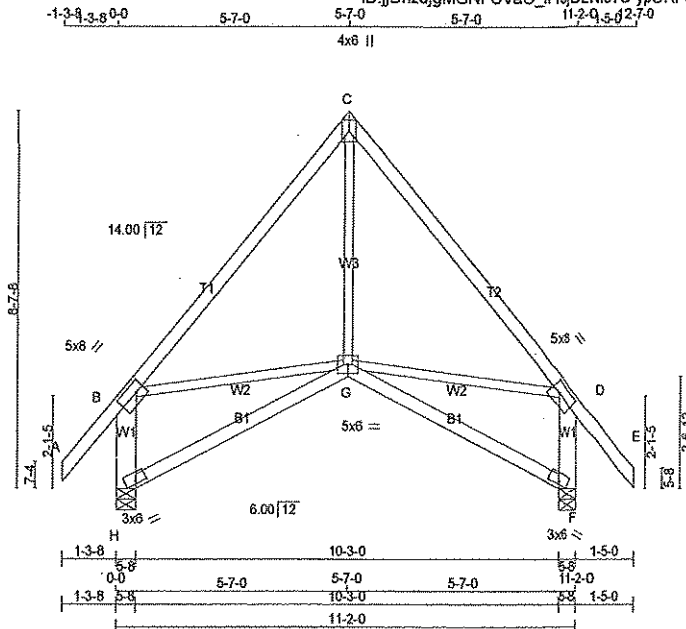


DRWG NO. TAM 19676-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281389	TRUSS NAME T9S	QUANTITY 2	PLY 1	JOB DESC. 44680	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:jjDnzujgMGNFCVaO_fHsjDzMo73-ypUKFcSDn_bTDQVQvni2kgPLWzuLx24jJZ_REzMUNE



Scale = 1:51.9

TOTAL WEIGHT = 2 X 59 = 118 lb
(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x6	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0	2.50	2.50
C	TTW+p	MT20	4.0	6.0		
D	TMVW-I	MT20	5.0	8.0	2.50	2.50
F	BVM-I	MT20	3.0	6.0	0.75	3.00
G	BBWW+p	MT20	5.0	6.0		
H	BVM-I	MT20	3.0	6.0	0.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	697	0	697	0	5-8	5-8
F	706	0	706	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE		WIND	DEAD	SOIL
		SNOW	LIVE			
H	563	325 / 0	117 / 0	0 / 0	120 / 0	0 / 0
F	569	331 / 0	117 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 41	-77.4	-77.4	0.11 (1)	10.00	G-C	0 / 340	0.08 (2)	
B-C	-445 / 0	-77.4	-77.4	0.32 (1)	6.25	B-G	0 / 293	0.07 (1)	
C-D	-445 / 0	-77.4	-77.4	0.32 (1)	6.25	G-D	0 / 293	0.07 (1)	
D-E	0 / 45	-77.4	-77.4	0.13 (1)	10.00				
H-B	-819 / 0	0.0	0.0	0.05 (1)	7.81				
F-D	-628 / 0	0.0	0.0	0.06 (1)	7.81				
H-G	0 / 0	-28.0	-28.0	0.26 (3)	10.00				
G-F	0 / 0	-28.0	-28.0	0.26 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CSI: TC=0.32 (B-C-1), BC=0.26 (F-G-3), WB=0.08 (C-G-2), SSI=0.11 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

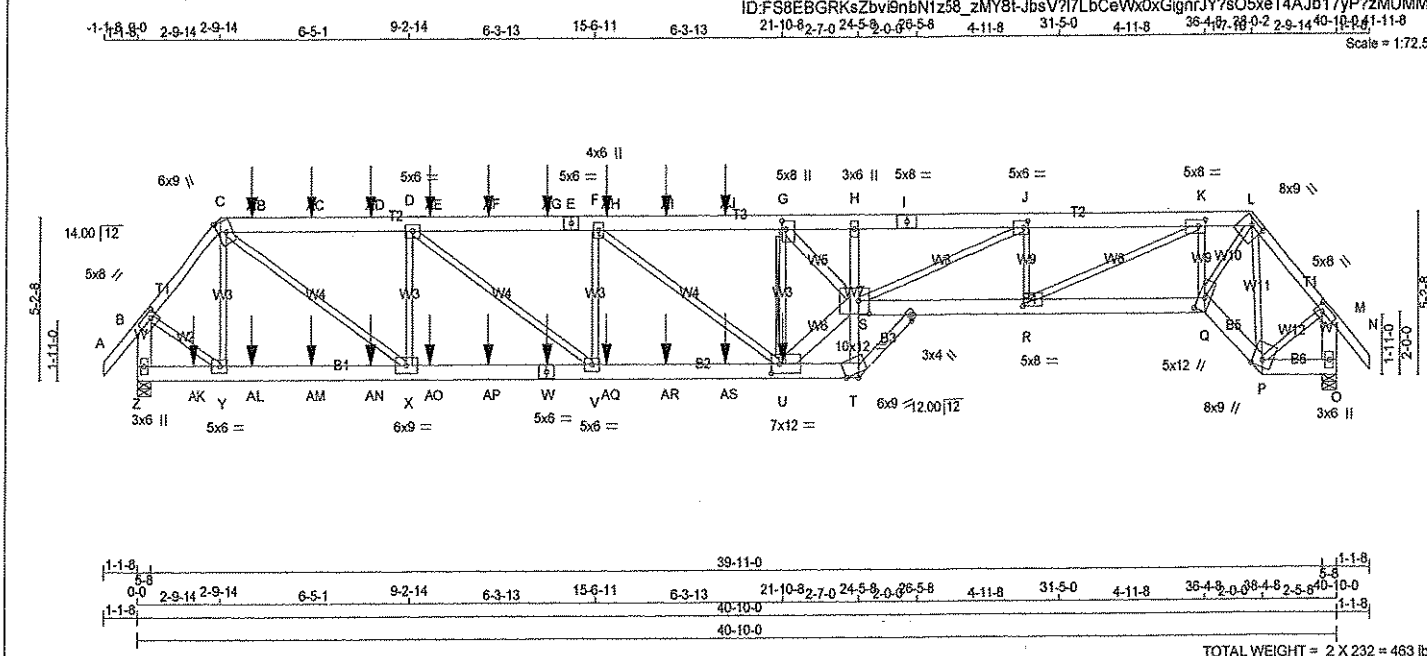
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (G) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



DWG NO. TAM 19672-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x6	DRY	No.2 SPF
E - I	2x6	DRY	No.2 SPF
I - L	2x6	DRY	No.2 SPF
L - N	2x4	DRY	No.2 SPF
Z - B	2x6	DRY	No.2 SPF
O - M	2x6	DRY	No.2 SPF
Z - W	2x6	DRY	No.2 SPF
W - T	2x6	DRY	No.2 SPF
S - Q	2x6	DRY	No.2 SPF
Q - P	2x6	DRY	No.2 SPF
P - O	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT			
T - H	2x4	DRY	No.2 SPF
U - S	2x6	DRY	No.2 SPF
G - S	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #/ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
L - N	12	TOP
C - E	12	SIDE(0.0)
E - I	12	SIDE(0.0)
I - L	12	TOP
Z - B	12	TOP
O - M	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
Z - W	12	SIDE(0.0)
W - T	12	SIDE(183.1)
S - Q	12	TOP
Q - P	12	TOP
P - O	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
G - U	6	SIDE(403.3)
2x3	1	6
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	5.0	8.0 2.25 2.50
C	TTWWH-m	MT20	6.0	9.0 Edge 3.25
D	TMVW-1	MT20	5.0	8.0
E	TS-1	MT20	5.0	6.0
F	TMVW-1	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	HORZ	UPLIFT
Z	4184	0	0
O	3809	0	0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD.

UNFACTORED REACTIONS						
1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED	3482	1845/0	840/0	0/0	798/0	0/0
Z	3142	1709/0	727/0	0/0	705/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF NO.2 T-BRACE AT G-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0/36	Y-C	-882/0
B-C	-3889/0	T-S	-24/0
C-AB	-6773/0	S-H	0/124
AB-AC	-6773/0	R-J	-2786/0
AC-AD	-6773/0	R-K	0/6824
AD-D	-6773/0	Q-K	-3218/0
D-AE	-9560/0	Q-L	0/6978
AE-AF	-9560/0	P-L	-3443/0
AF-AG	-9560/0	B-Y	0/2685
AG-E	-9560/0	G-Y	-7036/0
E-F	-9560/0	F-U	0/1669
F-AH	-10894/0	C-X	0/5462
AH-AI	-10894/0	V-F	-1812/0
AI-AJ	-10894/0	X-D	-2962/0
AJ-G	-10894/0	D-V	0/3488
G-H	-16733/0	P-M	0/2365
H-I	-16838/0	U-S	0/13724
I-J	-16838/0	G-S	0/8639
J-K	-12092/0	S-J	0/5233
K-L	-5772/0		
L-M	-3148/0		
M-N	0/36		
Z-B	-4206/0		
O-M	-3775/0		
Z-AK	0/0		
AK-Y	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/798 (0.61")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/494 (0.99")

CSI: TC=0.82 (H-J:1), BC=0.88 (R-S:1), WB=0.89 (S-U:1), SSI=0.15 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (G) (INPUT = 0.90)
JSI METAL = 0.95 (L) (INPUT = 1.00)



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TMWW+I	MT20	5.0	8.0	3.25	1.50
H	TMW+W	MT20	3.0	6.0		
I	TS-I	MT20	5.0	8.0		
J	TMWW-I	MT20	5.0	6.0	2.50	1.75
K	TMWW-I	MT20	5.0	8.0	2.50	2.75
L	TTWW-h	MT20	8.0	9.0	2.00	4.50
M	TMWW-I	MT20	5.0	8.0	2.50	2.50
O	BMV1+p	MT20	3.0	6.0		
P	BBWW+m	MT20	8.0	9.0	Edge	
Q	BBWW+m	MT20	5.0	12.0	5.00	2.50
R	BMWW-I	MT20	5.0	8.0	2.50	2.75
S	BBWWWW-I	MT20	10.0	12.0	5.00	4.25
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BBWWWW-I	MT20	7.0	12.0	3.50	3.25
V	BBWW-I	MT20	5.0	6.0		
W	BS-I	MT20	5.0	6.0		
X	BBWW-I	MT20	6.0	9.0		
Y	BBWW-I	MT20	5.0	6.0		
Z	BMV1+p	MT20	3.0	6.0		
AA	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 93.1 lbs FACTORED DOWN AT 3-11-4, 93.1 lbs FACTORED DOWN AT 5-11-4, 93.1 lbs FACTORED DOWN AT 7-11-4, 93.1 lbs FACTORED DOWN AT 9-11-4, 93.1 lbs FACTORED DOWN AT 11-11-4, 93.1 lbs FACTORED DOWN AT 13-11-4, 93.1 lbs FACTORED DOWN AT 15-11-4, AND 93.1 lbs FACTORED DOWN AT 17-11-4, AND 93.1 lbs FACTORED DOWN AT 19-11-4 ON TOP CHORD, AND 70.9 lbs FACTORED DOWN AT 1-11-4, 70.9 lbs FACTORED DOWN AT 3-11-4, 70.9 lbs FACTORED DOWN AT 5-11-4, 70.9 lbs FACTORED DOWN AT 7-11-4, 70.9 lbs FACTORED DOWN AT 9-11-4, 70.9 lbs FACTORED DOWN AT 11-11-4, 70.9 lbs FACTORED DOWN AT 13-11-4, 70.9 lbs FACTORED DOWN AT 15-11-4, 70.9 lbs FACTORED DOWN AT 17-11-4, AND 70.9 lbs FACTORED DOWN AT 19-11-4, AND 2259.5 lbs FACTORED DOWN AT 21-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

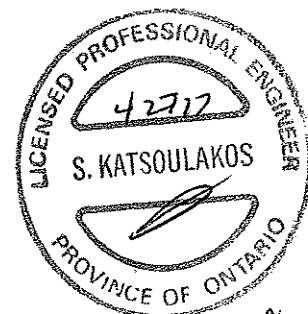
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
Y-AL	0 / 2382	-28.0	-28.0 0.24 (1)	10.00			
AL-AM	0 / 2382	-28.0	-28.0 0.24 (1)	10.00			
AM-AN	0 / 2382	-28.0	-28.0 0.24 (1)	10.00			
AN-X	0 / 2382	-28.0	-28.0 0.24 (1)	10.00			
X-AO	0 / 6773	-28.0	-28.0 0.51 (1)	10.00			
AO-AP	0 / 6773	-28.0	-28.0 0.51 (1)	10.00			
AP-W	0 / 6773	-28.0	-28.0 0.51 (1)	10.00			
W-V	0 / 6773	-28.0	-28.0 0.51 (1)	10.00			
V-AQ	0 / 9560	-28.0	-28.0 0.68 (1)	10.00			
AQ-AR	0 / 9560	-28.0	-28.0 0.68 (1)	10.00			
AR-AS	0 / 9560	-28.0	-28.0 0.68 (1)	10.00			
AS-U	0 / 9560	-28.0	-28.0 0.68 (1)	10.00			
U-T	0 / 141	-28.0	-28.0 0.09 (2)	10.00			
S-R	0 / 12092	-28.0	-28.0 0.86 (1)	10.00			
R-Q	0 / 5896	-28.0	-28.0 0.46 (1)	10.00			
Q-P	0 / 3273	-28.0	-28.0 0.22 (1)	10.00			
P-O	0 / 0	-28.0	-28.0 0.01 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	21-10-8	-2259	-2259	---	BACK	VERT	TOTAL
W	13-11-4	-41	-71	---	BACK	VERT	TOTAL
AB	3-11-4	-93	-93	---	BACK	VERT	TOTAL
AC	5-11-4	-93	-93	---	BACK	VERT	TOTAL
AD	7-11-4	-93	-93	---	BACK	VERT	TOTAL
AE	9-11-4	-93	-93	---	BACK	VERT	TOTAL
AF	11-11-4	-93	-93	---	BACK	VERT	TOTAL
AG	13-11-4	-93	-93	---	BACK	VERT	TOTAL
AH	15-11-4	-93	-93	---	BACK	VERT	TOTAL
AI	17-11-4	-93	-93	---	BACK	VERT	TOTAL
AJ	19-11-4	-93	-93	---	BACK	VERT	TOTAL
AK	1-11-4	-41	-71	---	BACK	VERT	TOTAL
AL	3-11-4	-41	-71	---	BACK	VERT	TOTAL
AM	5-11-4	-41	-71	---	BACK	VERT	TOTAL
AN	7-11-4	-41	-71	---	BACK	VERT	TOTAL
AO	9-11-4	-41	-71	---	BACK	VERT	TOTAL
AP	11-11-4	-41	-71	---	BACK	VERT	TOTAL
AQ	13-11-4	-41	-71	---	BACK	VERT	TOTAL
AR	15-11-4	-41	-71	---	BACK	VERT	TOTAL
AS	17-11-4	-41	-71	---	BACK	VERT	TOTAL



DWG NO. TAM 19684-17
STRUCTURAL
COMPONENT ONLY

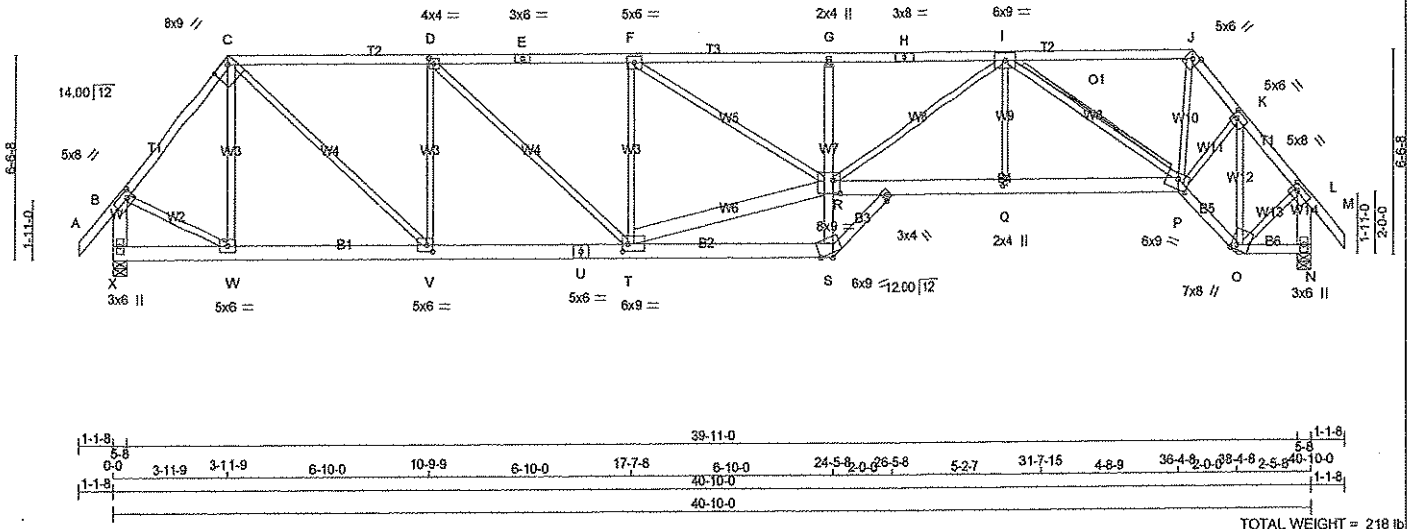
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T21S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbv9nbN1z58_zMY8t-nnQID58zMMVnZAWSGN14sIX2wVMCvoTqhsVxRzMUMML

1-1-8 3-11-9 3-11-9 6-10-0 10-9-9 6-10-0 17-7-8 6-10-0 24-5-8 2-0-5 5-2-7 31-7-15 5-2-7 36-4-8 2-5-8 40-10-0 1-1-8
Scale = 1:72.5



TOTAL WEIGHT = 218 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E SPF
C - E	2x4	DRY	1650F 1.5E SPF
E - H	2x4	DRY	1650F 1.5E SPF
H - J	2x4	DRY	1650F 1.5E SPF
J - M	2x4	DRY	1650F 1.5E SPF
X - B	2x6	DRY	No.2 SPF
N - L	2x6	DRY	No.2 SPF
X - U	2x6	DRY	No.2 SPF
U - S	2x6	DRY	No.2 SPF
R - P	2x6	DRY	No.2 SPF
P - O	2x4	DRY	No.2 SPF
O - N	2x4	DRY	No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF			
EXCEPT			
S - G	2x4	DRY	No.2 SPF
T - R	2x6	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
JT	VERT	DOWN	UP	BRG	BRG
X	2246	0	2246	0	5-8
N	2246	0	2246	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
X	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
N	1852	1008 / 0	428 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 36	W-C	-274 / 54
B-C	-1978 / 0	C-V	0 / 2104
C-D	-2848 / 0	V-D	-1225 / 0
D-E	-3550 / 0	D-T	0 / 954
E-F	-3550 / 0	T-F	-1474 / 0
F-G	-5105 / 0	S-R	0 / 134
G-H	-5123 / 0	R-G	-466 / 0
H-I	-5123 / 0	I-P	0 / 1251
I-J	-2040 / 0	Q-I	0 / 357
J-K	-2819 / 0	K-P	-2504 / 0
K-L	-1744 / 0	P-J	0 / 1894
L-M	0 / 36	M-K	0 / 1055
X-B	-2217 / 0	O-K	-1750 / 0
N-L	-2212 / 0	B-W	0 / 1362
		O-L	0 / 1331
		T-R	0 / 3877
		F-R	0 / 1809
X-W	0 / 0		
W-V	0 / 1277		
V-U	0 / 2848		
U-T	0 / 2848		
T-S	0 / 42		
R-Q	0 / 4091		
Q-P	0 / 4091		
P-O	0 / 1558		
O-N	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 43.8 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(65% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.37")
ALLOWABLE DEFL.(TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/825$ (0.59")

CSI: TC=0.77 (F-G:1), BC=0.55 (Q-R:1), WB=1.00 (I-P:1), SSI=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

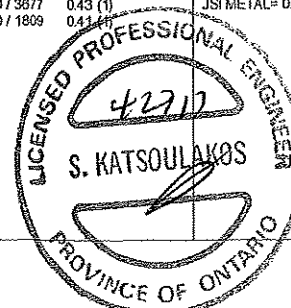
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (V) (INPUT = 0.90)
JSI METAL= 0.51 (I) (INPUT = 1.00)



DWG NO. TAM19685-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281390	T22S	1	1	TRUSS DESC.		

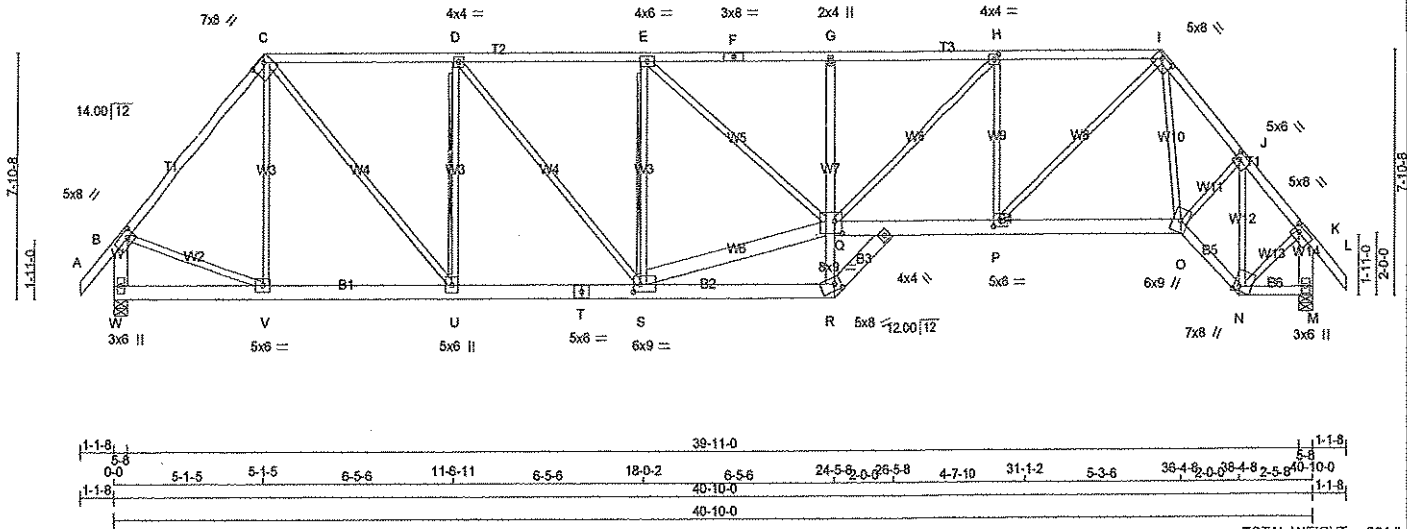
Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbvi9nbN1z58 zMY8t-nnQID58zMVmNZAWSGN14sX1SVOYCV7TqhsVxRzMUMU

-1-1-8.0 5-1-5 5-1-5 6-5-6 11-6-11 6-5-6 18-0-2 6-5-6 24-5-6 26-5-8 4-7-10 31-1-2 4-7-10 35-9-11 2-7-13 38-4-8 2-5-6 40-10-0 41-11-8

Scale = 1:72.5



TOTAL WEIGHT = 231 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
I - L	2x4 DRY	No.2	SPF		
W - B	2x6 DRY	No.2	SPF		
M - K	2x6 DRY	No.2	SPF		
W - T	2x6 DRY	No.2	SPF		
T - R	2x6 DRY	No.2	SPF		
Q - O	2x6 DRY	No.2	SPF		
O - N	2x4 DRY	No.2	SPF		
N - M	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		
R - G	2x4 DRY	No.2	SPF		
S - Q	2x6 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW-t	MT20	5.0	8.0	2.50	2.50	
C	TTWW-h	MT20	7.0	8.0	1.75	5.00	
D	TMVW-t	MT20	4.0	4.0			
E	TMVW-t	MT20	4.0	6.0			
F	TS-t	MT20	3.0	8.0			
G	TMVW-t	MT20	2.0	4.0			
H	TMVW-t	MT20	4.0	4.0	2.00	1.75	
I	TTWW-h	MT20	5.0	8.0	1.25	5.00	
J	TMVW-t	MT20	5.0	6.0	2.50	2.00	
K	TMVW-t	MT20	5.0	8.0	2.25	2.50	
M	BMV1+p	MT20	3.0	6.0			
N	BBWW+m	MT20	7.0	8.0	Edge	2.25	
O	BBWW+m	MT20	6.0	9.0			
P	BMVW-t	MT20	5.0	6.0	2.50	2.50	
Q	BBWWWWW-t	MT20	8.0	9.0	4.50	3.00	
R	BBW-m	MT20	5.0	8.0	2.50	2.50	
S	BBWWWW-t	MT20	6.0	9.0	2.75	2.75	
T	BS-t	MT20	5.0	6.0			
U	BMVWW-t	MT20	5.0	6.0			
V	BMVWW-t	MT20	5.0	6.0			
W	BMV1+p	MT20	3.0	6.0			
X	NP+h	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
W	2247	0	2247	0
M	2246	0	2246	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
W	1853	1008 / 0 429 / 0 0 / 0 0 / 0 416 / 0 0 / 0
M	1853	1008 / 0 429 / 0 0 / 0 0 / 0 416 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-U
2x4 DRY SPF No.2 T-BRACE AT E-S

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1 MAX	MAX.	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE	MAX
								(LBS)	CSI (LC)
FR-TO									
A-B	0 / 36	-77.4	-77.4	0.09 (1)	10.00	V-C	-178 / 120	0.20 (1)	
B-C	-2014 / 0	-77.4	-77.4	0.57 (1)	4.29	C-U	0 / 1723	0.39 (1)	
C-D	-2435 / 0	-77.4	-77.4	0.69 (1)	3.74	U-D	-1133 / 0	0.97 (1)	
D-E	-2928 / 0	-77.4	-77.4	0.77 (1)	3.39	D-S	0 / 764	0.17 (1)	
E-F	-3875 / 0	-77.4	-77.4	0.80 (1)	3.06	S-E	-1298 / 0	0.54 (1)	
F-G	-3875 / 0	-77.4	-77.4	0.80 (1)	3.06	R-Q	0 / 126	0.08 (1)	
G-H	-3882 / 0	-77.4	-77.4	0.73 (1)	3.05	Q-G	-454 / 0	0.10 (1)	
H-I	-3157 / 0	-77.4	-77.4	0.18 (1)	4.03	P-H	0 / 1002	0.23 (1)	
I-J	-2803 / 0	-77.4	-77.4	0.09 (1)	4.88	P-I	-1162 / 0	0.58 (1)	
J-K	-1746 / 0	-77.4	-77.4	0.09 (1)	10.00	I-O	0 / 480	0.11 (1)	
K-L	0 / 36	-77.4	-77.4	0.09 (1)	6.89	O-J	0 / 1028	0.23 (1)	
W-B	-2193 / 0	0.0	0.0	0.16 (1)	6.87	N-J	-1768 / 0	0.58 (1)	
M-K	-2212 / 0	0.0	0.0	0.17 (1)	10.00	B-V	0 / 1358	0.31 (1)	
W-V	0 / 0	-28.0	-28.0	0.11 (2)	10.00	N-K	0 / 1343	0.30 (1)	
V-U	0 / 1303	-28.0	-28.0	0.23 (2)	10.00	S-Q	0 / 3055	0.35 (1)	
U-T	0 / 2435	-28.0	-28.0	0.35 (1)	10.00	E-Q	0 / 1239	0.28 (1)	
T-S	0 / 2435	-28.0	-28.0	0.35 (1)	10.00				
S-R	0 / 24	-28.0	-28.0	0.13 (3)	10.00				
Q-P	0 / 3157	-28.0	-28.0	0.43 (1)	10.00				
P-O	0 / 1749	-28.0	-28.0	0.28 (2)	10.00				
O-N	0 / 1572	-28.0	-28.0	0.27 (1)	10.00				
N-M	0 / 0	-28.0	-28.0	0.05 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.42")

CSI: TC=0.80 (E/G:1), BC=0.43 (P/Q:1), WB=0.97 (D-U:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

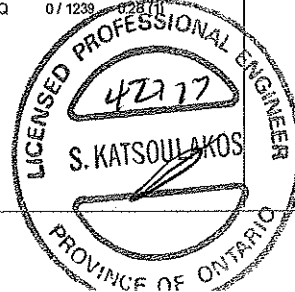
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PLI)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT= 0.80)
JSI METAL= 0.58 (I) (INPUT= 1.00)

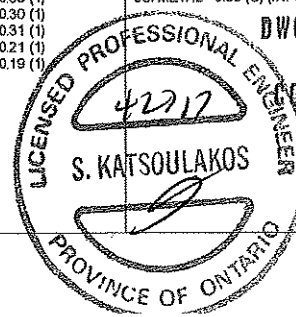


DRWG NO. TAM19606.17
STRUCTURAL
COMPONENT ONLY

2025 RELEASE UNDER E.O. 14176



JSI GRIP= 0.89 (Z) (INPUT = 0.80)
JSI META= 0.56 (C) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T24	1	1	TRUSS DESC.		

