

THE FOLLOWING DRAWING NUMBERS REPRESENT THE IDENTIFIERS OF THE SEALED DRAWINGS OF THE TRUSS COMPONENTS IDENTIFIED ON THIS LAYOUT:
(LAYOUT: 37834/240105)

(LAYOUT: 37834/240105)
DWG #TAM20100-15 THROUGH DWG #TAM20110-15, INCLUSIVE, AND ALL DATED 5-29-15;
DWG #TAM2457-15 THROUGH DWG #TAM2460-15, INCLUSIVE, AND ALL DATED 1-30-15;
DWG #TAM2462-15 AND DWG #TAM2463-15 AND BOTH DATED 1-30-15;
DWG #TAM2465-15 AND DWG #TAM2466-15 AND BOTH DATED 1-30-15;
DWG #TAM37450-14R1 DATED 1-14-15;
DWG #TAM37444-14 THROUGH DWG #TAM37446-14, INCLUSIVE, AND ALL DATED 9-08-14;
DWG #TAM37448-14 AND DWG #TAM37449-14 AND BOTH DATED 9-08-14;
DWG #TAM37441-14 AND DWG #TAM37452-14 AND BOTH DATED 9-08-14;
DWG #TAM37456-14 AND DWG #TAM37477-14 AND BOTH DATED 9-08-14;
DWG #TAM37459-14 AND DWG #TAM37462-14 AND BOTH DATED 9-08-14;
DWG #TAM37463-14 DATED 9-08-14;
DWG #TAM37465-14 THROUGH DWG #TAM37473-14, INCLUSIVE, AND ALL DATED 9-08-14;
DWG #TAM11042-15 THROUGH DWG #TAM11044-15, INCLUSIVE, AND ALL DATED 3-23-15;
DWG #TAM6305-14 DATED 3-05-14 (CONVENTIONAL FRAMED VALLEY STANDARD)
(STRUCTURAL COMPONENTS ONLY)

TRUSS PROFILE TO BE VERIFIED BY BUILDING DESIGNER

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC.
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally Braced so THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

CITY:	ON
GROUND SNOW LOAD	31.35 PSF
TC DEAD	3.0 PSF
BC LIVE	10.5 PSF
BC DEAD	7.0 PSF

TRUSSES DESIGNED CONFORM TO THE RELEVANT
SECTION OF THE 2012 ONTARIO
BUILDING CODE OF CANADA (RESIDENTIAL PART 9)

NOTES:

FIN. OVERHANG: 12" (AS PER PLAN)
HEEL: R.T.M. CANT. (AS PER PLAN) *BELL CURVE*
EXTERIOR WALLS: 2X6 (AS PER PLAN)
FASCIA BOARD: 2X6
SHEATHING: ASPHALT SHINGLE

HARDWARE:

(x28) LJS26DS (V)
(x8) LUS26-2 (VV)
(x12) LUS24 (O)
(x3) HGUS26-2 (XX)

T-140655

HATCH LEGEND

CONV FRM BY OTHERS	
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DATE: 6-7-77

BCIN:26064; FIRM:BCIN #29991

ENGINEERING ONLY-DIMENSIONS TO BE VERIFIED
ON SITE

SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER

DESIGNER. ALL CONVENTIONAL FRAMING TO BE SPECIFIED, REVIEWED, AND CONFIRMED BY BUILDING DESIGNER PRIOR TO TRUSS INSTALLATION. ALL NOTES DESIGNATING PER PLAN WORK DO NOT REPRESENT A PART OF THE SCOPE OF THIS SEAL. THIS WORK IS DELEGATED TO A QUALIFIED BUILDING DESIGNER HAVING RESPONSIBILITY FOR THIS PROJECT. ALL BEAMS NOT ADDRESSED IN THIS DESCRIPTION AND LABELLED ON THIS LAYOUT ARE BEAMS SPECIFIED BY THE BUILDING DESIGNER AND/OR PROJECT ENGINEER AND ARE TO BE REVIEWED AND CONFIRMED BY THE SAME DESIGNERS PRIOR TO FABRICATION TO ENSURE ADEQUATE LOAD CAPACITY WITH RESPECT TO THE ROOF TRUSS COMPONENTS REVIEWED IN THIS SUBMISSION.

LOT SPECIFIC SCHEDULE 1 REQUIRED FOR EACH
INDIVIDUAL LOT.

DWG #TAM ~~4~~ THROUGH DWG #TAM, ~~4~~ INCLUSIVE, DATED: ~~4~~

STRUCTURAL COMPONENTS ONLY; THIS LAYOUT MUST BE READ TOGETHER WITH REFERENCED TRUSS COMPONENT DRAWINGS AND SEALED HARDWARE DRAWINGS. HANGERS AND HARDWARE SPECIFIED ON THIS LAYOUT ONLY. REFER TO ATTACHED SEALED REFERENCED DRAWINGS FOR ADDITIONAL BRACING REQUIREMENTS:
(IF BRACING IS SHOWN TO BE REQUIRED PROVIDE THE FOLLOWING BRACING):

PROVIDE 1 ROW OF 2X 4 #2 SPF CONTINUOUS LATERAL BRACES ALONG WEB MEMBERS SPECIFIED ON REFERENCE 3-5/8" DEEP TRUSS COMPONENT DRAWINGS USING 2-1/4" COMMON WIRE NAILSPER WEB MEMBER. THEN PROVIDE 2 X 4 #2 SPF "X-BRACING" ALONG THESE WEB MEMBERS AT LOCATIONS INDICATED (*) USING THE SAME NAILING. PROVIDE BLOCKING IF REQUIRED (DO NOT ARCH BRACING).
PROVIDE 2 X 4 #2 SPF BRACE ALONG FULL LENGTH OF Laterally Braced Web MEMBERS OF 1 PLY TRUSS COMPONENTS REQUIRING "T-BRACE" USING 1 ROW OF 3-1/4" COMMON WIRE NAILS AT 6" O/C. SIMILARLY PROVIDE 2 X 6 BRACE WITH 2 ROWS OF NAILING FOR 2 PLY TRUSS COMPONENTS AND 2 X 8 BRACE WITH 3 ROWS OF NAILING FOR 3 PLY TRUSS COMPONENTS.

- (1) 2 X 6 #2 SPF ON FLAT OVER PLYWOOD SHEATHING.
- (2) 2 X 6 #2 SPF RIDGE BEAM SUPPORTED AT EACH END OVER BASE TRUSS COMPONENTS AND AT LOCATIONS MARKED BY "X".
- (3) 2 X 6 #2 SPF ROOF RAFTERS AT 24" O/C WITH MAXIMUM SUPPORT SPAN 6'-0" (ADD 2 X 4 VERTICAL SUPPORTS TO TRUSS BASE BELOW WHERE NECESSARY.

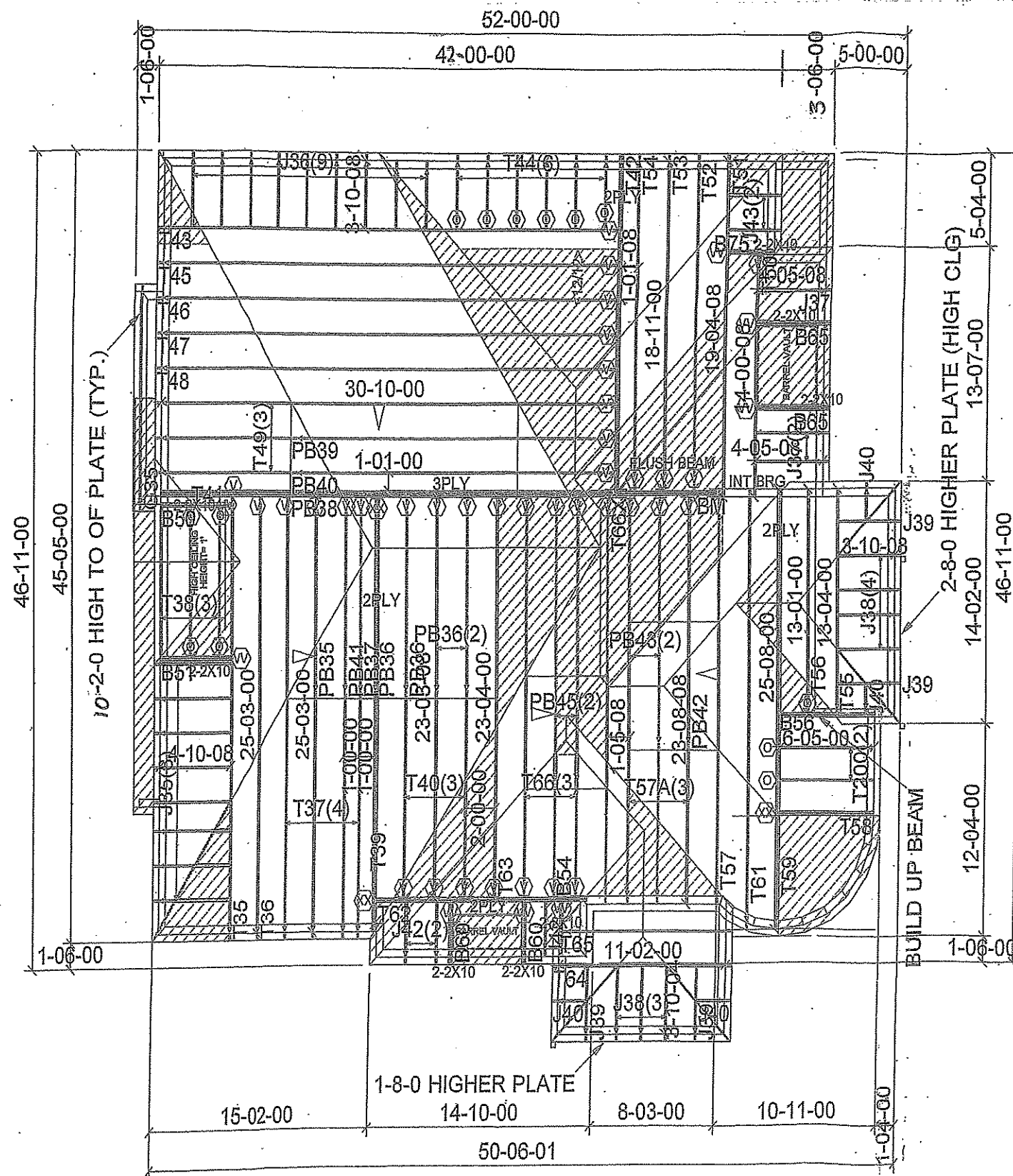
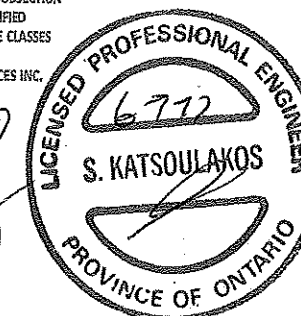
ALL CONVENTIONAL FRAMING TO BE DISTRIBUTED UNIFORMLY ALONG BASE TRUSS COMPONENTS. PROVIDE 2 X 4 KNEE-WALLS WHERE NECESSARY WITH STUDS AT 24" O/C.

ALL CONVENTIONAL FRAMING TO CONFORM TO ONTARIO BUILDING CODE
(CURRENT EDITION).

I REVIEWED AND TAKE RESPONSIBILITY FOR THE DESIGN WORK ON BEHALF OF A FIRM REGISTERED UNDER SUBSECTION 3.2.5 OF THE ONTARIO BUILDING CODE. I AM QUALIFIED AND THE FIRM IS REGISTERED, IN THE APPROPRIATE CLASSES AND/OR CATEGORIES.

REGISTERED FIRM: MICRO QTY ENGINEERING SERVICES INC.

DWG #TAM 29071-17
BCIN: 26064
FIRM: 29991
SEALED STRUCTURAL
COMPONENTS ONLY



		M101226 Job Track: 44269		Builder: GREENYORK HOMES- BRAMPTON		Location: DENTON 12		Elevation: 1		M:\JOBS\37560\234359	
Layout ID: 240105-D		Project: OSTIENS		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.							
Plan Log: 92466		Date: 9/5/2014		Designer: WA /36/		MITek ver 7.6.0					



Delivery Shiplist

DATE	06/01/15
SALES REP	Mario

JOB TRACK:37834 LAYOUT ID: 240105 LOCATION: BRAMPTON
 BUILDER: SUB-BUILDER:
 MODEL: DENTON 12 ELEVATION:

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T35 COMMON	8.00 0.00	25-03-00	05-09-00	2 X 6	2 X 6	01-03-08 00-00-00	00-05-08 05-09-00	133.92 82.00		
	1	G35 COMMON	12.00 0.00	12-02-00	06-11-08	2 X 4	2 X 4	00-05-00 00-05-00	00-10-08 00-10-08	52.60 34.84		
	1	T36 COMMON	8.00 0.00	25-03-00	07-09-00	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 07-09-00	119.06 74.83		
	4	T37 COMMON	8.00 0.00	25-03-00	09-09-00	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 09-09-00	521.96 322.68		
	3	T38 COMMON	12.00 0.00	08-10-08	06-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-11-00 01-11-08	127.65 87.51		
	1 2 Ply	T39 PIGGYBACK	8.00 0.00	25-03-00	09-09-00	2 X 4	2 X 6	00-00-00 00-00-00	00-08-12 09-09-00	301.96 183.00		
	3	T40 PIGGYBACK	8.00 0.00	23-04-00	09-09-00	2 X 4	2 X 4	00-00-00 00-00-00	02-00-01 09-09-00	372.42 231.99		
	1 3 Ply	T41 PIGGYBACK	12.00 0.00	30-08-00	09-11-12	2 X 6	2 X 6	01-03-08 00-00-00	00-05-08 09-11-12	703.98 423.00		
	1 2 Ply	T42 COMMON	12.00 0.00	19-04-08	10-11-08	2 X 4	2 X 6	00-05-00 00-00-00	00-05-08 10-11-08	300.38 185.66		
	1	T43 COMMON	12.00 0.00	30-10-00	03-03-12	2 X 4	2 X 6	00-05-00 00-00-00	00-05-08 03-03-12	157.72 99.17		
	6	T44 COMMON	12.00 0.00	03-10-08	04-09-00	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 04-09-00	117.30 82.98		
	1	T45 COMMON	12.00 0.00	30-10-00	04-07-12	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 04-07-12	121.06 77.67		
	1	T46 COMMON	12.00 0.00	30-10-00	05-11-12	2 X 6	2 X 4	01-03-08 00-00-00	00-03-08 05-11-12	131.39 82.33		
	1	T47 COMMON	12.00 0.00	30-10-00	07-03-12	2 X 4	2 X 4	00-05-00 00-00-00	00-03-08 07-03-12	142.25 87.67		
	1	T48 COMMON	12.00 0.00	30-10-00	08-07-12	2 X 6	2 X 4	00-05-00 00-00-00	00-03-08 08-07-12	155.44 95.50		
	3	T49 COMMON	12.00 0.00	30-10-00	09-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 09-11-12	493.17 303.51		
	1	T50 COMMON	12.00 0.00	14-00-08	05-04-00	2 X 6	2 X 6	00-00-00 00-00-00	00-10-08 05-04-00	72.12 46.00		
	1	T51 COMMON	12.00 0.00	19-04-08	03-10-00	2 X 4	2 X 6	00-05-00 00-00-00	00-05-08 03-10-00	88.37 55.67		



Delivery Shiplist

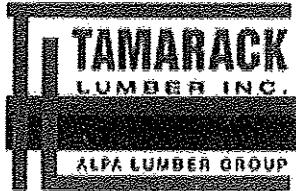
DATE	06/01/15
SALES REP	Mario

JOB TRACK: 37834	LAYOUT ID: 240105	LOCATION: BRAMPTON
BUILDER: (		SUB-BUILDER
MODEL: DENTON 12		ELEVATION:

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T52 COMMON	12.00 0.00	18-11-00	05-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 05-10-00	80.90 51.17		
	1	T53 COMMON	12.00 0.00	18-11-00	07-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 07-10-00	92.72 59.17		
	1	T54 COMMON	12.00 0.00	18-11-00	09-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 09-10-00	112.53 70.83		
	1	T55 HIP GIRDER	12.00 0.00	13-04-00	04-09-00	2 X 4	2 X 6	01-03-08 01-03-08	00-05-08 00-05-08	69.02 43.00		
	1	T56 HIP	12.00 0.00	13-01-00	09-05-00	2 X 4	2 X 4	00-00-00 00-00-00	03-09-08 03-06-08	75.35 48.50		
	1	T57 PIGGYBACK	12.00 0.00	24-02-00	11-03-08	2 X 4	2 X 4	00-00-00 00-00-00	02-04-08 03-06-08	149.61 93.33		
	3	T57A PIGGYBACK	12.00 0.00	23-08-08	11-03-08	2 X 4	2 X 4	00-00-00 00-00-00	02-04-08 04-00-00	446.82 276.00		
	1 2 Ply	T58 COMMON	12.00 0.00	06-05-00	07-03-08	2 X 4	2 X 6	01-03-08 00-00-00	00-05-08 07-03-08	75.56 48.00		
	1 2 Ply	T59 HIP GIRDER	12.00 0.00	25-08-00	07-03-08	2 X 4	2 X 6	01-03-08 00-00-00	00-05-08 03-06-08	279.32 174.34		
	1	T61 HIP	12.00 0.00	25-08-00	09-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 03-06-08	134.15 84.00		
	1 2 Ply	T62 COMMON	12.00 0.00	14-00-08	04-00-08	2 X 6	2 X 6	00-00-00 00-00-00	00-10-00 04-00-08	154.52 101.00		
	1	T63 COMMON	12.00 0.00	23-04-00	08-07-00	2 X 4	2 X 4	00-00-00 00-00-00	04-03-08 08-07-00	124.24 76.67		
	1	T64 COMMON	12.00 0.00	11-02-00	04-09-00	2 X 4	2 X 4	01-03-08 01-03-08	00-03-08 00-03-08	52.57 35.33		
	1	T65 COMMON	12.00 0.00	11-02-00	06-05-08	2 X 4	2 X 4	00-00-00 00-05-00	00-10-08 00-03-08	47.58 31.00		
	3	T66 PIGGYBACK	*** 0.00	23-04-00	10-07-00	2 X 4	2 X 4	00-00-00 00-00-00	10-07-00 04-03-08	424.29 261.00		
	1	T66X COMMON	*** 0.00	23-09-00	10-07-00	2 X 4	2 X 4	00-00-00 00-00-00	10-07-00 03-10-08	145.02 89.33		
	2	T200 COMMON	12.00 0.00	06-05-00	07-03-08	2 X 4	2 X 4	01-03-08 00-00-00	00-03-08 07-03-08	66.16 42.34		
	1	PB35 PIGGYBACK	8.00 0.00	10-11-11	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	36.64 24.00		



Delivery Shiplist

DATE	06/01/15
SALES REP	Mario

JOB TRACK:37834	LAYOUT ID: 240105	LOCATION: BRAMPTON
BUILDER:		SUB-BUILDER
MODEL: DENTON 12		ELEVATION:

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	4	PB36 PIGGYBACK	8.00 0.00	10-11-11	05-08-09	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 03-08-15	172.24 107.32		
	1	PB37 PIGGYBACK	8.00 0.00	10-11-11	04-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 03-08-15	44.66 29.50		
	1 3 Ply	PB38 PIGGYBACK	12.00 0.00	21-00-01	03-03-03	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 03-03-03	223.32 141.51		
	1	PB39 PIGGYBACK	12.00 0.00	21-02-01	01-02-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 01-02-08	68.38 43.00		
	1	PB40 PIGGYBACK	12.00 0.00	21-02-01	02-06-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 02-06-08	72.16 45.17		
	1	PB41 PIGGYBACK	8.00 0.00	10-11-11	03-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 03-07-00	37.70 24.17		
	1	PB42 PIGGYBACK	12.00 0.00	06-04-10	01-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	22.55 15.33		
	2	PB43 PIGGYBACK	12.00 0.00	06-04-10	03-07-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	43.50 28.34		
	2	PB45 PIGGYBACK	12.00 0.00	16-05-13	01-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 01-10-08	108.50 70.00		
	8	J35 JACK-OPEN	12.00 0.00	04-10-08	05-09-00	2 X 4 2 X 4	01-03-08 00-00-00	00-03-08 05-09-00	129.84 85.36		
	9	J36 JACK-OPEN	8.00 0.00	03-10-08	03-03-12	2 X 4 2 X 4	01-03-08 00-00-00	00-03-08 03-03-12	109.71 78.03		
	3	J37 JACK-OPEN	12.00 0.00	04-05-08	05-04-00	2 X 4 2 X 4	01-03-08 00-00-00	00-03-08 05-04-00	45.24 30.00		
	7	J38 JACK-OPEN	12.00 0.00	03-10-08	04-09-00	2 X 4 2 X 4	01-03-08 00-00-00	00-03-08 04-09-00	94.36 65.31		
	4	J39 JACK-OPEN	12.00 0.00	03-10-08	02-07-15	2 X 4 2 X 4	01-03-08 -02-01-01	00-03-08 00-03-08	40.44 29.32		
	4	J40 JACK-OPEN	12.00 0.00	01-10-08	02-07-15	2 X 4 2 X 4	01-03-08 -00-01-01	00-03-08 00-03-08	31.32 24.00		
	3	J42 JACK-OPEN	12.00 0.00	03-02-00	04-00-08	2 X 4 2 X 4	01-03-08 00-00-00	00-03-08 04-00-08	34.59 24.00		
	2	J43 JACK-OPEN	12.00 0.00	02-11-08	03-10-00	2 X 4 2 X 4	01-03-08 00-00-00	00-03-08 03-10-00	21.90 14.66		

TOTAL # TRUSS= 118.00

TOTAL BFT OF ALL TRUSSES=

5020.04 BFT. TOTAL WEIGHT OF ALL TRUSSES= 7980.16 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	06/01/15
SALES REP	Mario

JOB TRACK:37834

LAYOUT ID: 240105

LOCATION: BRAMPTON

BUILDER: '

SUB-BUILDER:f

MODEL: DENTON

ELEVATION:

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
36	Hangers	LJS26DS	
13	Hangers	LUS24	
8	Hangers	LUS26-2	

TOTAL # ITEMS= 60.00

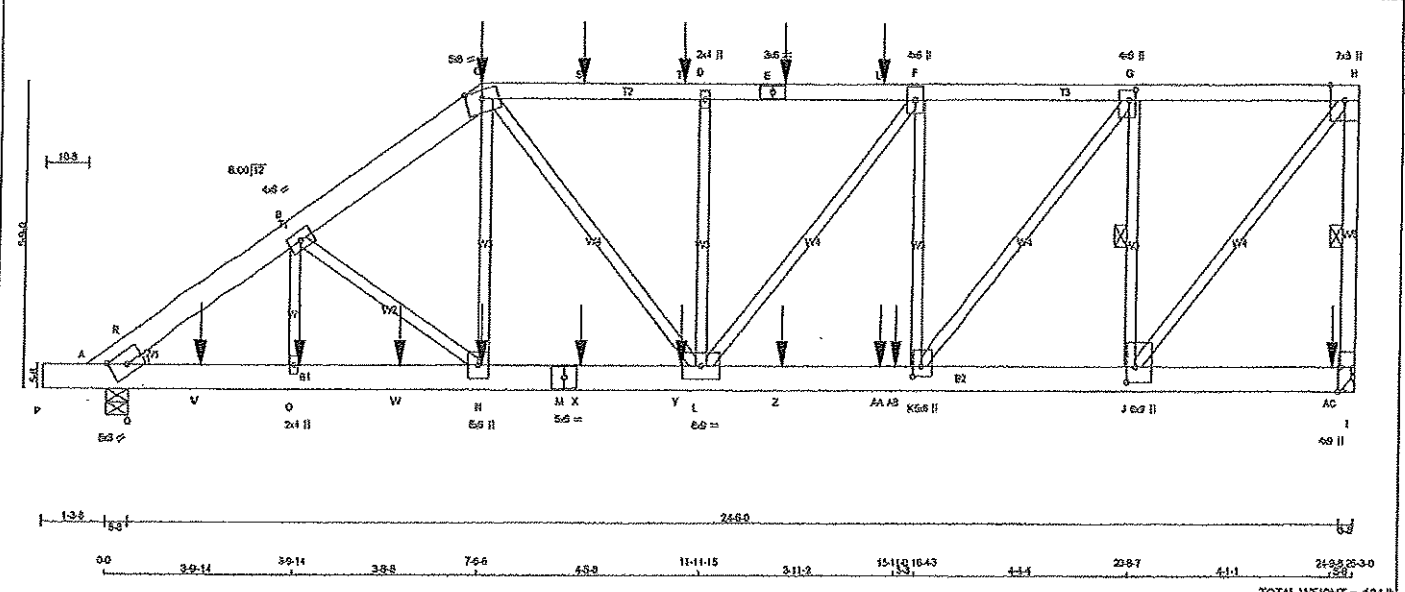


Table with 4 columns: LUMBER, N L G A. RULES, CHORDS, SIZE, LUMBER, DESCR.

Table with 5 columns: PLATES, TYPE, W, LEN, Y, X.

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 121.1 lbs

DESIGNER BEARINGS

Table with 8 columns: FACTORED, MAXIMUM FACTORED, INPUT, REQD, BEARING, HEEL, WEDGE.

Table with 8 columns: UNFACTORED REACTIONS, 1ST CASE, MAX. AN. COMPONENT REACTIONS.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68FT.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H, G, J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

Table with 10 columns: CHORDS, MAX. FACTORED, FACTORED, VERT. LOAD, LCI, MAX, WEBS, MAX. FACTORED, MAX, LCI.

Table with 7 columns: SPECIFIED CONCENTRATED LOADS (LBS), LOC, LCI, MAX, FACE, DIR, TYPE.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT.
NO FURTHER MODIFICATIONS WERE MADE.

SPECIFIED LOADS:
TOP CH LL = 25.6 PSF
DL = 3.0 PSF

BOT CH LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD ORDER ***
ADD'L USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2,4

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NSCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NSCC 2012, BCBC 2012
- CSA C89-09
- TRC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.65")
CALCULATED VERT. DEF. (LL) = L/999 (0.16")

ALLOWABLE DEF. (LL) = L/360 (0.65")
CALCULATED VERT. DEF. (LL) = L/999 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEF. (LL) = L/120 (0.19")
CALCULATED VERT. DEF. (LL) = L/999 (0.00")

ALLOWABLE DEF. (LL) = L/120 (0.19")
CALCULATED VERT. DEF. (LL) = L/999 (0.00")

CSI: TC=0.69 (C-D-I), BC=0.64 (K-L-I), WB=0.75 (H-I-I), SS=0.57 (K-L-I)

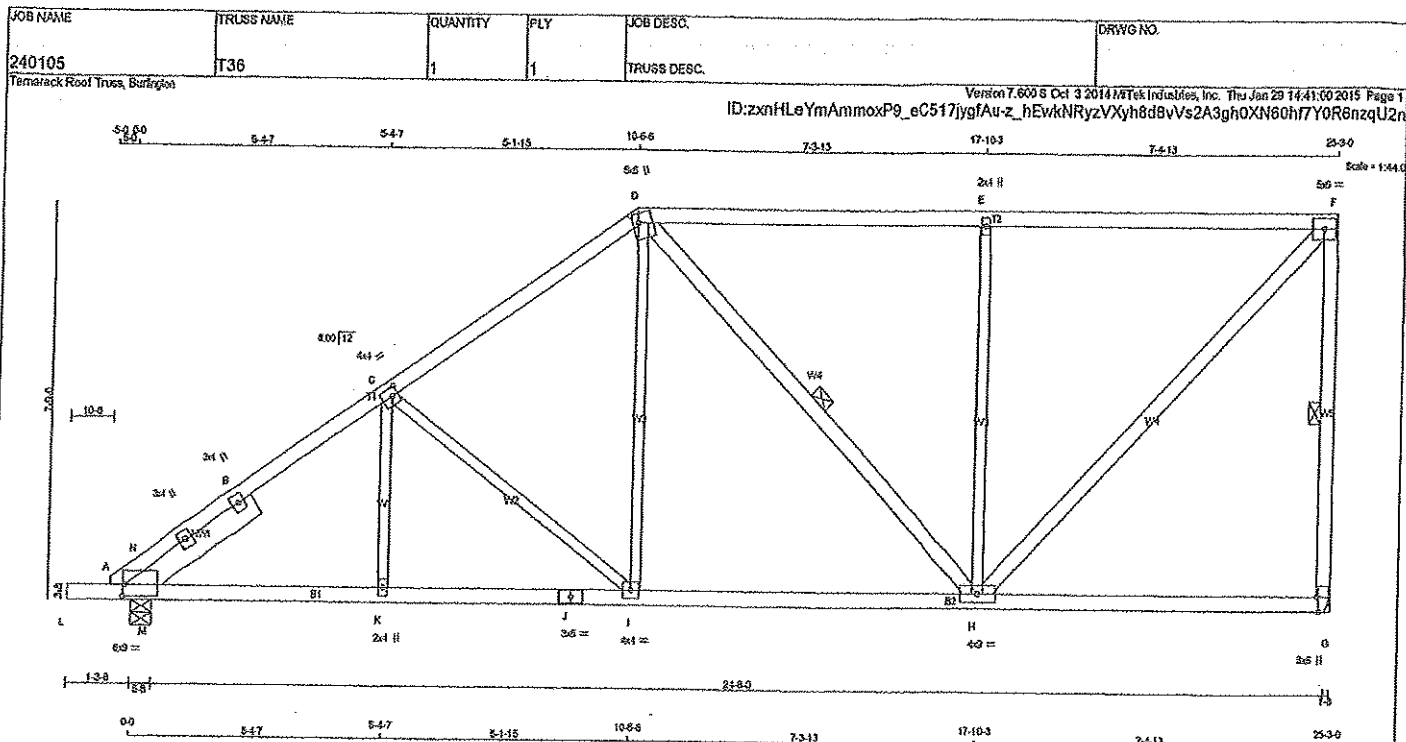
DOL LUMBER=1.00 NAL=1.00 L.S. BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



DWG NO. TAM 2457-15
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
G - F	2x4	DRY	No.2
D - F	2x4	DRY	No.2
A - D	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

