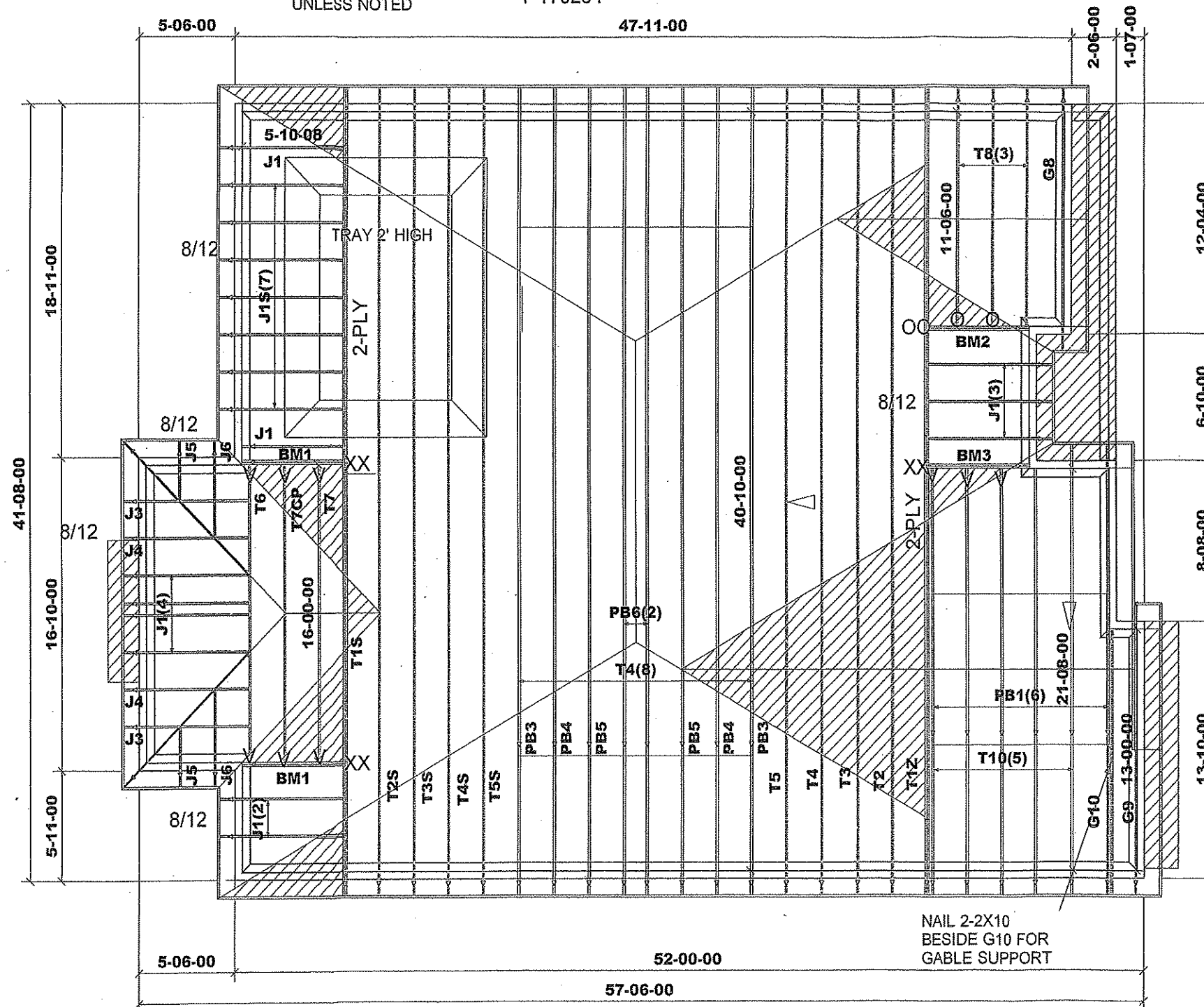


T-170284

HARDWARE
LUS24(O)
LUS24-2(OO)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

DWG #TAM 2031377
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY



NAIL 2-2X10
BESIDE G10 FOR
GABLE SUPPORT

MAY 1, 2017
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT:44080/281488)
DWG #TAM20287-17 THROUGH DWG #TAM20312-17, INCLUSIVE, AND ALL DATED 5-01-17;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

12" FINISH O.H.
BELL CURVE
3' STUCCO
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF @24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C.PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.3 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(0)
LUS24-2(OO)
LJS26DS(V)
LUS26-2(VV)
HGUS26-2(XX)

DATE: 5-01-17
BCIN:26064; FIRM:BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH INDIVIDUAL LOT.

DWG #TAM THROUGH DWG #TAM, INCLUSIVE, DATED ;

STRUCTURAL COMPONENTS ONLY; THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS ANDSEALED HARDWARE DRAWINGS. HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY. REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3/16" COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING" ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING FOR 3 PLY TRUSS COMPONENTS.

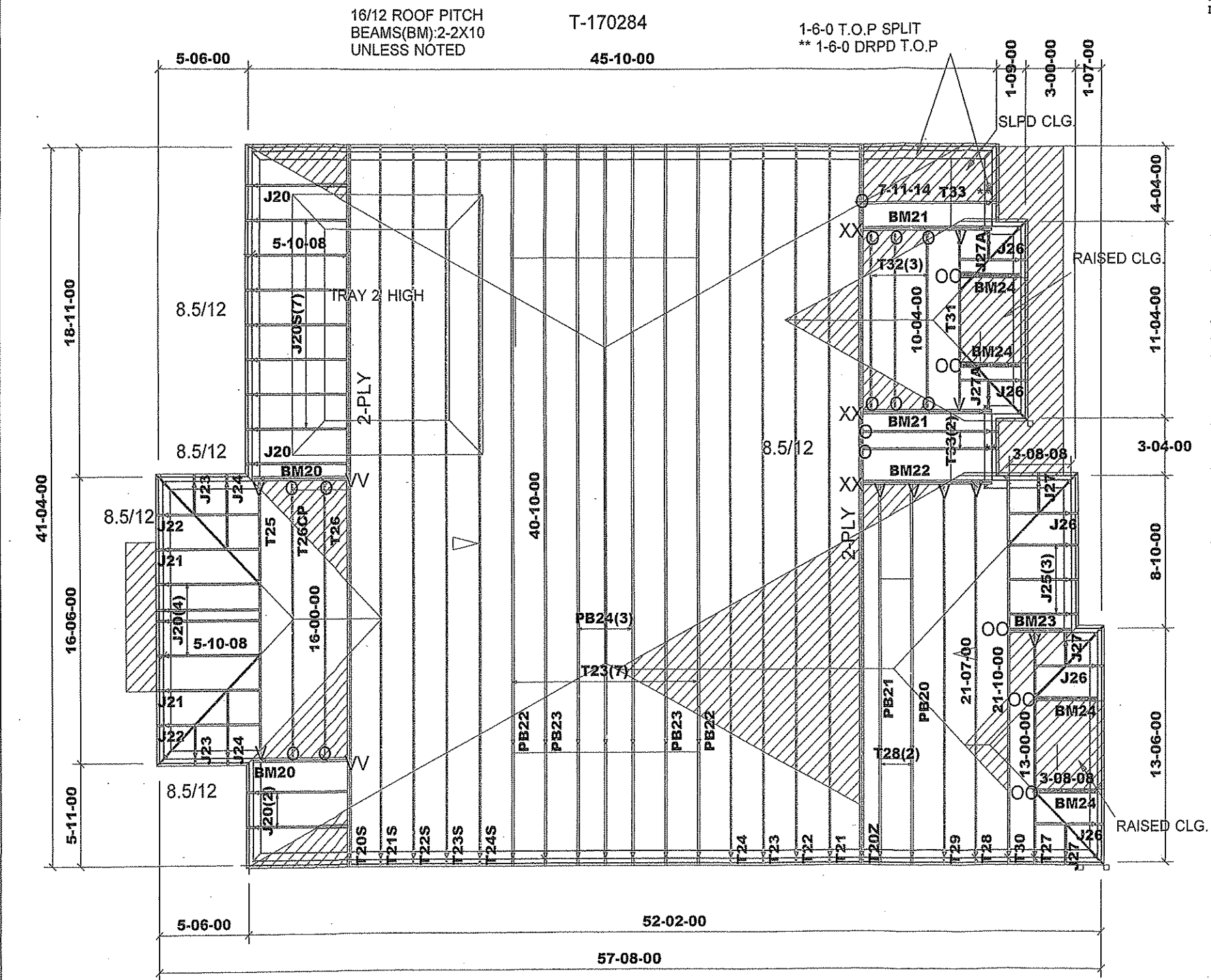
(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY "X".
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X 4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY.

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH STUDS AT24" O/C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG #TAM 20314-17
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL COMPONENTS ONLY



Job Track: 44080		Builder: GREENYORK HOMES /		Location: BRAMPTON		Model: YORK 3 / B		Elevation: C:\MTEK\CA742AJOB\B13000000000	
Layout ID: 281488		Project: DEGREY DRIVE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.					
Plan Log: 91851		Date: 4/28/2017		Designer: JG		Mitek ver 7.5.0			



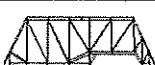
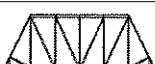
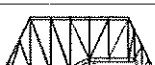
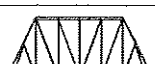
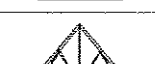
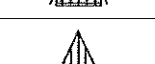
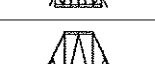
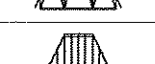
Delivery Shiplist

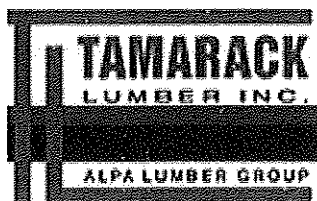
DATE	04/28/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281486 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1S HIP GIRDER	14.00	40-10-00	05-03-13	2 X 6	2 X 6	01-03-08	02-01-05	493.76		
	2 Ply		12.00							298.00		
	1	T1Z HIP GIRDER	14.00	40-10-00	05-03-13	2 X 4	2 X 6	01-03-08	02-01-05	441.46		
	2 Ply		0.00							266.66		
	1	T2 HIP	14.00	40-10-00	06-07-13	2 X 4	2 X 6	01-03-08	02-01-05	206.48		
			0.00							124.17		
	1	T2S HIP	14.00	40-10-00	06-07-13	2 X 4	2 X 6	01-03-08	02-01-05	225.22		
			12.00							139.17		
	1	T3 HIP	14.00	40-10-00	07-11-13	2 X 4	2 X 6	01-03-08	02-01-05	222.16		
			0.00							134.67		
	1	T3S HIP	14.00	40-10-00	07-11-13	2 X 4	2 X 6	01-03-08	02-01-05	232.23		
			12.00							145.67		
	9	T4 PIGGYBACK	14.00	40-10-00	09-03-13	2 X 4	2 X 6	01-03-08	02-01-05	2101.59		
			0.00							1243.53		
	1	T4S HIP	14.00	40-10-00	09-03-13	2 X 4	2 X 6	01-03-08	02-01-05	266.37		
			12.00							164.50		
	1	T5 HIP	14.00	40-10-00	10-07-13	2 X 4	2 X 6	01-03-08	02-01-05	255.31		
			0.00							152.67		
	1	T5S HIP	14.00	40-10-00	10-07-13	2 X 4	2 X 6	01-03-08	02-01-05	270.42		
			12.00							166.00		
	1	T6 HIP GIRDER	8.00	16-00-00	05-03-13	2 X 4	2 X 4	00-00-00	01-04-13	63.71		
			0.00							40.50		
	1	T7 COMMON	8.00	16-00-00	06-08-13	2 X 4	2 X 4	00-00-00	01-04-13	64.48		
			0.00							40.83		
	1	T7CP COMMON	8.00	16-00-00	06-07-13	2 X 4	2 X 4	00-00-00	01-04-13	64.38		
			0.00							40.83		
	3	T8 COMMON	14.00	11-06-00	08-09-13	2 X 4	2 X 4	01-03-08	02-01-05	174.60		
			0.00							111.51		
	1	G8 COMMON	14.00	11-06-00	08-09-13	2 X 4	2 X 4	01-03-08	02-01-05	67.94		
			0.00							44.17		
	1	G9 COMMON	14.00	13-00-00	09-08-05	2 X 4	2 X 4	01-03-08	02-01-05	76.89		
			0.00							48.17		
	5	T10 PIGGYBACK	14.00	21-08-00	10-00-00	2 X 4	2 X 4	01-03-08	02-01-05	629.65		
			0.00							396.65		
	1	G10 GABLE	14.00	21-08-00	10-00-00	2 X 4	2 X 4	01-03-08	02-01-05	129.52		
			0.00							82.17		



Delivery Shiplist

DATE	04/28/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281486 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	PB1 PIGGYBACK	14.00 0.00	07-01-07	04-07-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	153.30 100.02		
	2	PB3 PIGGYBACK	14.00 0.00	27-05-08	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	184.68 112.00		
	2	PB4 PIGGYBACK	14.00 0.00	27-05-08	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	194.90 122.66		
	2	PB5 PIGGYBACK	14.00 0.00	27-05-08	05-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	206.56 129.34		
	2	PB6 PIGGYBACK	14.00 0.00	27-05-08	06-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-10 00-05-10	219.70 136.34		
	11	J1 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	246.84 150.37		
	7	J1S JACK-OPEN	8.00 12.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	182.00 127.19		
	2	J3 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	33.68 20.66		
	2	J4 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	39.16 24.66		
	2	J5 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	18.34 11.34		
	2	J6 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	23.82 15.34		

TOTAL # TRUSS= 74.00

TOTAL BFT OF ALL TRUSSES=

4589.79 BFT. TOTAL WEIGHT OF ALL TRUSSES= 7489.15 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= 15.00



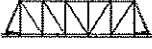
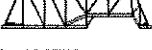
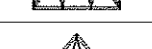
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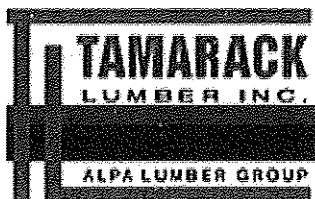
DATE	04/28/17
SALES REP	Rick

JOB TRACK:44080	LAYOUT ID: 281488	LOCATION: BRAMPTON
BUILDER: GREENYORK HOMES/DEGREY DRIVE.	SUB-BUILDER:	
MODEL: YORK 3	ELEVATION: B	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1 2 Ply	T20S HIP GIRDER	16.00 12.00	40-10-00	04-11-00	2 X 6	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	495.88 298.66		
	1 2 Ply	T20Z HIP GIRDER	16.00 0.00	40-10-00	04-11-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	451.04 271.34		
	1	T21 HIP	16.00 0.00	40-10-00	06-04-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	200.16 122.50		
	1	T21S HIP	16.00 12.00	40-10-00	06-04-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	211.67 132.67		
	1	T22 HIP	16.00 0.00	40-10-00	07-09-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	213.34 130.34		
	1	T22S HIP	16.00 12.00	40-10-00	07-09-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	227.54 141.00		
	8	T23 HIP	16.00 0.00	40-10-00	09-02-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	1890.64 1145.36		
	1	T23S HIP	16.00 12.00	40-10-00	09-02-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	259.23 162.00		
	1	T24 HIP	16.00 0.00	40-10-00	10-07-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	250.40 150.67		
	1	T24S HIP	16.00 12.00	40-10-00	10-07-00	2 X 4	2 X 6	00-05-00 00-05-00	00-05-08 00-05-08	264.54 164.00		
	1	T25 HIP GIRDER	8.50 0.00	16-00-00	04-11-00	2 X 4	2 X 4	00-00-00 00-00-00	00-09-01 00-09-01	61.57 40.00		
	1	T26 COMMON	8.50 0.00	16-00-00	06-05-01	2 X 4	2 X 4	00-00-00 00-00-00	00-09-01 00-09-01	65.21 42.67		
	1	T26CP COMMON	8.50 0.00	16-00-00	06-04-00	2 X 4	2 X 4	00-00-00 00-00-00	00-09-01 00-09-01	65.09 41.34		
	1	T27 HIP GIRDER	16.00 0.00	13-00-00	05-11-08	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 01-00-03	56.33 36.67		
	3	T28 HIP	16.00 0.00	21-07-00	08-07-08	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 01-04-03	339.54 217.50		
	1	T29 HIP	16.00 0.00	21-07-00	11-03-08	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 01-04-03	124.97 76.67		
	1	T30 HIP GIRDER	16.00 0.00	21-10-00	05-11-08	2 X 4	2 X 4	00-05-00 00-05-00	00-03-08 00-03-08	105.44 68.67		
	1	T31 HIP GIRDER	16.00 0.00	10-04-00	05-11-08	2 X 4	2 X 4	00-00-00 00-00-00	01-04-03 01-04-03	51.05 34.17		



Delivery Shiplist

DATE	04/28/17
SALES REP	Rick

JOB TRACK: 44080 LAYOUT ID: 281488 LOCATION: BRAMPTON
 BUILDER: GREENYORK HOMES/DEGREY DRIVE. SUB-BUILDER:
 MODEL: YORK 3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	T32 COMMON	16.00 0.00	10-04-00	08-02-14	2 X 4	2 X 4	00-00-00 00-00-00	01-04-03 01-04-03	145.47 89.01		
	3	T33 JACK-PARTIAL	8.50 8.50	07-11-14	06-05-00	2 X 4	2 X 4	00-05-00 00-00-00	00-09-01 04-11-00	102.21 66.51		
	1	PB20 PIGGYBACK	16.00 0.00	09-05-10	05-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-06-01 00-06-01	40.39 26.00		
	1	PB21 PIGGYBACK	16.00 0.00	09-05-10	06-09-13	2 X 4	2 X 4	00-00-00 00-00-00	00-06-01 00-06-01	35.52 22.17		
	2	PB22 PIGGYBACK	16.00 0.00	27-07-14	02-08-08	2 X 4	2 X 4	00-00-00 00-00-00	00-06-01 00-06-01	192.50 124.34		
	2	PB23 PIGGYBACK	16.00 0.00	27-07-14	04-01-08	2 X 4	2 X 4	00-00-00 00-00-00	00-06-01 00-06-01	200.00 124.00		
	3	PB24 PIGGYBACK	16.00 0.00	27-07-14	05-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-06-01 00-06-01	319.80 200.49		
	8	J20 JACK-OPEN	8.50 0.00	05-10-08	04-11-00	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 04-11-00	137.92 85.36		
	7	J20S JACK-OPEN	8.50 12.00	05-10-08	04-11-00	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 02-11-00	171.15 120.19		
	2	J21 JACK-OPEN	8.50 0.00	05-10-08	03-05-04	2 X 4	2 X 4	00-05-00 -02-01-01	00-03-08 00-03-08	28.66 18.66		
	2	J22 JACK-OPEN	8.50 0.00	05-10-08	02-00-04	2 X 4	2 X 4	00-05-00 -04-01-01	00-03-08 00-03-08	23.06 14.66		
	2	J23 JACK-OPEN	8.50 0.00	01-10-08	02-00-04	2 X 4	2 X 4	00-05-00 -00-01-01	00-03-08 00-03-08	13.94 9.34		
	2	J24 JACK-OPEN	8.50 0.00	01-10-08	03-05-04	2 X 4	2 X 4	00-05-00 01-10-15	00-03-08 00-05-06	19.52 13.34		
	3	J25 JACK-OPEN	16.00 0.00	03-08-08	05-11-08	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 05-11-08	42.66 27.99		
	5	J26 JACK-OPEN	16.00 0.00	03-08-08	03-04-12	2 X 4	2 X 4	00-05-00 -01-11-01	00-03-08 00-03-08	52.85 33.35		
	3	J27 JACK-OPEN	16.00 0.00	01-10-08	03-04-12	2 X 4	2 X 4	00-05-00 -00-01-01	00-03-08 00-03-08	25.44 15.99		
	2	J27A JACK-OPEN	16.00 0.00	01-07-08	03-04-12	2 X 4	2 X 4	00-00-00 -00-01-01	01-04-03 00-03-08	12.62 8.00		

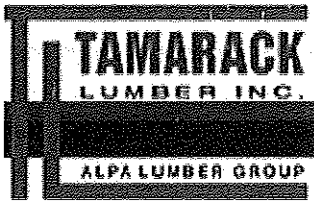
TOTAL # TRUSS= 80.00

TOTAL BFT OF ALL TRUSSES=

4275.63 BFT. TOTAL WEIGHT OF ALL TRUSSES= 6897.35 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	04/28/17
SALES REP	Rick

JOB TRACK:44080	LAYOUT ID: 281488	LOCATION: BRAMPTON
BUILDER: GREENYORK HOMES/DEGREY DRIVE.	SUB-BUILDER:	
MODEL: YORK 3	ELEVATION: B	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
9	Hangers	LJS26DS	
13	Hangers	LUS24	
5	Hangers	LUS24-2	
2	Hangers	LUS26-2	

TOTAL # ITEMS= 32.00

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:	
Building number, street name			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description		
B. Individual who reviews and takes responsibility for design activities				
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.		
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail	
Telephone number (519) 287-2242 Business		Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]				
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC – House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing – House ☐ Plumbing – All Buildings ☐ On-site Sewage Systems				
Description of designer's work GREENYORK HOMES – DEGREY DRIVE – MODEL: YORK 3 – ELEV. A (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM20313-17 DATED 5-01-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.				
D. Declaration of Designer				
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____				
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.				
Date		Signature of Designer		

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM 20313-17-S
DWG#TAM 20315-17-S

5117

Schedule 1: Designer Information

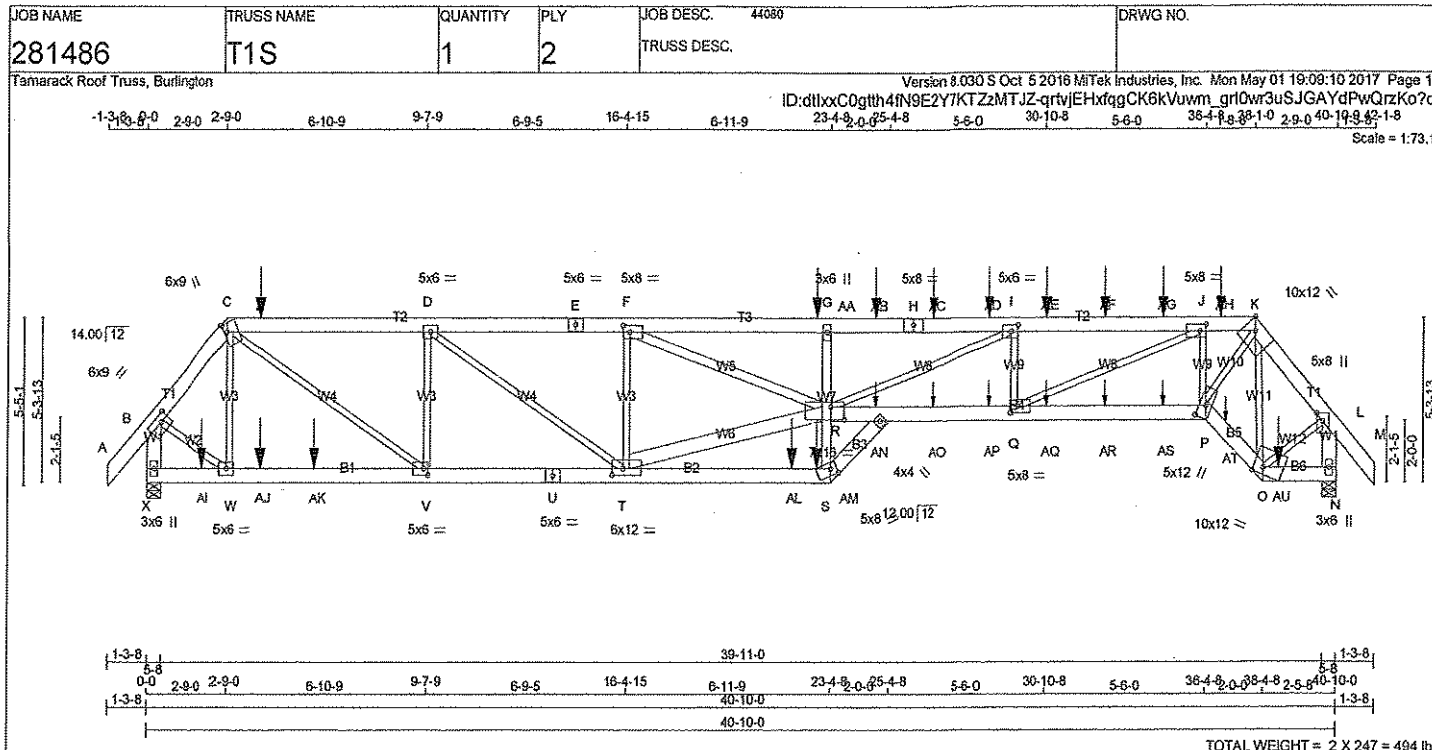
Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:	
Building number, street name			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description		
B. Individual who reviews and takes responsibility for design activities				
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.		
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail	
Telephone number (519) 287-2242 Business		Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]				
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC – House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing – House ☐ Plumbing – All Buildings ☐ On-site Sewage Systems				
Description of designer's work GREENYORK HOMES – DEGREY DRIVE – MODEL: YORK 3 – ELEV. B (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM20314-17 DATED 5-01-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.				
D. Declaration of Designer				
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____				
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.				
Date		Signature of Designer		

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d). of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM 20314-17-S 5777
 DWG#TAM 2031617-S



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - H	2x6	DRY	No.2	SPF	
H - K	2x6	DRY	No.2	SPF	
K - M	2x6	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
X - U	2x6	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - O	2x6	DRY	No.2	SPF	
O - N	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
S - G	2x4	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
F - R	2x4	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C 2	12	TOP
C - E 2	12	SIDE(0.0)
E - H 2	12	SIDE(183.1)
H - K 2	12	SIDE(0.0)
K - M 2	12	TOP
X - B 2	12	TOP
N - L 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
X - U 2	12	SIDE(0.0)
U - S 2	12	SIDE(122.0)
R - P 2	12	SIDE(0.0)
P - O 2	12	SIDE(0.0)
O - N 2	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	6.0	9.0	2.50	3.50
C TTVW+tm	MT20	6.0	9.0	3.25	1.25
D TMVW-I	MT20	5.0	6.0		
E TS-I	MT20	5.0	6.0		
F TMVW-I	MT20	5.0	8.0	2.50	2.75
G TMW+u	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
X	4740	0	0	5-8
N	4508	0	0	5-8

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD (0.25" U)

UNFACTORED REACTIONS

	1ST LCASE	MAX MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
X	3913	2123 / 0	908 / 0	0 / 0	0 / 0	880 / 0	0 / 0
N	3754	1985 / 0	908 / 0	0 / 0	0 / 0	861 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		VERT. LOAD (PLF)	LC1 CSI (L/C)	MAX. UNBRAC LENGTH	MAX. FACTORED		
MEMB.	FORCE (LBS)	FROM	TO				MEMB.	FORCE (LBS)	MAX CSI (L/C)
FR-TO		FROM	TO				FR-TO		
A-B	0 / 43	-77.4	-77.4	0.03 (1)	10.00	W-C	0 / 250	0.03 (3)	
B-C	-4423 / 0	-77.4	-77.4	0.05 (1)	5.43	S-R	0 / 1225	0.35 (1)	
C-Z	-6940 / 0	-77.4	-77.4	0.27 (1)	4.36	R-G	-805 / 0	0.25 (1)	
Z-D	-6940 / 0	-77.4	-77.4	0.27 (1)	4.36	Q-I	-2855 / 0	0.27 (1)	
D-E	-9181 / 0	-77.4	-77.4	0.31 (1)	3.86	Q-J	0 / 7293	0.90 (1)	
E-F	-9181 / 0	-77.4	-77.4	0.31 (1)	3.86	P-J	-3715 / 0	0.35 (1)	
F-AA	-16922 / 0	-77.4	-77.4	0.76 (1)	2.58	P-K	0 / 7673	0.95 (1)	
AA-G	-16922 / 0	-77.4	-77.4	0.76 (1)	2.58	K-O	-3935 / 0	0.83 (1)	
G-AB	-17032 / 0	-77.4	-77.4	0.92 (1)	2.34	B-W	0 / 3229	0.40 (1)	
AB-H	-17032 / 0	-77.4	-77.4	0.92 (1)	2.34	Q-L	0 / 2764	0.34 (1)	
H-AC	-17032 / 0	-77.4	-77.4	0.92 (1)	2.34	C-V	0 / 4989	0.62 (1)	
AC-AD	-17032 / 0	-77.4	-77.4	0.92 (1)	2.34	T-F	-3783 / 0	0.72 (1)	
AD-I	-17032 / 0	-77.4	-77.4	0.92 (1)	2.34	V-D	-2172 / 0	0.41 (1)	
IAE	-13470 / 0	-77.4	-77.4	0.60 (1)	2.98	D-T	0 / 2770	0.34 (1)	
AE-AF	-13470 / 0	-77.4	-77.4	0.60 (1)	2.98	T-R	0 / 9450	0.61 (1)	
AF-AG	-13470 / 0	-77.4	-77.4	0.60 (1)	2.98	F-R	0 / 8312	0.73 (1)	
AG-J	-13470 / 0	-77.4	-77.4	0.60 (1)	2.98	R-I	0 / 3889	0.48 (1)	
J-AH	-6851 / 0	-77.4	-77.4	0.25 (1)	4.43				
AH-K	-6851 / 0	-77.4	-77.4	0.25 (1)	4.43				
K-L	-3679 / 0	-77.4	-77.4	0.04 (1)	5.83				
L-M	0 / 43	-77.4	-77.4	0.03 (1)	10.00				
X-B	-5129 / 0	0.0	0.0	0.20 (1)	6.44				
N-L	-4461 / 0	0.0	0.0	0.17 (1)	6.81				
X-AI	0 / 0	-28.0	-28.0	0.47 (1)	10.00				
AI-W	0 / 0	-28.0	-28.0	0.47 (1)	10.00				
W-AJ	0 / 2847	-28.0	-28.0	0.75 (1)	10.00				
AJ-AK	0 / 2847	-28.0	-28.0	0.75 (1)	10.00				
AK-V	0 / 2847	-28.0	-28.0	0.75 (1)	10.00				
V-U	0 / 6940	-28.0	-28.0	0.62 (1)	10.00				
U-T	0 / 6940	-28.0	-28.0	0.62 (1)	10.00				
T-AL	0 / 174	-28.0	-28.0	0.52 (1)	10.00				
AL-AM	0 / 174	-28.0	-28.0	0.52 (1)	10.00				
AM-S	0 / 174	-28.0	-28.0	0.52 (1)	10.00				
R-AN	0 / 13470	-28.0	-28.0	0.97 (1)	10.00				
AN-AO	0 / 13470	-28.0	-28.0	0.97 (1)	10.00				

LICENSED PROFESSIONAL
ENGINEER
S. KATSOULAS
No. 5777
STATE OF FLORIDA

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA D88-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (1.36")
CALCULATED VERT. DEFL.(LL) = 1/726 (0.67")
ALLOWABLE DEFL.(TL) = 1/360 (1.36")
CALCULATED VERT. DEFL.(TL) = 1/448 (1.09")

CSI: TC=0.92 (G-I:1), BC=0.97 (Q-R:1), WB=0.95 (K-P:1), SSI=0.41 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

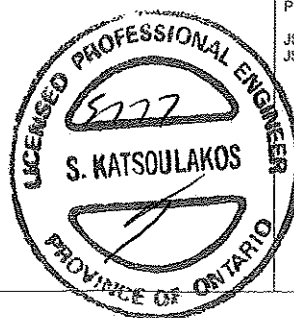
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.90 (R) (INPUT = 1.00)



DRWG NO. TAM 20263-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T1S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:09:10 2017 Page 2

ID:dtlxxC0gth4fN9E2Y7KTZMTJZ-qrvtJEHxfqCK6kVuvwm_grl0wr3uSJGAYdPwQrzKo?d

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TS-1	MT20	5.0	8.0		
I	TMWW-1	MT20	5.0	6.0	2.50	2.50
J	TMWW-1	MT20	5.0	8.0	2.50	2.50
K	TTWW-h	MT20	10.0	12.0	Edge	4.25
L	TMWW+p	MT20	5.0	8.0	3.25	2.25
N	BMV1+p	MT20	3.0	6.0		
O	BBWW-m	MT20	10.0	12.0	Edge	4.00
P	BBWW+m	MT20	5.0	12.0	5.00	2.50
Q	BMWW-1	MT20	5.0	8.0	2.50	2.50
R	BWWWWWW-1	MT20	7.0	16.0	5.00	5.50
S	BBW-m	MT20	5.0	8.0	2.50	2.50
T	BMWWWW-t	MT20	6.0	12.0	2.50	4.50
U	BS-1	MT20	5.0	6.0		
V	BMWW-1	MT20	5.0	6.0	2.50	1.75
W	BMWW-1	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		
Y	NP+h	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 149.9 lbs FACTORED DOWN AT 3-11-4, 149.9 lbs FACTORED DOWN AT 22-10-12, 212.3 lbs FACTORED DOWN AT 24-10-12, 212.3 lbs FACTORED DOWN AT 26-10-12, 212.3 lbs FACTORED DOWN AT 28-10-12, 212.3 lbs FACTORED DOWN AT 30-10-12, 212.3 lbs FACTORED DOWN AT 32-10-12, AND 212.3 lbs FACTORED DOWN AT 34-10-12, AND 212.3 lbs FACTORED DOWN AT 36-10-12 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 1358.6 lbs FACTORED DOWN AT 5-9-8, 1358.6 lbs FACTORED DOWN AT 22-0-8, 103.7 lbs FACTORED DOWN AT 22-10-12, AT 24-10-12, AT 26-10-12, AT 28-10-12, AT 30-10-12, AT 32-10-12, AT 34-10-12, AND AT 36-10-12, AND 99.1 lbs FACTORED DOWN AT 38-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
AO-AP	0 / 13470	-28.0	-28.0	0.97 (1)	10.00		
AP-Q	0 / 13470	-28.0	-28.0	0.97 (1)	10.00		
Q-AQ	0 / 6795	-28.0	-28.0	0.53 (1)	10.00		
AQ-AR	0 / 6795	-28.0	-28.0	0.53 (1)	10.00		
AR-AS	0 / 6795	-28.0	-28.0	0.53 (1)	10.00		
AS-P	0 / 6795	-28.0	-28.0	0.53 (1)	10.00		
P-AT	0 / 3690	-28.0	-28.0	0.24 (1)	10.00		
AT-O	0 / 3690	-28.0	-28.0	0.24 (1)	10.00		
O-AU	0 / 0	-28.0	-28.0	0.02 (2)	10.00		
AU-N	0 / 0	-28.0	-28.0	0.02 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Z	3-11-4	-150	-150	---	BACK	VERT	TOTAL
AA	22-10-12	-150	-150	---	BACK	VERT	TOTAL
AB	24-10-12	-212	-212	---	BACK	VERT	TOTAL
AC	26-10-12	-212	-212	---	BACK	VERT	TOTAL
AD	28-10-12	-212	-212	---	BACK	VERT	TOTAL
AE	30-10-12	-212	-212	---	BACK	VERT	TOTAL
AF	32-10-12	-212	-212	---	BACK	VERT	TOTAL
AG	34-10-12	-212	-212	---	BACK	VERT	TOTAL
AH	36-10-12	-212	-212	---	BACK	VERT	TOTAL
AI	1-11-4	-50	-87	---	BACK	VERT	TOTAL
AJ	3-11-4	-50	-87	---	BACK	VERT	TOTAL
AK	5-9-8	-1359	-1359	---	BACK	VERT	TOTAL
AL	22-0-8	-1359	-1359	---	BACK	VERT	TOTAL
AM	22-10-12	-59	-104	---	BACK	VERT	TOTAL
AN	24-10-12	---	---	---	BACK	VERT	TOTAL
AO	26-10-12	---	---	---	BACK	VERT	TOTAL
AP	28-10-12	---	---	---	BACK	VERT	TOTAL
AQ	30-10-12	---	---	---	BACK	VERT	TOTAL
AR	32-10-12	---	---	---	BACK	VERT	TOTAL
AS	34-10-12	---	---	---	BACK	VERT	TOTAL
AT	36-10-12	---	---	---	BACK	VERT	TOTAL
AU	38-10-12	-57	-99	---	BACK	VERT	TOTAL



DWG NO. TAM 20263-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:09:10 2017 Page 2
ID:dtlxCOgth4fN9E2Y7KTZzMTJZ-qrtvIEHxfqgCK6kVuwm_grl4Zr6ISlqAYdPwQrzKo?d

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMWW-t	MT20	6.0	9.0	3.00	3.25
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	8.0	9.0	4.25	4.50
S	BMWWW-t	MT20	8.0	9.0	4.25	4.50
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	6.0	9.0	3.00	3.25
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 149.9 lbs FACTORED DOWN AT 23-3-4,
AND 149.9 lbs FACTORED DOWN AT 25-3-4,
AND 149.9 lbs FACTORED DOWN AT 27-3-4 ON
TOP CHORD, AND 2614.8 lbs FACTORED DOWN
AT 21-9-8, 87.4 lbs FACTORED DOWN AT
23-3-4, 87.4 lbs FACTORED DOWN AT 25-3-4,
AND 87.4 lbs FACTORED DOWN AT 27-3-4, AND
1016.5 lbs FACTORED DOWN AT 29-2-8 ON
BOTTOM CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AC	27-3-4	-80	-87	---	FRONT	VERT	TOTAL
AD	29-2-8	-1016	-1016	---	FRONT	VERT	TOTAL