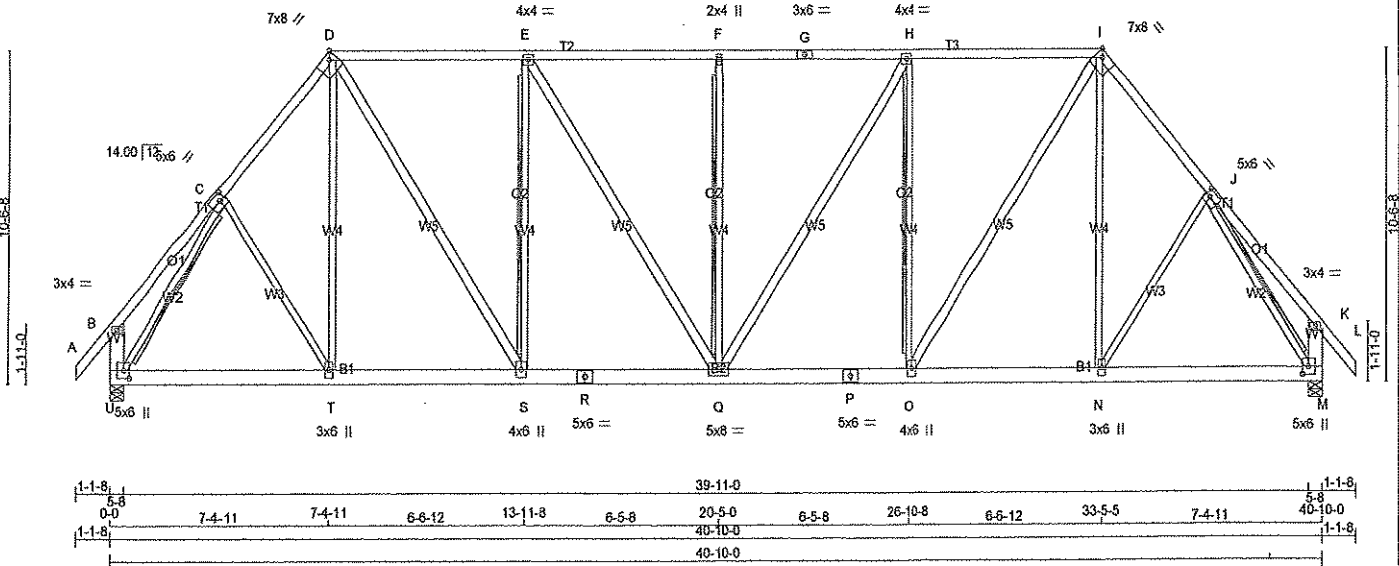
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ID:FS8EBGRKsZbvi9nbN1z58_zMY81-Fz_GQR9c7puEAK5fq5pJ0z4E5ubx0kc3Lc2UtzMUMK

-1-1-8 0 3-9-10 3-9-10 3-7-2 7-4-11 6-6-12 13-11-8 6-5-8 20-5-0 6-5-8 20-10-8 6-6-12 33-5-5 3-7-2 37-0-6 3-9-10 40-10-0 1-11-8

Scale = 1:7.17



TOTAL WEIGHT = 253 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - M	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
D - S	2x4	DRY	No.2	SPF	
E - Q	2x4	DRY	No.2	SPF	
Q - H	2x4	DRY	No.2	SPF	
O - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW-h	MT20	7.0	8.0	Edge	2.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV-p	MT20	3.0	4.0		
M	BMVW1+p	MT20	5.0	6.0	3.00	2.25
N	BMWWH	MT20	3.0	6.0		
O	BMWWH	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWH-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWH	MT20	4.0	6.0		
T	BMWWH	MT20	3.0	6.0		
U	BMVW1+p	MT20	5.0	6.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
U	2247	0	2247	0
M	2247	0	2247	0

UNFACTORED REACTIONS

	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
U	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
M	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, F-Q, H-O, C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 36	-77.4	-77.4 0.09 (1)	10.00	C-T	0 / 150	0.03 (2)
B-C	0 / 23	-77.4	-77.4 0.16 (1)	10.00	T-D	0 / 270	0.06 (3)
C-D	-2028 / 0	-77.4	-77.4 0.24 (1)	4.56	D-S	0 / 1263	0.20 (1)
D-E	-1987 / 0	-77.4	-77.4 0.65 (1)	4.08	S-E	-879 / 0	0.75 (1)
E-F	-2186 / 0	-77.4	-77.4 0.67 (1)	3.89	E-Q	0 / 372	0.06 (1)
F-G	-2186 / 0	-77.4	-77.4 0.67 (1)	3.89	Q-F	-460 / 0	0.39 (1)
G-H	-2186 / 0	-77.4	-77.4 0.67 (1)	3.89	H-Q	0 / 372	0.06 (1)
H-I	-1987 / 0	-77.4	-77.4 0.65 (1)	4.08	O-H	-879 / 0	0.75 (1)
I-J	-2028 / 0	-77.4	-77.4 0.24 (1)	4.56	O-I	0 / 1263	0.20 (1)
J-K	0 / 23	-77.4	-77.4 0.16 (1)	10.00	N-I	0 / 270	0.06 (3)
K-L	0 / 36	-77.4	-77.4 0.09 (1)	10.00	N-J	0 / 150	0.03 (2)
U-B	-206 / 0	0.0	0.0 0.01 (1)	7.81	U-C	-2316 / 0	0.76 (1)
M-K	-206 / 0	0.0	0.0 0.01 (1)	7.81	J-M	-2316 / 0	0.76 (1)
U-T	0 / 1245	-28.0	-28.0 0.27 (2)	10.00			
T-S	0 / 1300	-28.0	-28.0 0.28 (2)	10.00			
S-R	0 / 1987	-28.0	-28.0 0.30 (1)	10.00			
R-Q	0 / 1987	-28.0	-28.0 0.30 (1)	10.00			
Q-P	0 / 1987	-28.0	-28.0 0.30 (1)	10.00			
P-O	0 / 1987	-28.0	-28.0 0.30 (1)	10.00			
O-N	0 / 1300	-28.0	-28.0 0.28 (2)	10.00			
N-M	0 / 1245	-28.0	-28.0 0.27 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, EBCB 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.67 (E-F:1), BC=0.30 (Q-S:1), WB=0.76 (C-U:1), SS=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.90 (M) (INPUT = 0.90)
CSI METAL= 0.57 (C) (INPUT = 1.00)



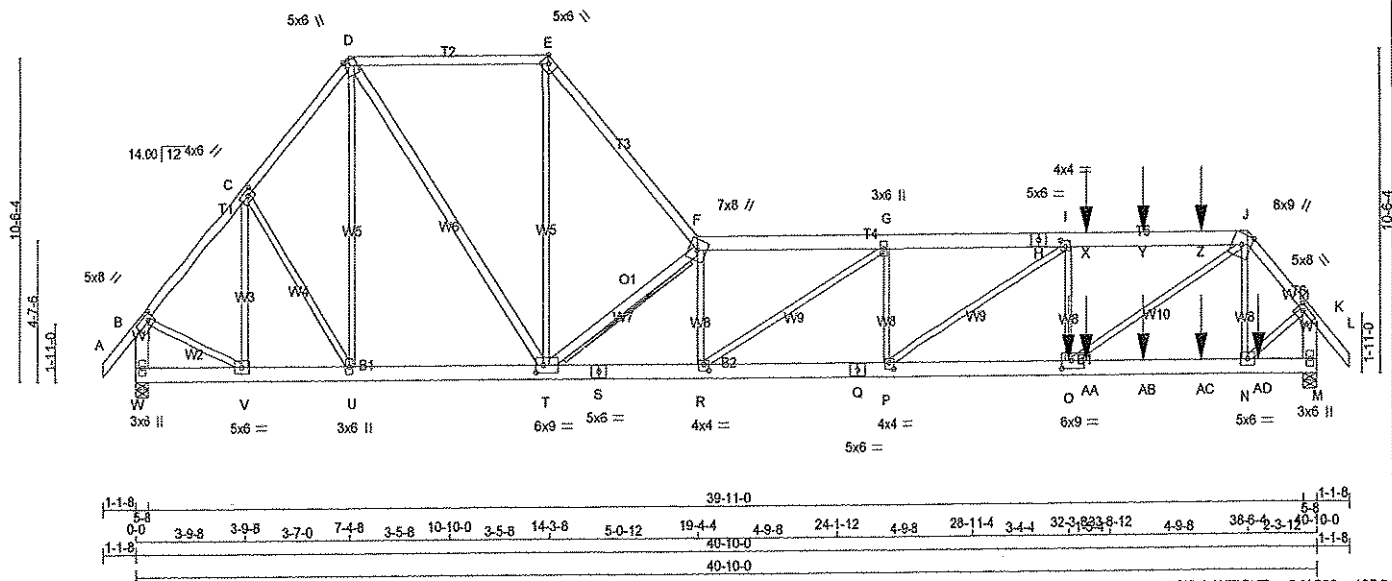
DWG NO. TAM 19688.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T25	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbv9nbN1z58_zMY8t-jAYednAEu715oUgrNoKYxAdTOIOogRPIH7Lc0KzMMUMJ

-1-4-8-0 3-9-8 3-9-8 3-7-0 7-4-8 3-5-8 10-10-0 3-5-8 14-3-8 5-0-12 19-4-4 4-9-8 24-1-12 4-9-8 28-11-4 3-4-4 32-3-8-12 4-9-8 38-5-4-10-10-0 1-1-8
Scale = 1/73.6



TOTAL WEIGHT = 2 X 233 = 465 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x6	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
W - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
W - S	2x6	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - T	2x4	DRY	No.2	SPF	
T - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
D - E 1	12	TOP
E - F 1	12	TOP
J - L 1	12	TOP
F - H 2	12	TOP
H - J 2	12	TOP
W - B 2	12	TOP
M - K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - S 2	12	TOP
S - Q 2	12	TOP
Q - M 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
I - O 1	4	SIDE(526.5)
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50 2.50
C	TMVW-t	MT20	4.0	6.0	2.00 2.75
D	TTWW+m	MT20	5.0	6.0	1.75 1.50
E	TTW-h	MT20	5.0	6.0	Edge 2.75
F	TTWW+m	MT20	7.0	8.0	
G	TMVW-t	MT20	3.0	6.0	
H	TS-t	MT20	5.0	6.0	
I	TMVW-L	MT20	4.0	4.0	2.50 2.00
J	TTWW+m	MT20	8.0	9.0	Edge

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
W	2876	0	2876	0
M	4777	0	4777	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
W	2375	1288 / 0	553 / 0
M	3688	2115 / 0	948 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF NO.2 T-BRACE AT F-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	CS (LC)	MEMB.	FORCE (LBS)	CS (LC)
FR-TO		FROM	TO	FR-TO		
A-B	0/36	-77.4	-77.4 0.05 (1)	10.00	V-C	-583 / 0
B-C	-2555 / 0	-77.4	-77.4 0.13 (1)	5.56	C-U	0 / 146
C-D	-2738 / 0	-77.4	-77.4 0.13 (1)	5.41	U-D	0 / 205
D-E	-2856 / 0	-77.4	-77.4 0.45 (1)	4.93	D-T	0 / 1968
E-F	-4381 / 0	-77.4	-77.4 0.40 (1)	4.33	T-E	0 / 2858
F-G	-8184 / 0	-77.4	-77.4 0.24 (1)	4.08	T-F	-6859 / 0
G-H	-8886 / 0	-77.4	-77.4 0.22 (1)	3.97	R-F	0 / 568
H-I	-8886 / 0	-77.4	-77.4 0.22 (1)	3.97	P-G	-81 / 167
I-X	-8397 / 0	-77.4	-77.4 0.32 (1)	3.95	O-J	0 / 6948
X-Y	-8397 / 0	-77.4	-77.4 0.32 (1)	3.95	N-J	-1182 / 0
Y-Z	-8397 / 0	-77.4	-77.4 0.32 (1)	3.95	B-V	0 / 1799
Z-J	-8397 / 0	-77.4	-77.4 0.32 (1)	3.95	N-K	0 / 3098
J-K	-4067 / 0	-77.4	-77.4 0.10 (1)	4.68	O-I	-1006 / 0
K-L	0 / 36	-77.4	-77.4 0.05 (1)	10.00	R-G	-837 / 0
W-B	-2616 / 0	0.0	0.0 0.11 (1)	7.81	P-I	0 / 583
M-K	-4875 / 0	0.0	0.0 0.19 (1)	6.58		
W-V	0 / 0	-28.0	-28.0 0.03 (2)	10.00		
V-U	0 / 1678	-28.0	-28.0 0.15 (1)	10.00		
U-T	0 / 1758	-28.0	-28.0 0.15 (1)	10.00		
T-S	0 / 8170	-28.0	-28.0 0.58 (1)	10.00		
S-R	0 / 8170	-28.0	-28.0 0.58 (1)	10.00		
R-Q	0 / 8886	-28.0	-28.0 0.64 (1)	10.00		
Q-P	0 / 8886	-28.0	-28.0 0.64 (1)	10.00		
P-O	0 / 8397	-28.0	-28.0 0.62 (1)	10.00		
O-AA	0 / 2618	-28.0	-28.0 0.29 (1)	10.00		
AA-AB	0 / 2618	-28.0	-28.0 0.29 (1)	10.00		
AB-AC	0 / 2618	-28.0	-28.0 0.29 (1)	10.00		
AC-N	0 / 2618	-28.0	-28.0 0.29 (1)	10.00		
N-AD	0 / 0	-28.0	-28.0 0.14 (2)	10.00		
AD-M	0 / 0	-28.0	-28.0 0.14 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.30")
ALLOWABLE DEFL. (TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.49")

CSI: TC=0.45 (D-E-I), BC=0.64 (P-R-I), WB=0.88 (J-O-I), SSI=0.14 (J-I-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

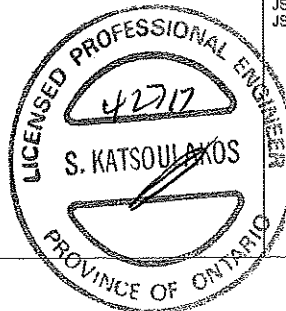
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.67 (Q) (INPUT = 1.00)



DWG NO. TAM 19689-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 281390	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Apr 27 15:52:10 2017 Page 2 ID:FS8EBGRKsZbvi9nbN1z58 zMY8t-jAYednAEu715oUgrNoKYxAdTOI0ogRPIH7Lc0KzMUMJ					

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TMVW-t	MT20	5.0	8.0	2.00	2.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	6.0	9.0	3.00	4.00
P	BMWW-t	MT20	4.0	4.0	2.50	2.00
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.50	2.00
S	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	6.0	9.0	3.00	3.00
U	BMWWW-t	MT20	3.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 93.1 lbs FACTORED DOWN AT
32-10-12, AND 93.1 lbs FACTORED DOWN AT
34-10-12, AND 93.1 lbs FACTORED DOWN AT
38-10-12 ON TOP CHORD, AND 2720.0 lbs
FACTORED DOWN AT 32-3-8, 70.0 lbs
FACTORED DOWN AT 32-10-12, 70.0 lbs
FACTORED DOWN AT 34-10-12, AND 70.0 lbs
FACTORED DOWN AT 38-10-12, AND 70.0 lbs
FACTORED DOWN AT 38-10-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	32-3-8	-2720	-2720	---	FRONT	VERT	TOTAL
X	32-10-12	-93	-93	---	FRONT	VERT	TOTAL
Y	34-10-12	-93	-93	---	FRONT	VERT	TOTAL
Z	36-10-12	-93	-93	---	FRONT	VERT	TOTAL
AA	32-10-12	-40	-70	---	FRONT	VERT	TOTAL
AB	34-10-12	-40	-70	---	FRONT	VERT	TOTAL
AC	36-10-12	-40	-70	---	FRONT	VERT	TOTAL
AD	38-10-12	-40	-70	---	FRONT	VERT	TOTAL



P12/12
DWG NO. TAM1968917
STRUCTURAL
COMPONENT ONLY

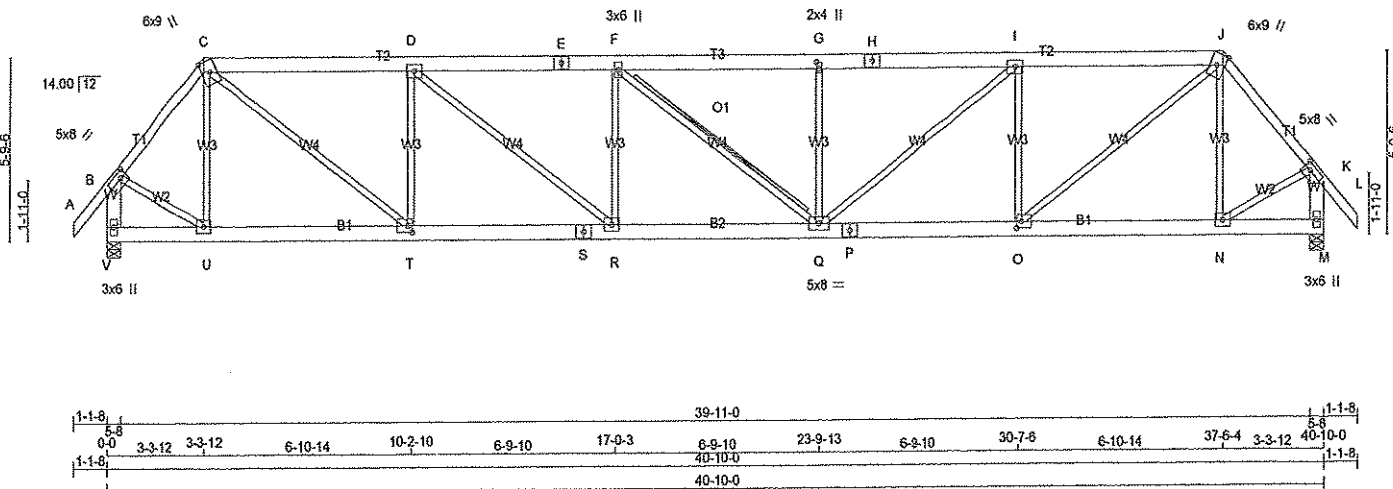
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
281390	T26	1	1	44080	

Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbvi9nbN1z58_zMY8HJAyEdnAEu715oUgrNoKYxAdUj1DgVqIH7LcOKzMUMJ

1-1-8 3-3-12 3-3-12 6-10-14 10-2-10 6-9-10 17-0-3 6-9-10 23-9-13 6-9-10 30-7-6 6-10-14 37-6-4 3-3-12 40-10-0 1-1-8
Scale = 1/7.5



TOTAL WEIGHT = 219 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SFF		
C - E	2x6	DRY	No.2	SPF		
E - H	2x6	DRY	No.2	SPF		
H - J	2x6	DRY	No.2	SPF		
J - L	2x4	DRY	No.2	SPF		
L - B	2x6	DRY	No.2	SPF		
M - K	2x6	DRY	No.2	SPF		
V - S	2x6	DRY	No.2	SPF		
S - P	2x6	DRY	No.2	SPF		
P - M	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	2.50	
C	TTWW+m	MT20	6.0	9.0	Edge	3.25	
D	TMWW-t	MT20	5.0	6.0			
E	TS-t	MT20	5.0	6.0			
F	TMWW-t	MT20	3.0	6.0			
G	TMW+w	MT20	2.0	4.0	2.50	1.00	
H	TS-t	MT20	5.0	6.0			
I	TMWW-t	MT20	5.0	6.0			
J	TTWW+m	MT20	6.0	9.0	Edge	3.25	
K	TMVW-t	MT20	5.0	8.0	2.25	2.50	
M	BMV1+p	MT20	3.0	6.0			
N, R, U							
N	BMWW-t	MT20	5.0	6.0			
O	BMWW-t	MT20	5.0	6.0	2.50	1.75	
P	BS-t	MT20	5.0	6.0			
Q	BMWWW-t	MT20	5.0	8.0			
S	BS-t	MT20	5.0	6.0			
T	BMWW-t	MT20	5.0	6.0	2.50	1.75	
V	BMV1+p	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
V	2247	0	2247	0	0	5-8	5-8	5-8	5-8
M	2247	0	2247	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
M	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 36	-77.4	-77.4 0.09 (1)	10.00	U-C	-355 / 0	0.16 (1)
B-C	-1936 / 0	-77.4	-77.4 0.21 (1)	4.66	C-T	0 / 2422	0.55 (1)
C-D	-3171 / 0	-77.4	-77.4 0.31 (1)	4.49	T-D	-1298 / 0	0.58 (1)
D-E	-4069 / 0	-77.4	-77.4 0.36 (1)	4.02	D-R	0 / 1140	0.26 (1)
E-F	-4069 / 0	-77.4	-77.4 0.36 (1)	4.02	R-F	-511 / 0	0.23 (1)
F-G	-4069 / 0	-77.4	-77.4 0.29 (1)	4.09	F-Q	-4 / 0	0.01 (1)
G-H	-4069 / 0	-77.4	-77.4 0.35 (1)	4.02	Q-G	-510 / 0	0.23 (1)
H-I	-4069 / 0	-77.4	-77.4 0.36 (1)	4.02	I-Q	0 / 1135	0.26 (1)
I-J	-3172 / 0	-77.4	-77.4 0.31 (1)	4.49	Q-I	-1294 / 0	0.58 (1)
J-K	-1936 / 0	-77.4	-77.4 0.21 (1)	4.66	O-J	0 / 2423	0.55 (1)
K-L	0 / 36	-77.4	-77.4 0.09 (1)	10.00	N-J	-355 / 0	0.16 (1)
V-B	-2233 / 0	0.0	0.0 0.16 (1)	6.84	B-U	0 / 1371	0.31 (1)
M-K	-2233 / 0	0.0	0.0 0.16 (1)	6.84	N-K	0 / 1371	0.31 (1)
V-U	0 / 0	-28.0	-28.0 0.11 (2)	10.00			
U-T	0 / 1253	-28.0	-28.0 0.23 (1)	10.00			
T-S	0 / 3171	-28.0	-28.0 0.44 (1)	10.00			
S-R	0 / 3171	-28.0	-28.0 0.44 (1)	10.00			
R-Q	0 / 4069	-28.0	-28.0 0.55 (1)	10.00			
Q-P	0 / 3172	-28.0	-28.0 0.44 (1)	10.00			
P-O	0 / 3172	-28.0	-28.0 0.44 (1)	10.00			
O-N	0 / 1253	-28.0	-28.0 0.23 (2)	10.00			
N-M	0 / 0	-28.0	-28.0 0.11 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.41")

CSI: TC=0.38 (G-I:1), BC=0.55 (Q-R:1), WB=0.58 (D-T:1), SSI=0.19 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

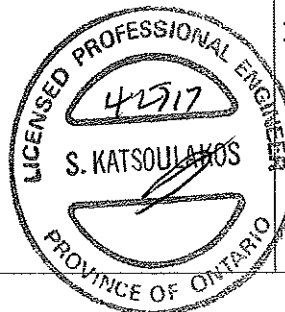
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.49 (P) (INPUT = 1.00)



DRWG NO. TAM 19690-17
STRUCTURAL
COMPONENT ONLY

DRWG NO.