REINFORCING MEMBERS

HW1	SIZE	DRY	LUMBER
HW1	2x8	DRY	No.2

ALL WEBS EXCEPT

H - F	SIZE	DRY	LUMBER
H - F	2x4	DRY	No.2
D - H	2x4	DRY	No.2

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1441	0	1441	0	IN-SX	IN-SX
A	1539	0	1539	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LC CASE		MAX. A&N COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
G	1183	657/0	270/0	0/0	0/0	257/0	0/0
A	1284	702/0	288/0	0/0	0/0	274/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-O, D-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM TO	CS (L/C)	UNBRAC LENGTH	FR-TO		CS (L/C)
G-F	-1359/0	0.0	0.0	0.37 (1)	5.57	H-F	0/1582
D-E	-1114/0	-84.3	-84.3	0.62 (1)	5.07	H-E	-770/0
E-F	-1114/0	-84.3	-84.3	0.62 (1)	5.05	D-H	-161/0
A-N	-2020/0	-84.3	-84.3	0.09 (1)	4.74	I-D	0/548
N-B	-1917/0	-84.3	-84.3	0.32 (1)	4.57	C-I	-603/0
B-C	-1917/0	-84.3	-84.3	0.32 (1)	4.57	K-C	0/207
C-D	-1494/0	-84.3	-84.3	0.30 (1)	5.06	M-N	0/179
L-A	0/0	-112.3	-112.3	0.08 (1)	10.00	M-B	-222/0
A-M	0/1542	-28.0	-28.0	0.35 (1)	10.00		
M-K	0/1700	-28.0	-28.0	0.41 (1)	10.00		
K-J	0/1700	-28.0	-28.0	0.40 (2)	10.00		
J-I	0/1700	-28.0	-28.0	0.40 (2)	10.00		
I-H	0/1227	-28.0	-28.0	0.54 (2)	10.00		
H-G	0/0	-28.0	-28.0	0.39 (2)	10.00		

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TSMR14	MT20	80	90	2.75	0.50
A	RT4	MT20	30	4.0		
A	RT4	MT20	30	4.0		
C	BMWW4	MT20	40	4.0	2.00	1.50
D	BMWW4	MT20	50	6.0	2.00	1.50
E	BMWW4	MT20	20	4.0		
F	BMWW4	MT20	50	6.0		
G	BMWW4	MT20	30	6.0		
H	BMWW4	MT20	40	4.0		
I	BMWW4	MT20	40	4.0		
J	BS4	MT20	30	6.0		
K	BMWW4	MT20	20	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012
- CSA 086-09
- TPI 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/240 (0.66)
CALCULATED VERT. DEFL. (LL) = L/999 (0.11)
ALLOWABLE DEFL. (TL) = L/240 (0.66)
CALCULATED VERT. DEFL. (TL) = L/999 (0.19)

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = L/120 (0.19)
CALCULATED VERT. DEFL. (LL) = L/999 (0.00)
ALLOWABLE DEFL. (TL) = L/120 (0.19)
CALCULATED VERT. DEFL. (TL) = L/999 (0.00)

CSL TC=0.62 (E-F), BC=0.54 (H-I), WB=0.68 (E-H-I), SSI=0.30 (E-F-I)

DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HERE ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRP(DRY)	SHEAR	SECTION
PL1	(PL1)	(PL1)	(PL1)
MT20	618	354	1667

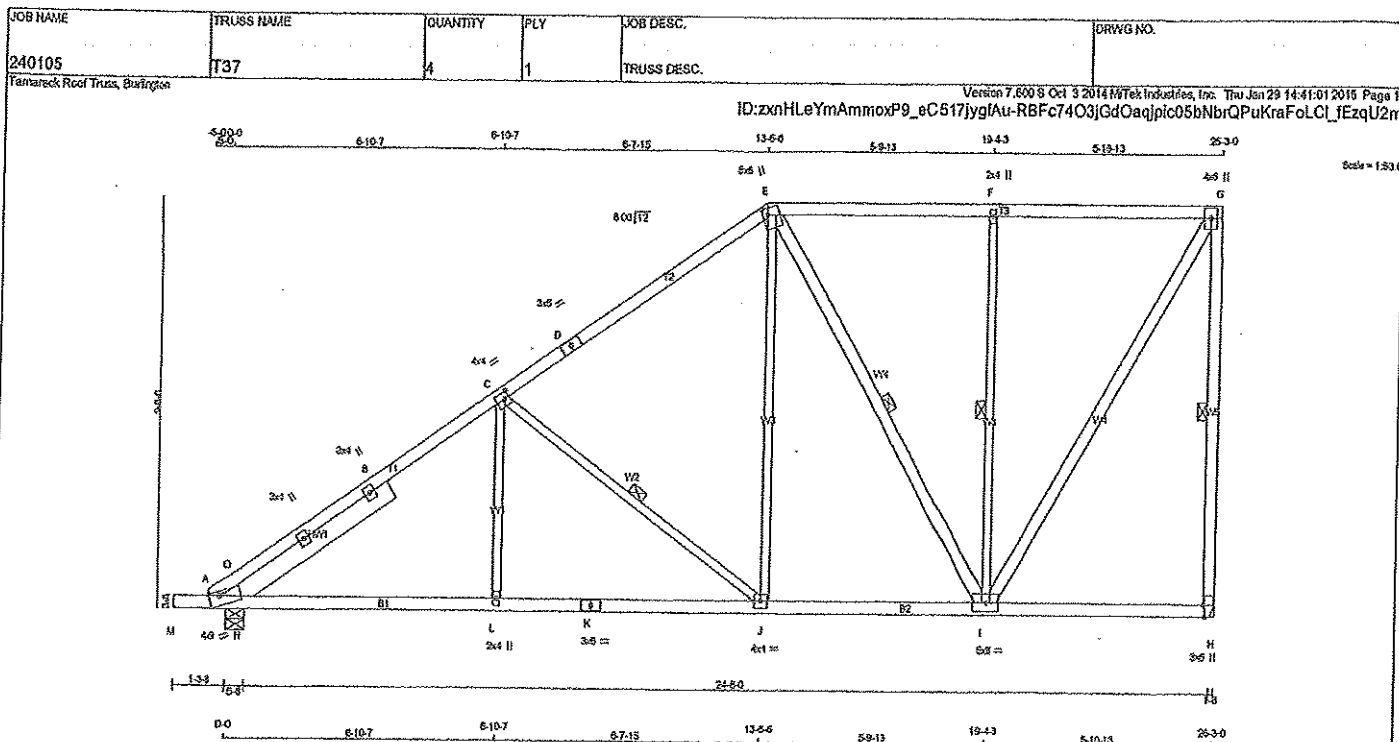
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRP= 0.89 (0) (INPUT = 0.80)
SSI METAL= 0.49 (A) (INPUT = 1.00)



DRWG NO. TAM 2450-15
STRUCTURAL
COMPONENT ONLY



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2

REINFORCING MEMBERS

MEMB.	SIZE	LUMBER	DESCR.
HW1	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

MEMB.	SIZE	LUMBER	DESCR.
E - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TB/MR1-m	MT20	4.0	9.0	1.25	Edge
A - RT#1	MT20	3.0	4.0		
A - RT#1	MT20	3.0	4.0		
C - TM/WV1	MT20	4.0	4.0	2.00	1.50
D - TS-1	MT20	3.0	6.0		
E - TM/WV1-m	MT20	5.0	6.0	2.00	1.50
F - TM/WV1-p	MT20	2.0	4.0		
G - TM/WV1-p	MT20	4.0	6.0		
H - BM/VW1	MT20	3.0	6.0	Edge	0.50
I - BM/VWV1	MT20	5.0	8.0		
J - BM/VWV1	MT20	4.0	4.0		
K - BS-1	MT20	3.0	6.0		
L - BM/VW	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGNER

FACTORED

JT	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
H	VERT	DOWN	BRG	BRG
H	HORIZ	UP/LIFT	IN-SX	IN-SX
A	1441	0	1441	0
A	1539	0	1539	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOL
H	1183	657/0	270/0	0/0	0/0	257/0	0/0
A	1204	702/0	289/0	0/0	0/0	274/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-H, C-J, E-I, F-I.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADINGS

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		VERT. LOAD	LC1 MAX	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO					
A-O	-2105/0	-84.3	-84.3 0.14 (1)	4.58	0.368
O-B	-1951/0	-84.3	-84.3 0.57 (1)	4.24	0.10/0
B-C	-1951/0	-84.3	-84.3 0.57 (1)	4.24	0.16/0
C-D	-1207/0	-84.3	-84.3 0.52 (1)	5.15	0.30 (1)
D-E	-1207/0	-84.3	-84.3 0.52 (1)	5.15	0.30 (1)
E-F	-739/0	-84.3	-84.3 0.38 (1)	6.25	0.11397
F-G	-739/0	-84.3	-84.3 0.38 (1)	6.25	0.11397
H-G	-1374/0	0.0	0.0 0.63 (1)	5.54	0.1145
M-A	0/0	-112.3	-112.3 0.06 (1)	10.00	
A-N	0/1871	-28.0	-28.0 0.39 (1)	10.00	
N-L	0/1625	-28.0	-28.0 0.49 (2)	10.00	
L-K	0/1625	-28.0	-28.0 0.49 (2)	10.00	
K-J	0/1625	-28.0	-28.0 0.49 (2)	10.00	
J-I	0/680	-28.0	-28.0 0.35 (2)	10.00	
I-H	0/0	-28.0	-28.0 0.23 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH LL = 25.0 PSF
 DL = 3.0 PSF
 BOT CH LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012
 - CSA 688-09
 - TPC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/250 (0.25)
 CALCULATED VERT. DEFL. (LL) = 1/999 (0.11)
 ALLOWABLE DEFL. (TL) = 1/600 (0.16)
 CALCULATED VERT. DEFL. (TL) = 1/999 (0.19)

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL. (LL) = 1/120 (0.19)
 CALCULATED VERT. DEFL. (LL) = 1/699 (0.00)
 ALLOWABLE DEFL. (TL) = 1/120 (0.19)
 CALCULATED VERT. DEFL. (TL) = 1/999 (0.00)

CS1 TO = 0.63 (G-H), BC = 0.49 (L-N), WB = 0.39 (C-J), SS = 0.24 (F-G-1)

DOL LUMBER = 1.00 NAIL = 1.00 LBS BEND = 1.10 COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPARISON LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHTHEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRP (DRY) SHEAR SECTION (PSI) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRP = 0.60 (A) (INPUT = 0.60)
 JSI METAL = 0.47 (A) (INPUT = 1.00)



DRWG NO. TAM 2458-15
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T38	3	1	TRUSS DESC.	

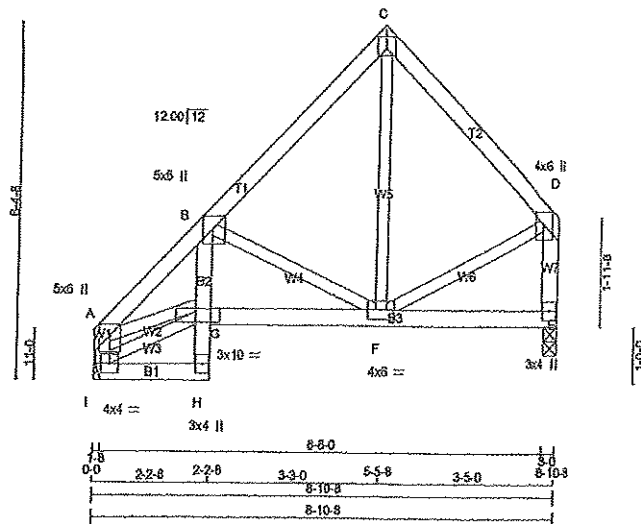
Tamarack Roof Truss, Burlington

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0-0 2-2-8 2-2-8 3-3-0 5-5-8 3-5-0 8-10-8

4x4 II

Scale = 1:39.2



TOTAL WEIGHT = 3 X 43 = 128 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
I - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW+p	MT20	5.0	6.0	Edge 2.25	
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
E	BMV+p	MT20	3.0	4.0		
F	BMVWVW-I	MT20	4.0	8.0		
G	BMVWVW-I	MT20	3.0	10.0	Edge 5.50	
H	BMV+p	MT20	3.0	4.0		
I	BMVWVW-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
I	498	0	498	0
E	498	0	498	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	409	227/0	93/0	0/0	0/0	89/0	0/0	0/0
E	409	227/0	93/0	0/0	0/0	89/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX CS (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX CS (LC)
FR-TO				FR-TO			
A-B	-705/0	-84.3	-84.3 0.09 (1)	B-F	-352/0	0.09 (1)	
B-C	-321/0	-84.3	-84.3 0.11 (1)	F-C	0/220	0.05 (2)	
C-D	-305/0	-84.3	-84.3 0.13 (1)	F-D	0/238	0.05 (1)	
I-A	-458/0	0.0	0.0 0.05 (1)	I-G	-20/0	0.09 (1)	
E-D	-461/0	0.0	0.0 0.05 (1)	A-G	0/530	0.12 (1)	
I-H	0/24	-28.0	-28.0 0.04 (2)				
H-G	0/51	0.0	0.0 0.04 (1)				
G-B	0/182	0.0	0.0 0.03 (1)				
G-F	0/533	-28.0	-28.0 0.15 (2)				
F-E	0/0	-28.0	-28.0 0.09 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 46.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.30")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = 1/360 (0.30")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.13 (C-D:1), BC=0.15 (F-G:2), WB=0.12 (A-G:1), SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX. MIN.	MAX. MIN.	MAX. MIN.	MAX. MIN.
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.72 (C) (INPUT = 0.90)

JSI METAL= 0.14 (G) (INPUT = 1.00)



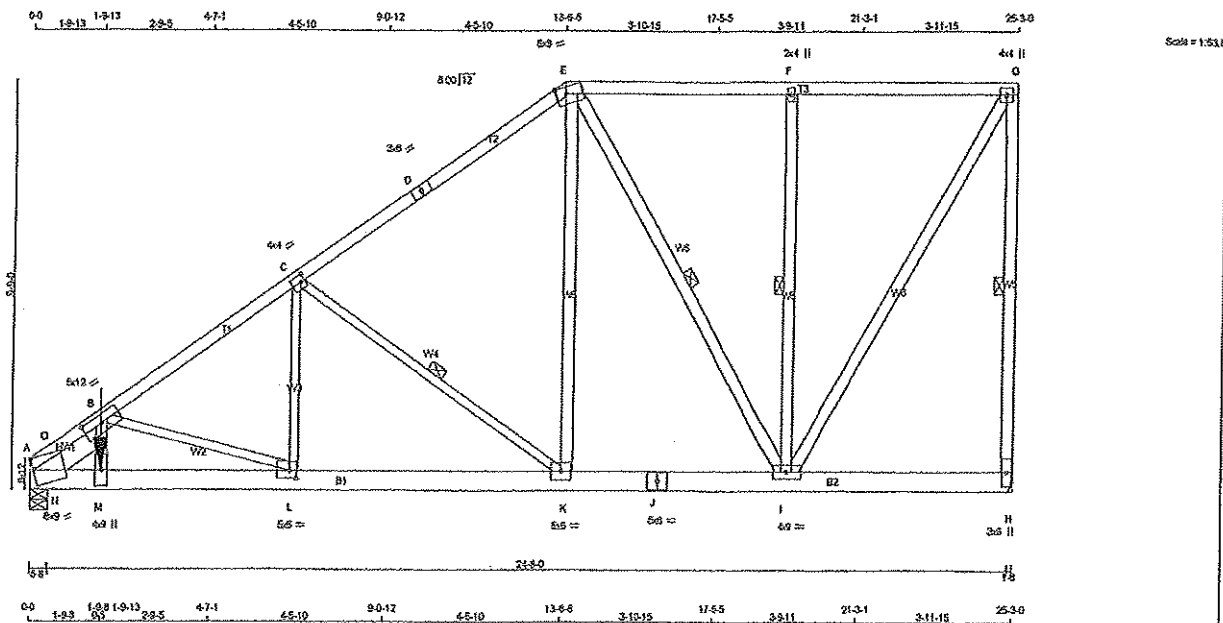
DRWG NO. TAM20103 -15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T39	1	2	TRUSS DESC.	

Tamersack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 151 = 302 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF
A - D	2x4	DRY	No.2				
D - E	2x4	DRY	No.2				
E - G	2x4	DRY	No.2				
H - G	2x4	DRY	No.2				
A - J	2x6	DRY	No.2				
J - H	2x6	DRY	No.2				

REINFORCING MEMBERS	HWI	2x0	DRY	No.2	SPF
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ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS			
A - D	1	12	TOP
D - E	1	12	TOP
E - G	1	12	TOP
G - H	1	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS			
A - J	2	2	SIDE (1404.0)
J - H	2	12	TOP
WEBS: (0.122'X3') SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMWV1-m	MT20	6.0	9.0	3.25	0.25
B	TMWVW4	MT20	6.0	12.0	2.25	6.00
C	TMWV4	MT20	4.0	4.0	2.00	1.25
D	TS-1	MT20	3.0	6.0		
E	TMWV-m	MT20	5.0	8.0	2.00	3.00
F	TMWV-w	MT20	2.0	4.0		
G	TMWV+P	MT20	4.0	4.0		
H	BMV4+P	MT20	3.0	6.0		
I	BMWVW4	MT20	4.0	9.0		
J	BS-1	MT20	5.0	6.0		
K	BMWVW4	MT20	5.0	6.0		
L	BMWVW4	MT20	5.0	6.0	2.25	2.00
M	BMWV-w	MT20	4.0	9.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5131.8 lb FACTORED DOWN AT 1-9.4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGNER BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	1787	0	1787	0
A	6160	0	6160	0

UNFACTORED REACTIONS	1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
H	1493	819 / 0
A	5018	2877 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-H, C-K, F-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS							WEBS						
MAX. FACTORED			FACTORED			MAX			MAX. FACTORED				
MEMB.	FORCE		VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX				
	(LBS)		(PLF)	CS1 (LC)	UNBRAC			(LBS)	CS1 (LC)				
FR-TO			FROM TO		LENGTH	FR-TO							
A-O	-7642 / 0	-84.3	-84.3	0.20 (1)	3.44	L-C	0 / 1513	0.19 (1)					
C-B	-6122 / 0	-84.3	-84.3	0.18 (1)	3.82	C-K	-1933 / 0	0.47 (1)					
B-C	-3532 / 0	-84.3	-84.3	0.40 (1)	4.58	K-E	0 / 1282	0.11 (1)					
C-D	-1717 / 0	-84.3	-84.3	0.38 (1)	6.13	I-F	-512 / 0	0.14 (1)					
D-E	-1717 / 0	-84.3	-84.3	0.38 (1)	6.13	M-B	0 / 4718	0.42 (1)					
E-F	-957 / 0	-84.3	-84.3	0.28 (1)	6.25	B-L	-3768 / 0	0.65 (1)					
F-G	-957 / 0	-84.3	-84.3	0.28 (1)	6.25	E-I	-874 / 0	0.28 (1)					
H-G	-1718 / 0	0.0	0.0	0.38 (1)	6.25	I-G	0 / 1855	0.18 (1)					
						N-O	0 / 2208	0.03 (1)					
A-N	0 / 5067	-28.0	-28.0	0.44 (1)	10.00	N-B	-2824 / 0	0.11 (1)					
N-M	0 / 6748	-28.0	-28.0	0.88 (1)	10.00								
M-L	0 / 6570	-28.0	-28.0	0.67 (1)	10.00								
L-K	0 / 2894	-28.0	-28.0	0.26 (1)	10.00								
K-J	0 / 1420	-28.0	-28.0	0.12 (2)	10.00								
J-I	0 / 1420	-28.0	-28.0	0.12 (2)	10.00								
I-H	0 / 0	-28.0	-28.0	0.06 (3)	10.00								

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE
M	1-9.6	-4183	-4183			FRONT	VERT	TOTAL



DRWG NO. TAM 246015

STRUCTURAL

COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

CL = 3.0 PSF

BOT CH. LL = 10.5 PSF

CL = 7.0 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD ORDER ***

ADDTL USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2,4

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBCS 2012

- CSA 688-09

- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/260 (0.84")

CALCULATED VERT. DEFL. (LL) = 1/599 (0.07")

ALLOWABLE DEFL. (TL) = 1/350 (0.84")

CALCULATED VERT. DEFL. (TL) = 1/599 (0.11")

CS1: TC=0.40 (B-C-I), RC=0.68 (M-N-I), WB=0.86 (B-L-I), SSI=0.14 (A-N-I)

DOL LUMBER=1.00 NAIL=1.00 L5 BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (3) (INPUT = 0.50)

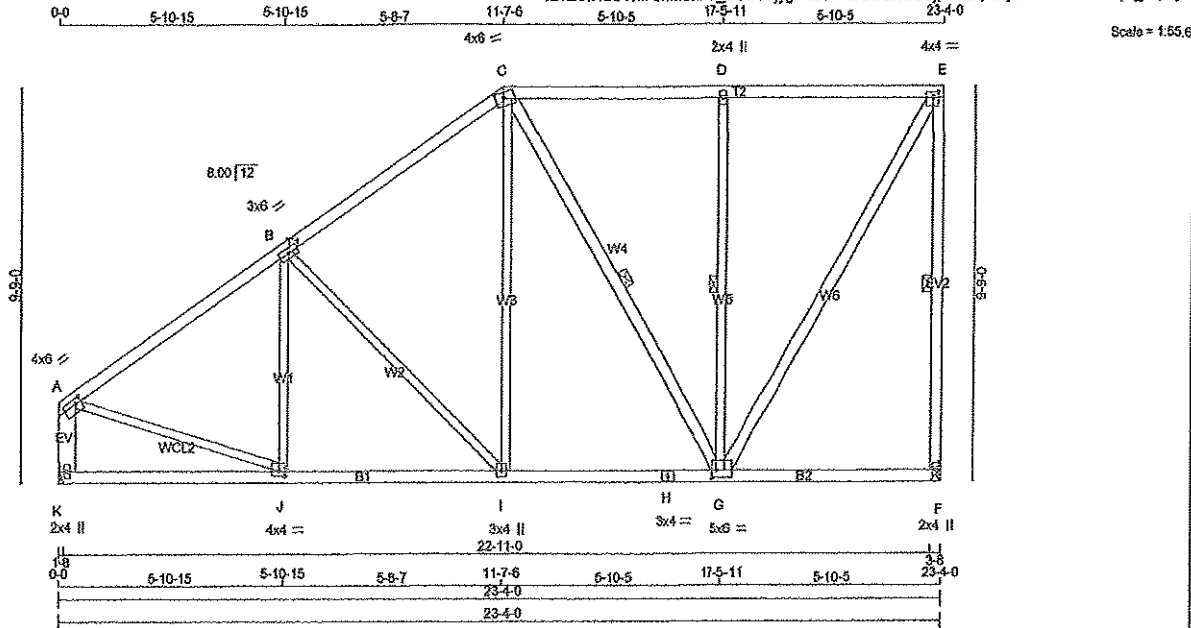
JSI METAL = 0.76 (3) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T40	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 124 = 372 lb
[M][F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-1	MT20	4.0	8.0 1.50 1.75
B	TMVW-1	MT20	3.0	6.0
C	TTWW-m	MT20	4.0	6.0 1.75 2.50
D	TMVW-w	MT20	2.0	4.0
E	TMVW-1	MT20	4.0	4.0 1.50 1.75
F	BMV1+p	MT20	2.0	4.0
G	BMVWW-1	MT20	5.0	6.0 2.25 1.50
H	BS-1	MT20	3.0	4.0
I	BMVW-1	MT20	3.0	4.0
J	BMVW-1	MT20	4.0	4.0 1.50 1.75
K	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
F	1310	0	1310	0	0			HANGER BY OTHERS	
K	1310	0	1310	0	0			MIN. SEAT SIZE: 1-8	HANGER BY OTHERS

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	WIND	DEAD
F	1075	597 / 0	245 / 0	0 / 0	233 / 0
K	1075	597 / 0	245 / 0	0 / 0	233 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-F, C-G, D-G.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (3)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	-1288 / 0	-84.3 -84.3 0.41 (1)	5.19	J-B	-130 / 97
B-C	-1011 / 0	-84.3 -84.3 0.38 (1)	5.73	B-I	-409 / 0
C-D	-655 / 0	-84.3 -84.3 0.37 (1)	6.25	I-C	0 / 461
D-E	-655 / 0	-84.3 -84.3 0.37 (1)	6.25	C-G	-307 / 0
F-E	-1244 / 0	0.0 0.0 0.57 (1)	5.76	G-D	-612 / 0
K-A	-1243 / 0	0.0 0.0 0.09 (1)	7.81	G-E	0 / 1244
K-J	0 / 0	-28.0 -28.0 0.24 (2)	10.00	A-J	0 / 1149
J-I	0 / 1105	-28.0 -28.0 0.37 (2)	10.00		
I-H	0 / 817	-28.0 -28.0 0.33 (2)	10.00		
H-G	0 / 817	-28.0 -28.0 0.33 (2)	10.00		
G-F	0 / 0	-28.0 -28.0 0.24 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2012
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.78")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.78")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.09")

CS: TC=0.57 (E-F:1), BC=0.37 (I-J:2), WB=0.53 (B-I:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

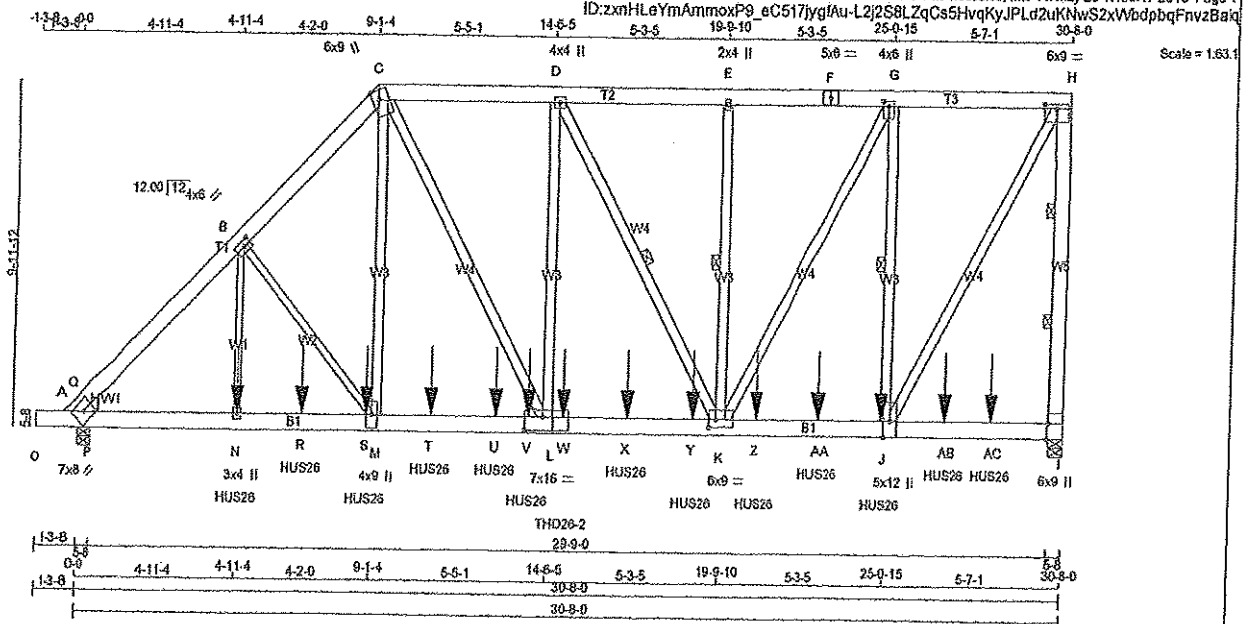
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.41 (J) (INPUT = 1.00)



DWG NO T4M3744.14
STRUCTURAL
COMPONENT ONLY



LUMBER

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT

CHORDS	SIZE	DRY	LUMBER	DESCR.
N - B	2x3	DRY	No.2	SPF
B - M	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122'X3') SPIRAL NAILS			
A - C	2	12	TOP
C - F	2	12	TOP
F - H	2	12	TOP
H - I	2	12	TOP
BOTTOM CHORDS: (0.122'X3') SPIRAL NAILS			
O - L	2	10	SIDE(372.1)
L - I	2	10	SIDE(484.0)
WEBS: (0.122'X3') SPIRAL NAILS			
N - B	1	6	SIDE(180.8)
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMH1-h	MT20	7.0	8.0		
B	TMVW-h	MT20	4.0	8.0	2.00	2.50
C	TTWW-h	MT20	8.0	8.0	Edge	2.25
D	TMVW-h	MT20	4.0	4.0		
E	TMVW-h	MT20	2.0	4.0		
F	TS-1	MT20	5.0	6.0		
G	TMVW-h	MT20	4.0	6.0	1.75	2.00
H	TMVW-h	MT20	8.0	8.0	1.50	4.50
I	BMVW-h	MT20	8.0	8.0	Edge	0.50
J	BMVW-h	MT20	5.0	12.0	Edge	
K	BMVW-h	MT20	8.0	8.0	2.75	2.50
L	BSWV-h	MT20	7.0	18.0	4.50	6.50
M	BMVW-h	MT20	4.0	8.0		
N	BMVW-h	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORIZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
A	11423	0	11423	0	5-8	4-10
I	12278	0	12278	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	9308	5283/0	2043/0	0/0	0/0	1981/0	0/0
I	9987	5698/0	2174/0	0/0	0/0	2117/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2 LATERAL BRACE(S) REQUIRED AT 1/3 LENGTH OF H.
1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E.K, G-J, D-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-Q	-15351/0	-84.3 -84.3 0.21 (1)	3.74	M-C	0/0999	0.33 (1)	
Q-B	-15930/0	-84.3 -84.3 0.27 (1)	3.68	N-B	0/3073	0.23 (1)	
B-C	-13653/0	-84.3 -84.3 0.15 (1)	4.02	B-M	-2589/0	0.69 (1)	
C-D	-11554/0	-84.3 -84.3 0.14 (1)	4.32	K-E	-445/0	0.07 (1)	
D-E	-10039/0	-84.3 -84.3 0.10 (1)	4.61	L-D	0/2429	0.13 (1)	
E-F	-10039/0	-84.3 -84.3 0.11 (1)	4.60	J-G	-8779/0	1.00 (1)	
F-G	-10039/0	-84.3 -84.3 0.11 (1)	4.60	C-L	0/3987	0.20 (1)	
G-H	-6541/0	-84.3 -84.3 0.09 (1)	5.45	D-K	-3209/0	0.65 (1)	
H-I	-11585/0	0.0 0.0 0.49 (1)	4.41	K-G	0/7218	0.39 (1)	
				J-H	0/12821	0.69 (1)	
O-A	0/0	-112.3 -112.3 0.01 (1)	10.00	P-Q	-9107/0	0.00 (1)	
A-P	0/11240	-28.0 -28.0 0.59 (1)	10.00				
P-N	0/11240	-28.0 -28.0 0.59 (1)	10.00				
N-R	0/11240	-28.0 -28.0 0.83 (1)	10.00				
R-S	0/11240	-28.0 -28.0 0.83 (1)	10.00				
S-M	0/11240	-28.0 -28.0 0.83 (1)	10.00				
M-T	0/0992	-28.0 -28.0 0.84 (1)	10.00				
T-U	0/0992	-28.0 -28.0 0.84 (1)	10.00				
U-V	0/0992	-28.0 -28.0 0.84 (1)	10.00				
V-L	0/0992	-28.0 -28.0 0.84 (1)	10.00				
L-W	0/11595	-28.0 -28.0 0.92 (1)	10.00				
W-X	0/11595	-28.0 -28.0 0.92 (1)	10.00				
X-Y	0/11595	-28.0 -28.0 0.92 (1)	10.00				
Y-K	0/11595	-28.0 -28.0 0.92 (1)	10.00				
K-Z	0/6541	-28.0 -28.0 0.70 (1)	10.00				
Z-AA	0/6541	-28.0 -28.0 0.70 (1)	10.00				
AA-J	0/6541	-28.0 -28.0 0.70 (1)	10.00				
J-AB	0/0	-28.0 -28.0 0.45 (1)	10.00				
AB-AC	0/0	-28.0 -28.0 0.45 (1)	10.00				
AC-I	0/0	-28.0 -28.0 0.45 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
J	24-11-4	-1282	-1282	---	FRONT	VERT	TOTAL
N	4-11-4	-2303	-2303	---	FRONT	VERT	TOTAL
R	6-11-4	-1426	-1426	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (1.04')
CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.18')
ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (1.04')
CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.29')

