

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170371

45-07-00

40-06-00

HIGH CLG.

1-02-00
3-04-00

2-04-00

9-00-00

5/12

7-10-00

7-10-00

11-08-00

45-00-00

2-4-0 T.O.P SPLIT

** 2-4-0 RAISED T.O.P & CLG.

12" FINISH O.H

R.T.M.C

2X6 EXTERIOR WALLS

ASPHALT SHINGLES

2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF
FRAMING TO CONFORM
TO PART 9 OF THE OBC.
LATEST EDITION ROOF
RAFTERS THAT MEET OR
CROSS OVER TRUSSES
ARE TO BE 2"X4"SPF
@24"o.c. WITH A 2"X4"SPF
VERTICAL POST TO THE
TRUSS UNDER AT EACH
CROSS POINT. POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS
AND BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

DESIGN CONFORMS WITH
THE RELEVANT SECTION
OF THE LATEST EDITION
OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa

TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS24(O)
LUS24-2(OO)
LJS26DS(V)
HGUS26-2(XX)

DATE: 5-22-17

BCIN:26064; FIRM:BCIN #29991

ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED
ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING
DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING
DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO
NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A
QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE
BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR
PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS
PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE
ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
INDIVIDUAL LOT.

DWG #TAM THROUGH DWG #TAM, INCLUSIVE, DATED;

STRUCTURAL COMPONENTS ONLY); THIS LAYOUT MUST BE READ TOGETHER WITH
REFERENCED TRUSS COMPONENT DRAWINGS AND SEALED HARDWARE DRAWINGS.
HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING
REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS
SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3-1/4"
COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING"
ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (") USING THE SAME
NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB
MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-
1/4" COMMON WIRE NAILS AT 6" O.C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF
NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING
FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS
COMPONENTS AND AT LOCATIONS MARKED BY "X".
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O.C. WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X
4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY).

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE
TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH
STUDS AT 24" O.C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE
(CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG #TAM 25504-17

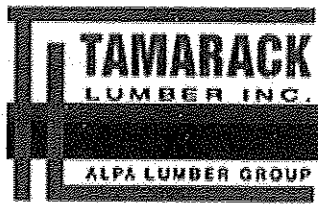
BCIN: 26064

FIRM: 29991

SEALED STRUCTURAL
COMPONENTS ONLY



Job Track: 44269		Builder: GREENYORK HOMES /		Location: BRAMPTON		Model: AUBURN 3 / 1	
Layout ID: 282293		Project: OSTIENSE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
Plan Log: 92456		Date: 5/18/2017		Designer: JG		MiTek ver 7.5.0	



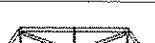
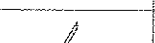
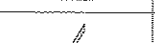
Delivery Shiplist

DATE	05/18/17
SALES REP	Rick

JOB TRACK:44269	LAYOUT ID: 282293	LOCATION: BRAMPTON
BUILDER: GREEN YORK HOMES	SUB-BUILDER:	
MODEL: AUBURN 3	ELEVATION: 1	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	10.00	37-10-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	380.18		
	2 Ply		0.00							230.66		
	1	T1Z HIP GIRDER	10.00	37-10-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	380.18		
	2 Ply		0.00							230.66		
	2	T2 HIP	10.00	37-10-00	05-10-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	358.20		
			0.00							221.34		
	2	T3 HIP	10.00	37-10-00	07-00-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	386.06		
			0.00							237.66		
	2	T4 HIP	10.00	37-10-00	08-02-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	401.74		
			0.00							246.66		
	5	T5 HIP	10.00	37-10-00	09-04-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	1079.95		
			0.00							661.65		
	2	T6 HIP	10.00	37-10-00	10-06-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	451.98		
			0.00							277.34		
	1	T7 HIP GIRDER	12.00	10-10-00	05-04-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	54.37		
			0.00							35.33		
	1	T8 HIP	12.00	10-10-00	09-05-08	2 X 4	2 X 4	01-03-08 01-03-08	04-02-08 04-02-08	60.56		
			0.00							37.50		
	1	T9 HIP GIRDER	10.00	18-08-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	79.94		
			0.00							50.50		
	1	T10 HIP	10.00	18-08-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	83.82		
			0.00							52.33		
	3	PB1 PIGGYBACK	10.00	17-11-15	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	177.93		
			0.00							111.99		
	23	J1 JACK-OPEN	7.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	397.90		
			0.00							245.41		
	2	J1Z JACK-OPEN	7.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	69.20		
	2 Ply		0.00							42.68		
	3	J3 JACK-OPEN	10.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	50.49		
			0.00							32.01		
	3	J4 JACK-OPEN	12.00	03-05-08	05-04-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-04-00	49.35		
			0.00							33.99		
	2	J5 JACK-OPEN	12.00	03-05-08	03-07-15	2 X 4	2 X 4	01-03-08 -01-08-01	01-10-08 00-03-08	27.50		
			0.00							18.66		
	2	J6 JACK-OPEN	12.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	21.72		
			0.00							15.00		



Delivery Shiplist

DATE	05/18/17
SALES REP	Rick

JOB TRACK: 44269	LAYOUT ID: 282293	LOCATION: BRAMPTON
BUILDER: GREEN YORK HOMES	SUB-BUILDER:	
MODEL: AUBURN 3	ELEVATION: 1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	J7 JACK-OPEN	5.00 0.00	05-06-08	02-07-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-07-12	89.40 55.98		
	4	J8 JACK-OPEN	5.00 0.00	04-02-08	02-01-01	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-01-01	46.92 32.00		

TOTAL # TRUSS= 71.00

TOTAL BFT OF ALL TRUSSES=

2869.35 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4647.39 LBS.

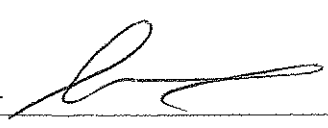
HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
1	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= 5.00

Schedule 1: Designer Information

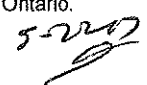
Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information		Application number:	
Building number, street name		Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description	
B. Individual who reviews and takes responsibility for design activities			
Name SAM KATSOUKAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.	
Street address R.R #1, PO BOX 61		Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail
Telephone number (519) 287-2242 Business	Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]			
☐ House ☐ HVAC – House ☒ Building Structural ☐ Small Buildings ☐ Building Services ☐ Plumbing – House ☐ Large Buildings ☐ Detection, Lighting and Power ☐ Plumbing – All Buildings ☐ Complex Buildings ☐ Fire Protection ☐ On-site Sewage Systems			
Description of designer's work <u>GREEN YORK HOMES – OSTIENSE – MODEL: AUBURN 3 – ELEV. 1</u> (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM25504-17 DATED 5-22-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.			
D. Declaration of Designer			
I, <u>SAM KATSOUKAKOS, P. ENG</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____			
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.			
Date		Signature of Designer 	

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d. of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM25504-17-S
DWG#TAM2550517-S

5-22-17


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44269	DRWG NO.
282293	T1	1	2	TRUSS DESC.		

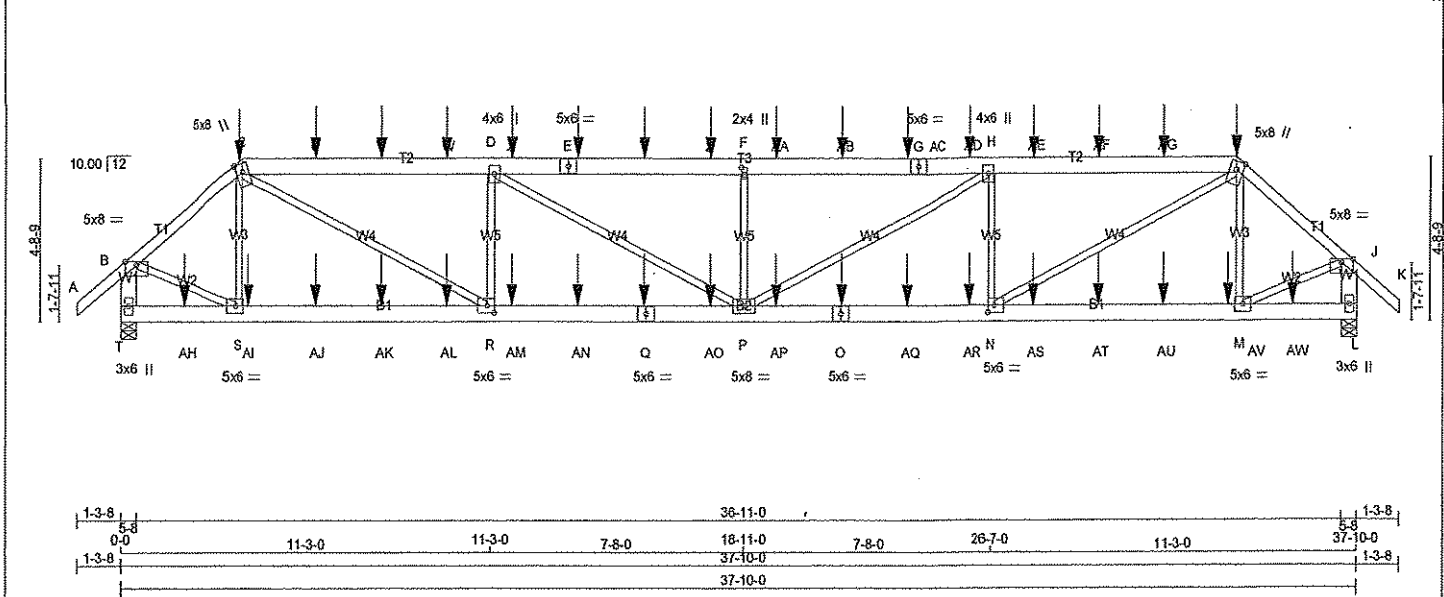
Tamarack Roof Truss, Burlington

Version 6.030 S Oct 5 2016 MiTek Industries, Inc. Thu May 18 16:22:21 2017 Page 1