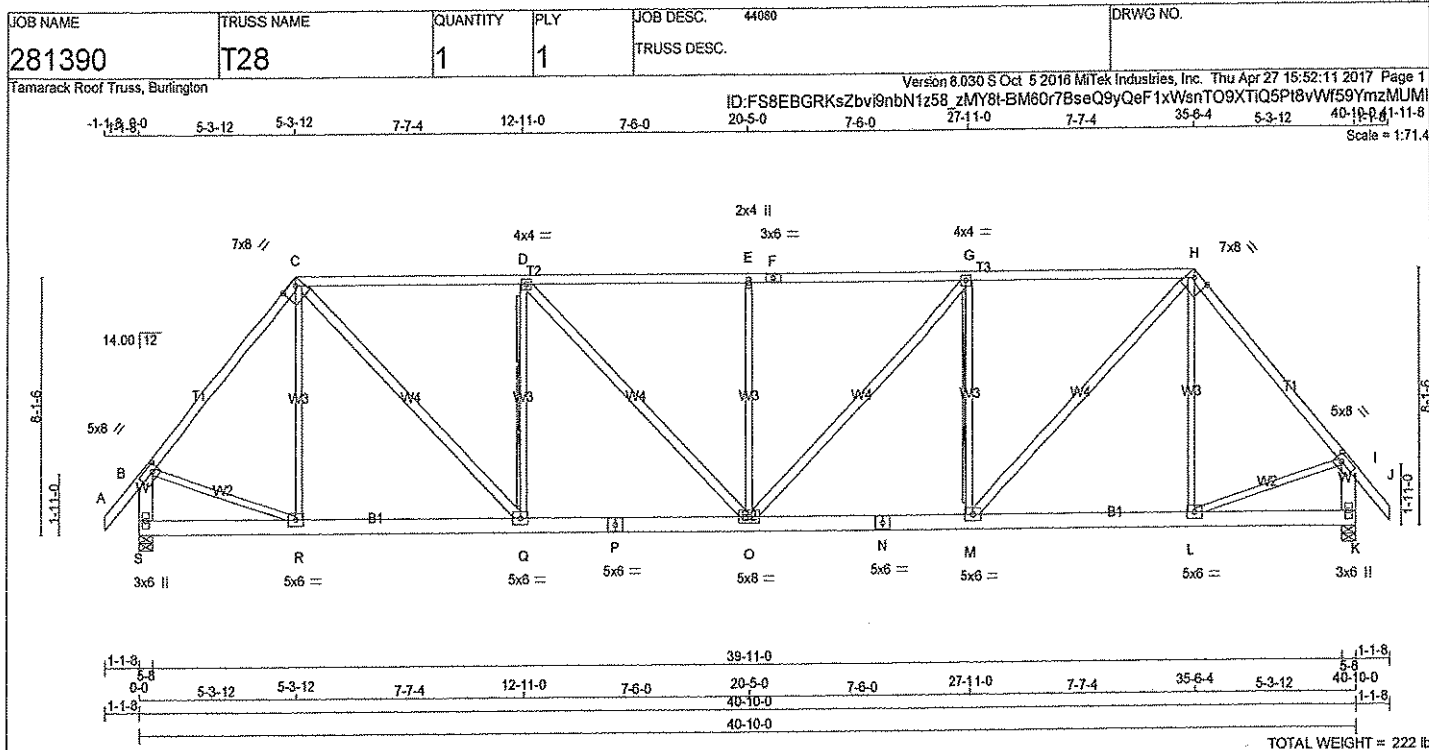
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TOTAL WEIGHT = 208 lb

JSI GRIP= 0.90 (T) (INPUT = 0.90)
JSI METAL= 0.43 (P) (INPUT = 1.00)

DWG NO. TAM 19691-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	SPF
C - F	2x4	DRY	SPF
F - H	2x4	DRY	SPF
H - J	2x4	DRY	SPF
S - B	2x6	DRY	SPF
K - I	2x6	DRY	SPF
S - P	2x6	DRY	SPF
P - N	2x6	DRY	SPF
N - K	2x6	DRY	SPF
ALL WEBS EXCEPT	2x3	DRY	SPF
C - Q	2x4	DRY	SPF
D - O	2x4	DRY	SPF
O - G	2x4	DRY	SPF
M - H	2x4	DRY	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	2.50
C	TTWW-h	MT20	7.0	8.0	2.00	5.25
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-l	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-h	MT20	7.0	8.0	2.00	5.25
I	TMVW-t	MT20	5.0	8.0	2.50	2.50
K	BMV-t+p	MT20	3.0	6.0		
L, M, Q, R	BMVW-t	MT20	5.0	6.0		
N	BS-l	MT20	5.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BS-l	MT20	5.0	6.0		
S	BMV-t+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D
	VERT	HORZ	DOWN	UP
JT	2247	0	2247	0
S	2247	0	2247	0
K	2247	0	2247	0

UNFACTORED REACTIONS

	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
S	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0
K	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF NO.2 T-BRACE AT D-Q, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. FACTORED	MEMB.	FORCE (LBS)	MAX. FACTORED	MAX. FACTORED
			CS1 (LC)			CS1 (LC)	CS1 (LC)
FR-TO	0 / 36	-77.4	-77.4 0.07 (1)	10.00	R-C	-153 / 155	0.18 (1)
A-B	-2019 / 0	-77.4	-77.4 0.45 (1)	4.92	C-Q	0 / 1741	0.28 (1)
C-D	-2530 / 0	-77.4	-77.4 0.77 (1)	4.08	Q-D	-1022 / 0	0.96 (1)
D-E	-2886 / 0	-77.4	-77.4 0.81 (1)	3.83	D-O	0 / 511	0.08 (1)
E-F	-2886 / 0	-77.4	-77.4 0.81 (1)	3.83	O-E	-535 / 0	0.64 (1)
F-G	-2886 / 0	-77.4	-77.4 0.77 (1)	4.08	G-G	0 / 511	0.08 (1)
G-H	-2530 / 0	-77.4	-77.4 0.45 (1)	4.92	H-G	-1022 / 0	0.96 (1)
H-I	-2019 / 0	-77.4	-77.4 0.07 (1)	10.00	I-H	-153 / 155	0.18 (1)
I-J	0 / 36	-77.4	-77.4 0.07 (1)	10.00	L-H	0 / 1741	0.28 (1)
S-B	-2192 / 0	0.0	0.0 0.16 (1)	6.90	B-R	0 / 1356	0.31 (1)
K-I	-2192 / 0	0.0	0.0 0.16 (1)	6.90	L-I	0 / 1356	0.31 (1)
S-R	0 / 0	-28.0	-28.0 0.13 (3)	10.00			
R-Q	0 / 1307	-28.0	-28.0 0.27 (2)	10.00			
Q-P	0 / 2530	-28.0	-28.0 0.38 (1)	10.00			
P-O	0 / 2530	-28.0	-28.0 0.38 (1)	10.00			
O-N	0 / 2530	-28.0	-28.0 0.38 (1)	10.00			
N-M	0 / 2530	-28.0	-28.0 0.38 (1)	10.00			
M-L	0 / 1307	-28.0	-28.0 0.27 (2)	10.00			
L-K	0 / 0	-28.0	-28.0 0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")

CSI: TC=0.81 (E-G:1), BC=0.38 (O-Q:1),
WB=0.96 (D-Q:1), SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

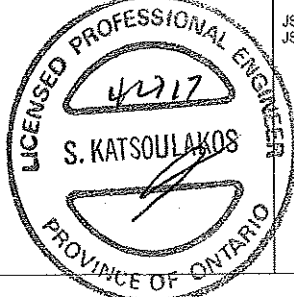
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.45 (P) (INPUT = 1.00)



DWG NO. TAM 19692-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281390	TRUSS NAME T29	QUANTITY 7	PLY 1	JOB DESC. 44080	TRUSS DESC.	DRWG NO.
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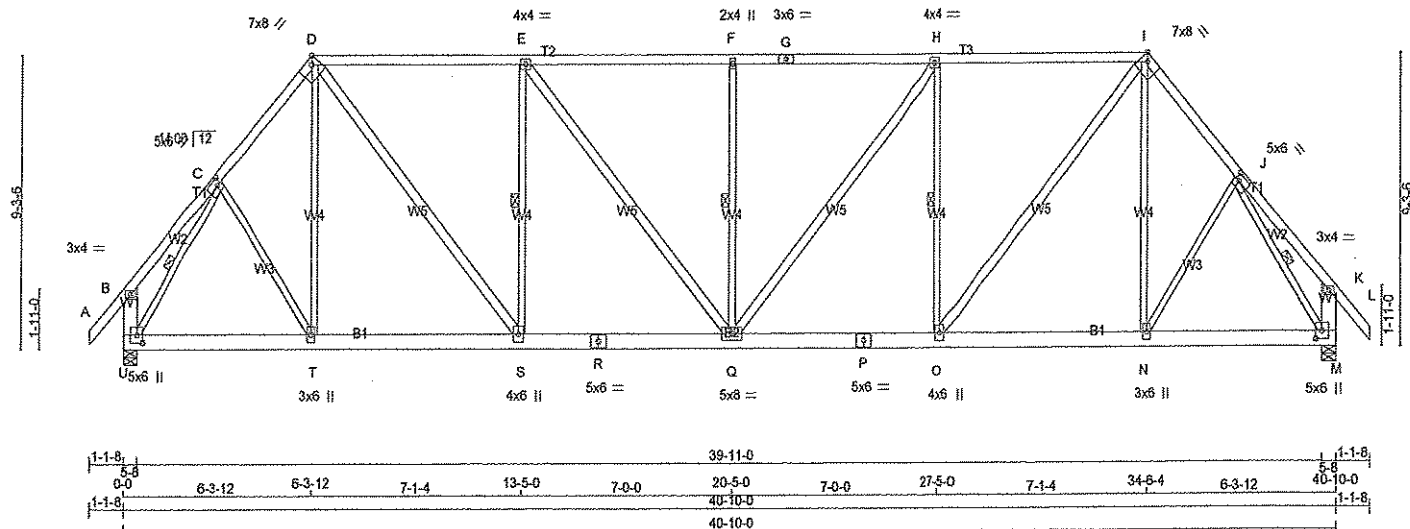
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Apr 27 15:52:11 2017 Page 1

ID:FS8EBGRKsZbv9nbN1z58_zMY8t-BM60r7BseQ9yQef1xWsnTO9WOIQPzBvW59YmzMUMI

1-1-8 0 3-3-2 3-3-2 3-0-10 6-3-12 7-1-4 13-5-0 7-0-0 20-5-0 7-0-0 27-5-0 7-1-4 34-6-4 3-0-10 37-6-14 3-3-2 40-10-0 1-1-8

Scale = 1:7.7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV-p	MT20	3.0	4.0
C	TMWW-t	MT20	5.0	6.0 2.50 1.75
D	TTWW-h	MT20	7.0	8.0 Edge 2.75
E	TMWW-t	MT20	4.0	4.0
F	TMW-w	MT20	2.0	4.0
G	TS-t	MT20	3.0	6.0
H	TMWW-t	MT20	4.0	4.0
I	TTWW-h	MT20	7.0	8.0 Edge 2.75
J	TMWW-t	MT20	5.0	6.0 2.50 1.75
K	TMV-p	MT20	3.0	4.0
M	BMVW1+p	MT20	5.0	6.0 3.00 2.25
N	BMWWH-t	MT20	3.0	6.0
O	BMWWH-t	MT20	4.0	6.0
P	BS-t	MT20	5.0	6.0
Q	BMWWH-t	MT20	5.0	8.0
R	BS-t	MT20	5.0	6.0
S	BMWWH-t	MT20	4.0	6.0
T	BMWWH-t	MT20	3.0	6.0
U	BMVW1+p	MT20	5.0	6.0 3.00 2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	HORZ	DOWN
U	2247	0	2247
M	2247	0	2247

UNFACTORED REACTIONS						
	1ST LCASE	MAX	MIN	COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
U	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0
M	1853	1008 / 0	429 / 0	0 / 0	0 / 0	416 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

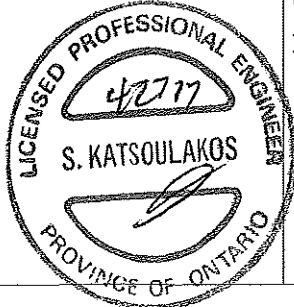
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-Q, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO			
A-B	0 / 36	-77.4	-77.4 0.09 (1)
B-C	0 / 18	-77.4	-77.4 0.10 (1)
C-D	-2039 / 0	-77.4	-77.4 0.17 (1)
D-E	-2234 / 0	-77.4	-77.4 0.83 (1)
E-F	-2502 / 0	-77.4	-77.4 0.88 (1)
F-G	-2502 / 0	-77.4	-77.4 0.88 (1)
G-H	-2502 / 0	-77.4	-77.4 0.88 (1)
H-I	-2234 / 0	-77.4	-77.4 0.83 (1)
I-J	-2039 / 0	-77.4	-77.4 0.17 (1)
J-K	0 / 18	-77.4	-77.4 0.10 (1)
K-L	0 / 36	-77.4	-77.4 0.09 (1)
U-B	-192 / 0	0.0	0.0 0.01 (1)
M-K	-192 / 0	0.0	0.0 0.01 (1)
U-T	0 / 1210	-28.0	-28.0 0.24 (2)
T-S	0 / 1308	-28.0	-28.0 0.26 (2)
S-R	0 / 2234	-28.0	-28.0 0.34 (1)
R-Q	0 / 2234	-28.0	-28.0 0.34 (1)
Q-P	0 / 2234	-28.0	-28.0 0.34 (1)
P-O	0 / 2234	-28.0	-28.0 0.34 (1)
O-N	0 / 1308	-28.0	-28.0 0.26 (2)
N-M	0 / 1210	-28.0	-28.0 0.24 (2)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.88 (E-F:1), BC=0.34 (O-Q:1), WB=0.57 (J-M:1), SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

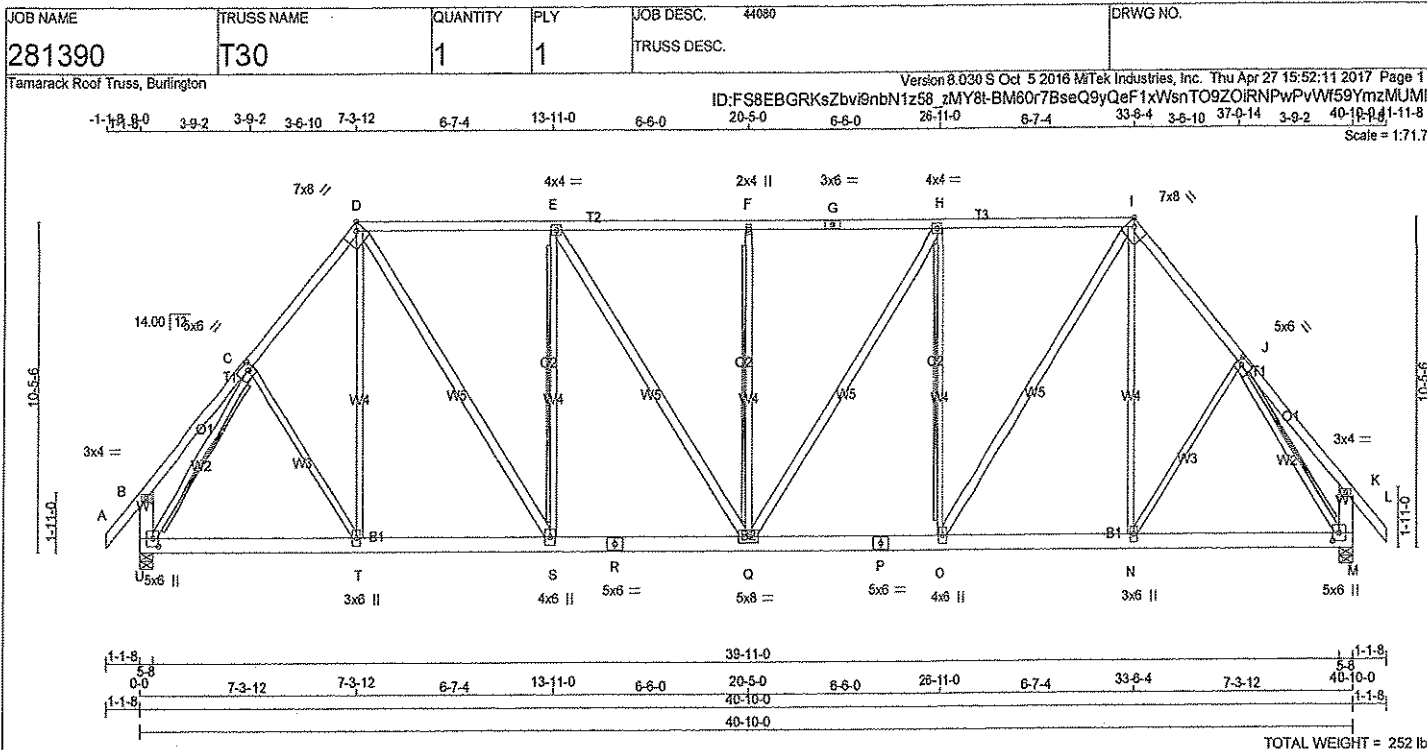
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.98 (C) (INPUT = 1.00)

DRWG NO. TAM19693-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - S	2x4	DRY	No.2
E - Q	2x4	DRY	No.2
Q - H	2x4	DRY	No.2
O - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.00
D TTWW-h	MT20	7.0	8.0	Edge	2.75
E TMWW-t	MT20	4.0	4.0		
F TMW-w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWW-t	MT20	4.0	4.0		
I TTWW-h	MT20	7.0	8.0	Edge	2.75
J TMWW-t	MT20	5.0	6.0	2.50	2.00
K TMV-p	MT20	3.0	4.0		
M BMVW1+p	MT20	5.0	6.0	3.00	2.25
N BMWW-t	MT20	3.0	6.0		
O BMWW-t	MT20	4.0	8.0		
P BS-t	MT20	5.0	6.0		
Q BMWW-t	MT20	5.0	8.0		
R BS-t	MT20	5.0	6.0		
S BMWW-t	MT20	4.0	6.0		
T BMWW-t	MT20	3.0	6.0		
U BMVW1+p	MT20	5.0	8.0	3.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2247	2247	0	5-8
U VERT	2247	2247	0	5-8
M VERT	2247	2247	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	1853	1008 / 0
U COMBINED	1853	1008 / 0
M COMBINED	1853	1008 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-S, F-Q, H-O, C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO				FR-TO			
A-B	0 / 36	-77.4	-77.4 0.09 (1)	10.00	C-T	0 / 154	0.03 (2)
B-C	0 / 23	-77.4	-77.4 0.15 (1)	10.00	T-D	0 / 266	0.06 (3)
C-D	-2029 / 0	-77.4	-77.4 0.24 (1)	4.57	D-S	0 / 1278	0.21 (1)
D-E	-2003 / 0	-77.4	-77.4 0.66 (1)	4.05	S-E	-884 / 0	0.74 (1)
E-F	-2207 / 0	-77.4	-77.4 0.69 (1)	3.86	E-Q	0 / 376	0.06 (1)
F-G	-2207 / 0	-77.4	-77.4 0.69 (1)	3.86	Q-F	-463 / 0	0.39 (1)
G-H	-2207 / 0	-77.4	-77.4 0.69 (1)	3.86	H-Q	0 / 376	0.06 (1)
H-I	-2003 / 0	-77.4	-77.4 0.68 (1)	4.05	O-H	-884 / 0	0.74 (1)
I-J	-2029 / 0	-77.4	-77.4 0.24 (1)	4.57	O-I	0 / 1278	0.21 (1)
J-K	0 / 23	-77.4	-77.4 0.15 (1)	10.00	N-I	0 / 266	0.06 (3)
K-L	0 / 36	-77.4	-77.4 0.09 (1)	10.00	N-J	0 / 154	0.03 (2)
U-B	-205 / 0	0.0	0.0 0.01 (1)	7.81	U-C	-2317 / 0	0.75 (1)
M-K	-205 / 0	0.0	0.0 0.01 (1)	7.81	J-M	-2317 / 0	0.75 (1)
U-T	0 / 1243	-28.0	-28.0 0.27 (2)	10.00			
T-S	0 / 1301	-28.0	-28.0 0.28 (2)	10.00			
S-R	0 / 2003	-28.0	-28.0 0.30 (1)	10.00			
R-Q	0 / 2003	-28.0	-28.0 0.30 (1)	10.00			
Q-P	0 / 2003	-28.0	-28.0 0.30 (1)	10.00			
P-O	0 / 2003	-28.0	-28.0 0.30 (1)	10.00			
O-N	0 / 1301	-28.0	-28.0 0.28 (2)	10.00			
N-M	0 / 1243	-28.0	-28.0 0.27 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.69 (E-F:1), BC=0.30 (O-Q:1), WB=0.75 (J-M:1), SS=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

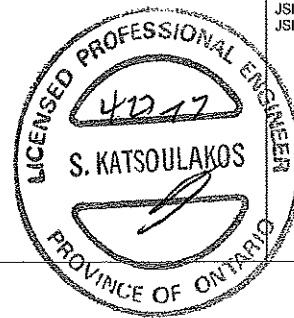
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	618	354	1667
822	2284	1658	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 1.00)



DRWG NO. TAM 1969417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T31S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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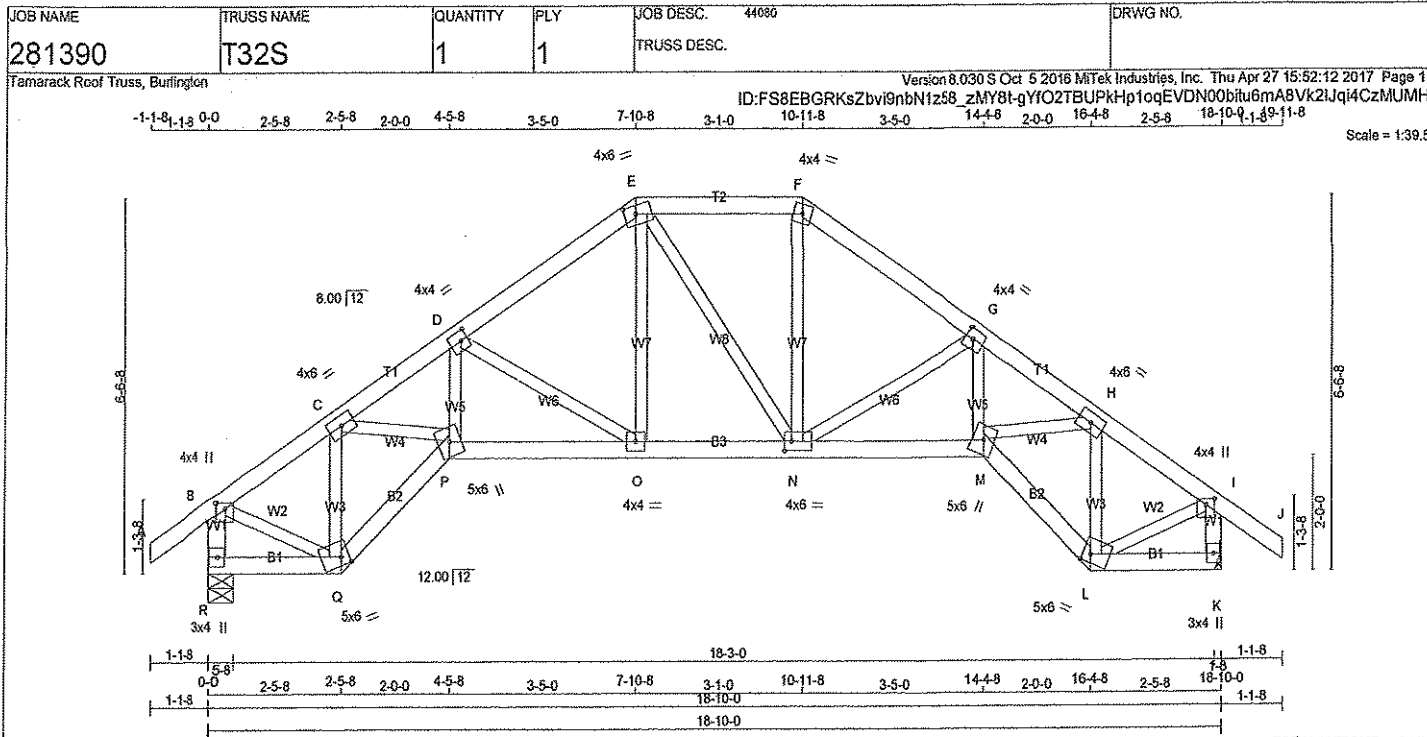
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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	8-10-12	-212	-212	---	FRONT	VERT	TOTAL
X	10-10-12	-212	-212	---	FRONT	VERT	TOTAL
Y	4-10-12	---	---	---	FRONT	VERT	TOTAL
Z	6-10-12	---	---	---	FRONT	VERT	TOTAL
AA	8-10-12	---	---	---	FRONT	VERT	TOTAL
AB	10-10-12	---	---	---	FRONT	VERT	TOTAL
AC	14-10-12	---	---	---	FRONT	VERT	TOTAL
AD	16-10-12	-47	-83	---	FRONT	VERT	TOTAL



DWG NO. TAM 19695-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-t	MT20	4.0	6.0		
I	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	BMVt+p	MT20	3.0	4.0		
L	BBWW-m	MT20	5.0	6.0	1.75	1.75
M	BBWW+m	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.50
O	BMWW-t	MT20	4.0	4.0		
P	BBWW+m	MT20	5.0	6.0		
Q	BBWW-m	MT20	5.0	6.0	1.75	1.75
R	BMVt+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
R	1085	0	1085	0
K	1085	0	1085	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	888	485 / 0	198 / 0	0 / 0	0 / 0	198 / 0	0 / 0	0 / 0
K	888	485 / 0	198 / 0	0 / 0	0 / 0	198 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 26	-77.4	-77.4 0.08 (1)	10.00	Q-C	-1044 / 0	0.18 (1)
B-C	-996 / 0	-77.4	-77.4 0.07 (1)	8.20	C-P	0 / 1025	0.23 (1)
C-D	-2136 / 0	-77.4	-77.4 0.10 (1)	4.60	P-D	0 / 633	0.14 (1)
D-E	-1294 / 0	-77.4	-77.4 0.13 (1)	5.55	D-O	-792 / 0	0.21 (1)
E-F	-1082 / 0	-77.4	-77.4 0.11 (1)	5.96	O-E	0 / 480	0.11 (1)
F-G	-1296 / 0	-77.4	-77.4 0.13 (1)	5.55	E-N	0 / 4	0.00 (1)
G-H	-2136 / 0	-77.4	-77.4 0.10 (1)	4.60	N-F	0 / 485	0.11 (1)
H-I	-996 / 0	-77.4	-77.4 0.07 (1)	6.20	N-G	-790 / 0	0.21 (1)
I-J	0 / 26	-77.4	-77.4 0.08 (1)	10.00	M-G	0 / 631	0.14 (1)
R-B	-1051 / 0	0.0	0.0 0.11 (1)	7.68	M-H	0 / 1025	0.23 (1)
K-I	-1051 / 0	0.0	0.0 0.11 (1)	7.68	L-H	-1044 / 0	0.18 (1)
					B-Q	0 / 890	0.20 (1)
					L-I	0 / 890	0.20 (1)
R-Q	0 / 0	-28.0	-28.0 0.05 (2)	10.00			
Q-P	0 / 1114	-28.0	-28.0 0.20 (1)	10.00			
P-O	0 / 1751	-28.0	-28.0 0.33 (1)	10.00			
O-N	0 / 1080	-28.0	-28.0 0.22 (1)	10.00			
N-M	0 / 1751	-28.0	-28.0 0.32 (1)	10.00			
M-L	0 / 1114	-28.0	-28.0 0.20 (1)	10.00			
L-K	0 / 0	-28.0	-28.0 0.05 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.83")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.83")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.13 (F-G:1), BC=0.33 (O-P:1), WB=0.23 (C-P:1), SSI=0.09 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

