

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170371

MAY 24, 2017
THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS
COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT: 44269/282407)
DWG #TAM25687-17 THROUGH DWG #TAM25703-17, INCLUSIVE, AND ALL DATED 5-24-17;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF
FRAMING TO CONFORM TO
PART 9 OF THE OBC. LATEST
EDITION ROOF RAFTERS
THAT MEET OR CROSS
OVER TRUSSES ARE TO BE
2"X4"SPF@24"o.c. WITH A
2"X4"SPF VERTICAL POST TO
THE TRUSS UNDER AT EACH
CROSS POINT. POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS AND
BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

DESIGN CONFORMS WITH
THE RELEVANT SECTION OF
THE LATEST EDITION OF
O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)
HUSC26-2(CC)

DATE: 5-24-17
BCIN: 26064; FIRM: BCIN #29991
ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED
ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING
DESIGNER
ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING
DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO
NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A
QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT.
ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE
BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR
PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS
PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE
ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
INDIVIDUAL LOT.

DWG #TAM THROUGH DWG #TAM, INCLUSIVE, DATED;

STRUCTURAL COMPONENTS ONLY; THIS LAYOUT MUST BE READ TOGETHER WITH
REFERENCED TRUSS COMPONENT DRAWINGS AND SEALED HARDWARE DRAWINGS.
HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY.
REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING
REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2 X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS
SPECIFIED ON REFERENCED SEALED TRUSS COMPONENT DRAWINGS USING 2 - 3/4"
COMMON WIRE NAILS PER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING"
ALONG THESE SAME WEB MEMBERS AT LOCATIONS INDICATED (") USING THE SAME
NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF T-BRACE ALONG FULL LENGTH OF LATERALLY BRACED WEB
MEMBERS OF 1 PLY TRUSSES COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-
1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF
NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING
FOR 3 PLY TRUSS COMPONENTS.

(1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
(2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS
COMPONENTS AND AT LOCATIONS MARKED BY "X".
(3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X
4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY).

ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE
TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH
STUDS AT 24" O/C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE
(CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN
WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION
3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED
AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES
AND/OR CATEGORIES.
REGISTERED FIRM: MICRO CITY ENGINEERING SERVICES INC.

DWG #TAM 25704-17
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY



44269		GREENYORK HOMES /		BRAMPTON		AUBURN 7 / 1	
282407		OSTIENSE		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
92456		Date: 5/23/2017 Designer: JG		C:\MITEK\A742A\JOBS\ 813000000005 Mitek ver 7.5.0			



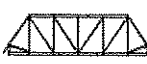
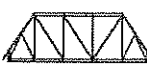
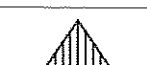
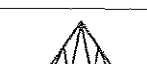
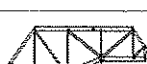
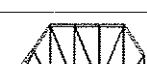
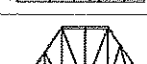
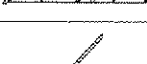
Delivery Shiplist

DATE	05/23/17
SALES REP	Rick

JOB TRACK:44269	LAYOUT ID: 282407	LOCATION: BRAMPTON
BUILDER: GREEN YORK HOMES	SUB-BUILDER:	
MODEL: AUBURN 7	ELEVATION: 1	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	10.00	37-10-00	05-00-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	361.78		
	2 Ply		0.00							227.34		
	1	T1Z HIP GIRDER	10.00	37-10-00	05-00-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	361.78		
	2 Ply		0.00							227.34		
	2	T2 HIP	10.00 0.00	37-10-00	06-03-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	363.92 224.66		
	2	T3 HIP	10.00 0.00	37-10-00	07-06-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	395.26 242.00		
	2	T4 HIP	10.00 0.00	37-10-00	08-09-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	421.96 261.34		
	6	T5 HIP	10.00 0.00	37-10-00	10-00-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	1328.16 817.98		
	2	T6 HIP	10.00 0.00	37-10-00	11-03-03	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	447.18 270.00		
	2	T7S ROOF	10.00 6.00	11-04-00	06-04-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	109.30 72.66		
	2	T8 COMMON	10.00 0.00	18-11-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	182.96 116.66		
	1	G8 GABLE	10.00 0.00	18-11-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	97.73 62.83		
	2	T8A COMMON	10.00 0.00	18-06-00	09-06-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-11-14	177.28 114.00		
	1	T9S HIP GIRDER	10.00 0.00	28-00-00	05-11-10	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	159.66 99.00		
	1	T10 HIP	10.00 0.00	27-09-00	07-07-10	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-10-03	132.29 85.17		
	1	T11 HIP	10.00 0.00	27-09-00	09-03-10	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-10-03	145.54 90.33		
	1	T12 FLAT GIRDER	0.00	05-10-08	02-00-00	2 X 6	2 X 6	00-00-00	02-00-00	57.80		
	2 Ply		0.00					00-00-00	02-00-00	37.34		
	2	PB1 PIGGYBACK	10.00 0.00	16-05-11	02-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	109.92 67.34		
	2	PB2 PIGGYBACK	10.00 0.00	16-05-11	03-07-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	115.60 74.66		
	22	J1 JACK-OPEN	7.50 0.00	05-10-08	05-00-03	2 X 4	2 X 4	01-03-08 00-00-00	01-04-02 05-00-03	386.32 234.74		



Delivery Shiplist

DATE	05/23/17
SALES REP	Rick

JOB TRACK:44269	LAYOUT ID: 282407	LOCATION: BRAMPTON
BUILDER: GREEN YORK HOMES	SUB-BUILDER:	
MODEL: AUBURN 7	ELEVATION: 1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	J2 JACK-OPEN	10.00 0.00	02-09-08	03-11-10	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-11-10	52.68 35.32		

TOTAL # TRUSS= 60

TOTAL BFT OF ALL TRUSSES=

3360.71 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5407.12 LBS.

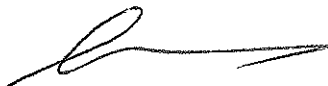
HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HUSC26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= 6

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:													
Building number, street name			Unit no.	Lot/con.												
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description														
B. Individual who reviews and takes responsibility for design activities																
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.														
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.												
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail													
Telephone number (519) 287-2242 Business		Fax number (519) 287-5750	Cell number													
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]																
<table style="width: 100%; border: none;"> <tr> <td>☐ House</td> <td>☐ HVAC – House</td> <td>☒ Building Structural</td> </tr> <tr> <td>☐ Small Buildings</td> <td>☐ Building Services</td> <td>☐ Plumbing – House</td> </tr> <tr> <td>☐ Large Buildings</td> <td>☐ Detection, Lighting and Power</td> <td>☐ Plumbing – All Buildings</td> </tr> <tr> <td>☐ Complex Buildings</td> <td>☐ Fire Protection</td> <td>☐ On-site Sewage Systems</td> </tr> </table>					☐ House	☐ HVAC – House	☒ Building Structural	☐ Small Buildings	☐ Building Services	☐ Plumbing – House	☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings	☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems
☐ House	☐ HVAC – House	☒ Building Structural														
☐ Small Buildings	☐ Building Services	☐ Plumbing – House														
☐ Large Buildings	☐ Detection, Lighting and Power	☐ Plumbing – All Buildings														
☐ Complex Buildings	☐ Fire Protection	☐ On-site Sewage Systems														
Description of designer's work GREENYORK HOMES – OSTIENSE – MODEL: AUBURN 7 – ELEV. 1 (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM25704-17 DATED 5-24-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.																
D. Declaration of Designer																
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name)																
☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories.																
Individual BCIN: <u>26064</u>																
Firm BCIN: <u>29991</u>																
☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code.																
Individual BCIN: _____																
Basis for exemption from registration: _____																
☐ The design work is exempt from the registration and qualification requirements of the Building Code.																
Basis for exemption from registration and qualification: _____																
I certify that:																
1. The information contained in this schedule is true to the best of my knowledge.																
2. I have submitted this application with the knowledge and consent of the firm.																
Date		Signature of Designer														
5-24-17																

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM25704-17-S
DWG#TAM 25705-17-S

5-24-17


[illegible]

JOB NAME 282407	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. 44269	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed May 24 09:22:19 2017 Page 2	
				ID:OTpCxmWV4rD8qAHS75NEMPzEuax-mT9HQ?G6VWJuzVAH?Ed_sH6 TCAGyGtj5l8L5zDLRo	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 205.8 lbs FACTORED DOWN AT 4-0-10, 205.8 lbs FACTORED DOWN AT 33-9-6, 104.7 lbs FACTORED DOWN AT 4-0-10, 101.4 lbs FACTORED DOWN AT 5-11-4, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, 101.4 lbs FACTORED DOWN AT 11-11-4, 101.4 lbs FACTORED DOWN AT 13-11-4, 101.4 lbs FACTORED DOWN AT 15-11-4, 101.4 lbs FACTORED DOWN AT 17-11-4, 101.4 lbs FACTORED DOWN AT 19-11-4, 101.4 lbs FACTORED DOWN AT 21-11-4, 101.4 lbs FACTORED DOWN AT 23-11-4, 101.4 lbs FACTORED DOWN AT 25-11-4, 101.4 lbs FACTORED DOWN AT 27-11-4, 101.4 lbs FACTORED DOWN AT 29-11-4, AND 101.4 lbs FACTORED DOWN AT 31-11-4, AND 104.6 lbs FACTORED DOWN AT 33-9-6 ON TOP CHORD, AND 71.3 lbs FACTORED DOWN AT 1-11-4, 71.3 lbs FACTORED DOWN AT 3-11-4, 71.3 lbs FACTORED DOWN AT 5-11-4, 71.3 lbs FACTORED DOWN AT 7-11-4, 71.3 lbs FACTORED DOWN AT 9-11-4, 71.3 lbs FACTORED DOWN AT 11-11-4, 71.3 lbs FACTORED DOWN AT 13-11-4, 71.3 lbs FACTORED DOWN AT 15-11-4, 71.3 lbs FACTORED DOWN AT 17-11-4, 71.3 lbs FACTORED DOWN AT 19-11-4, 71.3 lbs FACTORED DOWN AT 21-11-4, 71.3 lbs FACTORED DOWN AT 23-11-4, 71.3 lbs FACTORED DOWN AT 25-11-4, 71.3 lbs FACTORED DOWN AT 27-11-4, 71.3 lbs FACTORED DOWN AT 29-11-4, AND 71.3 lbs FACTORED DOWN AT 31-11-4, AND 71.3 lbs FACTORED DOWN AT 35-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