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37-10-0-1-8

Scale = 1:65.6



TOTAL WEIGHT = 2 X 190 = 380 LB

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
I - K	12	SIDE(61.0)
C - E	12	SIDE(61.0)
E - G	12	SIDE(61.0)
G - I	12	SIDE(61.0)
T - B	12	TOP
L - J	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T - Q	12	SIDE(163.1)
Q - O	12	SIDE(163.1)
O - L	12	SIDE(163.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TTWW+m	MT20	5.0	8.0	Edge
D	TMVW+H	MT20	4.0	6.0	
E	TS-I	MT20	5.0	6.0	
F	TMVW+w	MT20	2.0	4.0	2.50 1.00
G	TS-I	MT20	5.0	6.0	
H	TMVW+H	MT20	4.0	6.0	
I	TTWW+m	MT20	5.0	8.0	Edge
J	TMVW-p	MT20	5.0	8.0	Edge
L	BMVW+p	MT20	3.0	6.0	
M	BMVW+H	MT20	5.0	6.0	
N	BMVW+I	MT20	5.0	6.0	2.50 2.50
O	BS-I	MT20	5.0	6.0	
P	BMVW+H	MT20	5.0	8.0	
Q	BS-I	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
T	3608	0	3608	0	0	5-8	5-8		
L	3605	0	3605	0	0	5-8	5-8		
UNFACTORED REACTIONS									
1ST LCASE		MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
T	2987	1618 / 0	708 / 0	0 / 0	0 / 0	663 / 0	0 / 0		
L	2984	1616 / 0	705 / 0	0 / 0	0 / 0	662 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FROM TO	FR-TO	FROM TO	FR-TO
A-B	0 / 37	-84.3 -84.3 0.06 (1)	10.00	S-C	-823 / 25 0.10 (1)
B-C	-3701 / 0	-84.3 -84.3 0.17 (1)	4.76	M-I	-822 / 24 0.10 (1)
C-U	-6748 / 0	-84.3 -84.3 0.39 (1)	4.27	B-S	0 / 3012 0.37 (1)
U-V	-6748 / 0	-84.3 -84.3 0.39 (1)	4.27	M-J	0 / 3009 0.37 (1)
V-W	-6748 / 0	-84.3 -84.3 0.39 (1)	4.27	N-I	0 / 4457 0.55 (1)
W-D	-6748 / 0	-84.3 -84.3 0.39 (1)	4.27	C-R	0 / 4458 0.55 (1)
D-X	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96	N-H	-1839 / 0 0.27 (1)
X-E	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96	R-D	-1837 / 0 0.27 (1)
E-Y	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96	P-H	0 / 1382 0.17 (1)
Y-Z	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96	D-P	0 / 1377 0.17 (1)
Z-F	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96	P-F	-971 / 0 0.14 (1)
F-AA	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96		
AA-AB	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96		
AB-AC	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96		
AC-G	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96		
G-AD	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96		
AD-H	-7952 / 0	-84.3 -84.3 0.43 (1)	3.96		
H-AE	-6744 / 0	-84.3 -84.3 0.39 (1)	4.27		
AE-AF	-6744 / 0	-84.3 -84.3 0.39 (1)	4.27		
AF-AG	-6744 / 0	-84.3 -84.3 0.39 (1)	4.27		
AG-I	-6744 / 0	-84.3 -84.3 0.39 (1)	4.27		
I-J	-3698 / 0	-84.3 -84.3 0.17 (1)	4.76		
J-K	0 / 37	-84.3 -84.3 0.06 (1)	10.00		
T-B	-3605 / 0	0.0 0.0 0.13 (1)	7.39		
L-J	-3601 / 0	0.0 0.0 0.13 (1)	7.40		
T-AH	0 / 0	-28.0 -28.0 0.13 (2)	10.00		
AH-S	0 / 0	-28.0 -28.0 0.13 (2)	10.00		
S-AI	0 / 2866	-28.0 -28.0 0.27 (2)	10.00		
AI-AJ	0 / 2866	-28.0 -28.0 0.27 (2)	10.00		
AJ-AK	0 / 2866	-28.0 -28.0 0.27 (2)	10.00		
AK-AL	0 / 2866	-28.0 -28.0 0.27 (2)	10.00		
AL-R	0 / 2866	-28.0 -28.0 0.27 (2)	10.00		
R-AM	0 / 6748	-28.0 -28.0 0.52 (1)	10.00		
AM-AN	0 / 6748	-28.0 -28.0 0.52 (1)	10.00		
AN-O	0 / 6748	-28.0 -28.0 0.52 (1)	10.00		
Q-AO	0 / 6748	-28.0 -28.0 0.52 (1)	10.00		
AO-P	0 / 6748	-28.0 -28.0 0.52 (1)	10.00		
P-AP	0 / 6743	-28.0 -28.0 0.52 (1)	10.00		
AP-O	0 / 6743	-28.0 -28.0 0.52 (1)	10.00		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.26")
CALCULATED VERT. DEFL.(LL)= L/999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.26")
CALCULATED VERT. DEFL.(TL)= L/999 (0.40")

CSI: TC=0.43 (F-H:1), BC=0.52 (P-R:1), WB=0.55 (C-R:1), SSI=0.19 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(PRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)

JSI METAL= 0.59 (O) (INPUT = 1.00)

DRWG NO. TAM 25492-17

STRUCTURAL

COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 282293	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. 44289 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
R BMWW-I	MT20	5.0	6.0	2.50	2.50
S BMWW-I	MT20	5.0	6.0		
T BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS/NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 291.9 lbs FACTORED DOWN AT 3-8-4, 291.8 lbs FACTORED DOWN AT 34-1-12, 101.4 lbs FACTORED DOWN AT 5-10-12, 101.4 lbs FACTORED DOWN AT 7-10-12, 101.4 lbs FACTORED DOWN AT 9-10-12, 101.4 lbs FACTORED DOWN AT 11-10-12, 101.4 lbs FACTORED DOWN AT 13-10-12, 101.4 lbs FACTORED DOWN AT 15-10-12, 101.4 lbs FACTORED DOWN AT 17-10-12, 101.4 lbs FACTORED DOWN AT 19-10-12, 101.4 lbs FACTORED DOWN AT 21-10-12, 101.4 lbs FACTORED DOWN AT 23-10-12, 101.4 lbs FACTORED DOWN AT 25-10-12, 101.4 lbs FACTORED DOWN AT 27-10-12, AND 101.4 lbs FACTORED DOWN AT 29-10-12, AND 101.4 lbs FACTORED DOWN AT 31-10-12 ON TOP CHORD, AND 70.9 lbs FACTORED DOWN AT 1-10-12, 70.9 lbs FACTORED DOWN AT 3-10-12, 70.9 lbs FACTORED DOWN AT 5-10-12, 70.9 lbs FACTORED DOWN AT 7-10-12, 70.9 lbs FACTORED DOWN AT 9-10-12, 70.9 lbs FACTORED DOWN AT 11-10-12, 70.9 lbs FACTORED DOWN AT 13-10-12, 70.9 lbs FACTORED DOWN AT 15-10-12, 70.9 lbs FACTORED DOWN AT 17-10-12, 70.9 lbs FACTORED DOWN AT 19-10-12, 70.9 lbs FACTORED DOWN AT 21-10-12, 70.9 lbs FACTORED DOWN AT 23-10-12, 70.9 lbs FACTORED DOWN AT 25-10-12, 70.9 lbs FACTORED DOWN AT 27-10-12, 70.9 lbs FACTORED DOWN AT 29-10-12, 70.9 lbs FACTORED DOWN AT 31-10-12, AND 70.9 lbs FACTORED DOWN AT 33-10-12, AND 70.9 lbs FACTORED DOWN AT 35-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
O-AQ	0 / 6743	-28.0	-28.0	0.52 (1)	10.00				
AQ-AR	0 / 6743	-28.0	-28.0	0.52 (1)	10.00				
AR-N	0 / 6743	-28.0	-28.0	0.52 (1)	10.00				
N-AS	0 / 2863	-28.0	-28.0	0.27 (2)	10.00				
AS-AT	0 / 2863	-28.0	-28.0	0.27 (2)	10.00				
AT-AU	0 / 2863	-28.0	-28.0	0.27 (2)	10.00				
AU-AV	0 / 2863	-28.0	-28.0	0.27 (2)	10.00				
AV-M	0 / 2863	-28.0	-28.0	0.27 (2)	10.00				
M-AW	0 / 0	-28.0	-28.0	0.13 (2)	10.00				
AW-L	0 / 0	-28.0	-28.0	0.13 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-8-4	-292	-292	--	FRONT	VERT	TOTAL
E	13-10-12	-101	-101	--	FRONT	VERT	TOTAL
I	34-1-12	-292	-292	--	FRONT	VERT	TOTAL
O	21-10-12	-41	-71	--	FRONT	VERT	TOTAL
Q	15-10-12	-41	-71	--	FRONT	VERT	TOTAL
U	5-10-12	-101	-101	--	FRONT	VERT	TOTAL
V	7-10-12	-101	-101	--	FRONT	VERT	TOTAL
W	9-10-12	-101	-101	--	FRONT	VERT	TOTAL
X	11-10-12	-101	-101	--	FRONT	VERT	TOTAL
Y	15-10-12	-101	-101	--	FRONT	VERT	TOTAL
Z	17-10-12	-101	-101	--	FRONT	VERT	TOTAL
AA	19-10-12	-101	-101	--	FRONT	VERT	TOTAL
AB	21-10-12	-101	-101	--	FRONT	VERT	TOTAL
AC	23-10-12	-101	-101	--	FRONT	VERT	TOTAL
AD	25-10-12	-101	-101	--	FRONT	VERT	TOTAL
AE	27-10-12	-101	-101	--	FRONT	VERT	TOTAL
AF	29-10-12	-101	-101	--	FRONT	VERT	TOTAL
AG	31-10-12	-101	-101	--	FRONT	VERT	TOTAL
AH	1-10-12	-41	-71	--	FRONT	VERT	TOTAL
AI	3-10-12	-41	-71	--	FRONT	VERT	TOTAL
AJ	5-10-12	-41	-71	--	FRONT	VERT	TOTAL
AK	7-10-12	-41	-71	--	FRONT	VERT	TOTAL
AL	9-10-12	-41	-71	--	FRONT	VERT	TOTAL
AM	11-10-12	-41	-71	--	FRONT	VERT	TOTAL
AN	13-10-12	-41	-71	--	FRONT	VERT	TOTAL
AO	17-10-12	-41	-71	--	FRONT	VERT	TOTAL
AP	19-10-12	-41	-71	--	FRONT	VERT	TOTAL
AQ	23-10-12	-41	-71	--	FRONT	VERT	TOTAL
AR	25-10-12	-41	-71	--	FRONT	VERT	TOTAL
AS	27-10-12	-41	-71	--	FRONT	VERT	TOTAL
AT	29-10-12	-41	-71	--	FRONT	VERT	TOTAL
AU	31-10-12	-41	-71	--	FRONT	VERT	TOTAL
AV	33-10-12	-41	-71	--	FRONT	VERT	TOTAL
AW	35-10-12	-41	-71	--	FRONT	VERT	TOTAL



Per
DWG NO. TAM25492-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44289	DRWG NO.
282293	T1Z	1	2	TRUSS DESC.		