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = $\frac{1}{120}$ (0.19')
CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.00')
ALLOWABLE DEFL.(TL) = $\frac{1}{120}$ (0.19')
CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.00')

CS: TC=0.49 (H-I:1), BC=0.92 (K-L:1), WB=1.00 (G-J:1), SS=0.57 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (G) (INPUT = 0.80)
JSI METAL=0.92 (L) (INPUT = 1.00)



JOB NAME 240105	TRUSS NAME T41	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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Temarack Roof Truss, Burlington

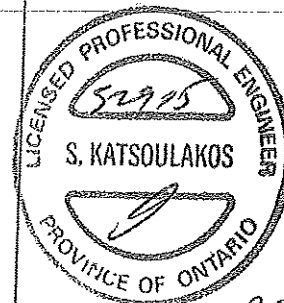
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HANGERS NOTES

- 1) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 4-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 2) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 8-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 3) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 8-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 4) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 10-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 5) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 12-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 6) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 13-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 7) USE USP THD26-2 (WITH 16D NAILS INTO GIRDER & 10D NAILS INTO INCOMING TRUSS) AT 15-0-0 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (2 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 8) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 16-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 9) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 19-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 10) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 20-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 11) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 22-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 12) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 24-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 13) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 26-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 14) USE USP HUS26 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 28-4-12 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD.
- 15) FILL ALL NAIL HOLES.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	GIR.	TYPE
S	8-11-4	-1413	-1413	---	FRONT	VERT	TOTAL
T	10-11-4	-1413	-1413	---	FRONT	VERT	TOTAL
U	12-11-4	-1413	-1413	---	FRONT	VERT	TOTAL
V	13-11-4	-1413	-1413	---	FRONT	VERT	TOTAL
W	15-0-0	-1759	-1759	---	FRONT	VERT	TOTAL
X	16-11-4	-1282	-1282	---	FRONT	VERT	TOTAL
Y	18-11-4	-1282	-1282	---	FRONT	VERT	TOTAL
Z	20-11-4	-1282	-1282	---	FRONT	VERT	TOTAL
AA	22-11-4	-1282	-1282	---	FRONT	VERT	TOTAL
AB	26-11-4	-1282	-1282	---	FRONT	VERT	TOTAL
AC	28-4-12	-1282	-1282	---	FRONT	VERT	TOTAL

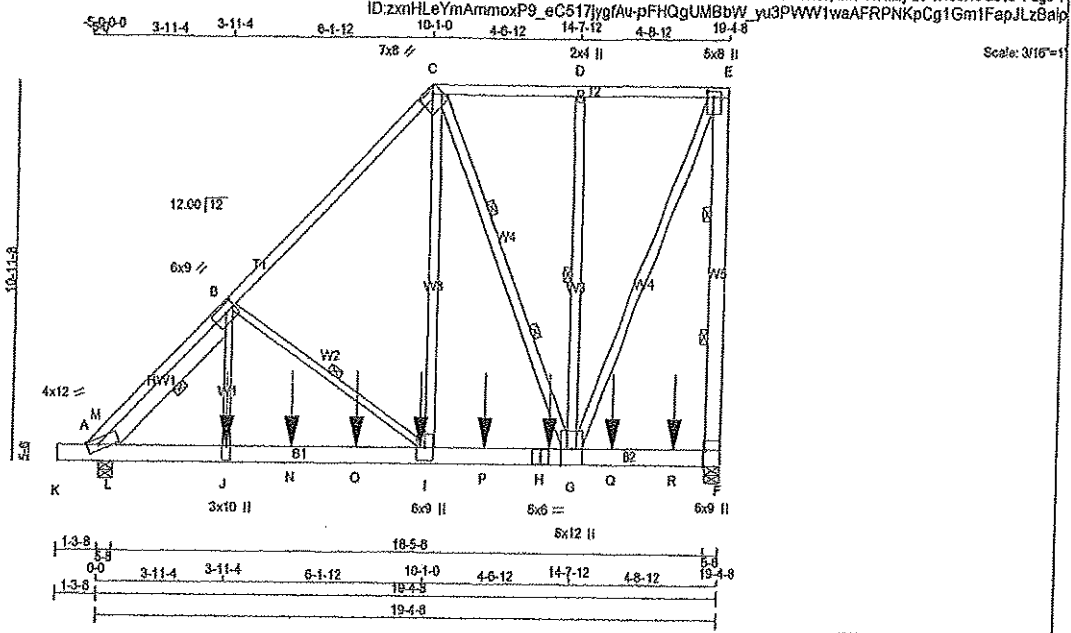


DRWG NO. TAN20101-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T42	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x6	DRY	No.2	SPF	
G - H	2x6	DRY	1650F 1.5E	SPF	
H - F	2x6	DRY	1650F 1.5E	SPF	

REINFORCING MEMBERS	SIZE	DRY	LUMBER	DESCR.
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
J - B	2x3	DRY	No.2	SPF
B - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS: (0.122'x3') SPIRAL NAILS		
A-C	1	12
C-E	1	12
E-G	2	12
BOTTOM CHORDS: (0.122'x3') SPIRAL NAILS		
K-H	2	12
H-F	2	12
WEBS: (0.122'x3') SPIRAL NAILS		
B-J	1	4
2x3	1	12
I-C	1	6
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	BMWV1-m	MT20	4.0	12.0	Edge	4.25
B	TMWVW1	MT20	6.0	9.0	2.75	4.00
C	TTWVW1	MT20	7.0	8.0	1.75	5.00
D	TMVW1	MT20	2.0	4.0		
E	TMVW1	MT20	5.0	8.0	2.25	2.25
F	BMV11	MT20	6.0	9.0	Edge	0.50
G	BMWVW1	MT20	8.0	12.0	Edge	
H	BS1	MT20	5.0	6.0		
I	BMWV1	MT20	6.0	9.0		
J	BMWV1	MT20	3.0	10.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
L(A)	8360	0	8360	0
F	8905	0	8905	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
L(A)	6774	3907 / 0
F	7247	4128 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L(A), F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2 LATERAL BRACE(S) REQUIRED AT 1/3 LENGTH OF E-F, C-G.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-G, B-I, B-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	WEBS	MAX. FACTORED	FACTORED	MAX
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	CS1 (LC)
FR-TO		FROM TO	LENGTH	FR-TO			
A-M	-2457 / 0	-84.3	-84.3 0.25 (1)	I-C	0 / 6792	0.60 (1)	
M-B	-2722 / 0	-84.3	-84.3 0.28 (1)	G-D	-449 / 0	0.14 (1)	
B-C	-6281 / 0	-84.3	-84.3 0.85 (1)	C-G	-3004 / 0	0.47 (1)	
C-D	-3319 / 0	-84.3	-84.3 0.19 (1)	G-E	0 / 8135	0.72 (1)	
D-E	-3319 / 0	-84.3	-84.3 0.19 (1)	J-B	0 / 4625	0.57 (1)	
F-E	-7587 / 0	0.0	0.0 0.62 (1)	B-I	-2799 / 0	0.58 (1)	
K-A	0 / 0	-112.3	-112.3 0.01 (1)	L-M	-555 / 0	0.60 (1)	
A-L	0 / 1905	-28.0	-28.0 0.43 (1)	L-B	-7660 / 0	0.48 (1)	
L-J	0 / 6743	-28.0	-28.0 0.62 (1)				
J-N	0 / 6704	-28.0	-28.0 0.66 (1)				
N-O	0 / 6704	-28.0	-28.0 0.66 (1)				
O-I	0 / 6704	-28.0	-28.0 0.66 (1)				
I-P	0 / 4511	-28.0	-28.0 0.74 (1)				
P-H	0 / 4511	-28.0	-28.0 0.74 (1)				
H-G	0 / 4511	-28.0	-28.0 0.74 (1)				
G-Q	0 / 0	-28.0	-28.0 0.48 (1)				
Q-R	0 / 0	-28.0	-28.0 0.48 (1)				
R-F	0 / 0	-28.0	-28.0 0.43 (1)				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	FACE	TYPE	TOTAL
JT							
H	13-11-4	-1703	-1703	---	FRONT	VERT	TOTAL
I	8-11-4	-1703	-1703	---	FRONT	VERT	TOTAL
J	3-11-4	-2958	-2958	---	FRONT	VERT	TOTAL
N	5-11-4	-1703	-1703	---	FRONT	VERT	TOTAL
O	7-11-4	-1739	-1739	---	FRONT	VERT	TOTAL
P	11-11-4	-1739	-1739	---	FRONT	VERT	TOTAL
Q	15-11-4	-1703	-1703	---	FRONT	VERT	TOTAL
R	17-11-4	-1703	-1703	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 2 X 150 = 300 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.6 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.09/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.66")

CALCULATED VERT. DEFL.(LL) = 1/599 (0.23")

ALLOWABLE DEFL.(TL) = 1/360 (0.66")

CALCULATED VERT. DEFL.(TL) = 1/649 (0.37")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = 1/120 (0.19")

CALCULATED VERT. DEFL.(LL) = 1/599 (0.01")

ALLOWABLE DEFL.(TL) = 1/120 (0.19")

CALCULATED VERT. DEFL.(TL) = 1/655 (0.02")

CS1: TC=0.85 (B-C-1), BC=0.85 (I-J-1), WB=0.72 (E-G-1), SS=0.74 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

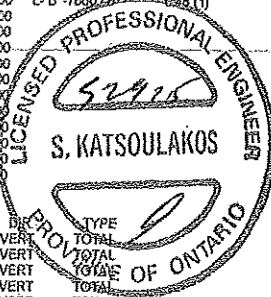
MT20 618 354 1667 822 2284 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (E) (INPUT = 0.90)

JSI METAL = 0.78 (A) (INPUT = 1.00)



DRWG NO. TAM20102-15

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

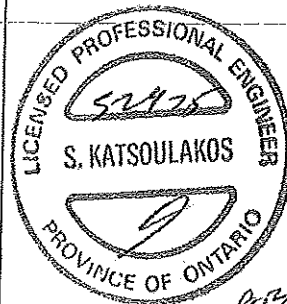
JOB NAME 240105	TRUSS NAME T42	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

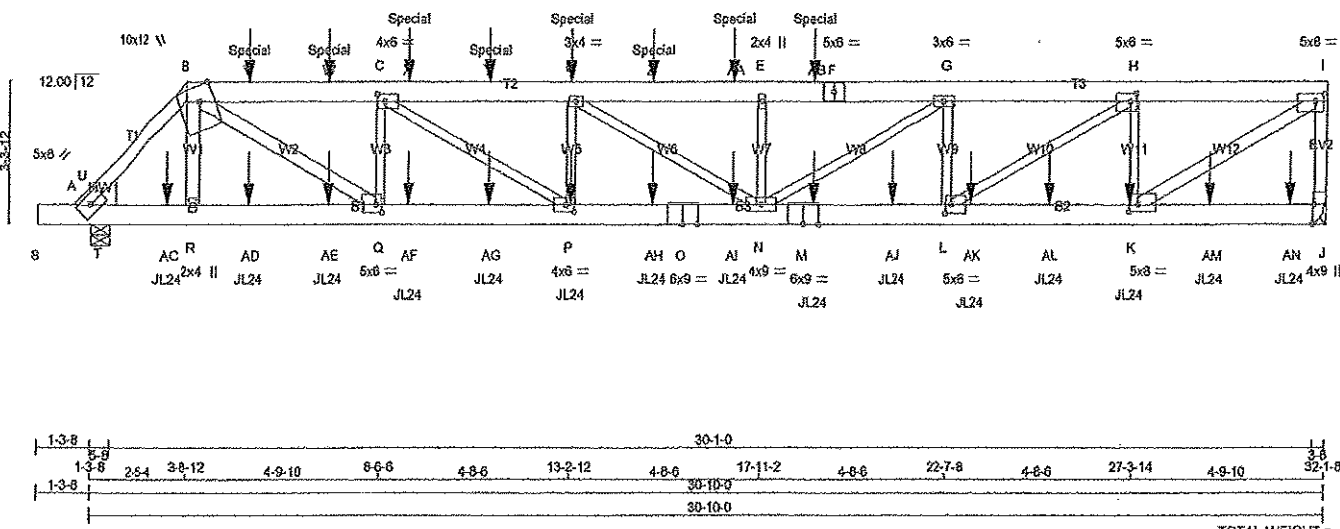
Version 7.010 5 Mar 24 2015 H&T&K Industries, Inc. Fri May 29 17:38:18 2015 Page 2
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2958.4 lbs FACTORED DOWN AT 3-11-4, 1702.9 lbs FACTORED DOWN AT 5-11-4, 1735.7 lbs FACTORED DOWN AT 7-11-4, 1702.9 lbs FACTORED DOWN AT 9-11-4, 1735.7 lbs FACTORED DOWN AT 11-11-4, 1702.9 lbs FACTORED DOWN AT 13-11-4, AND 1702.9 lbs FACTORED DOWN AT 15-11-4, AND 1702.9 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM20102-15
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
S - O	2x6	DRY	1650F 1.5E	SPF
O - M	2x6	DRY	1650F 1.5E	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT		2x3 DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT VERT	DOWN	IN-SX	IN-SX	2x8 L
A 2641 0	2641 0	5-8	3-10	
J 2988 0	2988 0			

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE WIND DEAD
A 2158 1216 / 0	479 / 0 0 / 0 462 / 0
J 2394 1423 / 0	482 / 0 0 / 0 468 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-U	-3403 / 0	-84.3 -84.3 0.19 (1)	3.63	R-B	0 / 291	0.05 (2)	
U-B	-3381 / 0	-84.3 -84.3 0.21 (1)	3.64	B-Q	0 / 3599	0.84 (1)	
B-V	-5492 / 0	-84.3 -84.3 0.41 (1)	3.42	Q-C	-1620 / 0	0.31 (1)	
V-W	-5492 / 0	-84.3 -84.3 0.41 (1)	3.42	C-P	0 / 1918	0.47 (1)	
W-C	-5492 / 0	-84.3 -84.3 0.41 (1)	3.42	P-D	-780 / 0	0.15 (1)	
C-X	-7131 / 0	-84.3 -84.3 0.51 (1)	2.84	D-N	0 / 491	0.12 (1)	
X-Y	-7131 / 0	-84.3 -84.3 0.51 (1)	2.84	N-E	-537 / 0	0.10 (1)	
Y-D	-7131 / 0	-84.3 -84.3 0.51 (1)	2.84	E-G	0 / 1039	0.26 (1)	
D-Z	-7551 / 0	-84.3 -84.3 0.59 (1)	2.78	L-G	-921 / 0	0.18 (1)	
Z-AA	-7551 / 0	-84.3 -84.3 0.59 (1)	2.78	G-H	0 / 2768	0.69 (1)	
AA-E	-7551 / 0	-84.3 -84.3 0.59 (1)	2.78	H-I	-1883 / 0	0.38 (1)	
E-AB	-7551 / 0	-84.3 -84.3 0.57 (1)	2.80	K-I	0 / 5000	0.88 (1)	
AB-F	-7551 / 0	-84.3 -84.3 0.57 (1)	2.80	T-U	-93 / 5	0.00 (1)	
F-G	-7551 / 0	-84.3 -84.3 0.57 (1)	2.80				
G-H	-6683 / 0	-84.3 -84.3 0.44 (1)	3.12				
H-I	-4288 / 0	-84.3 -84.3 0.27 (1)	3.66				
J-I	-2731 / 0	0.0 0.0 0.44 (1)	5.14				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2012
 - CSA 088-09
 - TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.04")
CALCULATED VERT. DEFL. (LL) = $L/120$ (0.42")
ALLOWABLE DEFL. (TL) = $L/360$ (1.04")
CALCULATED VERT. DEFL. (TL) = $L/559$ (0.67")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = $L/120$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/120$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.59 (D-E:1), BC=0.67 (H-P:1), WB=0.88 (I-K:1), SSI=0.20 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

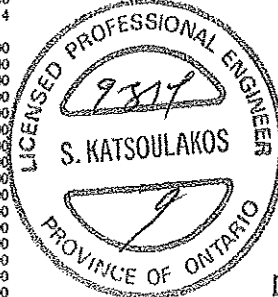
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 616 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (O) (INPUT = 0.80)
 JSI METAL= 0.97 (O) (INPUT = 1.00)

DWG NO TAM3744414 10/12
STRUCTURAL COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T43	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 6) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 11-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 7) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 13-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 8) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 15-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 9) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 17-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 10) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 19-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 11) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 21-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 12) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 23-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 13) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 25-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 14) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 27-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 15) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 29-11-4 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO BACK FACE OF BOTTOM CHORD.
- 16) FILL ALL NAIL HOLES.
- 17) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 63.6 lbs FACTORED DOWN AT 3-11-4, 63.6 lbs FACTORED DOWN AT 5-11-4, 63.6 lbs FACTORED DOWN AT 7-11-4, 63.6 lbs FACTORED DOWN AT 9-11-4, 63.6 lbs FACTORED DOWN AT 11-11-4, 63.6 lbs FACTORED DOWN AT 13-11-4, AND 63.6 lbs FACTORED DOWN AT 15-11-4, AND 63.6 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (3)

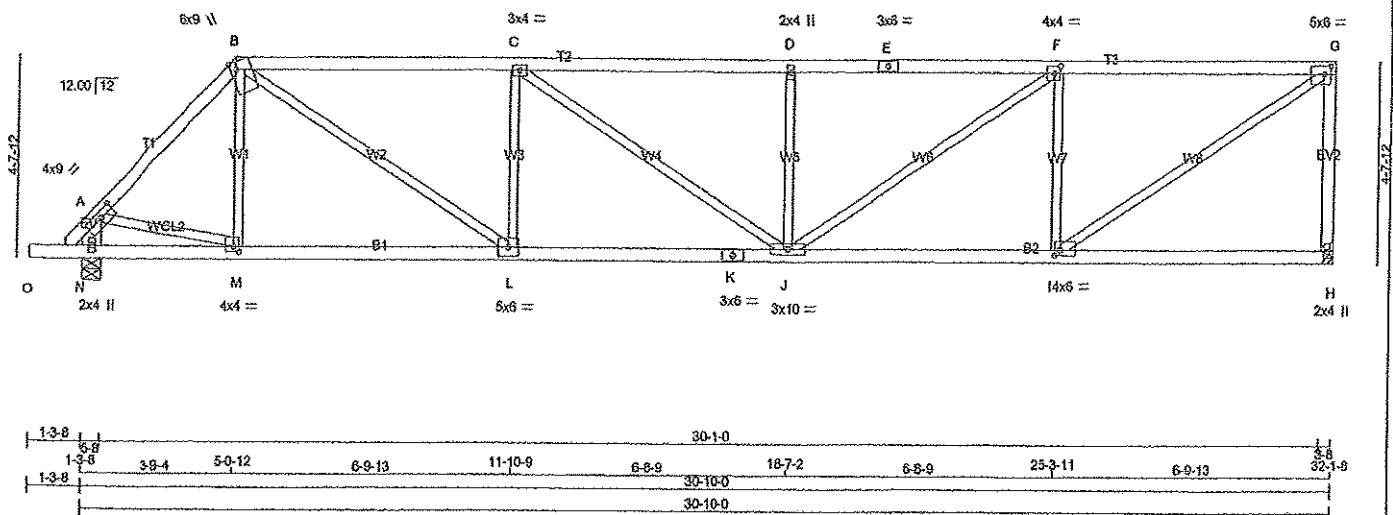
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
AM-AN	0/0	-28.0 -28.0	0.13 (1)	10.00			
AN-J	0/0	-28.0 -28.0	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
D	11-11-4	-64	-64	---	BACK	VERT	TOTAL
K	25-11-4	-190	-190	---	BACK	VERT	TOTAL
M	17-11-4	-42	-42	---	BACK	VERT	TOTAL
P	11-11-4	-42	-42	---	BACK	VERT	TOTAL
V	3-11-4	-64	-64	---	BACK	VERT	TOTAL
W	5-11-4	-64	-64	---	BACK	VERT	TOTAL
X	7-11-4	-64	-64	---	BACK	VERT	TOTAL
Y	9-11-4	-64	-64	---	BACK	VERT	TOTAL
Z	13-11-4	-64	-64	---	BACK	VERT	TOTAL
AA	15-11-4	-64	-64	---	BACK	VERT	TOTAL
AB	17-11-4	-64	-64	---	BACK	VERT	TOTAL
AC	1-11-4	-42	-42	---	BACK	VERT	TOTAL
AD	3-11-4	-42	-42	---	BACK	VERT	TOTAL
AE	5-11-4	-42	-42	---	BACK	VERT	TOTAL
AF	7-11-4	-42	-42	---	BACK	VERT	TOTAL
AG	9-11-4	-42	-42	---	BACK	VERT	TOTAL
AH	13-11-4	-42	-42	---	BACK	VERT	TOTAL
AI	15-11-4	-42	-42	---	BACK	VERT	TOTAL
AJ	19-11-4	-190	-190	---	BACK	VERT	TOTAL
AK	21-11-4	-190	-190	---	BACK	VERT	TOTAL
AL	23-11-4	-190	-190	---	BACK	VERT	TOTAL
AM	27-11-4	-190	-190	---	BACK	VERT	TOTAL
AN	29-11-4	-190	-190	---	BACK	VERT	TOTAL



DWG NO TAM37444.14
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - A	2x6	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	9.0	1.75	4.00
B	TTWW+m	MT20	6.0	9.0	Edge	1.75
C	TMVW-I	MT20	3.0	4.0		
D	TMVW-w	MT20	2.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMVW-I	MT20	4.0	4.0	2.00	1.75
G	TMVW-I	MT20	5.0	6.0	2.25	2.00
H	BMV-I+p	MT20	2.0	4.0		
I	BMVW-I	MT20	4.0	6.0	1.50	1.50
J	BMVW-I	MT20	3.0	10.0		
K	BS-I	MT20	3.0	6.0		
L	BMVW-I	MT20	5.0	6.0		
M	BMVW-I	MT20	4.0	4.0	1.50	1.50
N	BMV-I+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
H	1731	0	1731	0	0	HANGER BY OTHERS			
N	1876	0	1876	0	0	MIN. SEAT SIZE: 2-2	5-8	2-0	

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	WIND	DEAD
H	1421	789 / 0	324 / 0	0 / 0	303 / 0
N	1540	855 / 0	351 / 0	0 / 0	334 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	-1913 / 0	-84.3	-84.3 0.21 (1)	4.70	M-B	-13 / 156	0.04 (2)
B-C	-2877 / 0	-84.3	-84.3 0.65 (1)	3.50	B-L	0 / 1812	0.41 (1)
C-D	-3095 / 0	-84.3	-84.3 0.64 (1)	3.42	L-C	-778 / 0	0.25 (1)
D-E	-3095 / 0	-84.3	-84.3 0.65 (1)	3.42	C-J	0 / 259	0.06 (1)
E-F	-3095 / 0	-84.3	-84.3 0.65 (1)	3.42	J-D	-522 / 0	0.17 (1)
F-G	-2231 / 0	-84.3	-84.3 0.57 (1)	3.99	J-F	0 / 1030	0.23 (1)
H-G	-1853 / 0	0.0	0.0 0.57 (1)	8.45	I-F	-1211 / 0	0.38 (1)
N-A	-1699 / 0	0.0	0.0 0.11 (1)	7.59	I-G	0 / 2647	0.60 (1)
					A-M	0 / 1360	0.31 (1)
O-N	0 / 0	-112.3	-112.3 0.13 (1)	10.00			
N-M	0 / 0	-28.0	-28.0 0.20 (2)	10.00			
M-L	0 / 1347	-28.0	-28.0 0.41 (2)	10.00			
L-K	0 / 2877	-28.0	-28.0 0.59 (1)	10.00			
K-J	0 / 2877	-28.0	-28.0 0.59 (1)	10.00			
J-I	0 / 2231	-28.0	-28.0 0.57 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.31 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.65 (B-C:1), BC=0.59 (J-L:1), WB=0.60 (G-I:1), SSI=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSD) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.75 (K) (INPUT = 1.00)

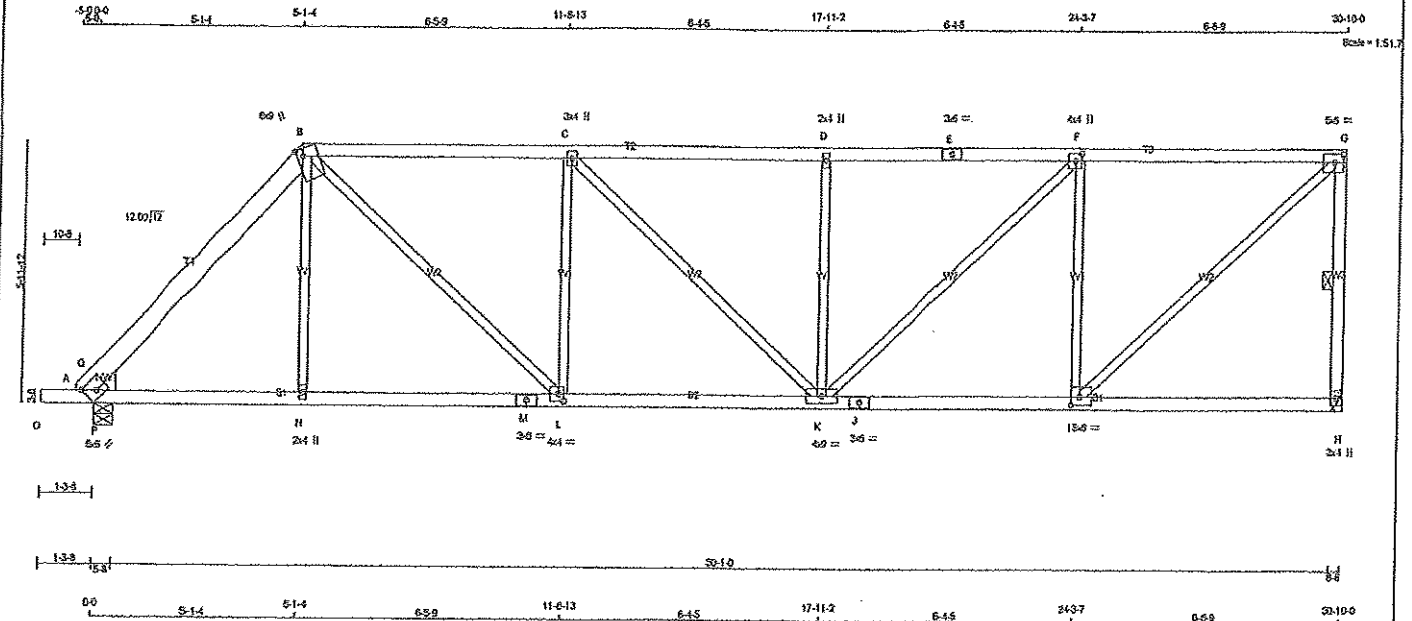


DWG NO T4537446 14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T46	1	1	TRUSS DESC.	