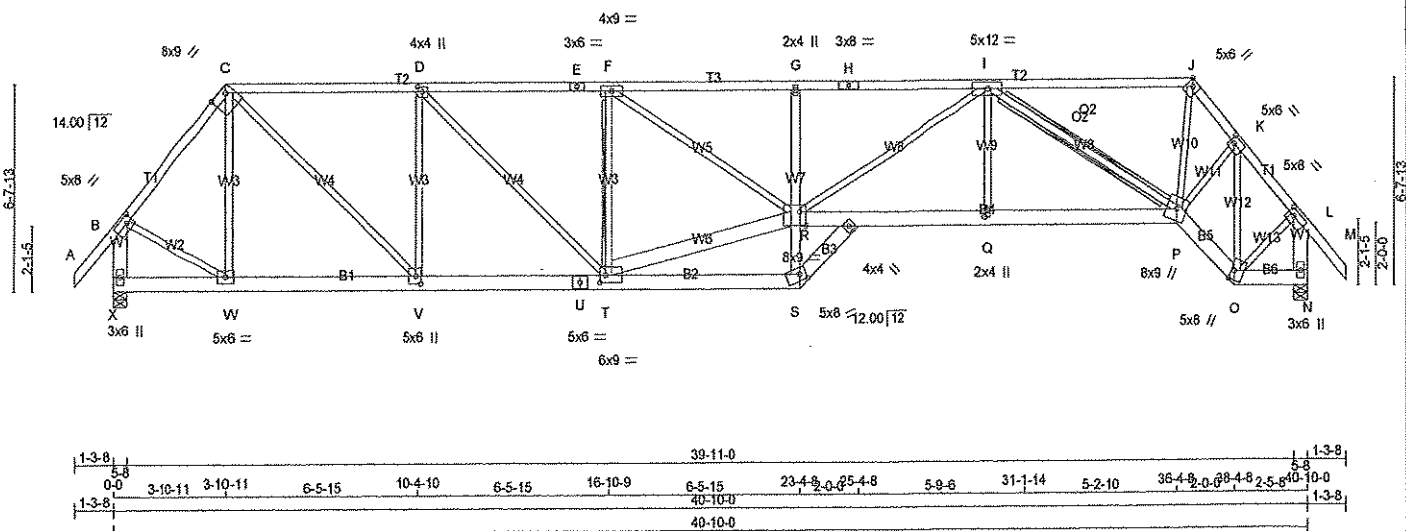
DWG NO. TAM 20264-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281486	T2S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Mon May 01 19:09:11 2017 Page 1
ID:dlxxC0gth4IN9E2Y7KTZMTJZ-11R-halZP8o3xFJhSeHDC2rBaEVLBI2KmH9UyHzKo7c

1-3-8 3-10-11 3-10-11 6-5-15 10-4-10 6-5-15 16-10-9 6-5-15 23-4-8 25-4-8 5-9-6 31-1-14 5-9-6 36-4-8 25-4-8 40-10-0 1-3-8
Scale = 1:72.8



TOTAL WEIGHT = 225 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF	
C - E	2x4	DRY	1650F 1.5E	SPF	
E - H	2x4	DRY	1650F 1.5E	SPF	
H - J	2x4	DRY	1650F 1.5E	SPF	
J - M	2x4	DRY	1650F 1.5E	SPF	
X - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
X - U	2x6	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - O	2x6	DRY	No.2	SPF	
O - N	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
S - G	2x4	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	2.50
C	TTWW-h	MT20	6.0	9.0	2.00	6.25
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	9.0		
G	TMVW-t	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	5.0	12.0		
J	TTWW-h	MT20	5.0	6.0	2.50	2.00
K	TMVW-t	MT20	5.0	8.0	2.25	2.50
L	TMVW-t	MT20	5.0	8.0	2.25	2.50
N	BMV1+p	MT20	3.0	6.0		
O	BBVW+m	MT20	5.0	8.0	3.50	1.25
P	BBVW+m	MT20	8.0	9.0		
Q	BMVW-h	MT20	2.0	4.0	2.50	1.00
R	BMVW-h	MT20	8.0	9.0	5.25	2.50
S	BBVW-m	MT20	5.0	8.0	2.50	2.50
T	BMVW-t	MT20	6.0	9.0	2.75	2.25
U	BS-t	MT20	5.0	8.0		
V	BMVW-t	MT20	5.0	6.0	3.00	2.00
W	BMVW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		
Y	NP-h	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
X	2259	0	2259	0
N	2259	0	2259	0

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
X	1881	1018 / 0	428 / 0
N	1881	1018 / 0	428 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-T

2x4 DRY SPF No.2 I-BRACE AT I-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS AT 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	FORCE (LBS)
	VERT. LOAD LC1	MAX. UNBRACED LENGTH FR-TO
	FROM TO	
FR-TO		
A-B	0 / 41	-77.4 -77.4 0.09 (1)
B-C	-1913 / 0	-77.4 -77.4 0.21 (1)
C-D	-2724 / 0	-77.4 -77.4 0.54 (1)
D-E	-3441 / 0	-77.4 -77.4 0.60 (1)
E-F	-3441 / 0	-77.4 -77.4 0.60 (1)
F-G	-5066 / 0	-77.4 -77.4 0.66 (1)
G-H	-5081 / 0	-77.4 -77.4 0.93 (1)
H-I	-5081 / 0	-77.4 -77.4 0.93 (1)
I-J	-2010 / 0	-77.4 -77.4 0.56 (1)
J-K	-2744 / 0	-77.4 -77.4 0.08 (1)
K-L	-1709 / 0	-77.4 -77.4 0.08 (1)
L-M	0 / 41	-77.4 -77.4 0.09 (1)
X-B	-2230 / 0	0.0 0.0 0.17 (1)
N-L	-2225 / 0	0.0 0.0 0.17 (1)
X-W	0 / 0	-28.0 -28.0 0.11 (2)
W-V	0 / 1233	-28.0 -28.0 0.22 (2)
V-U	0 / 2724	-28.0 -28.0 0.39 (1)
U-T	0 / 2724	-28.0 -28.0 0.39 (1)
T-S	0 / 40	-28.0 -28.0 0.13 (3)
R-Q	0 / 4138	-28.0 -28.0 0.57 (1)
Q-P	0 / 4138	-28.0 -28.0 0.57 (1)
P-O	0 / 1511	-28.0 -28.0 0.19 (1)
O-N	0 / 0	-28.0 -28.0 0.02 (2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.37")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/824 (0.59")

CSI: TC=0.93 (G-I:1), BC=0.57 (Q-R:1), WB=0.98 (I-P:1), SSI=0.26 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

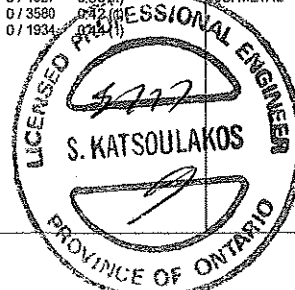
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)



DRWG NO. TAM 20266-17
STRUCTURAL
COMPONENT ONLY

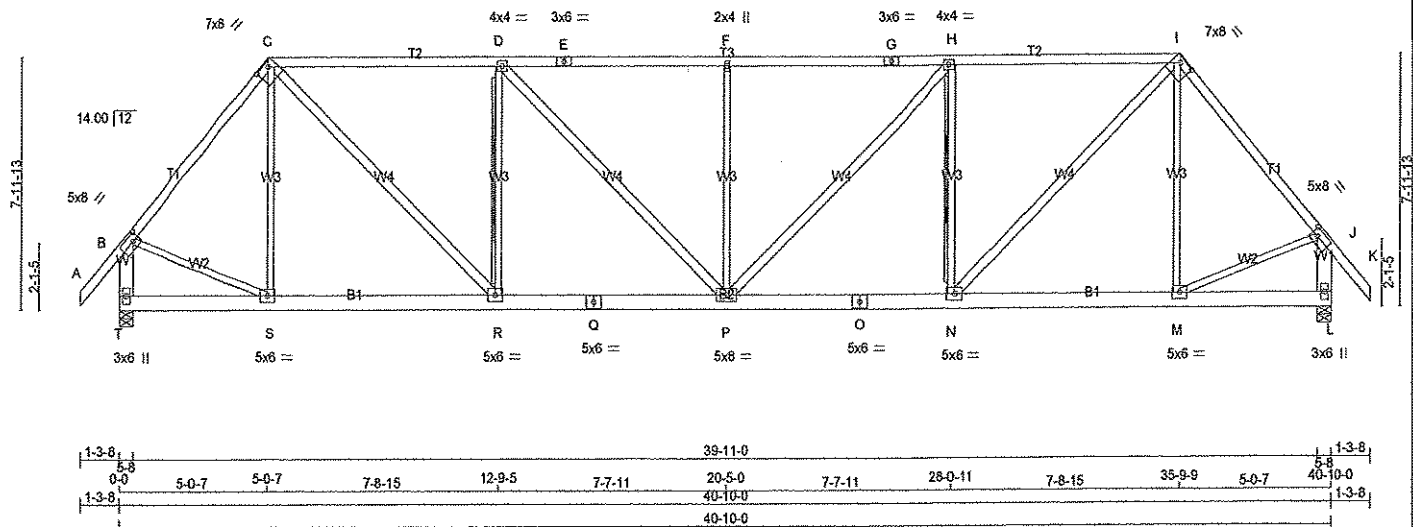
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:09:11 2017 Page 1

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1-3-8 5-0-7 5-0-7 7-8-15 12-9-5 7-7-11 20-5-0 7-7-11 28-0-11 7-8-15 35-9-9 5-0-7 40-10-0 1-3-8
Scale = 1:71.7



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF		
C - E	2x4	DRY	1650F 1.5E	SPF		
E - G	2x4	DRY	1650F 1.5E	SPF		
G - I	2x4	DRY	1650F 1.5E	SPF		
I - K	2x4	DRY	1650F 1.5E	SPF		
T - B	2x6	DRY	No.2	SPF		
L - J	2x6	DRY	No.2	SPF		
T - Q	2x6	DRY	No.2	SPF		
Q - O	2x6	DRY	No.2	SPF		
O - L	2x6	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
C - R	2x4	DRY	No.2	SPF		
D - P	2x4	DRY	No.2	SPF		
P - H	2x4	DRY	No.2	SPF		
N - I	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0	2.50	2.50
C	TTWW-h	MT20	7.0	8.0	1.75	5.00
D	TMVW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMVW-I	MT20	4.0	4.0		
I	TTWW-h	MT20	7.0	8.0	1.75	5.00
J	TMVW-I	MT20	5.0	8.0	2.50	2.50
L	BMV1+p	MT20	3.0	6.0		
M, N, R, S						
M	BMVW-I	MT20	5.0	6.0		
O	BS-I	MT20	5.0	6.0		
P	BMVW-I	MT20	5.0	6.0		
Q	BS-I	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
T	2260	0	2260	0
L	2260	0	2260	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
T	1862	1016 / 0	429 / 0
L	1862	1016 / 0	429 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-R, H-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 41	-77.4	-77.4	0.09 (1)	10.00	S-C	-211 / 121 0.24 (1)
B-C	-1985 / 0	-77.4	-77.4	0.39 (1)	5.02	C-R	0 / 1806 0.29 (1)
C-D	-2562 / 0	-77.4	-77.4	0.81 (1)	4.00	R-D	-1042 / 0 0.93 (1)
D-E	-2938 / 0	-77.4	-77.4	0.88 (1)	3.76	D-P	0 / 530 0.09 (1)
E-F	-2938 / 0	-77.4	-77.4	0.88 (1)	3.76	P-F	-545 / 0 0.62 (1)
F-G	-2938 / 0	-77.4	-77.4	0.88 (1)	3.76	P-H	0 / 530 0.09 (1)
G-H	-2938 / 0	-77.4	-77.4	0.88 (1)	3.76	N-H	-1042 / 0 0.93 (1)
H-I	-2562 / 0	-77.4	-77.4	0.81 (1)	4.00	N-I	0 / 1806 0.29 (1)
I-J	-1985 / 0	-77.4	-77.4	0.39 (1)	5.02	M-I	-211 / 121 0.24 (1)
J-K	0 / 41	-77.4	-77.4	0.09 (1)	10.00	B-S	0 / 1338 0.30 (1)
T-B	-2210 / 0	0.0	0.0	0.17 (1)	6.87	M-J	0 / 1338 0.30 (1)
L-J	-2210 / 0	0.0	0.0	0.17 (1)	6.87		

T-S	0 / 0	-28.0 -28.0 0.13 (2)	10.00
S-R	0 / 1271	-28.0 -28.0 0.27 (2)	10.00
R-Q	0 / 2562	-28.0 -28.0 0.39 (2)	10.00
Q-P	0 / 2562	-28.0 -28.0 0.39 (2)	10.00
P-O	0 / 2562	-28.0 -28.0 0.39 (2)	10.00
O-N	0 / 2562	-28.0 -28.0 0.39 (2)	10.00
N-M	0 / 1271	-28.0 -28.0 0.27 (2)	10.00
M-L	0 / 0	-28.0 -28.0 0.13 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 43.8 PSF	

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.17")
ALLOWABLE DEFL. (TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.27")

CSI: TC=0.86 (D-F:1), BC=0.39 (P-R:2), WB=0.93 (D-R:1), SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
1667	822	2284
1658		

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.83 (J) (INPUT = 0.90)
SI METAL= 0.45 (Q) (INPUT = 1.00)



DRWG NO. TAM 20267.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T3S	1	1	TRUSS DESC.		

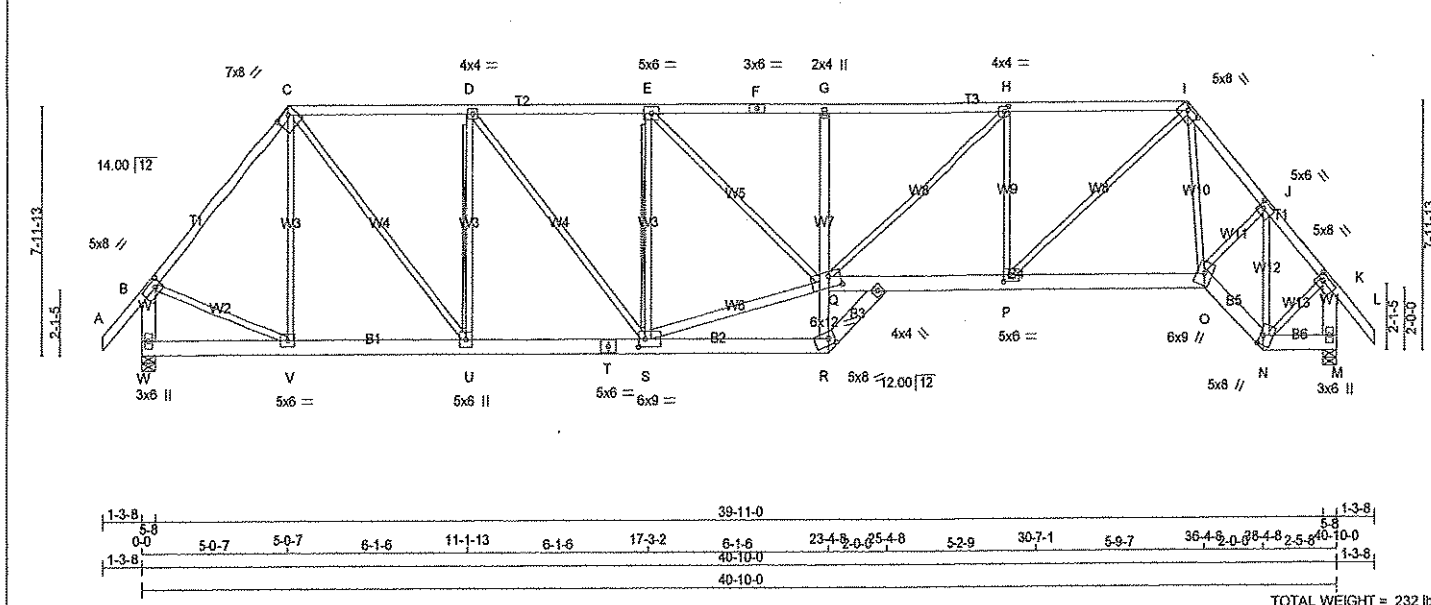
Tamarack Roof Truss, Burlington

Version 8.030 5 Oct 5 2016 Mitek Industries, Inc. Mon May 01 19:09:12 2017 Page 1

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1-3-8 5-0-7 5-0-7 6-1-6 11-1-13 6-1-6 17-3-2 6-1-6 23-4-8 25-4-8 5-2-9 30-7-1 5-2-9 35-9-9 2-6-15 38-4-8 2-5-6 40-10-0 19-8-4 2-1-8

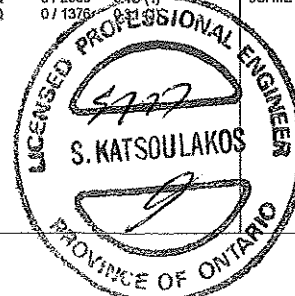
Scale = 1:72.8



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D	TOP CH. LL =	23.3	PSF
C - F	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL =	3.0	PSF
F - I	2x4	DRY	No.2	SPF	JT VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
I - L	2x4	DRY	No.2	SPF	W 2259	0	2259	0	5-8	5-8	
W - B	2x6	DRY	No.2	SPF	M 2259	0	2259	0	5-8	5-8	
M - K	2x6	DRY	No.2	SPF							
W - T	2x6	DRY	No.2	SPF							
T - R	2x6	DRY	No.2	SPF							
Q - O	2x6	DRY	No.2	SPF							
O - N	2x6	DRY	No.2	SPF							
N - M	2x6	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF							
EXCEPT											
R - G	2x4	DRY	No.2	SPF							
S - Q	2x4	DRY	No.2	SPF							
DRY: SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010			
				BRACING				THIS DESIGN COMPLIES WITH:			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.				- CSA 086-09			
				MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.				- TPIC 2011			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.				(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				2x4 DRY SPF No.2 T-BRACE AT D-U, E-S				ALLOWABLE DEFL.(LL) = L/360 (1.36")			
				FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.				CALCULATED VERT. DEFL.(LL) = L/999 (0.27")			
				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				ALLOWABLE DEFL.(TL) = L/360 (1.36")			
				LOADING				CALCULATED VERT. DEFL.(TL) = L/999 (0.44")			
				TOTAL LOAD CASES: (4)				CSI: TC=0.93 (G-H:1), BC=0.45 (P-Q:1), WB=0.59 (E-S:1), SSI=0.23 (H:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 0.50			
								AUTOSOLVE HEELS OFF			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 618 354 1667 822 2284 1656			
								PLATE PLACEMENT TOL = 0.250 inches			
								PLATE ROTATION TOL = 5.0 Deg.			
								JSI GRIP= 0.90 (I) (INPUT = 0.90)			
								JSI METAL= 0.58 (I) (INPUT = 1.00)			

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMVW-1	MT20	5.0	8.0 2.50 2.50
C TTWW-h	MT20	7.0	8.0 1.75 5.00
D TMVW-1	MT20	4.0	4.0 4.0
E TMVW-1	MT20	5.0	6.0
F TS-1	MT20	3.0	6.0
G TMVW-h	MT20	2.0	4.0
H TMVW-1	MT20	4.0	4.0 2.00 1.75
I TTWW-h	MT20	5.0	8.0 1.25 4.75
J TMVW-1	MT20	5.0	6.0 2.50 2.00
K TMVW-1	MT20	5.0	8.0 2.25 2.50
M BMV1+p	MT20	3.0	6.0
N BMVW-h	MT20	5.0	8.0 3.50 1.25
O BMVW-h	MT20	8.0	9.0
P BMVW-1	MT20	5.0	6.0 2.50 2.50
Q BWMVW-h	MT20	6.0	12.0 4.75 4.25
R BMVW-h	MT20	5.0	8.0 2.50 2.50
S BMVW-1	MT20	6.0	9.0 3.00 2.50
T BS-1	MT20	5.0	6.0
U BMVW-h	MT20	5.0	8.0
V BMVW-1	MT20	5.0	6.0
W BMV1+p	MT20	3.0	6.0
X NP-h	MT20	4.0	4.0

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0/41	-77.4	-77.4 0.11 (1)	10.00	V-C	-227/86	0.26 (1)
B-C	-1962/0	-77.4	-77.4 0.54 (1)	4.36	C-U	0/1703	0.38 (1)
C-D	-2344/0	-77.4	-77.4 0.61 (1)	3.89	U-D	-1163/0	0.50 (1)
D-E	-2650/0	-77.4	-77.4 0.68 (1)	3.52	D-S	0/817	0.18 (1)
E-F	-3666/0	-77.4	-77.4 0.69 (1)	3.14	S-E	-1391/0	0.59 (1)
F-G	-3666/0	-77.4	-77.4 0.69 (1)	3.14	R-Q	0/120	0.09 (1)
G-H	-3875/0	-77.4	-77.4 0.93 (1)	2.89	Q-G	-461/0	0.12 (1)
H-I	-3199/0	-77.4	-77.4 0.77 (1)	3.27	Q-H	0/902	0.20 (1)
I-J	-2727/0	-77.4	-77.4 0.17 (1)	4.08	P-H	-1130/0	0.59 (1)
J-K	-1710/0	-77.4	-77.4 0.09 (1)	5.02	P-I	0/2000	0.45 (1)
K-L	0/41	-77.4	-77.4 0.11 (1)	10.00	I-O	0/440	0.10 (1)
W-B	-2207/0	0.0	0.0 0.17 (1)	6.87	O-J	0/1018	0.23 (1)
M-K	-2225/0	0.0	0.0 0.17 (1)	6.85	N-J	-1749/0	0.56 (1)
W-V	0/0	-28.0	-28.0 0.11 (2)	10.00	B-V	0/1336	0.30 (1)
V-U	0/1289	-28.0	-28.0 0.22 (2)	10.00	N-K	0/1338	0.30 (1)
U-T	0/2344	-28.0	-28.0 0.33 (1)	10.00	S-Q	0/2983	0.48 (1)
T-S	0/2344	-28.0	-28.0 0.33 (1)	10.00	E-Q	0/1376	0.13 (1)
S-R	0/29	-28.0	-28.0 0.12 (3)	10.00			
Q-P	0/3200	-28.0	-28.0 0.45 (1)	10.00			
P-O	0/1710	-28.0	-28.0 0.31 (2)	10.00			
O-N	0/1524	-28.0	-28.0 0.19 (1)	10.00			
N-M	0/0	-28.0	-28.0 0.02 (2)	10.00			



DRWG NO. TAN20268-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T4	9	1	TRUSS DESC.		

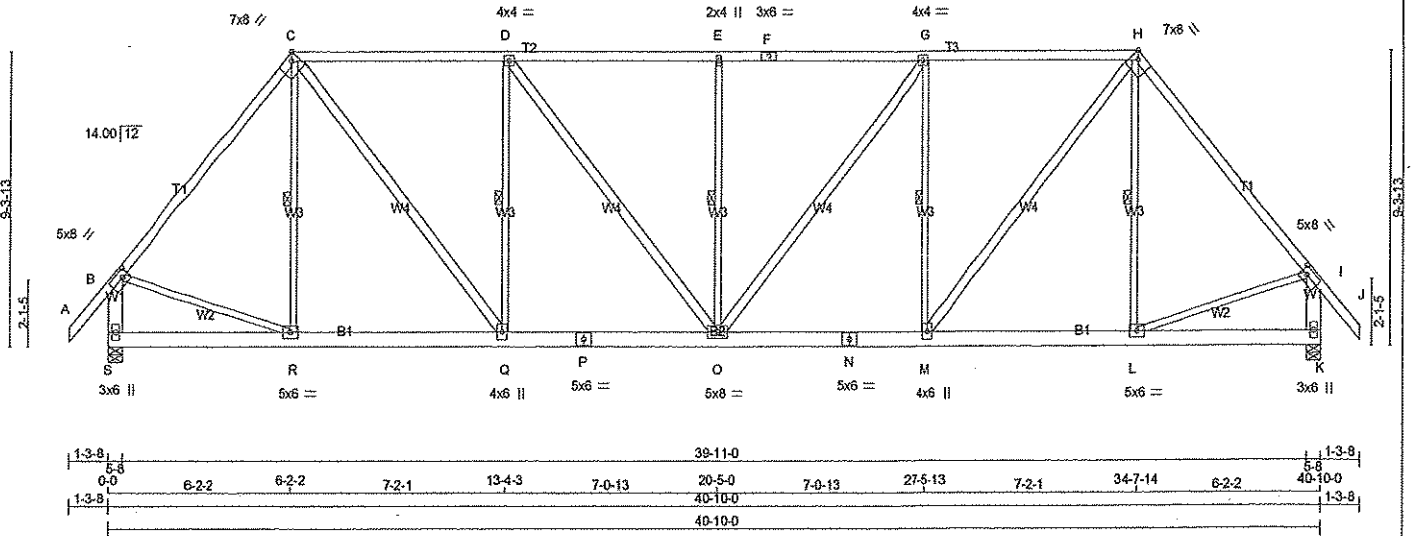
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:09:12 2017 Page 1

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1-3-8 0-0 6-2-2 6-2-2 7-2-1 13-4-3 7-0-13 20-5-0 7-0-13 27-5-13 7-2-1 34-7-14 6-2-2 40-10-0 42-1-8

Scale = 1:71.7



TOTAL WEIGHT = 9 X 234 = 2102 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF		
C - F	2x4	DRY	2100F 1.8E	SPF		
F - H	2x4	DRY	2100F 1.8E	SPF		
H - J	2x4	DRY	2100F 1.8E	SPF		
S - B	2x6	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
S - P	2x6	DRY	No.2	SPF		
P - N	2x6	DRY	No.2	SPF		
N - K	2x6	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
C - Q	2x4	DRY	No.2	SPF		
D - O	2x4	DRY	No.2	SPF		
O - G	2x4	DRY	No.2	SPF		
M - H	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	5.0	8.0	2.50	2.50
C TTWW-h	MT20	7.0	8.0	Edge	2.75
D TMVW-I	MT20	4.0	4.0		
E TMVW-w	MT20	2.0	4.0		
F TS-I	MT20	3.0	6.0		
G TMVW-I	MT20	4.0	4.0		
H TTWW-h	MT20	7.0	8.0	Edge	2.75
I TMVW-I	MT20	5.0	8.0	2.50	2.50
K BMV1+p	MT20	3.0	6.0		
L BMVW-I	MT20	5.0	6.0		
M BMVW-I	MT20	4.0	6.0		
N BS-I	MT20	5.0	6.0		
O BMVW-I	MT20	5.0	6.0		
P BS-I	MT20	5.0	6.0		
Q BMVW-I	MT20	4.0	6.0		
R BMVW-I	MT20	5.0	6.0		
S BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX
S	2260	0	2260	0	0	0	5-8	5-8	
K	2260	0	2260	0	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1882	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
K	1882	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R, D-Q, E-Q, G-M, H-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO					LENGTH	FR-TO			
A-B	0 / 41	-77.4	-77.4	0.07 (1)	10.00	R-C	-130 / 178	0.07 (1)	
B-C	-1978 / 0	-77.4	-77.4	0.50 (1)	5.27	C-Q	0 / 1496	0.24 (1)	
C-D	-2219 / 0	-77.4	-77.4	0.48 (1)	4.97	Q-D	-964 / 0	0.53 (1)	
D-E	-2493 / 0	-77.4	-77.4	0.48 (1)	4.74	D-O	0 / 441	0.07 (1)	
E-F	-2493 / 0	-77.4	-77.4	0.48 (1)	4.74	O-E	-504 / 0	0.28 (1)	
F-G	-2493 / 0	-77.4	-77.4	0.48 (1)	4.74	O-G	0 / 441	0.07 (1)	
G-H	-2219 / 0	-77.4	-77.4	0.48 (1)	4.97	M-G	-964 / 0	0.53 (1)	
H-I	-1978 / 0	-77.4	-77.4	0.50 (1)	5.27	M-H	0 / 1496	0.24 (1)	
I-J	0 / 41	-77.4	-77.4	0.07 (1)	10.00	L-H	-130 / 178	0.07 (1)	
S-B	-2191 / 0	0.0	0.0	0.17 (1)	6.90	B-R	0 / 1327	0.30 (1)	
K-I	-2191 / 0	0.0	0.0	0.17 (1)	6.90	L-I	0 / 1327	0.30 (1)	
S-R	0 / 0	-28.0	-28.0	0.14 (3)	10.00				
R-Q	0 / 1282	-28.0	-28.0	0.25 (2)	10.00				
Q-P	0 / 2220	-28.0	-28.0	0.33 (1)	10.00				
P-O	0 / 2220	-28.0	-28.0	0.33 (1)	10.00				
O-N	0 / 2220	-28.0	-28.0	0.33 (1)	10.00				
N-M	0 / 2220	-28.0	-28.0	0.33 (1)	10.00				
M-L	0 / 1282	-28.0	-28.0	0.25 (2)	10.00				
L-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 43.8	PSF

SPACING = 240 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.50 (B-C:1), BC=0.33 (O-Q:1), WB=0.53 (G-M:1), SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

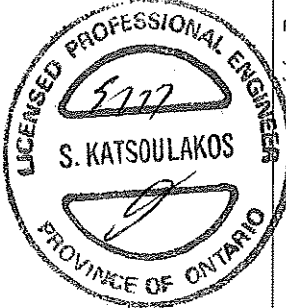
NAIL VALUES

PLATE GRIP(ORY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822
			2284	1656

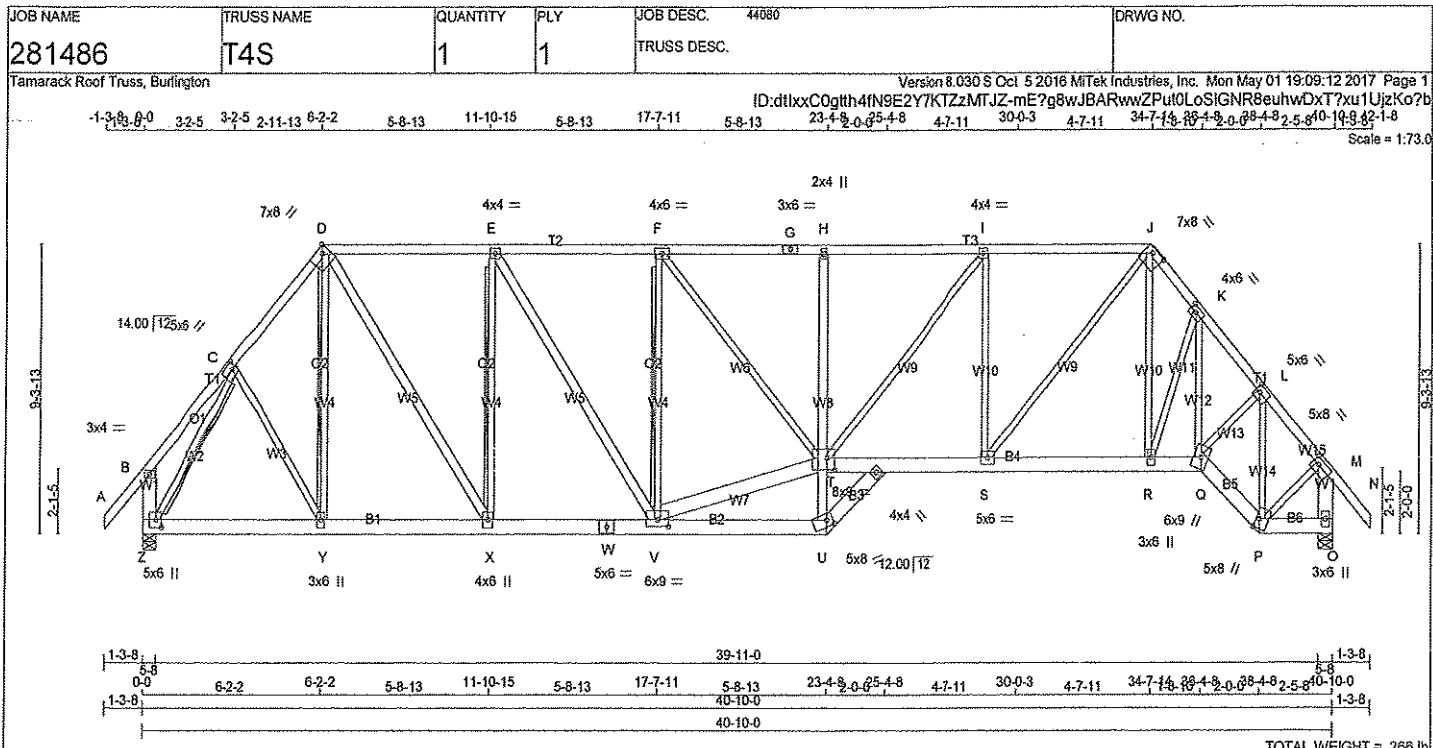
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)



DWG NO. TAM2016-11
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - N	2x4	DRY	No.2
N - B	2x6	DRY	No.2
O - M	2x6	DRY	No.2
M - W	2x6	DRY	No.2
W - U	2x6	DRY	No.2
U - Q	2x6	DRY	No.2
Q - P	2x6	DRY	No.2
P - O	2x6	DRY	No.2

ALL WEBS EXCEPT

D - X	2x4	DRY	No.2
E - V	2x4	DRY	No.2
U - H	2x4	DRY	No.2
V - T	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-I	MT20	5.0	6.0	2.50	1.75
D	TTWW-h	MT20	7.0	8.0	Edge	2.75
E	TMWW-I	MT20	4.0	4.0		
F	TMWW-I	MT20	4.0	6.0		
G	TS-I	MT20	3.0	6.0		
H	TMW+u	MT20	2.0	4.0		
I	TMWW-I	MT20	4.0	4.0		
J	TTWW-h	MT20	7.0	8.0	1.75	5.00
K	TMWW-I	MT20	4.0	6.0	2.00	2.75
L	TMWW-I	MT20	5.0	6.0	2.50	2.00
M	TMWW-I	MT20	5.0	8.0	2.25	2.50
O	BMV-I+p	MT20	3.0	6.0		
P	BBWW+u	MT20	5.0	8.0	3.50	1.25
Q	BBWW+u	MT20	6.0	9.0		
R	BMWW-I	MT20	3.0	6.0		
S	BMWW-I	MT20	5.0	6.0		
T	BMWWWW-I	MT20	8.0	9.0	4.50	3.00
U	BBW-m	MT20	5.0	8.0	2.50	2.50
V	BMWWWW-I	MT20	6.0	9.0	2.50	4.50
W	BS-I	MT20	5.0	6.0		
X	BMWW+u	MT20	4.0	6.0		
Y	BMWW+u	MT20	3.0	6.0		
Z	BMWW+u	MT20	5.0	6.0	3.00	2.25
AA	NP+u	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
O	2259	0	2259	0
Z	2259	0	2259	0

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
O	1661	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0
Z	1662	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, Z

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-Y
2x4 DRY SPF No.2 T-BRACE AT E-X, F-V, C-Z

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED	MAX. FACTORED
	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
FR-TO	FROM TO	LENGTH FR-TO
A-B	0 / 41	-77.4 -77.4 0.11 (1)
B-C	0 / 18	-77.4 -77.4 0.10 (1)
C-D	-1993 / 0	-77.4 -77.4 0.16 (1)
D-E	-2070 / 0	-77.4 -77.4 0.50 (1)
E-F	-2435 / 0	-77.4 -77.4 0.53 (1)
F-G	-3123 / 0	-77.4 -77.4 0.49 (1)
G-H	-3123 / 0	-77.4 -77.4 0.49 (1)
H-I	-3126 / 0	-77.4 -77.4 0.62 (1)
I-J	-2661 / 0	-77.4 -77.4 0.58 (1)
J-K	-2654 / 0	-77.4 -77.4 0.10 (1)
K-L	-2734 / 0	-77.4 -77.4 0.14 (1)
L-M	-1710 / 0	-77.4 -77.4 0.10 (1)
M-N	0 / 41	-77.4 -77.4 0.11 (1)
N-O	-203 / 0	0.0 0.0 0.02 (1)
O-M	-2225 / 0	0.0 0.0 0.17 (1)
Z-Y	0 / 1158	-28.0 -28.0 0.22 (2)
Y-X	0 / 1278	-28.0 -28.0 0.23 (2)
X-W	0 / 2070	-28.0 -28.0 0.30 (1)
W-V	0 / 2070	-28.0 -28.0 0.30 (1)
V-U	0 / 17	-28.0 -28.0 0.11 (3)
U-T	0 / 2861	-28.0 -28.0 0.37 (1)
T-S	0 / 1649	-28.0 -28.0 0.26 (1)
S-R	0 / 1767	-28.0 -28.0 0.27 (1)
R-Q	0 / 1521	-28.0 -28.0 0.19 (1)
Q-P	0 / 0	-28.0 -28.0 0.02 (2)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.62 (H-I:1), BC=0.37 (S-T:1), WB=0.94 (I-S:1), SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

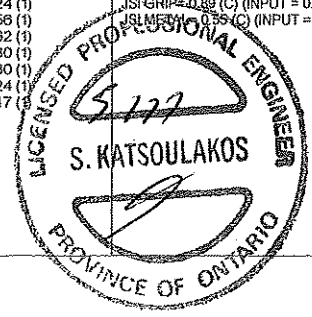
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

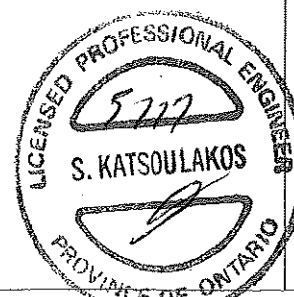
PLATE PLACEMENT TOL = 0.250 inches

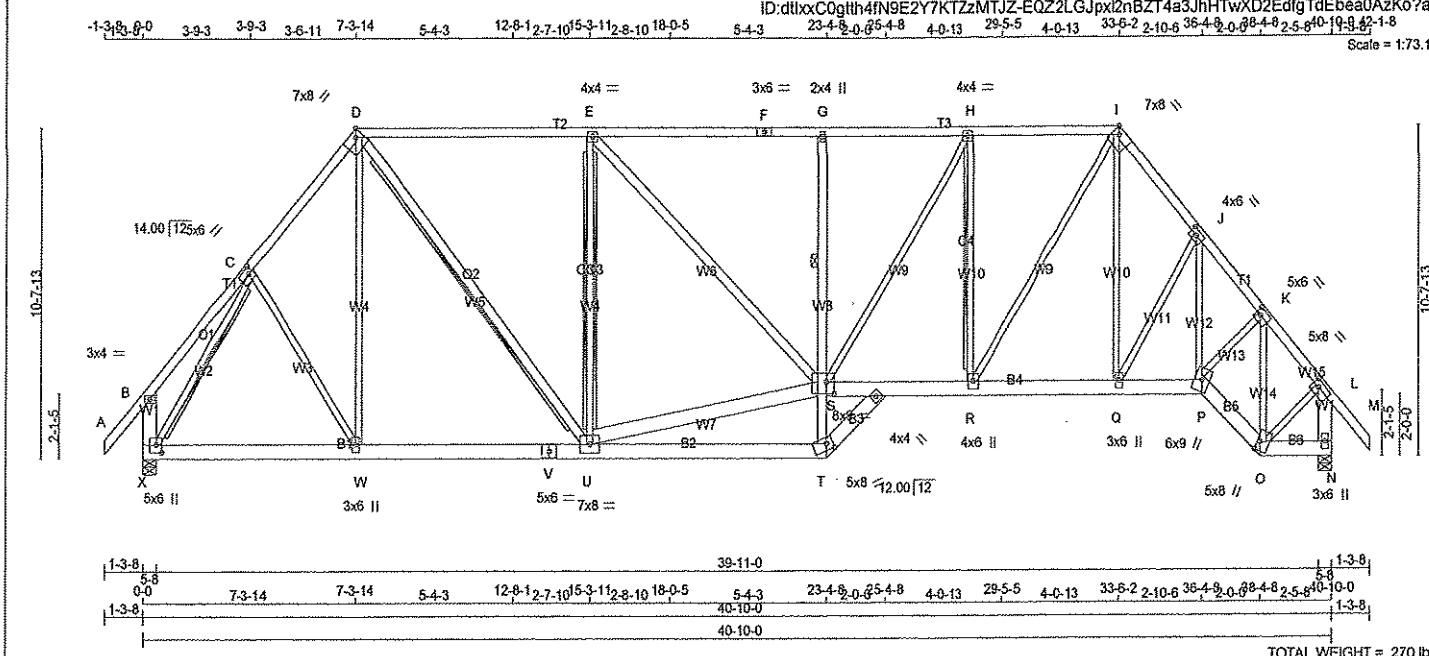
PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP = 0.88 (C) (INPUT = 0.90)
ISI MEMB = 0.95 (C) (INPUT = 1.00)



DRWG NO. TAM20270-11
STRUCTURAL
COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	1650F 1.5E
D - F	2x4	DRY	1650F 1.5E
F - I	2x4	DRY	1650F 1.5E
I - M	2x4	DRY	1650F 1.5E
X - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
X - V	2x6	DRY	No.2
V - T	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - O	2x6	DRY	No.2
O - N	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
T - G	2x4	DRY	No.2
D - U	2x4	DRY	No.2
U - S	2x6	DRY	No.2
E - S	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW-h	MT20	7.0	8.0	Edge	2.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW-w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMWW-t	MT20	4.0	6.0	2.00	2.75
K	TMWW-t	MT20	5.0	6.0	2.50	2.00
L	TMWW-t	MT20	5.0	8.0	2.25	2.50
N	BMV1+p	MT20	3.0	6.0		
O	BBWW+m	MT20	5.0	8.0	3.50	1.25
P	BBWW+m	MT20	6.0	9.0		
Q	BMWW-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMWWWW-t	MT20	8.0	9.0	4.50	3.00
T	BBW-m	MT20	5.0	8.0	2.50	2.50
U	BMWWWW-t	MT20	7.0	8.0		
V	BS-t	MT20	5.0	6.0		
W	BMWW-h	MT20	3.0	6.0		
X	BMV1+p	MT20	5.0	6.0	3.00	2.25
Y	NP-h	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT DOWN	REQD BRG IN-SX
JT	VERT	VERT	0	5-8
N	2260	0	0	5-8
X	2260	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE COMBINED	SNOW	MAX/MIN LIVE	PERM LIVE	WIND	DEAD	SOIL
JT	1852	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0
X	1852	1016 / 0	429 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, X

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.25 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-S.