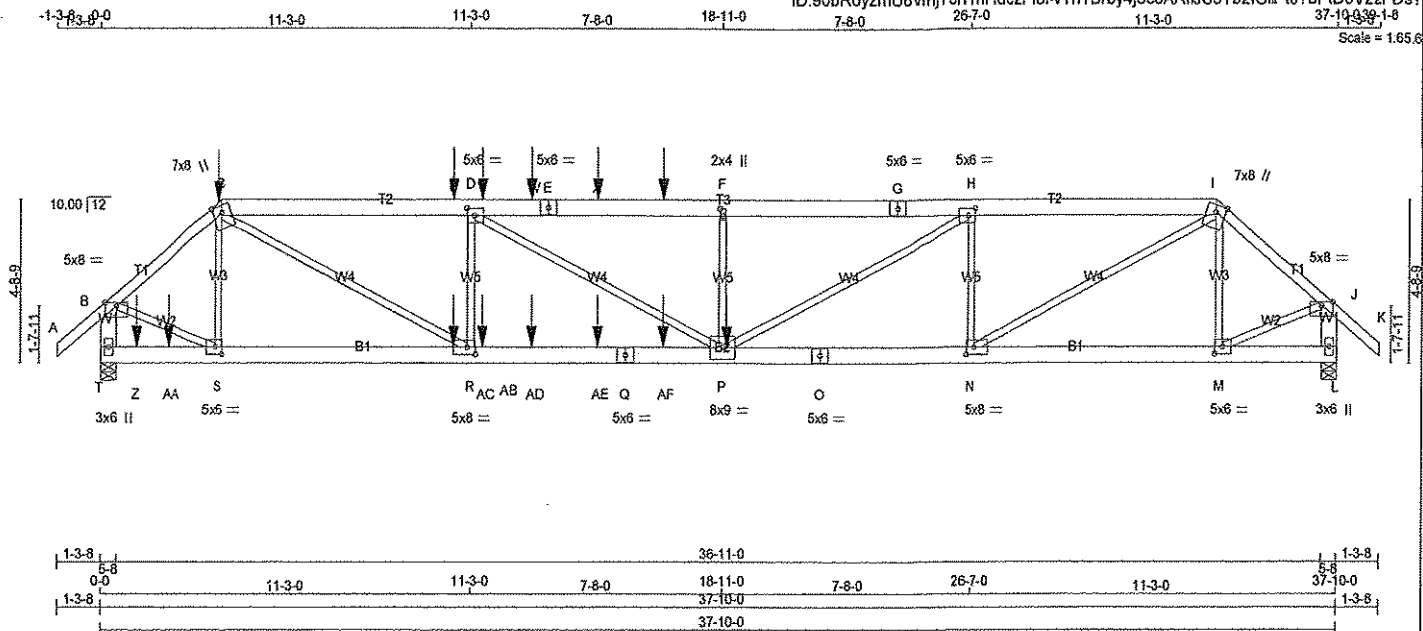
Tamarack Roof Truss, Burlington

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37-1939-1-8

Scale = 1:65.6



TOTAL WEIGHT = 2 X 190 = 380 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - G	2x6	DRY No.2	SPF
G - I	2x6	DRY No.2	SPF
I - K	2x4	DRY No.2	SPF
T - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
T - Q	2x6	DRY No.2	SPF
Q - O	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
I - K 1	12	TOP
C - E 2	12	SIDE(61.0)
E - G 2	12	SIDE(0.0)
G - I 2	12	TOP
T - B 2	12	TOP
L - J 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T - Q 2	12	SIDE(183.1)
Q - O 2	12	SIDE(183.1)
O - L 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
P - F 1	6	SIDE(352.2)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	Edge
C TTWW+m	MT20	7.0	8.0	Edge
D TMWW-I	MT20	5.0	6.0	2.50 2.75
E TS-I	MT20	5.0	6.0	
F TMVW+w	MT20	2.0	4.0	2.50 1.00
G TS-I	MT20	5.0	6.0	
H TMWW-I	MT20	5.0	6.0	2.50 2.75
I TTWW+m	MT20	7.0	8.0	Edge
J TMVW-p	MT20	5.0	8.0	Edge
L BMV1+p	MT20	3.0	6.0	
M BMWW-I	MT20	5.0	6.0	2.50 2.75
N BMWW-I	MT20	5.0	8.0	2.50 3.00
O BS-I	MT20	5.0	6.0	
P BMWWW-I	MT20	8.0	9.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
T	4274	0	4274	0	5-8
L	3662	0	3662	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM LIVE WIND DEAD SOIL
T	3508	1949 / 0	799 / 0 0 / 0 0 / 0 761 / 0 0 / 0
L	2970	1708 / 0	636 / 0 0 / 0 0 / 0 625 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		CHORDS	WEBS
TOTAL LOAD CASES: (4)		MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. UNBRAC. LENGTH FR-TO
FR-TO		FROM TO	
A-B	0 / 37	-84.3 -84.3 0.06 (1)	10.00
B-C	-4397 / 0	-84.3 -84.3 0.20 (1)	4.41
C-U	-8912 / 0	-84.3 -84.3 0.45 (1)	3.75
U-D	-8912 / 0	-84.3 -84.3 0.45 (1)	3.75
D-V	-11103 / 0	-84.3 -84.3 0.56 (1)	3.32
V-W	-11103 / 0	-84.3 -84.3 0.56 (1)	3.32
W-E	-11103 / 0	-84.3 -84.3 0.56 (1)	3.32
E-X	-11103 / 0	-84.3 -84.3 0.56 (1)	3.32
X-Y	-11103 / 0	-84.3 -84.3 0.56 (1)	3.32
Y-F	-11103 / 0	-84.3 -84.3 0.56 (1)	3.32
F-G	-11103 / 0	-84.3 -84.3 0.44 (1)	3.45
G-H	-11103 / 0	-84.3 -84.3 0.44 (1)	3.45
H-I	-7809 / 0	-84.3 -84.3 0.30 (1)	4.13
I-J	-3766 / 0	-84.3 -84.3 0.17 (1)	4.72
J-K	0 / 37	-84.3 -84.3 0.06 (1)	10.00
T-B	-4231 / 0	0.0 0.0 0.16 (1)	6.95
L-J	-3663 / 0	0.0 0.0 0.14 (1)	7.35

T-Z	0 / 0	-28.0 -28.0 0.09 (2)	10.00
Z-AA	0 / 0	-28.0 -28.0 0.09 (2)	10.00
AA-S	0 / 0	-28.0 -28.0 0.09 (2)	10.00
S-AB	0 / 3409	-28.0 -28.0 0.29 (1)	10.00
AB-R	0 / 3409	-28.0 -28.0 0.29 (1)	10.00
R-AC	0 / 8911	-28.0 -28.0 0.70 (1)	10.00
AC-AD	0 / 8911	-28.0 -28.0 0.70 (1)	10.00
AD-AE	0 / 8911	-28.0 -28.0 0.70 (1)	10.00
AE-Q	0 / 8911	-28.0 -28.0 0.70 (1)	10.00
Q-AF	0 / 8911	-28.0 -28.0 0.70 (1)	10.00
P-O	0 / 7809	-28.0 -28.0 0.58 (1)	10.00
O-N	0 / 7809	-28.0 -28.0 0.58 (1)	10.00
N-M	0 / 2919	-28.0 -28.0 0.25 (1)	10.00
M-L	0 / 0	-28.0 -28.0 0.09 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-8.4	-188	-188	—	FRONT	VERT	TOTAL
P	19-0.8	-2052	-2052	—	BACK	VERT	TOTAL
U	10-8-12	-527	-527	—	BACK	VERT	TOTAL
V	11-7.4	-101	-101	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.26")
CALCULATED VERT. DEFL. (LL) = L/999 (0.35")
ALLOWABLE DEFL. (TL) = L/360 (1.26")
CALCULATED VERT. DEFL. (TL) = L/815 (0.56")
CSI: TC=0.58 (D-F:1), BC=0.70 (P-R:1), WB=0.78 (C-R:1), SSI=0.30 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING SECTION.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.79 (Q) (INPUT = 1.00)

DWG NO. TAM25493-17

STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44269	DRWG NO.
282293	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	8.0	2.50	3.00
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

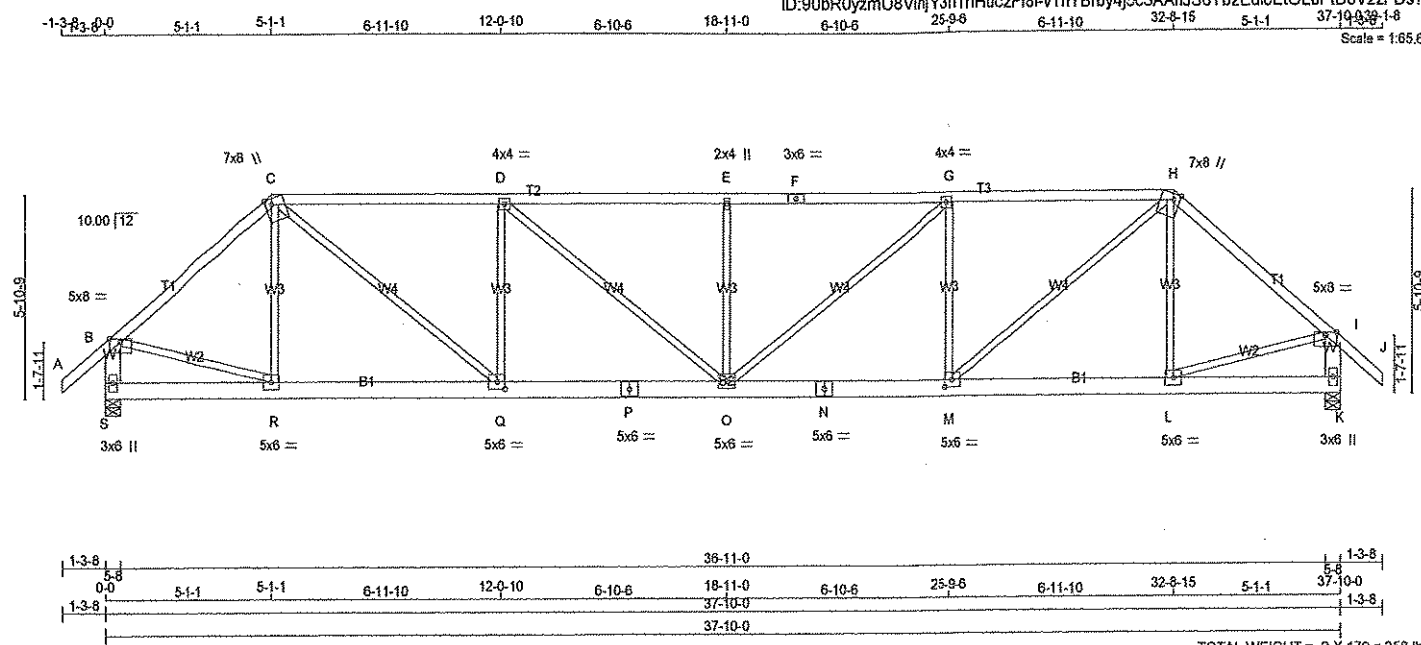
- SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 187.6 lbs FACTORED DOWN AT 3-8-4,
526.6 lbs FACTORED DOWN AT 10-8-12, 101.4
lbs FACTORED DOWN AT 11-7-4, 101.4 lbs
FACTORED DOWN AT 13-1-4, AND 101.4 lbs
FACTORED DOWN AT 15-1-4, AND 101.4 lbs
FACTORED DOWN AT 17-1-4 ON TOP CHORD,
AND 70.9 lbs FACTORED DOWN AT 1-0-12, 70.9
lbs FACTORED DOWN AT 2-0-8, 70.9 lbs
FACTORED DOWN AT 10-8-12, 70.9 lbs
FACTORED DOWN AT 11-7-4, 70.9 lbs
FACTORED DOWN AT 13-1-4, 70.9 lbs
FACTORED DOWN AT 15-1-4, AND 70.9 lbs
FACTORED DOWN AT 17-1-4, AND 2051.9 lbs
FACTORED DOWN AT 19-0-8 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	13-1-4	-101	-101	---	BACK	VERT	TOTAL
X	15-1-4	-101	-101	---	BACK	VERT	TOTAL
Y	17-1-4	-101	-101	---	BACK	VERT	TOTAL
Z	1-0-12	-41	-71	---	BACK	VERT	TOTAL
AA	2-0-8	-41	-71	---	BACK	VERT	TOTAL
AB	10-8-12	-41	-71	---	BACK	VERT	TOTAL
AC	11-7-4	-41	-71	---	BACK	VERT	TOTAL
AD	13-1-4	-41	-71	---	BACK	VERT	TOTAL
AE	15-1-4	-41	-71	---	BACK	VERT	TOTAL
AF	17-1-4	-41	-71	---	BACK	VERT	TOTAL



DWG NO. TAM25493-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 179 = 358 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	1650F 1.5E
C - F	2x4	DRY	1650F 1.5E
F - H	2x4	DRY	1650F 1.5E
H - J	2x4	DRY	1650F 1.5E
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 Edge
C	TTWW+m	MT20	7.0	8.0 Edge 2.25
D	TMWW-t	MT20	4.0	4.0
E	TMW+w	MT20	2.0	4.0
F	TS-t	MT20	3.0	6.0
G	TMWW-t	MT20	4.0	4.0
H	TTWW+m	MT20	7.0	8.0 Edge 2.25
I	TMVW-p	MT20	5.0	8.0 Edge
K	BMV1+p	MT20	3.0	6.0
L	BMWW-t	MT20	5.0	6.0
M	BMWW-t	MT20	5.0	6.0 2.50 2.75
N	BS-t	MT20	5.0	6.0
O	BMWWWW-t	MT20	5.0	6.0
P	BS-t	MT20	5.0	6.0
Q	BMWW-t	MT20	5.0	6.0 2.50 2.75
R	BMWW-t	MT20	5.0	6.0
S	BMV1+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
S	2240	0	2240	0	5-8
K	2240	0	2240	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM/LIVE
S	1823	1039 / 0	397 / 0	0 / 0
K	1823	1039 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 37	R-C	-213 / 119
B-C	-2258 / 0	C-Q	0 / 1948
C-D	-3257 / 0	Q-D	-1014 / 0
D-E	-3704 / 0	D-O	0 / 573
E-F	-3704 / 0	O-E	-533 / 0
F-G	-3704 / 0	O-G	0 / 573
G-H	-3257 / 0	M-G	-1014 / 0
H-I	-2258 / 0	M-H	0 / 1948
I-J	0 / 37	L-H	-213 / 119
S-B	-2188 / 0	B-R	0 / 1778
K-I	-2188 / 0	L-I	0 / 1778
S-R	0 / 0		
R-Q	0 / 1727		
Q-P	0 / 3257		
P-O	0 / 3257		
O-N	0 / 3257		
N-M	0 / 3257		
M-L	0 / 1727		
L-K	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.37")

CSI: TC=0.79 (E-G:1), BC=0.46 (M-O:1), WB=0.50 (D-Q:1), SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

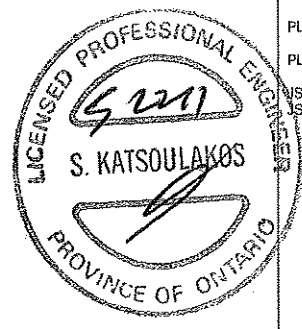
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

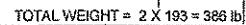
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.90 (R) (INPUT = 0.90)
ISI METAL= 0.57 (P) (INPUT = 1.00)



DWG NO. TAM25494-17
STRUCTURAL
COMPONENT ONLY



JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.62 (J) (INPUT = 1.00)