Yamack Roof Truss, Burlington

Version 7.600 6 Oct 3 2014 MTek Industries, Inc. Thu Jan 29 14:41:02 2015 Page 1
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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x6	DRY	No2
B - E	2x4	DRY	No2
E - G	2x4	DRY	No2
H - G	2x4	DRY	No2
O - M	2x4	DRY	No2
M - J	2x4	DRY	No2
J - H	2x4	DRY	No2
ALL WEBS	2x3	DRY	No2
DRY: SEASONED LUMBER			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TBMH1-h	MT20	50	6.0 Edges 3.00
B	TTWW+m	MT20	60	6.0 Edges 1.75
C	TMWW+l	MT20	30	4.0
D	TMWW+r	MT20	20	4.0
E	TS-1	MT20	30	6.0
F	TMWW+l	MT20	40	4.0 2.00 1.75
G	TMWW+l	MT20	50	6.0 2.25 2.50
H	BS-V1+p	MT20	30	4.0
I	BMWW+l	MT20	50	6.0 2.25 2.50
J	BS-1	MT20	30	6.0
K	BMWW+l	MT20	40	9.0
L	BMWW+l	MT20	40	4.0 2.00 1.50
M	BS-1	MT20	30	6.0
N	BMWW+r	MT20	20	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	HEEL	WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	1843	0	1843	0	0	5-5	2-0
H	1764	0	1764	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 2-4	

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
A	1513	840/0	245/0
H	1448	804/0	330/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-Q	-2092/0	-84.3	-84.3 0.18 (1)	5.45	N-B	0/330	0.97 (2)
Q-B	-2092/0	-84.3	-84.3 0.32 (1)	5.25	B-L	0/1193	0.27 (1)
B-C	-2387/0	-84.3	-84.3 0.53 (1)	3.94	L-C	-608/0	0.34 (1)
C-D	-2411/0	-84.3	-84.3 0.50 (1)	3.96	C-K	0/32	0.01 (1)
D-E	-2411/0	-84.3	-84.3 0.51 (1)	3.94	K-D	-492/0	0.27 (1)
E-F	-2411/0	-84.3	-84.3 0.51 (1)	3.94	K-F	0/365	0.22 (1)
F-G	-1692/0	-84.3	-84.3 0.48 (1)	4.58	F-H	-1268/0	0.70 (1)
H-G	-1692/0	0.0	0.0 0.29 (1)	5.11	I-G	0/2242	0.59 (1)
					P-Q	-199/95	0.00 (1)
O-A	0/0	-112.3	-112.3 0.04 (1)	10.00			
A-P	0/1489	-28.0	-28.0 0.45 (1)	10.00			
P-N	0/1489	-28.0	-28.0 0.46 (1)	10.00			
N-M	0/1493	-28.0	-28.0 0.42 (1)	10.00			
M-L	0/1493	-28.0	-28.0 0.42 (1)	10.00			
L-K	0/2308	-28.0	-28.0 0.50 (2)	10.00			
K-J	0/1692	-28.0	-28.0 0.49 (2)	10.00			
J-I	0/1692	-28.0	-28.0 0.49 (2)	10.00			
I-H	0/0	-28.0	-28.0 0.29 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012
- CSA S809
- TRC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/660 (1.05)
CALCULATED VERT. DEFL. (LL) = L/999 (0.16)
ALLOWABLE DEFL. (TL) = L/660 (1.05)
CALCULATED VERT. DEFL. (TL) = L/999 (0.26)

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = L/120 (0.19)
CALCULATED VERT. DEFL. (LL) = L/999 (0.00)
ALLOWABLE DEFL. (TL) = L/120 (0.19)
CALCULATED VERT. DEFL. (TL) = L/999 (0.00)

CSL TO=0.53 (B-C), BC=0.60 (K-L), WB=0.70 (F-I),
SSI=0.26 (F-G-1)

DOL LUMBER=1.00 NAL=1.00LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAL VALUES
PLATE GRP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1807 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRP= 0.50 (1) (INPUT = 0.50)
JSI METAL = 0.44 (1) (INPUT = 1.00)

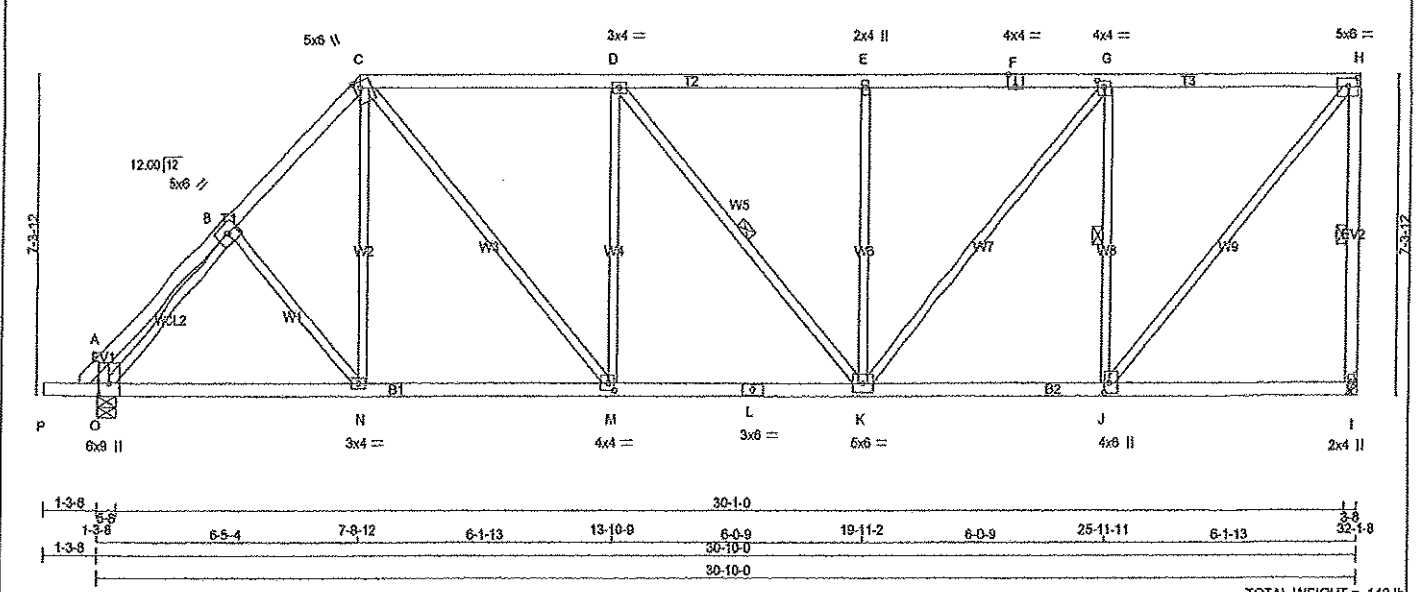


DWG NO. TAM 2462-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 7.500 S Oct 23 2013 15Tek Industries, Inc. Mon Sep 08 15:28:33 2014 Page 1
ID:zxnHLeYmAmnoXp9_eC517jygfAu-mTK7InLhs4aILy2MvYIKqXR6pk6ihUFnUFYPxyfYs
32-1-8
Scale = 1:51.4



LUMBER
N. L. G. A. RULES
CHORDS
SIZE
LUMBER
DESCR.
SPF
ALL WEBS
2x3
DRY
No.2
SPF
EXCEPT
DRY: SEASONED LUMBER.

TOTAL WEIGHT = 142 lb

CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY No.2	SPF	
C - F	2x4	DRY No.2	SPF	
F - H	2x4	DRY No.2	SPF	
I - H	2x4	DRY No.2	SPF	
O - A	2x4	DRY No.2	SPF	
P - L	2x4	DRY No.2	SPF	
L - I	2x4	DRY No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	THWV-1	MT20	5.0	6.0		
C	THWV-1	MT20	5.0	6.0	1.75	1.75
D	THWV-1	MT20	3.0	4.0		
E	THWV-1	MT20	2.0	4.0		
F	TS-1	MT20	4.0	4.0	Edge	2.00
G	THWV-1	MT20	4.0	4.0	1.75	2.00
H	THWV-1	MT20	5.0	6.0	2.25	2.75
I	BMV1+P	MT20	2.0	4.0		
J	BMVWV-1	MT20	4.0	6.0	2.75	1.50
K	BMVWV-1	MT20	5.0	6.0		
L	BS-1	MT20	3.0	6.0		
M	BMVWV-1	MT20	4.0	4.0	2.00	1.75
N	BMVWV-1	MT20	3.0	4.0		
O	THWV1+P	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	VERT	HORZ	DOWN	UP	BRG
I	1731	0	1731	0	0
O	1876	0	1876	0	0

UNFACTORED REACTIONS					
JT	COMBINED	SNOW	LIVE	WIND	DEAD
I	1421	789 / 0	324 / 0	0 / 0	308 / 0
O	1540	855 / 0	351 / 0	0 / 0	334 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H-I, D-K, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS						WEBS					
MAX. FACTORED		FACTORED				MAX. FACTORED					
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)			
FR-TO		FROM	TO			FR-TO					
A-B	0 / 19	-84.3	-84.3	0.12 (1)	10.00	B-N	-82 / 17	0.03 (1)			
B-C	-1830 / 0	-84.3	-84.3	0.14 (1)	4.85	N-C	0 / 339	0.08 (2)			
C-D	-1894 / 0	-84.3	-84.3	0.44 (1)	4.44	C-M	0 / 931	0.21 (1)			
D-E	-1857 / 0	-84.3	-84.3	0.41 (1)	4.60	M-D	-535 / 0	0.51 (1)			
E-F	-1857 / 0	-84.3	-84.3	0.41 (1)	4.51	D-K	-57 / 0	0.03 (1)			
F-G	-1857 / 0	-84.3	-84.3	0.41 (1)	4.51	K-E	-469 / 0	0.44 (1)			
G-H	-1276 / 0	-84.3	-84.3	0.40 (1)	5.23	K-G	0 / 891	0.20 (1)			
H-I	-1652 / 0	0.0	0.0	0.40 (1)	5.15	J-G	-1262 / 0	0.43 (1)			
O-A	-108 / 0	0.0	0.0	0.01 (1)	7.81	J-H	0 / 1836	0.44 (1)			
						O-B	-2042 / 0	0.78 (1)			
P-O	0 / 0	-112.3	-112.3	0.13 (1)	10.00						
O-N	0 / 1330	-28.0	-28.0	0.43 (2)	10.00						
N-M	0 / 1279	-28.0	-28.0	0.43 (2)	10.00						
M-L	0 / 1894	-28.0	-28.0	0.41 (1)	10.00						
L-K	0 / 1894	-28.0	-28.0	0.41 (1)	10.00						
K-J	0 / 1278	-28.0	-28.0	0.41 (2)	10.00						
J-I	0 / 0	-28.0	-28.0	0.26 (2)	10.00						

PROFESSIONAL ENGINEER

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, BCBC 2012, ABC 2012
-CSA 888-09
-TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 28.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/899 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/899 (0.20")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/899 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/899 (0.01")

CSI: TC=0.44 (C-D:1), BC=0.43 (M-N:2), WB=0.78 (B-O:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

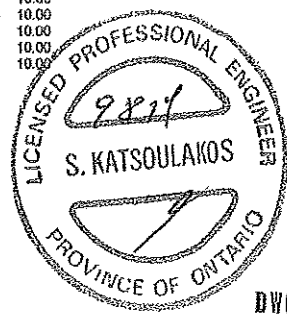
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	018	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.99 (L) (INPUT = 1.00)



DRG NO T474814
STRUCTURAL
COMPONENT ONLY

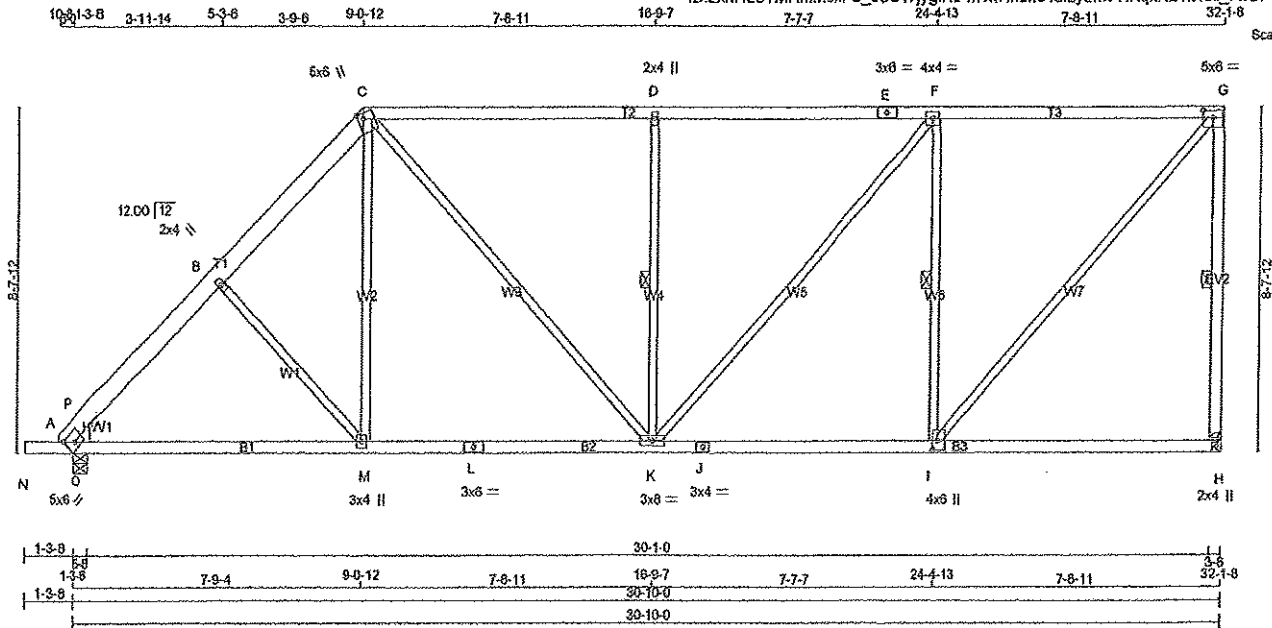
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T48	1	1	TRUSS DESC.	

Pamarack Roof Truss, Burlington

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ID:zxHLeYmAmoxP9_eC517jygfAu-mTk7nLhs4ally2MvYIKqR21k4cl_FnUFYPxyfyS

Scale = 1:50.5



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBMH-h	MT20	5.0	6.0	Edge 3.00
B	TBMW-w	MT20	2.0	4.0	
C	TTWW-w	MT20	5.0	6.0	1.75 1.75
D	TBMW-w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TBMW-w	MT20	4.0	4.0	
G	TBMW-w	MT20	5.0	6.0	2.25 3.00
H	BMW-t	MT20	2.0	4.0	
I	BMW-w	MT20	4.0	6.0	2.75 1.50
J	BS-t	MT20	3.0	4.0	
K	BMW-w	MT20	3.0	6.0	
L	BS-t	MT20	3.0	4.0	
M	BMW-w	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG		HEEL WEDGE	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX	2x8 L	2x8 L
A	1843	0	1843	0	0	5-8	2-0				
H	1764	0	1764	0	0						

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	WIND	DEAD
A	1513	840/0	345/0	0/0	328/0
H	1448	804/0	330/0	0/0	314/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

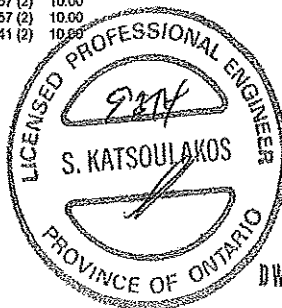
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-H, D-K, F-L.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MEMB.	MAX FACTORED FORCE (LBS)	MAX UNBRACED LENGTH (FT)	MAX FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
A-P	-2344/0	-84.3 -84.3	0.23 (1)	5.13	B-M	-406/0	0.23 (1)
P-B	-2151/0	-84.3 -84.3	0.25 (1)	5.28	M-C	0/575	0.13 (2)
B-C	-1933/0	-84.3 -84.3	0.13 (1)	5.68	C-K	0/583	0.13 (1)
C-D	-1728/0	-84.3 -84.3	0.68 (1)	4.21	K-D	-669/0	0.33 (1)
D-E	-1728/0	-84.3 -84.3	0.60 (1)	4.36	K-F	0/623	0.14 (1)
E-F	-1728/0	-84.3 -84.3	0.60 (1)	4.36	F-G	-1171/0	0.58 (1)
F-G	-1309/0	-84.3 -84.3	0.64 (1)	4.75	F-G	0/1928	0.43 (1)
H-G	-1678/0	0.0 0.0	0.57 (1)	5.13	O-P	0/380	0.00 (1)
N-A	0/0	-112.3 -112.3	0.04 (1)	10.00			
A-O	0/1604	-28.0 -28.0	0.37 (2)	10.00			
O-M	0/1604	-28.0 -28.0	0.58 (2)	10.00			
M-L	0/1330	-28.0 -28.0	0.53 (2)	10.00			
L-K	0/1330	-28.0 -28.0	0.53 (2)	10.00			
K-J	0/1309	-28.0 -28.0	0.57 (2)	10.00			
J-I	0/1309	-28.0 -28.0	0.57 (2)	10.00			
I-H	0/0	-28.0 -28.0	0.41 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(5% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.05')
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14')
 ALLOWABLE DEFL.(TL) = $L/360$ (1.05')
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.23')

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = $L/120$ (0.19')
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00')
 ALLOWABLE DEFL.(TL) = $L/120$ (0.19')
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00')

CSI: TC=0.68 (C-D:1), BC=0.57 (I-K:2), WB=0.58 (F-I:1), SSI=0.30 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	616	354	1697
	622	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
 JSI METAL= 0.48 (A) (INPUT = 1.00)

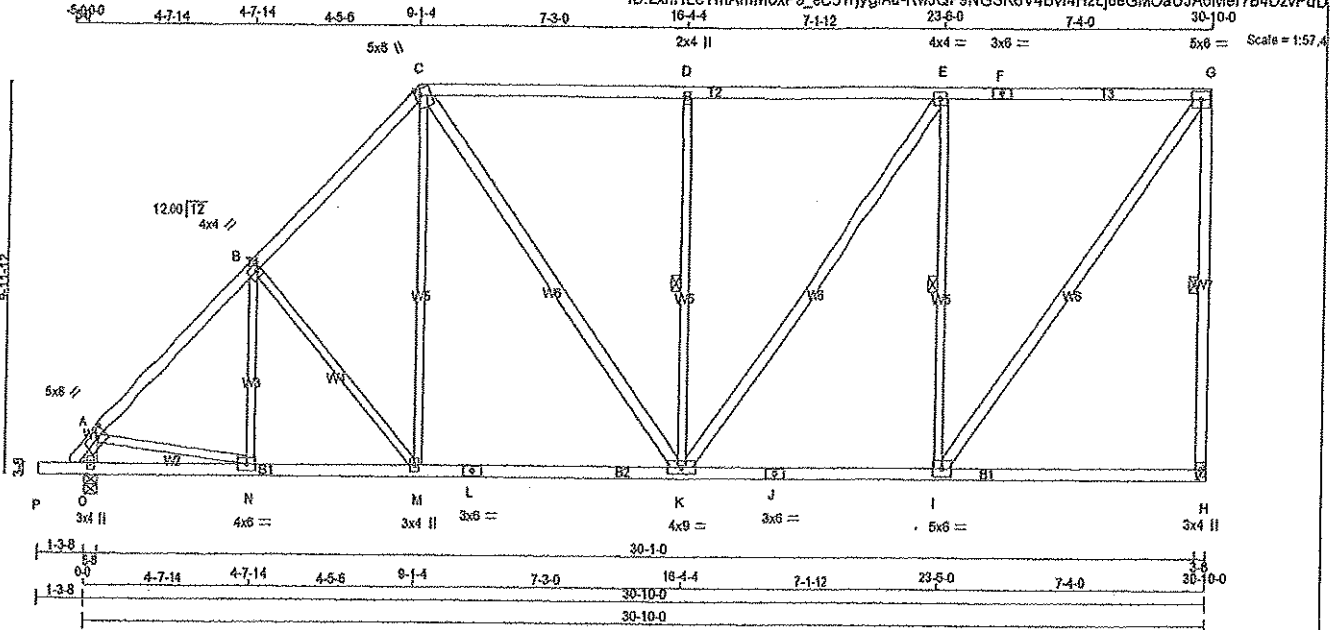
DWG NO TAM 3744914
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T49	3	1	TRUSS DESC.	

Tamareck Roof Truss, Burlington

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ID:zmHLeYmAmnoxP9_eC517jyg/Au-RwJQF9NGSK6V4Bv4HzLJeeGMOaUJA6Mer7B4DzvPdD



TOTAL WEIGHT = 3 X 164 = 493 lb

LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS					
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
O - A	2x6	DRY	No.2	SPF	
P - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
C - K	2x4	DRY	No.2	SPF	
K - E	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMWV-I	MT20	5.0	8.0	2.50	3.00
B TMWV-I	MT20	4.0	4.0	2.00	1.00
C TTWV+m	MT20	5.0	6.0	2.00	1.50
D TMWV-w	MT20	2.0	4.0		
E TMWV-I	MT20	4.0	4.0		
F TS-I	MT20	3.0	6.0		
G TMWV-I	MT20	5.0	6.0		
H BMV1+p	MT20	3.0	4.0		
I BMWV-I	MT20	5.0	6.0		
J BS-I	MT20	3.0	6.0		
K BMWV-I	MT20	4.0	9.0		
L BS-I	MT20	3.0	6.0		
M BMWV-I	MT20	3.0	4.0		
N BMWV-I	MT20	4.0	6.0		
O BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1731	0	0	0
H	1731	0	0	0
O	1876	0	0	0

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
O	1540	855 / 0	351 / 0	0 / 0	0 / 0	334 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF G-H, E-I, D-K. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	FR-TO	FR-TO
A-B	-1893 / 0	-84.3 -84.3 0.29 (1)
B-C	-1873 / 0	-84.3 -84.3 0.27 (1)
C-D	-1427 / 0	-84.3 -84.3 0.56 (1)
D-E	-1428 / 0	-84.3 -84.3 0.51 (1)
E-F	-1080 / 0	-84.3 -84.3 0.55 (1)
F-G	-1060 / 0	-84.3 -84.3 0.55 (1)
G-H	-1049 / 0	0.0 0.0 0.80 (1)
O-A	-1673 / 0	0.0 0.0 0.11 (1)
P-O	0 / 0	-112.3 -112.3 0.13 (1)
O-N	0 / 0	-28.0 -28.0 0.13 (2)
N-M	0 / 1387	-28.0 -28.0 0.38 (2)
M-L	0 / 1161	-28.0 -28.0 0.41 (2)
L-K	0 / 1161	-28.0 -28.0 0.41 (2)
K-J	0 / 1080	-28.0 -28.0 0.49 (2)
J-I	0 / 1080	-28.0 -28.0 0.49 (2)
I-H	0 / 0	-28.0 -28.0 0.36 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012
- CSA 688-08
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = L/120 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/120 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.80 (G-H:1), BC=0.49 (I-K:2), WB=0.79 (E-I:1), SS=0.29 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

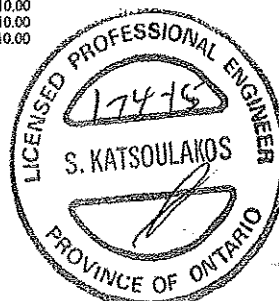
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

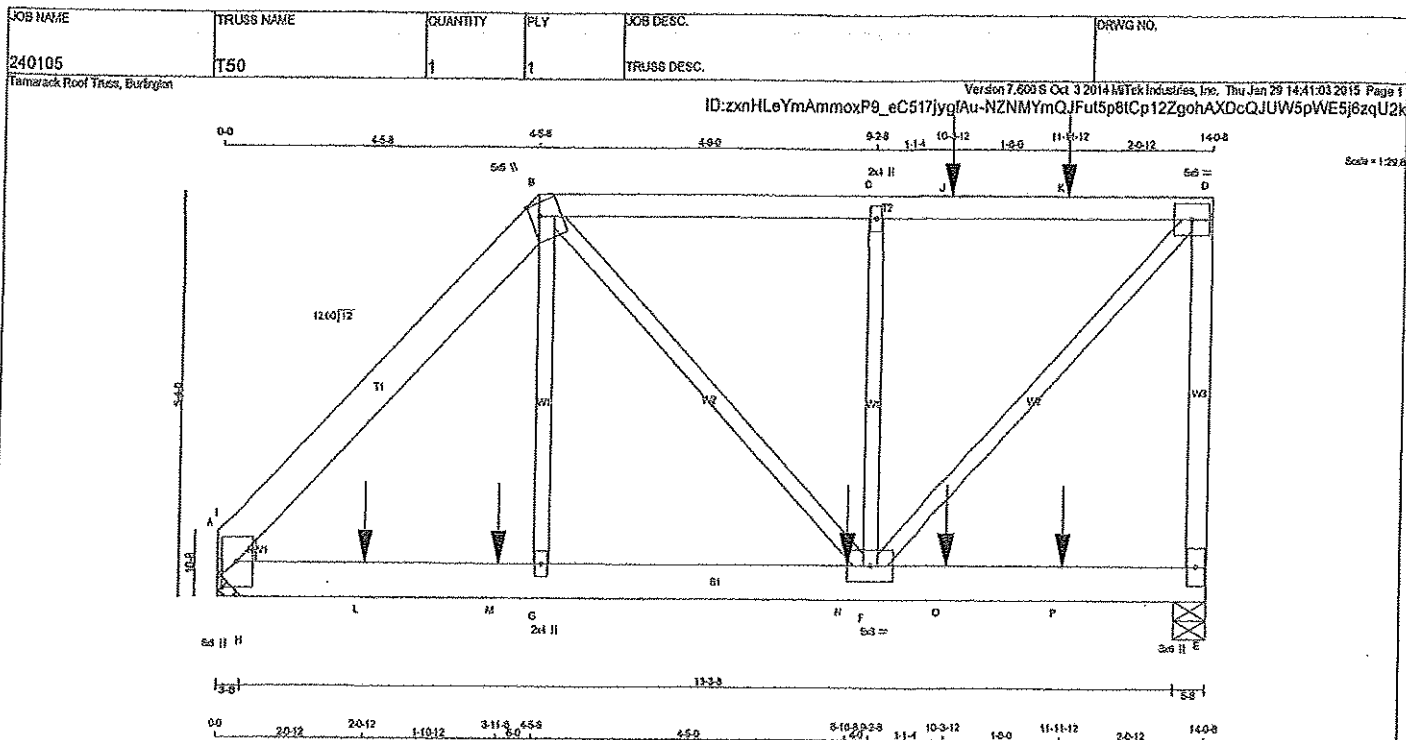
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.38 (A) (INPUT = 1.00)



DRG NO. TAM37450-181R-1
STRUCTURAL
COMPONENT ONLY



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x6	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
A - E	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	UP/LIFT			
E	1471	0	1471	0	5.8	1-10	
A	1425	0	1425	0			2x4 L

HANGER BY OTHERS 2x4 L

MIN. SEAT SIZE: 1-9

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. & MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PER. LIVE	WIND	DEAD	SOIL
E	1184	685/0	257/0	0/0	0/0	252/0	0/0
A	1165	654/0	261/0	0/0	0/0	251/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FORCE (LBS)	CHORDS		WEBS	
		MAX. FACTORED	FACTORED	MAX. FACTORED	MAX. FACTORED
FR-TO					
A-I	-1884/0	-84.3	-84.3 0.10 (1)	5.71	0.8
I-B	-1495/0	-84.3	-84.3 0.11 (1)	6.23	0.7
B-C	-1141/0	-84.3	-84.3 0.50 (1)	5.18	0.6
C-J	-1142/0	-84.3	-84.3 0.50 (1)	5.18	0.6
J-K	-1142/0	-84.3	-84.3 0.50 (1)	5.18	0.6
K-D	-1142/0	-84.3	-84.3 0.50 (1)	5.18	0.6
E-D	-1395/0	0.0	0.0 0.65 (1)	6.85	0.0
A-H	0/1031	-28.0	-28.0 0.10 (1)	10.00	0.0
H-L	0/1031	-28.0	-28.0 0.28 (1)	10.00	0.0
L-M	0/1031	-28.0	-28.0 0.28 (1)	10.00	0.0
M-G	0/1031	-28.0	-28.0 0.26 (1)	10.00	0.0
G-N	0/1045	-28.0	-28.0 0.26 (1)	10.00	0.0
N-F	0/1045	-28.0	-28.0 0.26 (1)	10.00	0.0
F-O	0/0	-28.0	-28.0 0.12 (2)	10.00	0.0
O-P	0/0	-28.0	-28.0 0.12 (2)	10.00	0.0
P-E	0/0	-28.0	-28.0 0.12 (2)	10.00	0.0

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
J	10-3-12	-70	-70		BACK	VERT	TOTAL
K	11-11-12	-70	-70		BACK	VERT	TOTAL
L	2-0-12	-35	-35		BACK	VERT	TOTAL
M	3-11-8	-410	-410		BACK	VERT	TOTAL
N	8-10-8	-410	-410		BACK	VERT	TOTAL
O	10-3-12	-35	-35		BACK	VERT	TOTAL
P	11-11-12	-35	-35		BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH LL = 25.6 PSF

DL = 3.0 PSF

BOT CH LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD ORDER ***

ADDTL USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2,4

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012

- CSA 006-09

- TPC 2011

(65% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/260 (0.47")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.03")

ALLOWABLE DEFL.(TL) = 1/260 (0.47")

CALCULATED VERT. DEFL.(TL) = 1/999 (0.05")

CSL: TC=0.65 (D-E-1), BC=0.26 (F-G-1), WB=0.41 (D-F-1), SS=0.35 (G-H-1)

DCL LUMBER=1.00 NAL=1.00 LS BEND=1.00 COLP=1.00

SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

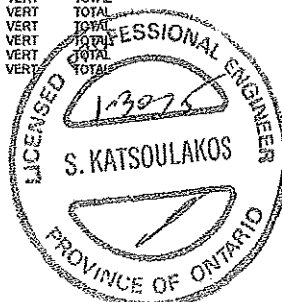
PLATE	GRP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	616 354 1687 622 2204 1555		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.80)

JSI METAL= 0.31 (F) (INPUT = 1.00)

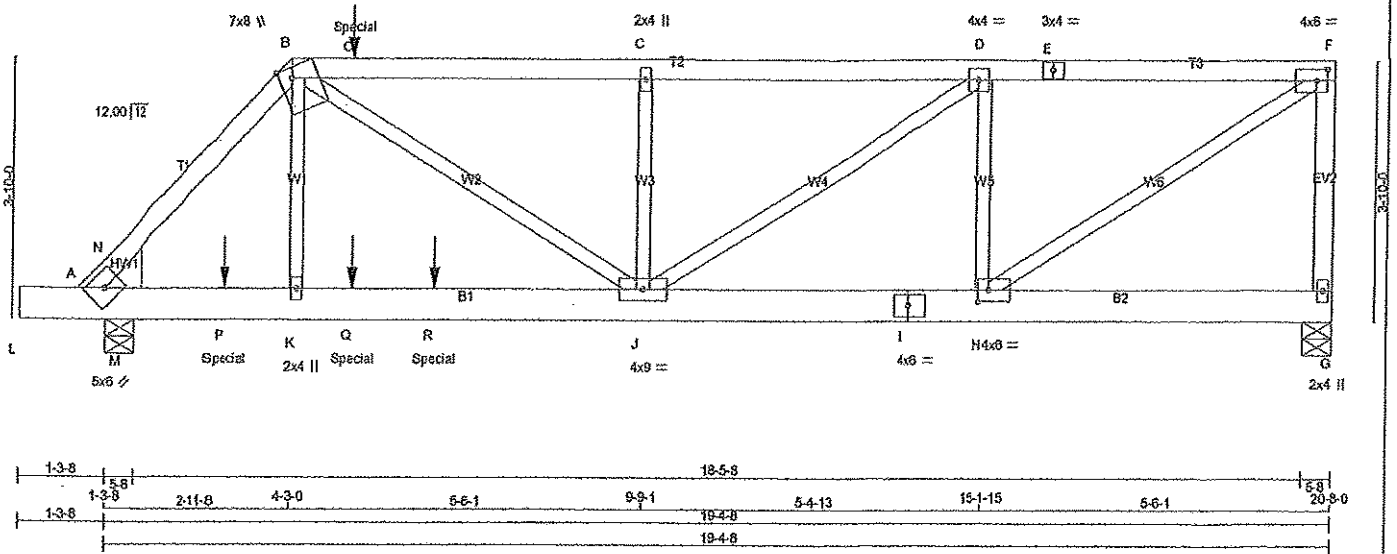


DWG NO. TAM 2403-15

STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T51	1	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					
Version 7.500 S Oct 23 2013 MTEK Industries, Inc. Mon Sep 08 15:28:34 2014 Page 1					
ID:zxHLeYmAmMoxP8_eC517ygfAu-EfIVy6MjdOcy6dYTFpZN8_GR7OqRDUP28_5xNyffYR					
10-8-3-8	2-11-8	4-3-0	5-6-1	9-9-1	5-4-13
15-1-15	5-6-1	20-8-0	Scale = 1:33.2		