2x4 DRY SPF No.2 T-BRACE AT H-R, C-X

2x4 DRY SPF No.2 I-BRACE AT E-U

2x3 DRY SPF No.2 T-BRACE AT D-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 41	-77.4	-77.4 0.09 (1)	C-W	0 / 179
B-C	0 / 23	-77.4	-77.4 0.13 (1)	W-D	0 / 261
C-D	-1993 / 0	-77.4	-77.4 0.17 (1)	T-S	0 / 157
D-E	-2065 / 0	-77.4	-77.4 0.83 (1)	S-G	-482 / 0
E-F	-2631 / 0	-77.4	-77.4 0.92 (1)	R-H	-968 / 0
F-G	-2631 / 0	-77.4	-77.4 0.92 (1)	R-I	0 / 1328
G-H	-2636 / 0	-77.4	-77.4 0.37 (1)	Q-I	0 / 457
H-I	-2284 / 0	-77.4	-77.4 0.26 (1)	Q-J	-406 / 0
I-J	-2444 / 0	-77.4	-77.4 0.10 (1)	P-J	0 / 356
J-K	-2729 / 0	-77.4	-77.4 0.08 (1)	P-K	0 / 1084
K-L	-1710 / 0	-77.4	-77.4 0.07 (1)	O-K	-1741 / 0
L-M	0 / 41	-77.4	-77.4 0.09 (1)	X-C	-2297 / 0
X-B	-218 / 0	0.0	0.0 0.02 (1)	O-L	0 / 1333
N-L	-2226 / 0	0.0	0.0 0.17 (1)	U-E	-1291 / 0
X-W	0 / 1203	-28.0	-28.0 0.28 (2)	D-U	0 / 1290
W-V	0 / 1277	-28.0	-28.0 0.32 (2)	U-S	0 / 1219
V-U	0 / 1277	-28.0	-28.0 0.32 (2)	E-S	0 / 804
U-T	0 / 11	-28.0	-28.0 0.21 (3)	S-H	0 / 667
S-R	0 / 2284	-28.0	-28.0 0.31 (1)		
R-Q	0 / 1578	-28.0	-28.0 0.23 (1)		
Q-P	0 / 1775	-28.0	-28.0 0.25 (1)		
P-O	0 / 1518	-28.0	-28.0 0.19 (1)		
O-N	0 / 0	-28.0	-28.0 0.02 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL) = L/360 (1.36")

CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.92 (E-G:1), BC=0.32 (U-W:2), WB=0.92 (E-U:1), SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (X) (INPUT = 0.90)

JSI METAL= 0.56 (C) (INPUT = 1.00)

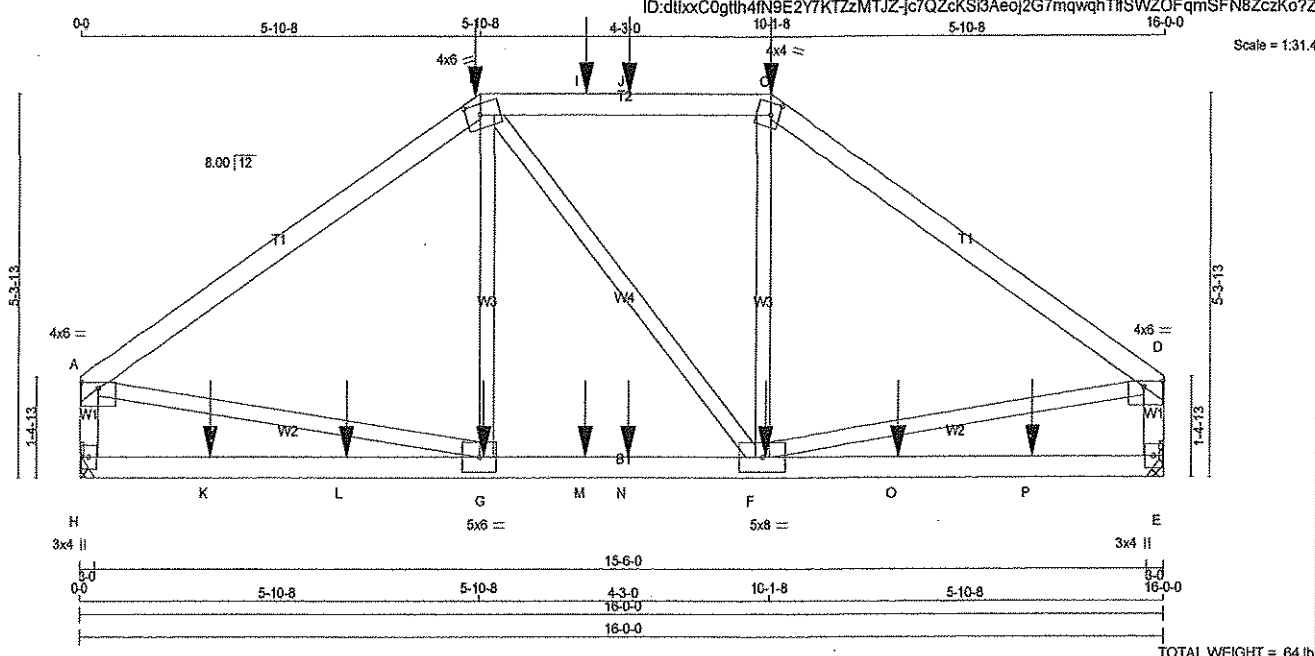
DRWG NO. YAM 2017-19

STRUCTURAL COMPONENT ONLY

PROFESSIONAL ENGINEER

S. KATSOUKAKOS

PROVINCE OF ONTARIO



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF
B - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
H - A	2x4 DRY	No.2	SPF
H - D	2x4 DRY	No.2	SPF
H - E	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	8.0	1.00	3.25
B	TTWW-m	MT20	4.0	8.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0	2.00	1.50
D	TMVW-p	MT20	4.0	8.0	1.00	3.25
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	5.0	8.0		
G	BMVWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 492.3 lbs FACTORED DOWN AT 5-10-8, 149.9 lbs FACTORED DOWN AT 7-5-4, AND 149.9 lbs FACTORED DOWN AT 8-0-12, AND 492.3 lbs FACTORED DOWN AT 10-1-8 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 87.4 lbs FACTORED DOWN AT 5-11-4, 87.4 lbs FACTORED DOWN AT 7-5-4, 87.4 lbs FACTORED DOWN AT 8-0-12, 87.4 lbs FACTORED DOWN AT 10-0-12, AND 87.4 lbs FACTORED DOWN AT 12-0-12, AND 87.4 lbs FACTORED DOWN AT 14-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1691	0	1691	0
E	1678	0	1678	0

MIN. SEAT SIZE: 3-0 HANGER BY OTHERS

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	1382	760 / 0	319 / 0	0 / 0	0 / 0	313 / 0	0 / 0
E	1382	754 / 0	316 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1916 / 0	-77.4	-77.4 0.70 (1)	G-B	0 / 417	0.10 (3)	
B-I	-1580 / 0	-77.4	-77.4 0.74 (1)	B-F	-20 / 0	0.01 (1)	
I-J	-1580 / 0	-77.4	-77.4 0.74 (1)	F-C	0 / 410	0.10 (3)	
J-C	-1580 / 0	-77.4	-77.4 0.74 (1)	A-G	0 / 1618	0.40 (1)	
C-D	-1900 / 0	-77.4	-77.4 0.70 (1)	F-D	0 / 1604	0.40 (1)	
H-A	-1580 / 0	0.0	0.0 0.18 (1)				
E-D	-1566 / 0	0.0	0.0 0.18 (1)				
H-K	0 / 0	-28.0	-28.0 0.42 (2)				
K-L	0 / 0	-28.0	-28.0 0.42 (2)				
L-G	0 / 0	-28.0	-28.0 0.42 (2)				
G-M	0 / 1593	-28.0	-28.0 0.60 (2)				
M-N	0 / 1593	-28.0	-28.0 0.60 (2)				
N-F	0 / 1593	-28.0	-28.0 0.60 (2)				
F-O	0 / 0	-28.0	-28.0 0.43 (2)				
O-P	0 / 0	-28.0	-28.0 0.43 (2)				
P-E	0 / 0	-28.0	-28.0 0.43 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-10-8	-492	-492		FRONT	VERT	TOTAL
C	10-1-8	-492	-492		FRONT	VERT	TOTAL
F	10-0-12	-50	-87		FRONT	VERT	TOTAL
G	5-11-4	-50	-87		FRONT	VERT	TOTAL
I	7-5-4	-150	-150		FRONT	VERT	TOTAL
J	8-0-12	-150	-150		FRONT	VERT	TOTAL
K	1-11-4	-50	-87		FRONT	VERT	TOTAL
L	3-11-4	-50	-87		FRONT	VERT	TOTAL
M	7-5-4	-50	-87		FRONT	VERT	TOTAL
N	8-0-12	-50	-87		FRONT	VERT	TOTAL
O	12-0-12	-50	-87		FRONT	VERT	TOTAL
P	14-0-12	-50	-87		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	23.3	
BOT CH.	LL	PSF
	10.5	
	7.0	
TOTAL LOAD	43.8	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")

CALCULATED VERT. DEFL.(LL) = L/999 (0.11")

ALLOWABLE DEFL.(TL)= L/360 (0.53")

CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.74 (B-C:1), BC=0.60 (F-G:2), WB=0.40 (A-G:1), SS=0.28 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

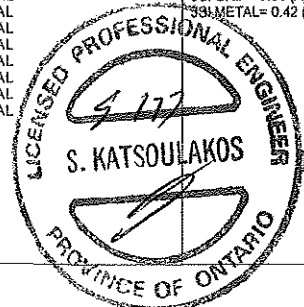
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

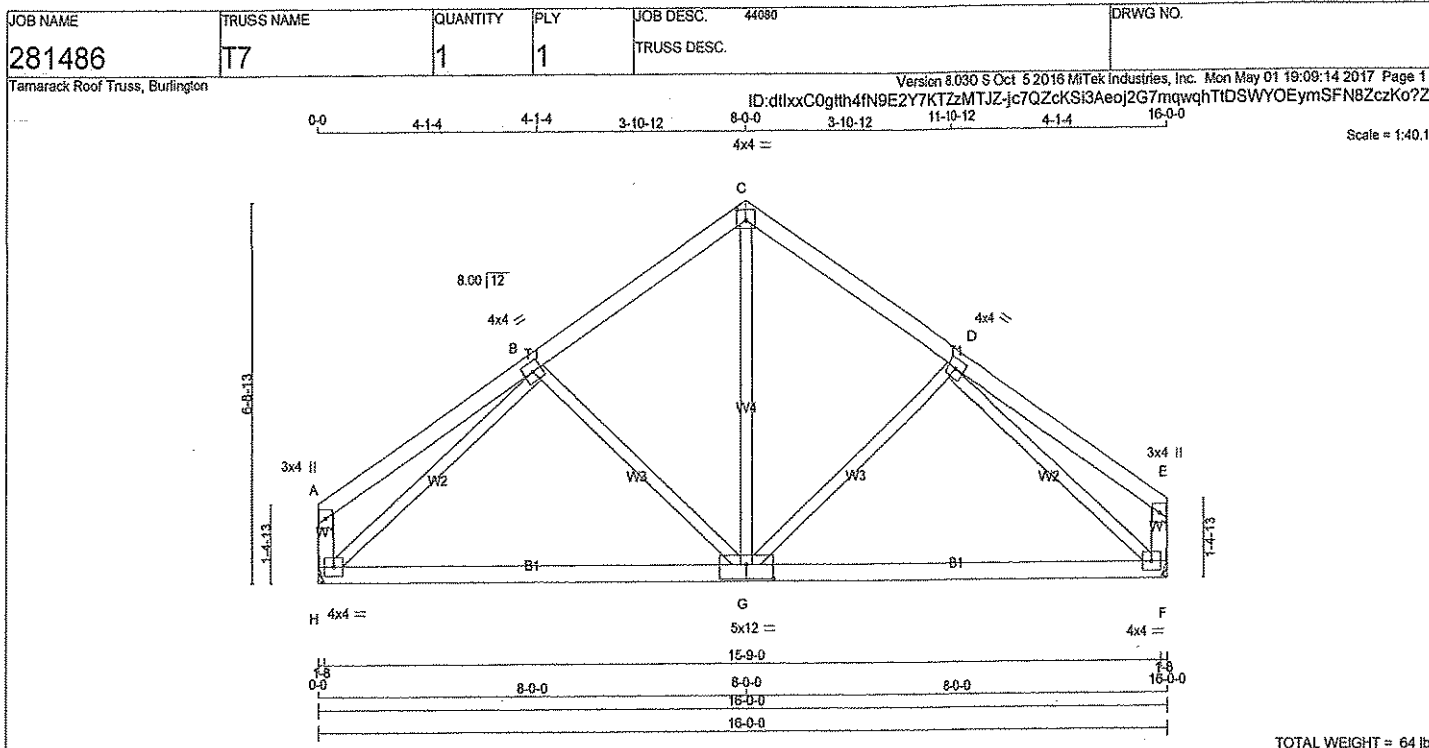
JSI GRIP= 0.89 (A) (INPUT = 0.90)

JSI METAL= 0.42 (A) (INPUT = 1.00)



DRWG NO. TAN2023-17

STRUCTURAL COMPONENT ONLY



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BSWWW-t	MT20	5.0	12.0	3.00	6.00
H	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	843	0	843	0	0	HANGER BY OTHERS
F	843	0	843	0	0	HANGER BY OTHERS

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
H	701	373 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0
F	701	373 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB. MAX. FACTORED FORCE (LBS)		MAX. FACTORED CSI (LC)	
FR-TO		FROM TO				FR-TO	
A-B	0 / 20	-77.4 -77.4	0.20 (1)	10.00	G-C	0 / 513	0.12 (2)
B-C	-692 / 0	-77.4 -77.4	0.15 (1)	6.25	G-D	-167 / 43	0.08 (1)
C-D	-692 / 0	-77.4 -77.4	0.15 (1)	6.25	B-G	-167 / 43	0.08 (1)
D-E	0 / 20	-77.4 -77.4	0.20 (1)	10.00	H-B	-934 / 0	0.46 (1)
H-A	-119 / 0	0.0 0.0	0.01 (1)	7.81	D-F	-934 / 0	0.46 (1)
F-E	-119 / 0	0.0 0.0	0.01 (1)	7.81			
H-G	0 / 682	-28.0 -28.0	0.60 (2)	10.00			
G-F	0 / 682	-28.0 -28.0	0.60 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/966 (0.20")

CSI: TC=0.20 (A-B:1), BC=0.60 (G-H:2), WB=0.46 (B-H:1), SSI=0.19 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

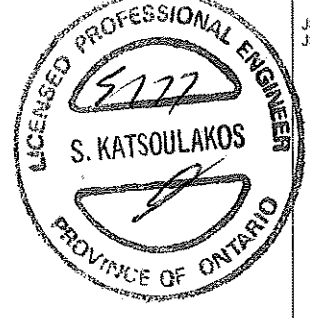
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1666

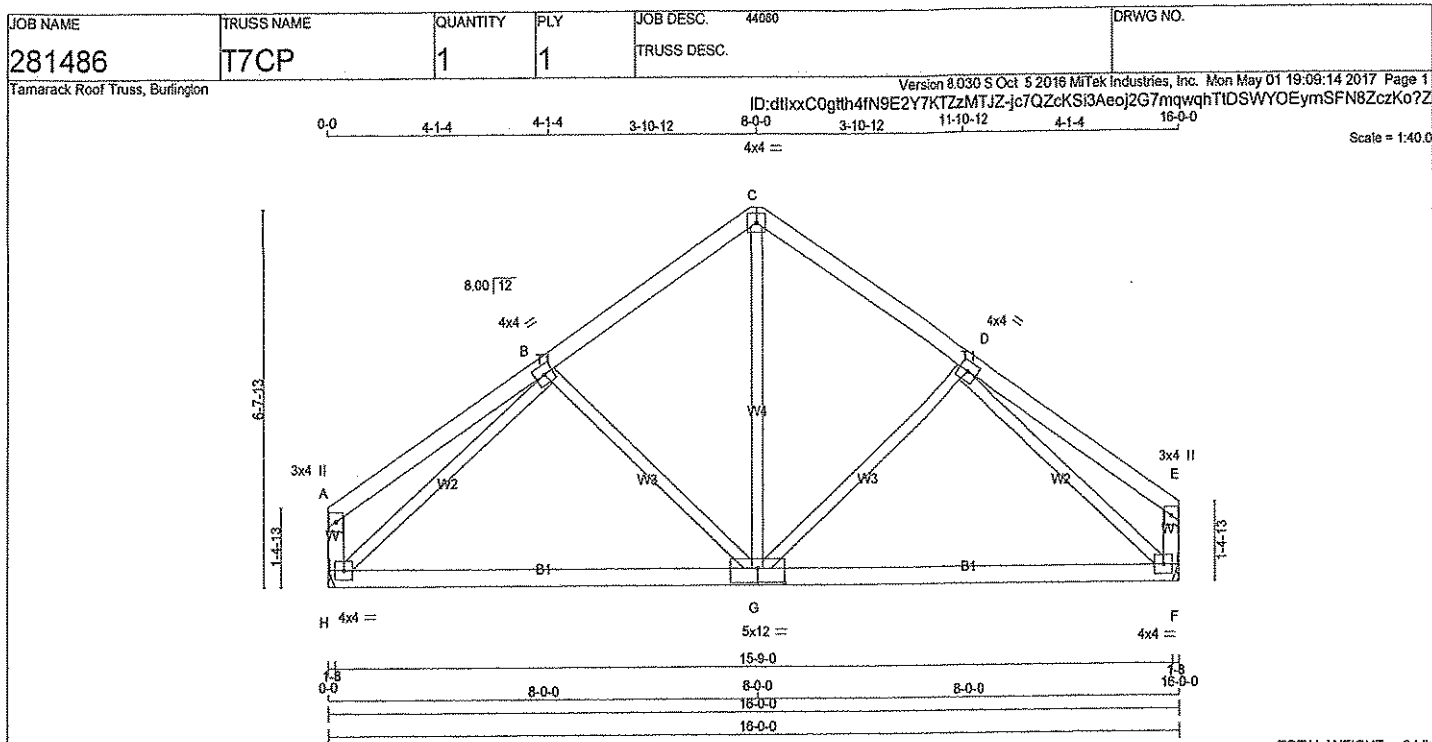
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.80)
JSI METAL= 0.60 (G) (INPUT = 1.00)



DRWG NO. TAM20274-17
STRUCTURAL COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
H - A	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0			
B	TMVW-1	MT20	4.0	4.0			
C	TTW-p	MT20	4.0	4.0			
D	TMVW-1	MT20	4.0	4.0			
E	TMV+p	MT20	3.0	4.0			
F	BMVW-1	MT20	4.0	4.0			
G	BSVW-1	MT20	5.0	12.0	3.00	6.00	
H	BMVW-1	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	843	0	843	0
F	843	0	843	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	701	373 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0
F	701	373 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO										
A-B	0 / 20	-77.4	-77.4	0.20	(1)	10.00	G-C	0 / 513	0.12	(2)
B-C	-692 / 0	-77.4	-77.4	0.15	(1)	6.25	G-D	-167 / 43	0.08	(1)
C-D	-692 / 0	-77.4	-77.4	0.15	(1)	6.25	B-G	-167 / 43	0.08	(1)
D-E	0 / 20	-77.4	-77.4	0.20	(1)	10.00	H-B	-934 / 0	0.46	(1)
H-A	-119 / 0	0.0	0.0	0.01	(1)	7.81	D-F	-934 / 0	0.46	(1)
F-E	-119 / 0	0.0	0.0	0.01	(1)	7.81				
H-G	0 / 682	-28.0	-28.0	0.60	(2)	10.00				
G-F	0 / 682	-28.0	-28.0	0.60	(2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, ECBC 2012, ABC 2014
- CSA 086-09
- TPI 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/988 (0.20")

CSI: TC=0.20 (A-B:1), BC=0.60 (G-H:2), WB=0.46 (B-H:1), SSI=0.19 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)



DWG NO. TAM 20275-17
STRUCTURAL
COMPONENT ONLY

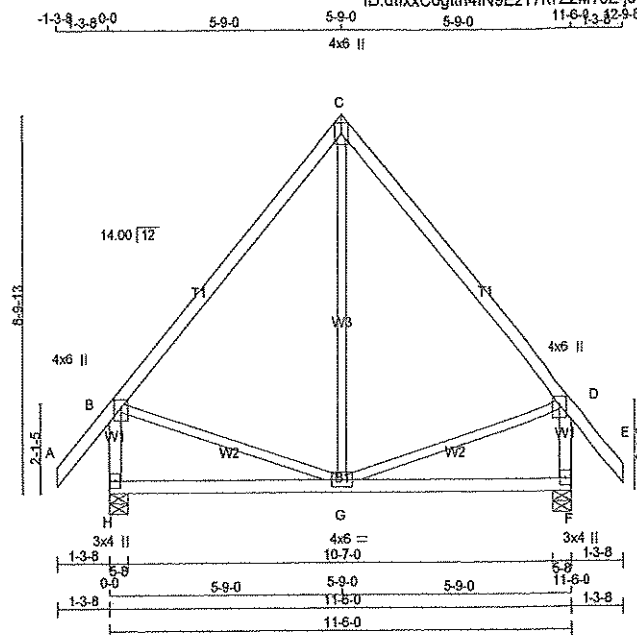
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T8	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8030 5 Oct 5 2016 Mitek Industries, Inc. Mon May 01 19:09:14 2017 Page 1

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Scale = 1:53.0



TOTAL WEIGHT = 3 X 58 = 175 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
H - B	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
H - F	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	4.0	6.0	2.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-1	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	714	0	714	0	5-8	5-8
F	714	0	714	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
H	577	333 / 0	121 / 0	0 / 0	0 / 0	123 / 0	0 / 0
F	577	333 / 0	121 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO					
A-B	0 / 41	-77.4	-77.4	0.11 (1)	10.00	G-C	0 / 270	0.06 (3)
B-C	-337 / 0	-77.4	-77.4	0.33 (1)	6.25	B-G	0 / 229	0.05 (1)
C-D	-337 / 0	-77.4	-77.4	0.33 (1)	6.25	G-D	0 / 229	0.05 (1)
D-E	0 / 41	-77.4	-77.4	0.11 (1)	10.00			
H-B	-553 / 0	0.0	0.0	0.08 (1)	7.81			
F-D	-553 / 0	0.0	0.0	0.08 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.27 (3)	10.00			
G-F	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.38")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.38")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.33 (C-D:1), BC=0.27 (G-H:3), WB=0.06 (C-G:3), SSI=0.14 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

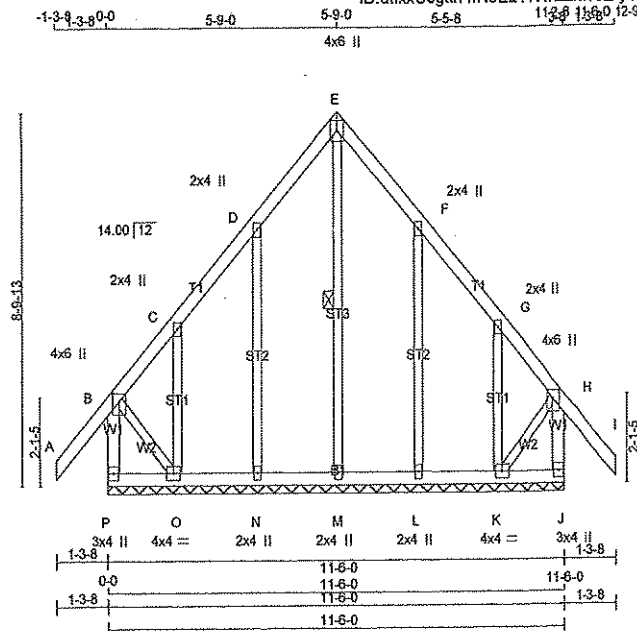
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



DWG NO. TAN20276.19
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 68 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 P - B 2x4 DRY No.2 SPF
 A - E 2x4 DRY No.2 SPF
 E - I 2x4 DRY No.2 SPF
 J - H 2x4 DRY No.2 SPF
 P - J 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
 ALL GABLE WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.25	2.00
C, D, F, G					
E TMVW+p	MT20	2.0	4.0		
H TMVW+p	MT20	4.0	6.0		
J BMV1+p	MT20	4.0	6.0	2.25	2.00
K BMVW1-t	MT20	4.0	4.0		
L, M, N					
L BMV1+w	MT20	2.0	4.0		
O BMVW1-t	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM TO			FR-TO		
P-B	-247 / 0	0.0	0.0	0.03 (1)	M-E	-107 / 0	0.05 (1)
A-B	0 / 41	-77.4	-77.4	0.11 (1)	N-D	-187 / 0	0.13 (1)
B-C	-55 / 0	-77.4	-77.4	0.10 (1)	O-C	-82 / 0	0.02 (1)
C-D	-9 / 0	-77.4	-77.4	0.05 (1)	L-F	-187 / 0	0.13 (1)
D-E	-28 / 0	-77.4	-77.4	0.05 (1)	K-G	-82 / 0	0.02 (1)
E-F	-28 / 0	-77.4	-77.4	0.05 (1)	B-O	0 / 22	0.00 (1)
F-G	-9 / 0	-77.4	-77.4	0.05 (1)	K-H	0 / 22	0.00 (1)
G-H	-55 / 0	-77.4	-77.4	0.10 (1)			
H-I	0 / 41	-77.4	-77.4	0.11 (1)			
J-H	-247 / 0	0.0	0.0	0.03 (1)			
P-O	0 / 0	-28.0	-28.0	0.02 (2)			
O-N	0 / 13	-28.0	-28.0	0.02 (2)			
N-M	0 / 9	-28.0	-28.0	0.02 (2)			
M-L	0 / 9	-28.0	-28.0	0.02 (2)			
L-K	0 / 13	-28.0	-28.0	0.02 (2)			
K-J	0 / 0	-28.0	-28.0	0.02 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (H-I:1), BC=0.02 (K-L:2), WB=0.13 (F-L:1), SSI=0.06 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (D) (INPUT = 0.90)
 JSI METAL= 0.08 (D) (INPUT = 1.00)



DWG NO. TAM 2027-19
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	G9	1	1	TRUSS DESC.		

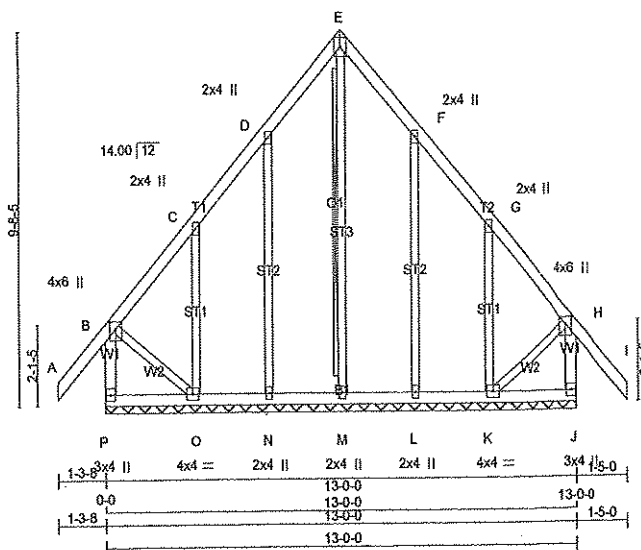
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Mon May 01 19:09:07 2017 Page 1

ID:dltxxC0gth4fN9E2Y7KTZzQGcN5DF2MvldTe7wDoCH2Dgh0dGef8vkrfBpWzKo?g

-1-3-8-0-0 6-6-0 6-6-0 6-2-8 12-8 13-0-0 14-5-0
4x6 II

Scale = 1:58.7



TOTAL WEIGHT = 77 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
P - B	2x4 DRY	No.2	SPF
A - E	2x4 DRY	No.2	SPF
E - I	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
P - J	2x4 DRY	No.2	SPF

ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER			

GABLE STUDS SPACED AT 2-0-0 O.C.

PLATES (table is in inches)			
JT	TYPE	PLATES	W L E N Y X
B	TMVW+p	MT20	4.0 6.0 2.25 2.00
C, D, F, G			
C	TMVW+w	MT20	2.0 4.0
E	TTW+p	MT20	4.0 6.0
H	TMVW+p	MT20	4.0 6.0 2.25 2.00
J	BMV1+p	MT20	3.0 4.0
K	BMVW1-t	MT20	4.0 4.0
L, M, N			
L	BMV1+w	MT20	2.0 4.0
O	BMVW1-t	MT20	4.0 4.0
P	BMV1+p	MT20	3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
2x4 DRY SPF No.2 T-BRACE AT E-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-229 / 0	0.0 0.0 0.03 (1)	7.81	M-E	-98 / 0	0.07 (1)	
A-B	0 / 41	-77.4 -77.4 0.11 (1)	10.00	N-D	-165 / 0	0.16 (1)	
B-C	-24 / 0	-77.4 -77.4 0.06 (1)	6.25	O-C	-187 / 0	0.07 (1)	
C-D	-34 / 0	-77.4 -77.4 0.06 (1)	6.25	L-F	-165 / 0	0.16 (1)	
D-E	-37 / 0	-77.4 -77.4 0.04 (1)	6.25	K-G	-187 / 0	0.07 (1)	
E-F	-37 / 0	-77.4 -77.4 0.04 (1)	6.25	B-O	0 / 29	0.01 (1)	
F-G	-34 / 0	-77.4 -77.4 0.06 (1)	6.25	K-H	0 / 29	0.01 (1)	
G-H	-24 / 0	-77.4 -77.4 0.06 (1)	6.25				
H-I	0 / 45	-77.4 -77.4 0.13 (1)	10.00				
J-H	-238 / 0	0.0 0.0 0.03 (1)	7.81				
P-O	0 / 0	-28.0 -28.0 0.04 (2)	10.00				
O-N	0 / 19	-28.0 -28.0 0.04 (2)	10.00				
N-M	0 / 17	-28.0 -28.0 0.03 (2)	10.00				
M-L	0 / 17	-28.0 -28.0 0.03 (2)	10.00				
L-K	0 / 19	-28.0 -28.0 0.04 (2)	10.00				
K-J	0 / 0	-28.0 -28.0 0.04 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13 (H-I:1), BC=0.04 (K-L:2), WB=0.16 (D-N:1), SSI=0.06 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.16 (C) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



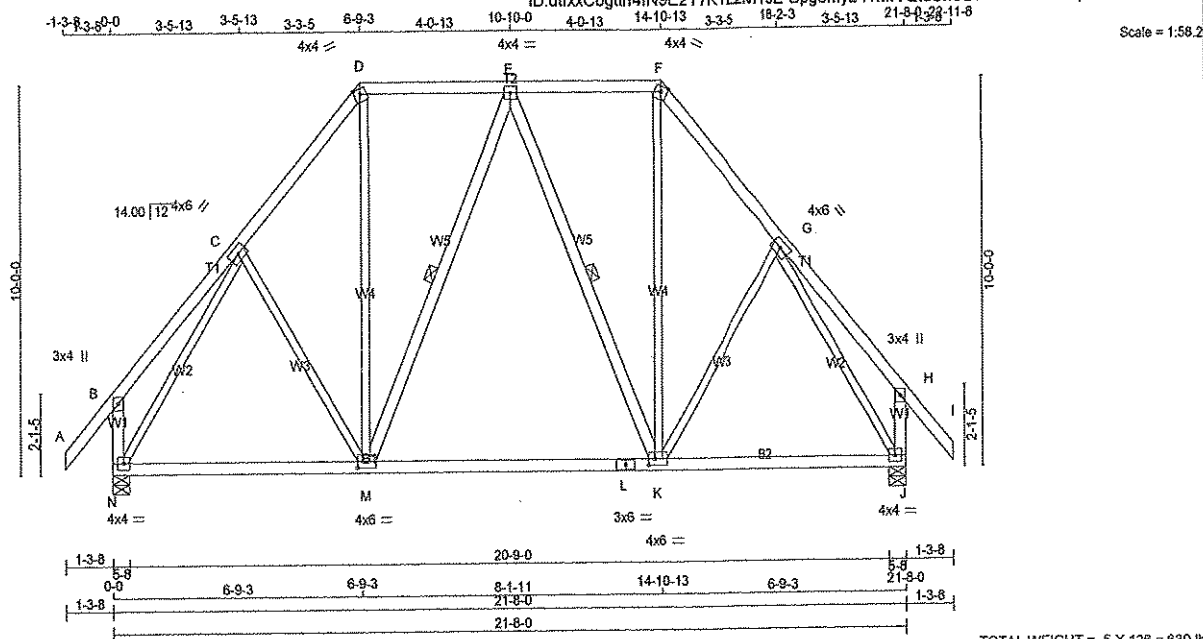
DWG NO. TAM 2027817
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	T10	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Mon May 01 19:09:15 2017 Page 1

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TOTAL WEIGHT = 5 X 126 = 630 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - 8	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
M - E	2x4	DRY	No.2
E - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0	Edge	
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		
K	BMVWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMVWW-t	MT20	4.0	6.0	2.00	1.50
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ
N	1250	0	1250	0
J	1250	0	1250	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1022	570 / 0	228 / 0	0 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
J	1022	570 / 0	228 / 0	0 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	
FR-TO		FROM	TO	FR-TO		FROM	TO	
A-B	0 / 41	-77.4	-77.4	0.11 (1)	10.00	C-M	-17 / 78	0.02 (3)
B-C	0 / 22	-77.4	-77.4	0.13 (1)	10.00	M-D	0 / 432	0.10 (1)
C-D	-892 / 0	-77.4	-77.4	0.11 (1)	6.25	M-E	-204 / 0	0.12 (1)
D-E	-567 / 0	-77.4	-77.4	0.16 (1)	6.25	E-K	-204 / 0	0.12 (1)
E-F	-567 / 0	-77.4	-77.4	0.16 (1)	6.25	K-F	0 / 432	0.10 (1)
F-G	-892 / 0	-77.4	-77.4	0.11 (1)	6.25	K-G	-17 / 78	0.02 (3)
G-H	0 / 22	-77.4	-77.4	0.13 (1)	10.00	N-C	-1124 / 0	0.87 (1)
H-I	0 / 41	-77.4	-77.4	0.11 (1)	10.00	G-J	-1124 / 0	0.87 (1)
N-B	-208 / 0	0.0	0.0	0.02 (1)	7.81			
J-H	-208 / 0	0.0	0.0	0.02 (1)	7.81			
N-M	0 / 572	-28.0	-28.0	0.43 (2)	10.00			
M-L	0 / 645	-28.0	-28.0	0.44 (2)	10.00			
L-K	0 / 645	-28.0	-28.0	0.44 (2)	10.00			
K-J	0 / 572	-28.0	-28.0	0.43 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.16 (D-E:1), BC=0.44 (K-M:2), WB=0.87 (C-N:1), SSI=0.16 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)



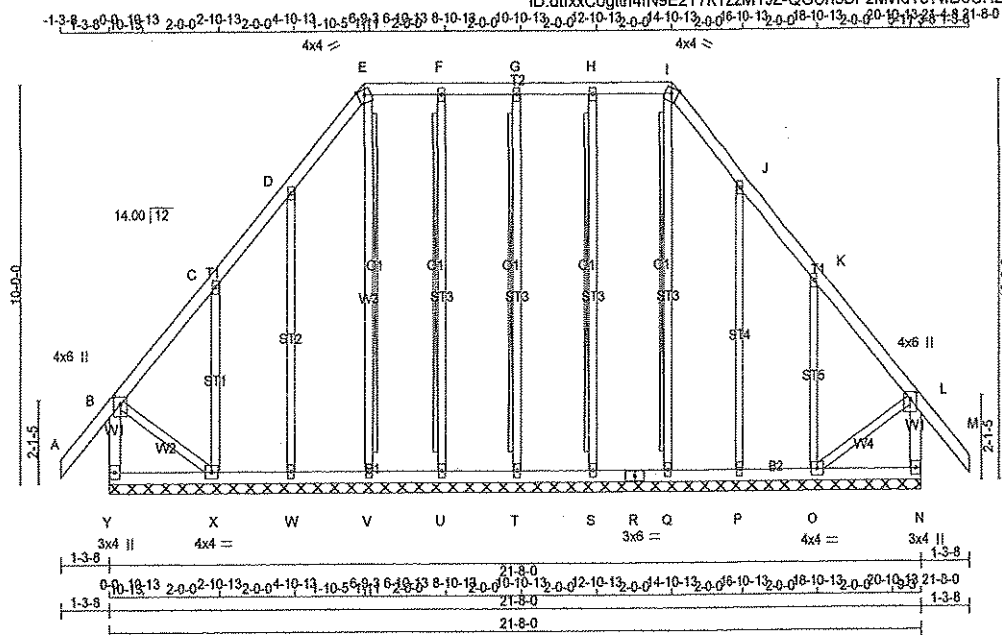
DWG NO. TAM 20229-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:09:07 2017 Page 1