DWG NO. TAM25495-17
STRUCTURAL
COMPONENT ONLY

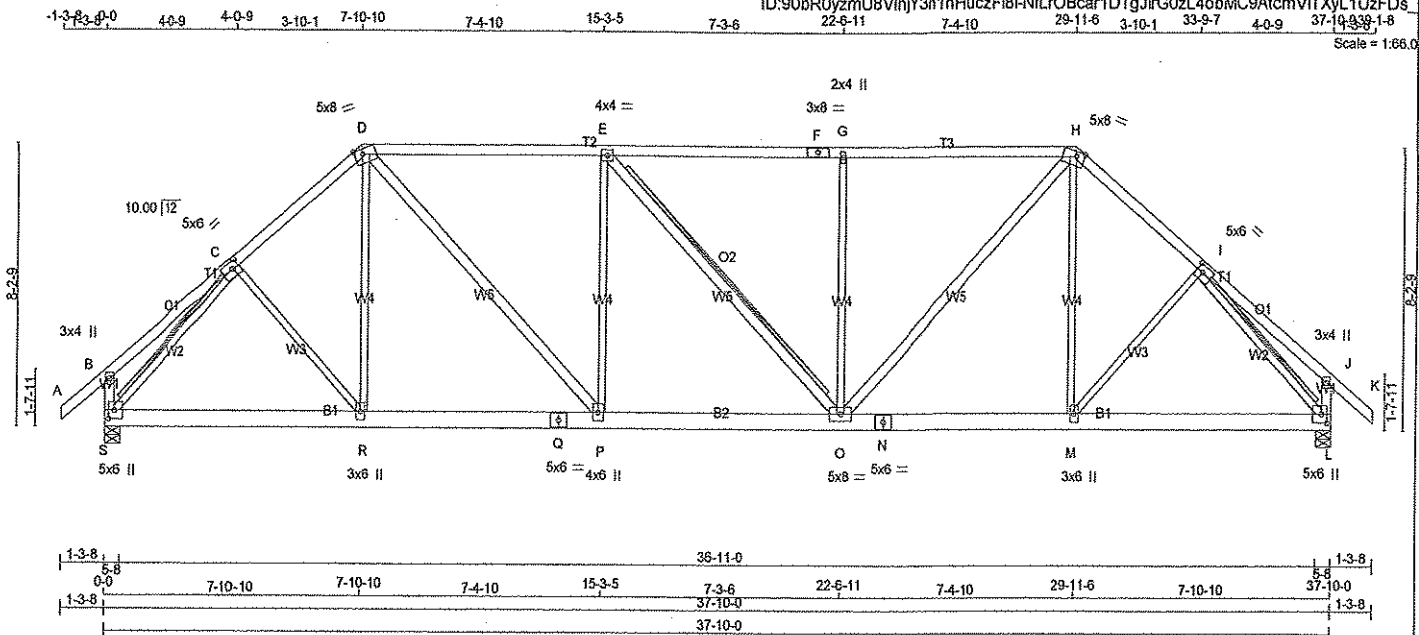
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44289	DRWG NO.
282293	T4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:90bR0yZmU8VihY3h1hHuczF8i-NlR0Bcar1DTgJrG0zL4obMC9AcmVITXyL1UzFDs

Scale = 1:66.0



TOTAL WEIGHT = 2 X 201 = 402 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
S - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
S - Q	2x6	DRY No.2	SPF
Q - N	2x6	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
D - P	2x4	DRY No.2	SPF
E - O	2x4	DRY No.2	SPF
O - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	5.0	6.0 2.50 2.25
D	TTWW-m	MT20	5.0	8.0 Edge 3.00
E	TMWW-t	MT20	4.0	4.0
F	TS-t	MT20	3.0	8.0
G	TMW+w	MT20	2.0	4.0
H	TTWW-m	MT20	5.0	8.0 Edge 3.00
I	TMWW-t	MT20	5.0	6.0 2.50 2.25
J	TMV+p	MT20	3.0	4.0
L	BMVW1+p	MT20	5.0	6.0 3.00 2.00
M	BMWW-t	MT20	3.0	6.0
N	BS-t	MT20	5.0	6.0
O	BMWW-t	MT20	5.0	8.0
P	BMWW-t	MT20	4.0	6.0
Q	BS-t	MT20	5.0	6.0
R	BMWW-t	MT20	3.0	6.0
S	BMVW1+p	MT20	5.0	6.0 3.00 2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG
S	2240	0	2240	0	5-8
L	2240	0	2240	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-O, C-S, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO	FROM	FR-TO	FROM
A-B	0 / 37	C-R	0 / 160
B-C	0 / 23	D-P	0 / 309
C-D	-2287 / 0	E-O	0 / 1194
D-E	-2537 / 0	F-G	-688 / 0
E-F	-2534 / 0	G-H	-5 / 0
F-G	-2534 / 0	H-I	-870 / 0
G-H	-2533 / 0	I-J	0 / 1188
H-I	-2262 / 0	J-K	0 / 311
I-J	0 / 23	K-L	0 / 160
J-K	0 / 37	L-M	-2525 / 0
K-L	-248 / 0	M-N	-2525 / 0
L-M	-248 / 0	N-O	0 / 160
M-N	0 / 1660	O-P	-28.0
N-O	0 / 1717	P-Q	-28.0
O-P	0 / 1717	Q-R	-28.0
P-Q	0 / 2537	R-S	-28.0
Q-R	0 / 1718	S-T	-28.0
R-S	0 / 1718	T-U	-28.0
S-T	0 / 1661	U-V	-28.0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14")
ALLOWABLE DEFL.(TL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.23")

CSI: TC=1.00 (D-E), BC=0.37 (O-P), WB=0.83 (G-O), SSI=0.29 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

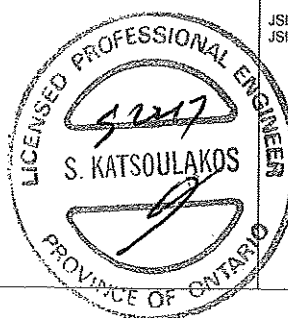
NAIL VALUES

PLATE	GRIP(ORY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)



BWU NO. TAM 25496 17
STRUCTURAL
COMPONENT ONLY

[illegible]

LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 5 X 216 = 1080 lb									
N. L. G. A. RULES										BEARINGS										DESIGN CRITERIA									
CHORDS		SIZE	LUMBER		DESCR.	FACTORED		MAXIMUM FACTORED		INPUT		REQ'D		SPECIFIED LOADS:															
A - D		2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		BRG		TOP CH. LL = 25.6 PSF															
D - F		2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 3.0 PSF															
F - H		2x4	DRY	No.2	SPF	U	2240	0	2240	0	0	5-8	5-8	BOT CH. LL = 10.5 PSF															
H - K		2x4	DRY	No.2	SPF	L	2240	0	2240	0	0	5-8	5-8	DL = 7.0 PSF															
U - B		2x6	DRY	No.2	SPF											TOTAL LOAD = 46.1 PSF													
L - J		2x6	DRY	No.2	SPF											SPACING = 24.0 IN./C/C													
U - R		2x6	DRY	No.2	SPF											LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12													
R - O		2x6	DRY	No.2	SPF											THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF													
O - L		2x6	DRY	No.2	SPF																								
						UNFACTORED REACTIONS																							
						1ST LCASE MAX /MIN. COMPONENT REACTIONS																							
						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																
						U	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0																
						L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0																
ALL WEBS EXCEPT																													
D - Q		2x4	DRY	No.2	SPF																								
E - P		2x4	DRY	No.2	SPF																								
P - H		2x4	DRY	No.2	SPF																								
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L																													

PLATES (table is in inches)					
J	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TMWV-t	MT20	4.0	4.0	2.00 1.25
D	TTWV-m	MT20	5.0	6.0	1.75 2.00
E	TMWV-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMW-w	MT20	2.0	4.0	
H	TTWV-m	MT20	5.0	6.0	1.75 2.00
I	TMWV-t	MT20	4.0	4.0	2.00 1.25
J	TMWV-p	MT20	5.0	8.0	Edge
L	BMV1+p	MT20	3.0	6.0	
M	BMWV-t	MT20	5.0	6.0	2.50 2.75
N	BMWV+t	MT20	3.0	6.0	
O	BS-t	MT20	5.0	6.0	
P	BMWVW-t	MT20	5.0	8.0	
Q	BMWV+t	MT20	4.0	6.0	
R	BS-t	MT20	5.0	6.0	
S	BMWV+t	MT20	3.0	6.0	
T	BMWV-t	MT20	6.0	6.0	2.50 2.75
U	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
<u>BEARINGS</u>							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2240	0	2240	0	0	5-8	5-8
L	2240	0	2240	0	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-P, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: {4}

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. UNBRAC	MAX. FACTORED		MAX. CSI (LC)		
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 (LC)		MEMB.	FORCE (LBS)			
FR-TO		FROM		LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	T-C	-333 / 8 0.15 (1)		
B-C	-2241 / 0	-84.3	-84.3	0.44 (1)	4.17	C-S	-121 / 0 0.11 (1)		
C-D	-2200 / 0	-84.3	-84.3	0.42 (1)	4.21	S-D	0 / 332 0.07 (2)		
D-E	-2215 / 0	-84.3	-84.3	0.66 (1)	3.89	D-Q	0 / 841 0.15 (1)		
E-F	-2211 / 0	-84.3	-84.3	0.65 (1)	3.89	Q-E	-580 / 0 0.32 (1)		
F-G	-2211 / 0	-84.3	-84.3	0.65 (1)	3.89	E-P	-6 / 0 0.60 (1)		
G-H	-2211 / 0	-84.3	-84.3	0.65 (1)	3.91	P-G	-583 / 0 0.32 (1)		
H-I	-2201 / 0	-84.3	-84.3	0.42 (1)	4.21	P-H	0 / 934 0.15 (1)		
I-J	-2241 / 0	-84.3	-84.3	0.44 (1)	4.17	N-H	0 / 335 0.08 (2)		
J-K	0 / 37	-84.3	-84.3	0.12 (1)	10.00	N-I	-119 / 0 0.11 (1)		
U-B	-2174 / 0	0.0	0.0	0.15 (1)	6.91	M-I	-338 / 8 0.15 (1)		
L-J	-2173 / 0	0.0	0.0	0.15 (1)	6.91	B-T	0 / 1801 0.41 (1)		
						M-J	0 / 1800 0.41 (1)		
U-T	0 / 0	-28.0	-28.0	0.07 (2)	10.00				
T-S	0 / 1744	-28.0	-28.0	0.26 (1)	10.00				
S-R	0 / 1666	-28.0	-28.0	0.26 (1)	10.00				
R-Q	0 / 1666	-28.0	-28.0	0.26 (1)	10.00				
Q-P	0 / 2215	-28.0	-28.0	0.32 (1)	10.00				
P-O	0 / 1667	-28.0	-28.0	0.25 (1)	10.00				
O-N	0 / 1667	-28.0	-28.0	0.25 (1)	10.00				
N-M	0 / 1743	-28.0	-28.0	0.26 (1)	10.00				
M-L	0 / 0	-28.0	-28.0	0.07 (2)	10.00				

A circular seal for a Professional Engineer. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "STATE OF TEXAS" at the bottom. The center of the seal features a five-pointed star.

TOTAL WEIGHT = $5 \times 216 \approx 1080$ lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.26")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
 ALLOWABLE DEFL.(TL) = $L/360$ (1.26")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.66 (D-E:1), BC=0.32 (P-Q:1), WB=0.41 (B-T:1), SS=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.44 (T) (INPUT = 1.00)



DWG NO. TAM 25492.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44269	DRWG NO.
282293	T6	2	1	TRUSS DESC.		