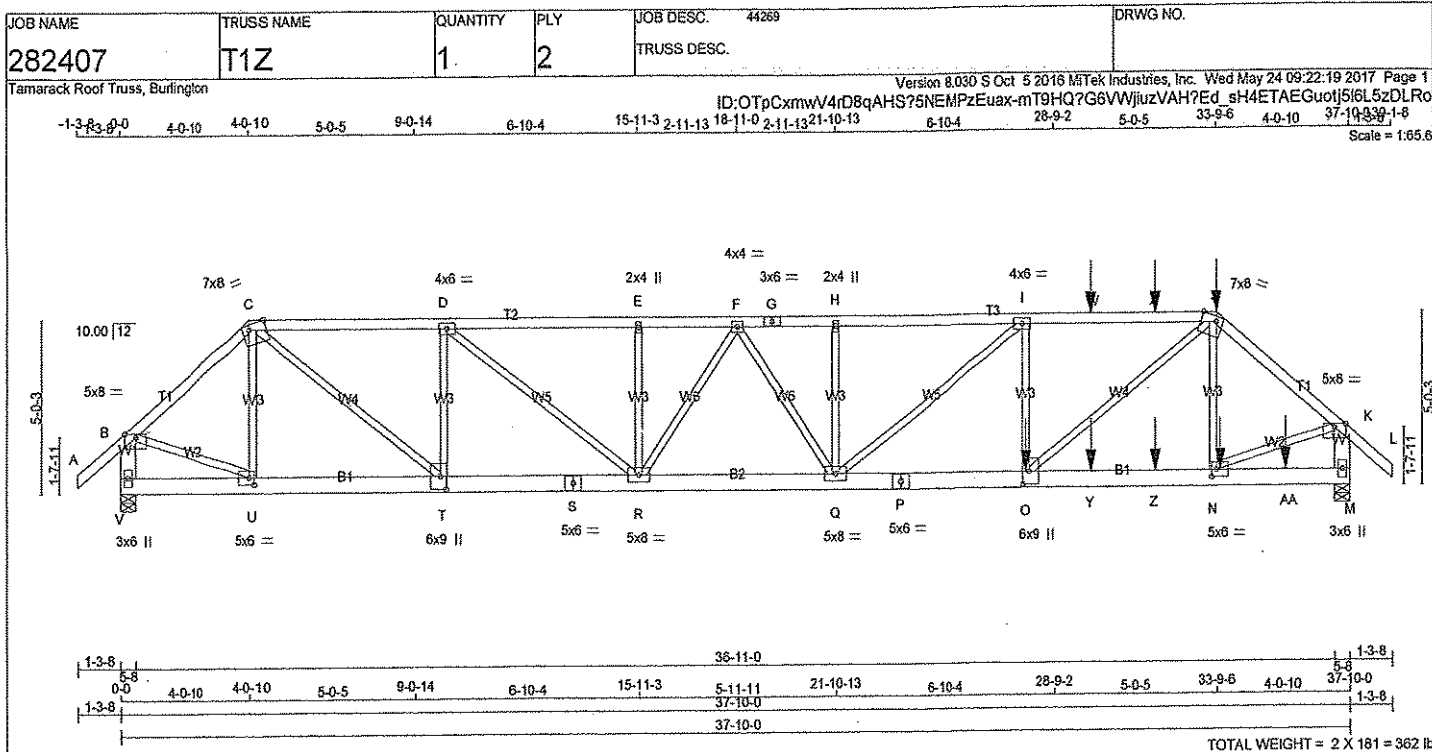
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
N-AO	0/0	-28.0	-28.0	0.08 (2)	10.00		
AO-M	0/0	-28.0	-28.0	0.08 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-10	-206	-206	---	FRONT	VERT	TOTAL
C	4-0-10	-105	-105	---	BACK	VERT	TOTAL
D	9-11-4	-101	-101	---	BACK	VERT	TOTAL
E	15-11-4	-101	-101	---	BACK	VERT	TOTAL
G	19-11-4	-101	-101	---	BACK	VERT	TOTAL
H	21-11-4	-101	-101	---	BACK	VERT	TOTAL
I	27-11-4	-101	-101	---	BACK	VERT	TOTAL
J	33-9-6	-206	-206	---	FRONT	VERT	TOTAL
J	33-9-6	-105	-105	---	BACK	VERT	TOTAL
N	33-11-4	-41	-71	---	BACK	VERT	TOTAL
O	27-11-4	-41	-71	---	BACK	VERT	TOTAL
P	23-11-4	-41	-71	---	BACK	VERT	TOTAL
Q	21-11-4	-41	-71	---	BACK	VERT	TOTAL
R	15-11-4	-41	-71	---	BACK	VERT	TOTAL
S	13-11-4	-41	-71	---	BACK	VERT	TOTAL
T	9-11-4	-41	-71	---	BACK	VERT	TOTAL
U	3-11-4	-41	-71	---	BACK	VERT	TOTAL
W	5-11-4	-101	-101	---	BACK	VERT	TOTAL
X	7-11-4	-101	-101	---	BACK	VERT	TOTAL
Y	11-11-4	-101	-101	---	BACK	VERT	TOTAL
Z	13-11-4	-101	-101	---	BACK	VERT	TOTAL
AA	17-11-4	-101	-101	---	BACK	VERT	TOTAL
AB	23-11-4	-101	-101	---	BACK	VERT	TOTAL
AC	25-11-4	-101	-101	---	BACK	VERT	TOTAL
AD	29-11-4	-101	-101	---	BACK	VERT	TOTAL
AE	31-11-4	-101	-101	---	BACK	VERT	TOTAL
AF	1-11-4	-41	-71	---	BACK	VERT	TOTAL
AG	5-11-4	-41	-71	---	BACK	VERT	TOTAL
AH	7-11-4	-41	-71	---	BACK	VERT	TOTAL
AI	11-11-4	-41	-71	---	BACK	VERT	TOTAL
AJ	17-11-4	-41	-71	---	BACK	VERT	TOTAL
AK	19-11-4	-41	-71	---	BACK	VERT	TOTAL
AL	25-11-4	-41	-71	---	BACK	VERT	TOTAL
AM	29-11-4	-41	-71	---	BACK	VERT	TOTAL
AN	31-11-4	-41	-71	---	BACK	VERT	TOTAL
AO	35-11-4	-41	-71	---	BACK	VERT	TOTAL



DWG NO. TAM 2560217
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
V - B	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
V - S	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-G	12	TOP
G-J	12	SIDE(61.0)
J-L	12	SIDE(61.0)
V-B	2	TOP
M-K	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	2	TOP
S-P	2	TOP
P-M	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(633.7)
1-O	4	
D-T	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TTWW-m	MT20	7.0	8.0	Edge 5.50
D	TMWV-i	MT20	4.0	6.0	
E	TMWV-w	MT20	2.0	4.0	
F	TMWV-i	MT20	4.0	4.0	
G	TS-i	MT20	3.0	6.0	
H	TMWV-t	MT20	2.0	4.0	
I	TMWV-i	MT20	4.0	6.0	
J	TTWW-m	MT20	7.0	8.0	Edge 5.50
K	TMWV-p	MT20	5.0	8.0	Edge
M	BMV1+p	MT20	3.0	6.0	
N	BMWV-i	MT20	5.0	6.0	2.50 1.75
O	BMWV-w	MT20	6.0	9.0	4.50 2.25
P	BS-i	MT20	5.0	6.0	
Q	BMWV-w-i	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG UPLIFT	REQ'D BRG IN-SX
JT VERT	3158	3158	0	5-8
M	5131	5131	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2569	1462 / 0	561 / 0	0 / 0	0 / 0	546 / 0	0 / 0
M	4199	2353 / 0	940 / 0	0 / 0	0 / 0	905 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0 / 37	-84.3	-84.3	0.06 (1)	10.00	U-C	-559 / 0
B-C	-3266 / 0	-84.3	-84.3	0.19 (1)	4.99	C-T	0 / 3686
C-D	-5393 / 0	-84.3	-84.3	0.45 (1)	3.85	T-D	-2108 / 0
D-E	-7382 / 0	-84.3	-84.3	0.62 (1)	3.23	O-I	-415 / 62
E-F	-7382 / 0	-84.3	-84.3	0.32 (1)	3.41	O-J	0 / 5936
F-G	-8572 / 0	-84.3	-84.3	0.37 (1)	3.13	N-J	-895 / 0
G-H	-8572 / 0	-84.3	-84.3	0.37 (1)	3.13	B-U	0 / 2608
H-I	-8572 / 0	-84.3	-84.3	0.82 (1)	2.84	N-K	0 / 4430
I-W	-8906 / 0	-84.3	-84.3	0.88 (1)	2.73	Q-I	-424 / 0
W-X	-8906 / 0	-84.3	-84.3	0.88 (1)	2.73	D-R	0 / 2519
X-J	-8906 / 0	-84.3	-84.3	0.88 (1)	2.73	Q-H	-363 / 0
J-K	-5547 / 0	-84.3	-84.3	0.28 (1)	3.95	R-E	-387 / 0
K-L	0 / 37	-84.3	-84.3	0.08 (1)	10.00	R-F	-1207 / 0
V-B	-3122 / 0	0.0	0.0	0.12 (1)	7.80	F-Q	0 / 991
M-K	-5102 / 0	0.0	0.0	0.19 (1)	6.45		
V-U	0 / 0	-28.0	-28.0	0.05 (2)	10.00		
U-T	0 / 2489	-28.0	-28.0	0.20 (1)	10.00		
T-S	0 / 5394	-28.0	-28.0	0.40 (1)	10.00		
S-R	0 / 5394	-28.0	-28.0	0.40 (1)	10.00		
R-Q	0 / 8035	-28.0	-28.0	0.57 (1)	10.00		
Q-P	0 / 8906	-28.0	-28.0	0.66 (1)	10.00		
P-O	0 / 8906	-28.0	-28.0	0.66 (1)	10.00		
O-Y	0 / 4230	-28.0	-28.0	0.36 (1)	10.00		
Y-Z	0 / 4230	-28.0	-28.0	0.36 (1)	10.00		
Z-N	0 / 4230	-28.0	-28.0	0.36 (1)	10.00		
N-AA	0 / 0	-28.0	-28.0	0.10 (2)	10.00		
AA-M	0 / 0	-28.0	-28.0	0.10 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE
N	33-9-6	-311	-311	---	FRONT	VERT
J	33-10-12	-41	-71	---	FRONT	VERT
O	27-10-8	-3131	-3131	---	FRONT	VERT
W	29-10-12	-101	-101	---	FRONT	VERT
X	31-10-12	-101	-101	---	FRONT	VERT
Y	29-10-12	-41	-71	---	FRONT	VERT
Z	31-10-12	-41	-71	---	FRONT	VERT
AA	35-10-12	-41	-71	---	FRONT	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 46.1	PSF	

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/976 (0.47")

CSI: TC=0.88 (I-J:1), BC=0.68 (O-Q:1), WB=0.73 (J-Q:1), SSI=0.18 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

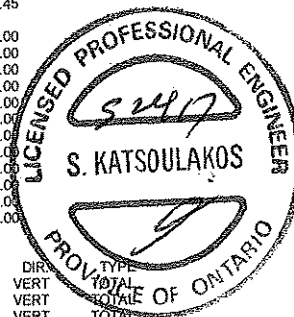
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.72 (S) (INPUT = 1.00)