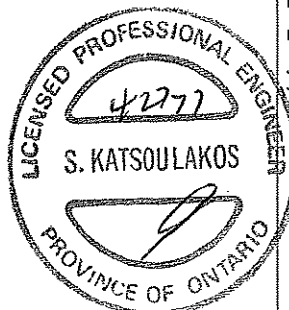
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

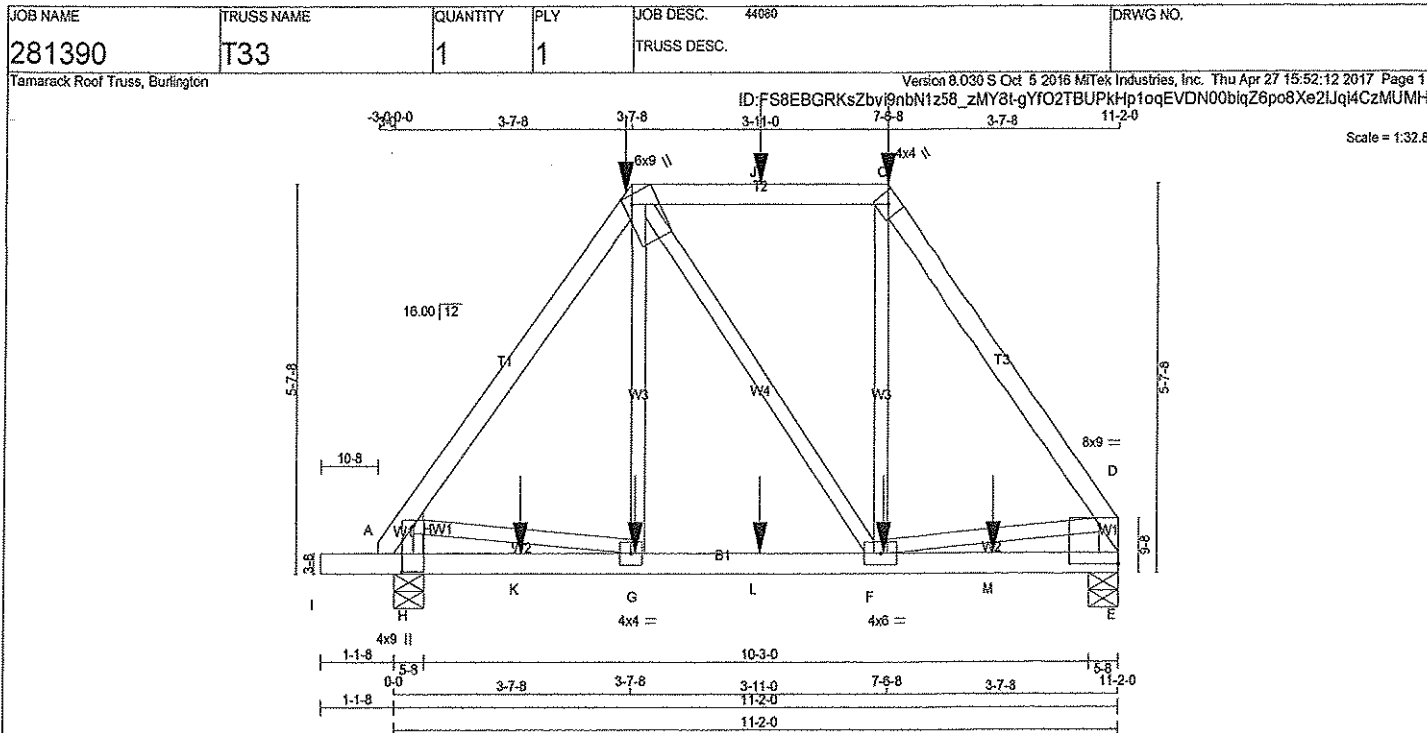
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



DWG NO. TAM 1969617
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 54 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
H - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
I - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, D, E, H						
A						
B	TTWW+m	MT20	6.0	9.0	Edge	1.50
C	TTW-h	MT20	4.0	4.0		
D	TBMVW1+I	MT20	8.0	9.0	6.75	Edge
F	BMVWVW-t	MT20	4.0	6.0		
G	BMVWVW-t	MT20	4.0	4.0		
H	TBMVW1+P	MT20	4.0	9.0	8.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.0 lbs FACTORED DOWN AT 3-7-8, AND 33.3 lbs FACTORED DOWN AT 5-7-0, AND 134.0 lbs FACTORED DOWN AT 7-6-8 ON TOP CHORD, AND 13.4 lbs FACTORED DOWN AND 1.1 lbs FACTORED UP AT 1-11-4, 13.4 lbs FACTORED DOWN AT 3-8-4, 13.4 lbs FACTORED DOWN AT 5-7-0, AND 13.4 lbs FACTORED DOWN AT 7-5-12, AND 13.4 lbs FACTORED DOWN AND 1.1 lbs FACTORED UP AT 9-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT	VERT	HORZ	UP/LIFT	IN-SX	
H	901	0	0	5-8	5-8
E	734	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
H	732	413 / 0	157 / 0	0 / 0	0 / 0	162 / 0	0 / 0
E	596	336 / 0	128 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED LC1 MAX (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LBS)	
FR-TO		FROM	TO	FR-TO		FROM	TO
H-A	-659 / 0	0.0	0.0 0.21 (1)	G-B	0 / 185	0.05 (3)	
A-B	-645 / 0	-77.4	-77.4 0.22 (1)	B-F	0 / 23	0.01 (1)	
B-J	-381 / 0	-77.4	-77.4 0.28 (1)	F-C	0 / 201	0.05 (3)	
J-C	-381 / 0	-77.4	-77.4 0.28 (1)	A-G	0 / 438	0.11 (1)	
C-D	-636 / 0	-77.4	-77.4 0.21 (1)	F-D	0 / 381	0.09 (1)	
E-D	-691 / 0	0.0	0.0 0.08 (1)				
I-H	0 / 0	-105.4	-105.4 0.14 (1)				
H-K	-106 / 0	-28.0	-28.0 0.09 (3)				
K-G	-106 / 0	-28.0	-28.0 0.09 (3)				
G-L	0 / 368	-28.0	-28.0 0.16 (2)				
L-F	0 / 368	-28.0	-28.0 0.16 (2)				
F-M	0 / 0	-28.0	-28.0 0.11 (3)				
M-E	0 / 0	-28.0	-28.0 0.11 (3)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-7-8	-134	-134	---	BACK	VERT	TOTAL
C	7-6-8	-134	-134	---	BACK	VERT	TOTAL
F	7-5-12	-8	-13	---	BACK	VERT	TOTAL
G	3-8-4	-8	-13	---	BACK	VERT	TOTAL
J	5-7-0	-33	-33	---	BACK	VERT	TOTAL
K	1-11-4	1	-13	1	BACK	VERT	TOTAL
L	5-7-0	-8	-13	---	BACK	VERT	TOTAL
M	9-2-12	1	-13	1	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.28 (B-C:1), BC=0.16 (F-G:2), WB=0.11 (A-G:1), SS=0.25 (A-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (F) (INPUT = 0.90)
JSI METAL= 0.16 (G) (INPUT = 1.00)



DRWG NO. TAM19692-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T34	1	1	TRUSS DESC.		

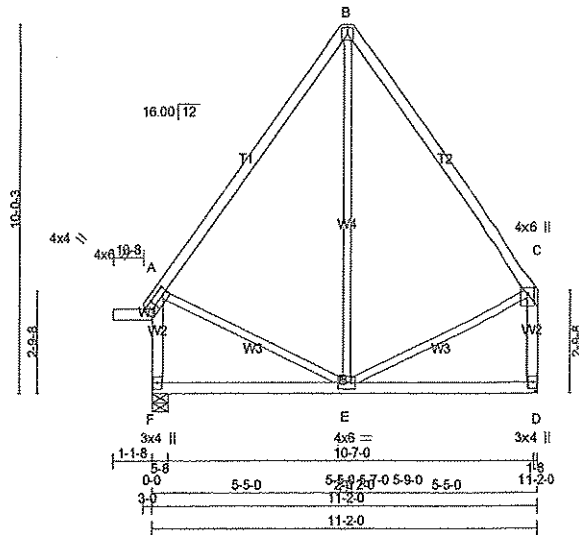
Tamarack Roof Truss, Burlington

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3-0-0 5-5-0 5-5-0 5-5-0 11-2-0
4x4 =

Scale = 1:61.5



TOTAL WEIGHT = 58 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVV+I	MT20	4.0	6.0	2.00 1.75
A	NP+I	MT20	4.0	4.0	2.00 1.75
B	TTW+p	MT20	4.0	4.0	
C	TMVV+p	MT20	4.0	6.0	2.00 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMVWW+I	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
F	588	0	588	0	0	5-8	5-8		
D	588	0	588	0	0			HANGER BY OTHERS	
								MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	489	260 / 0	117 / 0	0 / 0	0 / 0	112 / 0	0 / 0
D	489	260 / 0	117 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1 MAX	CS1 (LC)	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO						FR-TO			
A-B	-266 / 0	-77.4	-77.4	0.31 (1)	6.25	A-E	0 / 193	0.04 (1)	
B-C	-266 / 0	-77.4	-77.4	0.31 (1)	6.25	E-C	0 / 193	0.04 (1)	
F-A	-529 / 0	0.0	0.0	0.08 (1)	7.81	E-B	0 / 247	0.06 (3)	
D-C	-529 / 0	0.0	0.0	0.08 (1)	7.81				
F-E	0 / 0	-28.0	-28.0	0.25 (3)	10.00				
E-D	0 / 0	-28.0	-28.0	0.25 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.31 (A-B:1), BC=0.25 (E-F:3), WB=0.06 (B-E:3), SSI=0.13 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

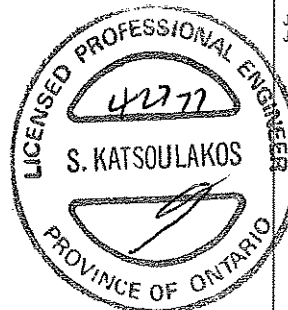
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

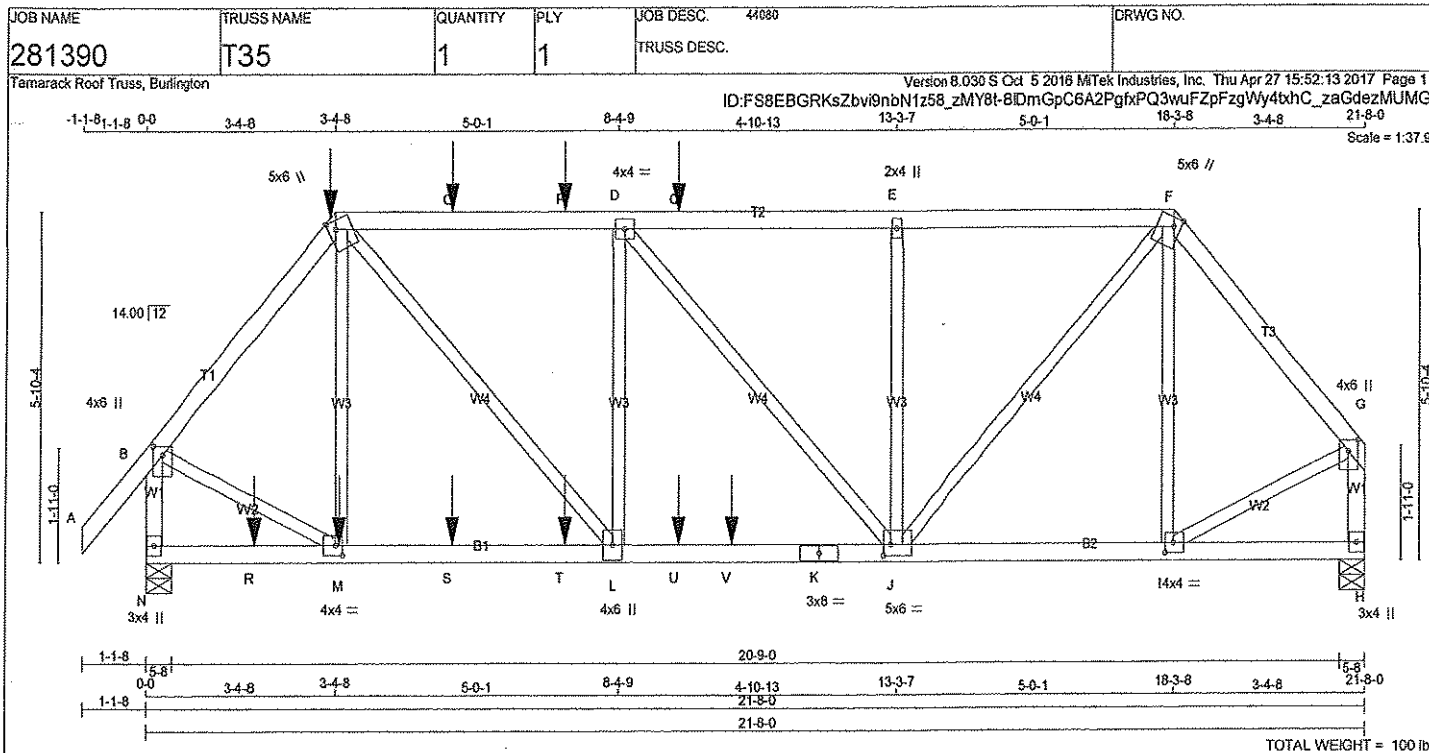
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL = 0.12 (A) (INPUT = 1.00)



DRWG NO. TAM1969817
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.75	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+y	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	1.75	1.50
G	TMVW+p	MT20	4.0	6.0	2.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMWW-t	MT20	5.0	6.0	2.25	1.50
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.6 lbs FACTORED DOWN AT 3-4-8, 53.2 lbs FACTORED DOWN AT 5-5-4, AND 53.2 lbs FACTORED DOWN AT 7-5-4, AND 53.2 lbs FACTORED DOWN AT 9-5-4 ON TOP CHORD, AND 30.8 lbs FACTORED DOWN AT 1-11-4, 30.8 lbs FACTORED DOWN AT 3-5-4, 30.8 lbs FACTORED DOWN AT 5-5-4, 30.8 lbs FACTORED DOWN AT 7-5-4, AND 30.8 lbs FACTORED DOWN AT 9-5-4, AND 512.9 lbs FACTORED DOWN AT 10-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	1834	0	1834	0	5-8	5-8
H	1494	0	1494	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
N	1487	848 / 0	317 / 0	0 / 0	0 / 0	323 / 0	0 / 0
H	1229	673 / 0	281 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
A-B	0 / 36	-77.4	M-C	-320 / 62
B-C	-1479 / 0	-77.4	C-L	0 / 1251
C-O	-1791 / 0	-77.4	L-D	-384 / 21
O-P	-1791 / 0	-77.4	D-J	-204 / 0
P-D	-1791 / 0	-77.4	J-E	-404 / 0
D-Q	-1656 / 0	-77.4	E-F	0 / 1267
Q-E	-1656 / 0	-77.4	F-M	-285 / 15
E-F	-1656 / 0	-77.4	M-B	0 / 1047
F-G	-1253 / 0	-77.4	I-G	0 / 887
N-B	-1779 / 0	0.0		
H-G	-1446 / 0	0.0		
N-R	0 / 0	-28.0		
R-M	0 / 0	-28.0		
M-S	0 / 951	-28.0		
S-T	0 / 951	-28.0		
T-L	0 / 951	-28.0		
L-U	0 / 1791	-28.0		
U-V	0 / 1791	-28.0		
V-K	0 / 1791	-28.0		
K-J	0 / 1791	-28.0		
J-I	0 / 805	-28.0		
I-H	0 / 0	-28.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-4-8	-189	-189		FRONT	VERT	TOTAL
M	3-5-4	-18	-31		FRONT	VERT	TOTAL
O	5-5-4	-53	-53		FRONT	VERT	TOTAL
P	7-5-4	-53	-53		FRONT	VERT	TOTAL
Q	9-5-4	-53	-53		FRONT	VERT	TOTAL
R	1-11-4	-18	-31		FRONT	VERT	TOTAL
S	5-5-4	-18	-31		FRONT	VERT	TOTAL
T	7-5-4	-18	-31		FRONT	VERT	TOTAL
U	9-5-4	-18	-31		FRONT	VERT	TOTAL
V	10-4-8	-513	-513		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")

CALCULATED VERT. DEFL.(LL) = L/999 (0.14")

ALLOWABLE DEFL.(TL)= L/360 (0.72")

CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.44 (C-D:1), BC=0.99 (J-L:1), WB=0.31 (F-J:1), SS=0.34 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

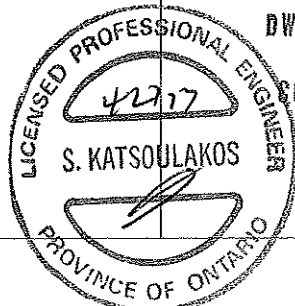
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)

JSI METAL= 0.73 (K) (INPUT = 1.00)



DWG NO. TAM 19699-17

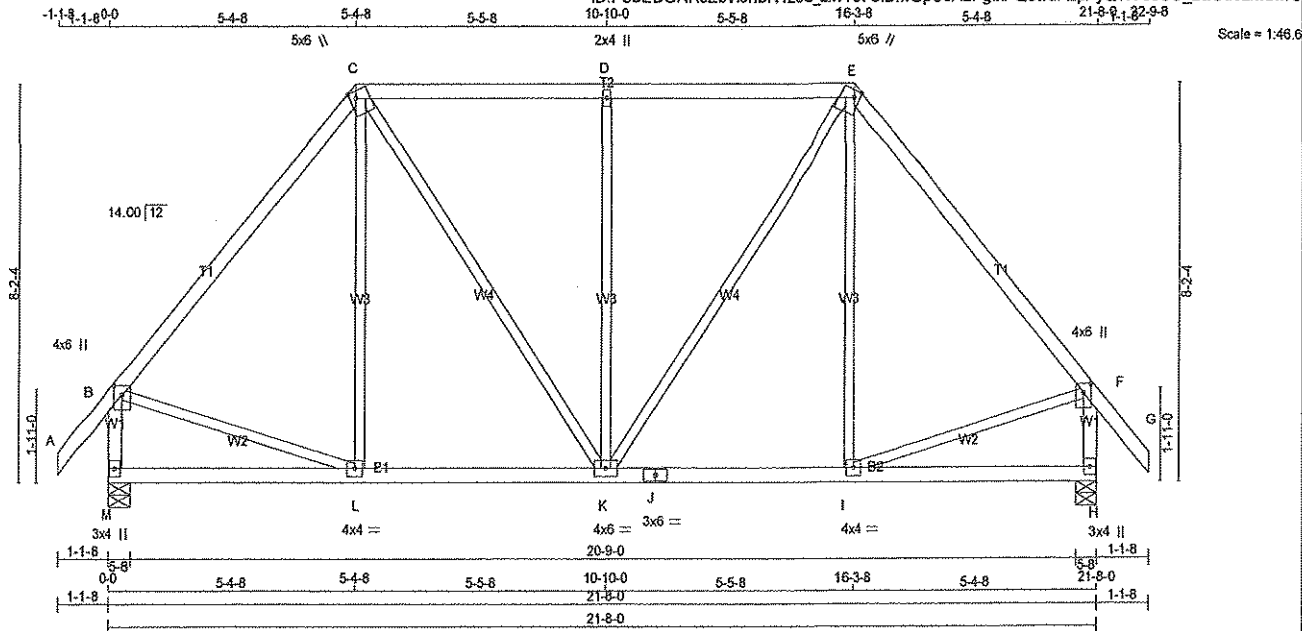
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T36	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbv9nbN1z58_zMY8t-8IDmGpC6A2PgfPQ3wuFzFyQW75tr9C_zagdezMUMG



TOTAL WEIGHT = 106 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1237	0	1237	0	0	5-8	5-8
H	1237	0	1237	0	0	5-8	5-8
UNFACTORED REACTIONS							
1ST CASE							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1014	562 / 0	228 / 0	0 / 0	0 / 0	224 / 0	0 / 0
H	1014	562 / 0	228 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED	MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED
	FORCE	VERT. LOAD	MAX. UNBRACED		FORCE	MAX. UNBRACED	
	(LBS)	(PLF)	LENGTH		(LBS)	LENGTH	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 36	-77.4	-77.4	0.09 (1)	10.00	L-C	0 / 209
B-C	-924 / 0	-77.4	-77.4	0.46 (1)	5.82	C-K	0 / 395
C-D	-824 / 0	-77.4	-77.4	0.41 (1)	6.09	K-D	-512 / 0
D-E	-824 / 0	-77.4	-77.4	0.41 (1)	6.09	K-E	0 / 395
E-F	-924 / 0	-77.4	-77.4	0.46 (1)	5.82	L-E	0 / 209
F-G	0 / 36	-77.4	-77.4	0.09 (1)	10.00	B-L	0 / 623
M-B	-1176 / 0	0.0	0.0	0.13 (1)	7.35	I-F	0 / 623
H-F	-1176 / 0	0.0	0.0	0.13 (1)	7.35		
M-L	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
L-K	0 / 599	-28.0	-28.0	0.28 (2)	10.00		
K-J	0 / 599	-28.0	-28.0	0.28 (2)	10.00		
J-I	0 / 599	-28.0	-28.0	0.28 (2)	10.00		
I-H	0 / 0	-28.0	-28.0	0.21 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.46 (B-C:1), BC=0.28 (K-L:2), WB=0.67 (D-K:1), SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (L) (INPUT = 0.90)
JSI METAL= 0.22 (F) (INPUT = 1.00)



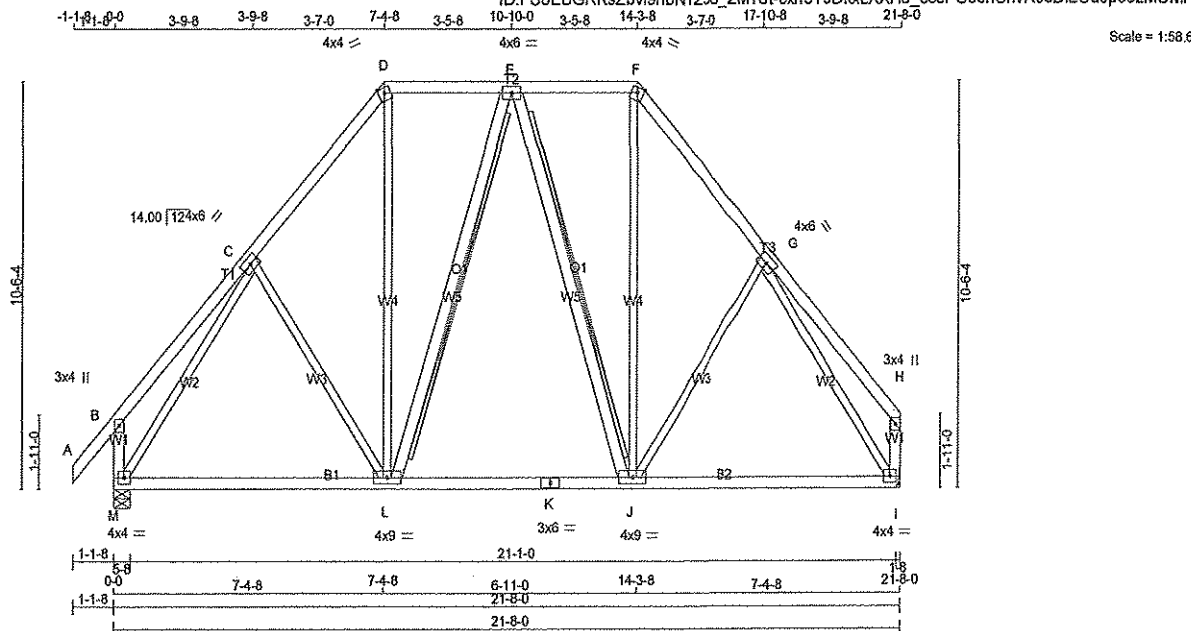
DRWG NO. TAM19700-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T37	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbvi9nbN1z58_zMY8t-cxn9T9DkxLXXH5_ccePU50nCnvRJCdILCdJp95zMUMF



TOTAL WEIGHT = 2 X 128 = 252 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
L - E	2x4	DRY	No.2	SPF	
E - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C, E, G							
C	TMWW-t	MT20	4.0	6.0			
D	TTVV-m	MT20	4.0	4.0	Edge		
F	TTVV-m	MT20	4.0	4.0	Edge		
H	TMV+p	MT20	3.0	4.0			
I	BMVW-t	MT20	4.0	4.0			
J	BMVW-t	MT20	4.0	9.0			
K	BS-t	MT20	3.0	6.0			
L	BMVW-t	MT20	4.0	9.0			
M	BMVW-t	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1237	0	1237	0
I	1141	0	1141	0