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY No.2	SPF
B - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A TBMH1-h	MT20	5.0	6.0	
B TTWW+m	MT20	7.0	8.0	Edge 2.25
C TMWw	MT20	2.0	4.0	
D TMWw	MT20	4.0	4.0	
E TS-1	MT20	3.0	4.0	
F TMWw	MT20	4.0	6.0	2.00 2.00
G BAW1-p	MT20	2.0	4.0	
H BMWw	MT20	4.0	6.0	2.00 2.00
I BS-1	MT20	4.0	6.0	
J BMWw	MT20	4.0	9.0	
K BMWw	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 40.4 lbs FACTORED DOWN AT 3-11-4 ON TOP CHORD, AND 16.0 lbs FACTORED DOWN AT 1-11-4, AND 16.0 lbs FACTORED DOWN AT 3-11-4, AND 620.0 lbs FACTORED DOWN AT 5-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	GROSS REACTION	BRG	HEEL
A	1700	0	1700	0	5-8
G	1299	0	1299	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	WIND	DEAD
A	1385	783 / 0	394 / 0	0 / 0	295 / 0	
G	1082	598 / 0	237 / 0	0 / 0	228 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-N	-2079 / 0	K-B	0 / 710
N-B	-2147 / 0	B-J	0 / 609
B-O	-2290 / 0	J-C	-498 / 0
O-C	-2290 / 0	C-D	0 / 766
C-D	-2290 / 0	D-E	-916 / 0
D-E	-1845 / 0	E-F	0 / 1942
E-F	-1845 / 0	F-G	-238 / 0
F-G	-1222 / 0		
L-A	0 / 0		
A-M	0 / 1497		
M-P	0 / 1497		
P-K	0 / 1497		
K-Q	0 / 1519		
Q-R	0 / 1519		
R-J	0 / 1519		
J-I	0 / 1645		
I-H	0 / 1645		
H-G	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	3-11-4	-40	-40	-	BACK	VERT	TOTAL
P	1-11-4	-9	-16	-	BACK	VERT	TOTAL
Q	3-11-4	-9	-16	-	BACK	VERT	TOTAL
R	5-2-8	-620	-620	-	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2012
 - CSA 088-09
 - TPC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (0.66")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.51 (B-C:1), BC=0.83 (J-K:1), WB=0.48 (F-H:1), GS=0.33 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 522 2264 1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
 JSI METAL= 0.44 (A) (INPUT = 1.00)



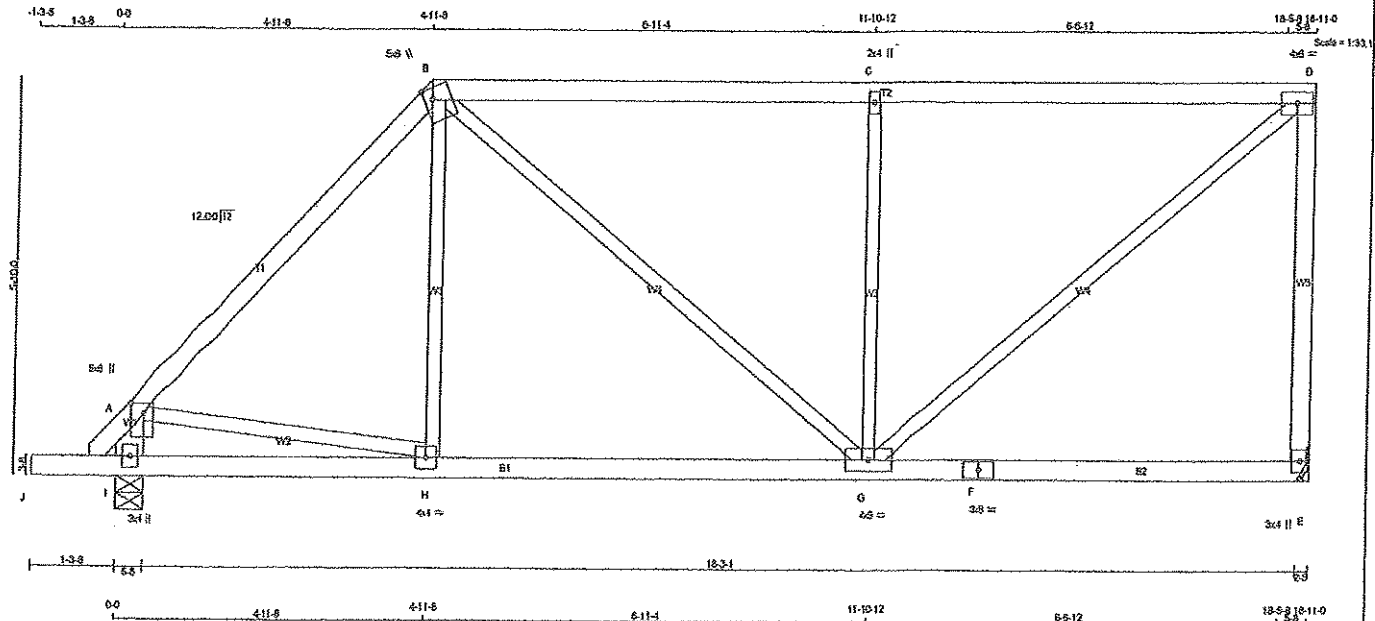
DWG NO T&M37452-14
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T52	1	1	TRUSS DESC.	

Tamersack Roof Truss, Burlington

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ID:zxnhLeYmAmnoxP9_eC517jygfAuITawgmlaoVWV1qfTDnLoMoTp8HUAJZ1BerSlkSzZ6xo



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2	
B - D	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	
I - A	2x6	DRY	No.2	
J - F	2x4	DRY	No.2	
F - E	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	50	6.0	1.75	2.75
B	TMWV+m	MT20	50	6.0	2.00	1.50
C	TMWVw	MT20	20	4.0		
D	TMWV4	MT20	40	6.0		
E	BMV1+p	MT20	30	4.0		
F	BS-4	MT20	30	6.0		
G	BMWVW-t	MT20	40	9.0		
H	BMWV1	MT20	40	4.0		
I	BMV1+p	MT20	30	4.0		

DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UP/LIFT
E	1062	0	0
I	1207	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE	MAX. LIVE	PERMANENT	WIND	DEAD	SOL
JT COMBINED	SNOW	LIVE	PERMANENT	WIND	DEAD
E	672	494/0	169/0	0/0	169/0
I	991	550/0	228/0	0/0	215/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI(LC)
FR-TO		FROM TO			LEN3TH	FR-TO	
A-B	-1022/0	-84.3	-84.3	0.29 (1)	5.56	H-B	0/269
B-C	-560/0	-84.3	-84.3	0.54 (1)	5.53	A-D	0/724
C-D	-961/0	-84.3	-84.3	0.55 (1)	5.52	G-H	0/124
E-D	-963/0	0.0	0.0	0.62 (1)	7.81	B-G	0/354
I-A	-1038/0	0.0	0.0	0.06 (1)	7.81	G-C	-727/0
J-I	0/0	-112.3	-112.3	0.13 (1)	10.00		
I-H	0/0	-28.0	-28.0	0.21 (2)	10.00		
H-G	0/723	-28.0	-28.0	0.43 (2)	10.00		
G-F	0/0	-28.0	-28.0	0.34 (3)	10.00		
F-E	0/0	-28.0	-28.0	0.34 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH LL = 25.6 PSF
DOT CH LL = 3.0 PSF
DOT CH LL = 10.5 PSF
DOT CH LL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012
- CSA 688-09
- TPC 2011

55% OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.63)
CALCULATED VERT. DEFL. (LL) = 1/999 (0.09)
ALLOWABLE DEFL. (TL) = 1/360 (0.63)
CALCULATED VERT. DEFL. (TL) = 1/999 (0.15)

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = 1/120 (0.19)
CALCULATED VERT. DEFL. (LL) = 1/599 (0.01)
ALLOWABLE DEFL. (TL) = 1/120 (0.19)
CALCULATED VERT. DEFL. (TL) = 1/599 (0.01)

CSI TO=0.62 (D-E-1), BC=0.43 (G-H-2), WB=0.38 (C-G-1),
SSI=0.29 (C-D-1)

DOL LUMBER=1.00 MAX L=1.00 LB BEND=1.10 COM/P=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.77 (0) (INPUT = 0.90)
JSI METAL = 0.27 (0) (INPUT = 1.00)



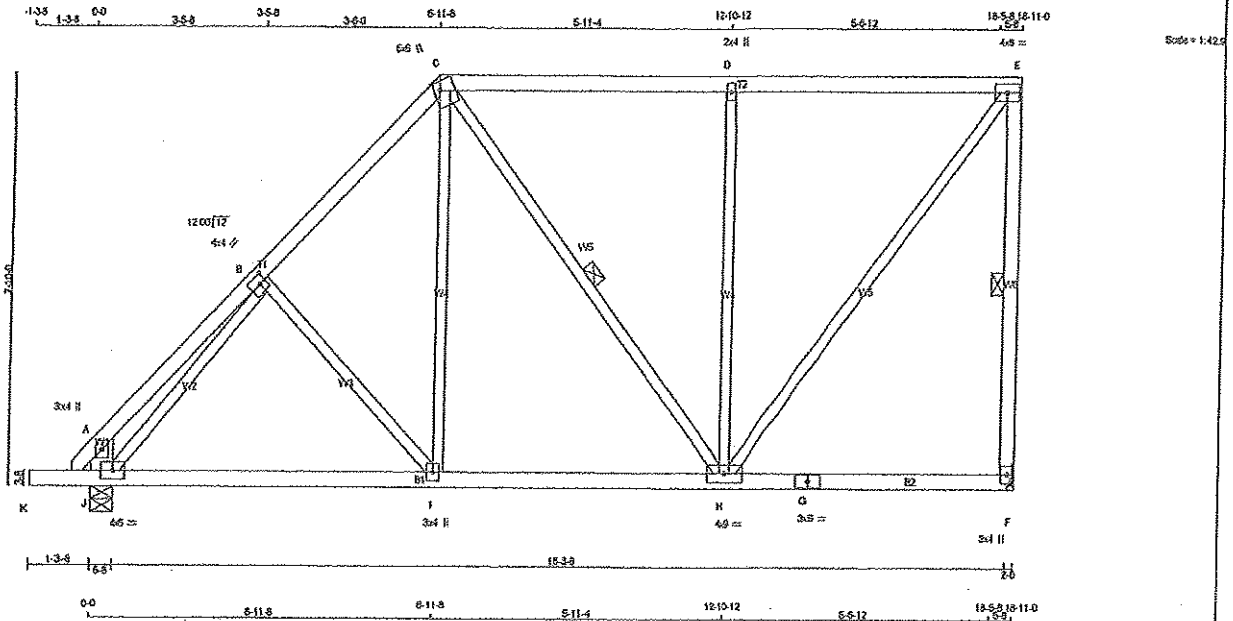
DWG NO. TAM 16042-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T53	1	1	TRUSS DESC.	

Tamarrack Roof Truss, Burlington

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ID:zxHLEymAmnoxP9_eC517jygfAu4TawgmiaoWW1qTDnLoMoTsoHVeJToBerSikSZ26xo



TOTAL WEIGHT = 63 LB

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
E - G	2x4	DRY	No.2	
G - I	2x4	DRY	No.2	
J - A	2x4	DRY	No.2	
K - G	2x4	DRY	No.2	
G - F	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVtp	MT20	30	4.0		
B	TMVW1	MT20	40	4.0	2.00	1.50
C	TTWV1m	MT20	50	6.0	2.00	1.50
D	TMVW1	MT20	20	4.0		
E	TMVW1	MT20	40	6.0		
F	BMV1tp	MT20	30	4.0		
G	BS1	MT20	30	6.0		
H	BMVW1	MT20	40	9.0		
I	BMVW1	MT20	30	4.0		
J	BMVW1	MT20	40	6.0		

DESIGNER: UNFACTORED REACTIONS

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	1062	0	1062	0	0
J	1207	0	1207	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERALIVE	WIND	DEAD	SOL
F	872	484/0	169/0	0/0	0/0	169/0	0/0
J	991	650/0	226/0	0/0	0/0	215/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-F, C-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX
		(LBS)	(PLF)			(LBS)	(LBS)			(LBS)	(PLF)			(LBS)
FR-TO								FR-TO						
A-B	0/24	-24.3	-24.3	0.16 (1)	10.00	I-C	0/389	0.09 (2)						
B-C	-924/0	-24.3	-24.3	0.14 (1)	6.25	H-E	0/1022	0.23 (1)						
C-D	-537/0	-24.3	-24.3	0.39 (1)	6.25	C-H	-49/0	0.03 (3)						
D-E	-638/0	-24.3	-24.3	0.39 (1)	8.25	H-D	-623/0	0.72 (1)						
E-F	-992/0	0.0	0.0	0.27 (1)	8.25	B-I	-173/15	0.03 (1)						
J-A	-106/0	0.0	0.0	0.01 (1)	7.81	J-B	-1158/0	0.48 (1)						
K-J	0/0	-112.3	-112.3	0.13 (1)	10.00									
J-I	0/756	-28.0	-28.0	0.38 (2)	10.00									
I-H	0/639	-28.0	-28.0	0.38 (2)	10.00									
H-G	0/0	-28.0	-28.0	0.21 (3)	10.00									
G-F	0/0	-28.0	-28.0	0.21 (3)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.60/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012
- CSA 688-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =	1/380 (0.63)
CALCULATED VERT. DEFL. (LL) =	1/999 (0.10)
ALLOWABLE DEFL. (TL) =	1/380 (0.63)
CALCULATED VERT. DEFL. (TL) =	1/999 (0.17)

CANTILEVER DEFLECTION:

ALLOWABLE DEFL. (LL) =	1/120 (0.19)
CALCULATED VERT. DEFL. (LL) =	1/999 (0.01)
ALLOWABLE DEFL. (TL) =	1/120 (0.19)
CALCULATED VERT. DEFL. (TL) =	1/999 (0.01)

CSL TO = 0.39 (D-E-I), BC = 0.38 (J-2), WB = 0.72 (D-H-I),
SSI = 0.25 (D-E-I)

DOL LUMBER = 1.00 NAIL = 1.00 LS BEND = 1.10 COMP = 1.10

SHEAR = 1.10 TENS = 1.10

COMPARISON LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRP(DRY)	SHEAR	SECTION
(P6)	(P6)	(P6)	(P6)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1956

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SSI GRIP = 0.87 (3) (INPUT = 0.90)
SSI METAL = 0.43 (5) (INPUT = 1.00)



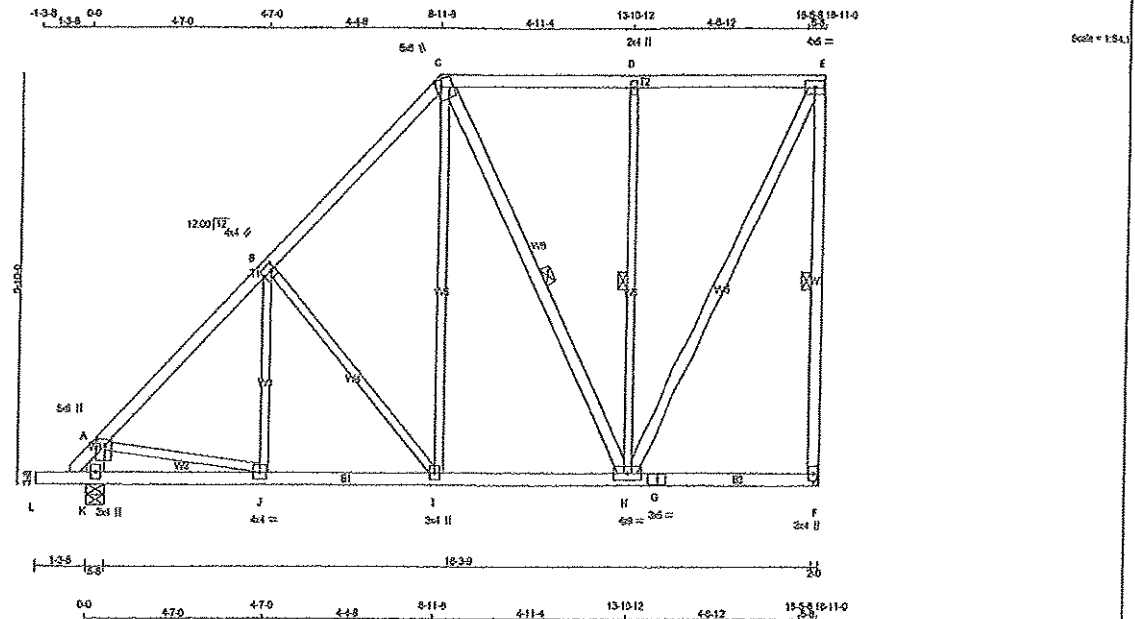
DWG NO. TAM 11045 15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T54	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:zxHLeYmAmnoxP9_eC517jygfAu-Df7i6tCZqauSp2PL3JbH700KhtMSoyLIVCrGuzZ8xm



TOTAL WEIGHT = 113 LB
(KIP)

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2		
C - E	2x4 DRY	No.2		
F - E	2x4 DRY	No.2		
K - A	2x4 DRY	No.2		
L - G	2x4 DRY	No.2		
G - F	2x4 DRY	No.2		
ALL WEBS	2x3 DRY	No.2		
EXCEPT				
H - E	2x4 DRY	No.2		
C - H	2x4 DRY	No.2		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	1082	0	1082	0	0
K	1207	0	1207	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMANENT	WIND	DEAD	SOL
F	872	484/0	199/0	0/0	0/0	169/0	0/0
K	991	550/0	226/0	0/0	0/0	215/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-F, C-H, D-H. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MAX. FACTORED			FACTORED			MAX. FACTORED			MAX.		
MEMB.	FORCE		VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE		MAX	
	(LBS)		(LBS)		CS1 (LBS)	UNBRACED		(LBS)		CS1 (LBS)	
FR-TO			FROM	TO		LENGTH	FR-TO				
A-B	-1050/0		-84.3	-84.3	0.24 (1)	5.85	J-B	0/207		0.05 (3)	
B-C	-781/0		-84.3	-84.3	0.22 (1)	6.25	B-D	-371/10		0.32 (1)	
C-D	-444/0		-84.3	-84.3	0.26 (1)	6.25	I-C	0/408		0.09 (2)	
D-E	-444/0		-84.3	-84.3	0.26 (1)	6.25	A-E	0/777		0.17 (1)	
F-E	-1005/0		0.0	0.0	0.47 (1)	6.24	H-E	0/853		0.15 (1)	
K-A	-1009/0		0.0	0.0	0.05 (1)	7.61	C-H	-190/70		0.12 (1)	
							H-D	-517/0		0.34 (1)	
L-K	0/0		-112.3	-112.3	0.13 (1)	10.00					
K-J	0/0		-28.0	-28.0	0.14 (3)	10.00					
J-I	0/772		-28.0	-28.0	0.22 (2)	10.00					
I-H	0/532		-28.0	-28.0	0.24 (2)	10.00					
H-G	0/0		-28.0	-28.0	0.18 (3)	10.00					
G-F	0/0		-28.0	-28.0	0.18 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH	LL = 23.6 PSF
	CL = 3.0 PSF
BOT CH	LL = 10.5 PSF
	CL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 60°/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
-PART 9 OF CBC 2012, CBC 2012
-CSA 086-09
-TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD EQUALS 23.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/260 (0.63)
CALCULATED VERT. DEFL. (LL) = L/699 (0.03)
ALLOWABLE DEFL. (CL) = L/260 (0.63)
CALCULATED VERT. DEFL. (CL) = L/699 (0.03)

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = L/120 (0.19)
CALCULATED VERT. DEFL. (LL) = L/699 (0.01)
ALLOWABLE DEFL. (CL) = L/120 (0.19)
CALCULATED VERT. DEFL. (CL) = L/699 (0.01)

CS1 TO 0.47 (E-F), SC=0.24 (H-I), WD=0.34 (D-H), SG=0.20 (D-E-I)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAL VALUES

PLATE	GRP (DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	618	354	1697

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRP = 0.84 (J) (INPUT = 0.80)

JSI METAL = 0.29 (J) (INPUT = 1.00)



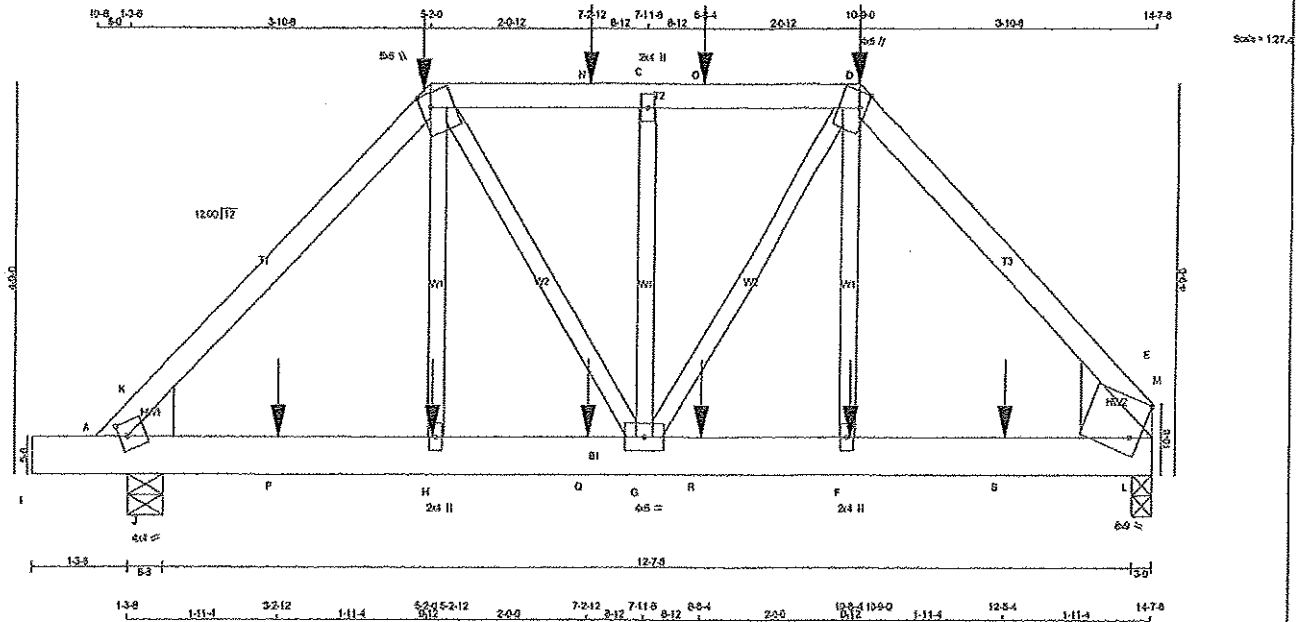
DWG NO. TAM 1109215
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T55	1	1	TRUSS DESC.	

Tamarack Roof Truss, BuEnghen

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ID:zxHLeYmAmnoxP9_eC517jygfAu-XYVYfrCPQmXuvZx4UqOySSwzLDXWQC42f_E1ygeLZ



TOTAL WEIGHT = 68 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
A	TBMH1-m	MT20	4.0	4.0	2.00 1.25
B	TTWW+m	MT20	5.0	6.0	2.00 1.50
C	TMM+w	MT20	2.0	4.0	
D	TTWW+m	MT20	4.0	6.0	Edge 1.00
E	TBMH1-m	MT20	8.0	9.0	5.50 Edge
F	BMW+w	MT20	2.0	4.0	
G	BMWWW-t	MT20	4.0	6.0	
H	BMW+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
A	1240	0	1240	0
E	1165	0	1165	0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	WIND	DEAD
A	1692	582 / 0	212 / 0	0 / 0	209 / 0
E	940	548 / 0	195 / 0	0 / 0	195 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-K	-1208 / 0	-84.3	-84.3	0.13 (1)	5.65	H-B	0 / 288
K-B	-1291 / 0	-84.3	-84.3	0.19 (1)	5.44	B-G	0 / 217
B-N	-1021 / 0	-84.3	-84.3	0.17 (1)	5.97	G-C	-402 / 0
N-C	-1021 / 0	-84.3	-84.3	0.17 (1)	5.97	G-D	0 / 352
C-O	-1021 / 0	-84.3	-84.3	0.17 (1)	5.97	F-D	0 / 174
O-D	-1021 / 0	-84.3	-84.3	0.17 (1)	5.97	J-K	-297 / 0
D-M	-1209 / 0	-84.3	-84.3	0.28 (1)	5.46	L-M	0 / 732
M-E	-1946 / 0	-84.3	-84.3	0.29 (1)	4.47		

I-A	0 / 0	-112.3	-112.3	0.03 (1)	10.00
A-J	0 / 898	-28.0	-28.0	0.26 (1)	10.00
J-P	0 / 898	-28.0	-28.0	0.26 (1)	10.00
P-H	0 / 898	-28.0	-28.0	0.26 (1)	10.00
H-Q	0 / 904	-28.0	-28.0	0.18 (1)	10.00
Q-G	0 / 904	-28.0	-28.0	0.18 (1)	10.00
G-R	0 / 831	-28.0	-28.0	0.15 (1)	10.00
R-F	0 / 831	-28.0	-28.0	0.15 (1)	10.00
F-S	0 / 830	-28.0	-28.0	0.21 (1)	10.00
S-L	0 / 830	-28.0	-28.0	0.21 (1)	10.00
L-E	0 / 830	-28.0	-28.0	0.21 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
B	3-10-8	-175	-175	---	BACK	VERT	TOTAL
D	9-5-8	-175	-175	---	BACK	VERT	TOTAL
F	9-4-12	-26	-26	---	BACK	VERT	TOTAL
H	3-11-4	-26	-26	---	BACK	VERT	TOTAL
N	5-11-4	-54	-54	---	BACK	VERT	TOTAL
O	7-4-12	-54	-54	---	BACK	VERT	TOTAL
P	1-11-4	-17	-17	---	BACK	VERT	TOTAL
Q	5-11-4	-26	-26	---	BACK	VERT	TOTAL
R	7-4-12	-26	-26	---	BACK	VERT	TOTAL
S	11-4-12	-17	-17	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	CL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	CL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2012
- CSA D88-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.29 (E-M:1), BC=0.26 (A-J:1), WB=0.13 (C-G:1), SSI=0.42 (E-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMB=1.00 SHEAR=1.00 TENS= 1.00

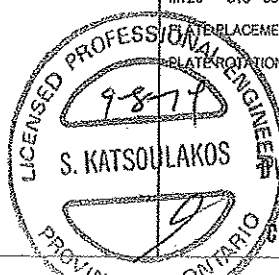
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	622	2264	1656

OVERLAPMENT TOL. = 0.250 inches
SLANT TOLERANCE TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



DRG NO T&M 37456 14
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T55	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

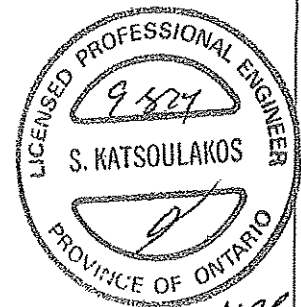
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 245.3 lbs FACTORED DOWN AT 3-10-8, 79.0 lbs FACTORED DOWN AT 5-11-4, AND 79.0 lbs FACTORED DOWN AT 7-4-12, AND 245.3 lbs FACTORED DOWN AT 9-5-8 ON TOP CHORD, AND 22.1 lbs FACTORED DOWN AT 1-11-4, 36.0 lbs FACTORED DOWN AT 3-11-4, 36.0 lbs FACTORED DOWN AT 5-11-4, 36.0 lbs FACTORED DOWN AT 7-4-12, AND 36.0 lbs FACTORED DOWN AT 9-4-12, AND 22.1 lbs FACTORED DOWN AT 11-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

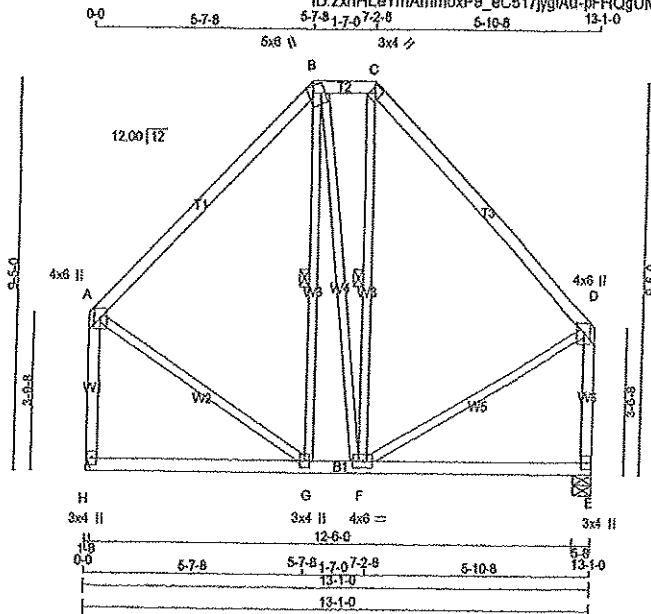


DWG NO TAM37452414
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T56	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:zxHLeYmAmnoxP9_eC517jyglAu-pFHQgUMBbW_yu3PWW1waAFRQKyZgBPm1FapJLzBap
Version 7.610 S Mar 24 2015 Mitek Industries, Inc. Fri May 20 17:38:18 2015 Page 1



Scale = 1:52.7

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - B	2x4	DRY	No. 2
B - C	2x4	DRY	No. 2
C - D	2x4	DRY	No. 2
H - A	2x4	DRY	No. 2
E - D	2x4	DRY	No. 2
H - E	2x4	DRY	No. 2
ALL WEBS	2x3	DRY	No. 2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.75	2.00
B	TTWV+m	MT20	5.0	6.0	2.00	1.50
C	TTW+h	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWV+1	MT20	4.0	6.0		
G	BMVWV+1	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	734	0	734	0	0	HANGER BY OTHERS	
E	734	0	734	0	0	MIN. SEAT SIZE: 1-8	
						5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	603	335/0	137/0	0/0	0/0	131/0	0/0
E	603	335/0	137/0	0/0	0/0	131/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF B-G, C-F.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	FACTORED	MAX	MEMB.	FORCE	FACTORED	MAX
	(LBS)	(PL)	CS1 (LC)		(LBS)	(PL)	CS1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	-381/0	-84.3	-84.3 0.34 (1)	G-B	-52/98	0.03 (1)	
B-C	-271/0	-84.3	-84.3 0.03 (1)	B-F	0/25	0.01 (2)	
C-D	-385/0	-84.3	-84.3 0.38 (1)	F-C	-41/122	0.03 (3)	
H-A	-538/0	0.0	0.0 0.16 (1)	A-G	0/314	0.07 (1)	
E-D	-666/0	0.0	0.0 0.14 (1)	F-D	0/308	0.07 (1)	
H-G	0/0	-28.0	-28.0 0.20 (2)				
G-F	0/268	-28.0	-28.0 0.26 (2)				
F-E	0/0	-28.0	-28.0 0.23 (3)				