DRWG NO. TAM25600217

STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 282407	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. 44289 TRUSS DESC.	DRWG NO.
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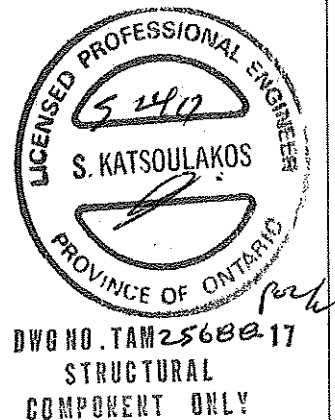
Version 8.030 S Oct 5 2016 M Tek Industries, Inc. Wed May 24 09:22:19 2017 Page 2
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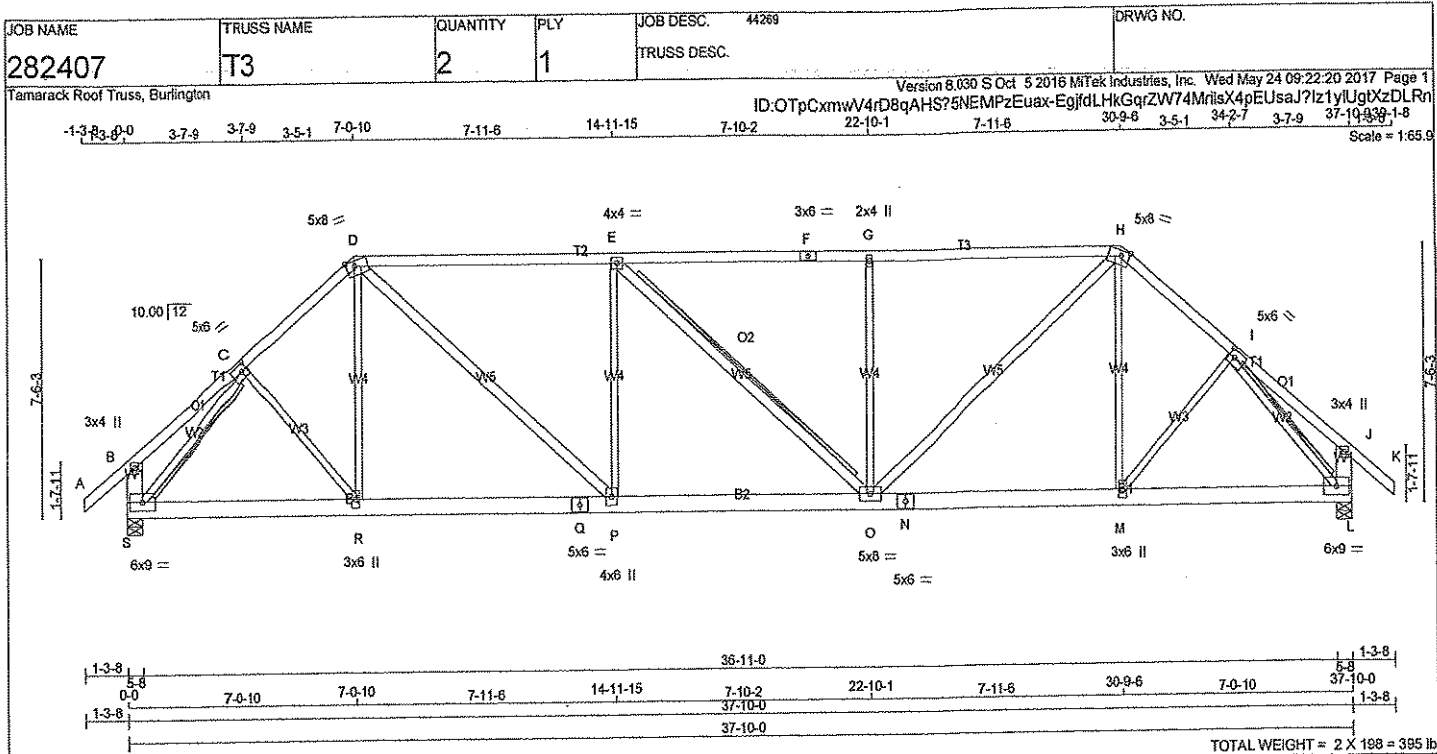
PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
R	BMWWH	MT20	5.0	8.0	
S	BS-I	MT20	5.0	8.0	
T	BMWWH	MT20	6.0	9.0	4.50 2.25
U	BMWWH	MT20	5.0	8.0	2.50 1.75
V	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 310.6 lbs FACTORED DOWN AT 33-9-6, AND 101.4 lbs FACTORED DOWN AT 29-10-12, AND 101.4 lbs FACTORED DOWN AT 31-10-12 ON TOP CHORD, AND 3130.9 lbs FACTORED DOWN AT 27-10-8, 71.3 lbs FACTORED DOWN AT 29-10-12, 71.3 lbs FACTORED DOWN AT 31-10-12, AND 71.3 lbs FACTORED DOWN AT 33-10-12, AND 71.3 lbs FACTORED DOWN AT 35-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY 1650F 1.5E	SPF
D - F	2x4	DRY 1650F 1.5E	SPF
F - H	2x4	DRY 1650F 1.5E	SPF
H - K	2x4	DRY 1650F 1.5E	SPF
S - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
S - Q	2x6	DRY No.2	SPF
Q - N	2x6	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
D - P	2x4	DRY No.2	SPF
E - O	2x4	DRY No.2	SPF
O - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	6.0	9.0		
M	BMVW1-t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMVW1-t	MT20	5.0	8.0		
P	BMVW1-t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMVW1-t	MT20	3.0	6.0		
S	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	2240	0	2240	0	0	5-8	5-8
L	2240	0	2240	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
S	1823	1039 / 0	397 / 0	0 / 0	0 / 0	397 / 0	0 / 0
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-O
2x3 DRY SPF No.2 T-BRACE AT C-S, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD C1 (LC)	MAX. UNBRAC CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.10 (1)	10.00	C-R	0 / 200	0.05 (2)	
B-C	0 / 19	-84.3	-84.3	0.12 (1)	10.00	R-D	0 / 271	0.06 (3)	
C-D	-2279 / 0	-84.3	-84.3	0.16 (1)	4.96	D-P	0 / 1408	0.23 (1)	
D-E	-2781 / 0	-84.3	-84.3	0.91 (1)	3.77	P-E	-720 / 0	0.69 (1)	
E-F	-2778 / 0	-84.3	-84.3	0.90 (1)	3.77	E-O	-5 / 0	0.00 (1)	
F-G	-2778 / 0	-84.3	-84.3	0.90 (1)	3.77	O-G	-721 / 0	0.69 (1)	
G-H	-2778 / 0	-84.3	-84.3	0.90 (1)	3.78	H-O	0 / 1402	0.23 (1)	
H-I	-2280 / 0	-84.3	-84.3	0.16 (1)	4.96	M-H	0 / 272	0.06 (3)	
I-J	0 / 19	-84.3	-84.3	0.12 (1)	10.00	M-I	0 / 200	0.05 (2)	
J-K	0 / 37	-84.3	-84.3	0.10 (1)	10.00	S-C	-2521 / 0	0.93 (1)	
S-B	-234 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-2522 / 0	0.93 (1)	
L-J	-234 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 1628	-28.0	-28.0	0.31 (2)	10.00				
R-Q	0 / 1732	-28.0	-28.0	0.33 (2)	10.00				
Q-P	0 / 1732	-28.0	-28.0	0.33 (2)	10.00				
P-O	0 / 2781	-28.0	-28.0	0.41 (1)	10.00				
O-N	0 / 1733	-28.0	-28.0	0.33 (2)	10.00				
N-M	0 / 1733	-28.0	-28.0	0.33 (2)	10.00				
M-L	0 / 1629	-28.0	-28.0	0.31 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.15")
ALLOWABLE DEFL.(TL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.25")

CSI: TC=0.91 (D-E:1), BC=0.41 (O-P:1), WB=0.93 (I-L:1), SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

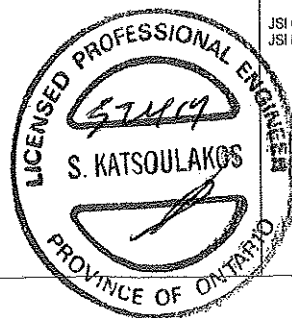
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLF) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

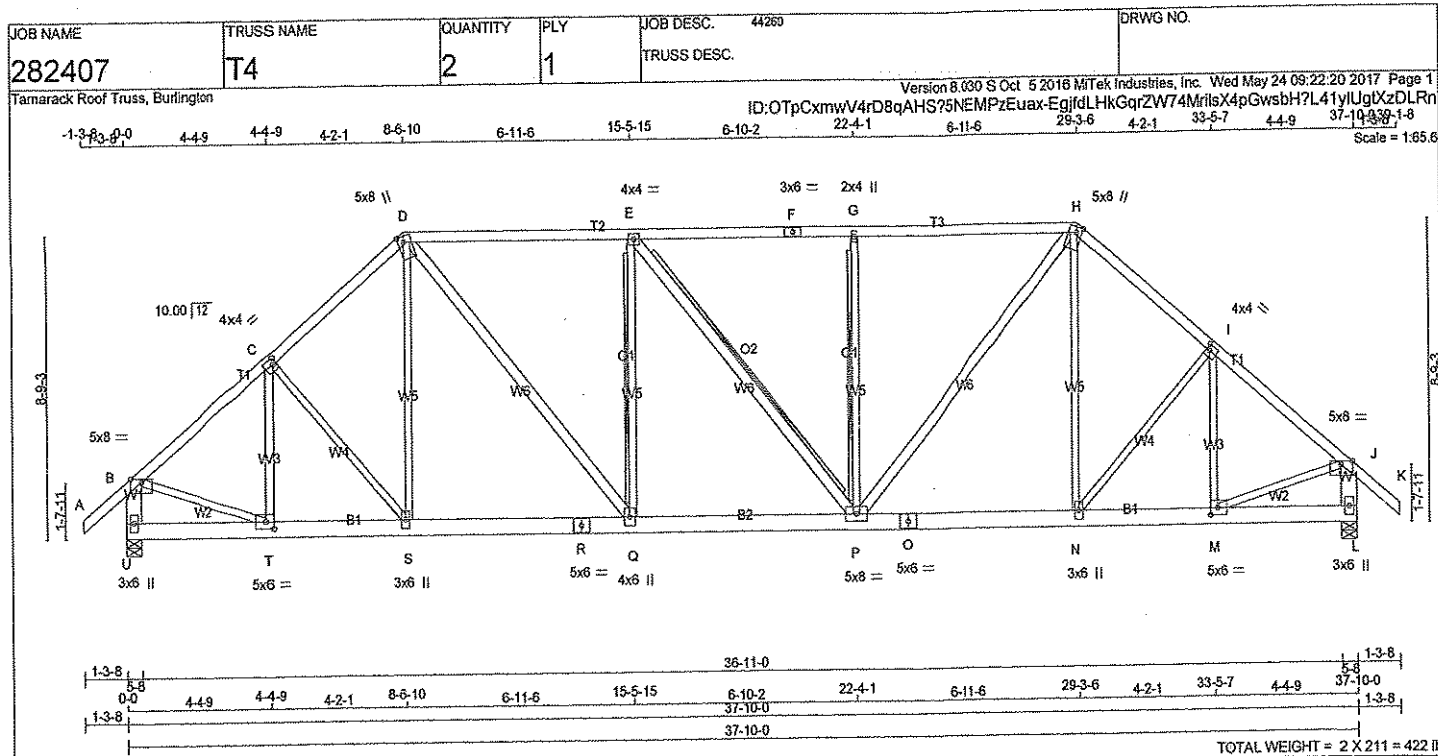
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)



DRWG NO. TAM25690-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
U - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
U - R	2x6	DRY No.2	SPF
R - O	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
D - Q	2x4	DRY No.2	SPF
E - P	2x4	DRY No.2	SPF
P - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	8.0	2.00	2.00
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	8.0	2.00	2.00
I	TMVW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.50
N	BMVW-t	MT20	3.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMVW-t	MT20	3.0	6.0		
T	BMVW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORIZ	FACTORED		MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
			DOWN	UPLIFT			
U	2240	0	2240	0	5-8	5-8	
L	2240	0	2240	0	5-8	5-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-Q, G-P
2x4 DRY SPF No.2 T-BRACE AT E-P

FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. LC1
FR-TO						FR-TO			
A-B	0 / 37		-84.3	-84.3	0.12 (1)	10.00	T-C	-388 / 0	0.15 (1)
B-C	-2215 / 0		-84.3	-84.3	0.36 (1)	4.27	C-S	-51 / 0	0.04 (1)
C-D	-2230 / 0		-84.3	-84.3	0.35 (1)	4.25	S-D	0 / 304	0.07 (2)
D-E	-2374 / 0		-84.3	-84.3	0.82 (1)	3.59	D-Q	0 / 1071	0.17 (1)
E-F	-2371 / 0		-84.3	-84.3	0.81 (1)	3.60	Q-E	-827 / 0	0.73 (1)
F-G	-2371 / 0		-84.3	-84.3	0.81 (1)	3.60	E-P	-8 / 0	0.00 (1)
G-H	-2371 / 0		-84.3	-84.3	0.81 (1)	3.62	P-G	-829 / 0	0.73 (1)
H-I	-2231 / 0		-84.3	-84.3	0.35 (1)	4.25	P-H	0 / 1064	0.17 (1)
I-J	-2215 / 0		-84.3	-84.3	0.36 (1)	4.27	N-H	0 / 307	0.07 (2)
J-K	0 / 37		-84.3	-84.3	0.12 (1)	10.00	N-I	-49 / 0	0.04 (1)
U-B	-2175 / 0		0.0	0.0	0.15 (1)	6.91	M-I	-388 / 0	0.15 (1)
L-J	-2175 / 0		0.0	0.0	0.15 (1)	6.91	B-T	0 / 1787	0.40 (1)
						M-J	0 / 1787	0.40 (1)	
U-T	0 / 0		-28.0	-28.0	0.07 (2)	10.00			
T-S	0 / 1722		-28.0	-28.0	0.26 (1)	10.00			
S-R	0 / 1691		-28.0	-28.0	0.27 (2)	10.00			
R-Q	0 / 1691		-28.0	-28.0	0.27 (2)	10.00			
Q-P	0 / 2374		-28.0	-28.0	0.35 (1)	10.00			
P-O	0 / 1692		-28.0	-28.0	0.27 (2)	10.00			
O-N	0 / 1692		-28.0	-28.0	0.27 (2)	10.00			
N-M	0 / 1721		-28.0	-28.0	0.27 (1)	10.00			
M-L	0 / 0		-28.0	-28.0	0.07 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.82 (D-E:1), BC=0.35 (P-Q:1), WB=0.73 (G-P:1), SSI=0.27 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

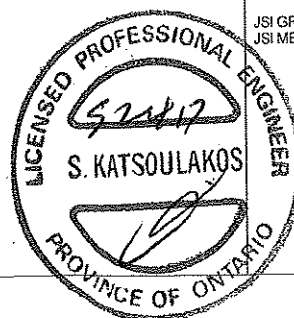
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