UNFACTORED REACTIONS

	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
M	1014	562 / 0	228 / 0	0 / 0
I	949	505 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-L, E-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM TO	CSI (LC)	UNBRAC	FR-TO		CSI (LC)	UNBRAC
A-B	0 / 36	-77.4	-77.4	0.09 (1)	10.00	C-L	-69 / 58	0.06 (1)
B-C	0 / 25	-77.4	-77.4	0.17 (1)	10.00	L-D	0 / 431	0.10 (1)
C-D	-884 / 0	-77.4	-77.4	0.14 (1)	6.25	E-E	-165 / 0	0.14 (1)
D-E	-561 / 0	-77.4	-77.4	0.11 (1)	6.25	E-J	-165 / 0	0.14 (1)
E-F	-561 / 0	-77.4	-77.4	0.11 (1)	6.25	J-F	0 / 431	0.10 (1)
F-G	-884 / 0	-77.4	-77.4	0.14 (1)	6.25	J-G	-69 / 58	0.06 (1)
G-H	0 / 25	-77.4	-77.4	0.16 (1)	10.00	M-C	-1119 / 0	0.97 (1)
M-B	-204 / 0	0.0	0.0	0.02 (1)	7.81	G-I	-1119 / 0	0.97 (1)
I-H	-108 / 0	0.0	0.0	0.01 (1)	7.81			
M-L	0 / 595	-28.0	-28.0	0.40 (2)	10.00			
L-K	0 / 612	-28.0	-28.0	0.41 (2)	10.00			
K-J	0 / 612	-28.0	-28.0	0.41 (2)	10.00			
J-I	0 / 595	-28.0	-28.0	0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.17 (B-C:1), BC=0.41 (J-L:2), WB=0.97 (C-M:1), SSI=0.17 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

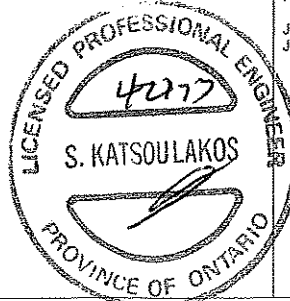
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1656

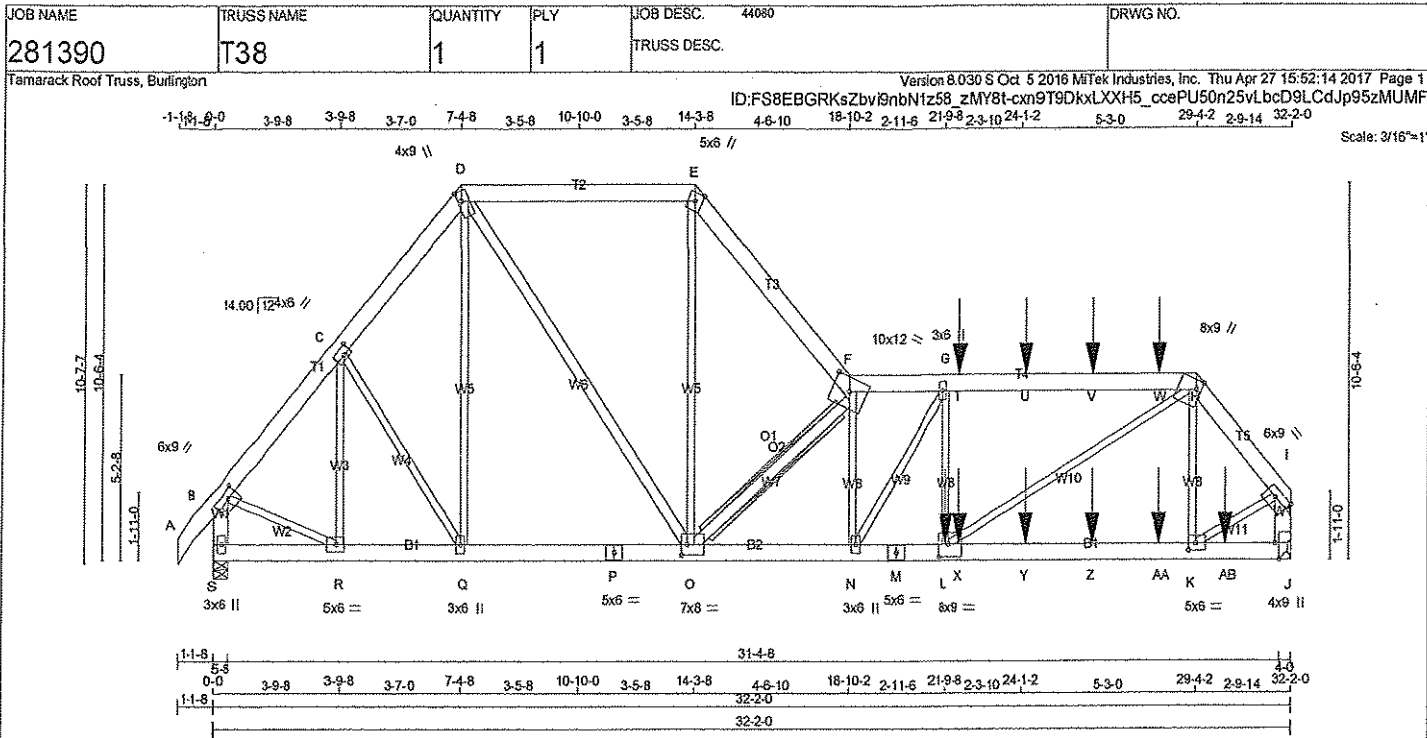
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.28 (G) (INPUT = 1.00)



DRWG NO. TAM19701-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY No.2	SPF
E - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - I	2x6	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
J - I	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - F	2x4	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	3.50
C	TMVW-t	MT20	4.0	6.0	2.50	2.75
D	TTWW+m	MT20	4.0	9.0	3.25	1.25
E	TTW+m	MT20	5.0	6.0	Edge	
F	TTWW-m	MT20	10.0	12.0	4.50	6.00
G	TMVW-t	MT20	3.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.00	1.75
I	TMVW-t	MT20	6.0	9.0	Edge	5.50
J	BMV1-t	MT20	4.0	9.0	Edge	1.50
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	8.0	9.0	4.25	4.00
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	3.0	6.0		
O	BMVW-t	MT20	7.0	8.0	3.50	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	3.0	6.0		
R	BMVW-t	MT20	5.0	6.0		
S	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 93.1 lbs FACTORED DOWN AT 22-2-12, 93.1 lbs FACTORED DOWN AT 24-2-12, AND 93.1 lbs FACTORED DOWN AT 26-2-12, AND 93.1 lbs FACTORED DOWN AT 28-2-12 ON TOP CHORD, AND 1458.0 lbs FACTORED DOWN AT 21-9-8, 70.9 lbs FACTORED DOWN AT 22-2-12, 70.9 lbs FACTORED DOWN AT 24-2-12, 70.9 lbs FACTORED DOWN AT 26-2-12, AND 70.9 lbs FACTORED DOWN AT 28-2-12, AND 70.9 lbs FACTORED DOWN AT 30-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
S	2383	0	5-8	5-8
J	3140	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
S	1961	1074 / 0	449 / 0	0 / 0	0 / 0	436 / 0	0 / 0	
J	2618	1379 / 0	637 / 0	0 / 0	0 / 0	602 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 I-BRACE AT F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 38	-77.4 -77.4	0.05 (1)	R-C	-412 / 0	0.23 (1)	
B-C	-2112 / 0	-77.4 -77.4	0.10 (1)	C-Q	0 / 45	0.01 (2)	
C-D	-2198 / 0	-77.4 -77.4	0.10 (1)	Q-D	0 / 247	0.06 (2)	
D-E	-2184 / 0	-77.4 -77.4	0.39 (1)	O-E	0 / 2079	0.51 (1)	
E-F	-3322 / 0	-77.4 -77.4	0.24 (1)	O-F	-4251 / 0	0.92 (1)	
F-G	-5162 / 0	-77.4 -77.4	0.51 (1)	N-F	0 / 263	0.07 (1)	
G-T	-5307 / 0	-77.4 -77.4	0.79 (1)	L-H	0 / 4053	1.00 (1)	
T-U	-5307 / 0	-77.4 -77.4	0.79 (1)	K-H	-460 / 104	0.17 (1)	
U-V	-5307 / 0	-77.4 -77.4	0.79 (1)	B-R	0 / 1470	0.36 (1)	
V-W	-5307 / 0	-77.4 -77.4	0.79 (1)	K-I	0 / 2060	0.51 (1)	
W-H	-5307 / 0	-77.4 -77.4	0.79 (1)	L-G	-451 / 127	0.16 (1)	
H-I	-2910 / 0	-77.4 -77.4	0.10 (1)	N-G	-279 / 0	0.14 (1)	
S-B	-2328 / 0	0.0 0.0	0.17 (1)	D-O	0 / 1380	0.24 (1)	
J-I	-3191 / 0	0.0 0.0	0.24 (1)				
S-R	0 / 0	-28.0 -28.0	0.06 (2)				
R-Q	0 / 1387	-28.0 -28.0	0.25 (1)				
Q-P	0 / 1407	-28.0 -28.0	0.26 (2)				
P-O	0 / 1407	-28.0 -28.0	0.26 (2)				
O-N	0 / 6159	-28.0 -28.0	0.78 (1)				
N-M	0 / 5307	-28.0 -28.0	0.78 (1)				
M-L	0 / 5307	-28.0 -28.0	0.78 (1)				
L-X	0 / 1864	-28.0 -28.0	0.49 (2)				
X-Y	0 / 1864	-28.0 -28.0	0.49 (2)				
Y-Z	0 / 1864	-28.0 -28.0	0.49 (2)				
Z-AA	0 / 1864	-28.0 -28.0	0.49 (2)				
AA-K	0 / 1864	-28.0 -28.0	0.49 (2)				
K-AB	0 / 0	-28.0 -28.0	0.30 (2)				
AB-J	0 / 0	-28.0 -28.0	0.30 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.07")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.23")
ALLOWABLE DEFL. (TL) = $L/360$ (1.07")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.38")

CSI: TC=0.79 (G-H-1), BC=0.78 (L-N-1), WB=1.00 (H-L-1), SSI=0.37 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

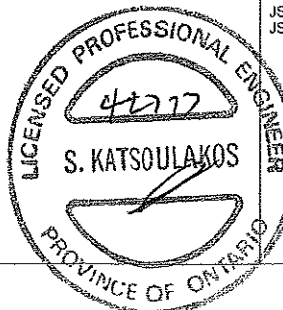
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.86 (M) (INPUT = 1.00)



DWG NO. TAM (9702-17)
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	T38	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

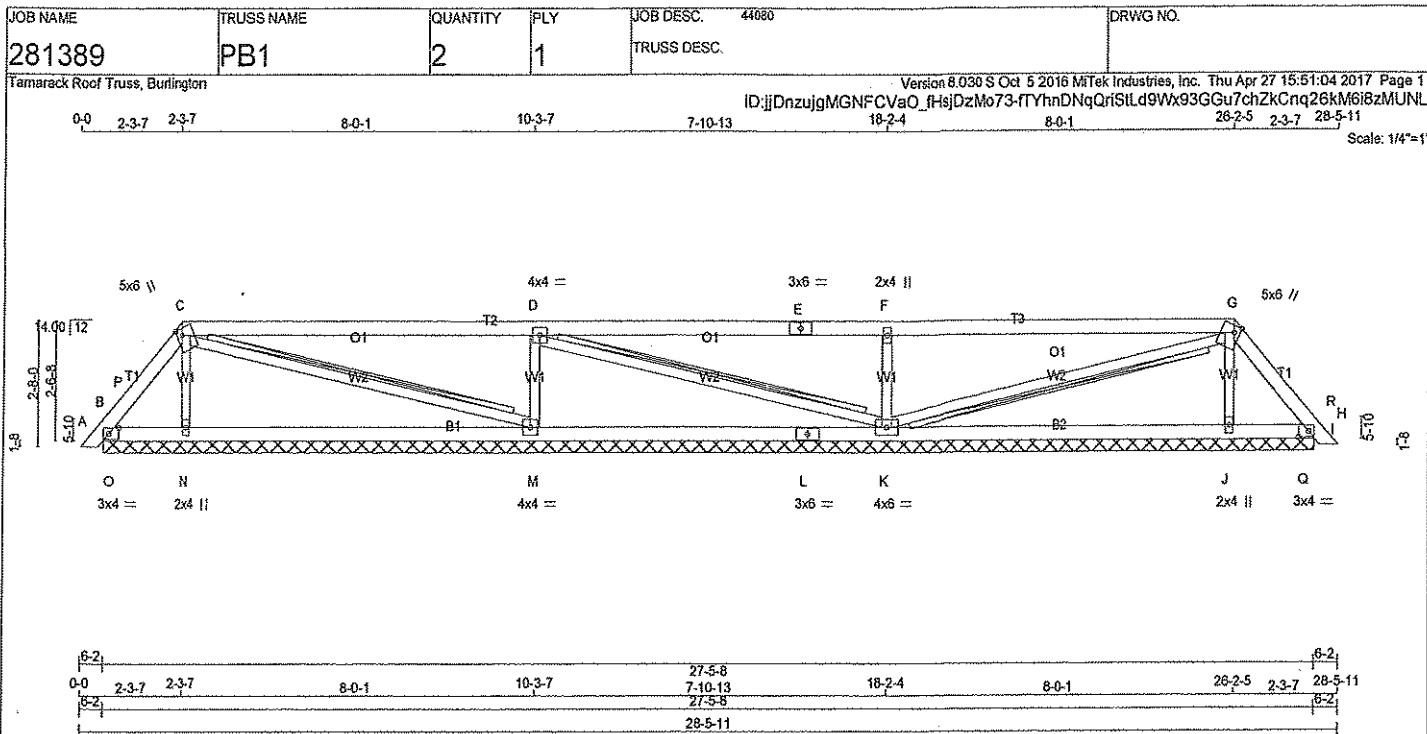
Version 8.030 8 Oct 5 2016 MiTek Industries, Inc. Thu Apr 27 15:52:14 2017 Page 2

ID:FS8EBGRKaZbv9nbN1z58 zMY8t-cxn9T9DkxLXXH5 ccePU50n25vLbcD9LCdJp95zMUMF

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	21-8-8	-1458	-1458	---	FRONT	VERT	TOTAL
T	22-2-12	-93	-93	---	FRONT	VERT	TOTAL
U	24-2-12	-93	-93	---	FRONT	VERT	TOTAL
V	26-2-12	-93	-93	---	FRONT	VERT	TOTAL
W	28-2-12	-93	-93	---	FRONT	VERT	TOTAL
X	22-2-12	-41	-71	---	FRONT	VERT	TOTAL
Y	24-2-12	-41	-71	---	FRONT	VERT	TOTAL
Z	26-2-12	-41	-71	---	FRONT	VERT	TOTAL
AA	28-2-12	-41	-71	---	FRONT	VERT	TOTAL
AB	30-2-12	-41	-71	---	FRONT	VERT	TOTAL



P622
 DWG NO. TAM 19702-17
 STRUCTURAL
 COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMB1-I	MT20	3.0	4.0	1.50	2.50
J	BMWW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMWW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
B	124	0	124	0
N	448	0	459	0
M	899	0	899	0
K	917	0	917	0
J	455	0	462	0
H	112	0	112	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	57	103/0	0/-35	0/0	0/0	0/-10	0/0
N	423	145/0	158/0	0/0	0/0	122/0	0/0
M	738	408/0	168/0	0/0	0/0	164/0	0/0
K	751	417/0	168/0	0/0	0/0	166/0	0/0
J	428	149/0	158/0	0/0	0/0	123/0	0/0
H	49	96/0	0/-35	0/0	0/0	0/-11	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-M, D-K, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/10	-77.4	-77.4 0.01 (1)	10.00	N-C	-228/0	0.04 (1)
B-P	0/185	-77.4	-77.4 0.08 (2)	10.00	C-M	-18/6	0.02 (1)
P-C	-91/0	-77.4	-77.4 0.05 (2)	6.25	M-D	-669/0	0.11 (1)
C-D	-20/4	-77.4	-77.4 0.67 (1)	6.25	D-K	-26/0	0.03 (1)
D-E	0/6	-77.4	-77.4 0.67 (1)	10.00	K-F	-676/0	0.11 (1)
E-F	0/6	-77.4	-77.4 0.67 (1)	10.00	F-G	-34/4	0.04 (1)
F-G	0/7	-77.4	-77.4 0.67 (1)	10.00	G-H	-235/0	0.04 (1)
G-R	-77/0	-77.4	-77.4 0.05 (2)	6.25	H-I	-261/0	0.00 (1)
R-H	0/187	-77.4	-77.4 0.08 (2)	10.00	I-J	-262/0	0.00 (1)
H-I	0/10	-77.4	-77.4 0.01 (1)	10.00			
B-O	-9/49	-28.0	-28.0 0.05 (1)	10.00			
O-N	-9/49	-28.0	-28.0 0.29 (2)	10.00			
N-M	-9/38	-28.0	-28.0 0.36 (2)	10.00			
M-L	-4/21	-28.0	-28.0 0.36 (2)	10.00			
L-K	-4/21	-28.0	-28.0 0.36 (2)	10.00			
K-J	-10/29	-28.0	-28.0 0.36 (3)	6.25			
J-Q	-9/40	-28.0	-28.0 0.29 (2)	10.00			
Q-H	-9/40	-28.0	-28.0 0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.67 (C-D:1), BC=0.36 (M-N:2), WB=0.11 (F-K:1), SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

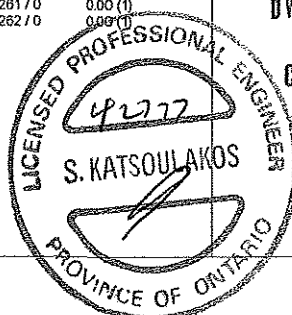
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (K) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)

DRWG NO. TAM 1967B-17
STRUCTURAL
COMPONENT ONLY

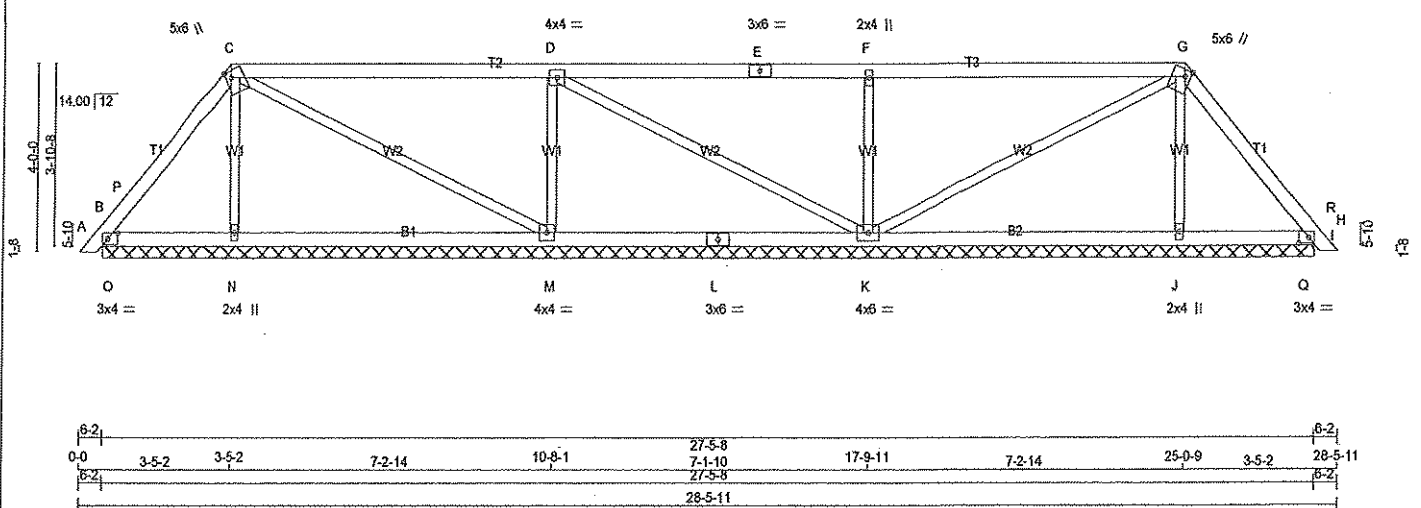
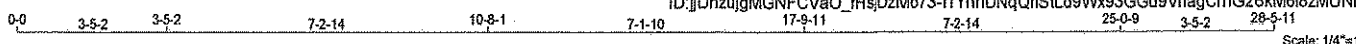


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	PB2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0 1.50 2.50
C	TTWW+m	MT20	5.0	6.0 1.75 1.50
D	TMWW-I	MT20	4.0	4.0
E	TS-I	MT20	3.0	6.0
F	TMWW+w	MT20	2.0	4.0
G	TTWW+m	MT20	5.0	6.0 1.75 1.50
H	TMB1-I	MT20	3.0	4.0 1.50 2.50
J	BMWWI+w	MT20	2.0	4.0
K	BMWWWWI-I	MT20	4.0	6.0
L	BS-I	MT20	3.0	6.0
M	BMWWI-I	MT20	4.0	4.0
N	BMWWI+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	255	0	255	0	0	27-5-8 (13-6-27-5-8	
N	393	0	393	0	0	27-5-8 (13-6-27-5-8	
M	801	0	801	0	0	27-5-8 (13-6-27-5-8	
K	877	0	877	0	0	27-5-8 (13-6-27-5-8	
J	400	0	400	0	0	27-5-8 (13-6-27-5-8	
H	229	0	229	0	0	27-5-8 (13-6-27-5-8	

UNFACTORED REACTIONS							
1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
JT COMBINED							
B	178	148/0	6/0	0/0	0/0	23/0	0/0
N	365	133/0	129/0	0/0	0/0	103/0	0/0
M	661	360/0	153/0	0/0	0/0	148/0	0/0
K	712	405/0	153/0	0/0	0/0	154/0	0/0
J	370	137/0	129/0	0/0	0/0	104/0	0/0
H	160	133/0	6/0	0/0	0/0	21/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0/10	-77.4	-77.4 0.01 (1)	10.00	N-C	-202/0	0.05 (1)
B-P	0/63	-77.4	-77.4 0.07 (1)	10.00	C-M	-36/0	0.05 (1)
P-C	-140/0	-77.4	-77.4 0.07 (1)	6.25	M-D	-582/0	0.14 (1)
C-D	-45/0	-77.4	-77.4 0.55 (1)	6.25	D-K	-64/0	0.09 (1)
D-E	0/13	-77.4	-77.4 0.55 (1)	10.00	K-F	-611/0	0.14 (1)
E-F	0/13	-77.4	-77.4 0.55 (1)	10.00	F-G	-77/0	0.10 (1)
F-G	0/13	-77.4	-77.4 0.55 (1)	10.00	G-H	-209/0	0.05 (1)
G-R	-108/0	-77.4	-77.4 0.07 (1)	6.25	H-I	-326/0	0.00 (1)
R-H	0/83	-77.4	-77.4 0.07 (1)	10.00			
H-I	0/10	-77.4	-77.4 0.01 (1)	10.00			
B-O	0/83	-28.0	-28.0 0.09 (1)	10.00			
O-N	0/83	-28.0	-28.0 0.23 (2)	10.00			
N-M	0/77	-28.0	-28.0 0.30 (2)	10.00			
M-L	0/45	-28.0	-28.0 0.30 (2)	10.00			
L-K	0/45	-28.0	-28.0 0.30 (2)	10.00			
K-J	0/55	-28.0	-28.0 0.30 (2)	10.00			
J-Q	0/62	-28.0	-28.0 0.23 (2)	10.00			
Q-H	0/62	-28.0	-28.0 0.09 (1)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
DL	=	3.0 PSF	
BOT CH.	LL	=	10.5 PSF
DL	=	7.0 PSF	
TOTAL LOAD = 43.8 PSF			
SPACING = 24.0 IN. C/C			

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55 (C-D:1), BC=0.30 (M-N:2), WB=0.14 (F-K:1), SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

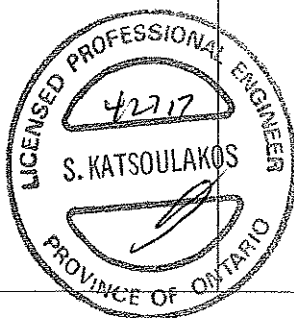
NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

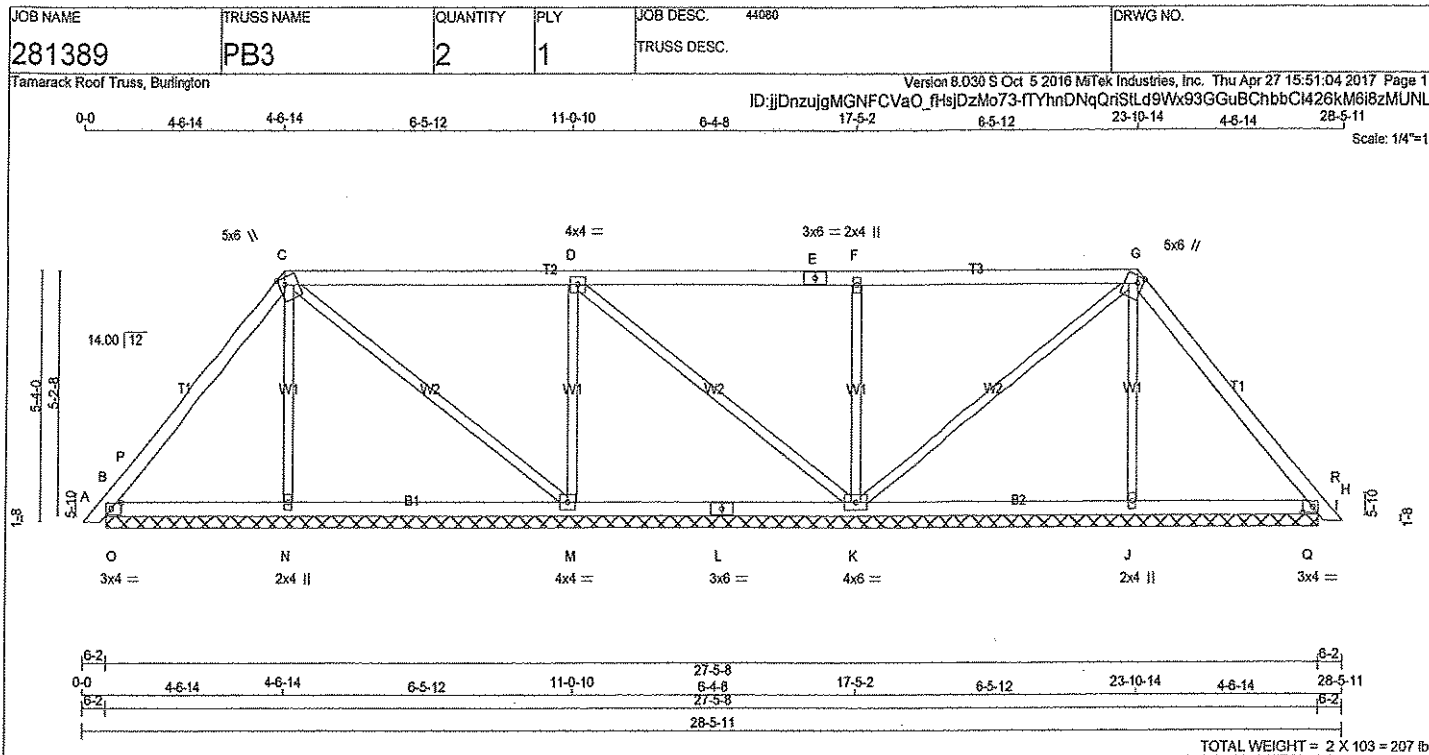
JSI GRIP= 0.68 (K) (INPUT = 0.90)