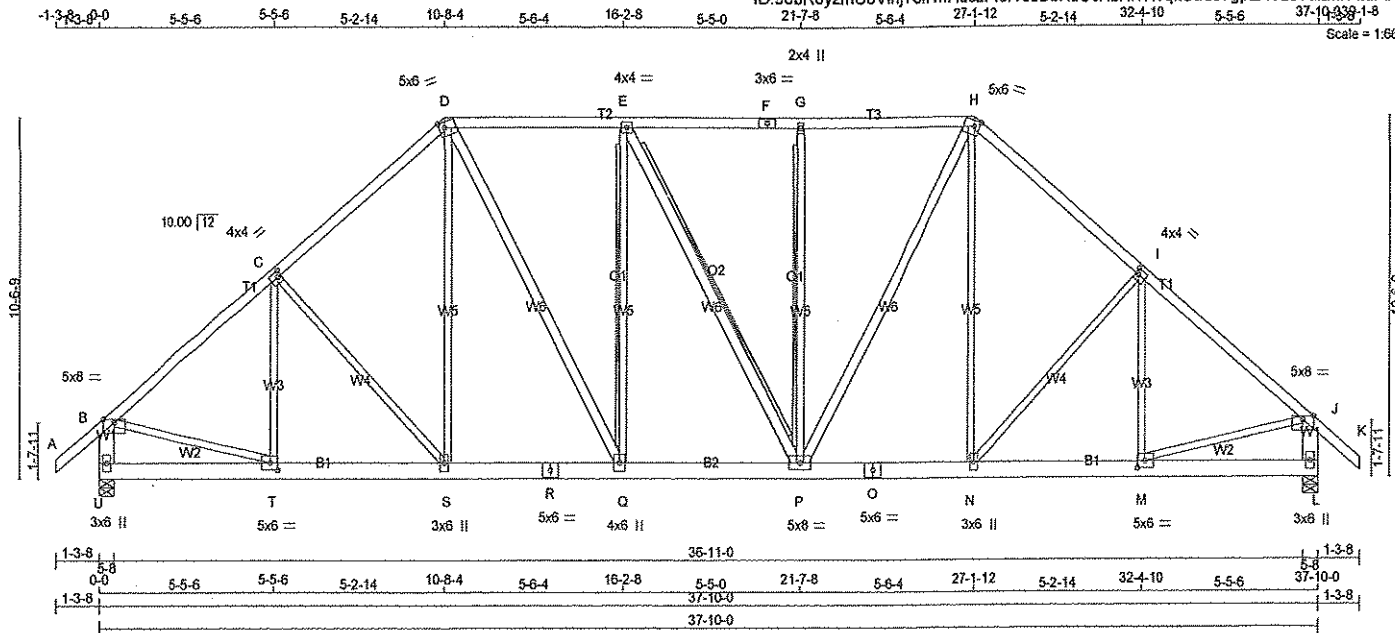
Tamarack Roof Truss, Burlington

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37-10-0-1-8

Scale = 1:60.4



TOTAL WEIGHT = 2 X 226 = 452 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - Q	2x4	DRY	No.2
E - P	2x4	DRY	No.2
P - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.25
D TTVW-m	MT20	5.0	6.0	2.00	2.00
E TMVW-t	MT20	4.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMVW-w	MT20	2.0	4.0		
H TTVW-m	MT20	5.0	6.0	2.00	2.00
I TMVW-t	MT20	4.0	4.0	2.00	1.25
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	6.0	2.50	2.75
N BMVW-t	MT20	3.0	6.0		
O BS-t	MT20	5.0	6.0		
P BMVW-t	MT20	5.0	8.0		
Q BMVW-t	MT20	4.0	6.0		
R BS-t	MT20	5.0	6.0		
S BMVW-t	MT20	3.0	6.0		
T BMVW-t	MT20	5.0	6.0	2.50	2.75
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
U	2240	0	2240	0	5-8
L	2240	0	2240	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-Q, E-P, G-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00
B-C	-2275 / 0	-84.3	-84.3	0.43 (1)	4.17
C-D	-2133 / 0	-84.3	-84.3	0.40 (1)	4.30
D-E	-1985 / 0	-84.3	-84.3	0.35 (1)	4.49
E-F	-1981 / 0	-84.3	-84.3	0.32 (1)	4.52
F-G	-1981 / 0	-84.3	-84.3	0.32 (1)	4.52
G-H	-1981 / 0	-84.3	-84.3	0.34 (1)	4.49
H-I	-2134 / 0	-84.3	-84.3	0.40 (1)	4.30
I-J	-2275 / 0	-84.3	-84.3	0.43 (1)	4.17
J-K	0 / 37	-84.3	-84.3	0.12 (1)	10.00
U-B	-2169 / 0	0.0	0.0	0.15 (1)	6.92
L-J	-2169 / 0	0.0	0.0	0.15 (1)	6.92
U-T	0 / 0	-28.0	-28.0	0.09 (2)	10.00
T-S	0 / 1774	-28.0	-28.0	0.25 (1)	10.00
S-R	0 / 1611	-28.0	-28.0	0.24 (1)	10.00
R-Q	0 / 1611	-28.0	-28.0	0.24 (1)	10.00
Q-P	0 / 1965	-28.0	-28.0	0.28 (1)	10.00
P-O	0 / 1612	-28.0	-28.0	0.24 (1)	10.00
O-N	0 / 1612	-28.0	-28.0	0.24 (1)	10.00
N-M	0 / 1773	-28.0	-28.0	0.26 (1)	10.00
M-L	0 / 0	-28.0	-28.0	0.09 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.43 (B-C:1), BC=0.28 (P-Q:1), WB=0.42 (G-P:1), SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

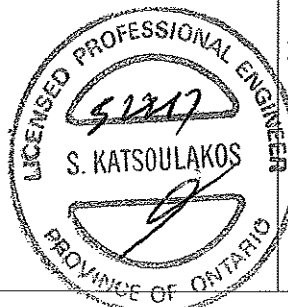
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.44 (T) (INPUT = 1.00)



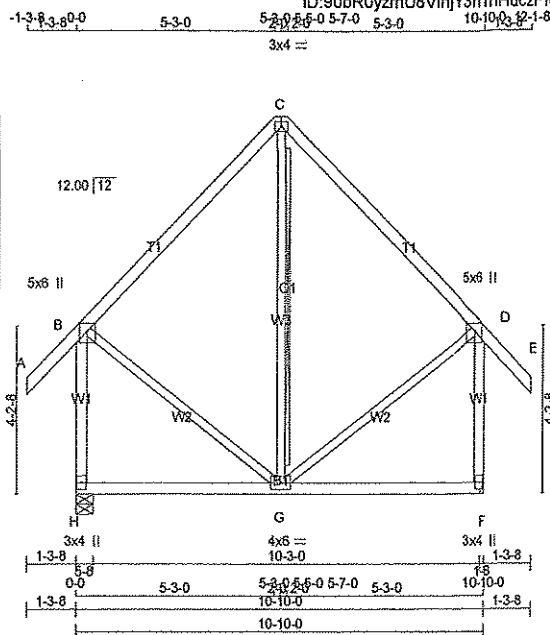
DWG NO. TAM25498-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44289	DRWG NO.
282293	T8	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:90bR0ymU8VihY3h1hHuczF18iJ2ScpseqNeTbwdvNR0p9DgsiztH4r61xrS5NzFDry



Scale = 1:50.8

TOTAL WEIGHT = 61 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-p	MT20	3.0	4.0		
D	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS	REACTION	GROSS	REACTION	BRG	BRG	IN-SX	IN-SX
H	726	0	726	0	0	5-8	5-8		
F	726	0	726	0	0				

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS	PERM	LIVE	WIND	DEAD	SOIL
H	579	349 / 0	114 / 0	0 / 0	0 / 0	117 / 0	0 / 0		
F	579	349 / 0	114 / 0	0 / 0	0 / 0	117 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	CS1 (LC)	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 42	-84.3	-84.3	0.12 (1)	10.00	B-G	0 / 231	0.05 (1)	
B-C	-268 / 0	-84.3	-84.3	0.32 (1)	6.25	G-D	0 / 231	0.05 (1)	
C-D	-268 / 0	-84.3	-84.3	0.32 (1)	6.25	G-C	-80 / 176	0.05 (1)	
D-E	0 / 42	-84.3	-84.3	0.12 (1)	10.00				
H-B	-668 / 0	0.0	0.0	0.20 (1)	7.81				
F-D	-668 / 0	0.0	0.0	0.20 (1)	7.81				
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00				
G-F	0 / 0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS1 TC=0.32 (C-D:1), BC=0.24 (G-H:3), WB=0.05 (C-G:1), SSI=0.13 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

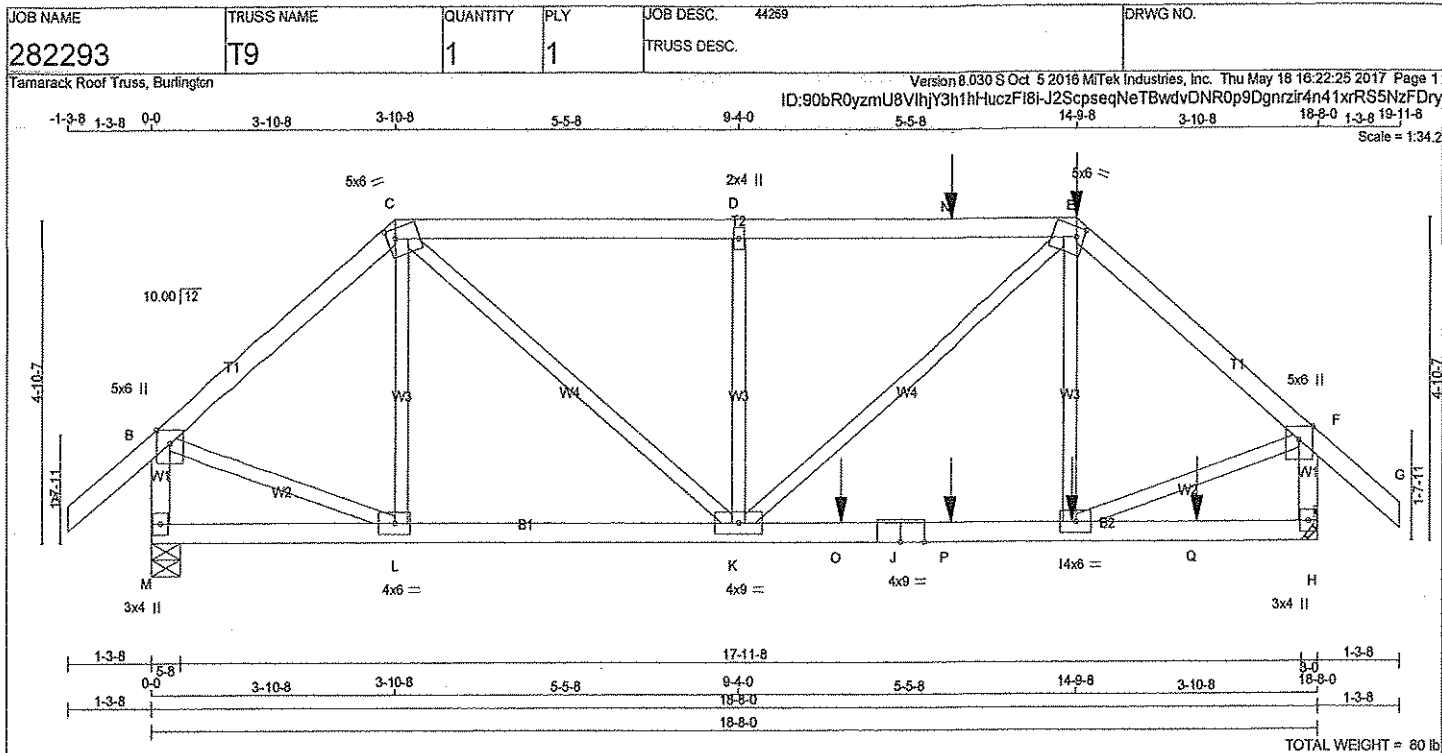
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (C) (INPUT = 0.60)
JSI METAL= 0.08 (D) (INPUT = 1.00)



DWG NO. TAM 25500-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	1.75	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-l	MT20	4.0	6.0		
J	BS-l	MT20	4.0	9.0		
K	BMVW-l	MT20	4.0	9.0		
L	BMVW-l	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 79.0 lbs FACTORED DOWN AT 12-8-12, AND 82.5 lbs FACTORED DOWN AT 14-9-8 ON TOP CHORD, AND 583.3 lbs FACTORED DOWN AT 10-11-8, 42.0 lbs FACTORED DOWN AT 12-8-12, AND 42.0 lbs FACTORED DOWN AT 14-8-12, AND 42.0 lbs FACTORED DOWN AT 16-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
M	1463	0	1463	0	5-8	5-8
H	1683	0	1683	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-0