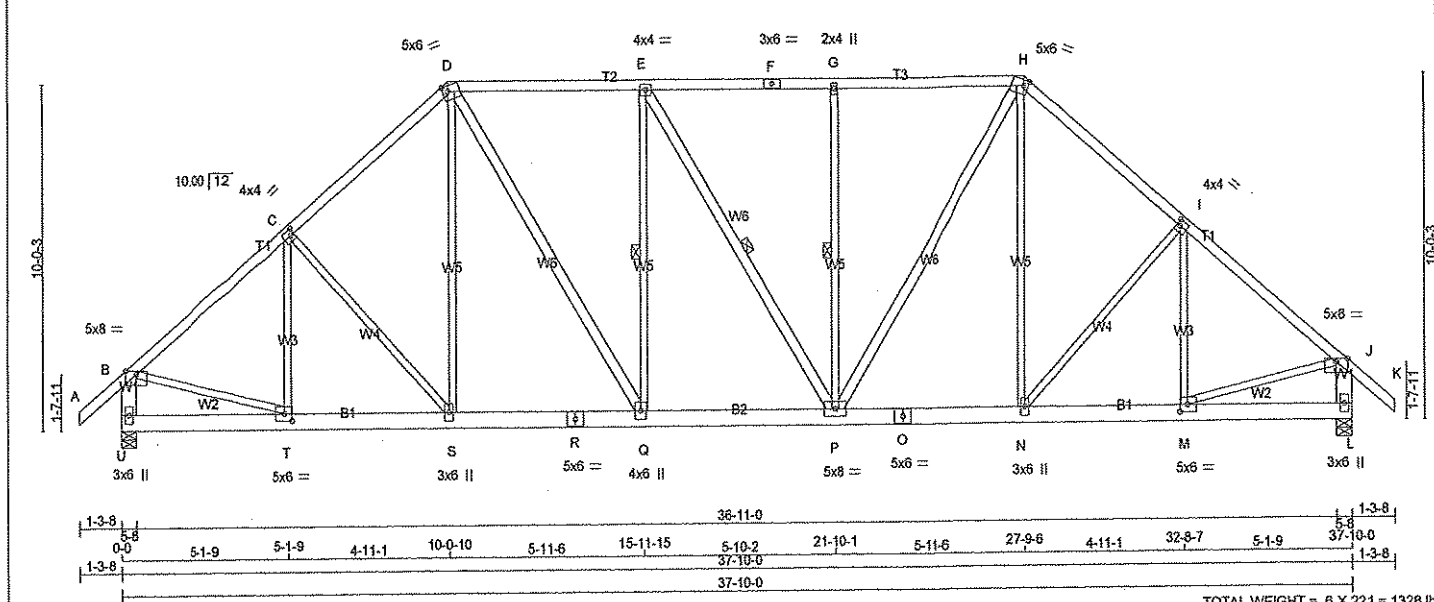
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (T) (INPUT = 0.90)
JSI METAL= 0.43 (T) (INPUT = 1.00)



DWG NO. TAM25691-17
STRUCTURAL
COMPONENT ONLY



LUMBER N. L. G. A. RULES				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS				BEARINGS				SPECIFIED LOADS:			
A - D	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D	TOP CH.	LL	= 25.6 PSF
D - F	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL	= 3.0 PSF	
F - H	2x4	DRY	No.2	SPF	JT VERT	HORZ	DOWN	HORZ	BOT CH.	LL	= 10.5 PSF
H - K	2x4	DRY	No.2	SPF	U	2240	0	2240	DL	= 7.0 PSF	
U - B	2x6	DRY	No.2	SPF	L	2240	0	2240	TOTAL LOAD	= 46.1 PSF	
L - J	2x6	DRY	No.2	SPF							
U - R	2x6	DRY	No.2	SPF							
R - O	2x6	DRY	No.2	SPF							
O - L	2x6	DRY	No.2	SPF							
ALL WEBS 2x3 DRY No.2				SPF							
EXCEPT											
D - Q	2x4	DRY	No.2	SPF							
E - P	2x4	DRY	No.2	SPF							
P - H	2x4	DRY	No.2	SPF							
DRY: SEASONED LUMBER.											
PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN	Y	X					
B	TMVW-p	MT20	5.0	8.0	Edge						
C	TMWW-I	MT20	4.0	4.0	2.00	1.25					
D	TTWW-m	MT20	5.0	6.0	2.00	1.75					
E	TMWW-I	MT20	4.0	4.0							
F	TS-I	MT20	3.0	6.0							
G	TMW+ w	MT20	2.0	4.0							
H	TTWW-m	MT20	5.0	6.0	2.00	1.75					
I	TMWW-I	MT20	4.0	4.0	2.00	1.25					
J	TMVW-p	MT20	5.0	8.0	Edge						
L	BMV1+p	MT20	3.0	6.0							
M	BMWW-I	MT20	5.0	6.0	2.50	2.75					
N	BMWW+I	MT20	3.0	6.0							
O	BS-I	MT20	5.0	6.0							
P	BMWWWW-I	MT20	5.0	8.0							
Q	BMWW+H	MT20	4.0	6.0							
R	BS-I	MT20	5.0	6.0							
S	BMWW+I	MT20	3.0	6.0							
T	BMWW-I	MT20	5.0	6.0	2.50	2.75					
U	BMV1+p	MT20	3.0	6.0							
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.											

UNFACTORED REACTIONS											
1ST LCASE MAX/MIN. COMPONENT REACTIONS											
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
U	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0				
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0				
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L											
BRACING											
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.23 FT.											
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.											
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.											
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, E-P, G-P.											
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW											
LOADING											
TOTAL LOAD CASES: (4)											
CHORDS											
MEMB. MAX. FACTORED FORCE (LBS)											
VERT. LOAD LC1 MAX (PLF)											
CSH (LC)											
WEBS											
MEMB. MAX. FACTORED FORCE (LBS)											
MAX CSH (LC)											
FR-TO											
T-C											
C-S											
S-D											
D-Q											
Q-E											
E-P											
P-G											
G-H											
H-I											
I-J											
J-K											
U-B											
L-J											
U-T											
T-S											
S-R											
R-Q											
Q-P											
P-O											
O-N											
N-M											
M-L											

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD	=	46.1 PSF	
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
- CSA 088-09			
- TPIC 2011			
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (1.26")			
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")			
ALLOWABLE DEFL.(TL)= L/360 (1.26")			
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")			
CSI: TC=0.41 (D-E-1), BC=0.30 (P-Q-1), WB=0.41 (B-T-1), SSI=0.23 (D-E-1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
COMP=1.10 SHEAR=1.10 TENS=1.10			
COMPANION LIVE LOAD FACTOR = 0.50			
AUTOSOLVE HEELS OFF			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 822 2284 1656
PLATE PLACEMENT TOL = 0.250 inches			
PLATE ROTATION TOL = 5.0 Deg.			
JSI GRIP= 0.88 (H) (INPUT = 0.90)			
JSI METAL= 0.44 (T) (INPUT = 1.00)			

LICENSED PROFESSIONAL ENGINEER

52412

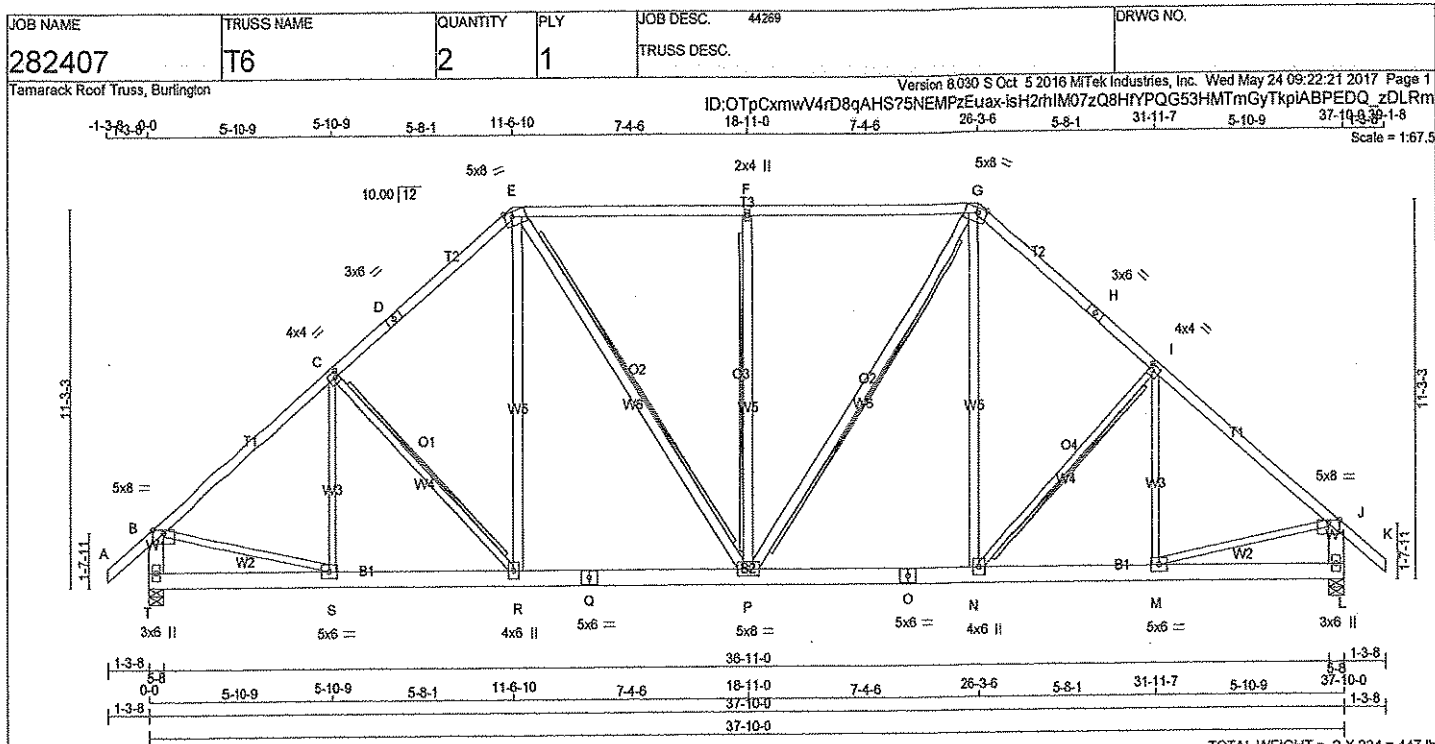
S. KATSOUKAKOS

PROVINCE OF ONTARIO

DRWG NO. TAM 25692-1

STRUCTURAL

COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
T	2240	0	2240	0	5-8	5-8
L	2240	0	2240	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
T	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0
L	1823	1039 / 0	397 / 0	0 / 0	0 / 0	387 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-R, E-P, G-P, I-N
2x4 DRY SPF No.2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	S-C	-212 / 119	0.14 (1)	
B-C	-2288 / 0	-84.3	-84.3	0.52 (1)	4.07	C-R	-308 / 0	0.36 (1)	
C-D	-2090 / 0	-84.3	-84.3	0.47 (1)	4.25	R-E	0 / 471	0.08 (2)	
D-E	-2090 / 0	-84.3	-84.3	0.47 (1)	4.25	E-P	0 / 583	0.09 (1)	
E-F	-1906 / 0	-84.3	-84.3	0.68 (1)	4.02	P-F	-762 / 0	0.64 (1)	
F-G	-1906 / 0	-84.3	-84.3	0.68 (1)	4.02	P-G	0 / 593	0.09 (1)	
G-H	-2090 / 0	-84.3	-84.3	0.47 (1)	4.25	N-G	0 / 471	0.08 (2)	
H-I	-2090 / 0	-84.3	-84.3	0.47 (1)	4.25	N-I	-308 / 0	0.36 (1)	
I-J	-2288 / 0	-84.3	-84.3	0.52 (1)	4.07	M-I	-212 / 119	0.14 (1)	
J-K	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B-S	0 / 1823	0.41 (1)	
T-B	-2164 / 0	0.0	0.0	0.15 (1)	6.93	M-J	0 / 1823	0.41 (1)	
L-J	-2164 / 0	0.0	0.0	0.15 (1)	6.93				
T-S	0 / 0	-28.0	-28.0	0.10 (2)	10.00				
S-R	0 / 1785	-28.0	-28.0	0.28 (1)	10.00				
R-Q	0 / 1578	-28.0	-28.0	0.29 (2)	10.00				
Q-P	0 / 1578	-28.0	-28.0	0.29 (2)	10.00				
P-O	0 / 1578	-28.0	-28.0	0.29 (2)	10.00				
O-N	0 / 1578	-28.0	-28.0	0.29 (2)	10.00				
N-M	0 / 1785	-28.0	-28.0	0.28 (1)	10.00				
M-L	0 / 0	-28.0	-28.0	0.10 (2)	10.00				



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00 1.25	
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge 3.00	
F	TMW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge 3.00	
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0	2.00 1.25	
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMVW-t	MT20	4.0	6.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (1.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CS1: TC=0.68 (E-F:1), BC=0.29 (P-R:2), WB=0.64 (F-P:1), SS1=0.30 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

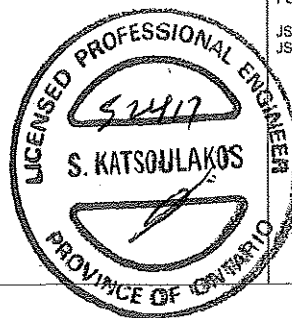
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667	822 2284 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.45 (S) (INPUT = 1.00)