JSI METAL= 0.22 (L) (INPUT = 1.00)



DRWG NO. TAM 19679-17

STRUCTURAL COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMWW+m	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMB1-I	MT20	3.0	4.0	1.50	2.50
J	BMWW1+w	MT20	2.0	4.0		
K	BMWWWW1-I	MT20	4.0	6.0		
L	BS-I	MT20	3.0	6.0		
M	BMWW1-I	MT20	4.0	4.0		
N	BMWW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG	REQRD BRG
B	372	0	372	0	27-5-8 (13-6-27-5-8)	27-5-8 (13-6-27-5-8)
N	349	0	350	0	27-5-8 (13-6-27-5-8)	27-5-8 (13-6-27-5-8)
M	690	0	690	0	27-5-8 (13-6-27-5-8)	27-5-8 (13-6-27-5-8)
K	881	0	881	0	27-5-8 (13-6-27-5-8)	27-5-8 (13-6-27-5-8)
J	351	0	351	0	27-5-8 (13-6-27-5-8)	27-5-8 (13-6-27-5-8)
H	331	0	331	0	27-5-8 (13-6-27-5-8)	27-5-8 (13-6-27-5-8)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	279	197/0	34/0	0/0	0/0	48/0	0/0
N	327	116/0	117/0	0/0	0/0	93/0	0/0
M	573	306/0	137/0	0/0	0/0	131/0	0/0
K	689	408/0	137/0	0/0	0/0	144/0	0/0
J	328	117/0	117/0	0/0	0/0	93/0	0/0
H	251	172/0	34/0	0/0	0/0	45/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FR-TO)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FR-TO)
A-B	0/10	-77.4	-77.4	0.01 (1)	10.00	N-C	-156/0	0.06 (1)	10.00
B-P	-75/104	-77.4	-77.4	0.11 (1)	6.25	C-M	-54/0	0.07 (1)	10.00
P-C	-205/0	-77.4	-77.4	0.18 (1)	6.25	M-D	-480/0	0.19 (1)	10.00
C-D	-78/0	-77.4	-77.4	0.44 (1)	6.25	D-K	-108/0	0.15 (1)	10.00
D-E	-6/8	-77.4	-77.4	0.44 (1)	10.00	K-F	-546/0	0.22 (1)	10.00
E-F	-6/8	-77.4	-77.4	0.44 (1)	10.00	F-G	-118/0	0.16 (1)	10.00
F-G	-6/8	-77.4	-77.4	0.44 (1)	10.00	G-H	-157/0	0.06 (1)	10.00
G-R	-152/0	-77.4	-77.4	0.16 (1)	6.25	Q-P	-513/37	0.00 (1)	10.00
R-H	-69/166	-77.4	-77.4	0.12 (1)	6.25	Q-R	-524/38	0.00 (1)	10.00
H-I	0/10	-77.4	-77.4	0.01 (1)	10.00				
B-O	0/124	-28.0	-28.0	0.14 (1)	10.00				
O-N	0/124	-28.0	-28.0	0.21 (2)	10.00				
N-M	0/121	-28.0	-28.0	0.24 (2)	10.00				
M-L	0/78	-28.0	-28.0	0.24 (2)	10.00				
L-K	0/78	-28.0	-28.0	0.24 (2)	10.00				
K-J	0/87	-28.0	-28.0	0.24 (2)	10.00				
J-Q	0/90	-28.0	-28.0	0.21 (2)	10.00				
Q-H	0/90	-28.0	-28.0	0.14 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TFC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44 (C-D:1), BC=0.24 (M-N:2), WB=0.22 (F-K:1), SSI=0.42 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

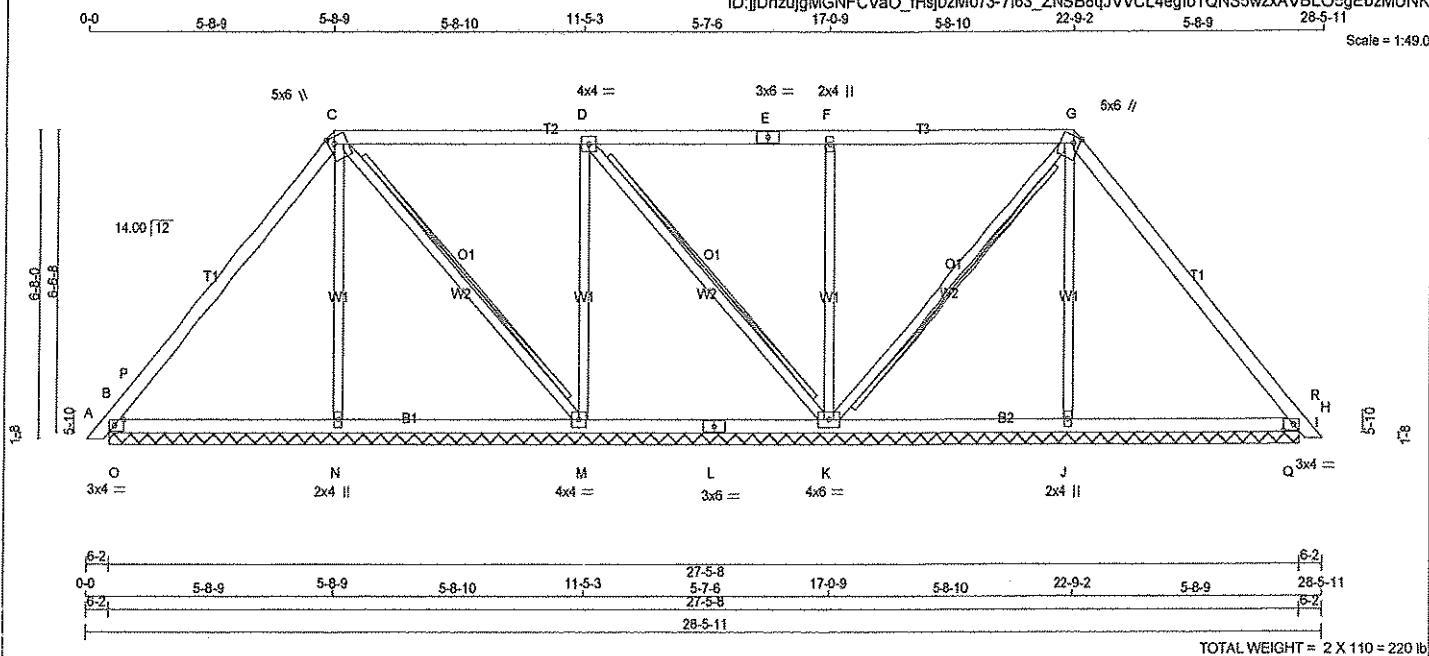
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.67 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



DRWG NO. TAM 1968017
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 110 = 220 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
B - L	2x4	DRY	No.2 SPF
L - H	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMB1-I	MT20	3.0	4.0	1.50	2.50
J	BMW1+w	MT20	2.0	4.0		
K	BMWWW1-I	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	487	0	487	0	27-5-8 (13-6-27-5-8)	
N	314	0	328	0	27-5-8 (13-6-27-5-8)	
M	572	0	572	0	27-5-8 (13-6-27-5-8)	
K	834	0	834	0	27-5-8 (13-6-27-5-8)	
J	312	0	327	0	27-5-8 (13-6-27-5-8)	
H	435	0	435	0	27-5-8 (13-6-27-5-8)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
		COMBINED	SNOW	LIVE	PERM./LIVE	WIND	DEAD	SOIL
B		375	247 / 0	58 / 0	0 / 0	0 / 0	71 / 0	0 / 0
N		269	99 / 0	113 / 0	0 / 0	0 / 0	88 / 0	0 / 0
M		478	250 / 0	118 / 0	0 / 0	0 / 0	111 / 0	0 / 0
K		656	408 / 0	117 / 0	0 / 0	0 / 0	131 / 0	0 / 0
J		298	97 / 0	113 / 0	0 / 0	0 / 0	88 / 0	0 / 0
H		340	215 / 0	58 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-M, D-K, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)
FR-TO				
A-B	0/10	-77.4 -77.4 0.01 (1)	10.00	N-C -102/14 0.07 (1)
B-P	-214/236	-77.4 -77.4 0.20 (1)	6.25	C-M -65/0 0.08 (1)
P-C	-280/0	-77.4 -77.4 0.27 (1)	6.25	M-D -377/0 0.26 (1)
C-D	-124/0	-77.4 -77.4 0.34 (1)	6.25	D-K -140/0 0.16 (1)
D-E	-32/0	-77.4 -77.4 0.34 (1)	6.25	K-F -481/0 0.34 (1)
E-F	-32/0	-77.4 -77.4 0.34 (1)	6.25	F-G -139/0 0.16 (1)
F-G	-32/0	-77.4 -77.4 0.34 (1)	6.25	G-H -99/14 0.07 (1)
G-R	-213/0	-77.4 -77.4 0.27 (1)	6.25	H-I -784/160 0.00 (1)
R-H	-207/315	-77.4 -77.4 0.21 (1)	6.25	I-J -869/158 0.00 (1)
H-I	0/10	-77.4 -77.4 0.01 (1)	10.00	
B-O	0/170	-28.0 -28.0 0.22 (1)	10.00	
O-N	0/170	-28.0 -28.0 0.23 (1)	10.00	
N-M	0/169	-28.0 -28.0 0.23 (2)	10.00	
M-L	0/125	-28.0 -28.0 0.18 (2)	10.00	
L-K	0/125	-28.0 -28.0 0.18 (2)	10.00	
K-J	0/125	-28.0 -28.0 0.22 (2)	10.00	
J-Q	0/127	-28.0 -28.0 0.23 (1)	10.00	
Q-H	0/127	-28.0 -28.0 0.22 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1), BC=0.23 (N-O:1), WB=0.34 (F-K:1), SSI=0.65 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

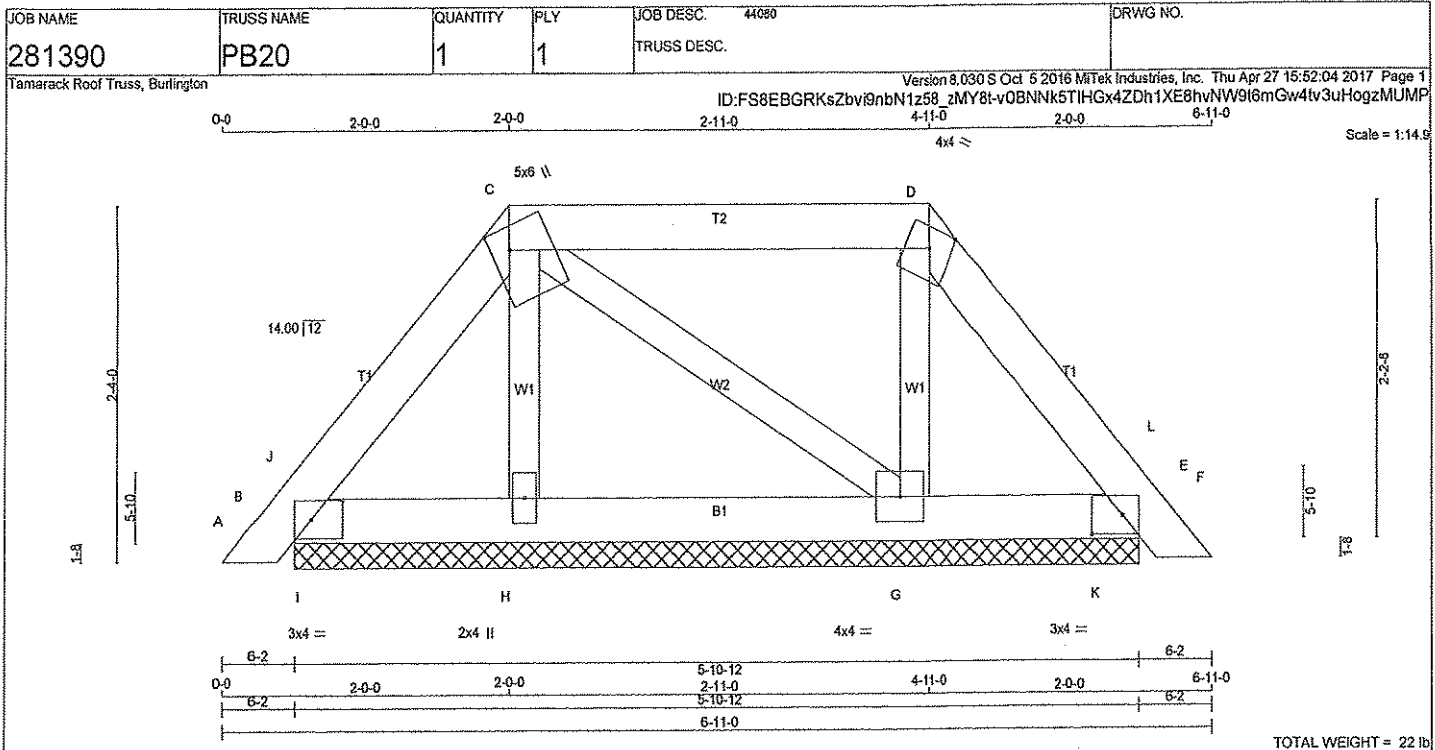
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.18 (L) (INPUT = 1.00)



DRWG NO. TAM 19681-17
STRUCTURAL COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TTWW-m	MT20	4.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0	1.50	2.50
G	BMWW1-I	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	160	0	160	0	5-10-12	5-10-12
E	146	0	146	0	5-10-12	5-10-12
H	174	0	174	0	5-10-12	5-10-12
G	203	0	203	0	5-10-12	5-10-12

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	116	88/0	10/0	0/0	0/0	18/0	0/0
E	107	80/0	10/0	0/0	0/0	17/0	0/0
H	158	63/0	52/0	0/0	0/0	43/0	0/0
G	178	81/0	52/0	0/0	0/0	45/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/10	-77.4 -77.4	0.01 (1)	10.00	H-C	-95/0	0.01 (1)
B-J	-60/0	-77.4 -77.4	0.01 (1)	6.25	C-G	-14/0	0.00 (1)
J-C	-77/0	-77.4 -77.4	0.02 (1)	6.25	C-D	-116/0	0.02 (1)
C-D	-30/0	-77.4 -77.4	0.11 (1)	6.25	I-J	-76/0	0.00 (1)
D-L	-30/0	-77.4 -77.4	0.02 (1)	6.25	K-L	-80/0	0.00 (1)
L-E	-40/0	-77.4 -77.4	0.01 (1)	6.25			
E-F	0/10	-77.4 -77.4	0.01 (1)	10.00			
B-I	0/47	-28.0 -28.0	0.03 (1)	10.00			
I-H	0/47	-28.0 -28.0	0.04 (2)	10.00			
H-G	0/41	-28.0 -28.0	0.04 (2)	10.00			
G-K	0/36	-28.0 -28.0	0.04 (2)	10.00			
K-E	0/36	-28.0 -28.0	0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	43.8	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OSC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (C-D:1), BC=0.04 (H-I:2), WB=0.02 (D-G:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

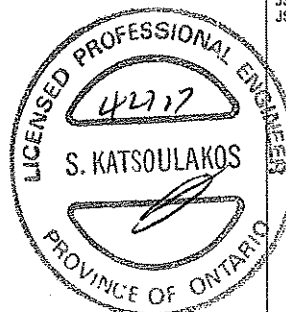
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.50)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DRWG NO. TAM 19703-17
STRUCTURAL
COMPONENT ONLY

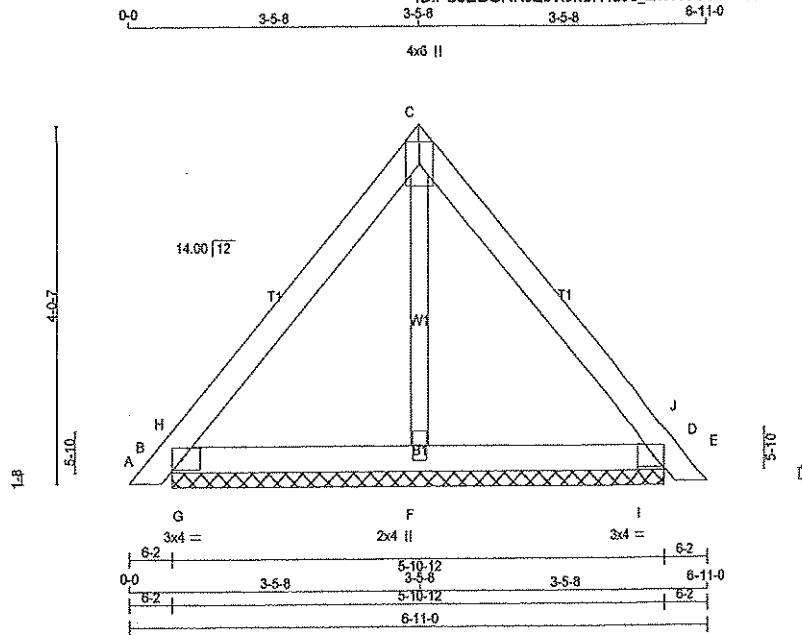
JOB NAME 281390	TRUSS NAME PB21	QUANTITY 1	PLY 1	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbv9nbN1258_zMY8t-v08NNk5TIHGx4ZDh1XE8hvNWR52GwEly3uHogzMUMP

Scale = 1:25.5



TOTAL WEIGHT = 21 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-H	MT20	3.0	4.0	1.50 2.50
C	TTW+P	MT20	4.0	6.0	
D	TMB1-H	MT20	3.0	4.0	1.50 2.50
F	BMW1+W	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	247	0	247	0	5-10-12	5-10-12
D	247	0	247	0	5-10-12	5-10-12
F	189	0	194	0	5-10-12	5-10-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
B	160	125/0	29/0	0/0	0/0	36/0	0/0
D	160	125/0	29/0	0/0	0/0	36/0	0/0
F	179	61/0	66/0	0/0	0/0	52/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/10	-77.4	-77.4 0.01 (1)	10.00	F-C	-36/10	0.01 (1)
B-H	-67/37	-77.4	-77.4 0.05 (1)	6.25	G-H	-285/42	0.00 (1)
H-C	-111/0	-77.4	-77.4 0.08 (1)	6.25	I-J	-285/42	0.00 (1)
C-J	-111/0	-77.4	-77.4 0.08 (1)	6.25			
J-D	-67/37	-77.4	-77.4 0.05 (1)	6.25			
D-E	0/10	-77.4	-77.4 0.01 (1)	10.00			
B-G	0/66	-28.0	-28.0 0.08 (1)	10.00			
G-F	0/66	-28.0	-28.0 0.09 (1)	10.00			
F-I	0/66	-28.0	-28.0 0.09 (1)	10.00			
I-D	0/66	-28.0	-28.0 0.08 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	DL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	43.8	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08 (C-H:1), BC=0.09 (F-G:1), WB=0.01 (C-F:1), SSI=0.23 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.29 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



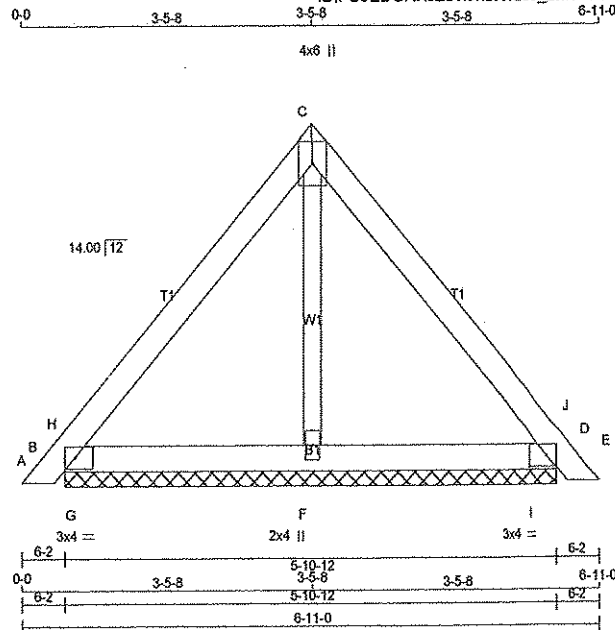
DWG NO. TAM19704-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
281390	PB21Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbvi9nbN1z58_zMY8t-v0BNNk5THGx4ZDh1XE8hvNXGt6jGwltv3uHogzMUMP



Scale = 1:25.5

TOTAL WEIGHT = 2 X 21 = 43 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1 12		TOP
C - E 1 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B - D 1 12		TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0	1.50	2.50
C TTW+p	MT20	4.0	6.0		
D TMB1-I	MT20	3.0	4.0	1.50	2.50
F BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	249	0	249	0
D	249	0	249	0
F	186	0	193	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
B	191	127 / 0	29 / 0	0 / 0	36 / 0	0 / 0
D	191	127 / 0	29 / 0	0 / 0	36 / 0	0 / 0
F	177	59 / 0	66 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 10	-77.4	-77.4	0.01 (1)	10.00	F-C	-34 / 9
B-H	-66 / 28	-77.4	-77.4	0.02 (1)	6.25	G-H	-275 / 41
H-C	-113 / 0	-77.4	-77.4	0.04 (1)	6.25	I-J	-275 / 41
C-J	-113 / 0	-77.4	-77.4	0.04 (1)	6.25		
J-D	-66 / 28	-77.4	-77.4	0.02 (1)	6.25		
D-E	0 / 10	-77.4	-77.4	0.01 (1)	10.00		
B-G	0 / 67	-28.0	-28.0	0.04 (1)	10.00		
G-F	0 / 67	-28.0	-28.0	0.04 (1)	10.00		
F-I	0 / 67	-28.0	-28.0	0.04 (1)	10.00		
I-D	0 / 67	-28.0	-28.0	0.04 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (C-H:1), BC=0.04 (F-G:1), WB=0.00 (C-F:1), SSI=0.11 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687
	822	2284	1656

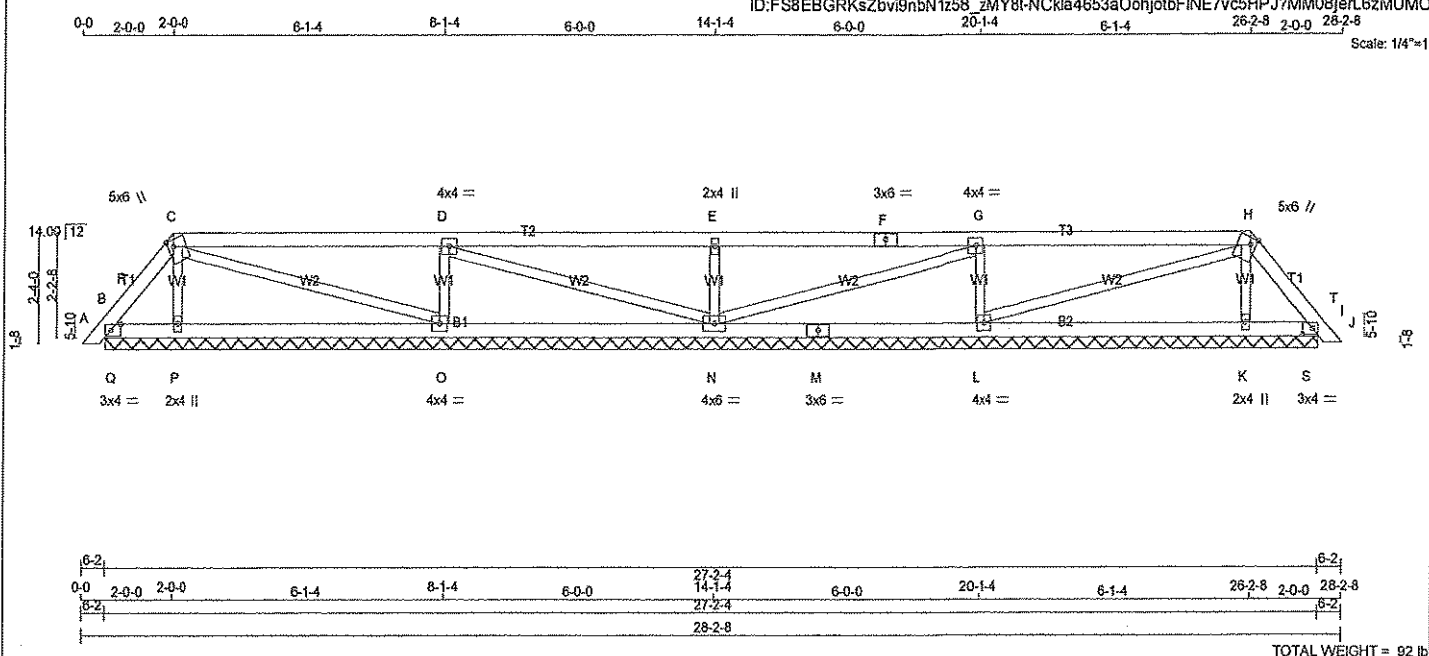
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (D) (INPUT = 0.50)
JSI METAL= 0.03 (B) (INPUT = 1.00)