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
M	1173	698 / 0	237 / 0	0 / 0	0 / 0	240 / 0	0 / 0
H	1356	793 / 0	281 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (L)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (L)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 37	-84.3	-84.3	0.13 (1)	10.00	L-C	-248 / 66	0.09 (1)	
B-C	-1258 / 0	-84.3	-84.3	0.28 (1)	5.38	C-K	0 / 1117	0.28 (1)	
C-D	-1815 / 0	-84.3	-84.3	0.60 (1)	4.17	K-D	-595 / 0	0.22 (1)	
D-N	-1815 / 0	-84.3	-84.3	0.60 (1)	4.17	K-E	0 / 803	0.20 (1)	
N-E	-1815 / 0	-84.3	-84.3	0.60 (1)	4.17	L-E	0 / 238	0.06 (3)	
E-F	-1584 / 0	-84.3	-84.3	0.30 (1)	4.92	B-L	0 / 1014	0.25 (1)	
F-G	0 / 37	-84.3	-84.3	0.13 (1)	10.00	I-F	0 / 1261	0.31 (1)	
M-B	-1405 / 0	0.0	0.0	0.16 (1)	6.84				
H-F	-1678 / 0	0.0	0.0	0.20 (1)	6.36				
M-L	0 / 0	-28.0	-28.0	0.12 (3)	10.00				
L-K	0 / 958	-28.0	-28.0	0.63 (1)	10.00				
K-O	0 / 1199	-28.0	-28.0	0.91 (1)	10.00				
O-J	0 / 1199	-28.0	-28.0	0.91 (1)	10.00				
J-P	0 / 1199	-28.0	-28.0	0.91 (1)	10.00				
P-I	0 / 1199	-28.0	-28.0	0.91 (1)	10.00				
I-Q	0 / 0	-28.0	-28.0	0.38 (1)	10.00				
Q-H	0 / 0	-28.0	-28.0	0.38 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
E	14-9-8	-83	-83	---	FRONT	VERT	TOTAL
I	14-8-12	-24	-42	---	FRONT	VERT	TOTAL
N	12-8-12	-79	-79	---	FRONT	VERT	TOTAL
O	10-11-8	-563	-563	---	FRONT	VERT	TOTAL
P	12-8-12	-24	-42	---	FRONT	VERT	TOTAL
Q	16-8-12	-24	-42	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL)= L/910 (0.25")

CSI: TC=0.60 (D-E:1), BC=0.91 (I-K:1), WB=0.31 (F-I:1), SSI=0.43 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

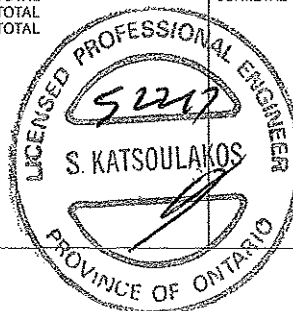
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.71 (J) (INPUT = 1.00)



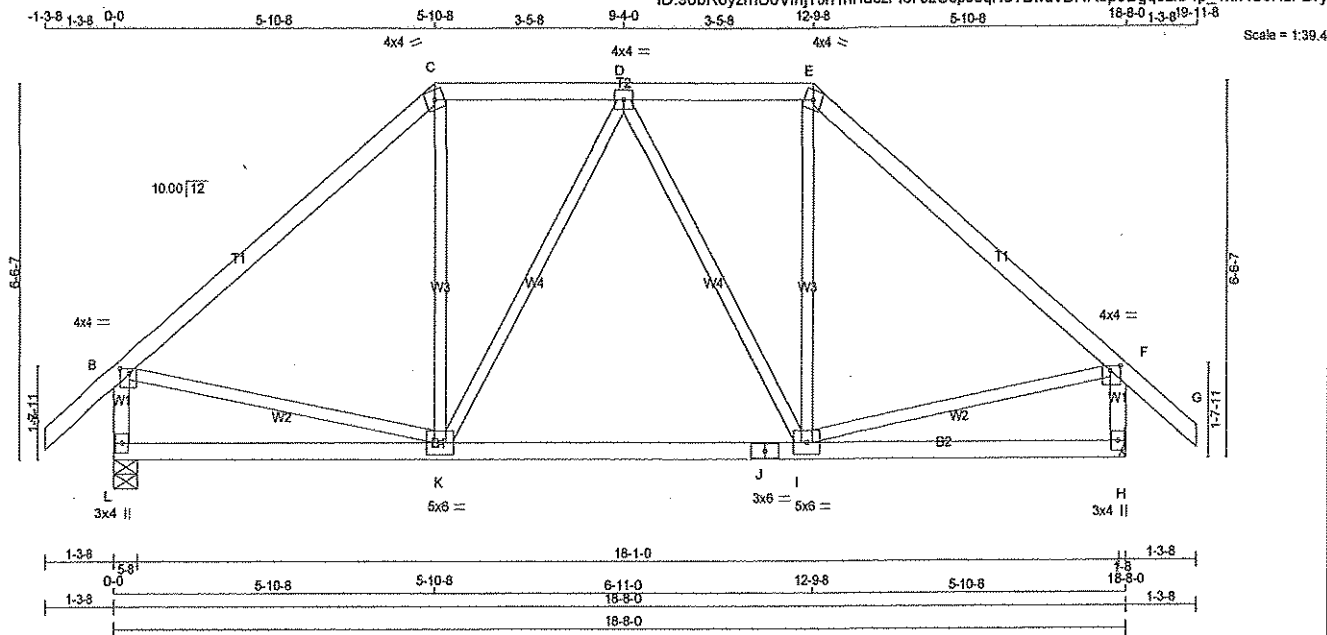
DWG NO. TAM25501-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44289	DRWG NO.
282293	T10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:90bR0ymU8VihjY3h1hHucZF18i-J2ScpseqNeTBwdvDNR0p9Dgq5zrb4p_1xrRS5NzFDry



TOTAL WEIGHT = 84 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	4.0	1.00 2.25
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMVW-p	MT20	4.0	4.0	1.00 2.25
H	BMVt+p	MT20	3.0	4.0	
I	BMVW-t	MT20	5.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMVW-t	MT20	5.0	6.0	
L	BMVt+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1164	0	1164	0	0
H	1164	0	1164	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
L	939	548 / 0	196 / 0	0 / 0
H	939	548 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)			
CHORDS		MEMB.		WEBS	
MEMB.	FORCE (LBS)	VERT.	LOAD	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 37	-84.3	-84.3	K-C	0 / 294
B-C	-908 / 0	-84.3	-84.3	K-D	-198 / 0
C-D	-702 / 0	-84.3	-84.3	D-I	-198 / 0
D-E	-702 / 0	-84.3	-84.3	I-E	0 / 294
E-F	-908 / 0	-84.3	-84.3	B-K	0 / 712
F-G	0 / 37	-84.3	-84.3	I-F	0 / 712
L-B	-1097 / 0	0.0	0.0		
H-F	-1097 / 0	0.0	0.0		
L-K	0 / 0	-28.0	-28.0		
K-J	0 / 794	-28.0	-28.0		
J-I	0 / 794	-28.0	-28.0		
I-H	0 / 0	-28.0	-28.0		

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF
SPACING	= 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.39 (E-F:1), BC=0.35 (I-K:2), WB=0.19 (D-K:1), SSI=0.15 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

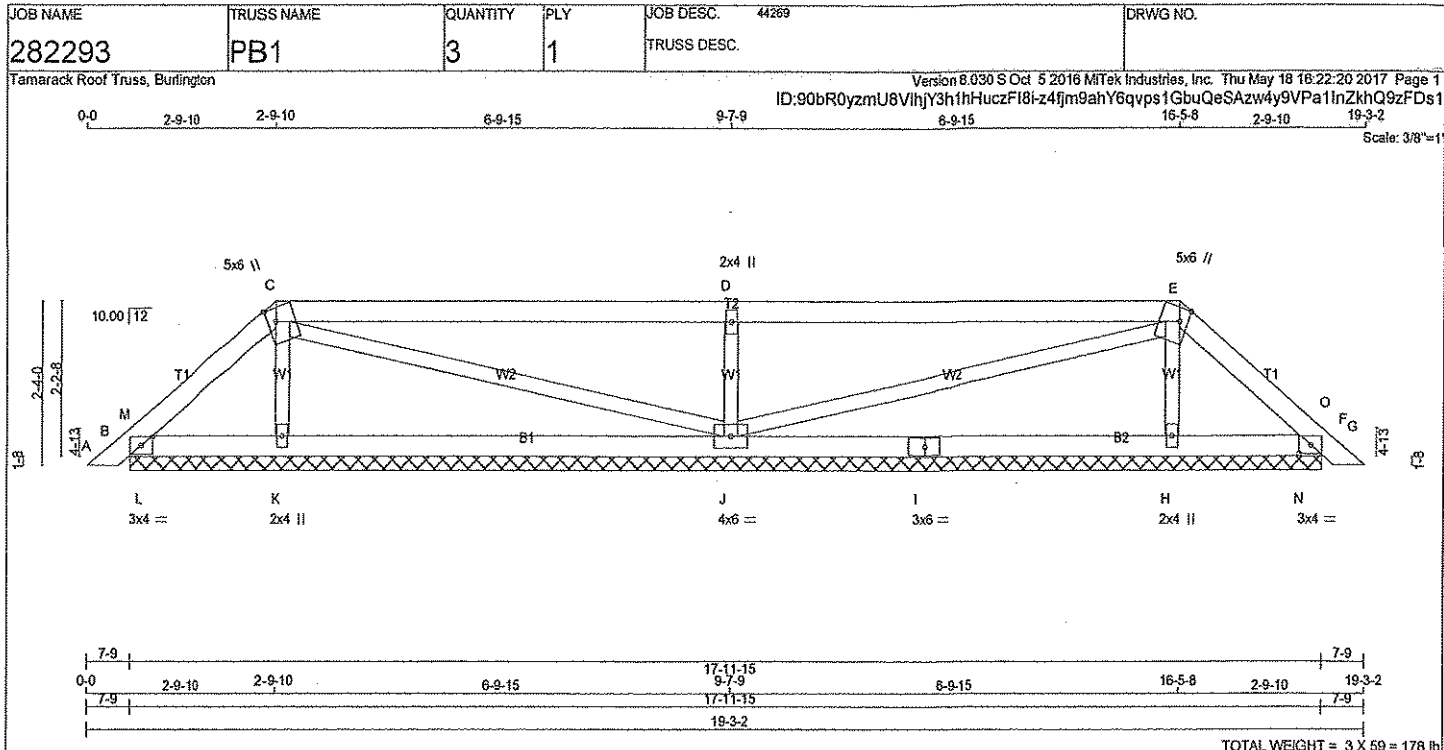
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.80)
JSI METAL= 0.35 (J) (INPUT = 1.00)



DWG NO. TAM2550211
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TWW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BS-I	MT20	3.0	6.0		
J	BMW1+w	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
B	172	0	172	0
K	419	0	419	0
J	929	0	929	0
H	419	0	419	0
F	172	0	172	0

17-11-15 (6-045) 11-15

17-11-15 (6-045) 11-15

17-11-15 (6-045) 11-15

17-11-15 (6-045) 11-15

17-11-15 (6-045) 11-15

17-11-15 (6-045) 11-15

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	113	109/0	0/-5	0/0	0/0	9/0	0/0
K	375	158/0	120/0	0/0	0/0	98/0	0/0
J	744	443/0	149/0	0/0	0/0	151/0	0/0
H	375	158/0	120/0	0/0	0/0	98/0	0/0
F	113	109/0	0/-5	0/0	0/0	9/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO								
A-B	0/13	-84.3	-84.3	0.02 (1)	10.00	K-C	-240/0	0.04 (1)
B-M	0/63	-84.3	-84.3	0.04 (2)	10.00	C-J	-23/0	0.02 (1)
M-C	-68/0	-84.3	-84.3	0.04 (1)	6.25	J-D	-719/0	0.11 (1)
C-D	-10/3	-84.3	-84.3	0.67 (1)	6.25	J-E	-23/0	0.02 (1)
D-E	-10/3	-84.3	-84.3	0.67 (1)	6.25	H-E	-240/0	0.04 (1)
E-O	-68/0	-84.3	-84.3	0.04 (1)	6.25	L-M	-189/0	0.00 (1)
O-F	0/63	-84.3	-84.3	0.04 (2)	10.00	N-O	-189/0	0.00 (1)
F-G	0/13	-84.3	-84.3	0.02 (1)	10.00			
B-L	-3/46	-28.0	-28.0	0.06 (1)	10.00			
L-K	-3/46	-28.0	-28.0	0.20 (2)	10.00			
K-J	-4/33	-28.0	-28.0	0.28 (2)	10.00			
J-I	-4/33	-28.0	-28.0	0.28 (2)	10.00			
I-H	-4/33	-28.0	-28.0	0.28 (2)	10.00			
H-N	-3/46	-28.0	-28.0	0.20 (2)	10.00			
N-F	-3/46	-28.0	-28.0	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.67 (C-D:1), BC=0.28 (H-J:2), WB=0.11 (D-J:1), SSI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