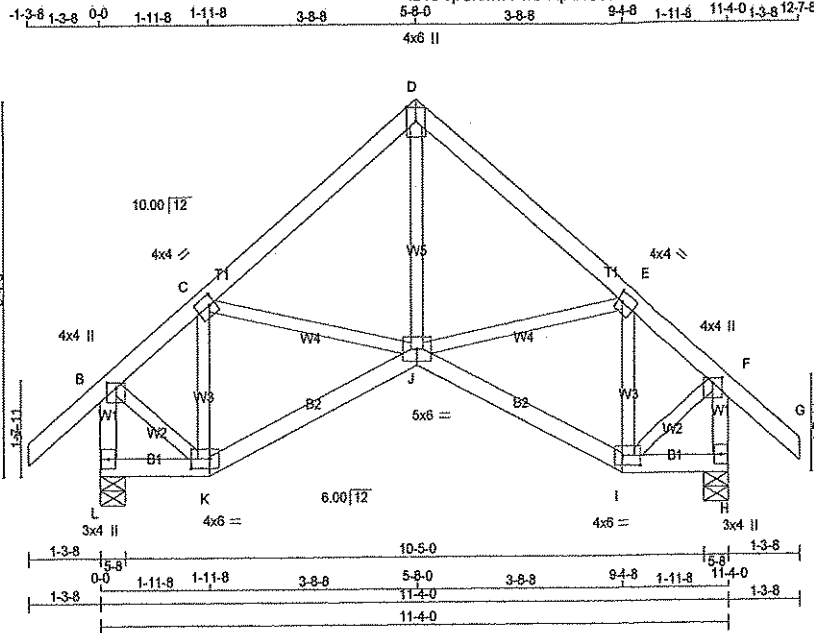
DRWG NO. TAM25693-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44269	DRWG NO.
282407	T7S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed May 24 09:22:22 2017 Page 1

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Scale = 1:38.4

TOTAL WEIGHT = 2 X 55 = 109 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00	
C	TMVW-i	MT20	4.0	4.0	2.00	1.25	
D	TTW+p	MT20	4.0	6.0	Edge		
E	TMVW-i	MT20	4.0	4.0	2.00	1.25	
F	TMVW+p	MT20	4.0	4.0	1.00	2.00	
H	BMV1+p	MT20	3.0	4.0			
I	BBVW+p	MT20	4.0	6.0	2.00	4.00	
J	BBVW-i	MT20	5.0	6.0			
K	BBVW-p	MT20	4.0	6.0	2.00	4.00	
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	SNOW	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	753	0	753	0	0	0	5-8	5-8	5-8
H	753	0	753	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	COMBINED	SOIL
L	601	361/0	119/0	0/0	0/0	122/0	0/0	601	0/0
H	601	361/0	119/0	0/0	0/0	122/0	0/0	601	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC1)	MEMB.	FORCE (LBS)	MAX. (LC1)	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0/37	-84.3	-84.3 0.12 (1)	10.00	J-D	0/418	0.09 (1)
B-C	-463/0	-84.3	-84.3 0.10 (1)	6.25	J-E	0/69	0.02 (1)
C-D	-555/0	-84.3	-84.3 0.15 (1)	6.25	I-E	-333/0	0.08 (1)
D-E	-555/0	-84.3	-84.3 0.15 (1)	6.25	C-J	0/69	0.02 (1)
E-F	-463/0	-84.3	-84.3 0.10 (1)	6.25	K-C	-333/0	0.08 (1)
F-G	0/37	-84.3	-84.3 0.12 (1)	10.00	B-K	0/431	0.10 (1)
L-B	-725/0	0.0	0.0 0.08 (1)	7.81	I-F	0/431	0.10 (1)
H-F	-725/0	0.0	0.0 0.08 (1)	7.81			
L-K	0/0	-28.0	-28.0 0.03 (2)	10.00			
K-J	0/388	-28.0	-28.0 0.16 (2)	10.00			
J-I	0/388	-28.0	-28.0 0.16 (2)	10.00			
I-H	0/0	-28.0	-28.0 0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.38")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.38")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.15 (D-E:1), BC=0.16 (J-K:2), WB=0.10 (B-K:1), SSI=0.11 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

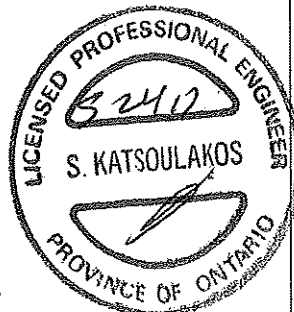
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1658

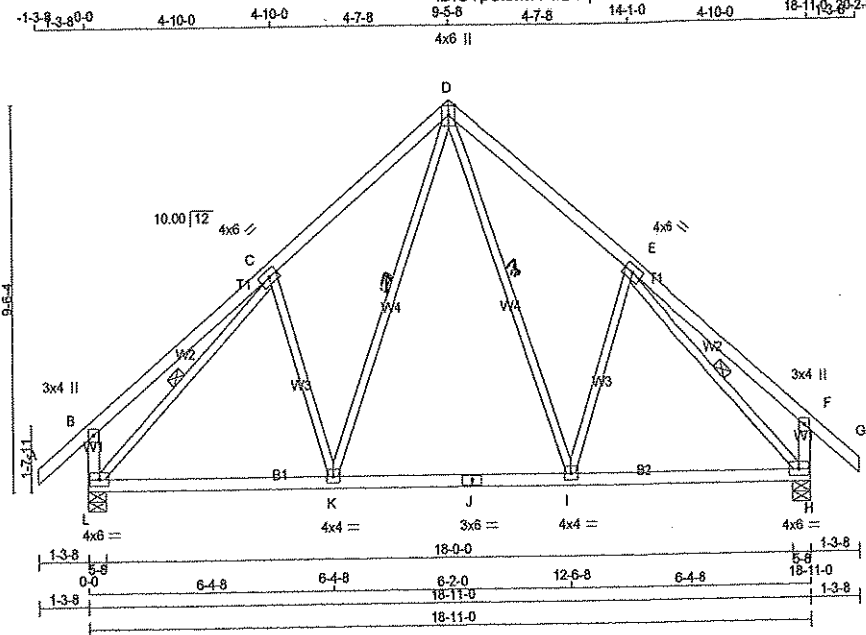
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.69 (I) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)



DRWG NO. TAM25694-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 91 = 183 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW-1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1178 0	1178 0	5-8	5-8
H	1178 0	1178 0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	951	555/0	199/0	0/0	0/0	197/0	0/0
H	951	555/0	199/0	0/0	0/0	197/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H. *0-5-0-14*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/37	-84.3 -84.3	0.12 (1)	10.00	D-I	0/437	0.10 (1)
B-C	0/30	-84.3 -84.3	0.31 (1)	10.00	I-E	-233/47	0.12 (1)
C-D	-918/0	-84.3 -84.3	0.25 (1)	6.15	K-D	0/437	0.10 (1)
D-E	-918/0	-84.3 -84.3	0.25 (1)	6.15	C-K	-233/47	0.12 (1)
E-F	0/30	-84.3 -84.3	0.31 (1)	10.00	L-C	-1128/0	0.37 (1)
F-G	0/37	-84.3 -84.3	0.12 (1)	10.00	E-H	-1128/0	0.37 (1)
L-B	-267/0	0.0 0.0	0.03 (1)	7.81			
H-F	-267/0	0.0 0.0	0.03 (1)	7.81			
L-K	0/754	-28.0 -28.0	0.34 (2)	10.00			
K-J	0/543	-28.0 -28.0	0.32 (2)	10.00			
J-I	0/543	-28.0 -28.0	0.32 (2)	10.00			
I-H	0/754	-28.0 -28.0	0.34 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 240 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.07")

ALLOWABLE DEFL.(TL)= L/360 (0.63")

CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CSI: TC=0.31 (E-F:1), BC=0.34 (H-I:2), WB=0.37 (E-H:1), SSI=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

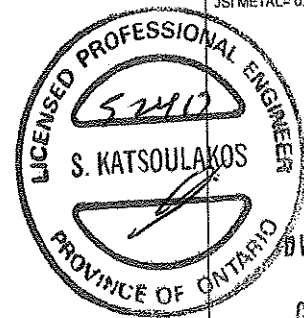
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

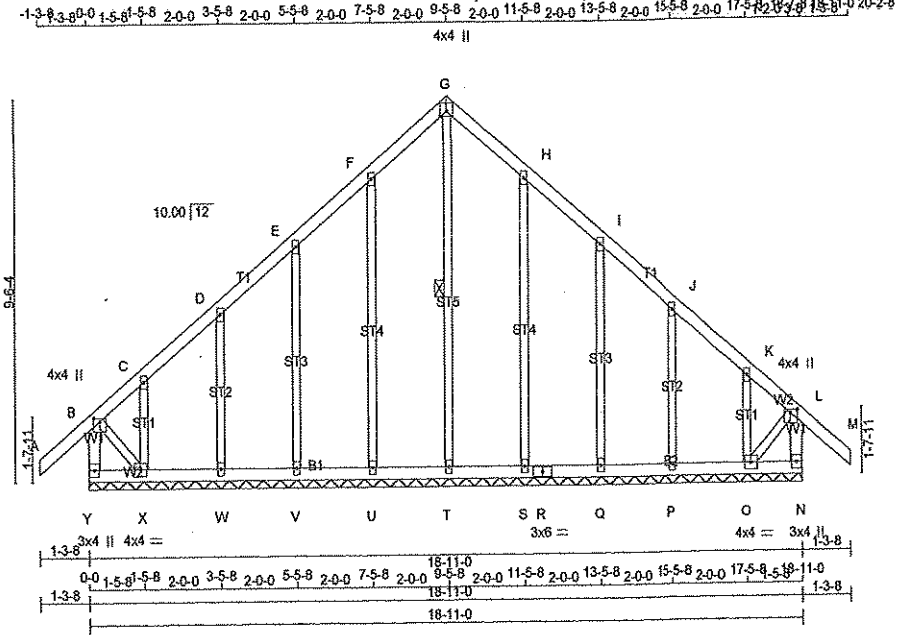
JSI GRIP= 0.88 (C) (INPUT = 0.90)

JSI METAL= 0.30 (E) (INPUT = 1.00)



DRWG NO. TAM 25695-17

STRUCTURAL COMPONENT ONLY



TOTAL WEIGHT = 98 lb (M)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF		
G - M	2x4	DRY	No.2	SPF		
Y - B	2x4	DRY	No.2	SPF		
N - L	2x4	DRY	No.2	SPF		
Y - R	2x4	DRY	No.2	SPF		
R - N	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
 ALL GABLE WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.
 GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMVW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P, Q, S, T, U, V, W						
R	BMV1+w	MT20	2.0	4.0		
BS-I		MT20	3.0	6.0		
X	BMVW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

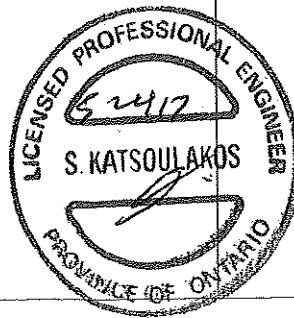
BEARINGS
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
 1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