DWG NO. TAM 19705-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMWW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	1.75	1.50
I	TMB1-I	MT20	3.0	4.0	1.50	2.50
K	BMWW1+w	MT20	2.0	4.0		
L	BMWW1-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWWW1-I	MT20	4.0	6.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMWW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	119	0	119	0	-30	27-2-4 (11-3-27-2-4)
P	333	0	337	0	0	27-2-4 (11-3-27-2-4)
O	703	0	703	0	0	27-2-4 (11-3-27-2-4)
N	615	0	615	0	0	27-2-4 (11-3-27-2-4)
L	703	0	703	0	0	27-2-4 (11-3-27-2-4)
K	333	0	337	0	0	27-2-4 (11-3-27-2-4)
I	119	0	119	0	-30	27-2-4 (11-3-27-2-4)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT I FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	67	87/0	0/-19	0/0	0/0	0/-1	0/0
P	312	110/0	113/0	0/0	0/0	90/0	0/0
O	576	320/0	129/0	0/0	0/0	127/0	0/0
N	512	271/0	124/0	0/0	0/0	118/0	0/0
L	576	320/0	129/0	0/0	0/0	127/0	0/0
K	312	110/0	113/0	0/0	0/0	90/0	0/0
I	67	87/0	0/-19	0/0	0/0	0/-1	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P, O, N, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/10	-77.4	-77.4 0.01 (1)	10.00	P-C	-171/0	0.03 (1)
B-R	0/95	-77.4	-77.4 0.04 (2)	10.00	C-O	-18/3	0.01 (1)
R-C	-73/0	-77.4	-77.4 0.03 (2)	6.25	O-D	-526/0	0.08 (1)
C-D	-13/1	-77.4	-77.4 0.42 (1)	6.25	D-N	-36/0	0.03 (1)
D-E	0/20	-77.4	-77.4 0.42 (1)	10.00	N-E	-428/0	0.08 (1)
E-F	0/20	-77.4	-77.4 0.42 (1)	10.00	N-G	-36/0	0.03 (1)
F-G	0/20	-77.4	-77.4 0.42 (1)	10.00	L-G	-526/0	0.08 (1)
G-H	-13/1	-77.4	-77.4 0.42 (1)	6.25	L-H	-18/3	0.01 (1)
H-T	-73/0	-77.4	-77.4 0.03 (2)	6.25	K-H	-171/0	0.03 (1)
T-I	0/95	-77.4	-77.4 0.04 (2)	10.00	Q-R	-150/0	0.00 (1)
I-J	0/10	-77.4	-77.4 0.01 (1)	10.00	S-T	-150/0	0.00 (1)
B-Q	-3/40	-28.0	-28.0 0.03 (1)	10.00			
Q-P	-3/40	-28.0	-28.0 0.16 (2)	10.00			
P-O	-4/31	-28.0	-28.0 0.21 (2)	10.00			
O-N	-1/14	-28.0	-28.0 0.21 (2)	10.00			
N-M	-1/14	-28.0	-28.0 0.21 (2)	10.00			
M-L	-1/14	-28.0	-28.0 0.21 (2)	10.00			
L-K	-4/31	-28.0	-28.0 0.21 (2)	10.00			
K-S	-3/40	-28.0	-28.0 0.16 (2)	10.00			
S-I	-3/40	-28.0	-28.0 0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	43.8	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42 (D-E:1), BC=0.21 (O-P:2), WB=0.08 (D-O:1), SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

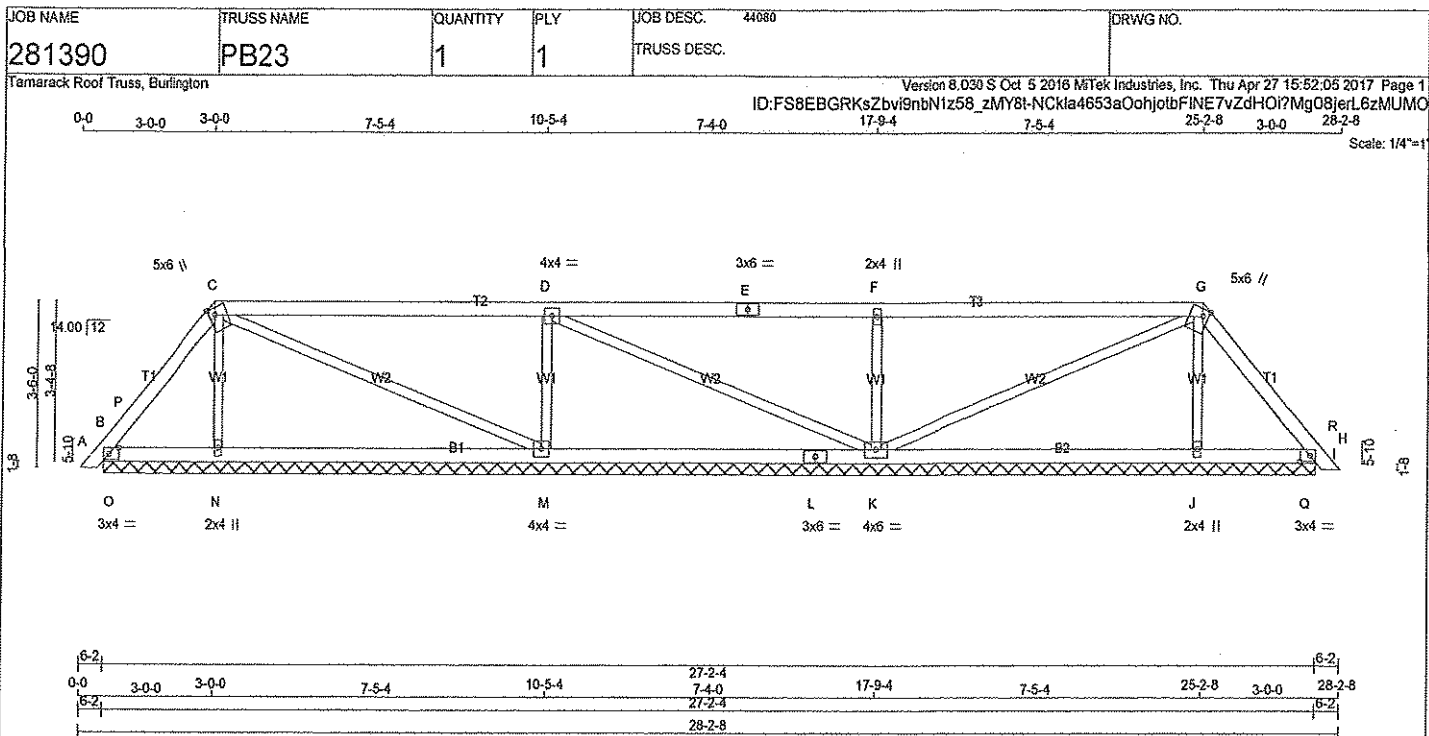
PLATE PLACEMENT TOL = 0.250 inches

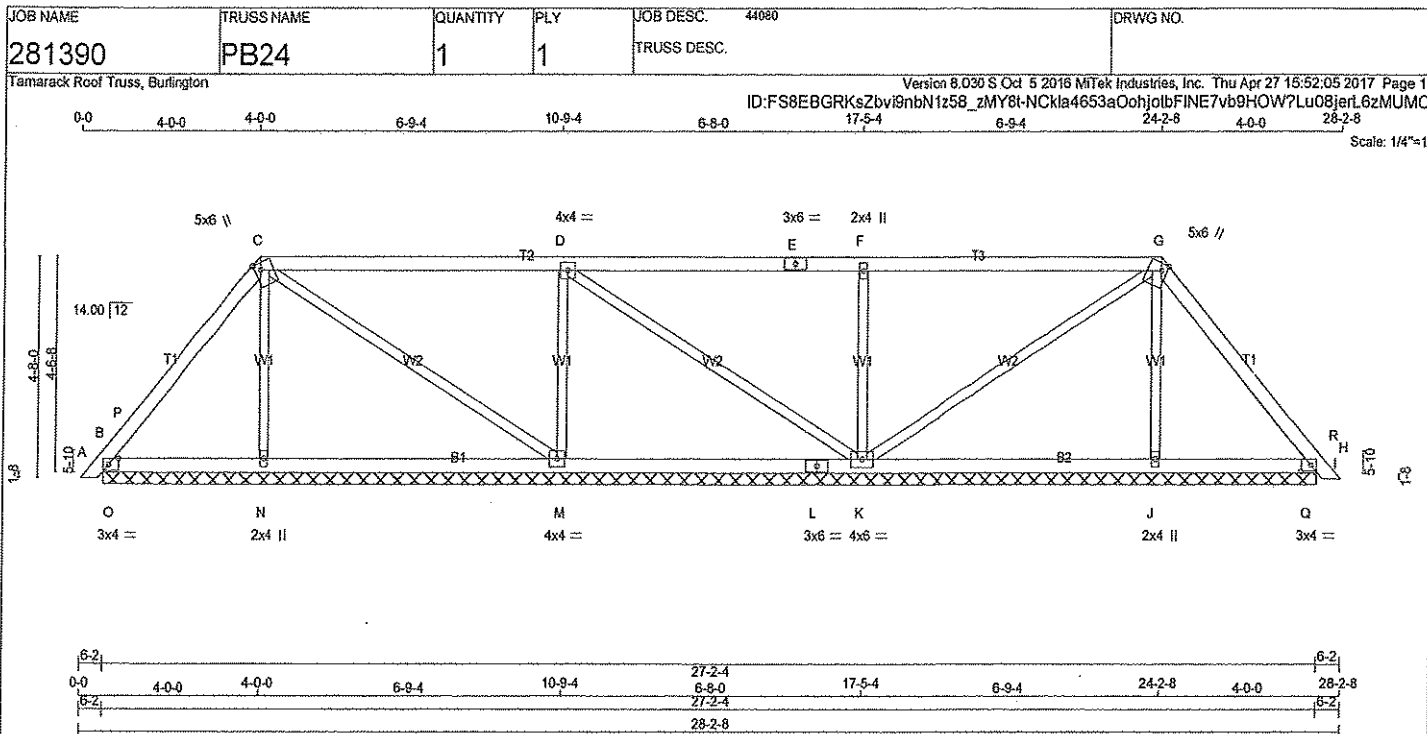
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.75 (N) (INPUT = 0.90)
JSI METAL= 0.14 (M) (INPUT = 1.00)



DWG NO. TAM 19706 - 17
STRUCTURAL COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMB1-I	MT20	3.0	4.0	1.50	2.50
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-I	MT20	4.0	6.0		
L	BS-I	MT20	3.0	6.0		
M	BMWW1-I	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX
B	315	0	315	0	27-2-4 (11-3-27-2-4)
N	366	0	366	0	27-2-4 (11-3-27-2-4)
M	736	0	736	0	27-2-4 (11-3-27-2-4)
K	859	0	859	0	27-2-4 (11-3-27-2-4)
J	370	0	370	0	27-2-4 (11-3-27-2-4)
H	281	0	281	0	27-2-4 (11-3-27-2-4)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED						
B	230	172/0	22/0	0/0	0/0	0/0
N	340	123/0	121/0	0/0	0/0	0/0
M	609	328/0	143/0	0/0	0/0	138/0
K	692	402/0	143/0	0/0	0/0	147/0
J	343	126/0	121/0	0/0	0/0	97/0
H	207	152/0	22/0	0/0	0/0	34/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PLATE BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM TO		FR-TO			
A-B	0/10	-77.4	-77.4 0.01 (1)	N-C	-177/0	0.05 (1)	10.00
B-P	-11/64	-77.4	-77.4 0.08 (1)	C-M	-45/0	0.06 (1)	6.25
P-C	-172/0	-77.4	-77.4 0.11 (1)	M-D	-524/0	0.16 (1)	6.25
C-D	-60/0	-77.4	-77.4 0.48 (1)	D-K	-87/0	0.11 (1)	6.25
D-E	0/13	-77.4	-77.4 0.48 (1)	K-F	-571/0	0.17 (1)	10.00
E-F	0/13	-77.4	-77.4 0.48 (1)	F-G	-99/0	0.13 (1)	10.00
F-G	0/14	-77.4	-77.4 0.48 (1)	G-H	-182/0	0.06 (1)	10.00
G-R	-128/0	-77.4	-77.4 0.11 (1)	H-I	-405/0	0.00 (1)	6.25
R-H	-6/114	-77.4	-77.4 0.09 (1)	Q-R	-414/0	0.00 (1)	10.00
H-I	0/10	-77.4	-77.4 0.01 (1)				
B-O	0/103	-28.0	-28.0 0.11 (1)				10.00
O-N	0/103	-28.0	-28.0 0.21 (2)				10.00
N-M	0/99	-28.0	-28.0 0.26 (2)				10.00
M-L	0/60	-28.0	-28.0 0.26 (2)				10.00
L-K	0/60	-28.0	-28.0 0.26 (2)				10.00
K-J	0/71	-28.0	-28.0 0.26 (2)				10.00
J-Q	0/75	-28.0	-28.0 0.21 (2)				10.00
Q-H	0/75	-28.0	-28.0 0.11 (1)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48 (C-D:1), BC=0.26 (M-N:2), WB=0.17 (F-K:1), SSI=0.33 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (K) (INPUT = 0.90)
JSI METAL= 0.12 (L) (INPUT = 1.00)



DRWG NO. TAM19708-17
STRUCTURAL
COMPONENT ONLY

DWONG.TAM(9709 -17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281390	PB26	1	1	TRUSS DESC.		

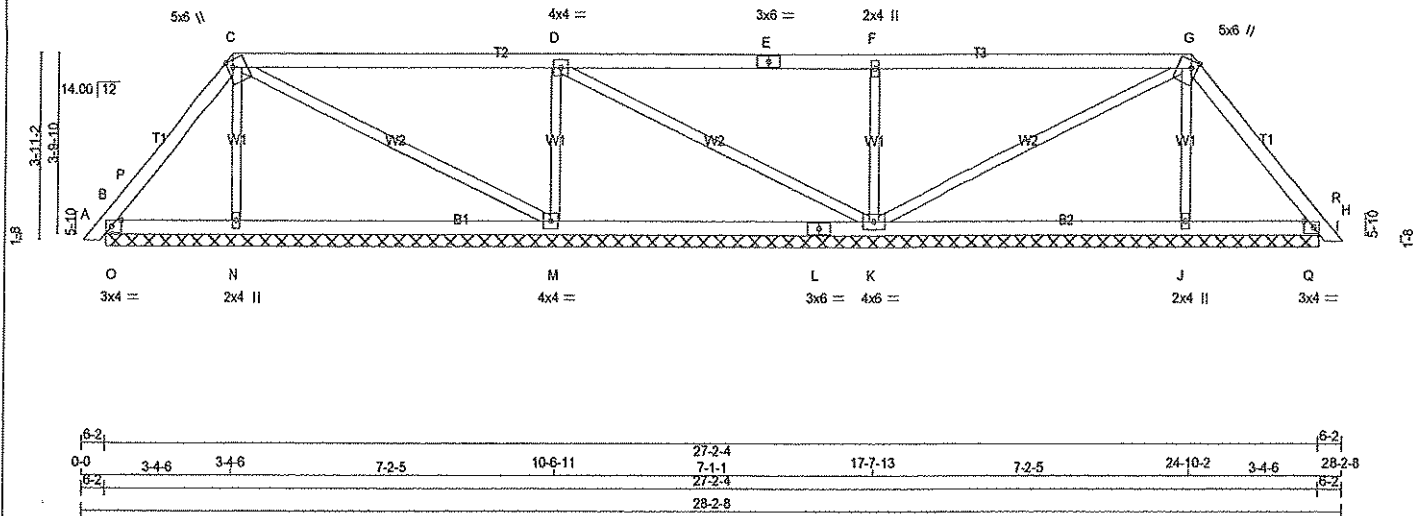
Tamarack Roof Truss, Burlington

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ID:FS8EBGRKsZbv9nbN1z58_zMY8t-rOI7oP7jquWfJtM48yGcmKSlyhEkohAMNNOIYzUMUN

0-0 3-4-6 3-4-6 7-2-5 10-6-11 7-1-1 17-7-13 7-2-5 24-10-2 3-4-6 28-2-8

Scale: 1/4"=1'



TOTAL WEIGHT = 66 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0 1.50 2.50
C	TTWW+m	MT20	5.0	6.0 1.75 1.50
D	TMWW-I	MT20	4.0	4.0
E	TS-I	MT20	3.0	6.0
F	TMWW+w	MT20	2.0	4.0
G	TTWW+m	MT20	5.0	6.0 1.75 1.50
H	TMB1-I	MT20	3.0	4.0 1.50 2.50
J	BMW1+w	MT20	2.0	4.0
K	BMWW1-I	MT20	4.0	6.0
L	BS-I	MT20	3.0	6.0
M	BMWW1-I	MT20	4.0	4.0
N	BMW1+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	249	0	249	0	0	27-2-4	(11-3-27-2-4)	27-2-4	(11-3-27-2-4)
N	390	0	390	0	0	27-2-4	(11-3-27-2-4)	27-2-4	(11-3-27-2-4)
M	796	0	796	0	0	27-2-4	(11-3-27-2-4)	27-2-4	(11-3-27-2-4)
K	869	0	869	0	0	27-2-4	(11-3-27-2-4)	27-2-4	(11-3-27-2-4)
J	398	0	398	0	0	27-2-4	(11-3-27-2-4)	27-2-4	(11-3-27-2-4)
H	224	0	224	0	0	27-2-4	(11-3-27-2-4)	27-2-4	(11-3-27-2-4)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
B	173	145/0	5/0
N	363	132/0	128/0
M	657	358/0	152/0
K	706	401/0	152/0
J	368	136/0	128/0
H	156	131/0	5/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/10	N-C	-201/0
B-F	0/64	C-M	-35/0
P-C	-137/0	M-D	-579/0
C-D	-43/0	D-K	-62/0
D-E	0/12	K-F	-607/0
E-F	0/12	F-G	-74/0
F-G	0/13	G-H	-208/0
G-R	-105/0	H-I	-316/0
R-H	0/78	I-J	-323/0
H-I	0/10	J-K	-323/0
B-O	0/81	K-L	-323/0
O-N	0/81	L-M	-323/0
N-M	0/75	M-L	-323/0
M-L	0/44	L-K	-323/0
L-K	0/44	K-J	-323/0
K-J	0/55	J-I	-323/0
J-I	0/61	I-H	-323/0
I-H	0/61	H-G	-323/0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.54 (C-D:1), BC=0.30 (M-N:2), WB=0.14 (F-K:1), SSI=0.26 (C-D:1)

DOL LUMBER=1.00 HAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.69 (K) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)



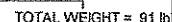
DRWG NO. TAM19710-17
STRUCTURAL
COMPONENT ONLY

TRUSS DESC.

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0-0	2-2-11	2-2-11	7-11-7	10-2-2	7-10-3	18-0-6	7-11-7	25-11-13	2-2-11	28-2-8
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Scale: 1/4" = 1'



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	117	0	117	0	-70	27-2.4	11.3-27-2.4
B	446	0	458	0	0	27-2.4	11.3-27-2.4
M	893	0	893	0	0	27-2.4	11.3-27-2.4
K	911	0	911	0	0	27-2.4	11.3-27-2.4
J	453	0	461	0	0	27-2.4	11.3-27-2.4
H	106	0	106	0	-71	27-2.4	11.3-27-2.4

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JB	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	51	100/0	0/-37	0/0	0/0	0/-12	0/0
N	422	144/0	158/0	0/0	0/0	123/0	0/0
M	734	404/0	187/0	0/0	0/0	168/0	0/0
K	746	414/0	187/0	0/0	0/0	165/0	0/0
J	426	148/0	158/0	0/0	0/0	123/0	0/0
H	44	93/0	0/-37	0/0	0/0	0/-13	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

2x3 DRY SPF No.2 T-BRACE AT C-M, D-K, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/10	-77.4	-77.4	0.01 (1)	10.00	N-C	-226/0	0.04 (1)	
B-P	0/186	-77.4	-77.4	0.08 (2)	10.00	C-M	-18/6	0.02 (1)	
P-C	-89/0	-77.4	-77.4	0.05 (2)	6.25	M-D	-666/0	0.10 (1)	
C-D	-20/4	-77.4	-77.4	0.68 (1)	6.25	D-K	-24/0	0.03 (1)	
D-E	0/6	-77.4	-77.4	0.68 (1)	10.00	K-F	-672/0	0.11 (1)	
E-F	0/6	-77.4	-77.4	0.68 (1)	10.00	K-G	-32/4	0.04 (1)	
F-G	0/6	-77.4	-77.4	0.68 (1)	10.00	J-G	-233/0	0.04 (1)	
G-R	-75/0	-77.4	-77.4	0.05 (2)	6.25	O-P	-260/0	0.00 (1)	
R-H	0/187	-77.4	-77.4	0.08 (2)	10.00	Q-R	-261/0	0.00 (1)	
H-I	0/10	-77.4	-77.4	0.01 (1)	10.00				
B-O	-9/48	-28.0	-28.0	0.05 (1)	10.00				
O-N	-9/48	-28.0	-28.0	0.29 (2)	10.00				
N-M	-10/37	-28.0	-28.0	0.35 (2)	10.00				
M-L	-4/20	-28.0	-28.0	0.35 (2)	10.00				
L-K	-4/20	-28.0	-28.0	0.35 (2)	10.00				
K-J	-10/28	-28.0	-28.0	0.35 (3)	6.25				
J-Q	-9/39	-28.0	-28.0	0.29 (2)	10.00				
Q-H	-9/39	-28.0	-28.0	0.05 (1)	10.00				

SPECIFIED LOADS:

SPECIFIED LOADS:			
TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD		= 43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.66 (C-D:1), BC=0.35 (M-N:2), WB=0.11 (F-K:1), SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

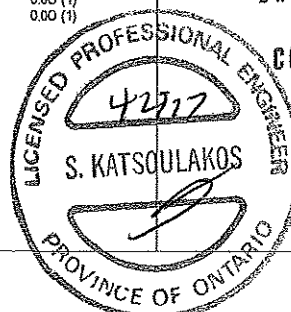
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX MIN						
MT20	618 354	1667 822	2284 1656			

PLATE PLACEMENT TOL = 0.250 inches

JSI GRIP= 0.74 (K) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)

DWG NO. TAM19711 -17
STRUCTURAL
COMPONENT ONLY



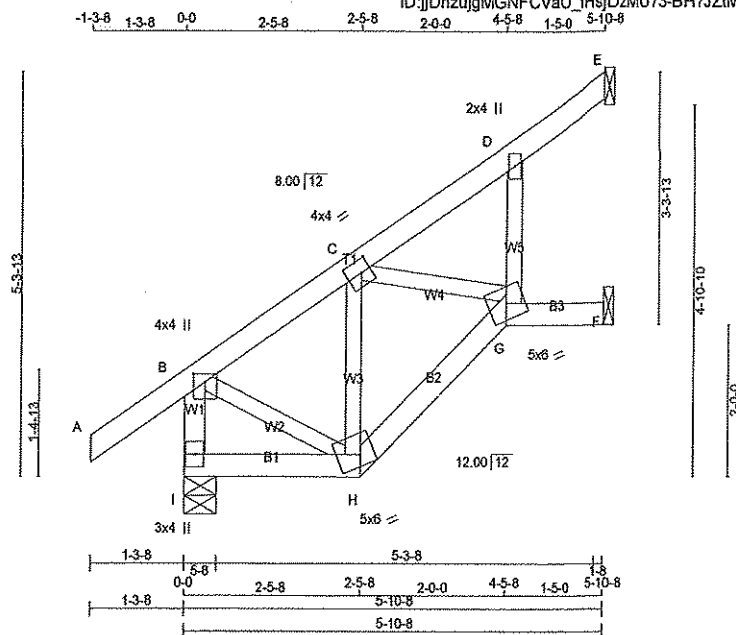
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281389	J1S	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: jDnzuJgMGNFCVaO_fHsDzMo73-BH7JZtMCfXabFB2zyEqj2L?_IH3TK9u4cZ9izMUNM

Scale = 1:29.8



TOTAL WEIGHT = 7 X 26 = 182 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		SPF	
I - B	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
I	416	0	416	0	0	5-8	5-8	5-8	5-8
E	290	0	290	0	0	1-8	1-8	1-8	1-8
F	20	0	35	0	0	1-8	1-8	1-8	1-8

SEE MITER STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) E, F

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
I	329	201/0	62/0	0/0	0/0	67/0	0/0		
E	232	137/0	47/0	0/0	0/0	49/0	0/0		
F	25	0/0	15/0	0/0	0/0	10/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
I-B	-381/0	0.0	0.0 0.04 (1)	H-C	-234/0	0.04 (1)	
A-B	0/29	-77.4	-77.4 0.10 (1)	G-D	0/312	0.07 (1)	
B-C	-230/0	-77.4	-77.4 0.16 (1)	B-H	0/233	0.05 (1)	
C-D	-121/0	-77.4	-77.4 0.45 (1)	C-G	-210/0	0.03 (1)	
D-E	0/131	-77.4	-77.4 0.47 (1)				
I-H	0/0	-28.0	-28.0 0.05 (2)				
H-G	0/290	-28.0	-28.0 0.07 (2)				
G-F	0/0	-28.0	-28.0 0.02 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/969$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/680$ (0.10")

CSI: TC=0.47 (D-E:1), BC=0.07 (G-H:2), WB=0.07 (D-G:1), SSI=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

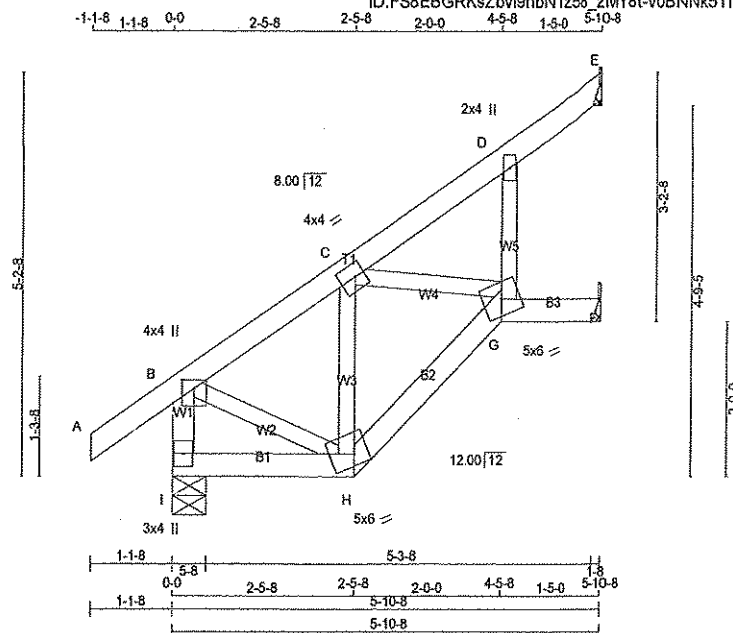
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.50)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 1960317
STRUCTURAL
COMPONENT ONLY