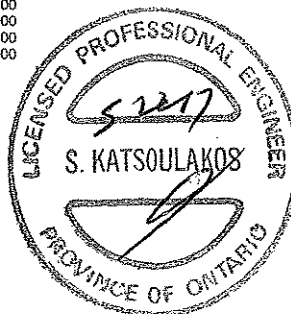
JSI GRIP= 0.74 (J) (INPUT = 0.90)

JSI METAL= 0.22 (I) (INPUT = 1.00)

DWG NO. TAM 25503.17

STRUCTURAL

COMPONENT ONLY



DWG NO. TAM 25503.17
STRUCTURAL
COMPONENT ONLY

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

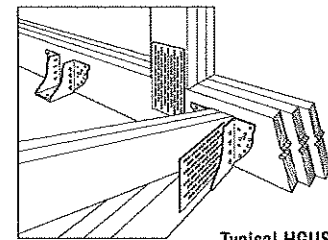
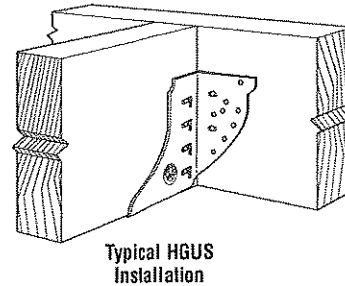
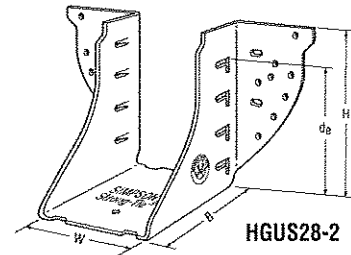
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

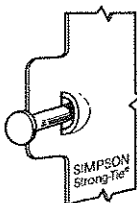
OPTIONS:

- See current catalogue for options

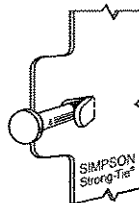


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½ ₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3½	9½	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4½	9½	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	66-16d	22-16d	10130	16400	7195	11645

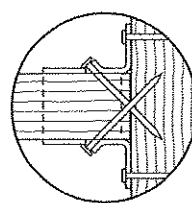
1. d_g is the distance from the seat of the hanger to the highest joist nail.



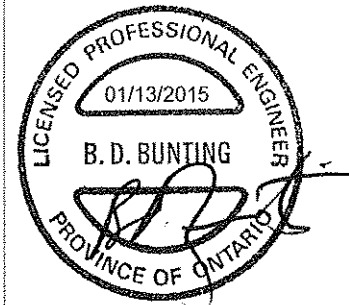
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

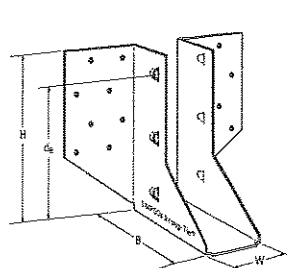
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

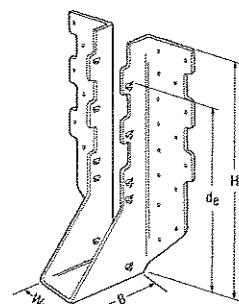
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

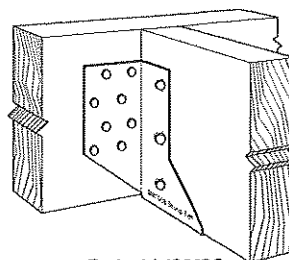
- See current catalogue for options



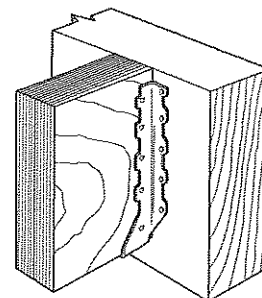
LJS26DS



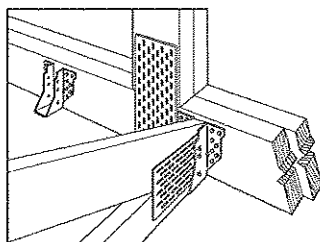
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



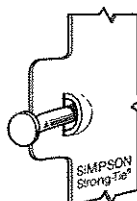
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

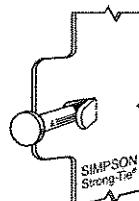
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4%	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1%	5%	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1%	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1%	9 3/32	3	7 3/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

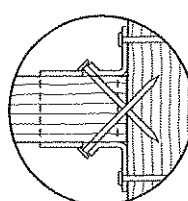


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

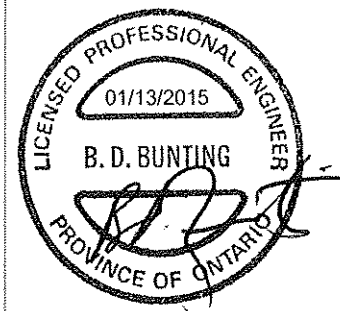
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

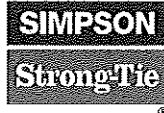
This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

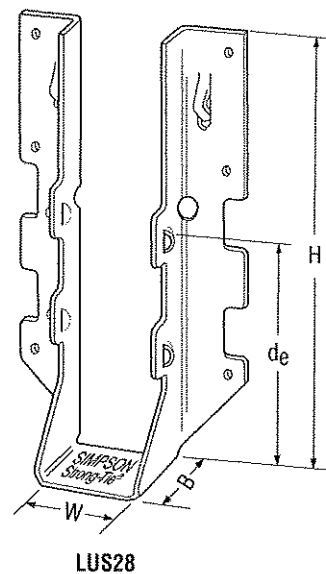
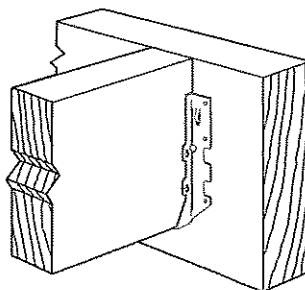
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

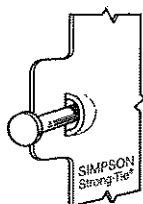
- These hangers cannot be modified.

Typical LUS Installation



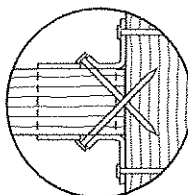
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1⅞	3⅞	1¾	1⅞	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅞	4-16d	2-16d	835	2020	590	1435
LUS26	18	1⅞	4⅞	1¾	3⅞	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅞	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅞	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅞	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¾	3⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅞	8⅞	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

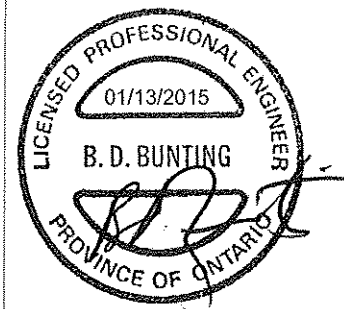


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

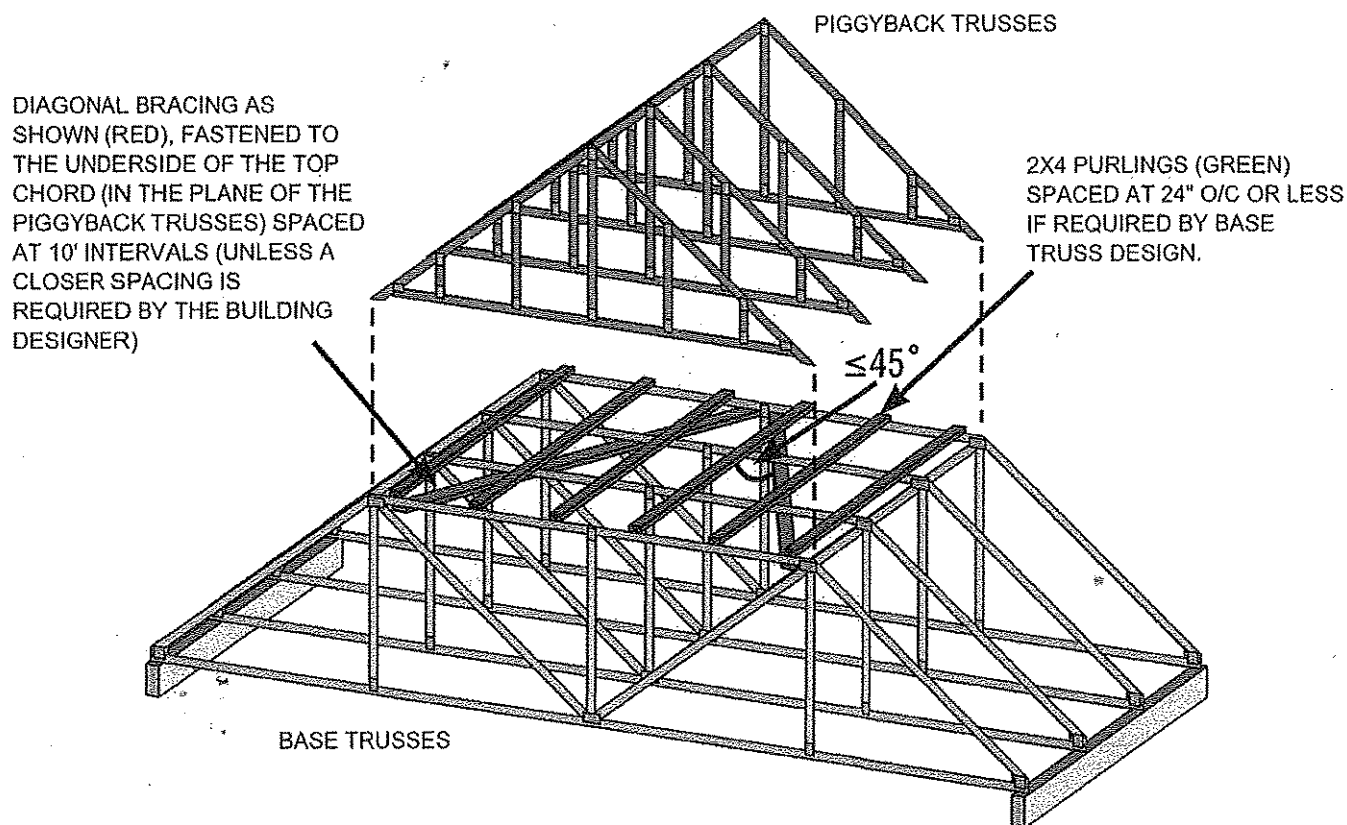
**TN 15-001
Piggyback Bracing**

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

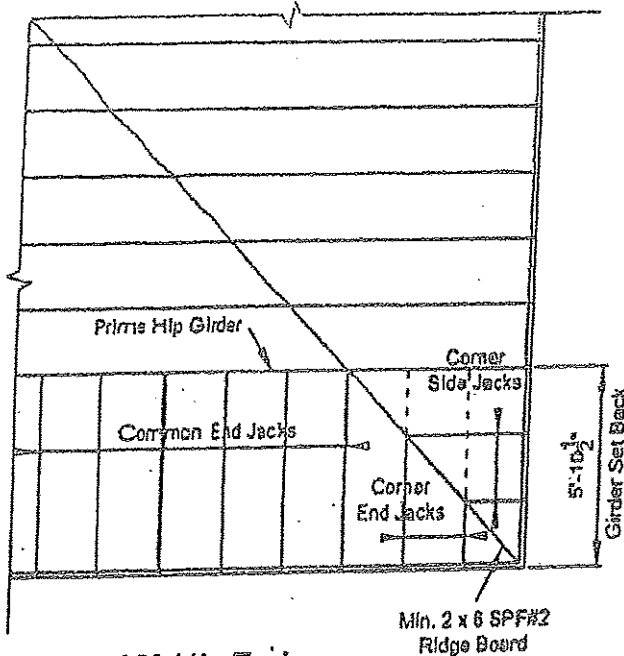
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

MICRO CITY ENGINEERING SERVICES INC.