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Scale = 1:56.8



TOTAL WEIGHT = 130 lb

GABLE STUDS SPACED AT 2'-0" OC.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-Q, H-S, G-T, F-U, E-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/41	-77.4	-77.4	0.11 (1)	10.00	Q-I	-109/0 0.08 (1)
B-C	-27/0	-77.4	-77.4	0.07 (1)	6.25	S-H	-176/0 0.14 (1)
C-D	-42/0	-77.4	-77.4	0.07 (1)	6.25	T-G	-149/0 0.12 (1)
D-E	-40/0	-77.4	-77.4	0.04 (1)	6.25	U-F	-173/0 0.13 (1)
E-F	-18/0	-77.4	-77.4	0.04 (1)	6.25	W-D	-157/0 0.17 (1)
F-G	-18/0	-77.4	-77.4	0.04 (1)	6.25	X-C	-206/0 0.09 (1)
G-H	-10/0	-77.4	-77.4	0.04 (1)	6.25	P-J	-152/0 0.17 (1)
H-I	-18/0	-77.4	-77.4	0.04 (1)	6.25	O-K	-211/0 0.09 (1)
I-J	-40/0	-77.4	-77.4	0.04 (1)	6.25	V-E	-108/0 0.08 (1)
J-K	-44/0	-77.4	-77.4	0.07 (1)	6.25	B-X	0/31 0.01 (1)
K-L	-26/0	-77.4	-77.4	0.07 (1)	6.25	O-L	0/31 0.01 (1)
L-M	0/41	-77.4	-77.4	0.11 (1)	10.00		
Y-B	-242/0	0.0	0.0	0.03 (1)	7.81		
N-L	-243/0	0.0	0.0	0.03 (1)	7.81		
Y-X	0/0	-28.0	-28.0	0.05 (2)	10.00		
X-W	0/22	-28.0	-28.0	0.05 (2)	10.00		
W-V	0/20	-28.0	-28.0	0.03 (2)	10.00		
V-U	0/18	-28.0	-28.0	0.03 (2)	10.00		
U-T	0/18	-28.0	-28.0	0.02 (2)	10.00		
T-S	0/18	-28.0	-28.0	0.02 (2)	10.00		
S-R	0/18	-28.0	-28.0	0.03 (2)	10.00		
R-Q	0/18	-28.0	-28.0	0.03 (2)	10.00		
Q-P	0/20	-28.0	-28.0	0.03 (2)	10.00		
P-O	0/22	-28.0	-28.0	0.05 (2)	10.00		
O-N	0/0	-28.0	-28.0	0.05 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIG 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (L-M:1), BC=0.05 (O-P:2), WB=0.17 (D-W:1), SSI=0.08 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

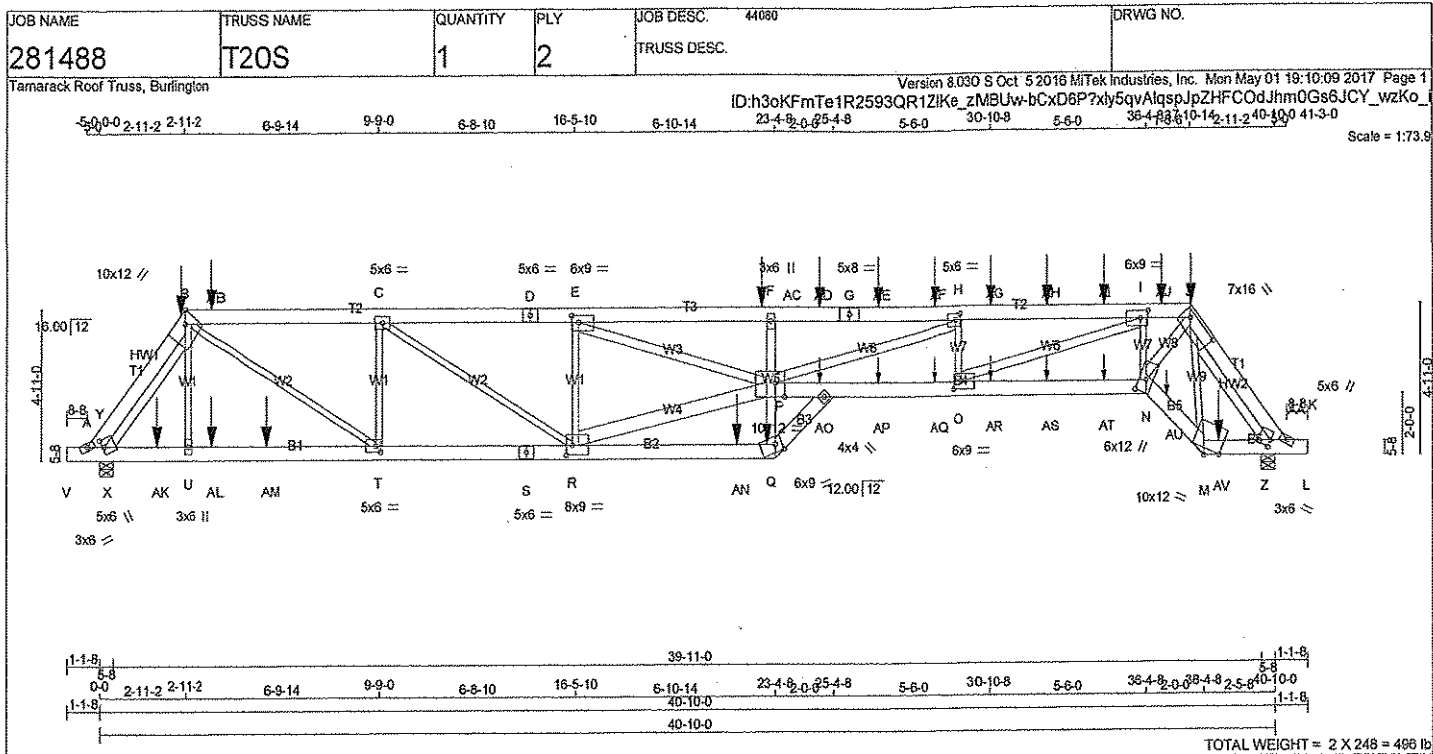
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.68 (I) (INPUT = 0.90)
JSI METAL= 0.06 (K) (INPUT = 1.00)



DWG NO. TADR 0250-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x6	DRY	2100F 1.8E
B - D	2x6	DRY	2100F 1.8E
D - G	2x6	DRY	2100F 1.8E
G - J	2x6	DRY	2100F 1.8E
J - K	2x6	DRY	2100F 1.8E
V - S	2x6	DRY	2100F 1.8E
S - Q	2x6	DRY	2100F 1.8E
P - N	2x6	DRY	2100F 1.8E
N - M	2x6	DRY	2100F 1.8E
M - L	2x6	DRY	2100F 1.8E

REINFORCING MEMBERS			
HW2	2x4	DRY	No.2
HW1	2x4	DRY	No.2

ALL WEBS EXCEPT			
Q - F	2x4	DRY	No.2
O - I	2x4	DRY	No.2
N - J	2x4	DRY	No.2
P - H	2x4	DRY	No.2
R - P	2x6	DRY	No.2
E - P	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B	2	12
B-D	2	12
D-G	2	12
G-J	2	12
J-K	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	2	12
S-Q	2	12
P-N	2	12
N-M	2	12
M-L	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TBMW1+m	MT20	5.0	6.0	3.00 0.50
B	TP-m	MT20	3.0	6.0	
C	1TWWW-h	MT20	10.0	12.0	Edge 4.50
D	TMWW-l	MT20	5.0	6.0	
E	TS-l	MT20	5.0	6.0	
F	TMWW-l	MT20	6.0	9.0	2.75 2.75
G	TMW+w	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION
X(A)	4766	0	4766
Z(K)	4589	0	4589

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS						
JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS		
X(A)	3937	2134 / 0	920 / 0	0 / 0	0 / 0	883 / 0
Z(K)	3824	2021 / 0	929 / 0	0 / 0	0 / 0	875 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X(A), Z(K)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (PLF)
FR-TO			
A-Y	-1976 / 0	-77.4	-77.4
Y-B	-3215 / 0	-77.4	-77.4
B-AB	-7239 / 0	-77.4	-77.4
AB-C	-7239 / 0	-77.4	-77.4
C-D	-9625 / 0	-77.4	-77.4
D-E	-9625 / 0	-77.4	-77.4
E-AC	-18840 / 0	-77.4	-77.4
AC-F	-18840 / 0	-77.4	-77.4
F-AD	-19039 / 0	-77.4	-77.4
AD-G	-19039 / 0	-77.4	-77.4
G-AE	-19039 / 0	-77.4	-77.4
AE-AF	-19039 / 0	-77.4	-77.4
AF-H	-19039 / 0	-77.4	-77.4
H-AQ	-14993 / 0	-77.4	-77.4
AG-AH	-14993 / 0	-77.4	-77.4
AH-AI	-14993 / 0	-77.4	-77.4
AI-I	-14993 / 0	-77.4	-77.4
I-AJ	-7260 / 0	-77.4	-77.4
AJ-J	-7260 / 0	-77.4	-77.4
J-AA	-4599 / 0	-77.4	-77.4
AA-K	-2691 / 0	-77.4	-77.4

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
U-B	0 / 1162	0.14 (1)	6.25
Q-P	0 / 1162	0.40 (1)	6.25
P-F	-703 / 0	0.30 (1)	5.31
O-H	-2808 / 0	0.25 (1)	5.31
O-I	0 / 8059	0.72 (1)	4.75
N-I	-3885 / 0	0.34 (1)	4.75
N-J	0 / 8214	0.73 (1)	3.43
J-M	-3167 / 0	0.43 (1)	3.43
P-H	0 / 4311	0.38 (1)	3.32
B-T	0 / 5103	0.63 (1)	3.32
R-E	-4008 / 0	0.65 (1)	3.32
T-C	-2097 / 0	0.34 (1)	3.32
C-R	0 / 2877	0.36 (1)	3.32
R-P	0 / 10023	0.65 (1)	3.85
E-P	0 / 9735	0.86 (1)	3.85
X-Y	-1753 / 0	0.00 (1)	3.85
X-B	-2196 / 0	0.40 (1)	3.85
Z-AA	-2620 / 0	0.00 (1)	5.29

V-A	0 / 0	-105.4	-105.4
A-X	0 / 1784	-28.0	-28.0
X-AK	0 / 2917	-28.0	-28.0
AK-U	0 / 2917	-28.0	-28.0
U-AL	0 / 2945	-28.0	-28.0
AL-AM	0 / 2945	-28.0	-28.0
AM-T	0 / 2945	-28.0	-28.0
T-S	0 / 7238	-28.0	-28.0
S-R	0 / 7238	-28.0	-28.0
R-AN	0 / 108	-28.0	-28.0
AN-Q	0 / 108	-28.0	-28.0
Q-AP	0 / 14993	-28.0	-28.0
AP-AQ	0 / 14993	-28.0	-28.0

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.39")
CALCULATED VERT. DEFL.(LL) = L/761 (0.66")
ALLOWABLE DEFL.(TL) = L/360 (1.39")
CALCULATED VERT. DEFL.(TL) = L/470 (1.06")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/359 (0.02")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/222 (0.04")

CSI: TC=0.45 (F-H:1), BC=0.46 (O-P:1), WB=0.86 (E-P:1), SSI=0.50 (K-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

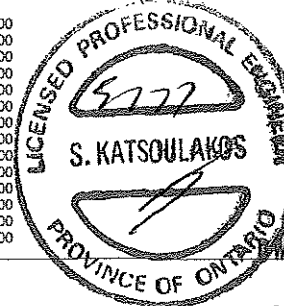
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

DRWG NO. TAM20287-17



STRUCTURAL COMPONENT ONLY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TS-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.00
I	TMWW-t	MT20	6.0	9.0	3.00	3.25
J	TTWWWW-t	MT20	7.0	16.0	2.25	12.75
K	TBMW1+m	MT20	5.0	6.0	2.75	0.25
K	TP-m	MT20	3.0	6.0		
M	BBW-m	MT20	10.0	12.0	Edge	
N	BBWW+m	MT20	6.0	12.0	5.00	3.00
O	BMWW-t	MT20	6.0	9.0	3.00	3.25
P	BWAWWWW-t	MT20	10.0	12.0	Edge	3.75
Q	BBW-m	MT20	6.0	9.0	3.00	2.75
R	BMWWWW-t	MT20	8.0	9.0	3.75	2.50
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	1.75
U	BMW+w	MT20	3.0	6.0		
W	NP+h	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 146.3 lbs FACTORED DOWN AT 2-11-2, 146.3 lbs FACTORED DOWN AT 37-11-2, 101.7 lbs FACTORED DOWN AT 3-11-4, 101.7 lbs FACTORED DOWN AT 22-10-12, 212.3 lbs FACTORED DOWN AT 24-10-12, 212.3 lbs FACTORED DOWN AT 26-10-12, 212.3 lbs FACTORED DOWN AT 28-10-12, 212.3 lbs FACTORED DOWN AT 30-10-12, 212.3 lbs FACTORED DOWN AT 32-10-12, AND 212.3 lbs FACTORED DOWN AT 34-10-12, AND 212.3 lbs FACTORED DOWN AT 36-10-12 ON TOP CHORD, AND 59.0 lbs FACTORED DOWN AT 1-11-4, 59.0 lbs FACTORED DOWN AT 3-11-4, 1339.1 lbs FACTORED DOWN AT 5-9-8, 1339.1 lbs FACTORED DOWN AT 22-0-8, 75.3 lbs FACTORED DOWN AT 23-1-0, AT 24-10-12, AT 26-10-12, AT 28-10-12, AT 30-10-12, AT 32-10-12, AT 34-10-12, AND AT 36-10-12, AND 70.7 lbs FACTORED DOWN AT 38-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. UNBRAC	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		
AQ-O	0/14993	-28.0	-28.0	0.46 (1)	10.00		
O-AR	0/7428	-28.0	-28.0	0.26 (1)	10.00		
AR-AS	0/7428	-28.0	-28.0	0.26 (1)	10.00		
AS-AT	0/7428	-28.0	-28.0	0.26 (1)	10.00		
AT-N	0/7428	-28.0	-28.0	0.26 (1)	10.00		
N-AU	0/3977	-28.0	-28.0	0.11 (1)	10.00		
AU-M	0/3977	-28.0	-28.0	0.11 (1)	10.00		
M-AV	0/2483	-28.0	-28.0	0.27 (1)	10.00		
AV-Z	0/2483	-28.0	-28.0	0.27 (1)	10.00		
Z-K	0/2493	-28.0	-28.0	0.26 (1)	10.00		
K-L	0/0	-105.4	-105.4	0.01 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-11-2	-146	-146	--	FRONT	VERT	TOTAL
J	37-11-2	-146	-146	--	FRONT	VERT	TOTAL
Q	23-1-0	-43	-75	--	BACK	VERT	TOTAL
AB	3-11-4	-102	-102	--	BACK	VERT	TOTAL
AC	22-10-12	-102	-102	--	BACK	VERT	TOTAL
AD	24-10-12	-212	-212	--	BACK	VERT	TOTAL
AE	26-10-12	-212	-212	--	BACK	VERT	TOTAL
AF	28-10-12	-212	-212	--	BACK	VERT	TOTAL
AG	30-10-12	-212	-212	--	BACK	VERT	TOTAL
AH	32-10-12	-212	-212	--	BACK	VERT	TOTAL
AI	34-10-12	-212	-212	--	BACK	VERT	TOTAL
AJ	36-10-12	-212	-212	--	BACK	VERT	TOTAL
AK	1-11-4	-34	-59	--	BACK	VERT	TOTAL
AL	3-11-4	-34	-59	--	BACK	VERT	TOTAL
AM	5-9-8	-1339	-1339	--	BACK	VERT	TOTAL
AN	22-0-8	-1339	-1339	--	BACK	VERT	TOTAL
AO	24-10-12	--	--	--	BACK	VERT	TOTAL
AP	26-10-12	--	--	--	BACK	VERT	TOTAL
AQ	28-10-12	--	--	--	BACK	VERT	TOTAL
AR	30-10-12	--	--	--	BACK	VERT	TOTAL
AS	32-10-12	--	--	--	BACK	VERT	TOTAL
AT	34-10-12	--	--	--	BACK	VERT	TOTAL
AU	36-10-12	--	--	--	BACK	VERT	TOTAL
AV	38-10-12	-40	-71	--	BACK	VERT	TOTAL

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.93 (E) (INPUT = 1.00)



1024

DWG NO. TAM 20287-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281488	TRUSS NAME T20Z	QUANTITY 1	PLY 2	JOB DESC. 44080 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:10 2017 Page 2
ID:h3okFmTe1R2593QR1ZIKzMBUw-4OVbJI0aWGDhWKK1PXr25VnK1cUVSI0Lzy6WMzKo_h

PLATES (table is in inches)

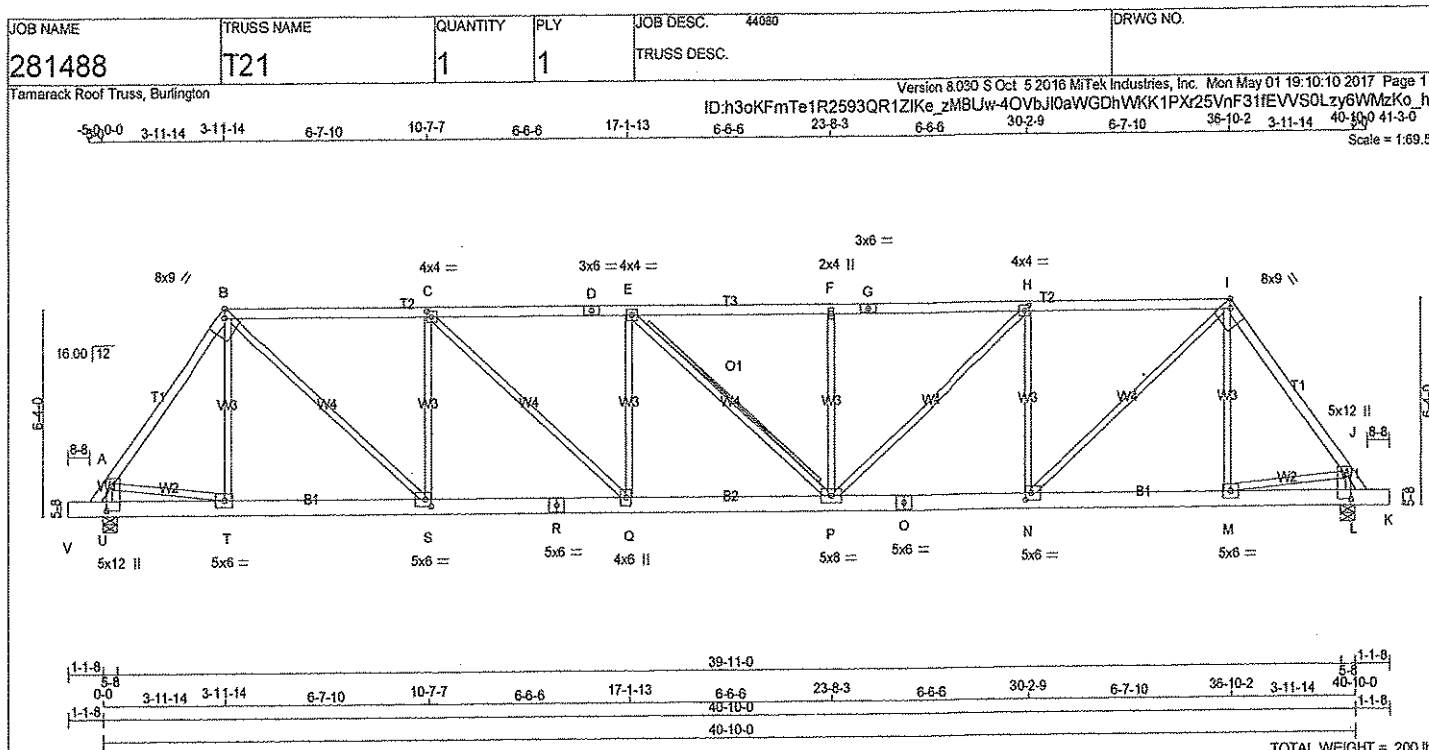
JT	TYPE	PLATES	W	LEN	Y	X
O	BMV1+p	MT20	3.0	6.0		
P	BMWWH	MT20	7.0	8.0		
Q	BMWWH	MT20	6.0	9.0	2.50	4.00
R	BMWWH	MT20	6.0	9.0		
S	BS-t	M1116	5.0	17.5		
T	BMWWH	MT20	8.0	9.0	4.25	4.50
U	BMWWH	MT20	8.0	9.0	4.25	4.50
V	BS-t	M1116	5.0	17.5		
W	BMWWH	MT20	6.0	9.0		
X	BMWWH	MT20	6.0	9.0	2.50	4.00
Y	BMWWH	MT20	7.0	8.0		
Z	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2841.1 lbs FACTORED DOWN AT 21-8-8, 378.8 lbs FACTORED DOWN AT 23-7-12, 378.8 lbs FACTORED DOWN AT 24-7-12, 1785.1 lbs FACTORED DOWN AT 25-9-8, AND 1785.1 lbs FACTORED DOWN AT 36-4-8, AND 378.8 lbs FACTORED DOWN AT 37-10-2 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

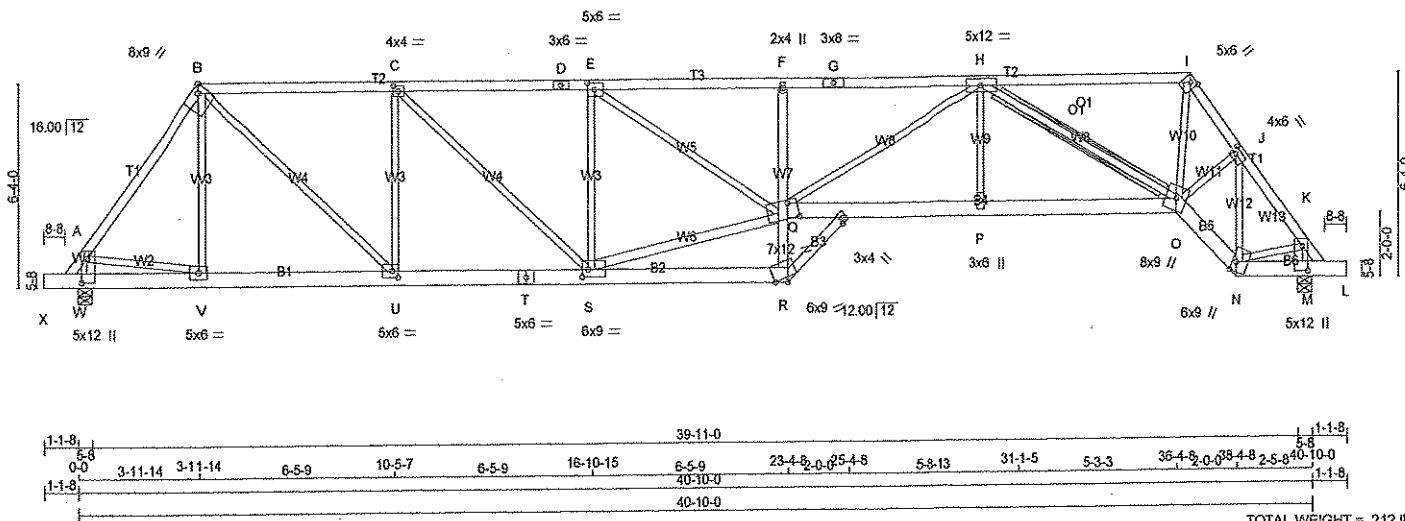


DWG NO. **TAM20Z008** 17
STRUCTURAL
COMPONENT ONLY



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES										BUILDING DESIGNER										SPECIFIED LOADS:									
CHORDS SIZE LUMBER DESCR.										BEARINGS										TOP CH. LL = 23.3 PSF									
A - B 2x4 DRY No.2 SPF										FACTORED MAXIMUM FACTORED INPUT REORD										DL = 3.0 PSF									
B - D 2x4 DRY No.2 SPF										GROSS REACTION GROSS REACTION BRG BRG										BOT CH. LL = 10.5 PSF									
D - G 2x4 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										DL = 7.0 PSF									
G - I 2x4 DRY No.2 SPF										U 2270 0 2270 0 0 5-8 5-8										TOTAL LOAD = 43.8 PSF									
I - J 2x4 DRY No.2 SPF										L 2270 0 2270 0 0 5-8 5-8																			
U - A 2x4 DRY No.2 SPF																													
L - J 2x4 DRY No.2 SPF																													
V - R 2x6 DRY No.2 SPF																													
R - O 2x6 DRY No.2 SPF																													
O - K 2x6 DRY No.2 SPF																													
ALL WEBS 2x3 DRY No.2 SPF																													
EXCEPT																													
DRY: SEASONED LUMBER.																													
PLATES (table is in inches)																													
JT TYPE PLATES W LEN Y X																													
A, J, L, U																													
A																													
B TTWW-h MT20 8.0 9.0 Edge 2.75																													
C TMWW-I MT20 4.0 4.0 2.00 1.75																													
D TS-I MT20 3.0 6.0																													
E TMWW-I MT20 4.0 4.0																													
F TMW+w MT20 2.0 4.0																													
G TS-I MT20 3.0 6.0																													
H TMWW-I MT20 4.0 4.0 2.00 1.75																													
I TTWW-h MT20 8.0 9.0 Edge 2.75																													
L TBMVW1+p MT20 5.0 12.0 9.00 2.00																													
M BMWW-I MT20 5.0 6.0																													
N BMWW-I MT20 5.0 6.0 2.50 2.25																													
O BS-I MT20 5.0 6.0																													
P BMWW-I MT20 5.0 6.0																													
Q BMWW-I MT20 4.0 6.0																													
R BS-I MT20 5.0 6.0																													
S BMWW-I MT20 5.0 6.0 2.50 2.25																													
T BMWW-I MT20 5.0 6.0																													
U TBMVW1+p MT20 5.0 12.0 9.00 2.00																													
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																													





LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - B	2x4	DRY	1650F 1.5E	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
I - K	2x4	DRY	1650F 1.5E	SPF
W - A	2x4	DRY	No.2	SPF
X - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A, K, M, R, W					
B	TTWW-h	MT20	8.0	9.0	Edge 2.75
C	TMWW-l	MT20	4.0	4.0	2.00 1.75
D	TS-l	MT20	3.0	6.0	
E	TMWW-l	MT20	5.0	6.0	2.50 2.50
F	TMWW-w	MT20	2.0	4.0	
G	TS-l	MT20	3.0	8.0	
H	TMWW-l	MT20	5.0	12.0	
I	TTW+h	MT20	5.0	8.0	Edge
J	TMWW-l	MT20	4.0	6.0	2.00 2.00
M	TBMWW1+P	MT20	5.0	12.0	9.00 2.00
N	BBWW+m	MT20	6.0	9.0	3.50 1.50
O	BBWW+m	MT20	8.0	9.0	
P	BMW+w	MT20	3.0	6.0	
Q	BBWWWW+MT20	MT20	7.0	12.0	3.50 5.75
R	BBW-m	MT20	6.0	9.0	Edge
S	BBWWWW-l	MT20	6.0	9.0	2.75 2.25
T	BS-l	MT20	5.0	6.0	
U	BMWW-l	MT20	5.0	6.0	2.50 2.25
V	BMWW-l	MT20	5.0	6.0	
W	TBMWW1+P	MT20	5.0	12.0	9.00 2.00
AA	NP+h	MT20	3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
W	2262	0	0	5-8
M	2277	0	0	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
W	1880	1000 / 0	451 / 0
M	1892	1007 / 0	453 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 I-BRACE AT H-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	-2215 / 0	-77.4	-77.4	0.25 (1)	4.94	V-B	-1 / 198
B-C	-2887 / 0	-77.4	-77.4	0.54 (1)	4.14	B-U	0 / 2087
C-D	-3801 / 0	-77.4	-77.4	0.61 (1)	3.74	U-C	-1233 / 0
D-E	-3801 / 0	-77.4	-77.4	0.61 (1)	3.74	C-S	0 / 1007
E-F	-5419 / 0	-77.4	-77.4	0.69 (1)	3.17	S-E	-1604 / 0
F-G	-5441 / 0	-77.4	-77.4	0.97 (1)	2.92	R-Q	0 / 129
G-H	-5441 / 0	-77.4	-77.4	0.97 (1)	2.92	Q-F	-469 / 0
H-I	-2113 / 0	-77.4	-77.4	0.55 (1)	4.63	H-O	0 / 1205
I-J	-3111 / 0	-77.4	-77.4	0.08 (1)	4.46	P-H	0 / 369
J-K	-2067 / 0	-77.4	-77.4	0.08 (1)	5.22	K-O	-2678 / 0
W-A	-2108 / 0	0.0	0.0	0.21 (1)	5.84	O-I	0 / 2228
X-W	0 / 0	-105.4	-105.4	0.04 (1)	10.00	I-J	0 / 822
W-V	0 / 0	-28.0	-28.0	0.10 (2)	10.00	J-K	-1504 / 0
V-U	0 / 1321	-28.0	-28.0	0.22 (2)	10.00	A-V	0 / 1333
U-T	0 / 2867	-28.0	-28.0	0.39 (1)	10.00	S-Q	0 / 3736
T-S	0 / 2867	-28.0	-28.0	0.39 (1)	10.00	E-Q	0 / 1282
S-R	0 / 54	-28.0	-28.0	0.13 (3)	10.00	M-K	0 / 1282
Q-P	0 / 4407	-28.0	-28.0	0.60 (1)	10.00	N-K	0 / 1282
P-O	0 / 4408	-28.0	-28.0	0.60 (1)	10.00		
O-N	0 / 1669	-28.0	-28.0	0.21 (1)	10.00		
N-M	0 / 0	-28.0	-28.0	0.02 (2)	10.00		
M-L	0 / 0	-105.4	-105.4	0.06 (1)	10.90		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.42")
 ALLOWABLE DEFL.(TL) = L/360 (1.36")
 CALCULATED VERT. DEFL.(TL) = L/718 (0.68")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.97 (F-H:1), BC=0.60 (P-Q:1), WB=0.99 (H-O:1), SSI=0.25 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

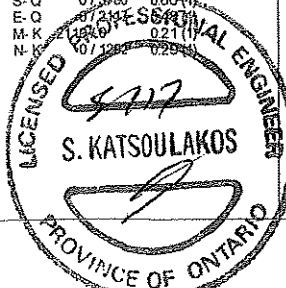
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1667 822 2284 1655

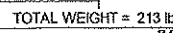
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (U) (INPUT = 0.90)
 JSI METAL=0.61 (O) (INPUT = 1.00)



DRWG NO. TAM20290-17
 STRUCTURAL
 COMPONENT ONLY

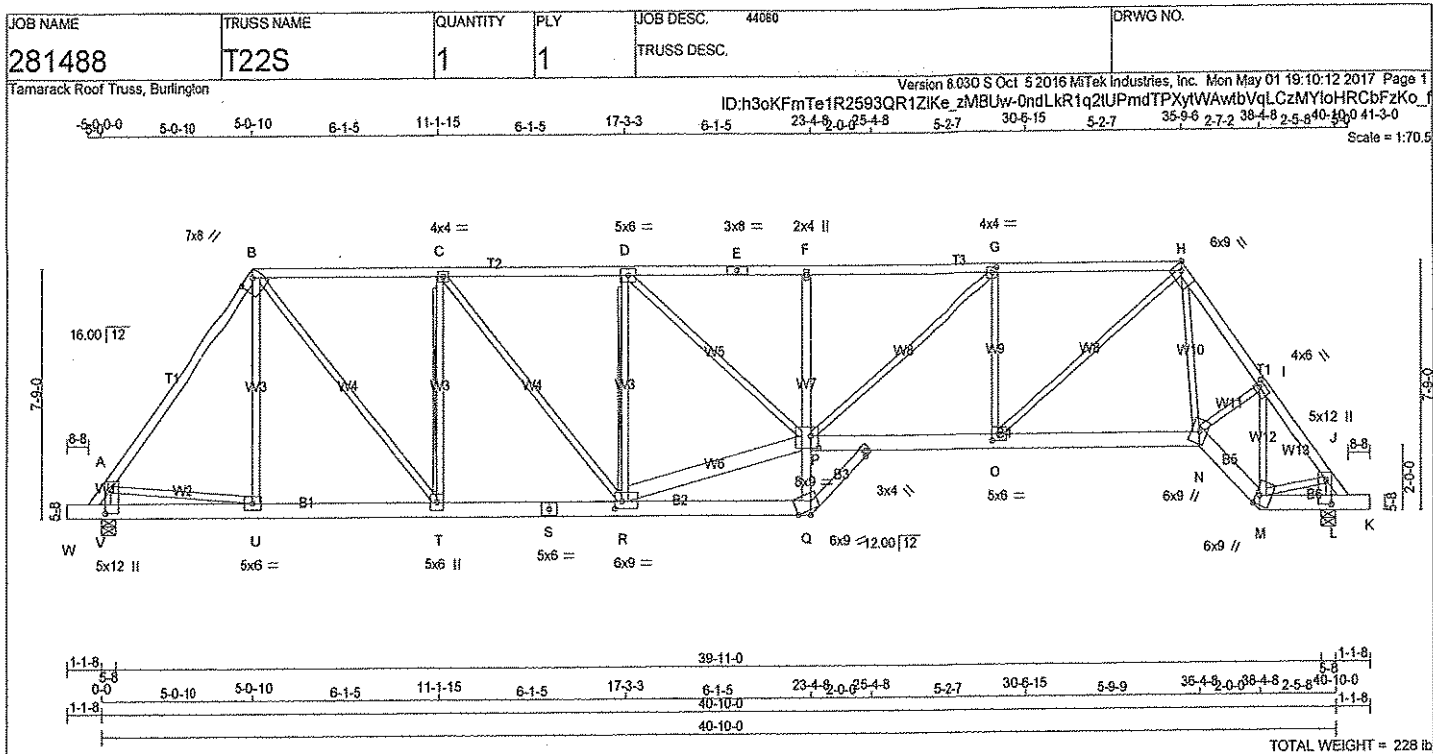


Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

V-U	0/0	-105.4	-105.4	0.04 (1)	10.00
U-T	0/0	-28.0	-28.0	0.10 (3)	10.00
T-S	0/1312	-28.0	-28.0	0.22 (2)	10.00
S-R	0/2430	-28.0	-28.0	0.34 (1)	10.00
R-Q	0/2430	-28.0	-28.0	0.34 (1)	10.00
Q-P	0/2849	-28.0	-28.0	0.41 (1)	10.00
P-O	0/2429	-28.0	-28.0	0.33 (1)	10.00
O-N	0/2429	-28.0	-28.0	0.33 (1)	10.00
N-M	0/1312	-28.0	-28.0	0.22 (2)	10.00
M-L	0/0	-28.0	-28.0	0.10 (3)	10.00
L-K	0/0	-105.4	-105.4	0.04 (1)	10.00

SI GRIP= 0.89 (D) (INPUT = 0.90)
SI METAL= 0.42 (R) (INPUT = 1.00)





LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF			
B - E	2x4	DRY	No.2	SPF			
E - H	2x4	DRY	No.2	SPF			
H - J	2x4	DRY	No.2	SPF			
V - A	2x4	DRY	No.2	SPF			
L - J	2x4	DRY	No.2	SPF			
W - S	2x6	DRY	No.2	SPF			
S - Q	2x6	DRY	No.2	SPF			
P - N	2x6	DRY	No.2	SPF			
N - M	2x6	DRY	No.2	SPF			
M - K	2x6	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							
Q - F	2x4	DRY	No.2	SPF			
R - P	2x6	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A, J, L, Q, V					
A					
B	TTWW-h	MT20	7.0	8.0	1.75 5.00
C	TMWW-l	MT20	4.0	4.0	
D	TMWW-l	MT20	5.0	6.0	
E	TS-l	MT20	3.0	8.0	
F	TMWW-w	MT20	2.0	4.0	
G	TMWW-l	MT20	4.0	4.0	2.00 1.75
H	TTWW-h	MT20	6.0	9.0	Edge 2.75
I	TMWW-l	MT20	4.0	6.0	2.00 2.00
L	TBMVW1*+p	MT20	5.0	12.0	9.00 2.00
M	BBWW+m	MT20	6.0	9.0	3.50 1.50
N	BBWW+m	MT20	6.0	9.0	
O	BMWW-l	MT20	5.0	6.0	2.50 2.50
P	BMWWWW-l	MT20	8.0	9.0	4.50 3.00
Q	BBW-m	MT20	6.0	9.0	Edge
R	BMWWWW-l	MT20	6.0	9.0	2.75 2.75
S	BS-l	MT20	5.0	6.0	
T	BMWW-h	MT20	5.0	6.0	
U	BMWW-l	MT20	5.0	6.0	
V	TBMVW1*+p	MT20	5.0	12.0	9.00 2.00
Z	NP+h	MT20	3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
V	2269	0	2269	0	5-8
L	2269	0	2269	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
V	1886	1003 / 0	452 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1886	1003 / 0	452 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-T
2x4 DRY SPF No.2 T-BRACE AT D-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-2198 / 0	A-B	0 / 232
B-C	-2420 / 0	B-T	0 / 1722
C-D	-2941 / 0	T-C	-1158 / 0
D-E	-4036 / 0	C-R	0 / 827
E-F	-4036 / 0	R-D	-1422 / 0
F-G	-4043 / 0	Q-P	0 / 120
G-H	-3340 / 0	P-F	-459 / 0
H-I	-3199 / 0	P-G	0 / 922
I-J	-2202 / 0	O-G	-1123 / 0
V-A	-2095 / 0	O-H	0 / 2038
L-J	-2116 / 0	H-N	0 / 933
		N-I	0 / 777
W-V	0 / 0	M-I	-1490 / 0
V-U	0 / 0	A-U	0 / 1319
U-T	0 / 1312	M-J	0 / 1354
T-S	0 / 2420	R-P	0 / 3084
S-R	0 / 2420	D-P	0 / 3084
R-Q	0 / 28		
P-O	0 / 3340		
O-N	0 / 1794		
N-M	0 / 1816		
M-L	0 / 0		
L-K	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.48")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.97 (F-G:1), BC=0.47 (O-P:1), WB=0.85 (C-T:1), SSI=0.23 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