TOTAL WEIGHT = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.0	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 48.1	PSF

SPACING = 240 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 068-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CS1 TC=0.38 (C-D:1), BC=0.20 (F-G:2), WB=0.07 (A-G:1), SS1=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (C) (INPUT = 0.60)
JSI METAL= 0.12 (G) (INPUT = 1.00)



DRWG NO. TAM20104-15
STRUCTURAL
COMPONENT ONLY

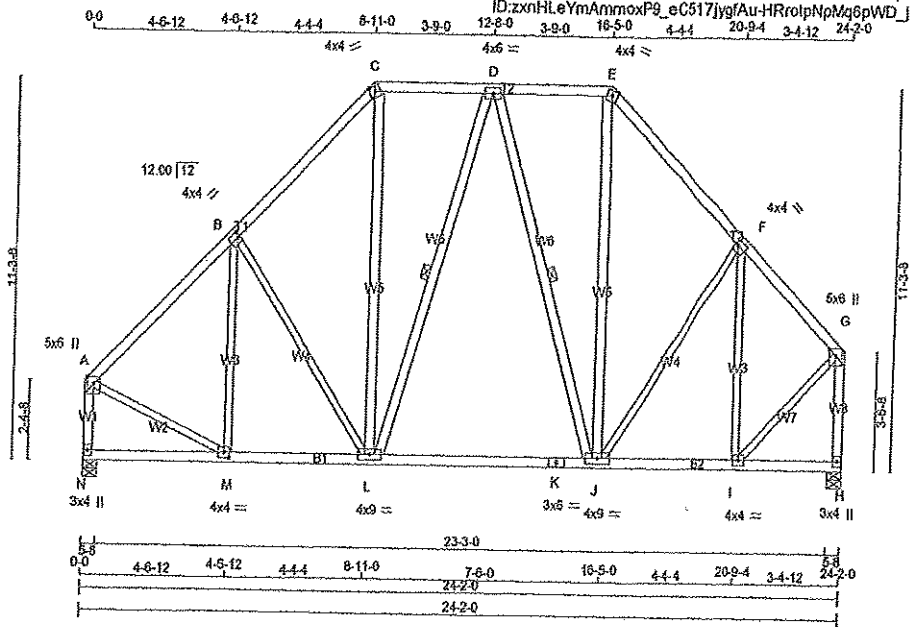
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T57	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 7.010 S Mar 24 2015 Mitek Industries, Inc. Fri May 29 17:38:19 2015 Page 1

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Scale = 1:64.7



TOTAL WEIGHT = 150 lb

LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE	LUMBER		DESC	
A - C	2x4	DRY	No.2		SPF	
C - E	2x4	DRY	No.2		SPF	
E - G	2x4	DRY	No.2		SPF	
N - A	2x4	DRY	No.2		SPF	
H - G	2x4	DRY	No.2		SPF	
N - K	2x4	DRY	No.2		SPF	
K - H	2x4	DRY	No.2		SPF	
ALL WEBS EXCEPT		2x3	DRY	No.2		SPF
L - C	2x4	DRY	No.2		SPF	
J - E	2x4	DRY	No.2		SPF	
L - D	2x4	DRY	No.2		SPF	
D - J	2x4	DRY	No.2		SPF	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
N	1357	0	1357	0	0	5-8	1-8		
H	1357	0	1357	0	0	5-8	1-8		

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ
N	1114	618/0	0	254/0	0/0	0/0	0/0	242/0	0/0	242/0	0/0	242/0	0/0	242/0	0/0	242/0	0/0
H	1114	618/0	0	254/0	0/0	0/0	0/0	242/0	0/0	242/0	0/0	242/0	0/0	242/0	0/0	242/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-L, D-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	-1099/0	-84.3	-84.3 0.24 (1)	5.77	M-B	-231/42	0.19 (1)
B-C	-1029/0	-84.3	-84.3 0.23 (1)	5.92	B-L	-173/0	0.23 (1)
C-D	-707/0	-84.3	-84.3 0.15 (1)	8.25	L-C	0/442	0.07 (1)
D-E	-685/0	-84.3	-84.3 0.15 (1)	8.25	E-J	0/350	0.05 (1)
E-F	-582/0	-84.3	-84.3 0.20 (1)	6.12	J-F	0/63	0.01 (2)
F-G	-857/0	-84.3	-84.3 0.18 (1)	6.25	F-G	-490/0	0.49 (1)
N-A	-1302/0	0.0	0.0 0.17 (1)	7.08	A-M	0/874	0.20 (1)
H-G	-1316/0	0.0	0.0 0.28 (1)	7.05	L-G	0/883	0.19 (1)
N-M	0/0	-28.0	-28.0 0.13 (2)	10.00	L-D	-130/20	0.10 (1)
M-L	0/799	-28.0	-28.0 0.33 (2)	10.00	D-J	-267/0	0.20 (1)
L-K	0/748	-28.0	-28.0 0.33 (2)	10.00			
K-J	0/748	-28.0	-28.0 0.33 (2)	10.00			
J-I	0/629	-28.0	-28.0 0.32 (2)	10.00			
I-H	0/0	-28.0	-28.0 0.07 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) =	1/360 (0.81")
CALCULATED VERT. DEFL. (LL) =	1/999 (0.08")
ALLOWABLE DEFL. (TL) =	1/360 (0.81")
CALCULATED VERT. DEFL. (TL) =	1/699 (0.13")

CS: TC=0.28 (G-H), BC=0.33 (J-L-2), WB=0.40 (F-I), SS=0.15 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1855

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.89)
JSI METAL = 0.30 (M) (INPUT = 1.00)



DWG NO. TAM 20106-15
STRUCTURAL
COMPONENT ONLY

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge		
B	TMVW-t	MT20	4.0	4.0	2.00	1.00	
C	TTW-m	MT20	4.0	4.0	Edge		
D	TMVW-t	MT20	4.0	6.0			
E	TTW-m	MT20	4.0	4.0	Edge		
F	TMVW-t	MT20	4.0	4.0	2.00	1.00	
G	TMVW+p	MT20	5.0	6.0	Edge		
H	BMV1+p	MT20	3.0	4.0			
I	BMVW-t	MT20	4.0	4.0			
J	BMVW-t	MT20	4.0	9.0			
K	BS-t	MT20	3.0	6.0			
L	BMVW-t	MT20	4.0	9.0			
M	BMVW-t	MT20	4.0	4.0	2.00	1.75	
N	BMV1+p	MT20	3.0	4.0			

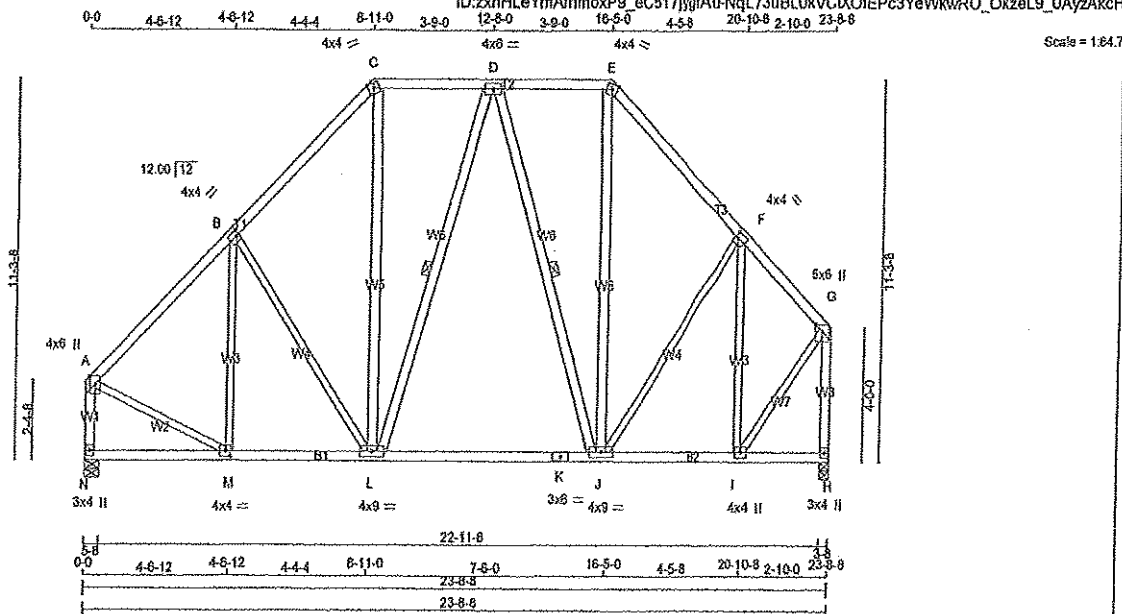
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T57A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:zxHLeYmAmoxP9_eC517jygfAuNqL73uBLOKVCIXOIEPc3YeWkuRO_OkzeL9_OAyZAKch



TOTAL WEIGHT = 3 X 140 = 420 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	UMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
N - A	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
N - K	2x4 DRY	No.2	SPF		
K - H	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		
L - C	2x4 DRY	No.2	SPF		
L - D	2x4 DRY	No.2	SPF		
D - J	2x4 DRY	No.2	SPF		
J - E	2x4 DRY	No.2	SPF		

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	1331	0	1331	0
H	1331	0	1331	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN	COMPONENT REACTIONS	PERMANENT	WIND	DEAD	SOIL
N	1093	607/0	249/0	0/0	0/0	237/0	0/0
H	1093	607/0	249/0	0/0	0/0	237/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-L, D-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)
FR-TO		FORCE (LBS)						LENGTH	FR-TO		FORCE (LBS)					
A-B	-1074/0	-84.3	-84.3	0.23 (1)	5.82	M-B	-223/46	0.18 (1)								
B-C	-999/0	-84.3	-84.3	0.23 (1)	5.99	B-L	-176/0	0.24 (1)								
C-D	-666/0	-84.3	-84.3	0.15 (1)	6.25	L-C	0/421	0.07 (1)								
D-E	-627/0	-84.3	-84.3	0.15 (1)	6.25	L-D	-103/53	0.09 (1)								
E-F	-607/0	-84.3	-84.3	0.19 (1)	6.25	D-J	-295/0	0.22 (1)								
F-G	-736/0	-84.3	-84.3	0.16 (1)	6.25	J-E	0/355	0.06 (2)								
N-A	-1276/0	0.0	0.0	0.16 (1)	7.13	J-F	0/136	0.03 (1)								
H-G	-1297/0	0.0	0.0	0.35 (1)	7.09	I-F	-585/0	0.48 (1)								
						I-G	0/870	0.20 (1)								
N-M	0/0	-28.0	-28.0	0.13 (2)	10.00	A-M	0/855	0.19 (1)								
M-L	0/781	-28.0	-28.0	0.32 (2)	10.00											
L-K	0/719	-28.0	-28.0	0.33 (2)	10.00											
K-J	0/719	-28.0	-28.0	0.33 (2)	10.00											
J-I	0/546	-28.0	-28.0	0.31 (2)	10.00											
I-H	0/0	-28.0	-28.0	0.05 (3)	10.00											

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOY CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.79')
CALCULATED VERT. DEFL. (LL) = $L/969$ (0.68')
ALLOWABLE DEFL. (TL) = $L/360$ (0.79')
CALCULATED VERT. DEFL. (TL) = $L/969$ (0.13')

CSI: TC=0.35 (G-H-1), BC=0.33 (J-L-2), WB=0.48 (F-I-1), SS=0.15 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 L S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PU)	(PU)	
MAX MIN	MAX MIN	MAX MIN	
MT20	610	354	1657

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.60)

JSI METAL= 0.29 (A) (INPUT = 1.00)



DWG NO. TAM20107-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T58	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMW+u	MT20	2.0	4.0		

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED STARTING AT 1-3-8 FROM THE LEFT END TO 4-2-0 TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF TOP CHORD. DESIGN OF UNSPECIFIED CONNECTIONS IS DELEGATED TO THE BUILDING DESIGNER.
- 2) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED AT 4-2-0 FROM THE LEFT END TO CONNECT INCOMING TRUSS(ES) (1 PLY) TO FRONT FACE OF BOTTOM CHORD. DESIGN OF UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

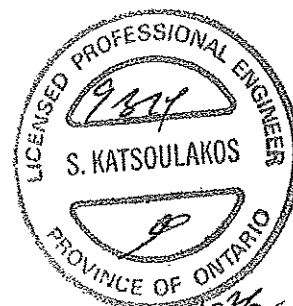
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

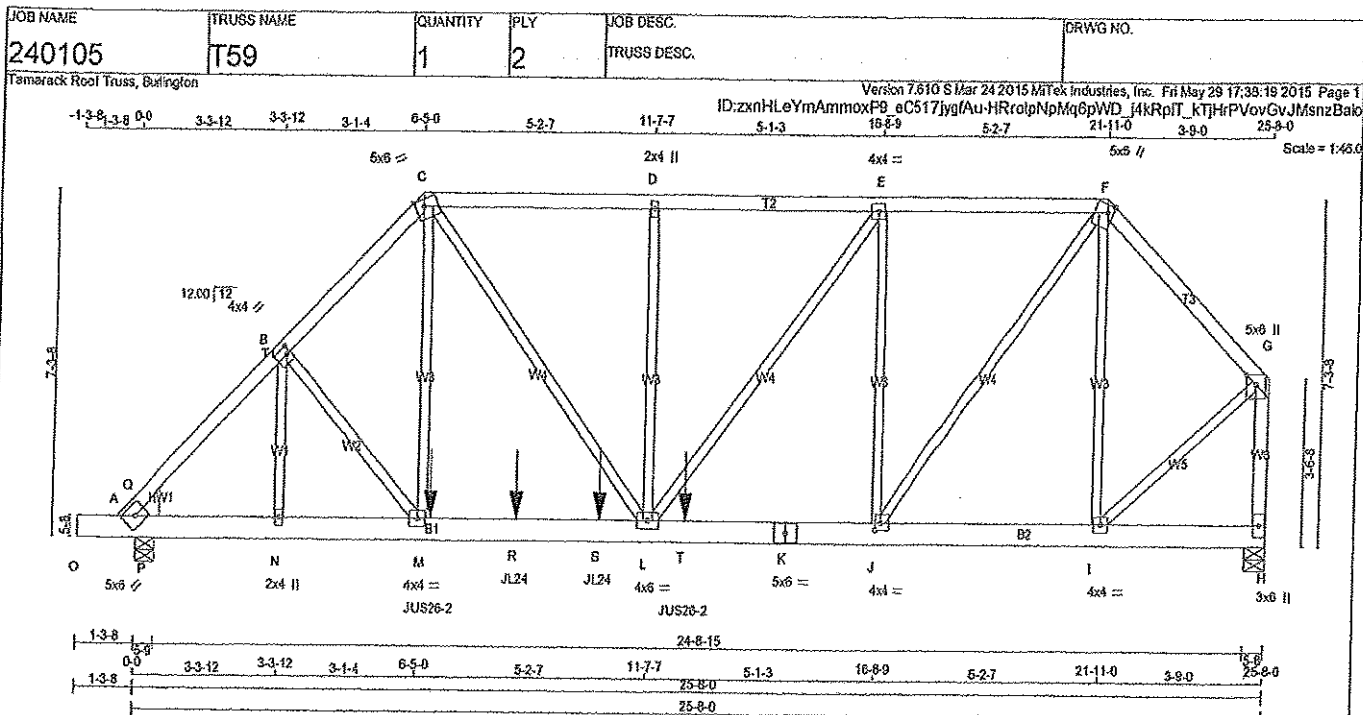
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.61 (A) (INPUT = 0.80)

JSI METAL = 0.13 (A) (INPUT = 1.00)



DWG NO T&M37459.14
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF
K - H	2x6	DRY No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'x3') SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - G	12	TOP
H - G	12	TOP
BOTTOM CHORDS: (0.122'x3') SPIRAL NAILS		
O - K	12	TOP
K - H	12	TOP
WEBS: (0.122'x3') SPIRAL NAILS		
M - C	6	SIDE(6.4)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TBMH1-h	MT20	5.0	6.0	
B TBMW1-h	MT20	4.0	4.0	2.00 1.00
C TTMW1-m	MT20	5.0	6.0	Edge 2.25
D TBMW1-w	MT20	2.0	4.0	
E TBMW1-i	MT20	4.0	4.0	
F TTMW1-m	MT20	5.0	6.0	2.00 1.50
G TBMW1-p	MT20	5.0	6.0	Edge
H BMV1-p	MT20	3.0	6.0	
I BMW1-t	MT20	4.0	4.0	
J BMW1-t	MT20	4.0	4.0	1.75 1.50
K BS-t	MT20	5.0	6.0	
L BMW1-t	MT20	4.0	4.0	
M BMW1-t	MT20	4.0	4.0	
N BMW1-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD		HEEL	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX	WEDGE	
A	3126	0	3126	0	0	5-8	1-11	5-8	1-11	2x8 L	
H	2517	0	2517	0	0	5-8	1-8	5-8	1-8		

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SO
A	2537	1457/0	546/0	0/0	0/0	535/0	0/0
H	2049	1167/0	447/0	0/0	0/0	495/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX	UNBRACED LENGTH FR-TO	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO						FR-TO			
A-Q	-3979/0	-84.3	-84.3	0.07 (1)	4.72	N-B	-151/107	0.01 (3)	
Q-B	-3930/0	-84.3	-84.3	0.11 (1)	4.72	B-M	-214/0	0.04 (1)	
B-C	-3773/0	-84.3	-84.3	0.11 (1)	4.72	M-C	0/1269	0.16 (1)	
C-D	-3472/0	-84.3	-84.3	0.21 (1)	4.84	C-L	0/1340	0.17 (1)	
D-E	-3472/0	-84.3	-84.3	0.23 (1)	4.82	L-D	-440/0	0.20 (1)	
E-F	-2659/0	-84.3	-84.3	0.20 (1)	5.32	L-E	0/1370	0.17 (1)	
F-G	-1617/0	-84.3	-84.3	0.13 (1)	6.25	J-F	-1567/0	0.70 (1)	
H-G	-2479/0	0.0	0.0	0.26 (1)	7.17	J-F	0/2312	0.29 (1)	
O-A	0/0	-112.3	-112.3	0.02 (1)	10.00	I-G	-908/0	0.41 (1)	
A-P	0/2785	-28.0	-28.0	0.22 (1)	10.00	P-Q	0/1644	0.20 (1)	
P-N	0/2785	-28.0	-28.0	0.23 (1)	10.00		-88/43	0.00 (1)	
N-M	0/2785	-28.0	-28.0	0.25 (1)	10.00				
M-R	0/2665	-28.0	-28.0	0.38 (1)	10.00				
R-S	0/2665	-28.0	-28.0	0.38 (1)	10.00				
S-L	0/2665	-28.0	-28.0	0.38 (1)	10.00				
L-T	0/2659	-28.0	-28.0	0.39 (1)	10.00				
T-K	0/2659	-28.0	-28.0	0.39 (1)	10.00				
K-J	0/2659	-28.0	-28.0	0.38 (1)	10.00				
J-I	0/1266	-28.0	-28.0	0.14 (1)	10.00				
I-H	0/0	-28.0	-28.0	0.04 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE
M	6-8-8	-726	-726	---	FRONT	VERT
R	6-7-12	-332	-332	---	FRONT	VERT
S	10-6-4	-332	-332	---	FRONT	VERT
T	12-5-8	-1226	-1226	---	FRONT	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, SCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.87)
CALCULATED VERT. DEFL. (LL) = $L/699$ (0.07)
ALLOWABLE DEFL. (TL) = $L/360$ (0.87)
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.11)

CANTILEVER DEFLECTION:

ALLOWABLE DEFL. (LL) = $L/120$ (0.16)
CALCULATED VERT. DEFL. (LL) = $L/699$ (0.00)
ALLOWABLE DEFL. (TL) = $L/120$ (0.16)
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00)

CSI: TC=0.26 (G-H:1), BC=0.38 (L-M:1), WB=0.70 (E-J:1), SS=0.39 (J-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.80

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 610 354 1637 622 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

DRWG NO. TAM 20105-15
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2



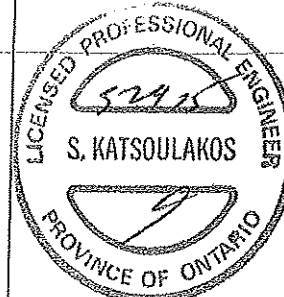
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T59	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) USE USP JUS2S-2 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 8-8-8 FROM THE LEFT END TO CONNECT INCOMING TRUSSES (2 PLY) TO FRONT FACE OF BOTTOM CHORD, SKEWED 0.0 DEG. TO THE RIGHT, SLOPING 0.0 DEG. DOWN.
- 2) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 8-7-12 FROM THE LEFT END TO CONNECT INCOMING TRUSSES (1 PLY) TO FRONT FACE OF BOTTOM CHORD, SKEWED 0.0 DEG. TO THE RIGHT, SLOPING 0.0 DEG. DOWN.
- 3) USE USP JL24 (WITH 10D NAILS INTO GIRDER & 10D15 NAILS INTO INCOMING TRUSS) AT 10-8-4 FROM THE LEFT END TO CONNECT INCOMING TRUSSES (1 PLY) TO FRONT FACE OF BOTTOM CHORD, SKEWED 0.0 DEG. TO THE RIGHT, SLOPING 0.0 DEG. DOWN.
- 4) USE USP JUS2S-2 (WITH 16D NAILS INTO GIRDER & 16D NAILS INTO INCOMING TRUSS) AT 12-5-8 FROM THE LEFT END TO CONNECT INCOMING TRUSSES (2 PLY) TO FRONT FACE OF BOTTOM CHORD, SKEWED 0.0 DEG. TO THE RIGHT, SLOPING 0.0 DEG. DOWN.
- 5) FILL ALL NAIL HOLES.



P02/2

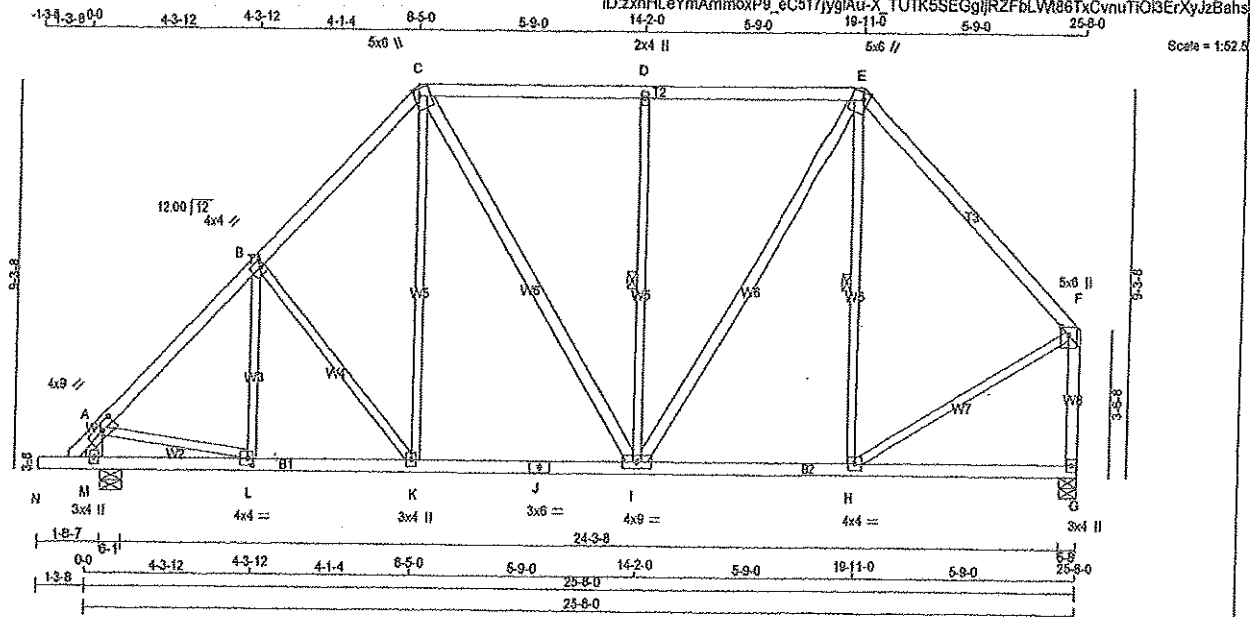
DWG NO. TAM20105-15
STRUCTURAL
COMPONENT ONLY

JOB NAME 240105	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:zxnhLeYmAmoxP8_eC517jyglAu-X TUTK5SEggRZFbLW86TxCvnuTIO3ErXyJzBahs



TOTAL WEIGHT = 134 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
EXCEPT				
C - I	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	4.0	9.0	2.00	4.00
B	TMWV-1	MT20	4.0	4.0	2.00	1.00
C	TMWV+m	MT20	5.0	6.0	2.00	1.50
D	TMWV+m	MT20	2.0	4.0		
E	TMWV+m	MT20	5.0	6.0	2.00	1.50
F	TMWV+p	MT20	3.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	4.0		
I	BS-1	MT20	4.0	9.0		
K	BMVW-1	MT20	3.0	4.0		
L	BMVW-1	MT20	4.0	4.0	2.00	1.50
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT		
G	1441	0	1441	0	5-8	1-9
M	1566	0	1566	0	6-1	1-11

UNFACTORED REACTIONS

JT	1ST LCASE		MAX MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
G	1183	667/0	270/0	0/0	0/0	257/0	0/0
M	1302	723/0	287/0	0/0	0/0	283/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-I, E-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	-1544/0	-84.3	-84.3	0.23 (1)	5.09	L-B	-14/153	0.03 (3)	
B-C	-1325/0	-84.3	-84.3	0.21 (1)	5.41	B-K	-305/0	0.23 (1)	
C-D	-1061/0	-84.3	-84.3	0.37 (1)	5.84	K-C	0/402	0.09 (2)	
D-E	-1061/0	-84.3	-84.3	0.37 (1)	5.84	I-C	0/269	0.04 (1)	
E-F	-1037/0	-84.3	-84.3	0.39 (1)	5.69	I-D	-591/0	0.33 (1)	
G-F	-1375/0	0.0	0.0	0.29 (1)	6.93	I-E	0/616	0.10 (1)	
M-A	-1398/0	0.0	0.0	0.09 (1)	7.81	H-F	-215/111	0.12 (1)	
						H-F	0/833	0.19 (1)	
N-M	0/0	-112.3	-112.3	0.13 (1)	10.00	A-L	0/1120	0.26 (1)	
M-L	0/0	-28.0	-28.0	0.11 (2)	10.00				
L-K	0/1112	-28.0	-28.0	0.27 (2)	10.00				
K-J	0/916	-28.0	-28.0	0.28 (2)	10.00				
J-I	0/916	-28.0	-28.0	0.28 (2)	10.00				
I-H	0/729	-28.0	-28.0	0.32 (2)	10.00				
H-G	0/0	-28.0	-28.0	0.23 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 6, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(65% OF 31.4 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.66')$
CALCULATED VERT. DEFL.(LL) = $L/999 (0.05')$
ALLOWABLE DEFL.(TL) = $L/360 (0.66')$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.09')$

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(L) = $L/120 (0.19')$
CALCULATED VERT. DEFL.(L) = $L/999 (0.01')$
ALLOWABLE DEFL.(TL) = $L/120 (0.19')$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.01')$

CSI: TC=0.39 (E-F:1), BC=0.32 (H-I:2), WB=0.33 (D-I:1), SSI=0.23 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (L) (INPUT = 0.90)
JSI METAL=0.42 (L) (INPUT = 1.00)



DWG NO. TAM20/00-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T62	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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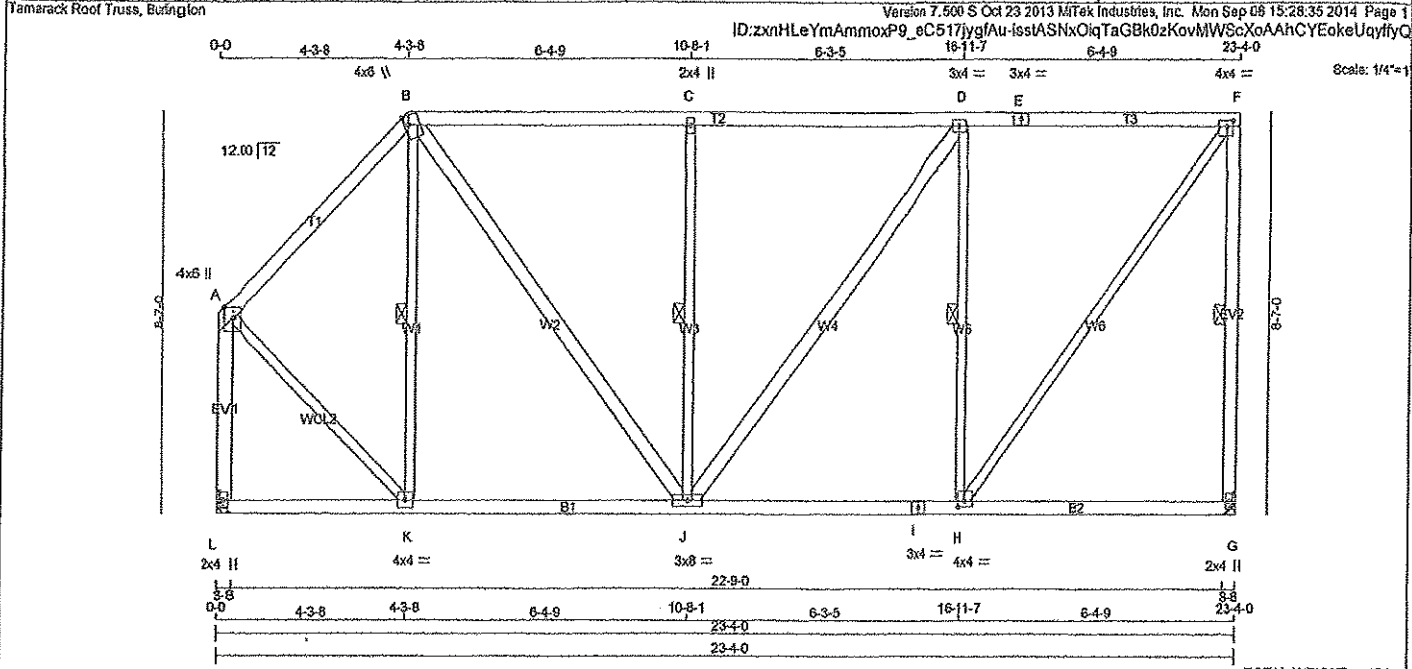
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	11-9-12	-1282	-1282	—	BACK	VERT	TOTAL
V	12-4-0	-368	-368	—	FRONT	VERT	TOTAL
W	13-3-4	-1282	-1282	—	BACK	VERT	TOTAL



DWG NO TAM 37462-14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T63	1	1	TRUSS DESC.	