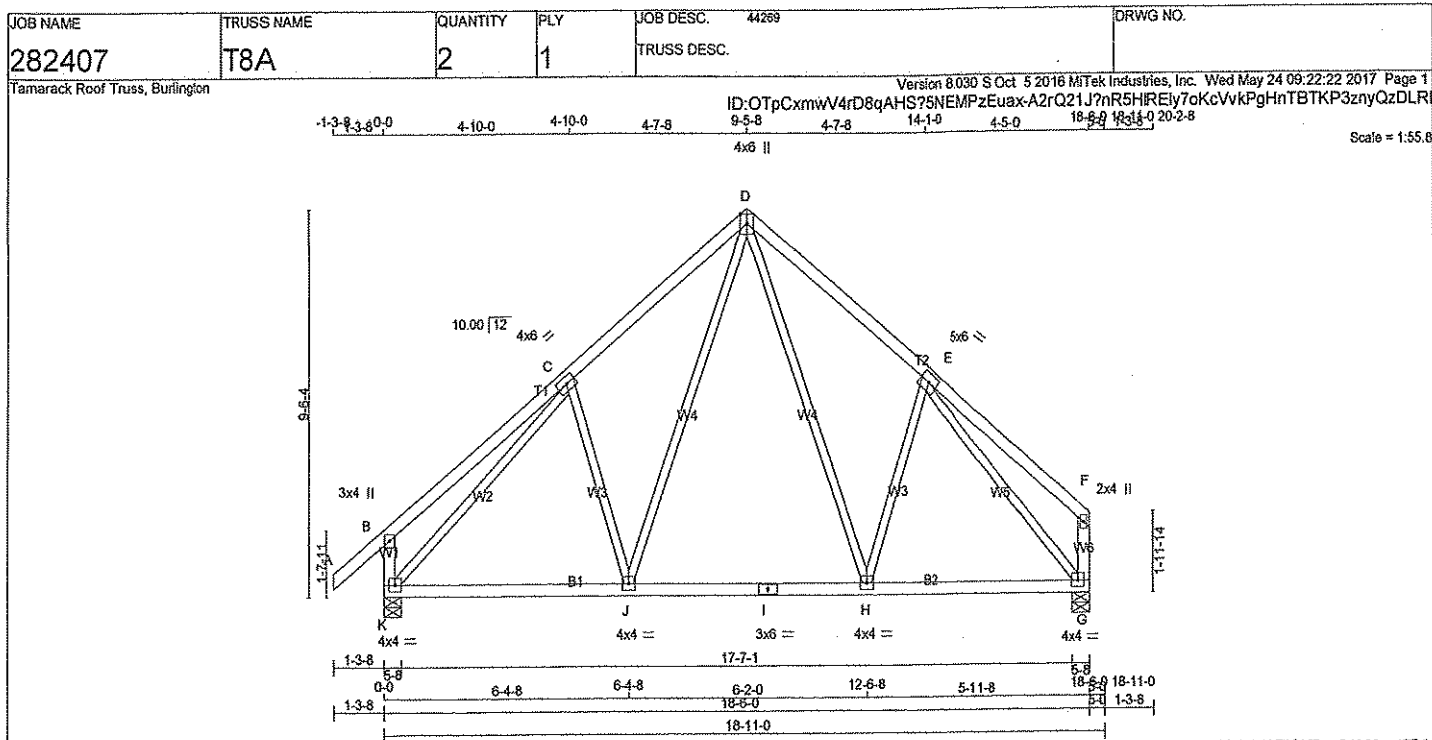
MEMB.	CHORDS		FACTORED		MAX. FACTORED	WEBS	FACTORED		MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	LC2			MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO						FR-TO			
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	T-G	-122/0	0.07 (1)	
B-C	-65/0	-84.3	-84.3	0.11 (1)	6.25	U-F	-192/0	0.22 (1)	
C-D	-20/0	-84.3	-84.3	0.04 (1)	6.25	V-E	-159/0	0.10 (1)	
D-E	-24/0	-84.3	-84.3	0.04 (1)	6.25	W-D	-180/0	0.05 (1)	
E-F	-16/0	-84.3	-84.3	0.05 (1)	6.25	X-C	-76/0	0.01 (1)	
F-G	-27/0	-84.3	-84.3	0.05 (1)	6.25	S-H	-192/0	0.22 (1)	
G-H	-27/0	-84.3	-84.3	0.05 (1)	6.25	Q-I	-159/0	0.10 (1)	
H-I	-16/0	-84.3	-84.3	0.05 (1)	6.25	O-K	-76/0	0.01 (1)	
I-J	-24/0	-84.3	-84.3	0.04 (1)	6.25	B-X	0/32	0.01 (1)	
J-K	-20/0	-84.3	-84.3	0.04 (1)	6.25	O-L	0/32	0.01 (1)	
K-L	-65/0	-84.3	-84.3	0.11 (1)	6.25				
L-M	0/37	-84.3	-84.3	0.12 (1)	10.00				
Y-B	-268/0	0.0	0.0	0.03 (1)	7.81				
N-L	-268/0	0.0	0.0	0.03 (1)	7.81				
Y-X	0/0	-28.0	-28.0	0.02 (2)	10.00				
X-W	0/21	-28.0	-28.0	0.02 (2)	10.00				
W-V	0/17	-28.0	-28.0	0.02 (2)	10.00				
V-U	0/15	-28.0	-28.0	0.02 (2)	10.00				
U-T	0/12	-28.0	-28.0	0.02 (2)	10.00				
T-S	0/12	-28.0	-28.0	0.02 (2)	10.00				
S-R	0/15	-28.0	-28.0	0.02 (2)	10.00				
R-Q	0/15	-28.0	-28.0	0.02 (2)	10.00				
Q-P	0/17	-28.0	-28.0	0.02 (2)	10.00				
P-O	0/21	-28.0	-28.0	0.02 (2)	10.00				
O-N	0/0	-28.0	-28.0	0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF
SPACING = 24.0 IN. C/C
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
 THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011
DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.
 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD
 CSI: TC=0.12 (L-M:1), BC=0.02 (W-X:2), WB=0.22 (H-S:1), SSI=0.07 (L-M:1)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10
 COMPANION LIVE LOAD FACTOR = 0.50
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 622 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.81 (G) (INPUT = 0.80)
 JSI METAL= 0.05 (H) (INPUT = 1.00)



DRWG NO. TAM 25696-17
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
G - F	2x4	DRY	No.2

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMWW+w	MT20	2.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UP
K	1147	0	1147	0
G	1038	0	1038	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
K	925	540 / 0	193 / 0	0 / 0	0 / 0	192 / 0	0 / 0
G	851	474 / 0	193 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 37	-84.3	-84.3	D-H	0 / 340	0.08 (1)	0.08 (1)
B-C	0 / 30	-84.3	-84.3	H-E	-149 / 78	0.08 (1)	0.08 (1)
C-D	-879 / 0	-84.3	-84.3	J-D	0 / 448	0.10 (1)	0.10 (1)
D-E	-832 / 0	-84.3	-84.3	C-J	-240 / 44	0.13 (1)	0.13 (1)
E-F	0 / 40	-84.3	-84.3	K-C	-1085 / 0	0.99 (1)	0.99 (1)
K-B	-267 / 0	0.0	0.0	E-G	-1082 / 0	0.89 (1)	0.89 (1)
				G-F	-124 / 0	0.01 (1)	0.01 (1)
K-J	0 / 726	-28.0	-28.0				
J-I	0 / 510	-28.0	-28.0				
I-H	0 / 510	-28.0	-28.0				
H-G	0 / 685	-28.0	-28.0				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.31 (B-C:1), BC=0.35 (J-K:2), WB=0.99 (C-K:1), SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

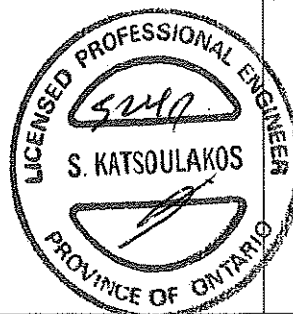
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

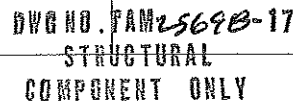
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)



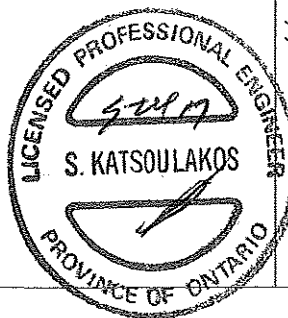
DWG NO. TAM 28697-17
STRUCTURAL
COMPONENT ONLY

Scale = 1:50.6

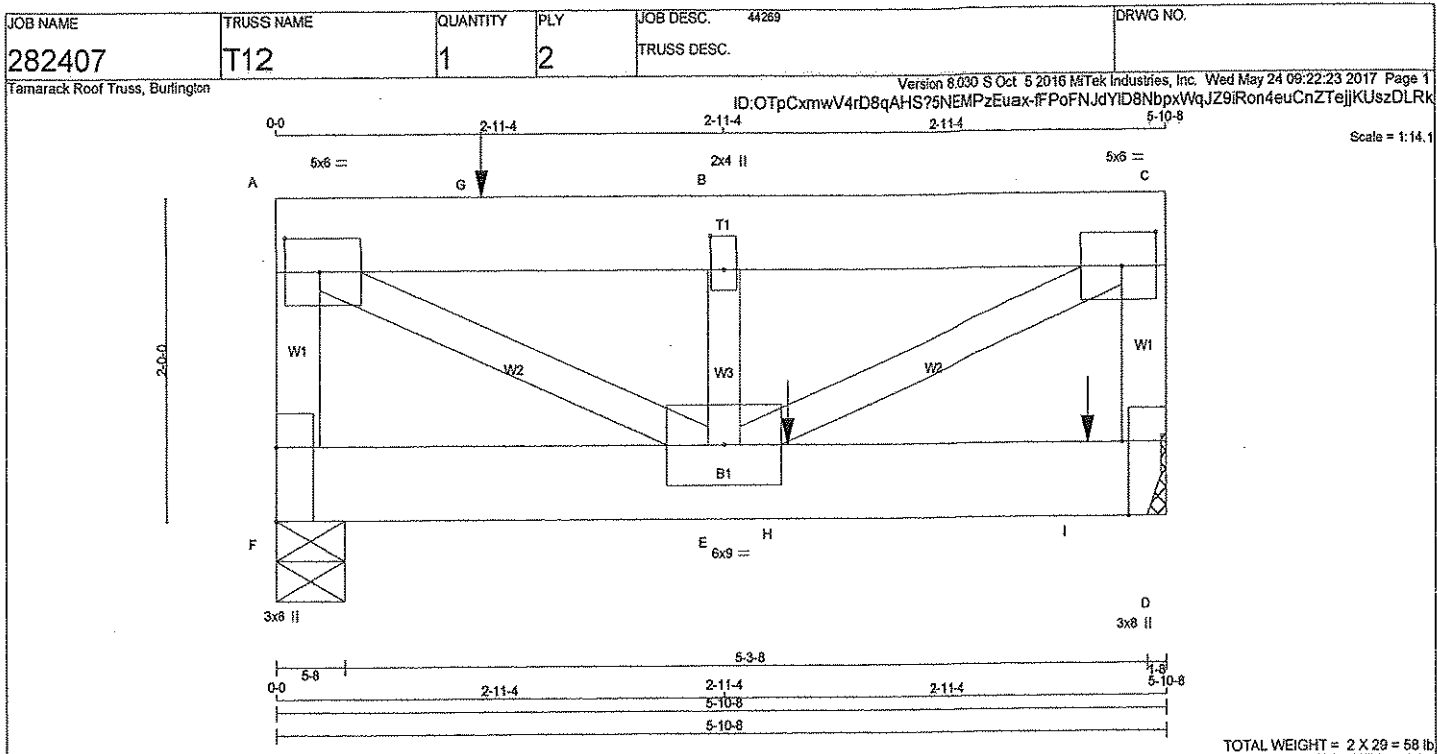
JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.47 (G) (INPUT = 1.00)



Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.		CHORDS				WEBS				DOL NUMBER=1.00 LBS NAIL=1.00 LBS NUT=1.00 COMP=1.10 SHEAR=1.10 TENS=1.10						
		MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	COMPANION LIVE LOAD FACTOR = 0.50						
FR-TO				FROM TO			FR-TO			TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .						
A-B	0 / 37			-84.3	-84.3	0.12 (1)	10.00	N-C	-201 / 75	0.09 (1)	NAIL VALUES					
B-C	-1551 / 0			-84.3	-84.3	0.26 (1)	5.04	C-M	-217 / 0	0.20 (1)	PLATE GRIP(DRY) SHEAR SECTION					
C-D	-1427 / 0			-84.3	-84.3	0.26 (1)	5.21	M-D	0 / 368	0.08 (2)	(PSI) (PLI) (PLI)					
D-E	-1237 / 0			-84.3	-84.3	0.41 (1)	5.26	D-L	0 / 294	0.05 (1)	MAX MIN MAX MIN MAX MIN					
E-F	-1237 / 0			-84.3	-84.3	0.41 (1)	5.26	L-E	-620 / 0	0.40 (1)	MT20 618 354 1667 822 2284 1656					
F-G	-1171 / 0			-84.3	-84.3	0.12 (1)	5.77	L-F	0 / 642	0.10 (1)	PLATE PLACEMENT TOL. = 0.250 inches					
G-H	0 / 19			-84.3	-84.3	0.14 (1)	10.00	J-F	-94 / 124	0.06 (1)	PLATE ROTATION TOL. = 5.0 Deg.					
O-B	-1618 / 0			0.0	0.0	0.17 (1)	6.50	J-G	0 / 335	0.08 (1)	JSH GRIP= 0.86 (B) (INPUT = 0.90)					
I-H	-109 / 0			0.0	0.0	0.03 (1)	7.81	B-N	0 / 1260	0.28 (1)	JSH METAL= 0.34 (G) (INPUT = 1.00)					
								G-I	-1552 / 0	0.56 (1)						
Q-N	0 / 0			-28.0	-28.0	0.13 (3)	10.00									
N-M	0 / 1214			-28.0	-28.0	0.30 (2)	10.00									
M-L	0 / 1074			-28.0	-28.0	0.30 (2)	10.00									
L-K	0 / 881			-28.0	-28.0	0.39 (2)	10.00									
K-J	0 / 881			-28.0	-28.0	0.39 (2)	10.00									
J-I	0 / 724			-28.0	-28.0	0.37 (2)	10.00									