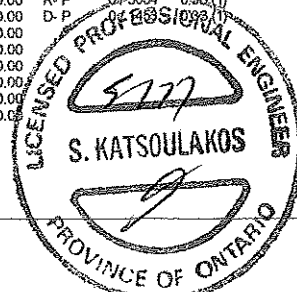
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1856

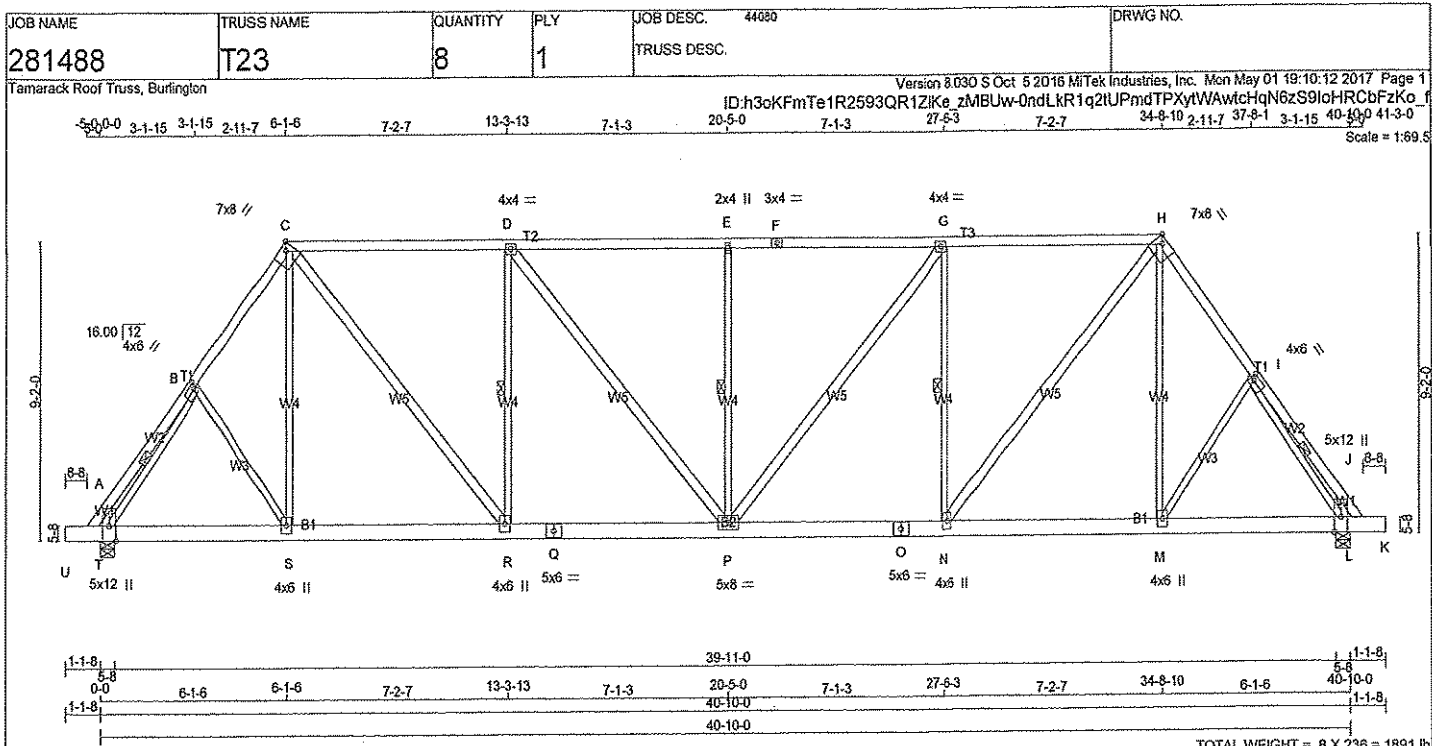
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.49 (M) (INPUT = 1.00)

DRWG NO. TAM20292-17
STRUCTURAL
COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
T - A	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
U - Q	2x6	DRY No.2	SPF
Q - O	2x6	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
C - R	2x4	DRY No.2	SPF
D - P	2x4	DRY No.2	SPF
P - G	2x4	DRY No.2	SPF
N - H	2x4	DRY No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A, J, L, T					
A					
B	TMWW-I	MT20	4.0	6.0	2.00 1.50
C	TTWW-h	MT20	7.0	8.0	Edge 2.75
D	TMWW-I	MT20	4.0	4.0	
E	TMWW-w	MT20	2.0	4.0	
F	TS-I	MT20	3.0	4.0	
G	TMWW-I	MT20	4.0	4.0	
H	TTWW-h	MT20	7.0	8.0	Edge 2.75
I	TMWW-I	MT20	4.0	6.0	2.00 1.50
L	TBMWWI**p	MT20	5.0	12.0	Edge
M, N, R, S					
M	BMWW-h	MT20	4.0	6.0	
O	BS-I	MT20	5.0	6.0	
P	BMWW-h	MT20	5.0	8.0	
Q	BS-I	MT20	5.0	6.0	
T	TBMWWI**p	MT20	5.0	12.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	VERT	HORZ	GROSS REACTION	GROSS REACTION		
T	2270	0	2270	0	5-8	5-8
L	2270	0	2270	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
T	1886	1003 / 0	452 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1886	1003 / 0	452 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-R, E-P, G-N, B-T, L-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)			
FR-TO		FROM TO				FR-TO					
A-B	-1 / 17	-77.4	-77.4	0.09 (1)	10.00	B-S	-39 / 58	0.02 (1)			
B-C	-2180 / 0	-77.4	-77.4	0.17 (1)	4.50	S-C	0 / 328	0.07 (2)			
C-D	-2257 / 0	-77.4	-77.4	0.87 (1)	3.61	C-R	0 / 1519	0.24 (1)			
D-E	-2536 / 0	-77.4	-77.4	0.92 (1)	3.39	R-D	-963 / 0	0.51 (1)			
E-F	-2536 / 0	-77.4	-77.4	0.92 (1)	3.39	D-P	0 / 443	0.07 (1)			
F-G	-2536 / 0	-77.4	-77.4	0.92 (1)	3.39	P-E	-508 / 0	0.27 (1)			
G-H	-2257 / 0	-77.4	-77.4	0.87 (1)	3.61	P-G	0 / 443	0.07 (1)			
H-I	-2180 / 0	-77.4	-77.4	0.17 (1)	4.50	N-G	-963 / 0	0.51 (1)			
I-J	-1 / 17	-77.4	-77.4	0.09 (1)	10.00	N-H	0 / 1519	0.24 (1)			
T-A	-94 / 0	0.0	0.0	0.01 (1)	7.81	M-H	0 / 328	0.07 (2)			
L-J	-94 / 0	0.0	0.0	0.01 (1)	7.81	M-I	-39 / 58	0.02 (1)			
						T-B	-2379 / 0	0.53 (1)			
						I-L	-2379 / 0	0.53 (1)			
U-T	0 / 0	-105.4	-105.4	0.04 (1)	10.00						
T-S	0 / 1311	-28.0	-28.0	0.25 (2)	10.00						
S-R	0 / 1291	-28.0	-28.0	0.25 (2)	10.00						
R-Q	0 / 2257	-28.0	-28.0	0.35 (1)	10.00						
Q-P	0 / 2257	-28.0	-28.0	0.35 (1)	10.00						
P-O	0 / 2257	-28.0	-28.0	0.35 (1)	10.00						
O-N	0 / 2257	-28.0	-28.0	0.35 (1)	10.00						
N-M	0 / 1291	-28.0	-28.0	0.25 (2)	10.00						
M-L	0 / 1311	-28.0	-28.0	0.25 (2)	10.00						
L-K	0 / 0	-105.4	-105.4	0.04 (1)	10.00						

SEALED PROFESSIONAL ENGINEER

5221

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TO=0.92 (D-E:1), BC=0.35 (P-R:1), WB=0.53 (I-L:1), SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

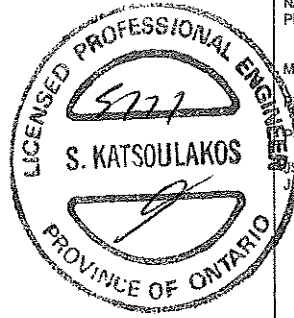
PLATE	GRIP(DRY)	SHEAR	SECTION			
(PS)	(PL)	(PL)	(PL)			
MAX MIN	MAX MIN	MAX MIN	MAX MIN			
MT20	618	354	1687	822	2284	1658

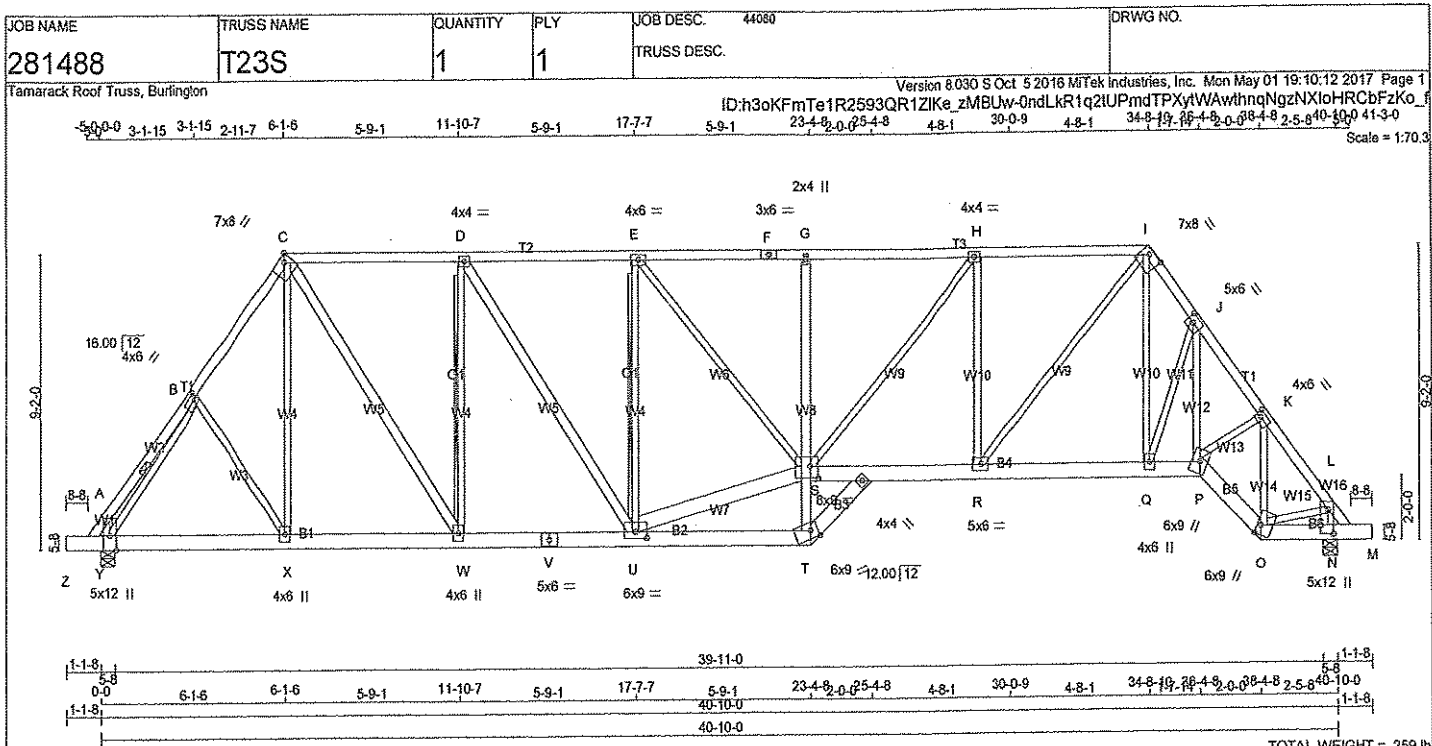
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP= 0.89 (B) (INPUT = 0.80)
JM METAL = 0.60 (B) (INPUT = 1.00)

DRWG NO. TAM 20243-17
STRUCTURAL COMPONENT ONLY





LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	DRY	No.2	SPF	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2	SPF	A - C	2x4	DRY
C - F	2x4	DRY	No.2	SPF	C - F	2x4	DRY
F - I	2x4	DRY	No.2	SPF	F - I	2x4	DRY
I - L	2x4	DRY	No.2	SPF	I - L	2x4	DRY
L - M	2x4	DRY	No.2	SPF	L - M	2x4	DRY
M - N	2x4	DRY	No.2	SPF	M - N	2x4	DRY
N - O	2x4	DRY	No.2	SPF	N - O	2x4	DRY
O - P	2x4	DRY	No.2	SPF	O - P	2x4	DRY
P - Q	2x4	DRY	No.2	SPF	P - Q	2x4	DRY
Q - R	2x4	DRY	No.2	SPF	Q - R	2x4	DRY
R - S	2x4	DRY	No.2	SPF	R - S	2x4	DRY
S - T	2x4	DRY	No.2	SPF	S - T	2x4	DRY
T - U	2x4	DRY	No.2	SPF	T - U	2x4	DRY
U - V	2x4	DRY	No.2	SPF	U - V	2x4	DRY
V - W	2x4	DRY	No.2	SPF	V - W	2x4	DRY
W - X	2x4	DRY	No.2	SPF	W - X	2x4	DRY
X - Y	2x4	DRY	No.2	SPF	X - Y	2x4	DRY
Y - Z	2x4	DRY	No.2	SPF	Y - Z	2x4	DRY
Z - A	2x4	DRY	No.2	SPF	Z - A	2x4	DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)				W			
JT	TYPE	PLATES	W	LEN	Y	X	
A, L, N, Y							
A	TMWW-I	MT20	4.0	6.0	2.00	1.50	
B	TTWW-h	MT20	7.0	8.0	Edge	2.75	
C	TMWW-I	MT20	4.0	4.0			
D	TMWW-I	MT20	4.0	6.0			
E	TS-I	MT20	3.0	6.0			
F	TMWW-w	MT20	2.0	4.0			
G	TMWW-I	MT20	4.0	4.0			
H	TTWW-h	MT20	7.0	8.0	1.75	5.00	
I	TMWW-I	MT20	5.0	6.0	2.50	2.50	
J	TMWW-I	MT20	4.0	6.0	2.00	2.00	
K	TBMWW1+P	MT20	5.0	12.0	9.00	2.00	
L	BBWW+m	MT20	6.0	9.0	3.50	1.50	
M	PBBWW+m	MT20	6.0	9.0			
N	Q, W, X						
O	BMWW-I	MT20	4.0	6.0			
P	BMWW-I	MT20	5.0	6.0			
Q	BBWW-m	MT20	8.0	9.0	4.50	3.00	
R	BBWW-m	MT20	6.0	9.0	3.00	2.75	
S	BMWW-I	MT20	6.0	9.0	2.50	4.50	
T	BS-I	MT20	5.0	6.0			
U	TBMWW1+P	MT20	5.0	12.0	Edge		
V	AC NP+h	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
Y	2262	0	2262	0	0	5-8	5-8	5-8	5-8
N	2277	0	2277	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS									
JT	1ST CASE	MAX	MIN	COMPONENT	REACTIONS				
Y	1880	1000 / 0	451 / 0	0 / 0	0 / 0	429 / 0	0 / 0	0 / 0	0 / 0
N	1893	1007 / 0	454 / 0	0 / 0	0 / 0	432 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-Y.

2x4 DRY SPF No.2 T-BRACE AT D-W, E-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC1) MAX. (LC2)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC1) MAX. (LC2)	
FR-TO				FR-TO			
A-B	-1 / 17	-77.4	-77.4 0.09 (1)	10.00	B-X	-40 / 57	0.02 (1)
B-C	-2168 / 0	-77.4	-77.4 0.17 (1)	4.51	X-C	0 / 301	0.07 (2)
C-D	-2092 / 0	-77.4	-77.4 0.50 (1)	4.19	C-W	0 / 1462	0.23 (1)
D-E	-2461 / 0	-77.4	-77.4 0.54 (1)	3.88	W-D	-1065 / 0	0.63 (1)
E-F	-3166 / 0	-77.4	-77.4 0.50 (1)	3.56	D-U	0 / 685	0.11 (1)
F-G	-3166 / 0	-77.4	-77.4 0.50 (1)	3.56	U-E	-1263 / 0	0.75 (1)
G-H	-3170 / 0	-77.4	-77.4 0.63 (1)	3.41	T-S	0 / 112	0.06 (1)
H-I	-2684 / 0	-77.4	-77.4 0.57 (1)	3.72	S-G	-420 / 0	0.13 (1)
I-J	-2745 / 0	-77.4	-77.4 0.12 (1)	4.12	R-H	-1061 / 0	0.89 (1)
J-K	-3104 / 0	-77.4	-77.4 0.17 (1)	3.88	R-I	0 / 1612	0.36 (1)
K-L	-2070 / 0	-77.4	-77.4 0.11 (1)	4.64	Q-I	0 / 729	0.16 (1)
Y-A	-94 / 0	0.0	0.0 0.01 (1)	7.81	Q-J	-643 / 0	0.26 (1)
					P-J	0 / 734	0.17 (1)
					P-K	0 / 830	0.19 (1)
Z-Y	0 / 0	-105.4	-105.4 0.04 (1)	10.00	O-K	-1514 / 0	0.36 (1)
Y-X	0 / 1304	-28.0	-28.0 0.22 (2)	10.00	Y-B	-2366 / 0	0.53 (1)
X-W	0 / 1284	-28.0	-28.0 0.23 (2)	10.00	U-S	0 / 2603	0.31 (1)
W-V	0 / 2092	-28.0	-28.0 0.30 (1)	10.00	E-S	0 / 1089	0.24 (1)
V-U	0 / 2092	-28.0	-28.0 0.30 (1)	10.00	S-H	0 / 55	0.02 (1)
U-T	0 / 17	-28.0	-28.0 0.11 (3)	10.00	N-L	-2111 / 0	0.21 (1)
S-R	0 / 2684	-28.0	-28.0 0.37 (1)	10.00	O-L	-1301 / 0	0.29 (1)
R-Q	0 / 1636	-28.0	-28.0 0.26 (1)	10.00			
Q-P	0 / 1840	-28.0	-28.0 0.29 (1)	10.00			
P-O	0 / 1700	-28.0	-28.0 0.21 (1)	10.00			
O-N	0 / 0	-28.0	-28.0 0.02 (2)	10.00			
N-M	0 / 0	-105.4	-105.4 0.08 (1)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL = 23.3	PSF	
DL = 3.0	PSF		
BOT CH.	LL = 10.5	PSF	
DL = 7.0	PSF		
TOTAL LOAD = 43.8	PSF		
SPACING = 24.0	IN. C/C		

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.36")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.21")

ALLOWABLE DEFL.(TL) = $L/360$ (1.36")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.34")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = $L/120$ (0.19")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")

ALLOWABLE DEFL.(TL) = $L/120$ (0.19")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.63 (G-H:1), BC=0.37 (R-S:1), WB=0.89 (H-R:1), SS=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1855

PLATE PLACEMENT TOL. = 0.250 inches

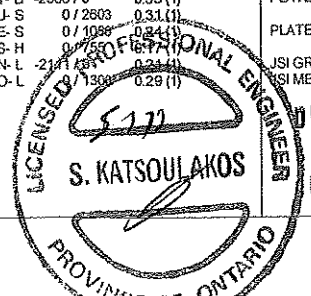
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.59 (B) (INPUT = 1.00)

DRWG NO. TAM20244-17

STRUCTURAL COMPONENT ONLY



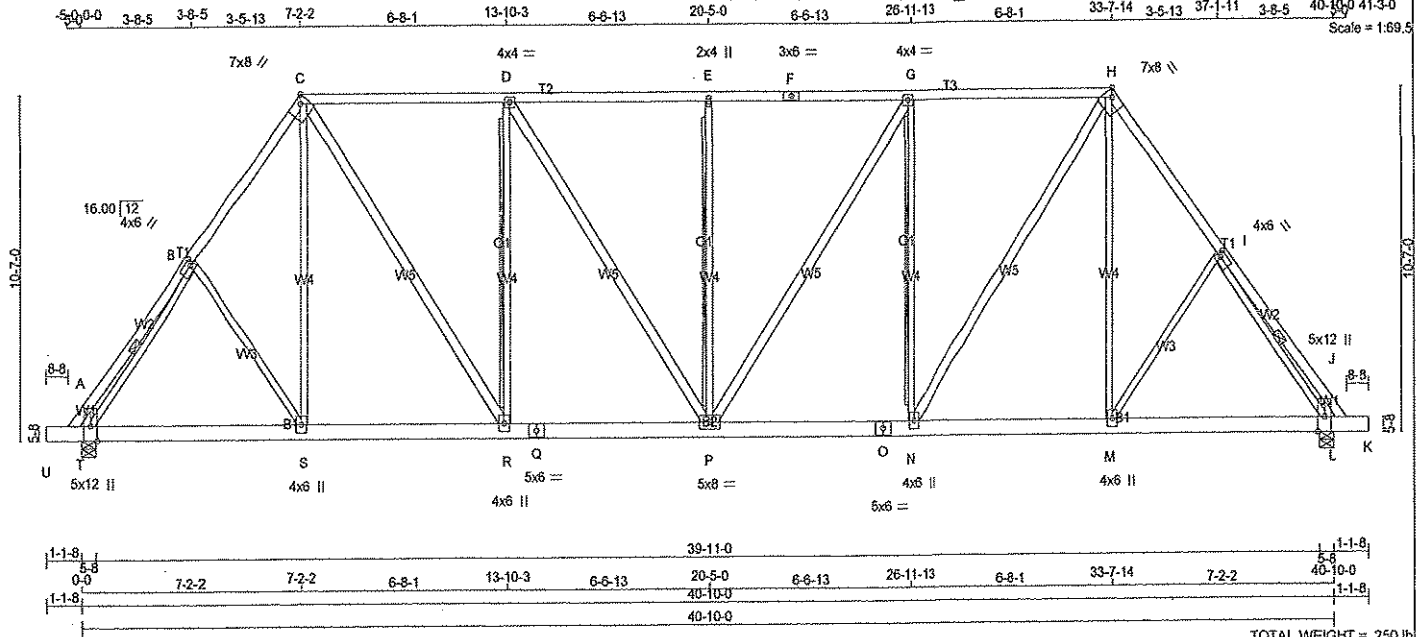
JOB NAME 281488	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. 44080	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M Tek Industries, Inc. Mon May 01 19:10:13 2017 Page 1

ID:h3okFmTe1R2593QR1ZiKe_zMBUw-UzBjxn2SpBcGNn2c5f0j7PrREK2lrgS1xAm7hzKo_e

Scale = 1:69.5



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
T - A	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
U - Q	2x6	DRY No.2	SPF
Q - O	2x6	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
C - R	2x4	DRY No.2	SPF
D - P	2x4	DRY No.2	SPF
N - G	2x4	DRY No.2	SPF
P - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A, J, L, T				
A	TMWW-t	MT20	4.0	6.0 2.00 1.50
C	TTWW-h	MT20	7.0	8.0 Edge 2.75
D	TMWW-t	MT20	4.0	4.0
E	TMWW-w	MT20	2.0	4.0
F	TS-t	MT20	3.0	6.0
G	TMWW-t	MT20	4.0	4.0
H	TTWW-h	MT20	7.0	8.0 Edge 2.75
I	TMWW-t	MT20	4.0	6.0 2.00 1.50
L	TBMWW-t+p	MT20	5.0	12.0 Edge
M, N, R, S				
M	BMWW-t	MT20	4.0	6.0
O	BS-t	MT20	5.0	6.0
P	BMWW-t	MT20	5.0	8.0
Q	BS-t	MT20	5.0	6.0
T	TBMWW-t+p	MT20	5.0	12.0 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
T	2270	0	2270	0	5-8
L	2270	0	2270	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
T	1888	1003 / 0	452 / 0	0 / 0	431 / 0	0 / 0
L	1888	1003 / 0	452 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-T, I-L
2x4 DRY SPF No.2 T-BRACE AT D-R, E-P, G-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 23	B-S	-94 / 42
B-C	-2139 / 0	S-C	0 / 378
C-D	-1973 / 0	C-R	0 / 1291
D-E	-2178 / 0	R-D	-890 / 0
E-F	-2178 / 0	D-P	0 / 378
F-G	-2178 / 0	P-E	-488 / 0
G-H	-1973 / 0	E-P	0 / 378
H-I	-2139 / 0	P-G	-890 / 0
I-J	0 / 23	G-N	0 / 1291
T-A	-108 / 0	N-H	0 / 378
L-J	-108 / 0	H-I	-94 / 42
		M-I	-2359 / 0
		I-L	-2359 / 0
U-T	0 / 0		
T-S	0 / 1316		
S-R	0 / 1265		
R-Q	0 / 1974		
Q-P	0 / 1974		
P-O	0 / 1974		
O-N	0 / 1974		
N-M	0 / 1265		
M-L	0 / 1316		
L-K	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.70 (E-G:1), BC=0.30 (P-R:1), WB=0.77 (G-N:1), SSI=0.24 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

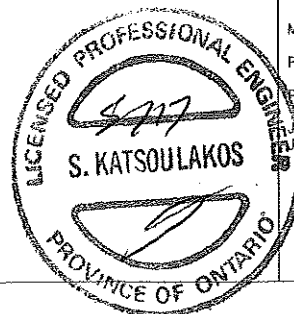
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.87 (B) (INPUT = 0.90)
SI METAL= 0.60 (I) (INPUT = 1.00)



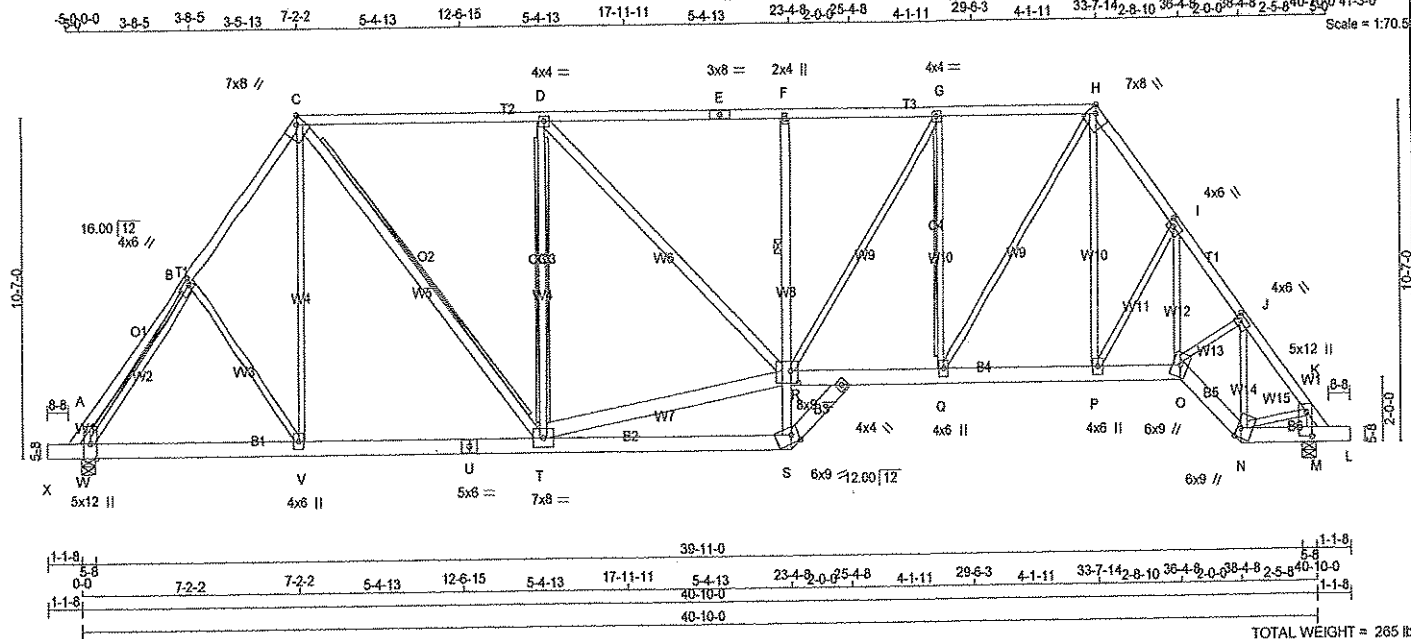
DRWG NO. **TAR 0245-17**
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281488	T24S	1	1	TRUSS DESC.		

Tamareck Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Mon May 01 19:10:13 2017 Page 1

ID:h3oKfMTe1R2593QR1ZiKe_zMBUw-UzBjxn2SpBcGnN2c5Oj7PnEjZiPWS1xAm7hzKo_e



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
W - A	2x4	DRY	No.2	SPF	
X - U	2x6	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - N	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
C - T	2x4	DRY	No.2	SPF	
S - F	2x4	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
D - R	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A, K, M, W					
A	TMWW-t	MT20	4.0	6.0	2.00 1.50
C	TTWW-h	MT20	7.0	8.0	Edge 2.75
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	8.0	
F	TMWW-t	MT20	2.0	4.0	
G	TMWW-t	MT20	4.0	4.0	
H	TTWW-h	MT20	7.0	8.0	Edge 2.75
I	TMWW-t	MT20	4.0	6.0	2.00 2.50
J	TMWW-t	MT20	4.0	6.0	2.00 2.00
M	TBMWW-t	MT20	5.0	12.0	9.00 2.00
N	BBWW-m	MT20	6.0	9.0	3.50 1.50
O	BBWW-m	MT20	6.0	9.0	
P, Q, V					
P	BMWW-t	MT20	4.0	6.0	
R	BBWWWW-t	MT20	8.0	9.0	4.50 3.00
S	BBWW-m	MT20	6.0	9.0	3.00 2.75
T	BMWWWW-t	MT20	7.0	8.0	
U	BS-t	MT20	5.0	6.0	
W	TBMWW-t	MT20	5.0	12.0	Edge
AA	NP-h	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
M	2270	0	2270	0
W	2270	0	2270	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1887	1003 / 0	453 / 0	0 / 0	0 / 0	431 / 0	0 / 0	0 / 0
W	1887	1003 / 0	453 / 0	0 / 0	0 / 0	431 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.25 FT.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-R.

2x3 DRY SPF No.2 T-BRACE AT C-T

2x4 DRY SPF No.2 I-BRACE AT D-T

2x4 DRY SPF No.2 T-BRACE AT G-Q, B-W

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
A-B	0 / 23	-77.4	-77.4	0.12 (1)	10.00	B-V	-93 / 43	0.07 (1)	
B-C	-2141 / 0	-77.4	-77.4	0.17 (1)	5.08	V-C	0 / 391	0.09 (2)	
C-D	-2076 / 0	-77.4	-77.4	0.85 (1)	4.20	C-T	0 / 1313	0.21 (1)	
D-E	-2854 / 0	-77.4	-77.4	0.94 (1)	3.75	T-D	-1296 / 0	0.91 (1)	
E-F	-2854 / 0	-77.4	-77.4	0.94 (1)	3.75	S-R	0 / 158	0.05 (2)	
F-G	-2858 / 0	-77.4	-77.4	0.38 (1)	4.42	R-F	-487 / 0	0.07 (1)	
G-H	-2297 / 0	-77.4	-77.4	0.27 (1)	4.82	Q-G	-973 / 0	0.49 (1)	
H-I	-2631 / 0	-77.4	-77.4	0.10 (1)	4.75	Q-H	0 / 1348	0.30 (1)	
I-J	-3207 / 0	-77.4	-77.4	0.09 (1)	4.41	P-H	0 / 896	0.16 (1)	
J-K	-2201 / 0	-77.4	-77.4	0.08 (1)	5.10	P-I	-601 / 0	0.33 (1)	
M-K	-2117 / 0	0.0	0.0	0.21 (1)	5.83	O-I	0 / 852	0.19 (1)	
W-A	-108 / 0	0.0	0.0	0.01 (1)	7.81	O-J	0 / 822	0.19 (1)	
						N-J	-1484 / 0	0.36 (1)	
X-W	0 / 0	-105.4	-105.4	0.04 (1)	10.00	N-K	0 / 1349	0.30 (1)	
W-V	0 / 1317	-28.0	-28.0	0.28 (2)	10.00	W-B	-2361 / 0	0.70 (1)	
V-U	0 / 1266	-28.0	-28.0	0.33 (2)	10.00	R-G	0 / 674	0.15 (1)	
U-T	0 / 1266	-28.0	-28.0	0.33 (2)	10.00	T-R	0 / 1266	0.21 (1)	
T-S	0 / 12	-28.0	-28.0	0.22 (3)	10.00	D-R	0 / 7812	0.13 (1)	
R-Q	0 / 2297	-28.0	-28.0	0.32 (1)	10.00				
Q-P	0 / 1568	-28.0	-28.0	0.23 (1)	10.00				
P-O	0 / 1909	-28.0	-28.0	0.27 (1)	10.00				
O-N	0 / 1809	-28.0	-28.0	0.22 (1)	10.00				
N-M	0 / 0	-28.0	-28.0	0.02 (2)	10.00				
M-L	0 / 0	-105.4	-105.4	0.04 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.94 (D-F:1), BC=0.33 (T-V:2), WB=0.81 (D-T:1), SSI=0.30 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1666

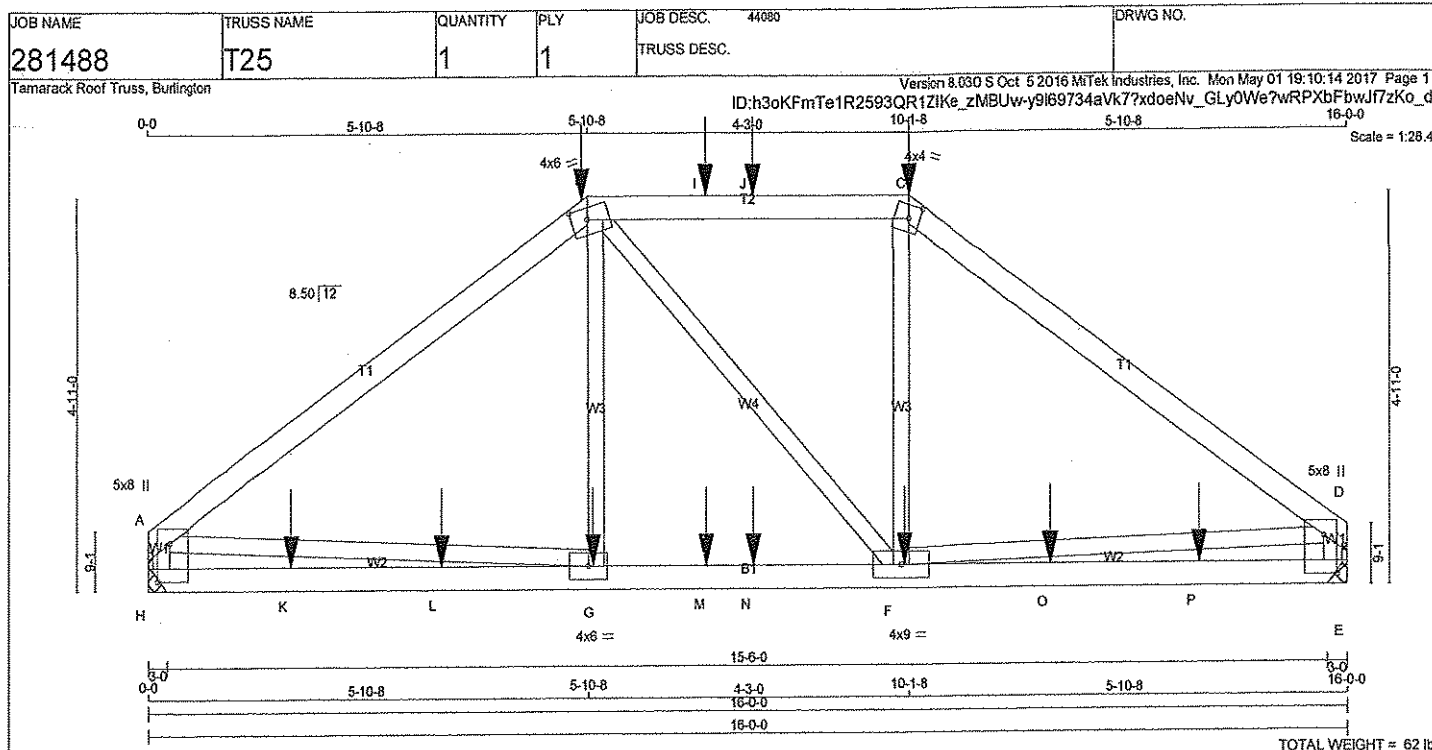
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.60 (B) (INPUT = 1.00)

DWG NO. TAM20296.11
STRUCTURAL
COMPONENT ONLY





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, D, E, H						
A	TMBMWV1+pmT20		5.0	8.0	5.75	2.00
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTWW-m	MT20	4.0	4.0	2.00	1.75
D	TMBMWV1+pmT20		5.0	8.0	5.75	2.00
F	BMWWV-l	MT20	4.0	9.0		
G	BMWWV-l	MT20	4.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 335.9 lbs FACTORED DOWN AT 5-10-8, 101.7 lbs FACTORED DOWN AT 7-5-4, AND 101.7 lbs FACTORED DOWN AT 8-0-12, AND 335.9 lbs FACTORED DOWN AT 10-1-8 ON TOP CHORD, AND 59.0 lbs FACTORED DOWN AT 1-11-4, 59.0 lbs FACTORED DOWN AT 3-11-4, 59.0 lbs FACTORED DOWN AT 5-11-4, 59.0 lbs FACTORED DOWN AT 7-5-4, 59.0 lbs FACTORED DOWN AT 8-0-12, 59.0 lbs FACTORED DOWN AT 10-0-12, AND 59.0 lbs FACTORED DOWN AT 12-0-12, AND 59.0 lbs FACTORED DOWN AT 14-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
H	1420	0	1420	0	0	HANGER BY OTHERS
E	1411	0	1411	0	0	HANGER BY OTHERS