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	4.0	6.0 2.75 2.00
B	TMVW+m	MT20	4.0	6.0 Edge 1.00
C	TMVW+w	MT20	2.0	4.0
D	TMVW-l	MT20	3.0	4.0
E	TS-l	MT20	3.0	4.0
F	TMVW-l	MT20	4.0	4.0 1.50 1.75
G	BMVl+p	MT20	2.0	4.0
H	BMVW-l	MT20	4.0	4.0 1.50 1.75
I	BS-l	MT20	3.0	4.0
J	BMVW-l	MT20	3.0	4.0
K	BMVW-l	MT20	4.0	4.0
L	BMVl+p	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	REMARKS
G	1310 0	1310 0	0	0	HANGER BY OTHERS
L	1310 0	1310 0	0	0	MIN. SEAT SIZE: 1-8 HANGER BY OTHERS

UNFACTORED REACTIONS					
JT	1ST LCASE COMBINED	SNOW	LIVE	WIND	DEAD
G	1075	597 / 0	245 / 0	0 / 0	233 / 0
L	1075	597 / 0	245 / 0	0 / 0	233 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-G, B-K, C-J, D-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING					
TOTAL LOAD CASES: (3)					
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	-910 / 0	-84.3 -84.3 0.21 (1)	6.25	K-B	-342 / 0
B-C	-964 / 0	-84.3 -84.3 0.41 (1)	5.79	B-J	0 / 651
C-D	-964 / 0	-84.3 -84.3 0.37 (1)	5.88	J-C	-580 / 0
D-E	-768 / 0	-84.3 -84.3 0.40 (1)	6.25	J-D	0 / 295
E-F	-768 / 0	-84.3 -84.3 0.40 (1)	6.25	H-D	-821 / 0
G-F	-1238 / 0	0.0 0.0 0.42 (1)	5.77	H-F	0 / 1289
L-A	-1287 / 0	0.0 0.0 0.40 (1)	7.16	A-K	0 / 767
L-K	0 / 0	-28.0 -28.0 0.20 (2)	10.00		
K-J	0 / 587	-28.0 -28.0 0.27 (2)	10.00		
J-I	0 / 768	-28.0 -28.0 0.37 (2)	10.00		
I-H	0 / 768	-28.0 -28.0 0.37 (2)	10.00		
H-G	0 / 0	-28.0 -28.0 0.28 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.78")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.78")
CALCULATED VERT. DEFL. (TL) = L/999 (0.11")

CSI: TC=0.42 (F-G:1), BC=0.37 (H-J:2), WB=0.30 (D-H:1), SSI=0.25 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)



DWG NO TAM3746314
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 53 lb

DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 245.3 lbs
FACTORED DOWN AT 3-10, 8, AND 79.0 lbs FACTORED DOWN AT 6-7, 8, AND 245.3 lbs FACTORED DOWN AT 7-8 ON TOP CHORD, AND 22.1 lbs FACTORED DOWN AT 1-11, 4, 58.0 lbs FACTORED DOWN AT 3-11, 4, 58.0 lbs FACTORED DOWN AT 5-7, 8, AND 38.0 lbs FACTORED DOWN AT 7-12, 12, AND 22.1 lbs FACTORED DOWN AT 9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

1 DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY RULI (NVC)

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I. E.

BRACING

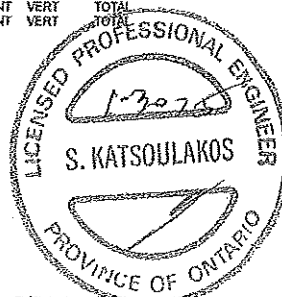
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 5.06FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	LC1 (LBS)	MAX UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LBS)
FR-TO		FROM TO			LENGTH (FT)			
A-B	-924/0	-84.3	-84.3	0.27 (1)	6.07	H-B	0/187	0.05 (2)
B-M	-654/0	-84.3	-84.3	0.29 (1)	6.25	B-C	0/5	0.00 (2)
M-C	-654/0	-84.3	-84.3	0.29 (1)	6.25	G-C	0/201	0.05 (2)
C-D	-934/0	-84.3	-84.3	0.27 (1)	6.06	A-H	0/656	0.18 (1)
I-A	-934/0	0.0	0.0	0.06 (1)	7.81	G-D	0/657	0.16 (1)
F-D	-934/0	0.0	0.0	0.06 (1)	7.81			
J-I	0/0	-112.3	-112.3	0.14 (1)	10.00			
I-N	0/0	-28.0	-28.0	0.12 (2)	10.00			
N-H	0/0	-28.0	-28.0	0.12 (2)	10.00			
H-O	0/652	-28.0	-28.0	0.20 (2)	10.00			
O-E	0/652	-28.0	-28.0	0.20 (2)	10.00			
G-P	0/0	-28.0	-28.0	0.13 (2)	10.00			
P-F	0/0	-28.0	-28.0	0.13 (2)	10.00			
F-E	0/0	-112.3	-112.3	0.14 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)						
JT	LOC.	LG1	MAX.	FACE	DR.	TYPE
B	3-10-8	-175	-175	FRONT	VERT	TOTAL
C	7-3-8	-175	-175	FRONT	VERT	TOTAL
G	7-2-12	-28	-28	FRONT	VERT	TOTAL
H	3-11-4	-28	-28	FRONT	VERT	TOTAL
M	5-7-0	-54	-54	FRONT	VERT	TOTAL
N	1-11-4	-17	-17	FRONT	VERT	TOTAL
O	5-7-0	-26	-26	FRONT	VERT	TOTAL
P	8-2-12	-17	-17	FRONT	VERT	TOTAL



DWG NO. TAM 2465-15
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY
USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF
5.92/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO
LOADCASE(S): 1,2,4

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012
- CSA G88-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(L) = $L/360$ (0.37)
CALCULATED VERT. DEFL.(L) = $L/999$ (0.02)
ALLOWABLE DEFL.(TL) = $L/360$ (0.37)
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03)

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = $1/120$ (0.19)
CALCULATED VERT. DEFL.(LL) = $1/999$ (0.01)
ALLOWABLE DEFL.(TL) = $1/120$ (0.19)
CALCULATED VERT. DEFL.(TL) = $1/999$ (0.01)

CSI: TC=0.29 (B-C:1), BC=0.20 (G-H:2), WB=0.16 (D-G:1),
SSI=0.16 (B-C:1)

DOE LUMBER=1.00 NAE=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

MAX VALUES					
PLATE	GRIP (DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 DEG

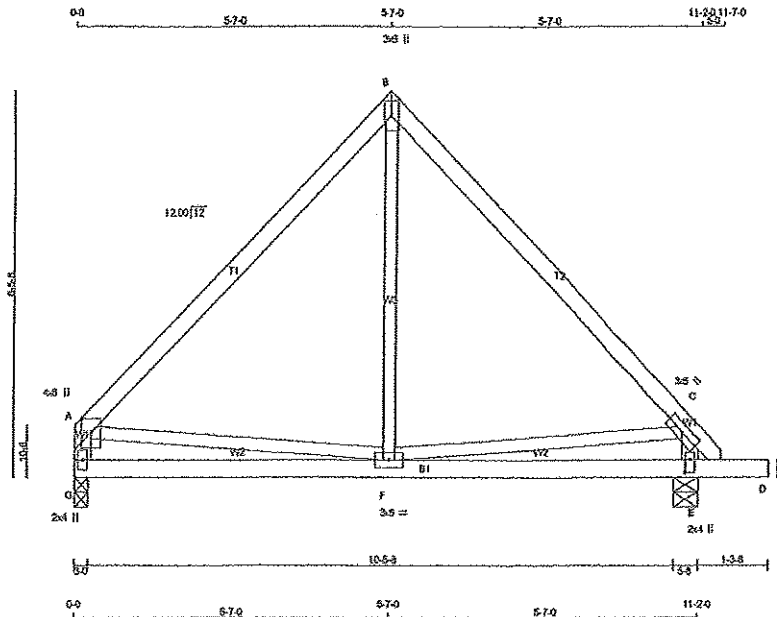
JSI GRP=0.89 (G) (INPUT=0.99)
JSI METAL=0.24 (F) (INPUT=1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:37.5

TOTAL WEIGHT = 48 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
E - C	2x4	DRY	No.2	SPF	
G - A	2x4	DRY	No.2	SPF	
G - D	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.75	2.00	
B	TTVW+p	MT20	3.0	6.0			
C	TMVW-l	MT20	3.0	8.0			
E	BMVl+p	MT20	2.0	4.0	2.50	1.00	
F	BMVWV-l	MT20	3.0	6.0			
G	BMVl+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	627	0	627	0	0	3-0	1-8
E	772	0	772	0	0	5-8	1-8

UNFACTORED REACTIONS					
	1ST LCASE	MAX / MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	WIND	DEAD
G	515	265 / 0	117 / 0	0 / 0	112 / 0
E	634	352 / 0	144 / 0	0 / 0	137 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (3)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	UNBRAC LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	-429 / 0	-84.3	-84.3	0.34 (1)	6.25	F-B	0 / 295	0.07 (2)	
B-C	-429 / 0	-84.3	-84.3	0.34 (1)	6.25	F-C	0 / 305	0.07 (1)	
E-C	-567 / 0	0.0	0.0	0.09 (1)	7.61	A-F	0 / 305	0.07 (1)	
G-A	-567 / 0	0.0	0.0	0.09 (1)	7.61				
G-F	0 / 0	-28.0	-28.0	0.25 (2)	10.00				
F-E	0 / 0	-28.0	-28.0	0.25 (2)	10.00				
E-D	0 / 0	-112.3	-112.3	0.13 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 068-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = $L/120$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/120$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.34 (B-C:1), BC=0.25 (F-G:2), WB=0.07 (C-F:1), SSI=0.13 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (F) (INPUT = 0.90)
JSI METAL= 0.12 (A) (INPUT = 1.00)



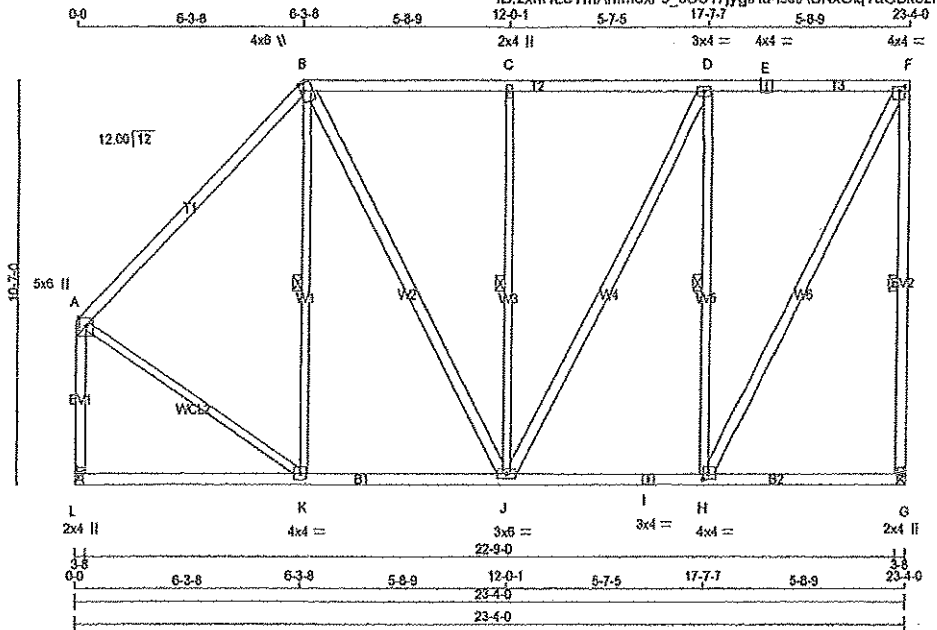
DWG NO TAM 37465.14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T66	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:zxHLeYmAmnoxP9_eC517jyg(Au-iss)ASNxOiqTaGBk0zKovMWOAXobAdaYEokeUqyffQ



TOTAL WEIGHT = 4 X 141 = 568 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
B - J	2x4	DRY	No.2	SPF	
J - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - B	TMW+p	MT20	5.0	6.0	Edge		
B - E	TTWW+m	MT20	4.0	6.0	Edge 1.00		
C - D	TMW+w	MT20	2.0	4.0			
D - E	TMW-w	MT20	3.0	4.0			
E - F	TS-1	MT20	4.0	4.0	Edge 2.00		
F - G	TMW-1	MT20	4.0	4.0	1.50 1.75		
G - H	BMW-1+p	MT20	2.0	4.0			
H - I	BMW-1	MT20	4.0	4.0	1.75 1.50		
I - J	BS-1	MT20	3.0	4.0			
J - K	BMW-1	MT20	3.0	6.0			
K - L	BMW-1	MT20	4.0	4.0			
L - A	BMW-1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED		MAXIMUM FACTORED		RECORD
	GROSS VERT	GROSS HORZ	GROSS DOWN	GROSS HORZ	
G	1310	0	1310	0	0
L	1310	0	1310	0	0

UNFACTORED REACTIONS				
JT	1ST LOASE	MAX MIN. COMPONENT REACTIONS		
		COMBINED	SNOW	LIVE
G	1075	597 / 0	245 / 0	0 / 0
L	1075	597 / 0	245 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-G, B-K, C-J, D-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	-851 / 0	-84.3	-84.3 0.29 (1)	6.25	K-B	-172 / 87	0.14 (1)
B-C	-772 / 0	-84.3	-84.3 0.31 (1)	6.25	B-J	0 / 358	0.06 (1)
C-D	-772 / 0	-84.3	-84.3 0.29 (1)	6.25	J-C	-518 / 0	0.41 (1)
D-E	-584 / 0	-84.3	-84.3 0.31 (1)	6.25	J-D	0 / 392	0.06 (1)
E-F	-584 / 0	-84.3	-84.3 0.31 (1)	6.25	H-D	-869 / 0	0.68 (1)
G-F	-1247 / 0	0.0	0.0 0.70 (1)	5.76	H-F	0 / 1204	0.19 (1)
L-A	-1238 / 0	0.0	0.0 0.39 (1)	7.22	A-K	0 / 705	0.16 (1)
L-K	0 / 0	-28.0	-28.0 0.27 (2)	10.00			
K-J	0 / 599	-28.0	-28.0 0.34 (2)	10.00			
J-I	0 / 584	-28.0	-28.0 0.30 (2)	10.00			
I-H	0 / 584	-28.0	-28.0 0.30 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.23 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 088-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.70 (F-G-1), BC=0.34 (J-K-2), WB=0.68 (D-H-1), SSI=0.22 (D-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

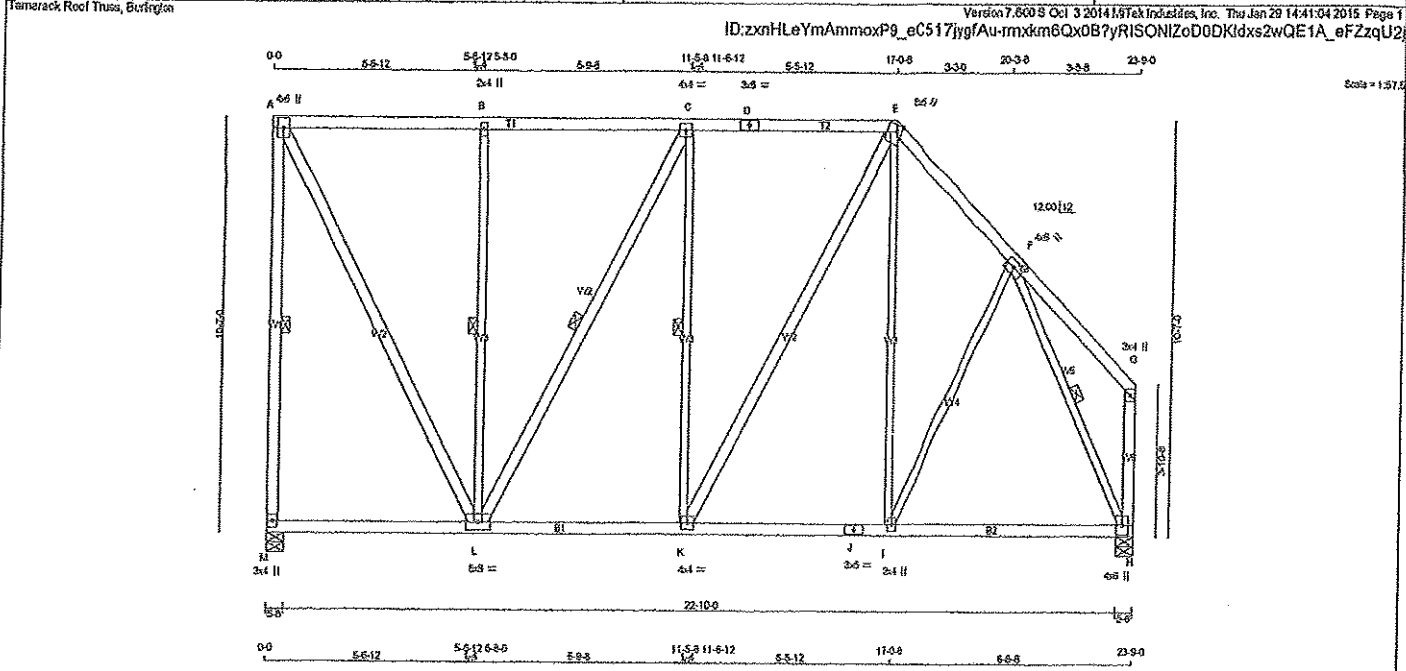
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (D) (INPUT = 0.90)
JSI METAL = 0.23 (F) (INPUT = 1.00)



DWG NO TAM 3746614
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T66X	1	1	TRUSS DESC.	



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD	SPECIFIED LOADS:	
M - A	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	RECORD		TOP CH. LL = 25.6 PSF	
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG		BOT CH. LL = 10.5 PSF	
D - E	2x4	DRY	No.2	SPF	JT VERT	DOWN	HORZ	UPLIFT	IN-SX	DL = 7.0 PSF	
E - G	2x4	DRY	No.2	SPF	M 1333	0	1333	0	5-8	1-8	
H - I	2x4	DRY	No.2	SPF	H 1333	0	1333	0	5-8	1-8	
M - J	2x4	DRY	No.2	SPF							
J - H	2x4	DRY	No.2	SPF							
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS					SPACING = 24.0 IN. C/C	
K - E	2x4	DRY	No.2	SPF	1ST CASE	MAX. A/N	COMPONENT REACTIONS			LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12	
A - L	2x4	DRY	No.2	SPF	JT COMBINED	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL
L - C	2x4	DRY	No.2	SPF	M 1095	608/0	249/0	0/0	0/0	238/0	0/0
				SPF	H 1095	608/0	249/0	0/0	0/0	238/0	0/0

DRY, SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010			
PLATES (table is in inches)				BRACING				THIS DESIGN COMPLIES WITH:			
JT TYPE	PLATES	W	LEN Y X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.				- PART 9 OF CBC 2012, CBC 2012			
A TMWVp	MT20	4.0	6.0	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.				- CSA 665-09			
B TMWw	MT20	2.0	4.0	1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF A-M, F-H, B-L, C-K, C-L				- TPIC 2011			
C TMWVt	MT20	4.0	4.0	END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
D TS-t	MT20	3.0	6.0	LOADING				ALLOWABLE DEFL. (LL) = L/360 (0.79)			
E TTWVt	MT20	3.0	6.0	TOTAL LOAD CASES: (4)				CALCULATED VERT. DEFL. (LL) = 1/999 (0.09)			
F TMWVt	MT20	3.0	4.0	CHORDS				ALLOWABLE DEFL. (LL) = L/360 (0.79)			
G TMVtp	MT20	3.0	4.0	MEMB. FORCE FACTORED				CALCULATED VERT. DEFL. (LL) = 1/999 (0.14)			
H BMWVt	MT20	3.0	4.0	MAX. FACTORED				CSK TC=0.71 (A-M:1), BC=0.37 (I-K:2), WB=0.49 (F-H:1), SS=0.23 (A-B:1)			
I BMWVt	MT20	3.0	4.0	VERT. LOAD LC1				DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10 COMB=1.10			
J BS-t	MT20	3.0	6.0	FROM TO				SHEAR=1.10 TENS=1.10			
K BMWVt	MT20	3.0	4.0	MAX. FACTORED				COMPANION LIVE LOAD FACTOR = 0.50			
L BMWVt	MT20	3.0	4.0	VERT. LOAD LC1				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
M BMVtp	MT20	3.0	4.0	FROM TO				NAIL VALUES			

NAIL VALUES				PLATE ROTATION TOL. = 5.0 Deg.			
PLATE GRP(DRY)	SHEAR	SECTION		JSI GRP= 0.81 (F) (NPUT= 0.80)			
(PS)	(PL)	(PL)		JSI METAL= 0.32 (F) (NPUT= 1.00)			
MAX MIN	MAX MIN	MAX MIN					
MT20	516	354	1827	922	2284	1656	



DRWG NO. TAM 240105-15
STRUCTURAL
COMPONENT ONLY

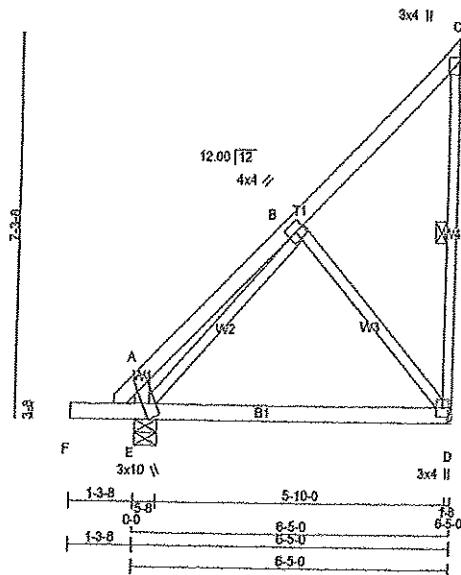
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	T200	2	1	TRUSS DESC.	

Tamrack Roof Truss, Burlington

Version 7.610 S Mar 24 2015 MTEK Industries, Inc. Fri May 20 17:35:20 2015 Page 1
ID: zxnHLeYmAmmaxP9_eC517jygfAu-kdPA49NR77Eg8MZvdSy2FgWw7dX85h3VZ3vOEzBain

-1-3-8 1-3-8 0-0 3-2-8 3-2-8 3-2-8 6-5-0

Scale = 1/4" = 1'-0"



CHORDS	SIZE	DRY	NO. 2	DESCR.
A - C	2x4	DRY	No. 2	SPF
D - C	2x3	DRY	No. 2	SPF
E - A	2x4	DRY	No. 2	SPF
F - D	2x4	DRY	No. 2	SPF

ALL WEBS 2x3 DRY No. 2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TBMWW-I	MT20	4.0	4.0		
C	TBMW+p	MT20	3.0	4.0		Edge
D	TBMWWI+p	MT20	3.0	4.0		
E	TBMWWI+m	MT20	3.0	10.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
D	380	0	380	0
E	505	0	505	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL
D	286	164/0	87/0	0/0	0/0	64/0	0/0
E	415	230/0	95/0	0/0	0/0	80/0	0/0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.0 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-D.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORIZ. LOAD (LBS)	MAX. CS (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO								
A-B	0/24	-84.3	-84.3	0.15 (1)	10.00	B-D	-220/0	0.08 (1)
B-C	-22/0	-84.3	-84.3	0.11 (1)	8.25	E-B	-231/0	0.08 (1)
D-C	-110/0	0.0	0.0	0.04 (1)	6.25			
E-A	-95/0	0.0	0.0	0.01 (1)	7.81			
F-E	0/0	-112.3	-112.3	0.13 (1)	10.00			
E-D	0/150	-28.0	-28.0	0.35 (2)	10.00			

TOTAL WEIGHT = 2 X 33 = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.21")
CALCULATED VERT. DEFL. (LL) = L/710 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (0.21")
CALCULATED VERT. DEFL. (TL) = L/426 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL. (LL) = L/120 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/120 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.15 (A-B: 1), EC=0.35 (D-E: 2), WB=0.08 (B-D: 1), SSI=0.12 (D-E: 2)

DOL LUMBER=1.09 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (E) (INPUT = 0.50)
JSI METAL= 0.09 (B) (INPUT = 1.00)

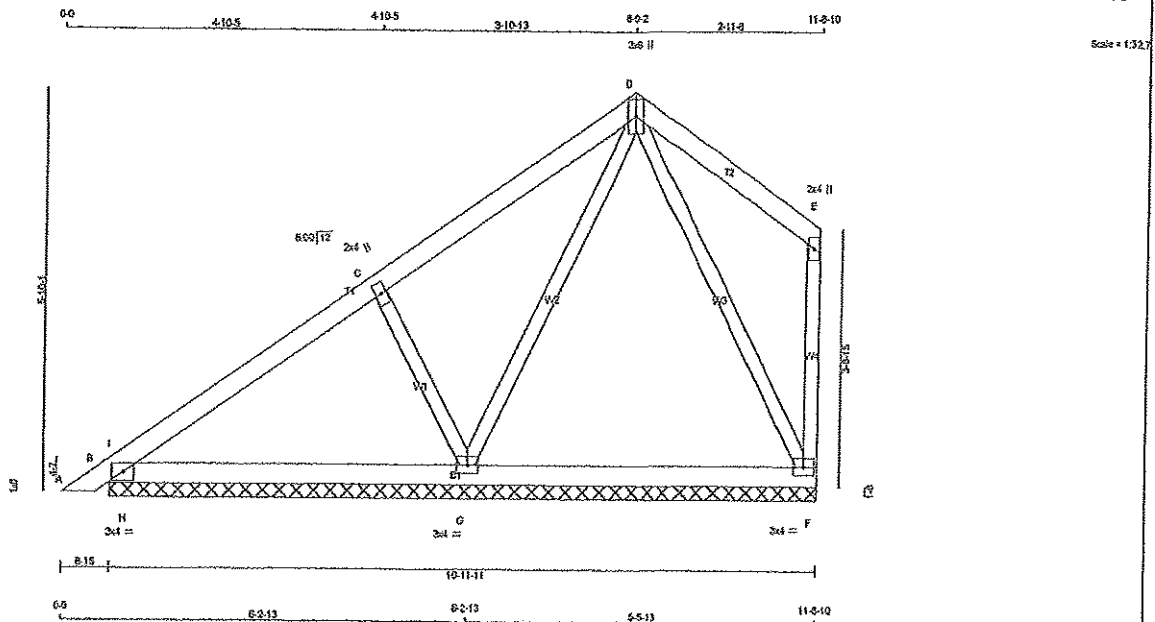


DWG NO. TAM 20100-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	PB36	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 7.500 5 Oct 23 2013 Mitek Industries, Inc. Fri Sep 05 16:28:54 2014 Page 1
ID:zxHLeYmAmnoxP9_eC517jyglAu-7il_Sqc46GFNUq25vIT8EdW0HP0cUwGTpxWBygel6



TOTAL WEIGHT = 4 X 43 = 172 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x3	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMS1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	3.0	6.0	
E	TMW+p	MT20	2.0	4.0	
F	BMVW1-t	MT20	3.0	4.0	
G	BMVW1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	291	0	291	0	0	10-11-11	1-8
B	348	0	348	0	0	10-11-11	1-8
G	649	0	649	0	0	10-11-11	1-8

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS			
		SNOW	LIVE	WIND	DEAD
F	233	140 / 0	46 / 0	0 / 0	47 / 0
B	271	174 / 0	46 / 0	0 / 0	51 / 0
G	545	282 / 0	138 / 0	0 / 0	125 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS		FACTORED				WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	LENGTH	FR-TO			CS1 (LC)	
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	C-G	-346 / 0	0.07 (1)	
B-I	-231 / 0	-84.3	-84.3	0.07 (2)	6.25	G-D	-163 / 0	0.10 (1)	
I-C	-158 / 0	-84.3	-84.3	0.22 (1)	6.25	D-F	-125 / 0	0.08 (1)	
C-D	-15 / 0	-84.3	-84.3	0.22 (1)	6.25	H-I	-47 / 145	0.00 (1)	
D-E	0 / 0	-84.3	-84.3	0.13 (1)	10.00				
F-E	-125 / 0	0.0	0.0	0.04 (1)	7.81				
B-H	0 / 153	-28.0	-28.0	0.07 (1)	10.00				
H-G	0 / 153	-28.0	-28.0	0.25 (2)	10.00				
G-F	0 / 60	-28.0	-28.0	0.24 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2012
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22 (C-I-1), BC=0.25 (G-H-2), WB=0.10 (D-G-1), SSI=0.14 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2264 1656

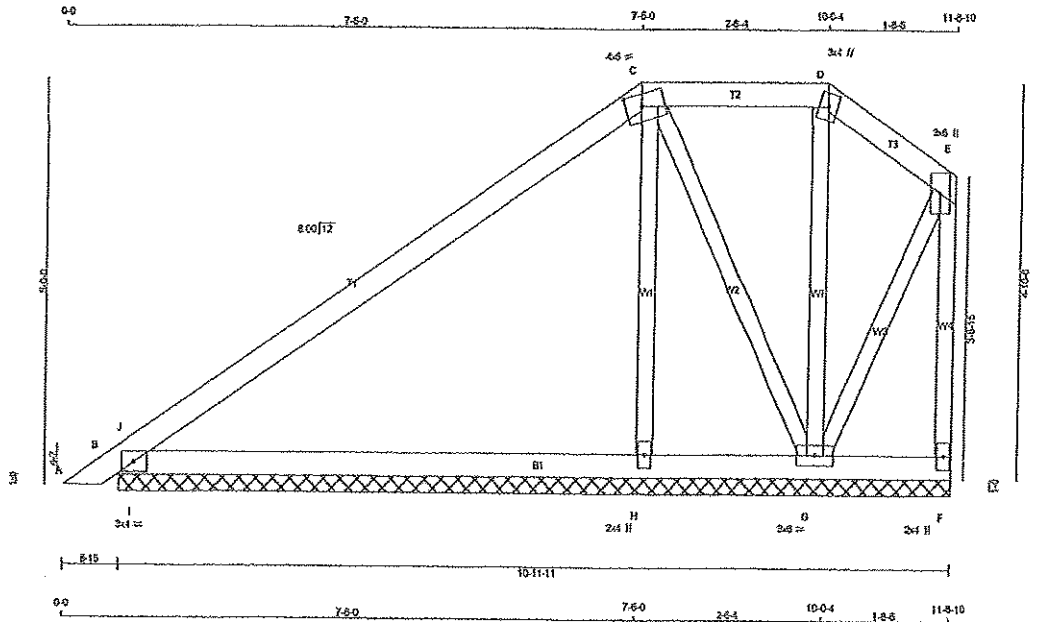
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.33 (G) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DWG NO TAM 3746874
STRUCTURAL
COMPONENT ONLY