DWG NO. TAM 2570017
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	2	TOP
C - D	2	SIDE(278.7)
A - C	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(122.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	BMV1-t	MT20	3.0	8.0	Edge	0.50
E	BMVWV-t	MT20	6.0	9.0		
F	BMV1-t	MT20	3.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2411.6 lbs FACTORED DOWN AT 1-4-4 ON TOP CHORD, AND 1529.5 lbs FACTORED DOWN AT 3-4-4, AND 1536.2 lbs FACTORED DOWN AT 5-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	2978	0	2978	0
D	3159	0	3159	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	2429	1374 / 0	536 / 0
D	2575	1459 / 0	567 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)
FR-TO		FROM TO	LENGTH	FR-TO		FROM TO	LENGTH
F - A	-2940 / 0	0.0	0.0 0.17 (1)	6.71	A - E	0 / 3459	0.43 (1)
A - G	-3063 / 0	-84.3	-84.3 0.65 (1)	5.13	E - B	-1541 / 0	0.12 (1)
G - B	-3063 / 0	-84.3	-84.3 0.65 (1)	5.13	E - C	0 / 3459	0.43 (1)
B - C	-3063 / 0	-84.3	-84.3 0.11 (1)	6.16			
D - C	-1640 / 0	0.0	0.0 0.09 (1)	7.81			
F - E	0 / 0	-28.0	-28.0 0.02 (3)	10.00			
E - H	0 / 0	-28.0	-28.0 0.29 (1)	10.00			
H - I	0 / 0	-28.0	-28.0 0.29 (1)	10.00			
I - D	0 / 0	-28.0	-28.0 0.29 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-4-4	-2412	-2412	---	TOP	VERT	TOTAL
H	3-4-4	-1530	-1530	---	BACK	VERT	TOTAL
I	5-4-4	-1536	-1536	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.65 (A-B:1), BC=0.29 (D-E:1), WB=0.43 (A-E:1), SS=0.54 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1667 822 2284 1656	

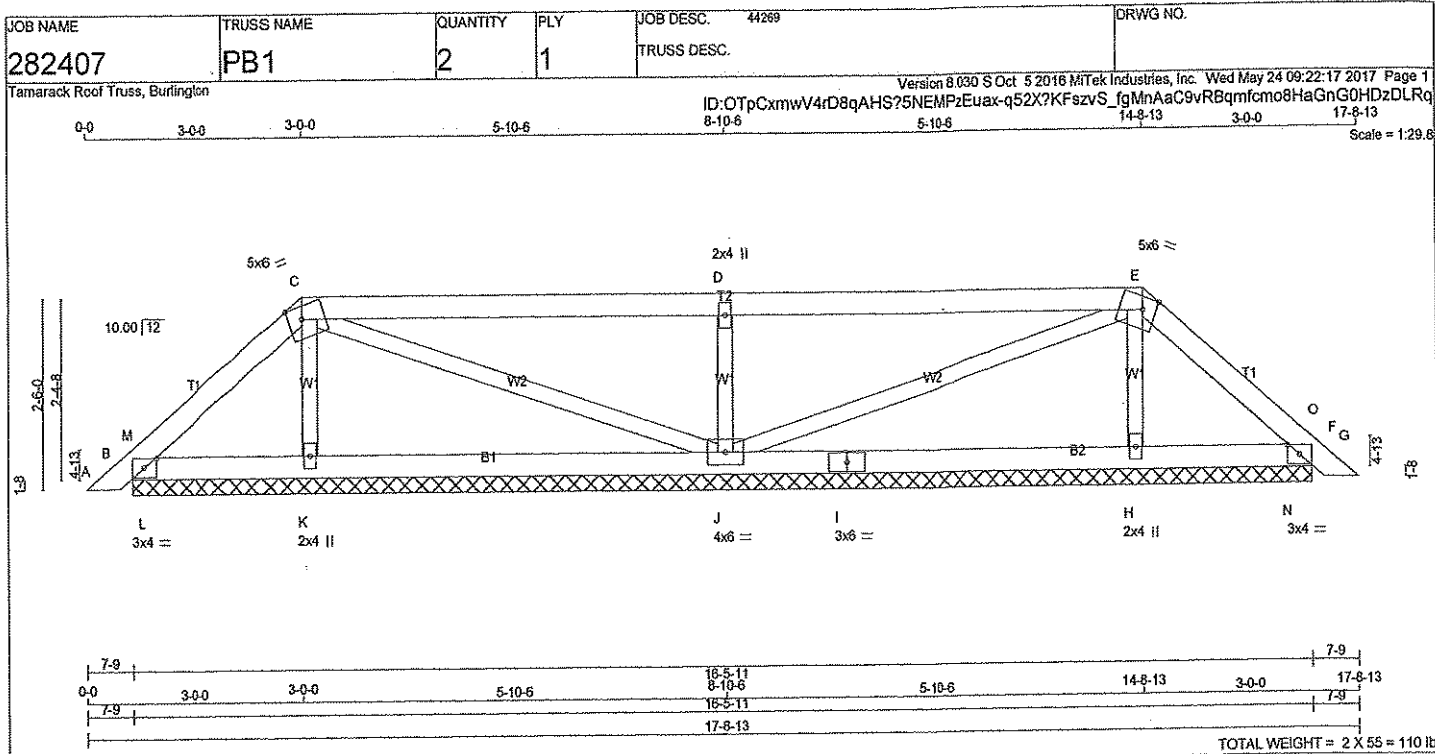
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.38 (A) (INPUT = 1.00)



DRWG NO. TAM 25701-17
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW-m	MT20	5.0	6.0	2.00	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.25
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BS-I	MT20	3.0	6.0		
J	BMWW1-I	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	199	0	199	0	16-5-11 (6-6-11)5-11	
K	369	0	369	0	16-5-11 (6-6-11)5-11	
J	805	0	805	0	16-5-11 (6-6-11)5-11	
H	369	0	369	0	16-5-11 (6-6-11)5-11	
F	199	0	199	0	16-5-11 (6-6-11)5-11	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS				
		COMBINED	SNOW	LIVE	PERMLIVE	WIND
B	140	115/0	7/0	0/0	0/0	18/0
K	327	142/0	101/0	0/0	0/0	84/0
J	644	385/0	128/0	0/0	0/0	131/0
H	327	142/0	101/0	0/0	0/0	84/0
F	140	115/0	7/0	0/0	0/0	18/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED	MAX. CS1 (LC)
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)			FORCE (LBS)	
FR-TO		FROM	TO		FR-TO		
A-B	0/13	-84.3	-84.3	0.02 (1)	10.00	K-C	-213/0 0.03 (1)
B-M	-13/21	-84.3	-84.3	0.03 (1)	6.25	C-J	-30/0 0.02 (1)
M-C	-73/0	-84.3	-84.3	0.05 (1)	6.25	J-D	-618/0 0.10 (1)
C-D	-12/0	-84.3	-84.3	0.49 (1)	6.25	J-E	-30/0 0.02 (1)
D-E	-12/0	-84.3	-84.3	0.49 (1)	6.25	H-E	-213/0 0.03 (1)
E-O	-73/0	-84.3	-84.3	0.05 (1)	6.25	L-M	-181/0 0.00 (1)
O-F	-13/21	-84.3	-84.3	0.03 (1)	6.25	N-O	-181/0 0.00 (1)
F-G	0/13	-84.3	-84.3	0.02 (1)	10.00		
B-L	0/51	-28.0	-28.0	0.06 (1)	10.00		
L-K	0/51	-28.0	-28.0	0.14 (2)	10.00		
K-J	0/40	-28.0	-28.0	0.21 (2)	10.00		
J-I	0/40	-28.0	-28.0	0.21 (2)	10.00		
I-H	0/40	-28.0	-28.0	0.21 (2)	10.00		
H-N	0/51	-28.0	-28.0	0.14 (2)	10.00		
N-F	0/51	-28.0	-28.0	0.06 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.49 (D-E:1), BC=0.21 (J-K:2), WB=0.10 (D-J:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
			MAX MIN	MAX MIN
MT20	618	354	1667	822 2284 1655

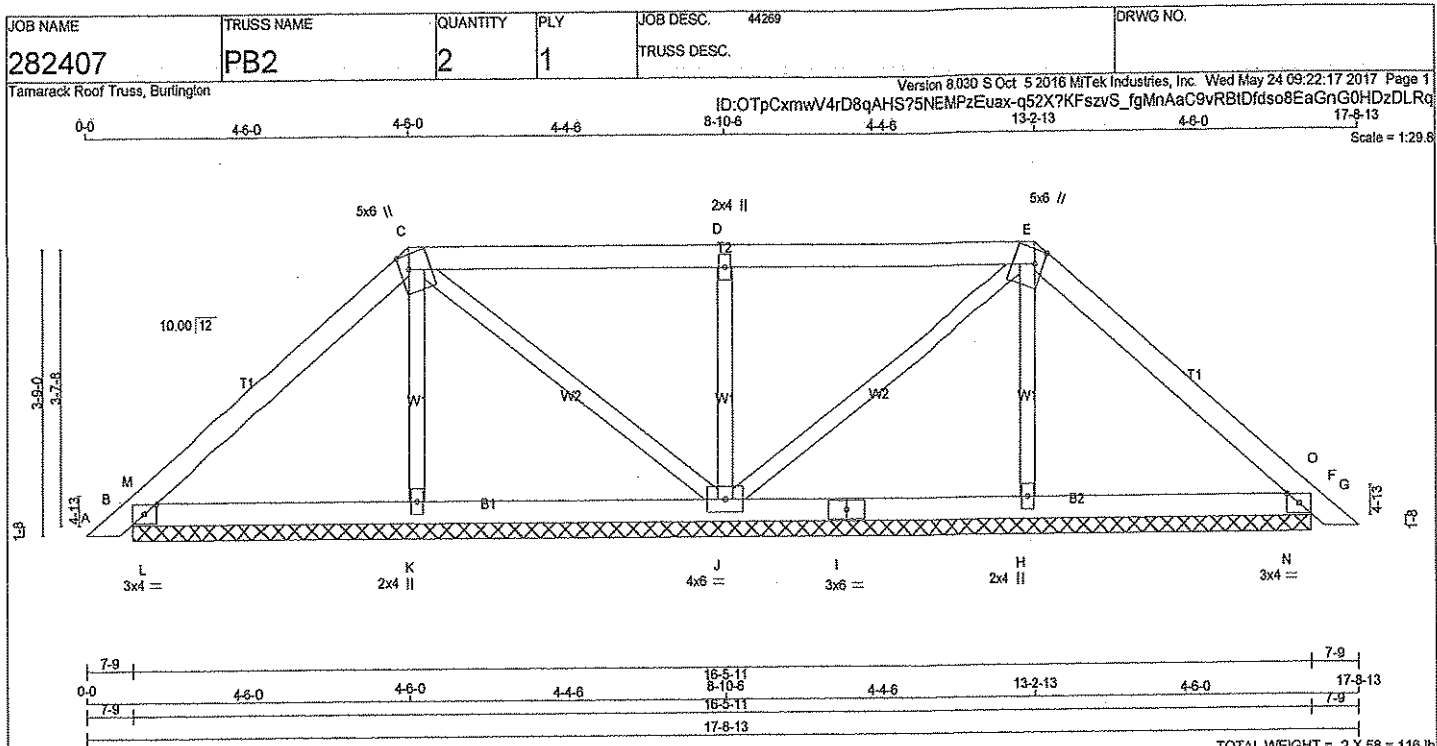
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.84 (E) (INPUT = 0.50)
JSI METAL=0.11 (D) (INPUT = 1.00)



DRWG NO. TAM 25702-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
B - I	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BS-I	MT20	3.0	6.0		
J	BMW1+w	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	328	0	328	0	16-5-11 (6-8-11)5-11	IN-SX
K	321	0	321	0	16-5-11 (6-8-11)5-11	IN-SX
J	641	0	641	0	16-5-11 (6-8-11)5-11	IN-SX
H	321	0	321	0	16-5-11 (6-8-11)5-11	IN-SX
F	328	0	328	0	16-5-11 (6-8-11)5-11	IN-SX

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	252	168/0	39/0	0/0	0/0	46/0	0/0
K	286	123/0	89/0	0/0	0/0	74/0	0/0
J	503	317/0	90/0	0/0	0/0	97/0	0/0
H	286	123/0	89/0	0/0	0/0	74/0	0/0
F	252	168/0	39/0	0/0	0/0	46/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	MAX. UNBRAC LENGTH	FR-TO			
A-B	0/13	-84.3 -84.3	0.02 (1)	10.00	K-C	-155/0	0.03 (1)
B-M	-95/7	-84.3 -84.3	0.07 (1)	6.25	C-J	-70/0	0.03 (1)
M-C	-133/0	-84.3 -84.3	0.16 (1)	6.25	J-D	-459/0	0.10 (1)
C-D	-34/0	-84.3 -84.3	0.27 (1)	6.25	J-E	-70/0	0.03 (1)
D-E	-34/0	-84.3 -84.3	0.27 (1)	6.25	H-E	-155/0	0.03 (1)
E-O	-133/0	-84.3 -84.3	0.16 (1)	6.25	L-M	-352/79	0.00 (1)
O-F	-95/7	-84.3 -84.3	0.07 (1)	6.25	N-O	-352/79	0.00 (1)
F-G	0/13	-84.3 -84.3	0.02 (1)	10.00			
B-L	0/95	-28.0 -28.0	0.14 (1)	10.00			
L-K	0/95	-28.0 -28.0	0.14 (1)	10.00			
K-J	0/91	-28.0 -28.0	0.13 (2)	10.00			
J-I	0/91	-28.0 -28.0	0.13 (2)	10.00			
I-H	0/91	-28.0 -28.0	0.13 (2)	10.00			
H-N	0/95	-28.0 -28.0	0.14 (1)	10.00			
N-F	0/95	-28.0 -28.0	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27 (C-D:1), BC=0.14 (H-N:1), WB=0.10 (D-J:1), SS=0.29 (F-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