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1170	638 / 0	270 / 0	0 / 0	0 / 0	285 / 0	0 / 0
E	1163	632 / 0	268 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	MEMB.	MAX. FACTORED	FORCE	
	(LBS)		LC1 MAX		(LBS)		
			(PLF) CSI (LC)				
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-1714 / 0	-77.4	-77.4 0.68 (1)	4.26	G-B	0 / 411	0.10 (3)
B-I	-1392 / 0	-77.4	-77.4 0.59 (1)	4.58	B-F	-13 / 0	0.01 (1)
I-J	-1392 / 0	-77.4	-77.4 0.59 (1)	4.58	F-C	0 / 407	0.10 (3)
J-C	-1392 / 0	-77.4	-77.4 0.59 (1)	4.58	A-G	0 / 1400	0.35 (1)
C-D	-1701 / 0	-77.4	-77.4 0.68 (1)	4.27	F-D	0 / 1389	0.34 (1)
H-A	-1321 / 0	0.0	0.0 0.15 (1)	7.00			
E-D	-1311 / 0	0.0	0.0 0.14 (1)	7.02			
H-K	0 / 0	-28.0	-28.0 0.36 (2)	10.00			
K-L	0 / 0	-28.0	-28.0 0.36 (2)	10.00			
L-G	0 / 0	-28.0	-28.0 0.36 (2)	10.00			
G-M	0 / 1401	-28.0	-28.0 0.52 (2)	10.00			
M-N	0 / 1401	-28.0	-28.0 0.52 (2)	10.00			
N-F	0 / 1401	-28.0	-28.0 0.52 (2)	10.00			
F-O	0 / 0	-28.0	-28.0 0.37 (2)	10.00			
O-P	0 / 0	-28.0	-28.0 0.37 (2)	10.00			
P-E	0 / 0	-28.0	-28.0 0.37 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-10-8	-336	-336		FRONT	VERT	TOTAL
C	10-1-8	-336	-336		FRONT	VERT	TOTAL
F	10-0-12	-34	-59		FRONT	VERT	TOTAL
G	5-11-4	-34	-59		FRONT	VERT	TOTAL
I	7-5-4	-102	-102		FRONT	VERT	TOTAL
J	8-0-12	-102	-102		FRONT	VERT	TOTAL
K	1-11-4	-34	-59		FRONT	VERT	TOTAL
L	3-11-4	-34	-59		FRONT	VERT	TOTAL
M	7-5-4	-34	-59		FRONT	VERT	TOTAL
N	8-0-12	-34	-59		FRONT	VERT	TOTAL
O	12-0-12	-34	-59		FRONT	VERT	TOTAL
P	14-0-12	-34	-59		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.68 (A-B:1), BC=0.52 (F-G:2), WB=0.35 (A-G:1), SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

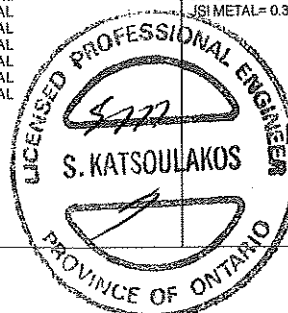
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 1.00)



DRWG NO. TAM 20297-17
STRUCTURAL
COMPONENT ONLY



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 20	-77.4	-77.4 0.19 (1)	10.00	C-G	0 / 416	0.09 (1)
B-C	-892 / 0	-77.4	-77.4 0.16 (1)	6.25	G-D	-242 / 9	0.05 (1)
C-D	-892 / 0	-77.4	-77.4 0.16 (1)	6.25	H-C	0 / 416	0.09 (1)
D-E	0 / 20	-77.4	-77.4 0.19 (1)	10.00	S-H	-242 / 9	0.05 (1)
I-A	-119 / 0	0.0	0.0 0.01 (1)	7.81	I-B	-1045 / 0	0.44 (1)
F-E	-119 / 0	0.0	0.0 0.01 (1)	7.81	D-F	-1045 / 0	0.44 (1)
I-H	0 / 810	-28.0	-28.0 0.27 (2)	10.00			
H-G	0 / 549	-28.0	-28.0 0.25 (2)	10.00			
G-F	0 / 810	-28.0	-28.0 0.27 (2)	10.00			

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.41 (D) (INPUT = 1.00)

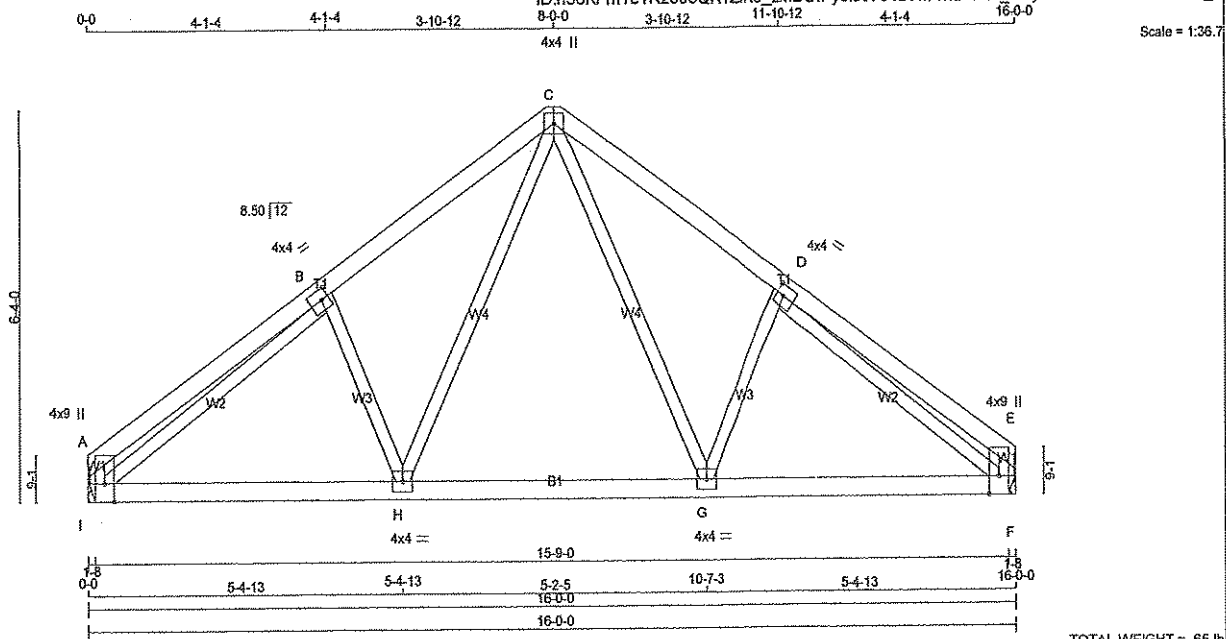
DWG NO. TAM 20248-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281488	TRUSS NAME T26CP	QUANTITY 1	PLY 1	JOB DESC. 44080	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:14 2017 Page 1

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TOTAL WEIGHT = 65 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
I - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, E, F, I						
A	TMBMW1**p	MT20	4.0	9.0	Edge	
B	TMBMW1	MT20	4.0	4.0	2.00	1.50
C	TTWW+p	MT20	4.0	4.0		
D	TMBMW1	MT20	4.0	4.0	2.00	1.50
E	TMBMW1**p	MT20	4.0	9.0	Edge	
G	BMWW1	MT20	4.0	4.0		
H	BMWW1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
I	843	0	843	0	IN-SX	IN-SX
F	843	0	843	0	MIN. SEAT SIZE: 1-8	HANGER BY OTHERS

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
I	701	373 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0
F	701	373 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 20	-77.4	-77.4	0.19 (1)	10.00	C-G	0 / 416
B-C	-892 / 0	-77.4	-77.4	0.16 (1)	6.25	G-D	-242 / 9
C-D	-892 / 0	-77.4	-77.4	0.16 (1)	6.25	H-C	0 / 416
D-E	0 / 20	-77.4	-77.4	0.19 (1)	10.00	B-H	-242 / 9
I-A	-119 / 0	0.0	0.0	0.01 (1)	7.81	I-B	-1045 / 0
F-E	-119 / 0	0.0	0.0	0.01 (1)	7.81	D-F	-1045 / 0
I-H	0 / 810	-28.0	-28.0	0.27 (2)	10.00		
H-G	0 / 549	-28.0	-28.0	0.25 (2)	10.00		
G-F	0 / 810	-28.0	-28.0	0.27 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	23.3	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	43.8	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.53")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.53")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.19 (A-B:1), BC=0.27 (H-I:2), WB=0.44 (B-I:1), SSI=0.12 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1607
	822	2264	1655

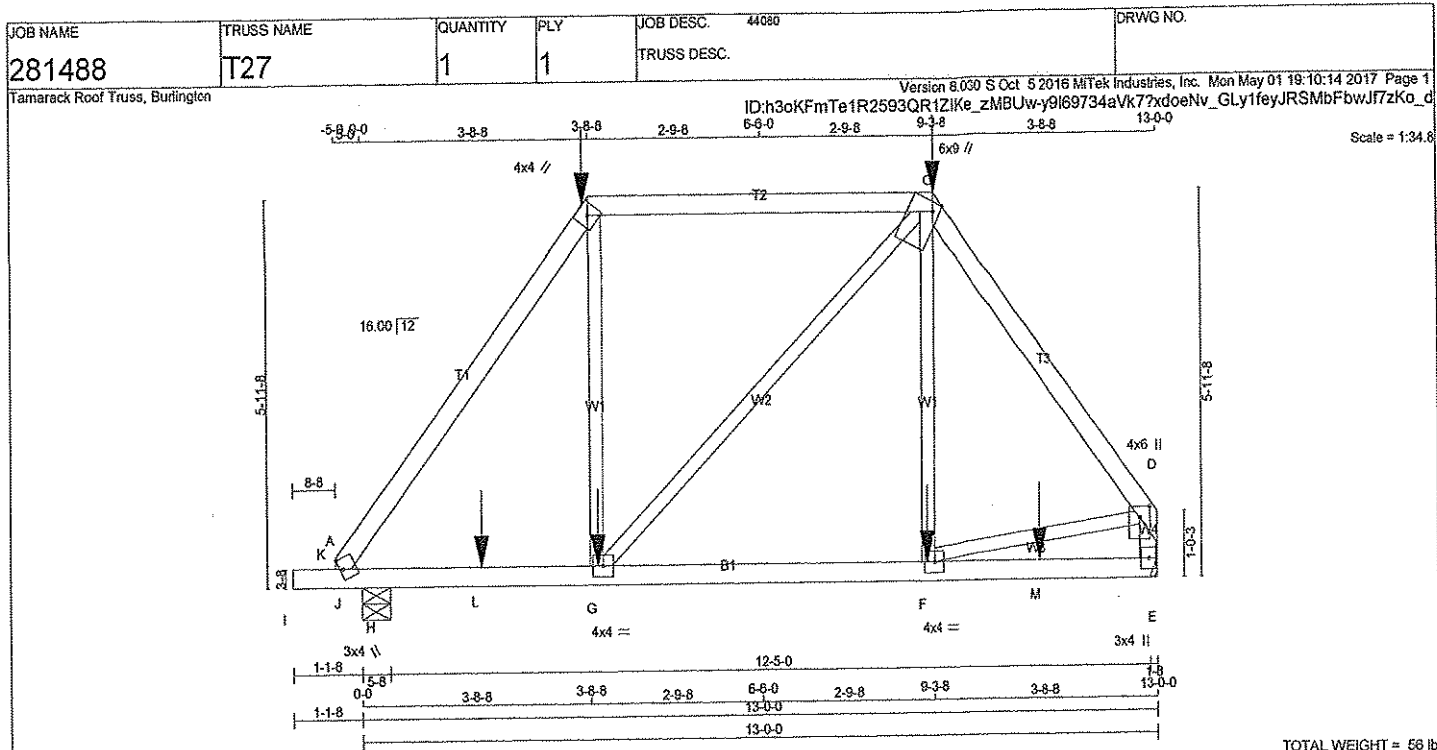
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL = 0.41 (D) (INPUT = 1.00)



DRWG NO. TAM 20299-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 58 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
I - E	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM+m	MT20	3.0	4.0		Edge
B	TTW+h	MT20	4.0	4.0		
C	TTWW+m	MT20	6.0	9.0		Edge 1.50
D	TMVV+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 111.4 lbs FACTORED DOWN AT 3-8-8, AND 111.4 lbs FACTORED DOWN AT 9-3-8 ON TOP CHORD, AND 28.9 lbs FACTORED DOWN AT 1-11-4, 394.3 lbs FACTORED DOWN AT 3-10-0, AND 394.3 lbs FACTORED DOWN AT 9-2-0, AND 26.8 lbs FACTORED DOWN AT 11-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	1172	0	1172	0	5-8	5-8
H	1346	0	1346	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	941	554 / 0	189 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	1084	631 / 0	223 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-K	0 / 681	-77.4	-77.4	0.61 (1)	10.00	G-B	0 / 414	0.10 (1)	
K-B	-1079 / 0	-77.4	-77.4	0.56 (1)	5.23	G-C	-78 / 0	0.10 (1)	
B-C	-612 / 0	-77.4	-77.4	0.47 (1)	6.25	F-C	0 / 472	0.12 (1)	
C-D	-1104 / 0	-77.4	-77.4	0.24 (1)	5.73	F-D	0 / 667	0.17 (1)	
E-D	-1138 / 0	0.0	0.0	0.13 (1)	7.42	J-K	-2165 / 0	0.00 (1)	
I-A	0 / 0	-105.4	-105.4	0.04 (1)	10.00				
A-J	0 / 605	-28.0	-28.0	0.32 (1)	10.00				
J-H	0 / 605	-28.0	-28.0	0.75 (1)	10.00				
H-L	0 / 605	-28.0	-28.0	0.75 (1)	10.00				
L-G	0 / 605	-28.0	-28.0	0.75 (1)	10.00				
G-F	0 / 687	-28.0	-28.0	0.28 (2)	10.00				
F-M	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
M-E	0 / 0	-28.0	-28.0	0.20 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-8-8	-111	-111	---	FRONT	VERT	TOTAL
C	9-3-8	-111	-111	---	FRONT	VERT	TOTAL
F	9-2-0	-394	-394	---	FRONT	VERT	TOTAL
G	3-10-0	-394	-394	---	FRONT	VERT	TOTAL
L	1-11-4	-9	-27	---	FRONT	VERT	TOTAL
M	11-0-12	-9	-27	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/229 (0.19")
CALCULATED VERT. DEFL.(LL) = L/234 (0.07")
ALLOWABLE DEFL.(TL) = L/229 (0.19")
CALCULATED VERT. DEFL.(TL) = L/229 (0.11")

CSI: TC=0.61 (A-K:1), BC=0.75 (H-J:1), WB=0.17 (D-F:1), SSI=0.99 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (A) (INPUT = 0.90)
JSI METAL= 0.33 (A) (INPUT = 1.00)

DWG NO. TAM20300-17
STRUCTURAL COMPONENT ONLY

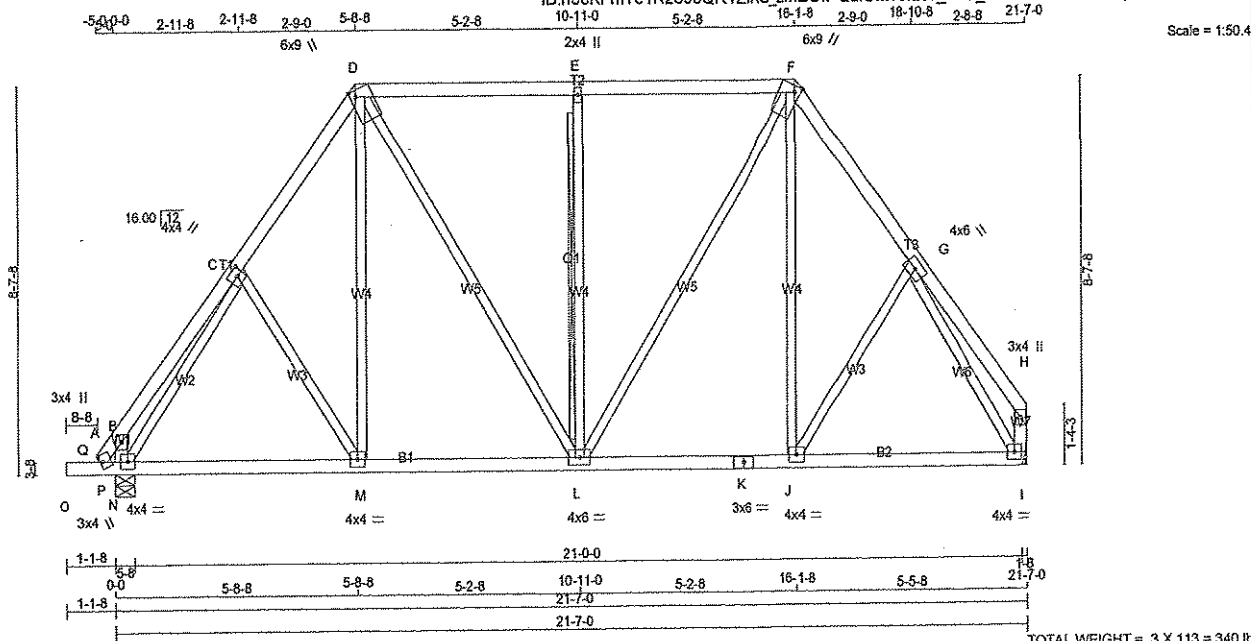


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281488	T28	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MTek Industries, Inc. Mon May 01 19:10:15 2017 Page 1

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TOTAL WEIGHT = 3 X 113 = 340 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
N - B	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
O - K	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - TBM+m	MT20	3.0	4.0	Edge			
B - TMV+p	MT20	3.0	4.0				
C - TMW+H	MT20	4.0	4.0	2.00	1.75		
D - TTWW+m	MT20	6.0	9.0	Edge	1.50		
E - TMW+w	MT20	2.0	4.0				
F - TTWW+m	MT20	6.0	9.0	Edge	1.50		
G - TMW+H	MT20	4.0	6.0				
H - TMV+p	MT20	3.0	4.0				
I - BMW+H	MT20	4.0	4.0				
J - BMW+H	MT20	4.0	4.0				
K - BS-H	MT20	3.0	6.0				
L - BMW+H	MT20	4.0	6.0				
M - BMW+H	MT20	4.0	4.0				
N - BMW+H	MT20	4.0	4.0				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	1259	0	1259	0
I	1134	0	1134	0

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM LIVE
N	1047	559 / 0	251 / 0	0 / 0
I	942	501 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-Q	-200 / 0	-77.4	-77.4	0.05 (1)	C-M	-88 / 6	0.04 (1)	
Q-B	-357 / 0	-77.4	-77.4	0.09 (1)	M-D	0 / 287	0.08 (2)	
B-C	-360 / 0	-77.4	-77.4	0.09 (1)	D-L	0 / 358	0.08 (1)	
C-D	-979 / 0	-77.4	-77.4	0.09 (1)	L-E	-485 / 0	0.57 (1)	
D-E	-768 / 0	-77.4	-77.4	0.27 (1)	E-F	0 / 376	0.08 (1)	
E-F	-768 / 0	-77.4	-77.4	0.27 (1)	F-G	0 / 271	0.06 (2)	
F-G	-687 / 0	-77.4	-77.4	0.08 (1)	G-H	-28 / 53	0.01 (1)	
G-H	0 / 18	-77.4	-77.4	0.08 (1)	H-I	-770 / 0	0.34 (1)	
H-I	-259 / 50	0.0	0.0	0.03 (1)	I-J	-1152 / 0	0.48 (1)	
I-J	-76 / 0	0.0	0.0	0.01 (1)	J-K	-182 / 0	0.00 (1)	
Q-A	0 / 0	-105.4	-105.4	0.04 (1)	K-L			
A-P	0 / 209	-28.0	-28.0	0.10 (2)	L-M			
P-N	0 / 209	-28.0	-28.0	0.24 (2)	M-N			
N-M	0 / 625	-28.0	-28.0	0.28 (2)	N-O			
M-L	0 / 578	-28.0	-28.0	0.23 (2)	O-P			
L-K	0 / 568	-28.0	-28.0	0.27 (2)	P-Q			
K-J	0 / 568	-28.0	-28.0	0.27 (2)				
J-I	0 / 582	-28.0	-28.0	0.27 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	43.8	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, ECBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.72")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.72")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = $L/190$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/190$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.27 (D-E-1), EC=0.28 (M-N-2), WB=0.57 (E-L-1), SSI=0.29 (N-P-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

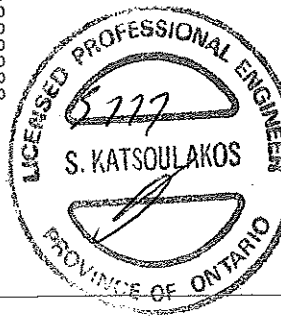
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1687	822
	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

DRWG NO. TAM 20301-17
STRUCTURAL
COMPONENT ONLY



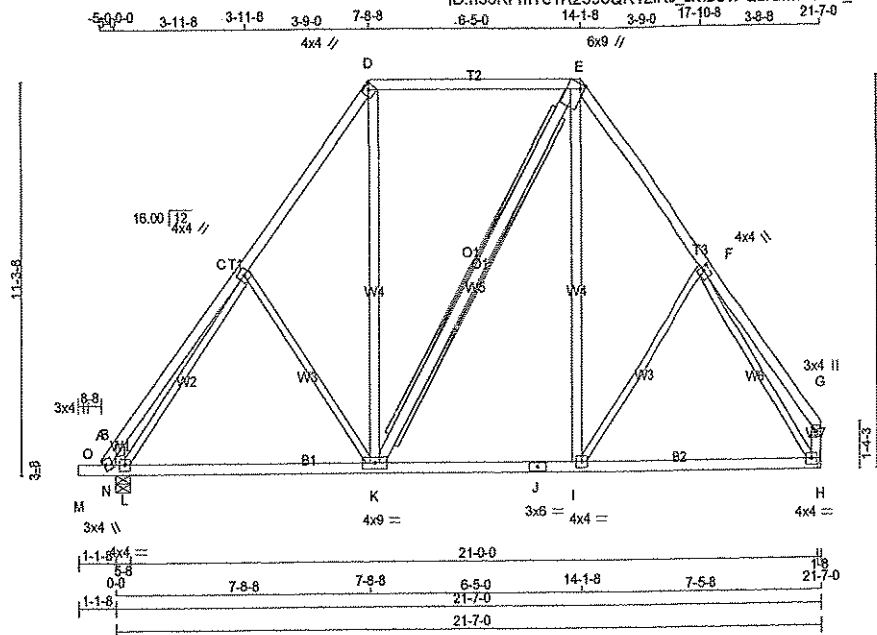
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44060	DRWG NO.
281488	T29	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:15 2017 Page 1

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Scale = 1:65.1



TOTAL WEIGHT = 125 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
C - K	2x3	DRY No.2	SPF
I - F	2x3	DRY No.2	SPF
L - C	2x3	DRY No.2	SPF
F - H	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 1259	DOWN 1259	IN-SX 5-8	IN-SX 5-8
H	HORZ 1134	HORZ 1134	UPLIFT 0	HANGER BY OTHERS
				MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
L	1047	556 / 0	251 / 0	0 / 0	0 / 0	239 / 0	0 / 0	
H	942	501 / 0	226 / 0	0 / 0	0 / 0	215 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 1-BRACE AT E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO								
A-O	-432 / 0	-77.4	-77.4	0.06 (1)	6.25	C-K	-181 / 0	0.17 (1)
O-B	-656 / 0	-77.4	-77.4	0.10 (1)	6.25	K-D	0 / 392	0.06 (2)
B-C	-547 / 0	-77.4	-77.4	0.12 (1)	6.25	K-E	-7 / 3	0.01 (3)
C-D	-888 / 0	-77.4	-77.4	0.15 (1)	6.25	I-E	0 / 397	0.06 (2)
D-E	-520 / 0	-77.4	-77.4	0.41 (1)	6.25	I-F	-127 / 25	0.12 (1)
E-F	-888 / 0	-77.4	-77.4	0.15 (1)	6.25	L-C	-575 / 0	0.50 (1)
F-G	0 / 28	-77.4	-77.4	0.17 (1)	10.00	F-H	-1112 / 0	0.92 (1)
L-B	-263 / 193	0.0	0.0	0.03 (3)	7.81	N-O	-267 / 0	0.00 (1)
H-G	-102 / 0	0.0	0.0	0.01 (1)	7.81			
M-A	0 / 0	-105.4	-105.4	0.04 (1)	10.00			
A-N	0 / 333	-28.0	-28.0	0.17 (2)	10.00			
N-L	0 / 333	-28.0	-28.0	0.40 (2)	10.00			
L-K	0 / 621	-28.0	-28.0	0.42 (2)	10.00			
K-J	0 / 519	-28.0	-28.0	0.41 (2)	10.00			
J-I	0 / 519	-28.0	-28.0	0.41 (2)	10.00			
I-H	0 / 588	-28.0	-28.0	0.41 (2)	10.00			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM+m	MT20	3.0	4.0		Edge
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-h	MT20	4.0	4.0		Edge
E	TTWW+m	MT20	6.0	9.0		Edge 1.50
F	TMWW-t	MT20	4.0	4.0		2.00 1.25
G	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = L/190 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/190 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.41 (D-E:1), BC=0.42 (K-L:2), WB=0.92 (F-H:1), SSI=0.51 (L-N:2)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 622	2284 1656

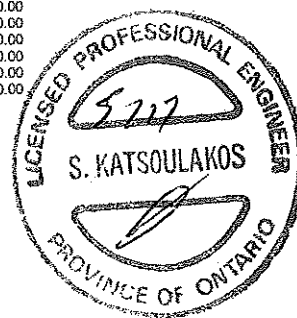
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)

DRW NO. 1-AM20302-17

STRUCTURAL COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281488	T30	1	1	TRUSS DESC.		

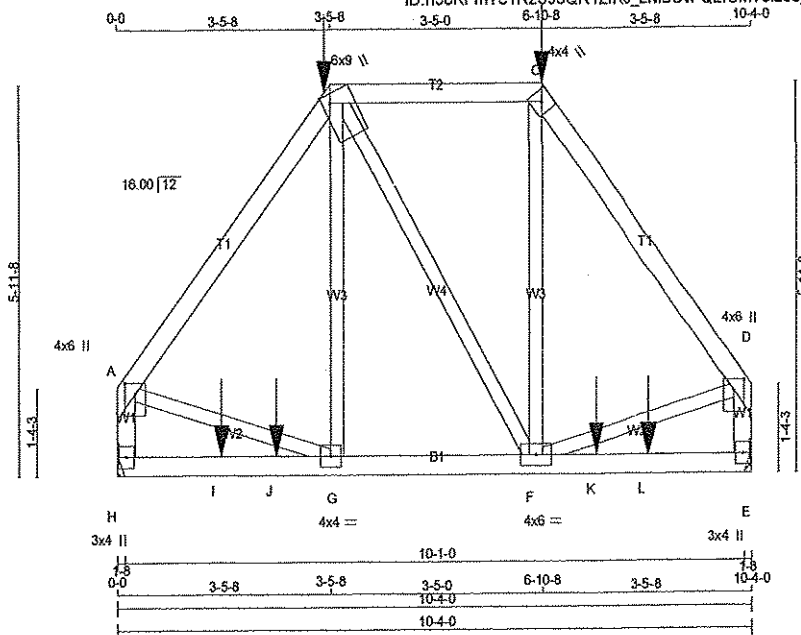
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:15 2017 Page 2
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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
AA	19-10-12	-9	-27	--	FRONT	VERT
						TOTAL



DWG NO. YAM 20303-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 51 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	9.0	Edge	1.50
C	TTW-h	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-l	MT20	4.0	6.0		
G	BMVWW-l	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 92.4 lbs FACTORED DOWN AT 3-5-8, AND 92.4 lbs FACTORED DOWN AT 6-10-8 ON TOP CHORD, AND 26.8 lbs FACTORED DOWN AT 1-8-4, 394.3 lbs FACTORED DOWN AT 2-7-0, AND 394.3 lbs FACTORED DOWN AT 7-9-0, AND 26.8 lbs FACTORED DOWN AT 8-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1046	0	1046	0
E	1046	0	1046	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN	COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
H	837	497 / 0	166 / 0	0 / 0	0 / 0	175 / 0	0 / 0	
E	837	497 / 0	166 / 0	0 / 0	0 / 0	175 / 0	0 / 0	

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC2 (PLF)	MEMB. UNBRAC LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	FACTORED MAX. (LBS)
FR-TO									
A-B	-812 / 0	-77.4	-77.4	0.20 (1)	8.25	G-B	0 / 296	0.07 (2)	
B-C	-480 / 0	-77.4	-77.4	0.17 (1)	8.25	B-F	0 / 2	0.00 (2)	
C-D	-814 / 0	-77.4	-77.4	0.20 (1)	8.25	F-C	0 / 289	0.07 (2)	
H-A	-916 / 0	0.0	0.0	0.10 (1)	7.81	A-G	0 / 501	0.12 (1)	
E-D	-918 / 0	0.0	0.0	0.10 (1)	7.81	F-D	0 / 502	0.12 (1)	
H-I	0 / 0	-28.0	-28.0	0.34 (1)	10.00				
I-J	0 / 0	-28.0	-28.0	0.34 (1)	10.00				
J-G	0 / 0	-28.0	-28.0	0.34 (1)	10.00				
G-F	0 / 489	-28.0	-28.0	0.29 (1)	10.00				
F-K	0 / 0	-28.0	-28.0	0.34 (1)	10.00				
K-L	0 / 0	-28.0	-28.0	0.34 (1)	10.00				
L-E	0 / 0	-28.0	-28.0	0.34 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-5-8	-92	-92	---	BACK	VERT	TOTAL
C	6-10-8	-92	-92	---	BACK	VERT	TOTAL
I	1-8-4	-15	-27	---	BACK	VERT	TOTAL
J	2-7-0	-394	-394	---	BACK	VERT	TOTAL
K	7-9-0	-394	-394	---	BACK	VERT	TOTAL
L	8-7-12	-15	-27	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 43.8	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL) = L/360 (0.34")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.20 (C-D:1), BC=0.34 (G-H:1), WB=0.12 (D-F:1), SSI=0.33 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

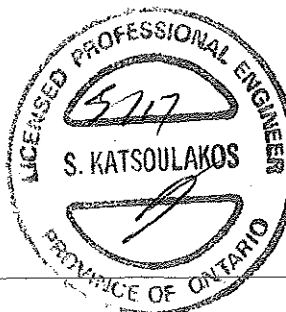
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1657
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.80)
 JSI METAL= 0.20 (C) (INPUT = 1.00)



DWG NO. TAM20304-11
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
281488	T32	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

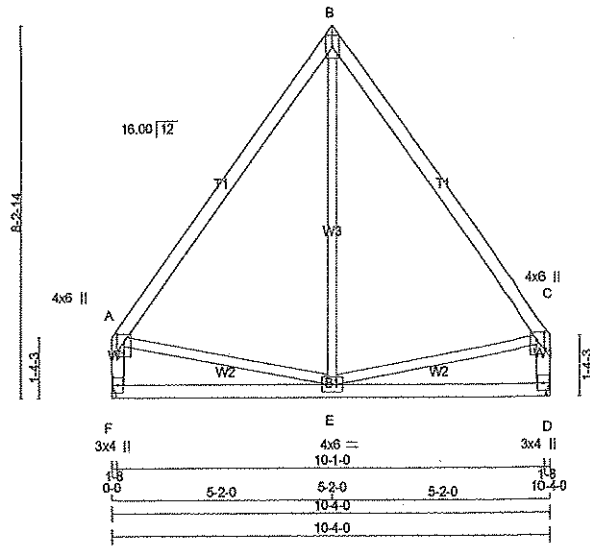
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:16 2017 Page 1

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0-0 5-2-0 5-2-0 5-2-0 10-4-0

4x6 II

Scale = 1:50.1



TOTAL WEIGHT = 3 X 48 = 145 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	4.0	6.0		
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW+1	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	544	0	544	0	0
D	544	0	544	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
F	COMBINED	SNOW	LIVE
D	452	241 / 0	108 / 0
F	452	241 / 0	108 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. MEMB.	FORCE
	(LBS)	(PLF)	CS1 (LC)	(LBS)
FR-TO		FROM TO	LENGTH FR-TO	CS1 (LC)
A-B	-318 / 0	-77.4 -77.4	0.27 (1)	6.25
B-C	-318 / 0	-77.4 -77.4	0.27 (1)	6.25
F-A	-489 / 0	0.0 0.0	0.05 (1)	7.81
D-C	-489 / 0	0.0 0.0	0.05 (1)	7.81
F-E	0 / 0	-28.0 -28.0	0.22 (3)	10.00
E-D	0 / 0	-28.0 -28.0	0.22 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.27 (A-B:1), BC=0.22 (D-E:3), WB=0.06 (B-E:3), SSI=0.12 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

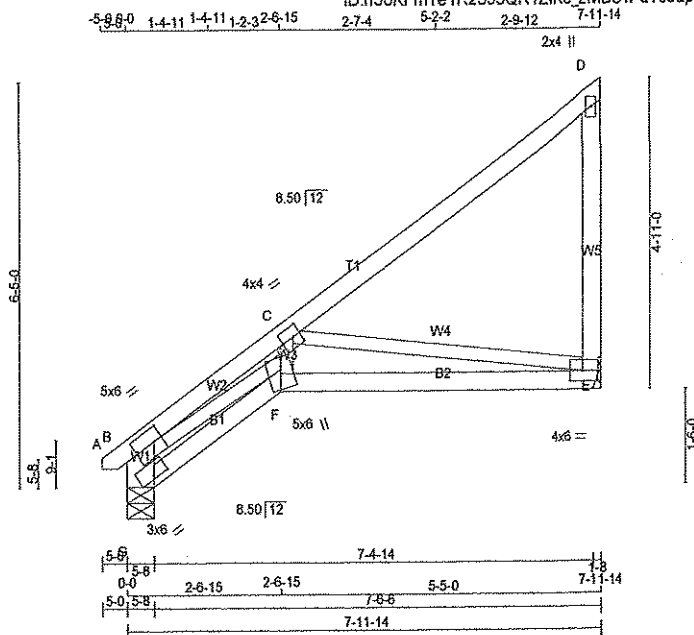
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.29 (C) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)



DWG NO. TAM20305-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 3 X 34 = 102 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - B	2x6 DRY No.2	SPF			
A - D	2x4 DRY No.2	SPF			
G - F	2x4 DRY No.2	SPF			
F - E	2x4 DRY No.2	SPF			
ALL WEBS	2x3 DRY No.2	SPF			
EXCEPT					
E - D	2x4 DRY No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	6.0	2.50	1.75
C	TMVW-1	MT20	4.0	4.0	2.00	1.50
D	TMVW-1	MT20	2.0	4.0		
E	BMVW-1	MT20	4.0	6.0		
F	BDVW-1	MT20	5.0	6.0		
G	BVM-1	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT		DOWN	IN-SX	IN-SX
G	445	0	5-8	5-8
E	407	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	365	202 / 0	82 / 0	0 / 0	0 / 0	81 / 0	0 / 0
E	339	179 / 0	82 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
G-B	-409 / 0	0.0 0.0 0.03 (1)	7.81	0.1207 0.27 (1)
A-B	0 / 9	-77.4 -77.4 0.01 (1)	10.00	0.1587 0.13 (1)
B-C	-1337 / 0	-77.4 -77.4 0.15 (1)	5.48	E-D -170 / 0 0.04 (1)
C-D	-18 / 0	-77.4 -77.4 0.26 (1)	6.25	C-E -1048 / 0 0.48 (1)
G-F	0 / 0	-28.0 -28.0 0.06 (2)	10.00	
F-E	0 / 1038	-28.0 -28.0 0.34 (2)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
 ALLOWABLE DEFL.(TL) = L/360 (0.26")
 CALCULATED VERT. DEFL.(TL) = L/833 (0.11")

CSI: TC=0.26 (C-D:1), BC=0.34 (E-F:2), WB=0.48 (C-E:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

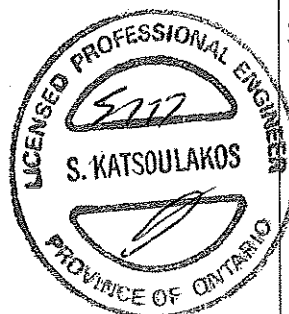
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (FLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1653

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
 JSI METAL= 0.43 (C) (INPUT = 1.00)

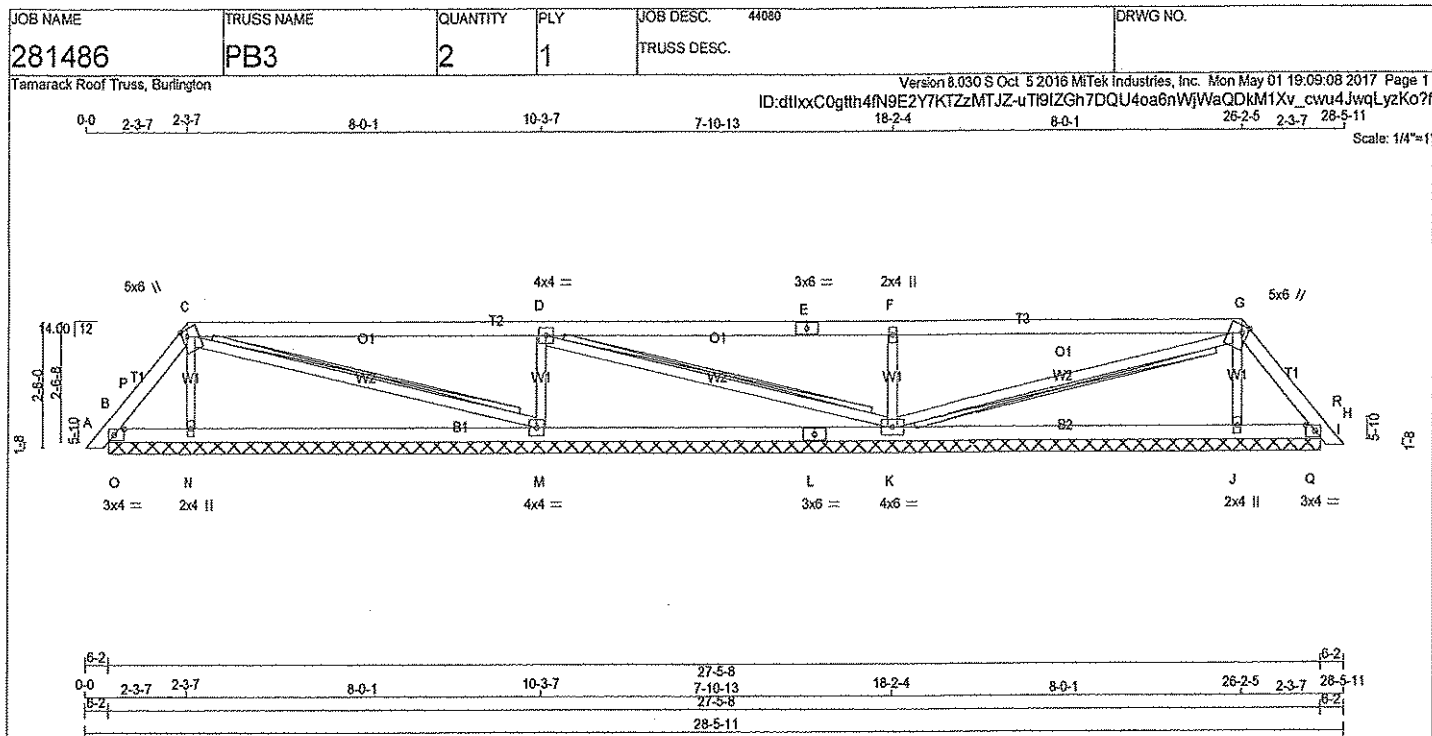


DWG NO. TAM20306.17
 STRUCTURAL
 COMPONENT ONLY



[M]

DWG NO. TAM 2028-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

No.2

SPF

A - C

2x4

DRY

No.2

SPF

C - E

2x4

DRY

No.2

SPF

E - G

2x4

DRY

No.2

SPF

G - I

2x4

DRY

No.2

SPF

B - L

2x4

DRY

No.2

SPF

L - H

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMB1-I

MT20

3.0

4.0

1.50

2.50

C

TTWW+m

MT20

5.0

6.0

1.75

1.50

D

TMWW-I

MT20

4.0

4.0

E

TS-I

MT20

3.0

6.0

F

TMW+w

MT20

2.0

4.0

G

TTWW+m

MT20

5.0

6.0

1.75

1.50

H

TMB1-I

MT20

3.0

4.0

1.50

2.50

J

BMW1+w

MT20

2.0

4.0

K

BMWW1-I

MT20

4.0

6.0

L

BS-I

MT20

3.0

6.0

M

BMWW1-I

MT20

4.0

4.0

N

BMW1+w

MT20

2.0

4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQ'D

GROSS REACTION

GROSS REACTION

BRG

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

B

124

0

124

0

-65

27-5-8 (11-6-27-5-8

N

448

0

459

0

0

27-5-8 (11-6-27-5-8

M

899

0

899

0

0

27-5-8 (11-6-27-5-8

K

917

0

917

0

0

27-5-8 (11-6-27-5-8

J

455

0

462

0

0

27-5-8 (11-6-27-5-8

H

112

0

112

0

-66

27-5-8 (11-6-27-5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE

MAX/MIN COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

B

57

103 / 0

0 / -35

0 / 0

0 / 0

0 / -10

0 / 0

N

423

145 / 0

158 / 0

0 / 0

0 / 0

122 / 0

0 / 0

M

738

408 / 0

168 / 0

0 / 0

0 / 0

164 / 0

0 / 0

K

751

417 / 0

168 / 0

0 / 0

0 / 0

166 / 0

0 / 0

J

428

149 / 0

158 / 0

0 / 0

0 / 0

123 / 0

0 / 0

H

49

96 / 0

0 / -35

0 / 0

0 / 0

0 / -11

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-M, D-K, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD LC1

MAX

MAX.