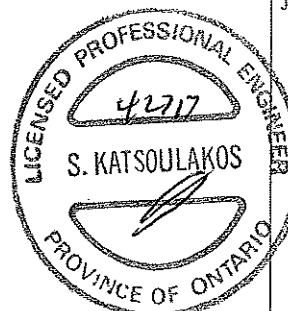
Scale = 1:29.2



TOTAL WEIGHT = $7 \times 25 = 178 \text{ lb}$

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

CHORDS					WEB S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSH (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSH (LC)		
FR-TO		FROM TO			FR-TO				
I-B	-368 / 0	0.0	0.0	0.04 (1)	7.81	H-C	-237 / 0	0.04 (1)	
A-B	0 / 126	-77.4	-77.4	0.08 (1)	10.00	G-D	0 / 312	0.07 (1)	
B-C	-240 / 0	-77.4	-77.4	0.18 (1)	6.25	B-H	0 / 239	0.05 (1)	
C-D	-121 / 0	-77.4	-77.4	0.45 (1)	6.25	C-G	-216 / 0	0.03 (1)	
D-E	0 / 131	-77.4	-77.4	0.47 (1)	10.00				
I-H	0 / 0	-28.0	-28.0	0.05 (2)	10.00				
H-G	0 / 302	-28.0	-28.0	0.07 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.02 (2)	10.00				



DWG NO. TAM 19712-17
STRUCTURAL
COMPONENT ONLY

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

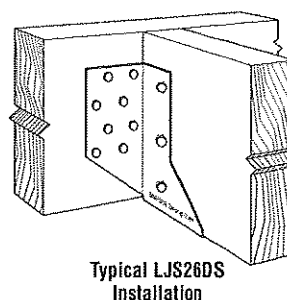
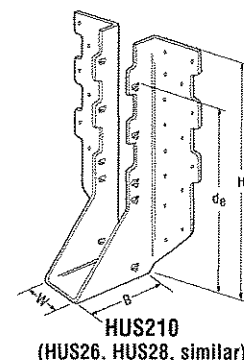
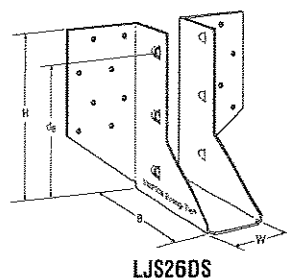
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

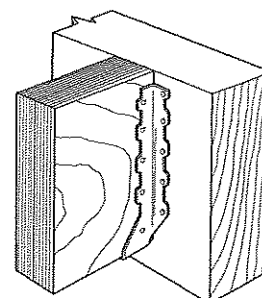
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

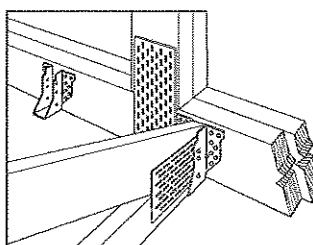
- See current catalogue for options



Typical LJS26DS Installation



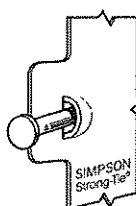
Typical HUS Installation



Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

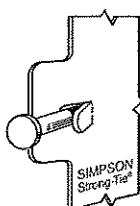
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4¾	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1½	5¾	3	3½	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1½	7¾	3	6¾	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1½	9¾	3	7¾	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

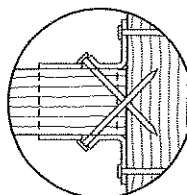


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

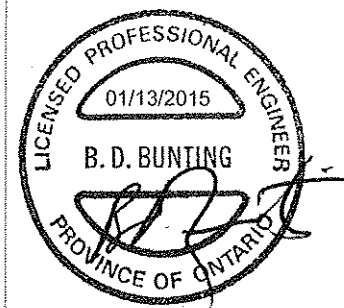
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

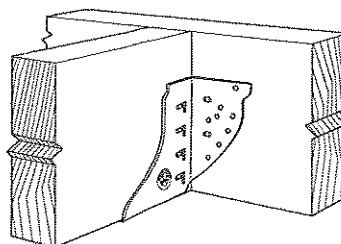
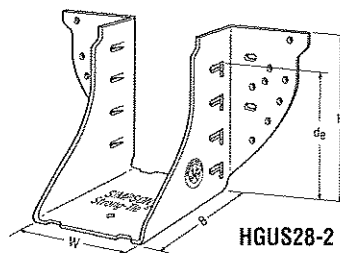
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

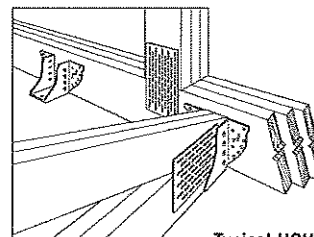
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



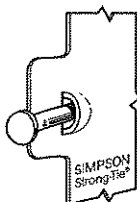
Typical HGUS Installation



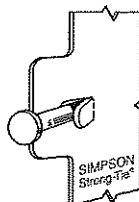
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 $\frac{1}{8}$	5 $\frac{1}{8}$	5	4 $\frac{3}{8}$	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 $\frac{1}{8}$	5 $\frac{1}{8}$	4	4 $\frac{1}{8}$	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 $\frac{1}{8}$	5 $\frac{1}{8}$	4	4 $\frac{1}{8}$	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 $\frac{1}{8}$	5 $\frac{1}{8}$	4	4 $\frac{1}{8}$	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 $\frac{1}{8}$	7 $\frac{1}{8}$	5	6 $\frac{1}{8}$	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 $\frac{1}{8}$	7 $\frac{1}{8}$	4	6 $\frac{1}{8}$	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 $\frac{1}{8}$	7 $\frac{1}{8}$	4	6 $\frac{1}{8}$	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 $\frac{1}{8}$	7 $\frac{1}{8}$	4	6 $\frac{1}{8}$	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 $\frac{1}{8}$	9 $\frac{1}{8}$	4	8 $\frac{1}{8}$	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 $\frac{1}{8}$	9 $\frac{1}{8}$	4	8 $\frac{1}{8}$	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 $\frac{1}{8}$	9 $\frac{1}{8}$	4	8 $\frac{1}{8}$	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 $\frac{1}{8}$	10 $\frac{1}{8}$	4	10 $\frac{1}{8}$	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 $\frac{1}{8}$	12 $\frac{1}{8}$	4	11 $\frac{1}{8}$	66-16d	22-16d	10130	16400	7195	11645

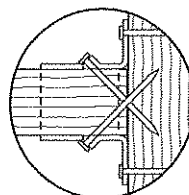
1. d_g is the distance from the seat of the hanger to the highest joist nail.



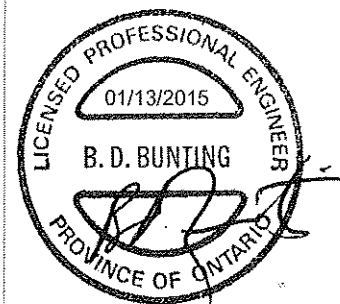
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

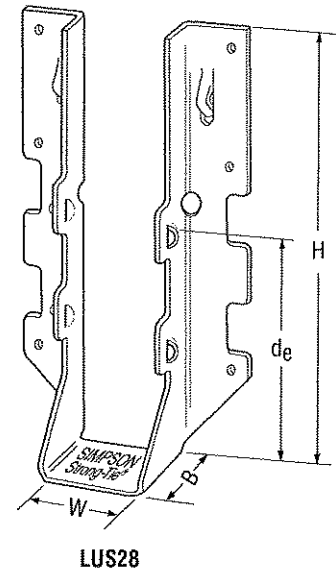
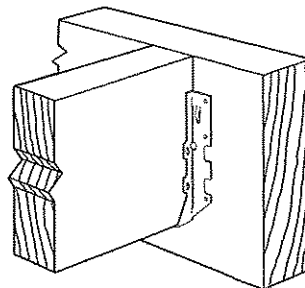
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

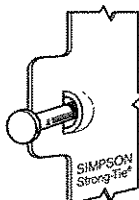
- These hangers cannot be modified.

Typical LUS Installation



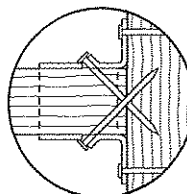
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D. Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/8	1 3/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 15/16	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

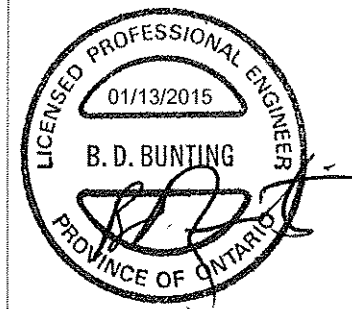


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC LUS15 1/15 exp 12/16

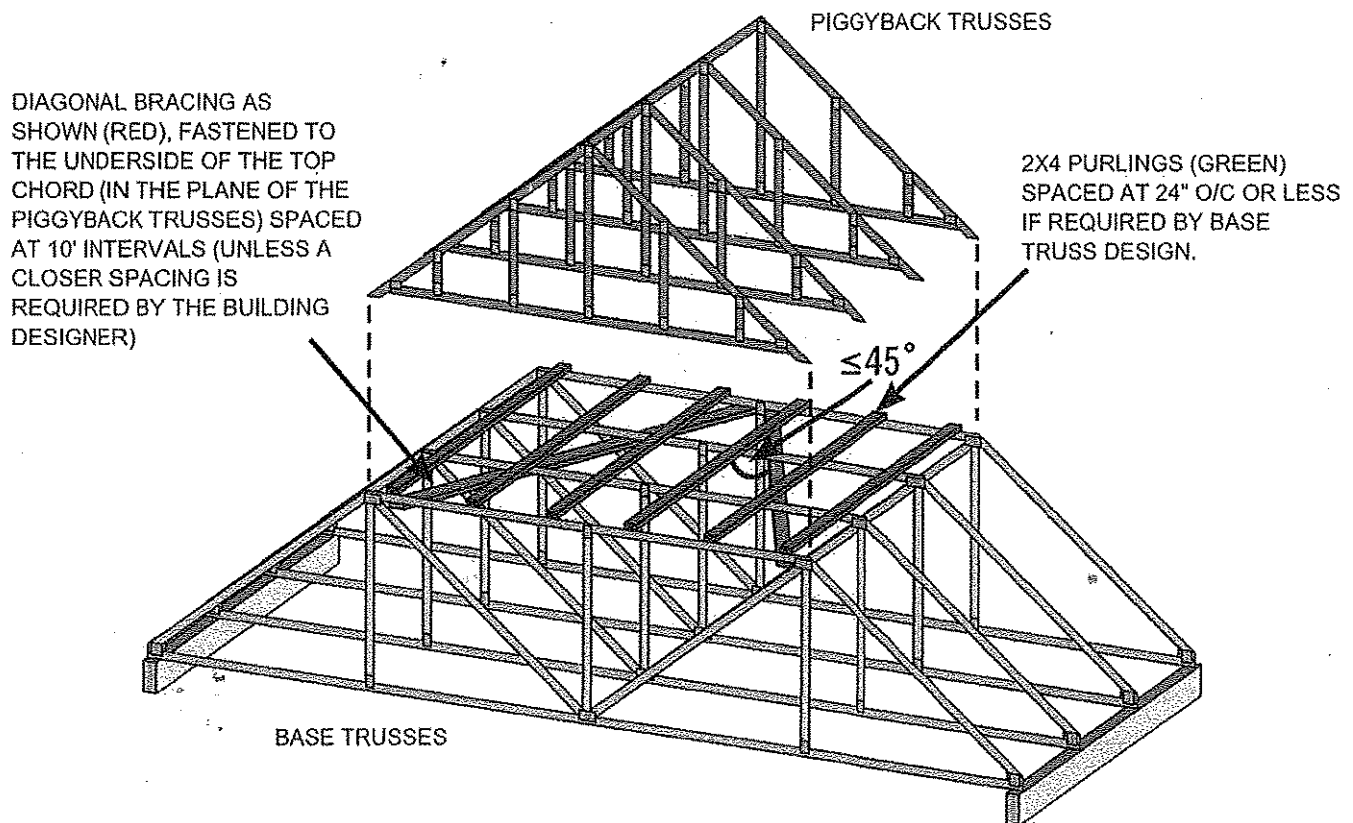
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Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

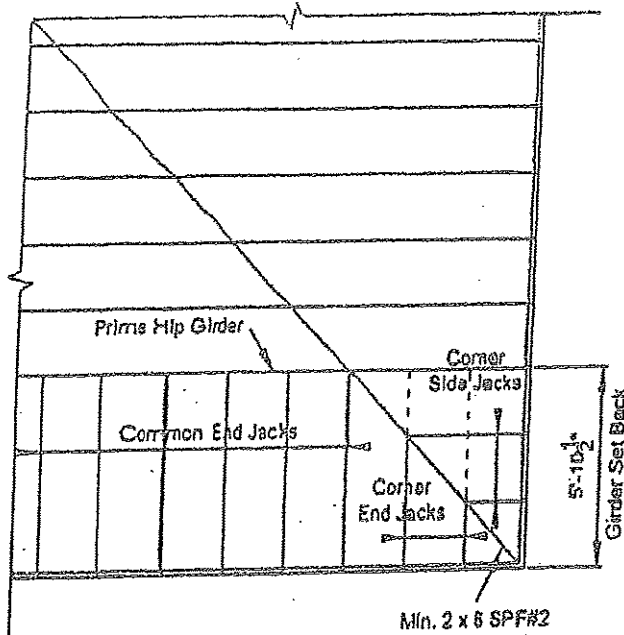
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

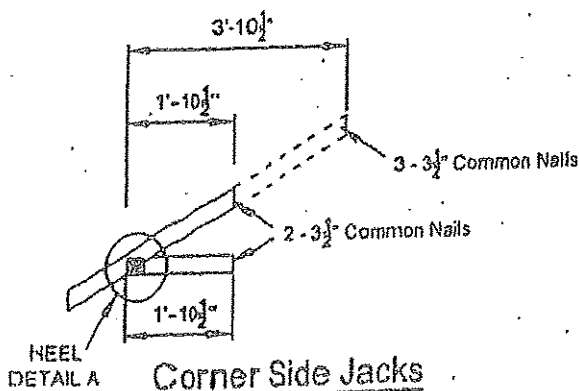
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

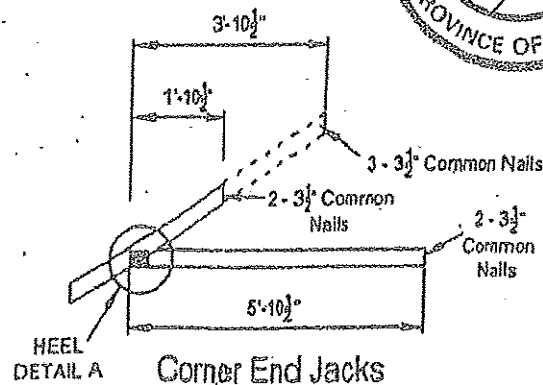
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

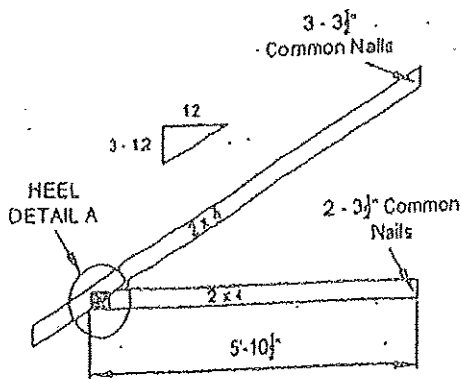
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



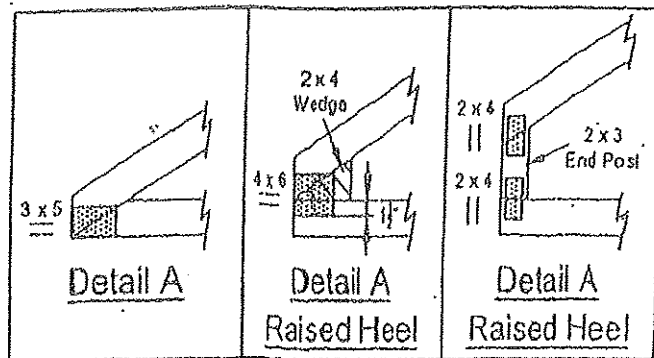
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

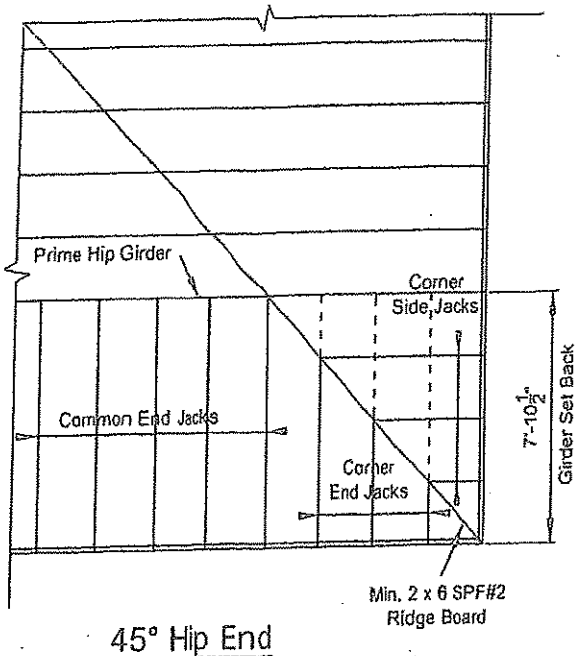
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

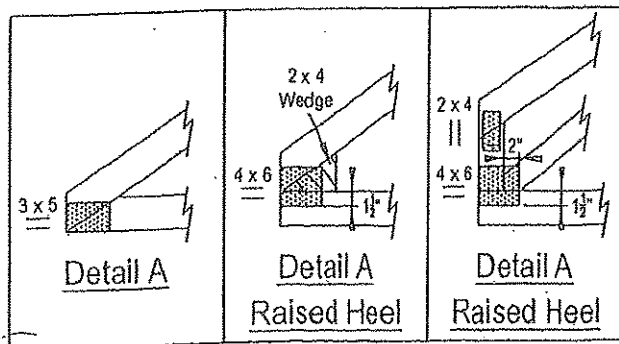
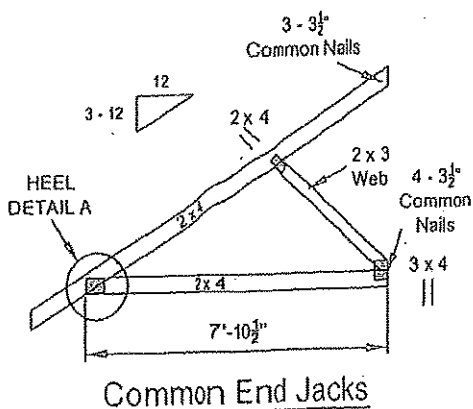
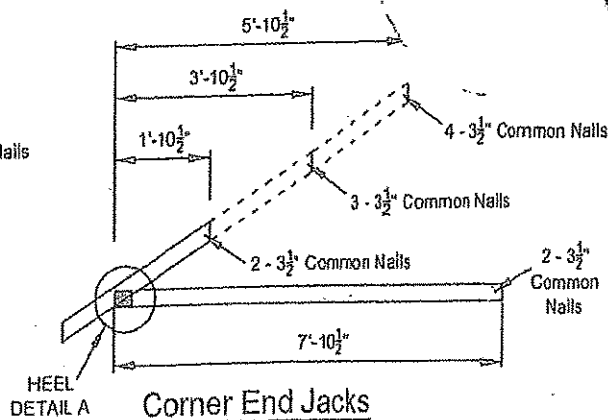
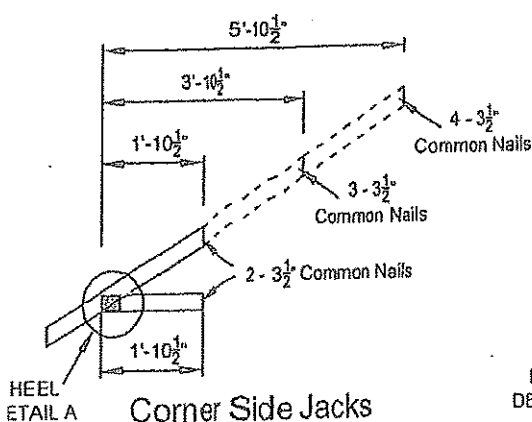
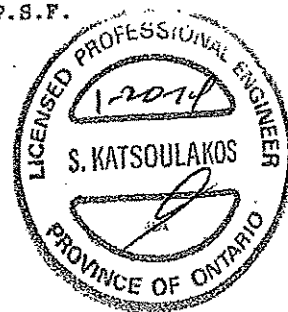
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.



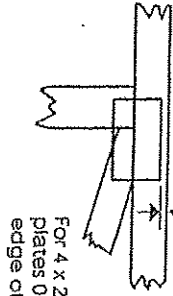
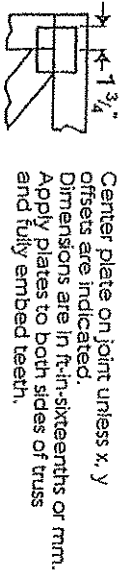
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NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

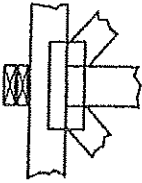
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



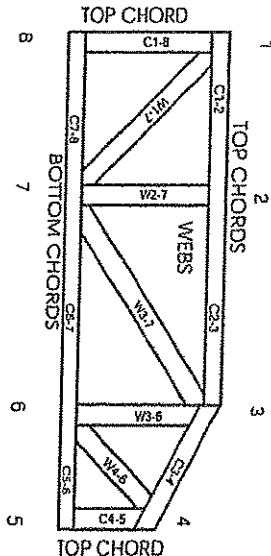
BEARING



Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing,
BCSI: Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11896-L, 10319-L, 13270-L, 12891-R

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Mitek Engineering Reference Sheet MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

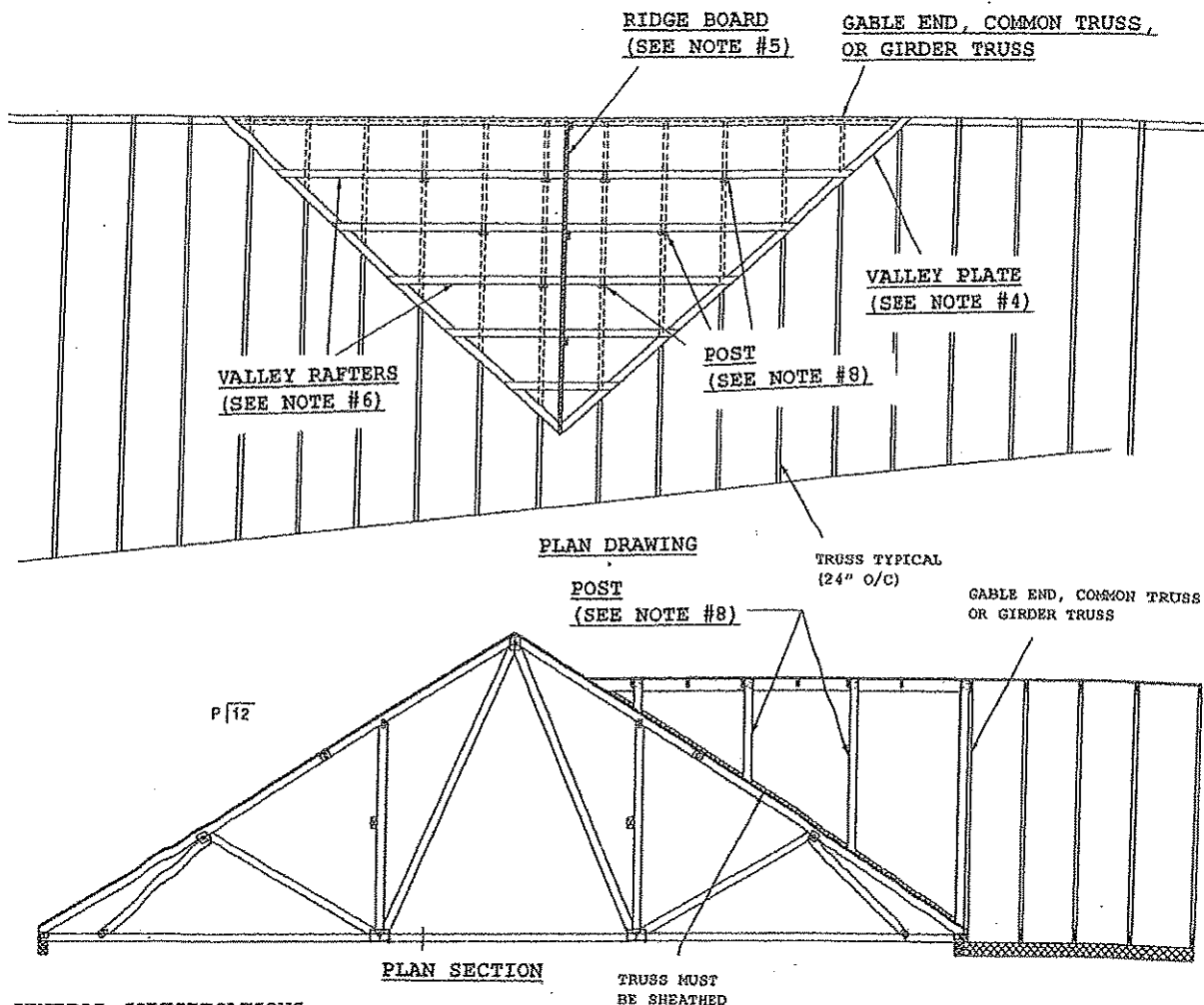
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, L or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully, knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

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R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

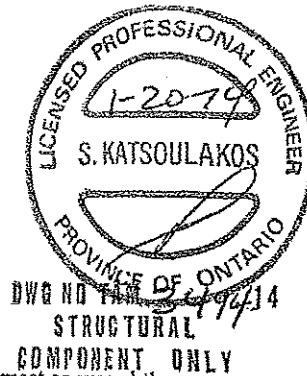


DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.