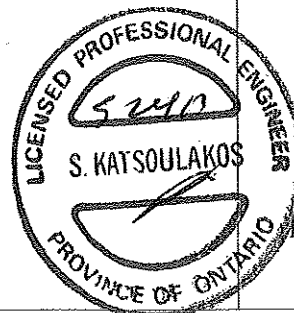
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

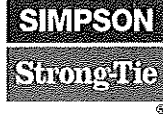
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.59 (C) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



DRWG NO. TAM25703-17
STRUCTURAL
COMPONENT ONLY

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

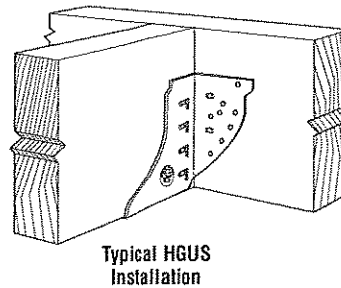
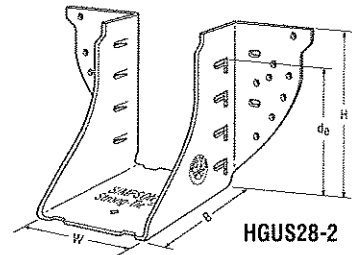
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

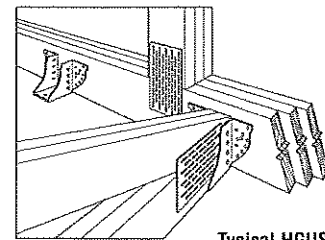
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



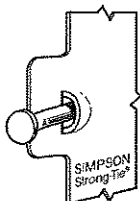
Typical HGUS Installation



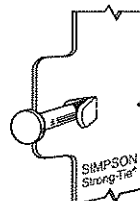
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1⅝	5⅝	5	4⅝	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅝	5⅝	4	4⅝	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅝	5⅝	4	4⅝	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅝	5⅝	4	4⅝	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅝	5	6⅝	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅝	7⅝	4	6⅝	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅝	7⅝	4	6⅝	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅝	7⅝	4	6⅝	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅝	9⅝	4	8⅝	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4⅝	9⅝	4	8⅝	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅝	9⅝	4	8⅝	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅝	10⅝	4	10⅝	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅝	12⅝	4	11⅝	66-16d	22-16d	10130	16400	7195	11645

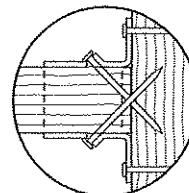
1. d_g is the distance from the seat of the hanger to the highest joist nail.



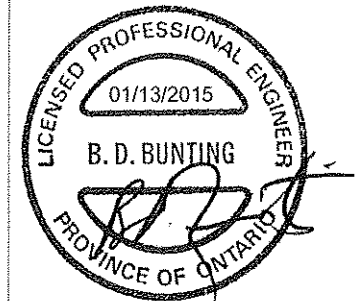
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

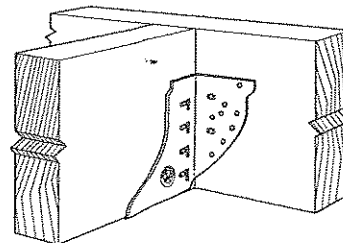
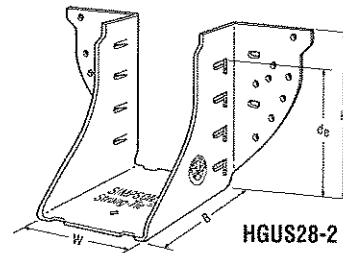
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

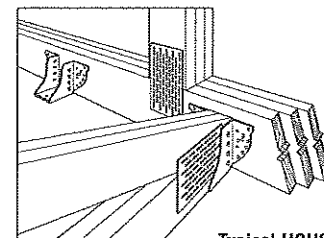
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



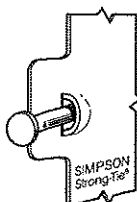
Typical HGUS Installation



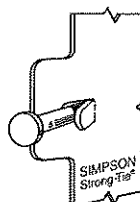
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 3/32	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 1/4	7 1/4	5	6 1/4	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645

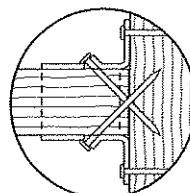
1. d_g is the distance from the seat of the hanger to the highest joist nail.



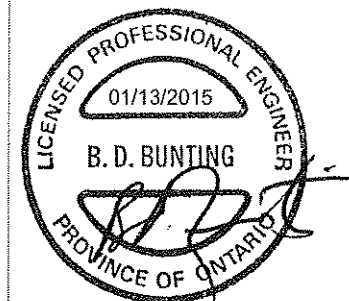
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15-1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

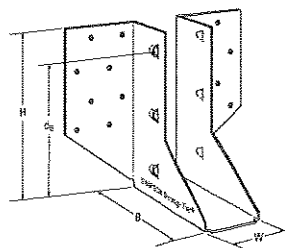
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

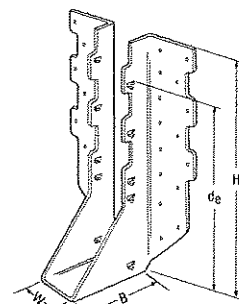
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

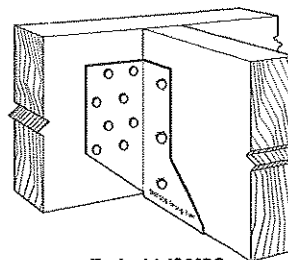
- See current catalogue for options



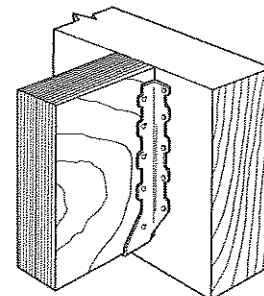
LJS26DS



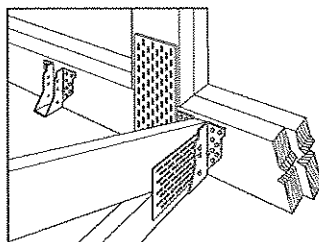
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



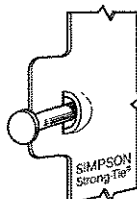
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

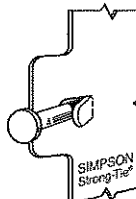
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4%	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1%	5½	3	3½	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1%	7½	3	6¾	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1%	9¾	3	7¾	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

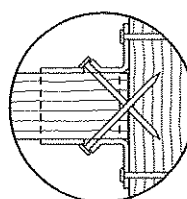


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

FACE MOUNT HANGERS LUS/HUS Double Shear Joist Hangers



This product is preferable to similar connectors because of
 a) easier installation, b) higher capacities, c) lower installed cost,
 or a combination of these features.

See Hanger tables on pages 79-84. See Hanger Options on page 230 for hanger modifications, which may result in reduced resistances.

All hangers in this series have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation, and the use of standard nails for all connections. (Do not bend or remove tabs.)

MATERIAL: See tables, pages 79-84.

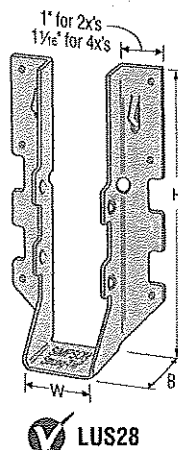
FINISH: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pages 14-17.

INSTALLATION: • Use all specified fasteners. See General Notes.

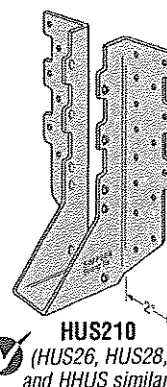
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances.
- Not designed for welded or nailer applications.
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated resistance.
- With 3x carrying members, use 16dx2½" nails into the header and 16d commons into the joist with no reduction in resistances. With a single 2x carrying member, use 10dx1½" nails into the header and 10d commons into the joist, reduce the resistance to 0.64 of the table value.

OPTIONS: • LUS hangers cannot be modified.

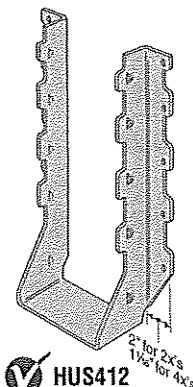
- HUS hangers available with the header flanges turned in for 2-2x (3¼") and 4x only, with no reduction in resistances. See the HUSC Concealed Flange illustration.
- See Hanger Options, page 230.



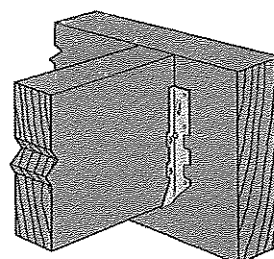
✓ **LUS28**



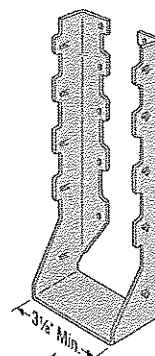
✓ **HUS210**
(HUS26, HUS28,
and HHUS similar)



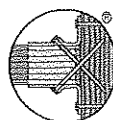
✓ **HUS412**



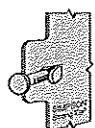
**Typical LUS28
Installation
Standard
LUS28
Installation
use .148x3"
(10d common)**



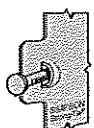
✓ **HUSC
Concealed Flanges**
(not available for HHUS,
HGUS and HUS2x)



Double
Shear
Nailing
Top View



Double Shear
Nailing
Side View
Do not bend
tab back



Dome Double Shear
Nailing prevents tabs from
breaking off (available on
some models)
U.S. Patent 5,603,580

HUCQ Heavy Duty Joist Hangers

The HUCQ series are heavy duty joist hangers that incorporate Simpson Strong-Tie® Strong-Drive® SDS Heavy-Duty Connector screws. Designed and tested for installation at the end of a beam or on a post, they provide a strong connection with fewer fasteners than nailed hangers.

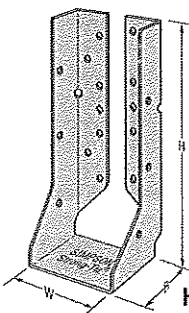
MATERIAL: 14 gauge

FINISH: Galvanized. Most models available in stainless steel or ZMAX® coating.

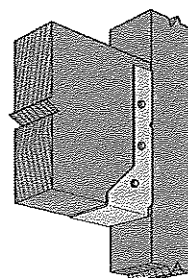
INSTALLATION: • Use all specified fasteners. See General Notes.

- Strong-Drive SDS Heavy-Duty Connector screws supplied.
- For use on solid sawn or engineered wood products.

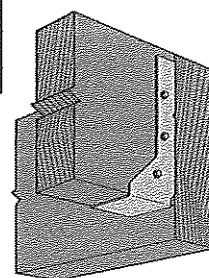
These products are available with additional corrosion protection. Additional products on this page may also be available with this option, check with Simpson Strong-Tie for details.



HUCQ410



**Typical HUCQ
Installation
on a Post**



**Typical HUCQ
Installation
on a Beam**



**Strong-Drive® ¼"x2½" SDS
HEAVY-DUTY CONNECTOR Screw**
(See page 25 for more information)

Model No.	Dimensions (in)			Fasteners		Factored Resistance			
	W	H	B	Face	Joist	D.Fir-L		S-P-F	
						Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
						lbs	lbs	lbs	lbs
						kN	kN	kN	kN
■ HUCQ310-SDS	2½	9	3	8-¼"x2½" SDS	4-¼"x2½" SDS	2140	5235	1930	3770
■ HUCQ210-2-SDS	3¼	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	9.52	23.29	8.59	16.77
■ HUCQ410-SDS	3½	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
■ HUCQ412-SDS	3½	11	3	14-¼"x2½" SDS	6-¼"x2½" SDS	14.28	32.34	12.90	30.36
■ HUCQ210-3-SDS	4¾	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	3210	7270	2900	6825
■ HUCQ610-SDS	5½	9	3	12-¼"x2½" SDS	6-¼"x2½" SDS	14.28	32.34	12.90	30.36
■ HUCQ612-SDS	5½	11	3	14-¼"x2½" SDS	6-¼"x2½" SDS	3210	9090	2900	7645
						14.28	40.43	12.90	34.01

1. Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as cantilever construction.
2. When using structural composite lumber columns, Strong-Drive® SDS Heavy-Duty Connector screws must be applied to the wide face of the column.