WEBS

MAX.

FACTORED

MEMB.

FORCE

VERT. LOAD

LC1

MAX

MAX.

MEMB.

FORCE

MAX

(LBS)

(PLF)

(LC)

(LBS)

(LBS)

(LBS)

(LBS)

(LBS)

FR-TO

FROM

TO

LENGTH

FR-TO

CS1 (LC)

CS1 (LC)

A-B

0 / 10

-77.4

-77.4

0.01 (1)

10.00

N-C

-228 / 0

0.04 (1)

B-P

0 / 185

-77.4

-77.4

0.08 (2)

10.00

C-M

-18 / 6

0.02 (1)

P-C

-91 / 0

-77.4

-77.4

0.05 (2)

6.25

M-D

-669 / 0

0.11 (1)

C-D

-20 / 4

-77.4

-77.4

0.67 (1)

6.25

D-K

-28 / 0

0.03 (1)

D-E

0 / 6

-77.4

-77.4

0.67 (1)

10.00

K-F

-676 / 0

0.11 (1)

E-F

0 / 6

-77.4

-77.4

0.67 (1)

10.00

K-G

-34 / 4

0.04 (1)

F-G

0 / 7

-77.4

-77.4

0.67 (1)

10.00

J-G

-235 / 0

0.04 (1)

G-R

-77 / 0

-77.4

-77.4

0.05 (2)

6.25

O-P

-261 / 0

0.00 (1)

R-H

0 / 187

-77.4

-77.4

0.08 (2)

10.00

Q-R

-262 / 0

0.00 (1)

H-I

0 / 10

-77.4

-77.4

0.01 (1)

10.00

B-O

-9 / 49

-28.0

-28.0

0.05 (1)

10.00

O-N

-9 / 49

-28.0

-28.0

0.29 (2)

10.00

N-M

-9 / 38

-28.0

-28.0

0.36 (2)

10.00

M-L

-4 / 21

-28.0

-28.0

0.36 (2)

10.00

L-K

-4 / 21

-28.0

-28.0

0.36 (2)

10.00

K-J

-10 / 29

-28.0

-28.0

0.36 (3)

6.25

J-Q

-9 / 40

-28.0

-28.0

0.29 (2)

10.00

Q-H

-9 / 40

-28.0

-28.0

0.05 (1)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

23.3

PSF

DL

=

3.0

PSF

BOT CH.

LL

=

10.5

PSF

DL

=

7.0

PSF

TOTAL LOAD

=

43.8

PSF

SPACING

=

24.0

IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, ECBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.67 (C-D:1), BC=0.36 (M-N:2), WB=0.11 (F-K:1), SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2264 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (K) (INPUT = 0.90)

JSI METAL= 0.12 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER

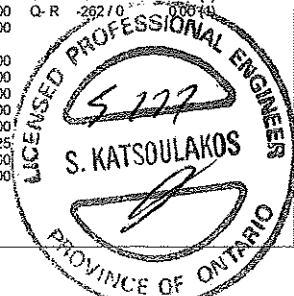
5127

S. KATSOUKAKOS

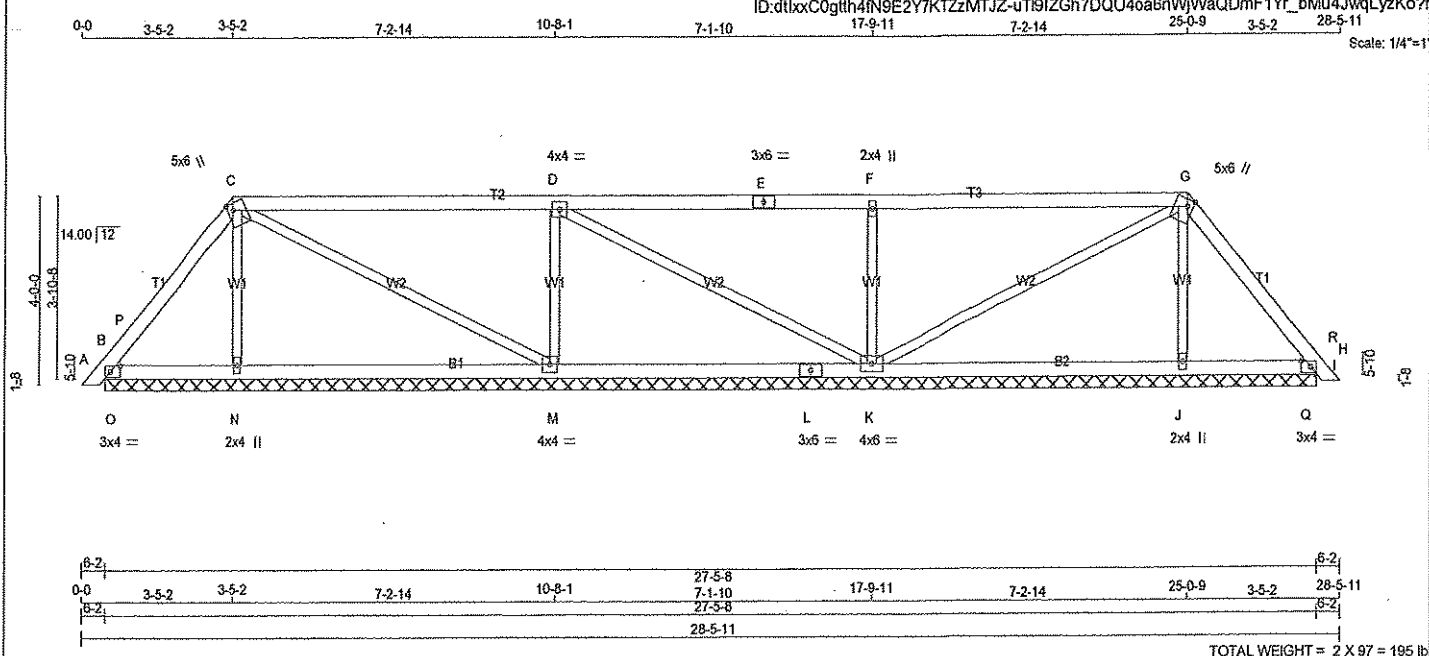
DWG NO. TAN2020217

STRUCTURAL

COMPONENT ONLY



DWG NO. TAM2018217
STRUCTURAL COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQ'D	
	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
B	255	0	255	0	27-5-8	11-6-27-5-8		
N	393	0	393	0	27-5-8	11-6-27-5-8		
M	801	0	801	0	27-5-8	11-6-27-5-8		
K	877	0	877	0	27-5-8	11-6-27-5-8		
J	400	0	400	0	27-5-8	11-6-27-5-8		
H	229	0	229	0	27-5-8	11-6-27-5-8		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMWW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.50
H	TMB1-I	MT20	3.0	4.0	1.50	2.50
J	BMW1+w	MT20	2.0	4.0		
K	BMWWW1-I	MT20	4.0	6.0		
L	BS-I	MT20	3.0	6.0		
M	BMWWW1-t	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	178	148 / 0	6 / 0	0 / 0	0 / 0	23 / 0	0 / 0
N	365	133 / 0	129 / 0	0 / 0	0 / 0	103 / 0	0 / 0
M	661	360 / 0	153 / 0	0 / 0	0 / 0	148 / 0	0 / 0
K	712	405 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
J	370	137 / 0	129 / 0	0 / 0	0 / 0	104 / 0	0 / 0
H	160	133 / 0	6 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

CSI: TC=0.55 (C-D:1), BC=0.30 (M-N:2), WB=0.14 (F-K:1), SSI=0.26 (C-D:1)

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

LOADING

TOTAL LOAD CASES: (4)

COMPANION LIVE LOAD FACTOR = 0.50

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED			
		VERT. LOAD (PLF)	LC1 MAX (LC)			FORCE (LBS)	MAX (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0/10	-77.4	-77.4	0.01 (1)	10.00	N-C	-202/0	0.05 (1)	
B-P	0/63	-77.4	-77.4	0.07 (1)	10.00	C-M	-36/0	0.05 (1)	
P-C	-140/0	-77.4	-77.4	0.07 (1)	6.25	M-D	-582/0	0.14 (1)	
C-D	-45/0	-77.4	-77.4	0.55 (1)	6.25	D-K	-64/0	0.09 (1)	
D-E	0/13	-77.4	-77.4	0.55 (1)	10.00	K-F	-611/0	0.14 (1)	
E-F	0/13	-77.4	-77.4	0.55 (1)	10.00	K-G	-77/0	0.10 (1)	
F-G	0/13	-77.4	-77.4	0.55 (1)	10.00	J-G	-209/0	0.05 (1)	
G-R	-108/0	-77.4	-77.4	0.07 (1)	6.25	O-P	-326/0	0.00 (1)	
R-H	0/83	-77.4	-77.4	0.07 (1)	10.00	Q-R	-333/0	0.00 (1)	
H-I	0/10	-77.4	-77.4	0.01 (1)	10.00				
B-O	0/83	-28.0	-28.0	0.09 (1)	10.00				
C-N	0/83	-28.0	-28.0	0.23 (2)	10.00				
N-M	0/77	-28.0	-28.0	0.30 (2)	10.00				
M-L	0/45	-28.0	-28.0	0.30 (2)	10.00				
L-K	0/45	-28.0	-28.0	0.30 (2)	10.00				
K-J	0/56	-28.0	-28.0	0.30 (2)	10.00				
J-Q	0/62	-28.0	-28.0	0.23 (2)	10.00				
Q-H	0/62	-28.0	-28.0	0.09 (1)	10.00				



Professional Engineer Seal with the number 5177.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

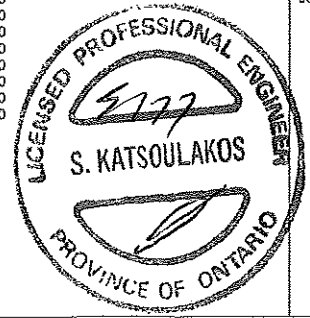
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.68 (K) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)



DWG NO. TAM202B3.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	PB5	2	1	TRUSS DESC.		

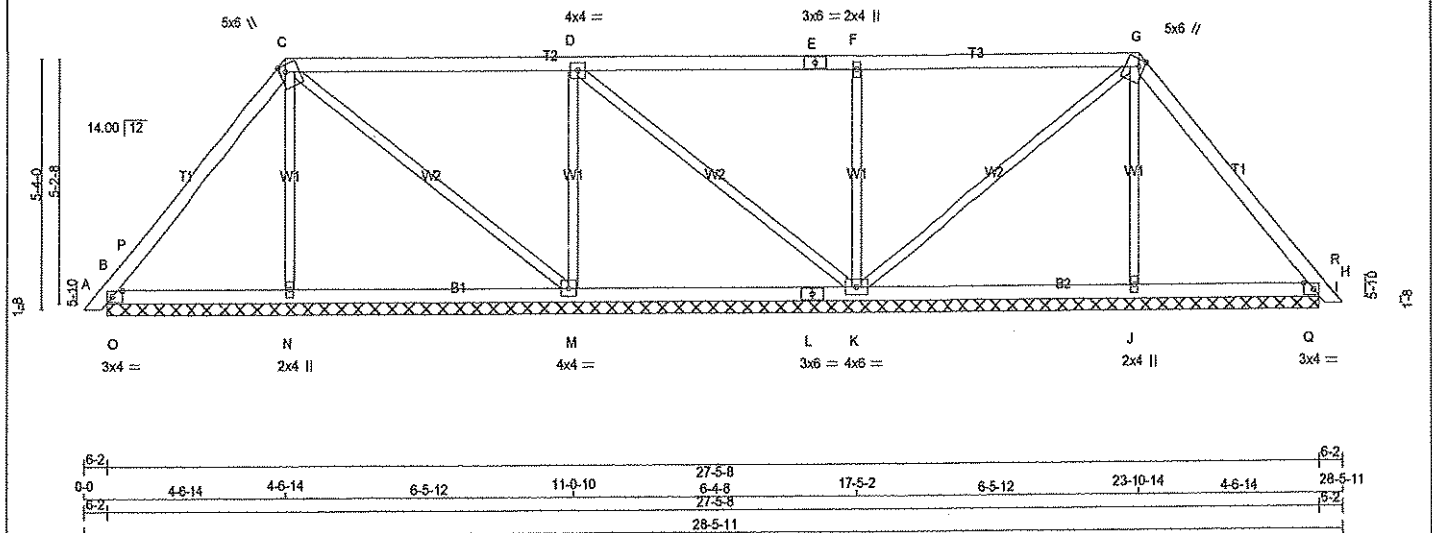
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:09:08 2017 Page 1

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0-0 4-6-14 4-6-14 6-5-12 11-0-10 6-4-8 17-5-2 6-5-12 23-10-14 4-6-14 28-5-11

Scale: 1/4"=1'



TOTAL WEIGHT = 2 X 103 = 207 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	372	0	372	0	0	27-5-8	11-6-27-5-8		
N	349	0	350	0	0	27-5-8	11-6-27-5-8		
M	690	0	690	0	0	27-5-8	11-6-27-5-8		
K	661	0	661	0	0	27-5-8	11-6-27-5-8		
J	351	0	351	0	0	27-5-8	11-6-27-5-8		
H	331	0	331	0	0	27-5-8	11-6-27-5-8		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	279	197 / 0	34 / 0	0 / 0	0 / 0	48 / 0	0 / 0
N	327	116 / 0	117 / 0	0 / 0	0 / 0	93 / 0	0 / 0
M	573	306 / 0	137 / 0	0 / 0	0 / 0	131 / 0	0 / 0
K	689	408 / 0	137 / 0	0 / 0	0 / 0	144 / 0	0 / 0
J	328	117 / 0	117 / 0	0 / 0	0 / 0	93 / 0	0 / 0
H	251	172 / 0	34 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		UNBRAC LENGTH FR-TO			
A-B	0 / 10	-77.4	-77.4 0.01 (1)	10.00	N-C	-156 / 0	0.06 (1)
B-P	-75 / 104	-77.4	-77.4 0.11 (1)	6.25	C-M	-54 / 0	0.07 (1)
P-C	-205 / 0	-77.4	-77.4 0.16 (1)	6.25	M-D	-480 / 0	0.19 (1)
C-D	-78 / 0	-77.4	-77.4 0.44 (1)	6.25	D-K	-108 / 0	0.15 (1)
D-E	-6 / 8	-77.4	-77.4 0.44 (1)	10.00	K-F	-546 / 0	0.22 (1)
E-F	-6 / 8	-77.4	-77.4 0.44 (1)	10.00	F-G	-118 / 0	0.16 (1)
F-G	-6 / 8	-77.4	-77.4 0.44 (1)	10.00	G-H	-157 / 0	0.06 (1)
G-R	-152 / 0	-77.4	-77.4 0.16 (1)	6.25	O-P	-513 / 37	0.00 (1)
R-H	-69 / 166	-77.4	-77.4 0.12 (1)	6.25	Q-R	-524 / 36	0.00 (1)
H-I	0 / 10	-77.4	-77.4 0.01 (1)	10.00			
B-O	0 / 124	-28.0	-28.0 0.14 (1)	10.00			
O-N	0 / 124	-28.0	-28.0 0.21 (2)	10.00			
N-M	0 / 121	-28.0	-28.0 0.24 (2)	10.00			
M-L	0 / 78	-28.0	-28.0 0.24 (2)	10.00			
L-K	0 / 78	-28.0	-28.0 0.24 (2)	10.00			
K-J	0 / 87	-28.0	-28.0 0.24 (2)	10.00			
J-Q	0 / 90	-28.0	-28.0 0.21 (2)	10.00			
Q-H	0 / 80	-28.0	-28.0 0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44 (C-D:1), BC=0.24 (M-N:2), WB=0.22 (F-K:1), SS=0.42 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.67 (E) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)



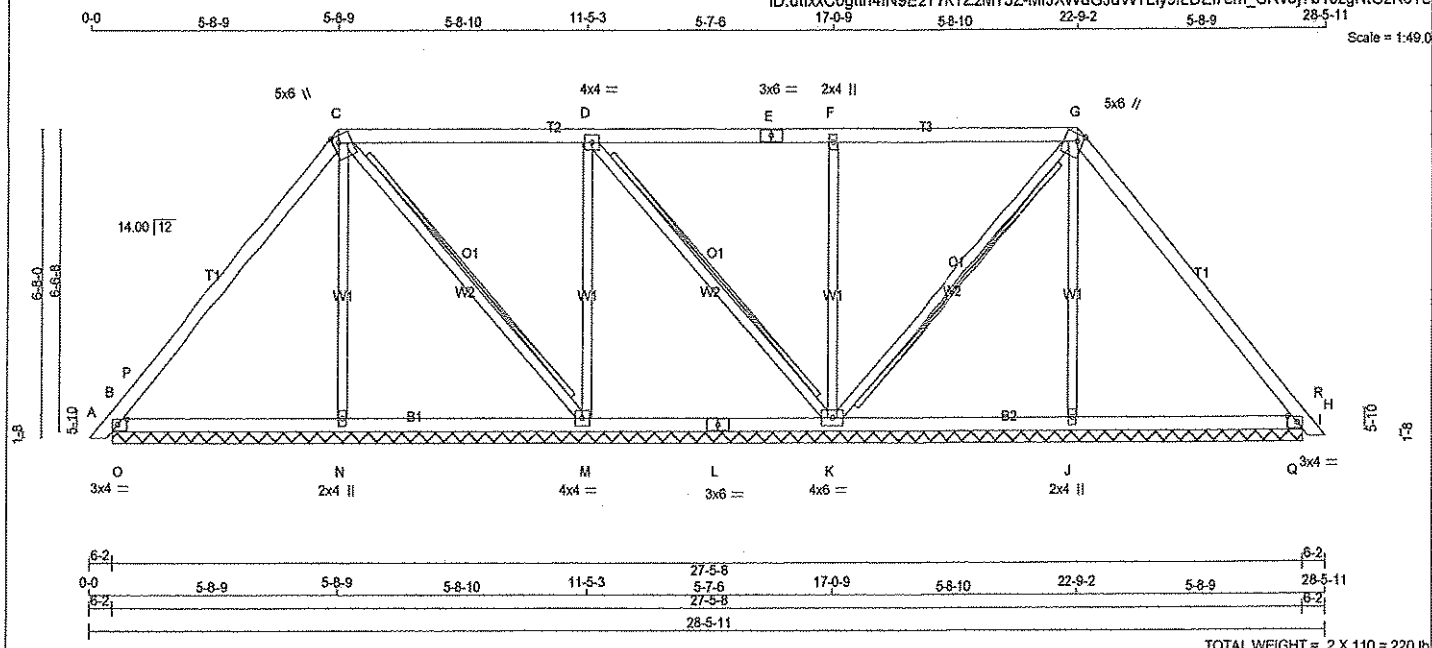
DWG NO. TAM 2018/117
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	PB6	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M-Tek Industries, Inc. Mon May 01 19:09:09 2017 Page 1

ID:dlbxC0gtth4fN9E2Y7KTZzMTJZ-MfJXWuGJuWYLly9lDEI7em_CRv8j?b1JzgNtOzKo7e



LUMBER	SIZE	DRY	DESCR.
N. L. G. A. RULES			
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	487	0	487	0	0	27-5-8 (13-6-27-5-8)	
N	314	0	328	0	0	27-5-8 (13-6-27-5-8)	
M	572	0	572	0	0	27-5-8 (13-6-27-5-8)	
K	834	0	834	0	0	27-5-8 (13-6-27-5-8)	
J	312	0	327	0	0	27-5-8 (13-6-27-5-8)	
H	435	0	435	0	0	27-5-8 (13-6-27-5-8)	

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD = 43.8 PSF			
SPACING = 24.0 IN. C/C			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMB1-I	MT20	3.0	4.0	1.50 2.50
C	TTWW+m	MT20	5.0	6.0	1.75 1.50
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW+w	MT20	2.0	4.0	
G	TTWW+m	MT20	5.0	6.0	1.75 1.50
H	TMB1-I	MT20	3.0	4.0	1.50 2.50
J	BMW1+w	MT20	2.0	4.0	
K	BMWWW1-t	MT20	4.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW1-t	MT20	4.0	4.0	
N	BMW1+w	MT20	2.0	4.0	

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
B	375	247 / 0	58 / 0	0 / 0	0 / 0	71 / 0	0 / 0
N	299	99 / 0	113 / 0	0 / 0	0 / 0	88 / 0	0 / 0
M	478	250 / 0	118 / 0	0 / 0	0 / 0	111 / 0	0 / 0
K	656	408 / 0	117 / 0	0 / 0	0 / 0	131 / 0	0 / 0
J	298	97 / 0	113 / 0	0 / 0	0 / 0	88 / 0	0 / 0
H	340	215 / 0	58 / 0	0 / 0	0 / 0	67 / 0	0 / 0

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-M, D-K, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 10	-77.4 -77.4	0.01 (1)	10.00	N-C	-102 / 14	0.07 (1)
B-P	-214 / 236	-77.4 -77.4	0.20 (1)	6.25	C-M	-65 / 0	0.08 (1)
P-C	-280 / 0	-77.4 -77.4	0.27 (1)	6.25	M-D	-377 / 0	0.26 (1)
C-D	-124 / 0	-77.4 -77.4	0.34 (1)	6.25	D-K	-140 / 0	0.16 (1)
D-E	-32 / 0	-77.4 -77.4	0.34 (1)	6.25	K-F	-481 / 0	0.34 (1)
E-F	-32 / 0	-77.4 -77.4	0.34 (1)	6.25	K-G	-139 / 0	0.16 (1)
F-G	-32 / 0	-77.4 -77.4	0.34 (1)	6.25	J-G	-99 / 14	0.07 (1)
G-R	-213 / 0	-77.4 -77.4	0.27 (1)	6.25	O-P	-794 / 160	0.00 (1)
R-H	-207 / 315	-77.4 -77.4	0.21 (1)	6.25	Q-R	-597 / 154	0.06 (1)
H-I	0 / 10	-77.4 -77.4	0.01 (1)	10.00			
B-Q	0 / 170	-28.0 -28.0	0.22 (1)	10.00			
O-N	0 / 170	-28.0 -28.0	0.23 (1)	10.00			
N-M	0 / 169	-28.0 -28.0	0.23 (2)	10.00			
M-L	0 / 125	-28.0 -28.0	0.18 (2)	10.00			
L-K	0 / 125	-28.0 -28.0	0.18 (2)	10.00			
K-J	0 / 125	-28.0 -28.0	0.22 (2)	10.00			
J-Q	0 / 127	-28.0 -28.0	0.23 (1)	10.00			
Q-H	0 / 127	-28.0 -28.0	0.22 (1)	10.00			

CS: TC=0.34 (C-D:1), BC=0.23 (N-O:1), WB=0.34 (F-K:1), SSI=0.65 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

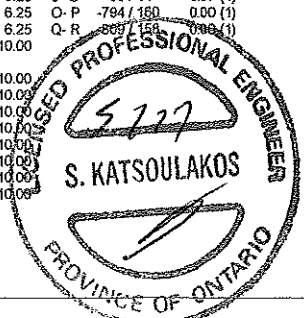
NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PL)	(PL)			
MT20	618	354	1667	822	2284
					1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)

JSI METAL = 0.18 (L) (INPUT = 1.00)



DWG NO. TAM2028517

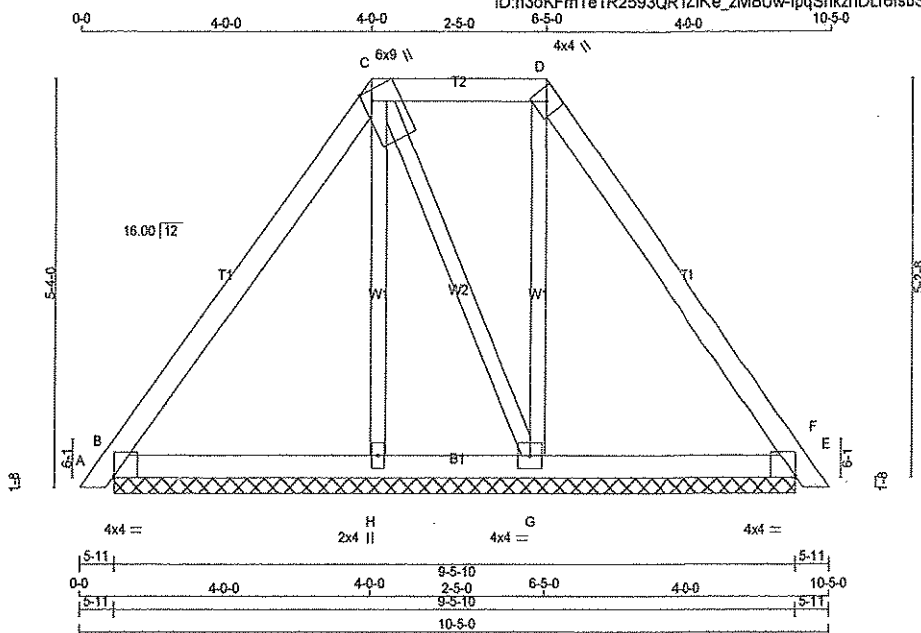
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281488	PB20	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:07 2017 Page 1

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Scale = 1/29.5

TOTAL WEIGHT = 40 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	4.0	4.0	0.25
C TTWW+m	MT20	6.0	9.0	Edge 1.50
D TTW-h	MT20	4.0	4.0	
E TMB1-I	MT20	4.0	4.0	0.25
G BMWW1-t	MT20	4.0	4.0	
H BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	IN-SX	BRG	IN-SX
B	295	0	0	295	0	0	9-5-10	9-5-10	
E	267	0	0	267	0	0	9-5-10	9-5-10	
H	187	0	0	186	0	0	9-5-10	9-5-10	
G	304	0	0	304	0	0	9-5-10	9-5-10	

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	225	152 / 0	32 / 0	0 / 0	0 / 0	41 / 0	0 / 0
E	206	135 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0
H	179	59 / 0	67 / 0	0 / 0	0 / 0	62 / 0	0 / 0
G	258	129 / 0	67 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 10	-77.4	-77.4 0.01 (1)	10.00	H-C	-97 / 0	0.04 (1)
B-C	-108 / 0	-77.4	-77.4 0.16 (1)	6.25	C-G	-49 / 0	0.02 (1)
C-D	-40 / 0	-77.4	-77.4 0.08 (1)	6.25	G-D	-169 / 0	0.07 (1)
D-E	-75 / 0	-77.4	-77.4 0.16 (1)	6.25			
E-F	0 / 10	-77.4	-77.4 0.01 (1)	10.00			
B-H	0 / 64	-28.0	-28.0 0.06 (2)	10.00			
H-G	0 / 62	-28.0	-28.0 0.07 (2)	10.00			
G-E	0 / 44	-28.0	-28.0 0.08 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16 (B-C:1), BC=0.08 (B-H:2), WB=0.07 (D-G:1), SSI=0.08 (B-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



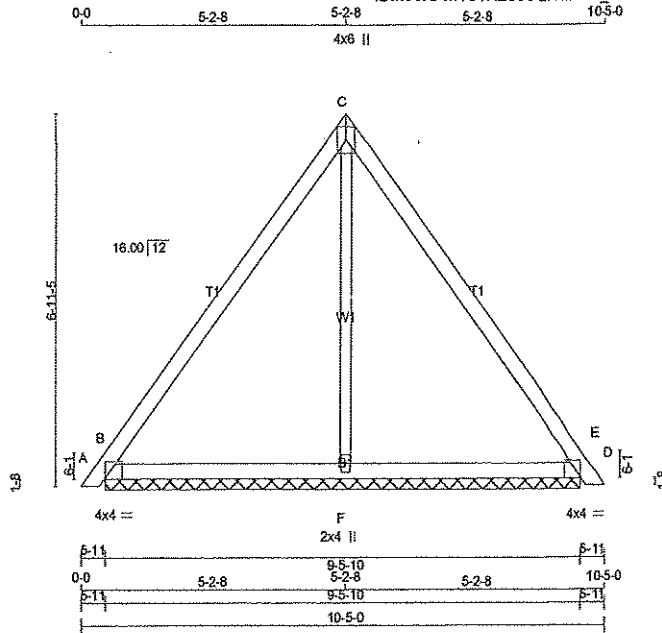
DWG NO. TAM 20307-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281488	PB21	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:07 2017 Page 1

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Scale = 1:42.4

TOTAL WEIGHT = 38 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UP	BRG	IN-SX	IN-SX
B	361	0	361	0	0	0	9-5-10	9-5-10	9-5-10
D	361	0	361	0	0	0	9-5-10	9-5-10	9-5-10
F	331	0	356	0	0	0	9-5-10	9-5-10	9-5-10

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
B	274	187 / 0	37 / 0	0 / 0	0 / 0	49 / 0	0 / 0	274	187 / 0	37 / 0	0 / 0
D	274	187 / 0	37 / 0	0 / 0	0 / 0	49 / 0	0 / 0	274	187 / 0	37 / 0	0 / 0
F	320	100 / 0	124 / 0	0 / 0	0 / 0	96 / 0	0 / 0	320	100 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 10	-77.4	-77.4 0.01 (1)	F-C	-166 / 0	0.12 (1)	10.00
B-C	-125 / 0	-77.4	-77.4 0.30 (1)				6.25
C-D	-125 / 0	-77.4	-77.4 0.30 (1)				6.25
D-E	0 / 10	-77.4	-77.4 0.01 (1)				10.00
B-F	0 / 74	-28.0	-28.0 0.19 (2)				10.00
F-D	0 / 74	-28.0	-28.0 0.19 (2)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30 (B-C:1), BC=0.19 (D-F:2), WB=0.12 (C-F:1), SSI=0.11 (B-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

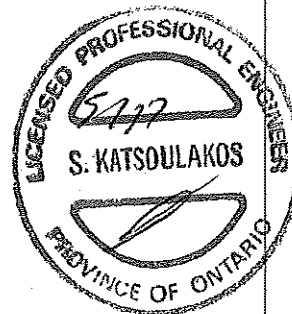
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DWG NO. TAM20308-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44680	DRWG NO.
281488	PB22	2	1	TRUSS DESC.		

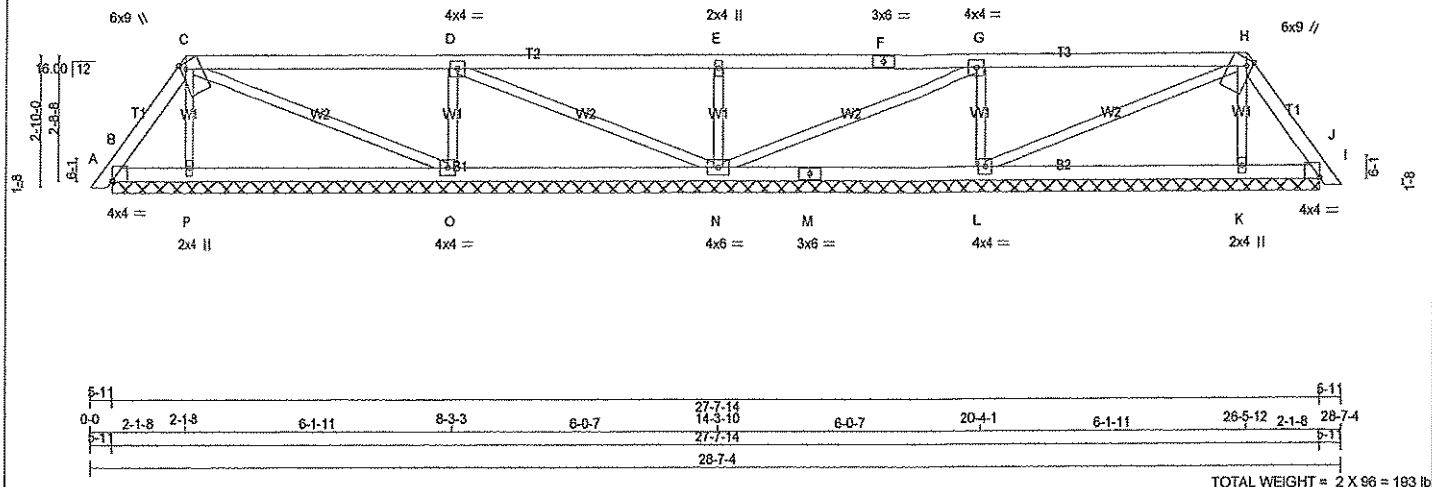
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:08 2017 Page 1

ID:h3oKfMTe1R2593QR1ZiKe_zMBUw-7?Nqu4_J_fzzH0Ael6oa04i37D1A1i7t7?SUzKo_

0-0 2-1-8 2-1-8 6-1-11 8-3-3 6-0-7 14-3-10 6-0-7 20-4-1 6-1-11 26-5-12 2-1-8 26-7-4

Scale = 1:48.8



TOTAL WEIGHT = 2 X 96 = 193 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
B - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	149	0	149	0	0	27-7-14 (11-82) 14			
P	315	0	315	0	0	27-7-14 (11-82) 14			
O	703	0	703	0	0	27-7-14 (11-82) 14			
N	636	0	636	0	0	27-7-14 (11-82) 14			
L	703	0	703	0	0	27-7-14 (11-82) 14			
K	315	0	315	0	0	27-7-14 (11-82) 14			
I	149	0	149	0	0	27-7-14 (11-82) 14			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT I FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	94	98/0	0/-10	0/0	0/0	6/0	0/0
P	295	103/0	107/0	0/0	0/0	85/0	0/0
O	577	318/0	131/0	0/0	0/0	128/0	0/0
N	527	283/0	125/0	0/0	0/0	120/0	0/0
L	577	318/0	131/0	0/0	0/0	128/0	0/0
K	295	103/0	107/0	0/0	0/0	85/0	0/0
I	94	98/0	0/-10	0/0	0/0	6/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P, O, N, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/10	-77.4 -77.4	0.01 (1)	P-C	-172/0	0.03 (1)	
B-C	-85/0	-77.4 -77.4	0.04 (1)	C-O	-25/0	0.02 (1)	
C-D	-18/0	-77.4 -77.4	0.42 (1)	O-D	-520/0	0.09 (1)	
D-E	0/31	-77.4 -77.4	0.43 (1)	D-N	-53/0	0.04 (1)	
E-F	0/31	-77.4 -77.4	0.43 (1)	N-E	-430/0	0.07 (1)	
F-G	0/31	-77.4 -77.4	0.43 (1)	N-G	-53/0	0.04 (1)	
G-H	-18/0	-77.4 -77.4	0.42 (1)	L-G	-520/0	0.09 (1)	
H-I	-85/0	-77.4 -77.4	0.04 (1)	L-H	-25/0	0.02 (1)	
I-J	0/10	-77.4 -77.4	0.01 (1)	K-H	-172/0	0.03 (1)	
B-P	0/49	-28.0 -28.0	0.16 (2)				
P-O	0/42	-28.0 -28.0	0.22 (2)				
O-N	0/19	-28.0 -28.0	0.22 (2)				
N-M	0/19	-28.0 -28.0	0.22 (2)				
M-L	0/19	-28.0 -28.0	0.22 (2)				
L-K	0/42	-28.0 -28.0	0.22 (2)				
K-I	0/49	-28.0 -28.0	0.16 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010.