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45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

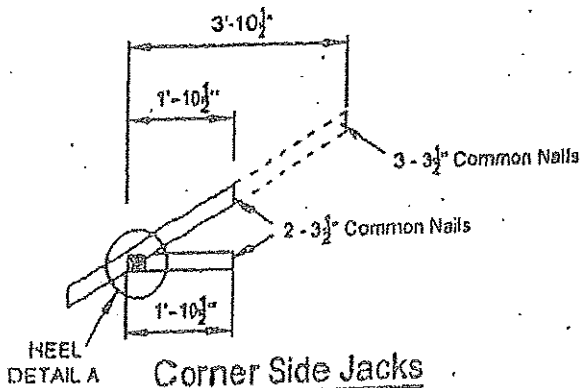
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

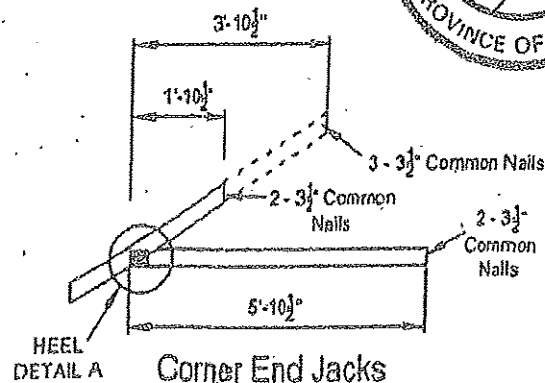
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

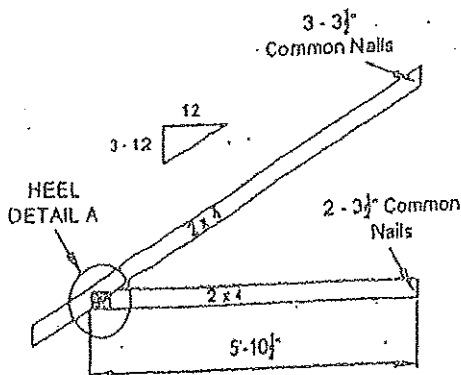
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STRUCTURAL
COMPONENT ONLY



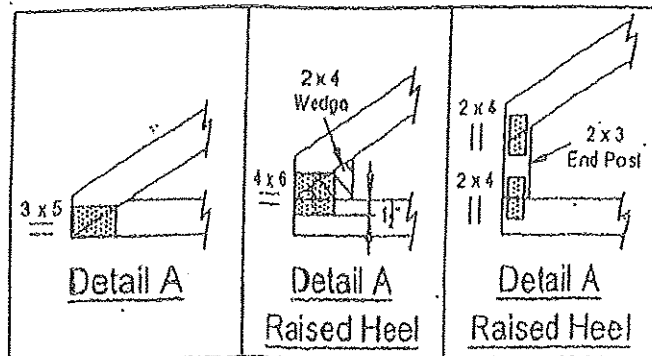
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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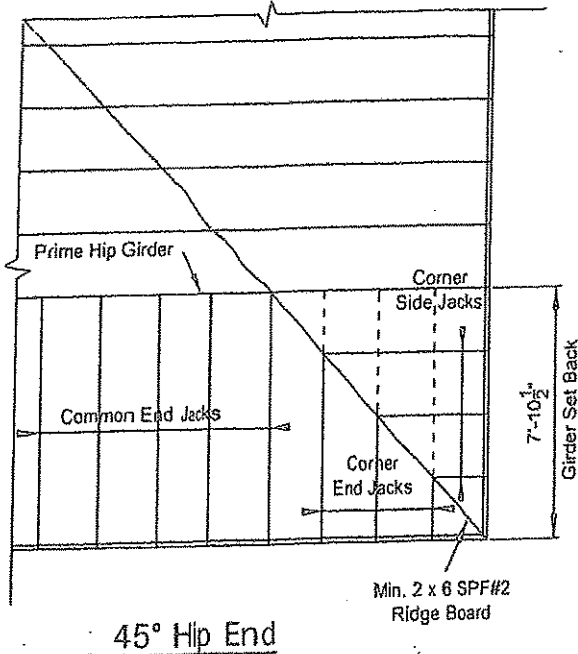
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

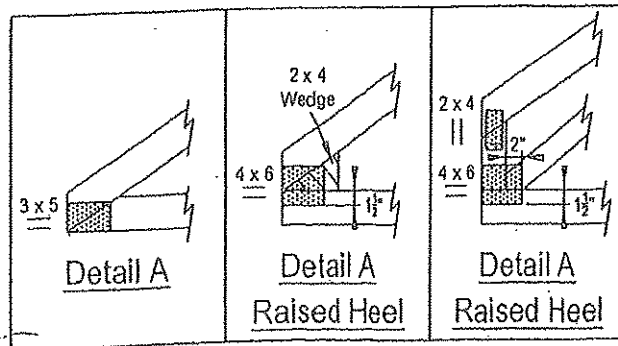
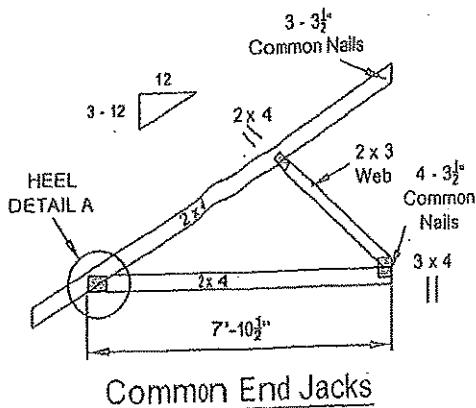
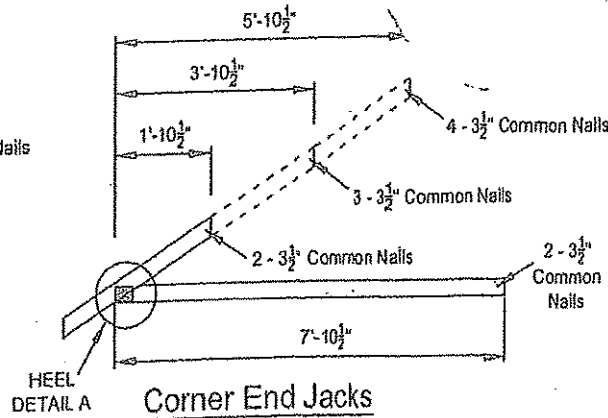
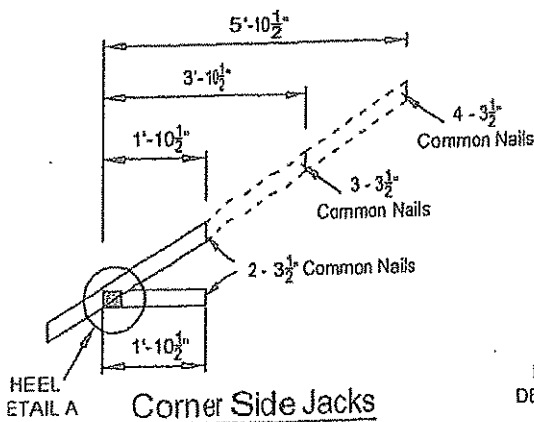
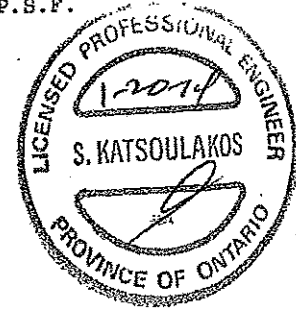
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.



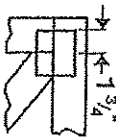
DWG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY



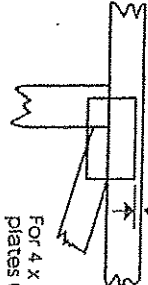
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mittek software or upon request.

PLATE SIZE

4 X 4

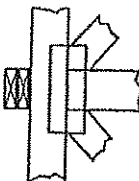
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

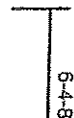
BEARING



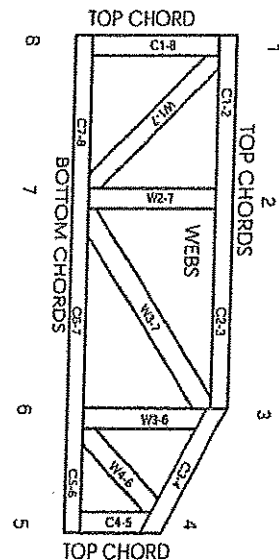
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

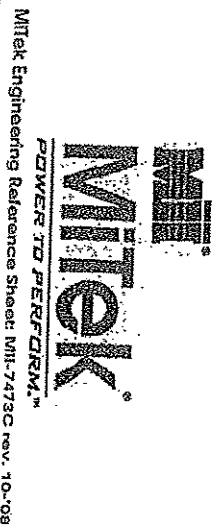
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

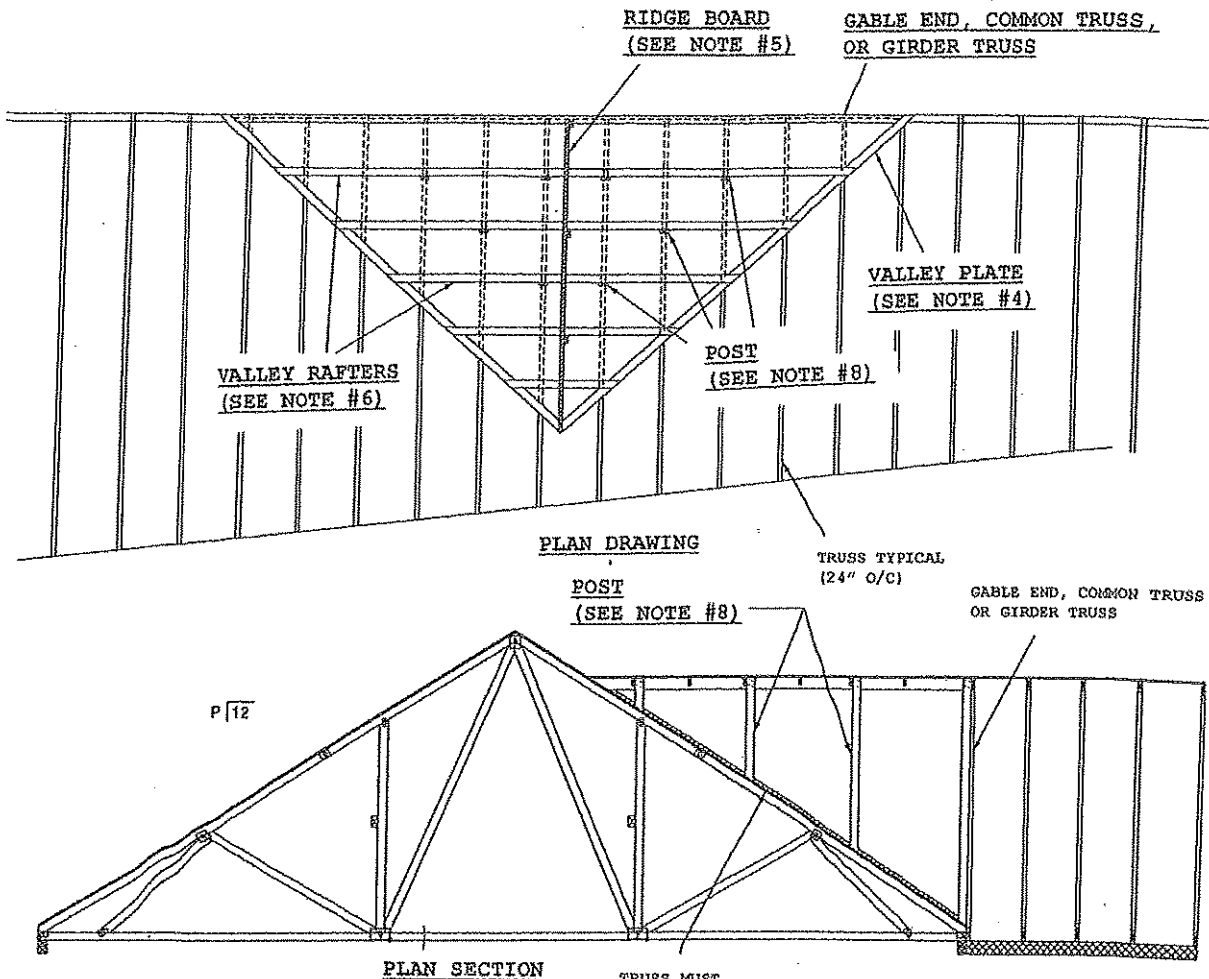
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Nuts and washers at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

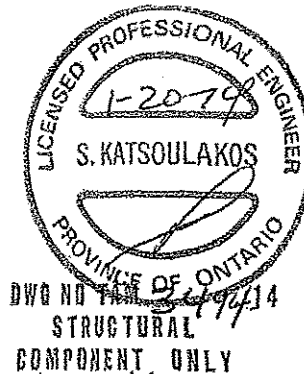


DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.