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43 (D-E:1), BC=0.22 (O-P:2), WB=0.09 (D-O:1), SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)



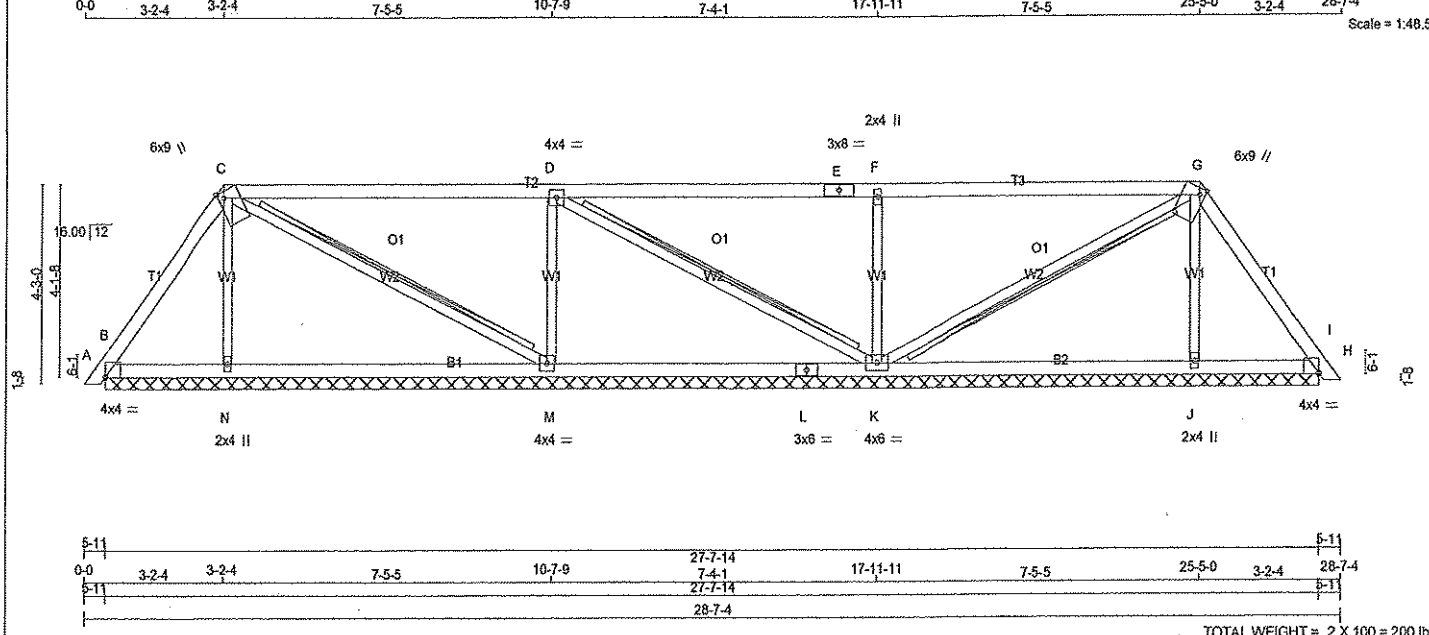
DRWG NO. TAM20309-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 281488	TRUSS NAME PB23	QUANTITY 2	PLY 1	JOB DESC. 44080	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Mon May 01 19:10:08 2017 Page 1

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
B - L	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	0.25	
C	TTWW+m	MT20	6.0	9.0	Edge	1.50
D	TMWW-I	MT20	4.0	4.0		
E	TS-I	MT20	3.0	8.0		
F	TMWW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.50
H	TMB1-I	MT20	4.0	4.0	0.25	
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-I	MT20	4.0	6.0		
L	BS-I	MT20	3.0	8.0		
M	BMWW1-I	MT20	4.0	4.0		
N	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT		
B	243	0	243	0	27-7-14 (11-82) 3-14	
N	389	0	389	0	27-7-14 (11-82) 3-14	
M	821	0	821	0	27-7-14 (11-82) 3-14	
K	907	0	907	0	27-7-14 (11-82) 3-14	
J	388	0	388	0	27-7-14 (11-82) 3-14	
H	212	0	212	0	27-7-14 (11-82) 3-14	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	168	143 / 0	4 / 0	0 / 0	0 / 0	21 / 0	0 / 0
N	362	131 / 0	129 / 0	0 / 0	0 / 0	103 / 0	0 / 0
M	678	367 / 0	158 / 0	0 / 0	0 / 0	153 / 0	0 / 0
K	736	419 / 0	158 / 0	0 / 0	0 / 0	159 / 0	0 / 0
J	368	136 / 0	129 / 0	0 / 0	0 / 0	103 / 0	0 / 0
H	147	125 / 0	4 / 0	0 / 0	0 / 0	19 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-M, D-K, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS		MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)
FR-TO					FR-TO		
A-B	0 / 10	-77.4	-77.4	0.01 (1)	N-C	-217 / 0	0.06 (1)
B-C	-129 / 0	-77.4	-77.4	0.10 (1)	C-M	-32 / 0	0.04 (1)
C-D	-40 / 0	-77.4	-77.4	0.58 (1)	M-D	-566 / 0	0.15 (1)
D-E	0 / 22	-77.4	-77.4	0.58 (1)	D-K	-70 / 0	0.08 (1)
E-F	0 / 22	-77.4	-77.4	0.58 (1)	K-F	-629 / 0	0.16 (1)
F-G	0 / 22	-77.4	-77.4	0.58 (1)	K-G	-77 / 0	0.09 (1)
G-H	-92 / 0	-77.4	-77.4	0.10 (1)	J-G	-226 / 0	0.08 (1)
H-I	0 / 10	-77.4	-77.4	0.01 (1)			
B-N	0 / 75	-28.0	-28.0	0.23 (2)			
N-M	0 / 69	-28.0	-28.0	0.32 (2)			
M-L	0 / 41	-28.0	-28.0	0.32 (2)			
L-K	0 / 41	-28.0	-28.0	0.32 (2)			
K-J	0 / 47	-28.0	-28.0	0.32 (2)			
J-H	0 / 54	-28.0	-28.0	0.23 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.58 (D-F:1), BC=0.32 (M-N:2), WB=0.16 (F-K:1), SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822

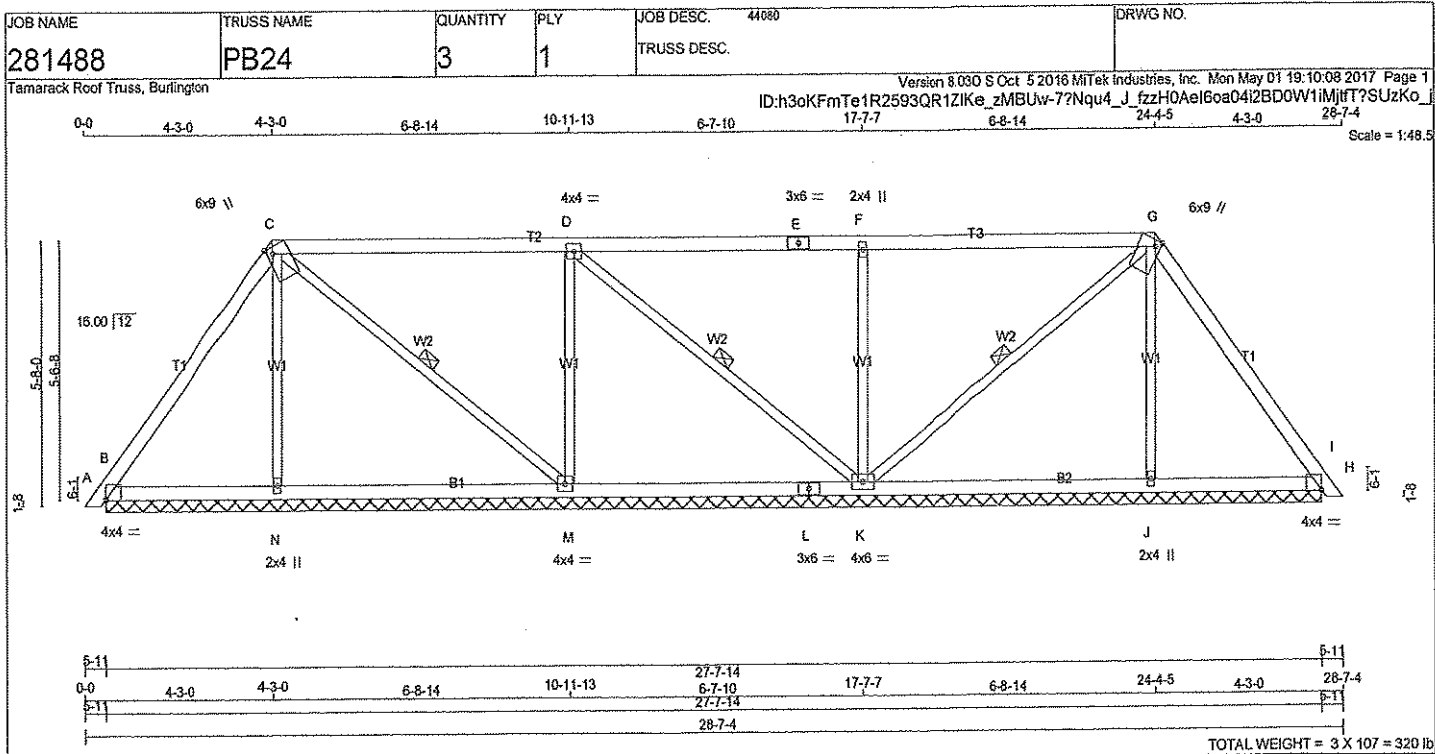
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.69 (E) (INPUT = 0.50)
JSI METAL= 0.49 (E) (INPUT = 1.00)



DWG NO. TAM 20310-17
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
B - L	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	329	0	329	0	27-7-14 (11-82) 1/4	11-82 1/4
N	371	0	371	0	27-7-14 (11-82) 1/4	11-82 1/4
M	713	0	713	0	27-7-14 (11-82) 1/4	11-82 1/4
K	900	0	900	0	27-7-14 (11-82) 1/4	11-82 1/4
J	376	0	376	0	27-7-14 (11-82) 1/4	11-82 1/4
H	281	0	281	0	27-7-14 (11-82) 1/4	11-82 1/4

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LC CASE	MAX MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
B	242	-178 / 0	25 / 0	0 / 0	39 / 0	0 / 0
N	346	124 / 0	124 / 0	0 / 0	98 / 0	0 / 0
M	592	315 / 0	142 / 0	0 / 0	135 / 0	0 / 0
K	720	428 / 0	142 / 0	0 / 0	150 / 0	0 / 0
J	350	127 / 0	124 / 0	0 / 0	99 / 0	0 / 0
H	209	149 / 0	25 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, D-K, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED		FACTORED		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED		
	FORCE (LBS)	VERT. LOAD (LBS)	(PLF)	MAX. CSI (LC)			FORCE (LBS)	MAX. CSI (LC)	
FR-TO			FROM	TO		FR-TO			
A-B	0 / 10	-77.4	-77.4	0.01 (1)	10.00	N-C	-208 / 0	0.10 (1)	
B-C	-151 / 0	-77.4	-77.4	0.19 (1)	6.25	C-M	-44 / 0	0.02 (1)	
C-D	-49 / 0	-77.4	-77.4	0.47 (1)	6.25	M-D	-496 / 0	0.23 (1)	
D-E	0 / 42	-77.4	-77.4	0.48 (1)	10.00	D-K	-116 / 0	0.06 (1)	
E-F	0 / 42	-77.4	-77.4	0.48 (1)	10.00	K-F	-589 / 0	0.26 (1)	
F-G	0 / 42	-77.4	-77.4	0.48 (1)	10.00	K-G	-115 / 0	0.06 (1)	
G-H	-91 / 0	-77.4	-77.4	0.19 (1)	6.25	J-G	-211 / 0	0.10 (1)	
H-I	0 / 10	-77.4	-77.4	0.01 (1)	10.00				
B-N	0 / 89	-28.0	-28.0	0.20 (2)	10.00				
N-M	0 / 85	-28.0	-28.0	0.26 (2)	10.00				
M-L	0 / 49	-28.0	-28.0	0.26 (2)	10.00				
L-K	0 / 49	-28.0	-28.0	0.26 (2)	10.00				
K-J	0 / 49	-28.0	-28.0	0.26 (2)	10.00				
J-H	0 / 54	-28.0	-28.0	0.20 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL =	3.0	PSF
BOT CH.	LL = 10.5	PSF
DL =	7.0	PSF
TOTAL LOAD =	43.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48 (D-F:1), BC=0.26 (M-N:2), WB=0.26 (F-K:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667
	622	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (K) (INPUT = 0.80)
 JSI METAL= 0.14 (E) (INPUT = 1.00)



DWG NO. YAM2031/-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281486	J1S	7	1	TRUSS DESC.		

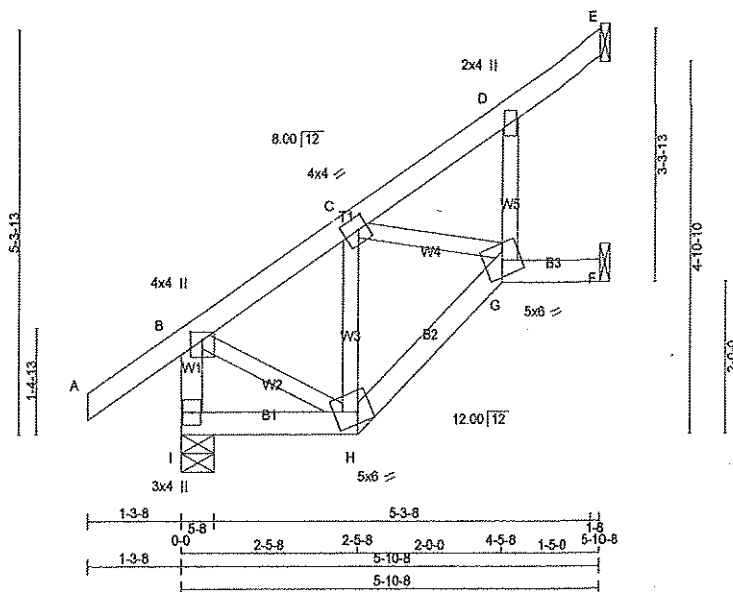
Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Mon May 01 19:09:07 2017 Page 1

ID:dtlxxC0gth4fN9E2Y7KTZMTJZ-QGcn5DF2MvldTe?wDoCH2DgckdFEF9FkrfBgPwZKo7g

-1-3-8 1-3-8 0-0 2-5-8 2-5-8 2-0-0 4-5-8 1-5-0 5-10-8

Scale = 1:29.8



TOTAL WEIGHT = 7 X 26 = 182 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF		
A - E	2x4	DRY	No.2	SPF		
I - H	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
G	BBWW-m	MT20	5.0	6.0		
H	BBWW-m	MT20	5.0	6.0	Edge	2.25
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	UPLIFT
I	416	0	416	0
E	290	0	290	0
F	20	0	35	0

SEE MITEK STANDARD DETAIL B37578H FOR CONNECTION TO JOINT(S) E, F

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX MIN. SNOW	MAX MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	329	201/0	62/0	0/0	0/0	67/0	0/0
E	232	137/0	47/0	0/0	0/0	49/0	0/0
F	25	0/0	15/0	0/0	0/0	10/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
I-B	-381/0	0.0 0.0 0.04 (1)	H-C	-234/0
A-B	0/29	-77.4 -77.4 0.10 (1)	G-D	0/312
B-C	-230/0	-77.4 -77.4 0.16 (1)	B-H	0/233
C-D	-121/0	-77.4 -77.4 0.45 (1)	C-G	-210/0
D-E	0/131	-77.4 -77.4 0.47 (1)		
I-H	0/0	-28.0 -28.0 0.05 (2)		
H-G	0/290	-28.0 -28.0 0.07 (2)		
G-F	0/0	-28.0 -28.0 0.02 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 43.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/680 (0.10")

CSI: TC=0.47 (D-E:1), BC=0.07 (G-H:2), WB=0.07 (D-G:1), SSI=0.19 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 20180617
STRUCTURAL
COMPONENT ONLY

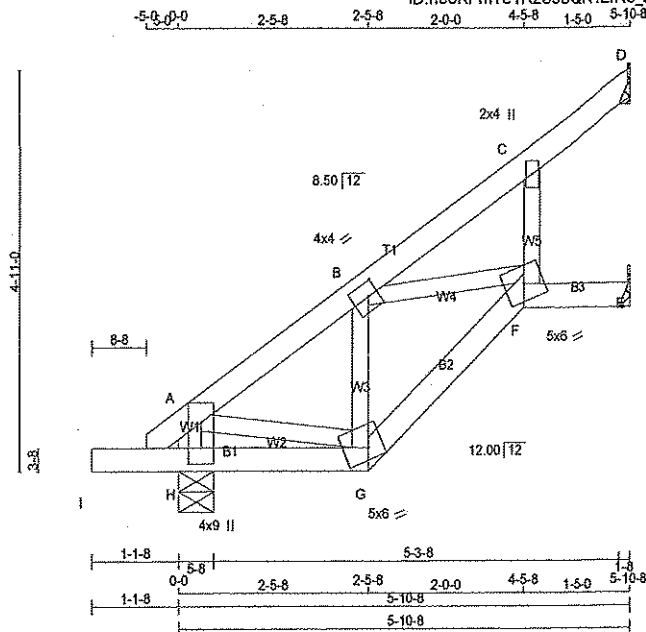
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44080	DRWG NO.
281488	J20S	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon May 01 19:10:07 2017 Page 1

ID:h3oKfMTe1R2593QR1ZIKe_zMBUw-fpqShkzhDLr6fbsSkPHLTs9tZpkwI6Ze?jRw1zKo_k

Scale = 1:27.6



TOTAL WEIGHT = 7 X 24 = 171 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A							
B	TMWW-1	MT20	4.0	4.0	2.00	1.50	
C	TMWW-w	MT20	2.0	4.0			
F	BBWW-m	MT20	5.0	6.0			
G	BBWW-m	MT20	5.0	6.0	Edge	2.25	
H							
H	TBMWW1*+p	MT20	4.0	9.0	0.25	2.00	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	428	0	428	0
D	290	0	290	0
E	20	0	35	0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	356	189 / 0	85 / 0	0 / 0	0 / 0	81 / 0	0 / 0
D	232	137 / 0	47 / 0	0 / 0	0 / 0	49 / 0	0 / 0
E	25	0 / 0	15 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)
FR-TO		FROM	TO					
H-A	-275 / 0	0.0	0.0	0.03 (1)	7.81	G-B	-238 / 0	0.04 (1)
A-B	-300 / 0	-77.4	-77.4	0.16 (1)	6.25	F-C	0 / 312	0.07 (1)
B-C	-126 / 0	-77.4	-77.4	0.45 (1)	6.25	B-F	-252 / 0	0.04 (1)
C-D	0 / 136	-77.4	-77.4	0.47 (1)	10.00	A-G	0 / 272	0.06 (1)
I-H	0 / 0	-105.4	-105.4	0.09 (1)	10.00			
H-G	0 / 0	-28.0	-28.0	0.05 (2)	10.00			
G-F	0 / 362	-28.0	-28.0	0.08 (1)	10.00			
F-E	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 43.8 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/663$ (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.47 (C-D:1), BC=0.09 (H-I:1), WB=0.07 (C-F:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.33 (C) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)



DWG NO. TAM 203/2-17
STRUCTURAL
COMPONENT ONLY

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

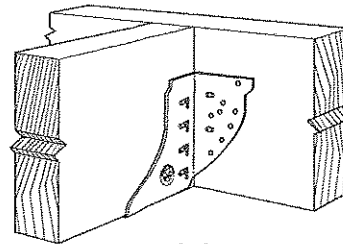
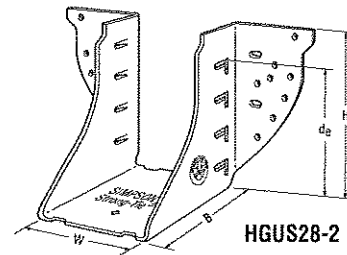
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

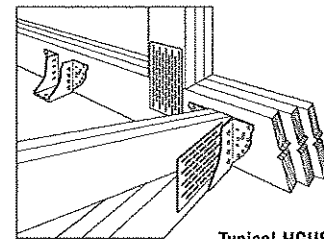
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



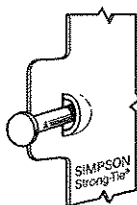
Typical HGUS
Installation



Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

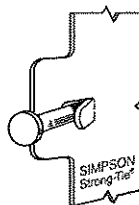
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _p ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 3/32	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 1/16	5 1/16	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 1/16	5 1/2	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 1/16	5 1/16	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 3/8	7 1/8	5	6 1/8	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 3/16	7 3/16	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 1/16	7 1/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 3/16	7 3/16	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 3/8	9 3/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGU210-3	12	4 1/16	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGU210-4	12	6 1/16	9 3/16	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGU212-4	12	6 1/16	10 3/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGU214-4	12	6 1/16	12 3/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645

1. d_p is the distance from the seat of the hanger to the highest joist nail.

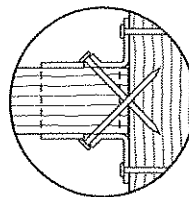


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

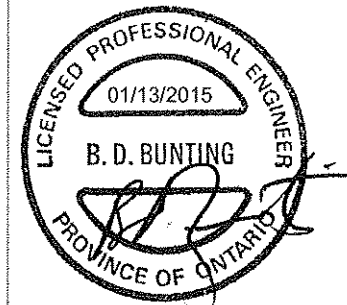
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

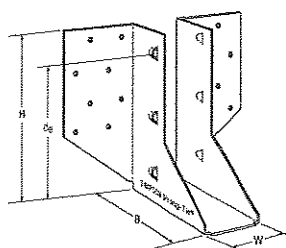
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

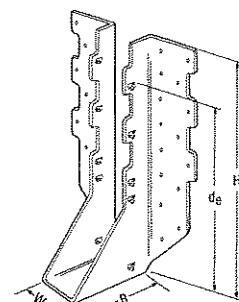
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

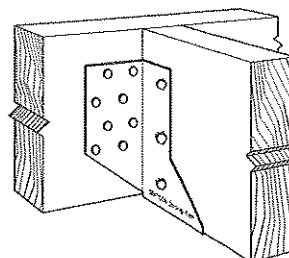
- See current catalogue for options



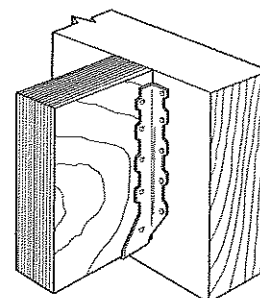
LJS26DS



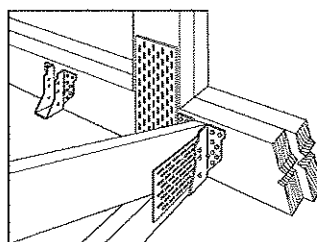
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



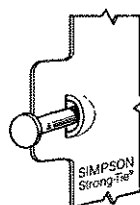
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

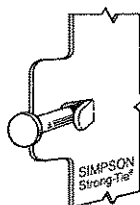
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

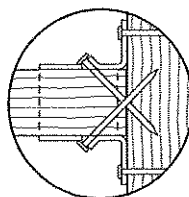


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

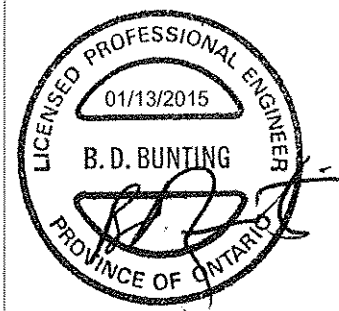
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

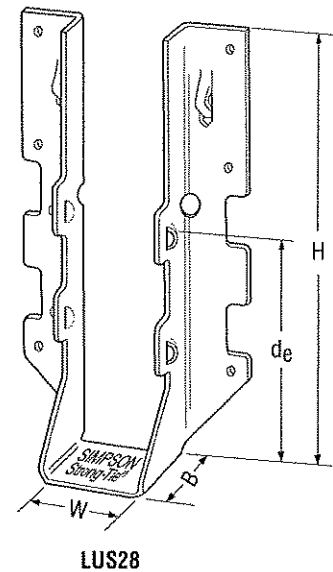
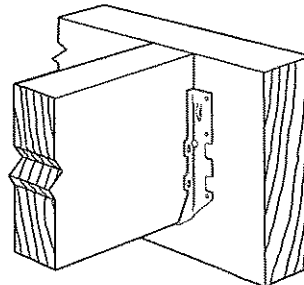
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

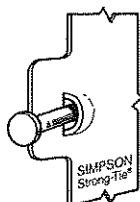
- These hangers cannot be modified.

Typical LUS Installation



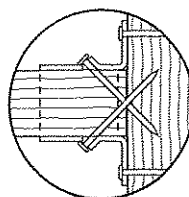
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1⅞	3⅜	1¾	1⅝	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅜	3⅜	2	1⅝	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅞	4⅜	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅜	4⅜	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅜	4⅝	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅜	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅜	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅝	1¾	3⅝	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅜	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅜	8⅝	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

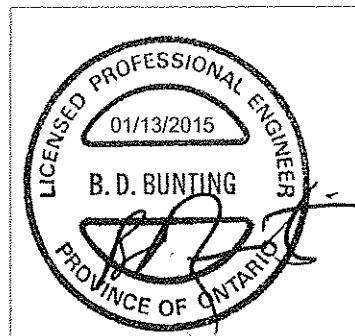


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECUS15 1/15 exp. 12/16

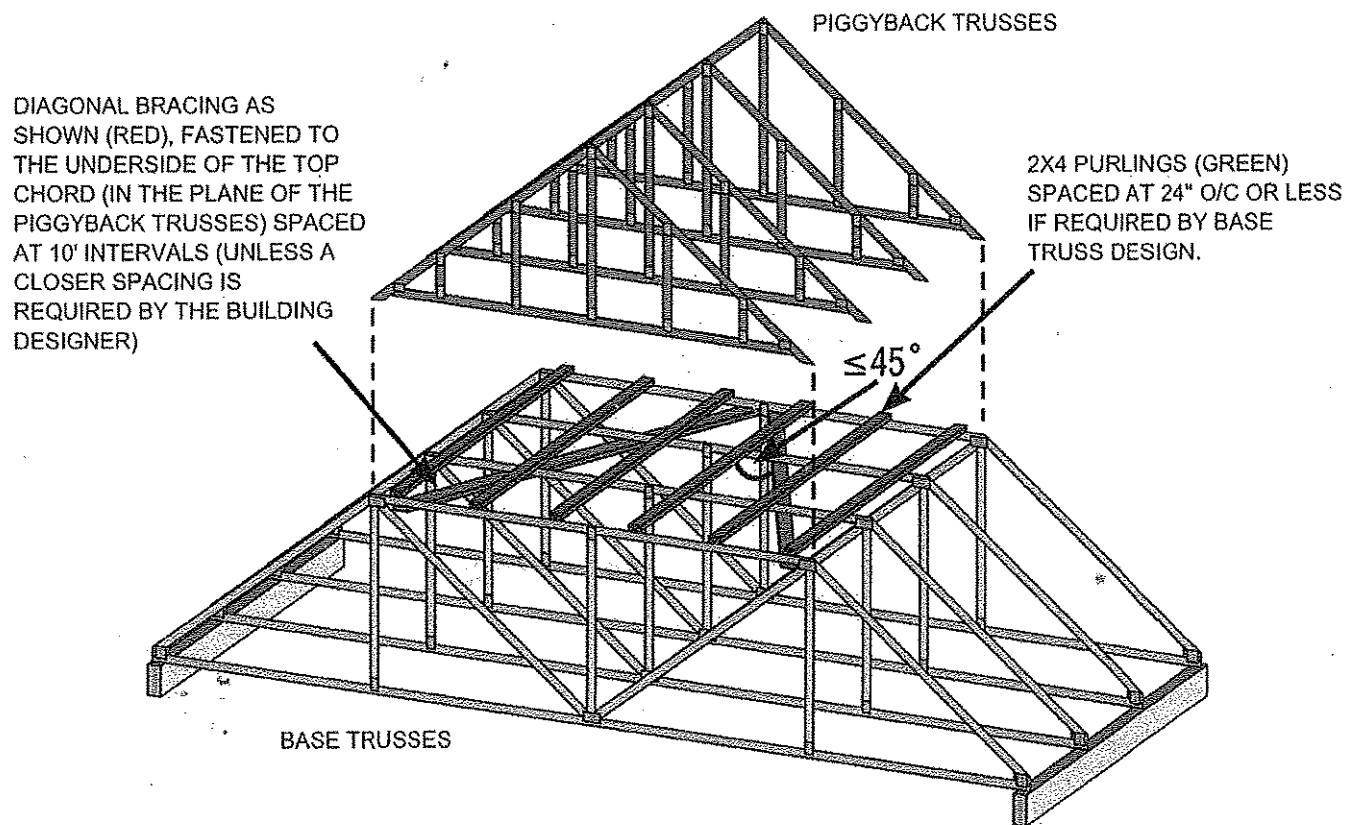
800-999-5099
www.strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

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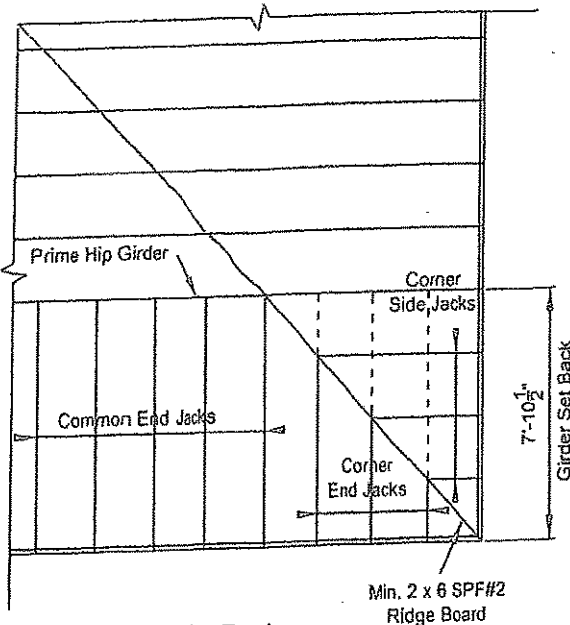
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LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

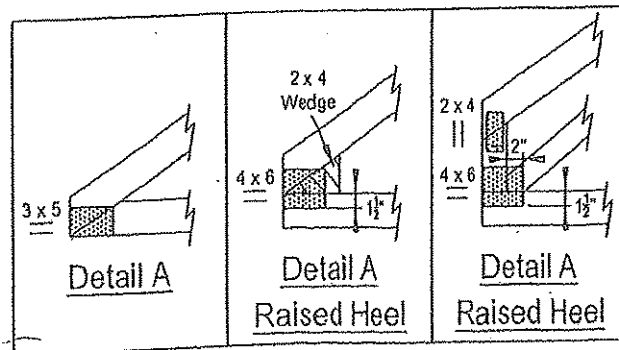
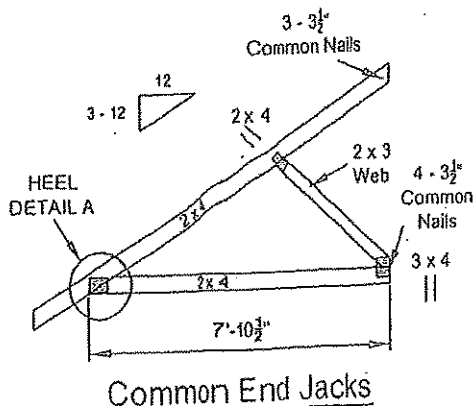
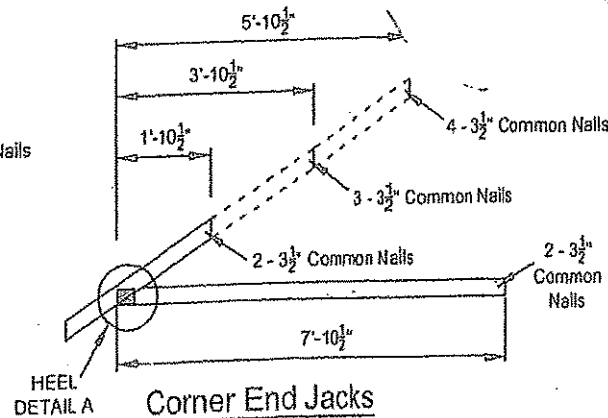
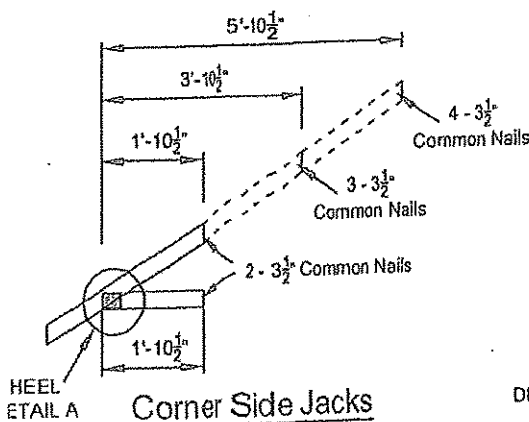
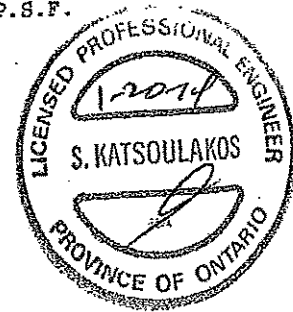
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 44.8 P.S.F.



45° Hip End

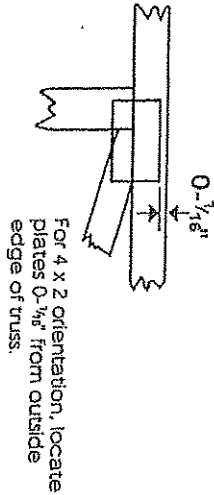
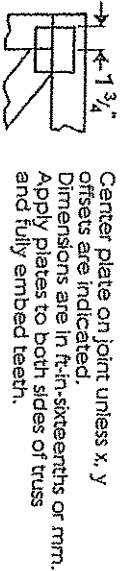
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NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

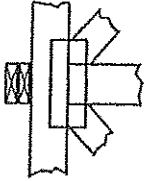
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



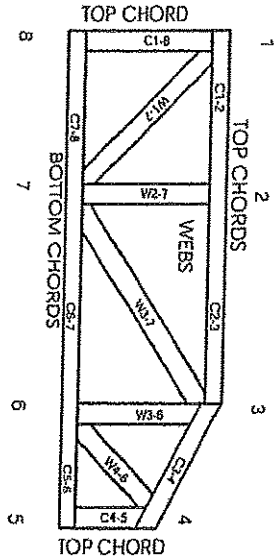
BEARING



Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information.
Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in feet-inches-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

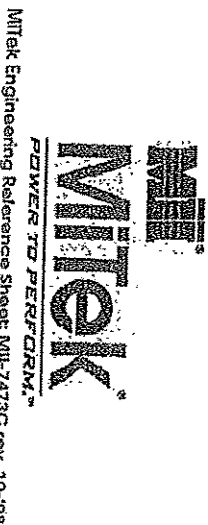
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

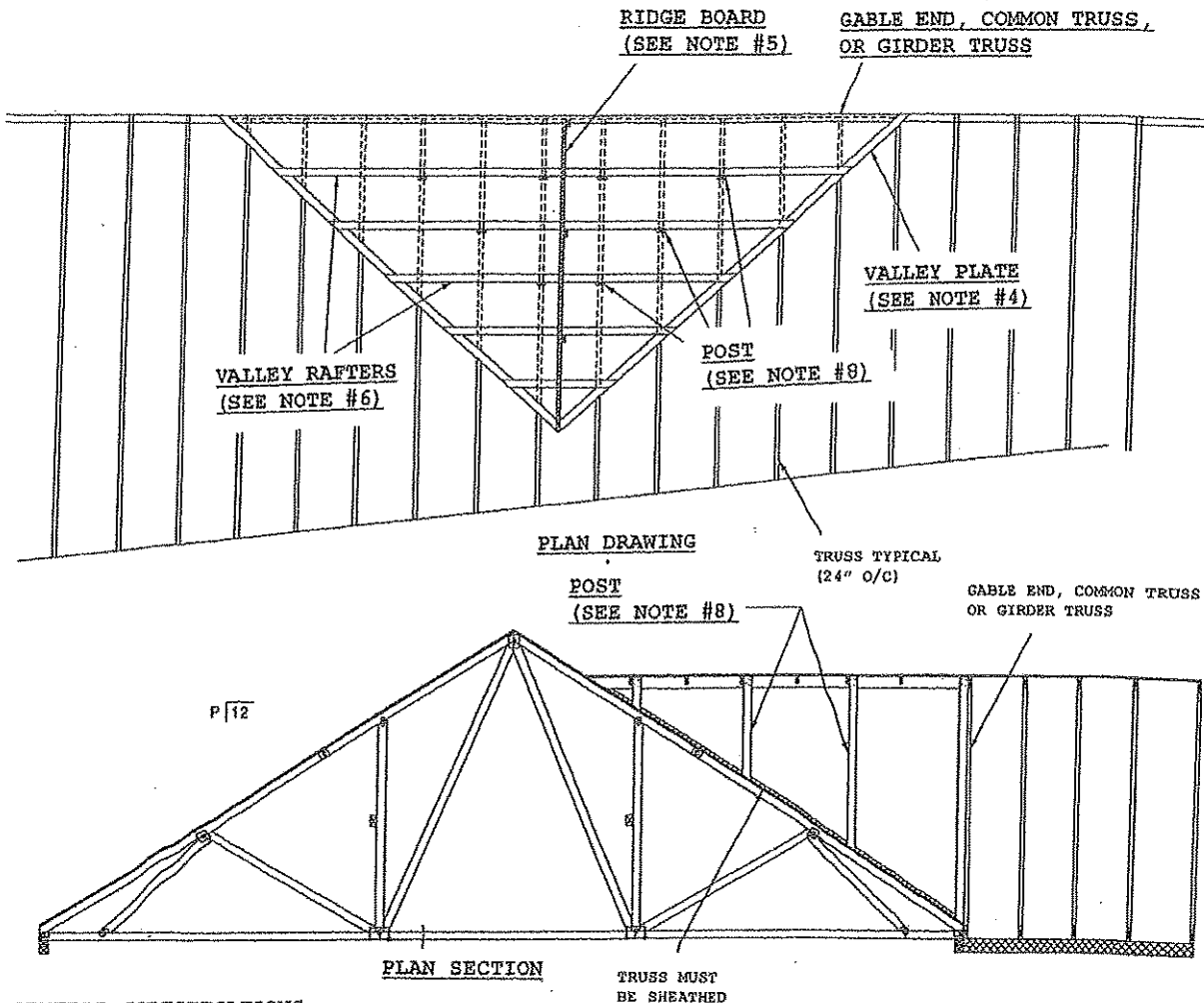
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

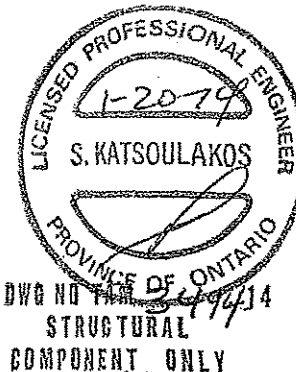


DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.