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x3	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-J	MT20	3.0	4.0	
C	TTWW-m	MT20	4.0	6.0	1.75 2.50
D	TTWW-m	MT20	3.0	4.0	
E	TMVW+p	MT20	3.0	6.0	
F	BMV1+p	MT20	2.0	4.0	
G	BMVWW1-i	MT20	3.0	6.0	
H	BMVW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	IN-SX	BRG	IN-SX	BRG
F	210	0	210	0	0	10-11-11	1-8		
B	480	0	480	0	0	10-11-11	1-8		
H	405	0	405	0	0	10-11-11	1-8		
G	183	0	183	0	0	10-11-11	1-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	WIND	DEAD
F	173	94/0	41/0	0/0	38/0
B	384	243/0	68/0	0/0	74/0
H	373	141/0	129/0	0/0	103/0
G	119	117/0	0/-7	0/0	9/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0/14	-84.3	-84.3 0.03 (1)	H-C	-88/2	0.03 (1)	
B-J	-103/183	-84.3	-84.3 0.26 (1)	C-G	-205/0	0.09 (1)	
J-C	-168/0	-84.3	-84.3 0.50 (1)	G-D	-155/0	0.05 (1)	
C-D	-30/0	-84.3	-84.3 0.09 (1)	G-E	0/71	0.02 (1)	
D-E	-40/0	-84.3	-84.3 0.04 (1)	I-J	-835/25	0.00 (1)	
F-E	-157/0	0.0	0.0 0.05 (1)				
B-I	0/127	-28.0	-28.0 0.38 (1)				
I-H	0/127	-28.0	-28.0 0.38 (1)				
H-G	0/128	-28.0	-28.0 0.34 (2)				
G-F	0/0	-28.0	-28.0 0.07 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(56 % OF 31.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.50 (C-J:1), EC=0.38 (H-I:1), WB=0.09 (C-G:1), SI=0.67 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

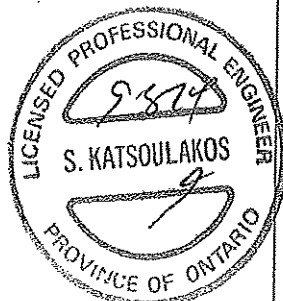
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

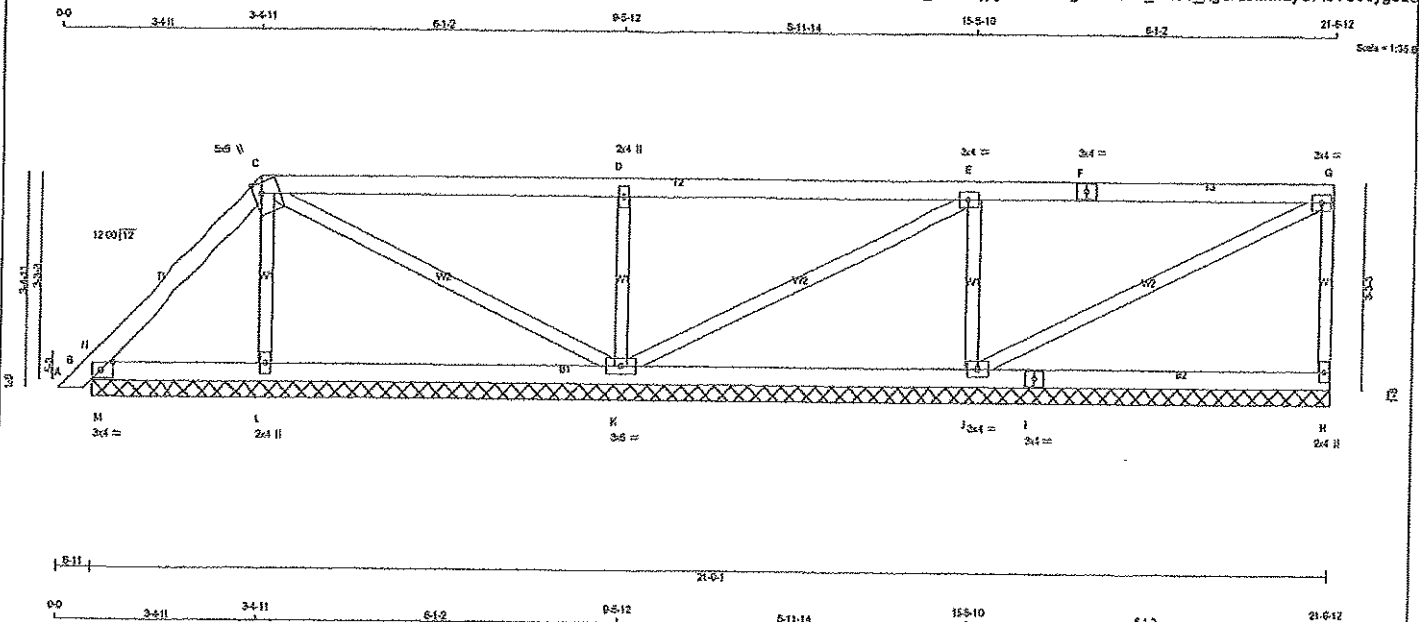


DRG NO TAM 3746914
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	PB38	1	3	TRUSS DESC.	

Tamrack Roof Truss, Burlington

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ID:zxnHLeYmAmnoxP9_eC5t7jyg/Au-busMg9ditaNE8_dHS?_xgSAi3hnnLyCPibYU3dygeLs



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x3	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER WITH (0.122\"X3\") SPIRAL NAILS STAGGERED THROUGHOUT BOTH FACES AS PER NAILING PATTERN

CHORDS	#ROWS	SPACING(IN)	LOAD(PLF)
A-C	1	12	TOP
C-F	1	12	TOP
F-G	1	12	TOP
G-H	1	12	TOP
B-I	1	12	TOP
I-H	1	12	TOP
WEBS	2x3	6	

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0 1.50 2.50
C	TTWW+m	MT20	5.0	6.0 2.00 1.50
D	TMW+rw	MT20	2.0	4.0
E	TMWV-I	MT20	3.0	4.0
F	TS-I	MT20	3.0	4.0
G	TMW-I	MT20	3.0	4.0
H	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
H	280	0	280	0	0	21-0-1 (15-1148)			
B	233	0	233	0	0	21-0-1 (15-1148)			
L	381	0	381	0	0	21-0-1 (15-1148)			
K	775	0	775	0	0	21-0-1 (15-1148)			
J	728	0	728	0	0	21-0-1 (15-1148)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS					
1ST LCASE		MAX MIN COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	WIND	DEAD
H	228	129 / 0	51 / 0	0 / 0	49 / 0
B	168	131 / 0	13 / 0	0 / 0	24 / 0
L	342	142 / 0	110 / 0	0 / 0	90 / 0
K	621	370 / 0	124 / 0	0 / 0	128 / 0
J	603	328 / 0	143 / 0	0 / 0	134 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, L, K, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (3)

CHORDS					WEBS				
		MAX. FACTORED	FACTORED				MAX. FACTORED		
MEMB.		FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.		FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM TO			FR-TO		FROM TO		
A-B	0 / 12	-84.3	-84.3	0.00 (1)	10.00	L-C	-212 / 0	0.01 (1)	
B-N	0 / 28	-84.3	-84.3	0.02 (1)	10.00	C-K	-80 / 0	0.02 (1)	
N-C	-90 / 0	-84.3	-84.3	0.03 (1)	6.25	K-D	-560 / 0	0.04 (1)	
C-D	0 / 23	-84.3	-84.3	0.14 (1)	10.00	K-E	-39 / 0	0.01 (1)	
D-E	0 / 23	-84.3	-84.3	0.14 (1)	10.00	J-E	-542 / 0	0.03 (1)	
E-F	-12 / 0	-84.3	-84.3	0.14 (1)	6.25	J-G	0 / 13	0.00 (1)	
F-G	-12 / 0	-84.3	-84.3	0.14 (1)	6.25	M-N	-265 / 0	0.00 (1)	
H-G	-212 / 0	0.0	0.0	0.02 (1)	7.81				
B-M	0 / 57	-28.0	-28.0	0.03 (1)	10.00				
M-L	0 / 57	-28.0	-28.0	0.06 (2)	10.00				
L-K	0 / 50	-28.0	-28.0	0.07 (2)	10.00				
K-J	0 / 12	-28.0	-28.0	0.09 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.09 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.09 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.14 (D-E:1), BC=0.09 (J-K:2), WB=0.04 (D-K:1), SS=0.08 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

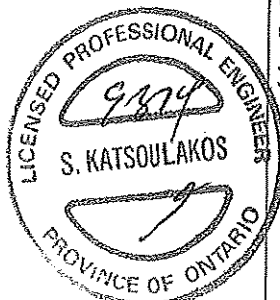
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX 1/4\"X1/4\" MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.10 (F) (INPUT = 1.00)



DRG NO TAM 37490.14
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

101/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	PB38	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:zxHLeYmAmnoxP9_eC517jvg/Au-busMgqdtanE6_dHS7_xgSAI3hndLYCPibYU3dygeLs

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	BS-I	MT20	3.0	4.0		
J	BMWW1-I	MT20	3.0	4.0		
K	BMWW1-I	MT20	3.0	6.0		
L	BMW1+u	MT20	2.0	4.0		



Re 2/2
DWG NO T&M 37470.14
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 68 LB

LUMBER					DESCR
N.L.G.A. RULES					
CHORDS SIZE					LUMBER
A - C	2x4	DRY	No.2		
C - G	2x4	DRY	No.2		SPF
G - I	2x4	DRY	No.2		
J - L	2x3	DRY	No.2		SPF
B - L	2x4	CRY	No.2		
L - J	2x4	DRY	No.2		SPF
ALL WEBS 2x3 DRY No.2					
DRY, SEASONED LUMBER.					SPF

PLATES (table in Inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMS-I	MT20	3.0	4.0	1.50	2.50
C TTWW+rm	MT20	4.0	8.0	2.25	0.75
D, E, H					
F TMWW-t	MT20	3.0	4.0		
F TMW+w	MT20	2.0	4.0		
G TS-I	MT20	3.0	4.0		
I TMVW-t	MT20	3.0	4.0		
J BMVt+p	MT20	2.0	4.0		
K, N, O					
K BMWW-t	MT20	3.0	4.0		
L BS-I	MT20	3.0	4.0		
M BMWW-t	MT20	3.0	8.0		
P BMWt+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
J	184	0	184	0	0	21-2-1 (5-2-5)-8		
B	80	0	80	0	-7	21-2-1 (5-2-5)-8		
P	241	0	241	0	0	21-2-1 (5-2-5)-8		
O	505	0	505	0	0	21-2-1 (5-2-5)-8		
N	444	0	444	0	0	21-2-1 (5-2-5)-8		
M	443	0	443	0	0	21-2-1 (5-2-5)-8		
K	519	0	519	0	0	21-2-1 (5-2-5)-8		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD
	COMBINED	SNOW	LIVE			
J	151	84/0	34/0	0/0	33/0	
B	39	64/0	0/-19	0/0	0/-5	
P	223	82/0	79/0	0/0	62/0	
O	408	237/0	88/0	0/0	85/0	
N	366	201/0	85/0	0/0	60/0	
M	362	203/0	81/0	0/0	78/0	
K	426	238/0	98/0	0/0	93/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, B, P, Q, N, M, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1	MAX (PLF)	CSI (LC)	MAX UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0/12	-84.3	-84.3	0.01 (1)	10.00	P-C	-120/0	0.02 (1)
B-R	-15/31	-84.3	-84.3	0.02 (2)	8.25	C-O	-8/0	0.00 (1)
R-C	-49/0	-84.3	-84.3	0.01 (2)	8.25	O-D	-389/0	0.05 (1)
C-D	-7/0	-84.3	-84.3	0.20 (1)	10.00	D-N	-10/0	0.00 (1)
D-E	0/2	-84.3	-84.3	0.20 (1)	10.00	N-E	-328/0	0.05 (1)
E-F	0/11	-84.3	-84.3	0.15 (1)	10.00	E-M	-9/0	0.00 (1)
F-G	0/11	-84.3	-84.3	0.21 (1)	10.00	M-F	-330/0	0.05 (1)
G-H	0/11	-84.3	-84.3	0.21 (1)	10.00	H-H	-11/0	0.00 (1)
H-I	0/0	-84.3	-84.3	0.20 (1)	10.00	K-H	-369/0	0.05 (1)
I-J	-139/0	0.0	0.0	0.02 (1)	7.81	K-I	0/0	0.00 (1)
						Q-R	-82/0	0.00 (1)
B-Q	0/27	-28.0	-28.0	0.01 (1)	10.00			
Q-P	0/27	-28.0	-28.0	0.08 (2)	10.00			
P-O	0/13	-28.0	-28.0	0.09 (2)	10.00			
O-N	0/8	-28.0	-28.0	0.08 (2)	10.00			
N-M	-2/0	-28.0	-28.0	0.09 (2)	10.00			
M-L	0/0	-28.0	-28.0	0.12 (2)	10.00			
L-K	0/0	-28.0	-28.0	0.12 (2)	10.00			
K-J	0/0	-28.0	-28.0	0.12 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- T/C 2011

(55 % OF 31.4 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21 (F-H), EC=0.12 (K-M2), WB=0.05 (D-Q), SS=0.16 (H-L)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618	354	1687 622 2284 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

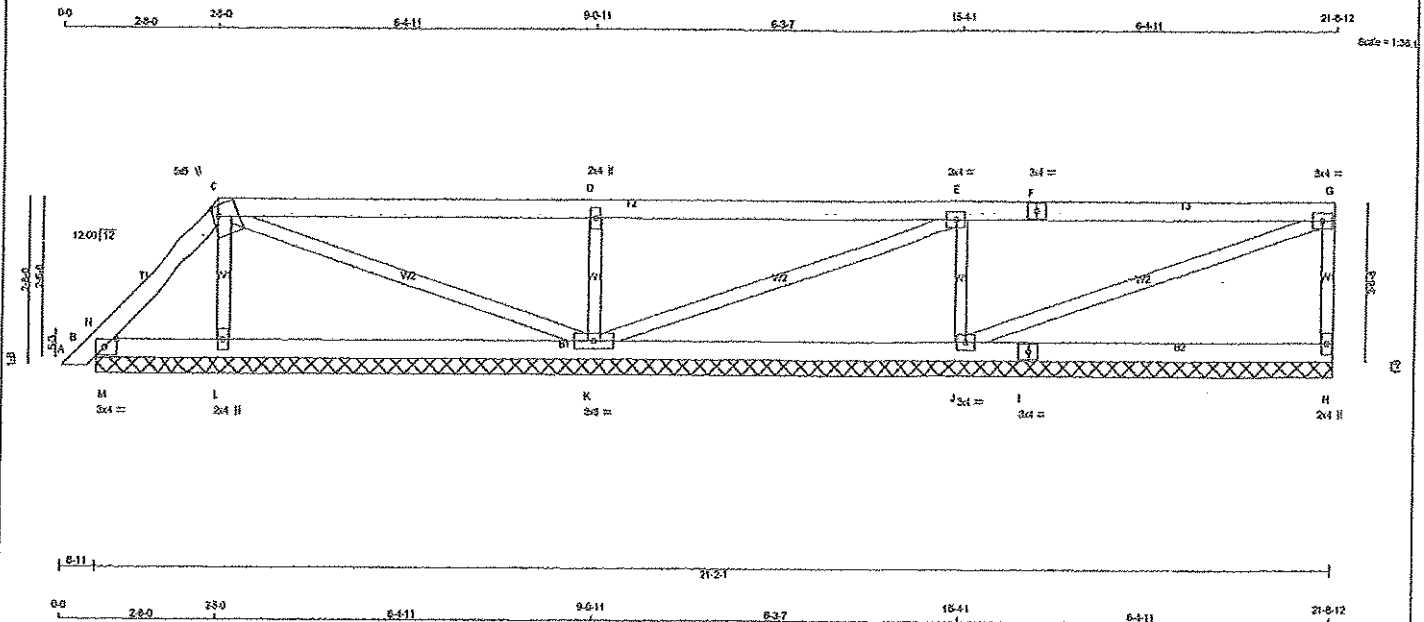
JSI GRIP= 0.88 (E) (INPUT = 0.80)
JSI METAL= 0.07 (G) (INPUT = 1.00)

DWG NO TAM37471.14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	PB40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:znHLeYmAmnoxP9_eC517jygfAu-35QkVdKeuV5k8BT0iVADfo44x4OXZxF11b4ygeLr



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
H - I	2x3	DRY	No.2
B - J	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.50
C	TTWW+m	MT20	5.0	6.0	2.00 1.25
D	TWW+u	MT20	2.0	4.0	
E	TWW+u	MT20	3.0	4.0	
F	TS-I	MT20	3.0	4.0	
G	TWW+u	MT20	3.0	4.0	
H	BMV1+p	MT20	2.0	4.0	
I	BS-I	MT20	3.0	4.0	
J	BMWW1-I	MT20	3.0	4.0	
K	BMWW1-I	MT20	3.0	8.0	
L	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
H	290	0	290	0	0	0	21-2-1 (15-11418)		
B	168	0	168	0	0	0	21-2-1 (15-11418)		
L	396	0	396	0	0	0	21-2-1 (15-11418)		
K	783	0	783	0	0	0	21-2-1 (15-11418)		
J	777	0	777	0	0	0	21-2-1 (15-11418)		

VALUE IN PARENTHESES INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	WIND	DEAD
H	237	133 / 0	53 / 0	0 / 0	61 / 0
B	111	106 / 0	0 / -5	0 / 0	9 / 0
L	357	147 / 0	118 / 0	0 / 0	85 / 0
K	631	371 / 0	130 / 0	0 / 0	130 / 0
J	642	359 / 0	150 / 0	0 / 0	141 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, L, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	CS1 (LC)	MEMB.	FORCE (LBS)	VERT. LOAD	CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 12	-84.3	-84.3 0.01 (1)	L-C	-225 / 0	0.04 (1)	
B-N	0 / 65	-84.3	-84.3 0.04 (2)	C-K	-48 / 0	0.04 (1)	
N-C	-70 / 0	-84.3	-84.3 0.04 (1)	K-D	-587 / 0	0.09 (1)	
C-D	0 / 14	-84.3	-84.3 0.46 (1)	K-E	-23 / 0	0.02 (1)	
D-E	0 / 13	-84.3	-84.3 0.46 (1)	J-E	-579 / 0	0.09 (1)	
E-F	-8 / 0	-84.3	-84.3 0.46 (1)	J-G	0 / 8	0.00 (1)	
F-G	-8 / 0	-84.3	-84.3 0.46 (1)	M-N	-204 / 0	0.00 (1)	
H-G	-219 / 0	0.0	0.0 0.04 (1)				
B-M	0 / 43	-28.0	-28.0 0.06 (1)				
M-L	0 / 43	-28.0	-28.0 0.18 (2)				
L-K	0 / 32	-28.0	-28.0 0.21 (2)				
K-J	0 / 8	-28.0	-28.0 0.28 (2)				
J-I	0 / 0	-28.0	-28.0 0.28 (2)				
I-H	0 / 0	-28.0	-28.0 0.28 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2012
- CSA 088-09
- TPC 2011

(55% OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.46 (C-D-1), BC=0.28 (J-K-2), WB=0.09 (D-K-1), SS=0.25 (E-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

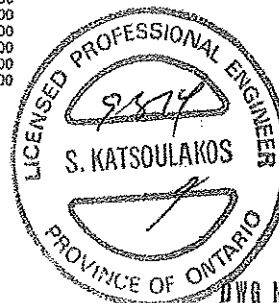
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1636

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

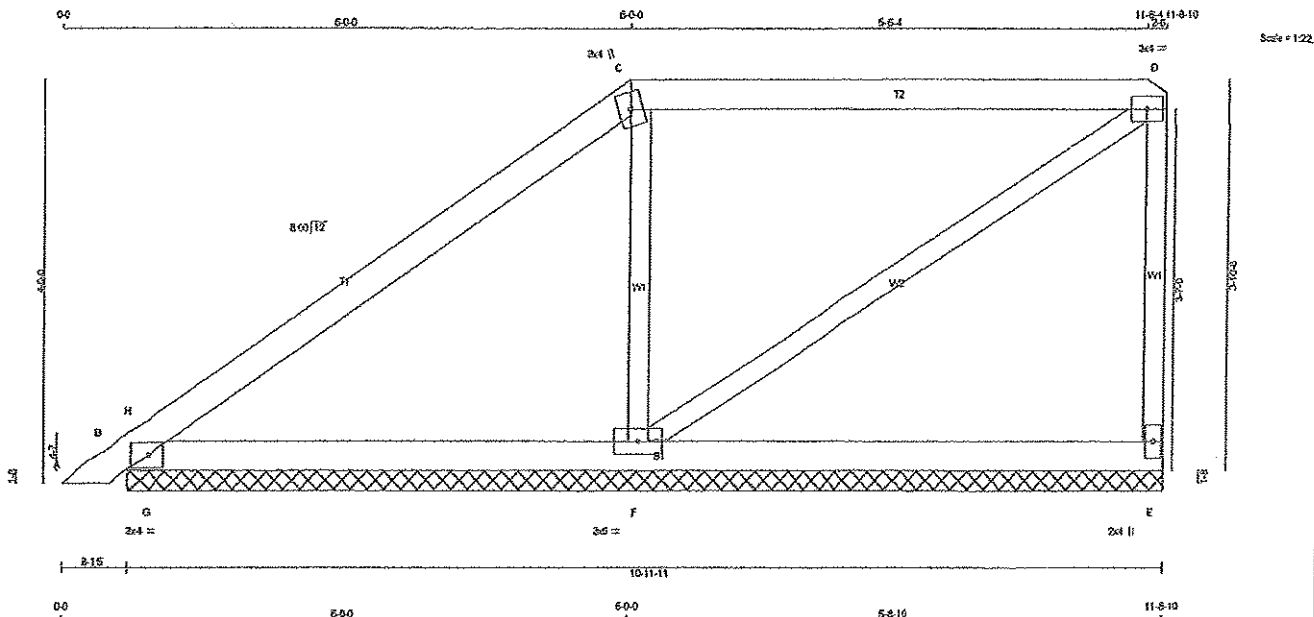


DWG NO TAM37472-14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	PB41	1	1	TRUSS DESC.	

Tamareck Roof Truss, Bufington

Version 7.500 © Oct 23 2013 MTek Industries, Inc. Fri Sep 05 16:26:57 2014 Page 1
ID:zxHLeYmAmMoxP9_eC517jygfAu-XH_65reyPBdxLmfaQ0PIfZOUPrstAv1b7WygLe



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW+m	MT20	3.0	4.0	
D	TMAW-I	MT20	3.0	4.0	
E	BMV1+p	MT20	2.0	4.0	
F	BMVW1-I	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED		MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION		GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	333	0	333	0	10-11-11
B	388	0	388	0	10-11-11
F	568	0	568	0	10-11-11

UNFACTORED REACTIONS					
	1ST LCASE		MAX	MIN	COMPONENT REACTIONS
JT	COMBINED		SNOW	LIVE	WIND
E	284	163	0	49	0
B	290	195	0	49	0
F	487	239	0	133	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURUN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.08FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS						WEBS					
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
	FORCE	(PLF)					FORCE	(PLF)			
FR-TO			FROM	TO	LENGTH	FR-TO			FROM	TO	LENGTH
A-B	0/14	-84.3	-84.3	0.03	(1)	10.00	F-C	-376	0	0.09	(1)
B-H	-65/70	-84.3	-84.3	0.13	(1)	6.25	F-D	0/76	0.02	(1)	
H-C	-112/0	-84.3	-84.3	0.30	(1)	6.25	G-H	-508	0	0.00	(1)
C-D	-84/0	-84.3	-84.3	0.47	(1)	6.25					
E-D	-282/0	0.0	0.0	0.09	(1)	7.81					
B-G	0/85	-28.0	-28.0	0.23	(1)	10.00					
G-F	0/85	-28.0	-28.0	0.27	(2)	10.00					
F-E	0/0	-28.0	-28.0	0.26	(2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 088-09
- TPIC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47 (C-D:1), BC=0.27 (F-G:2), WB=0.09 (C-F:1), SSI=0.41 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

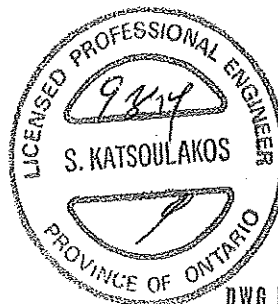
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 615 354 1687 822 2284 1656

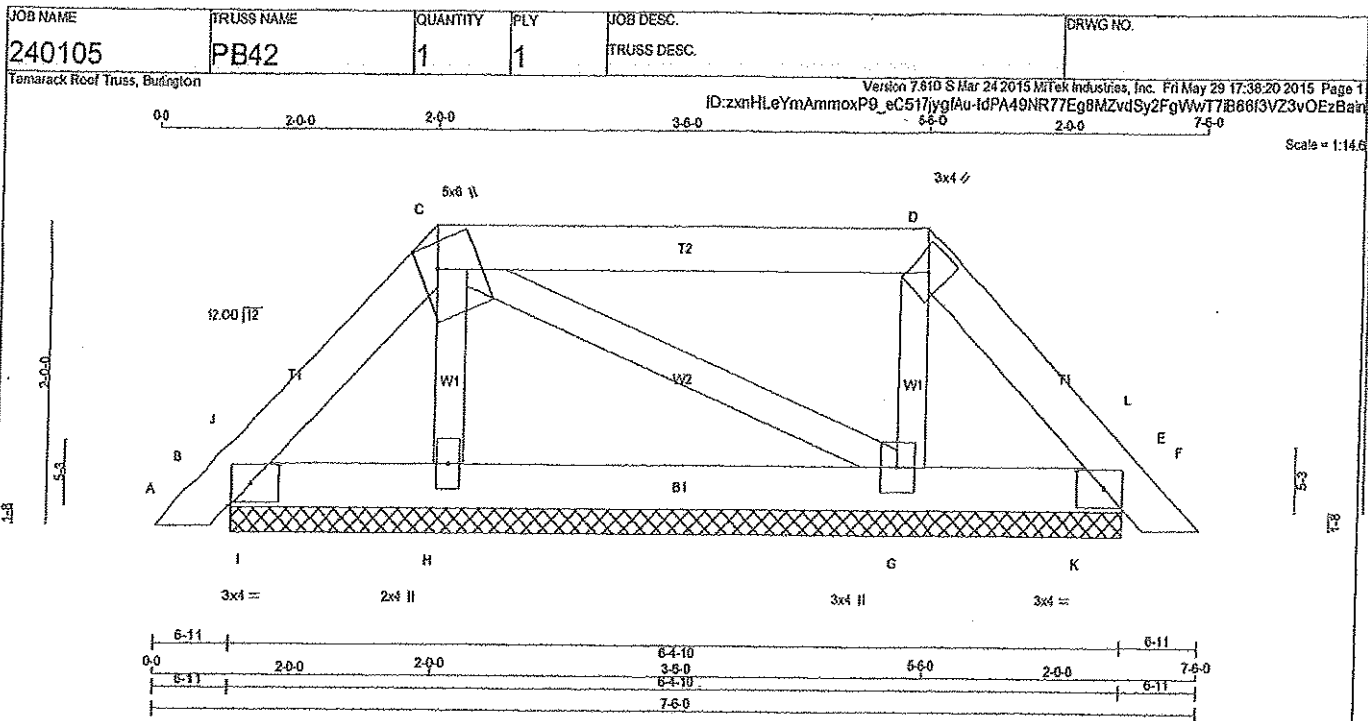
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)



DWG NO TAM 3773.14
STRUCTURAL
COMPONENT ONLY



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D	SPECIFIED LOADS:	
A - C	2x4	DRY	No. 2	SPF	FACTORED	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH.	LL = 25.6 PSF
C - D	2x4	DRY	No. 2	SPF	JT	VERT	HORZ	DOWN	HORZ	BOT CH.	LL = 3.0 PSF
D - F	2x4	DRY	No. 2	SPF	B	159	0	159	0	DL = 10.5 PSF	
B - E	2x4	DRY	No. 2	SPF	E	148	0	148	0	DL = 7.0 PSF	
ALL WEBS	2x3	DRY	No. 2	SPF	H	232	0	232	0	TOTAL LOAD = 46.1 PSF	
DRY: SEASONED LUMBER.					G	254	0	254	0	SPACING = 24.0 IN. C/C	

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND
B	TMB1-I	MT20	3.0	4.0	1.50	2.50	B	112	93/0	5/0	0/0
C	TTWW+m	MT20	5.0	6.0	2.00	1.50	E	105	88/0	5/0	0/0
D	TTW+h	MT20	3.0	4.0			H	204	91/0	62/0	0/0
E	TMB1-I	MT20	3.0	4.0	1.50	2.50	G	219	104/0	62/0	0/0
G	BMWW1+I	MT20	3.0	4.0							
H	BMWW1+I	MT20	2.0	4.0							

CHORDS				WEBS				THIS DESIGN COMPLIES WITH:			
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD LC1	MAX	MAX	MEMB.	MAX. FACTORED	- PART 9 OF OBC 2012, BCBC 2012, ABC 2014	- CSA 086-09		
FR-TO	FORCE (LBS)	VERT. LOAD LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	- TPIC 2011	(55% OF 31.4 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD)		
A-B	0/12	-84.3	-84.3	0.01 (1)	10.00	H-C	-139/0	CS: TC=0.18 (C-D:1), BC=0.05 (H:2), WB=0.02 (D-G:1), SS=0.12 (C-D:1)	DO: LUMBER=1.00 NAIL=1.00 LS BEND=1.10		
B-J	-57/4	-84.3	-84.3	0.01 (1)	6.25	C-G	-13/0	COMP=1.10 SHEAR=1.10 TENS=1.10	COMPARISON LIVE LOAD FACTOR = 0.50		
J-C	-74/0	-84.3	-84.3	0.02 (1)	6.25	G-D	-155/0	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.	NAIL VALUES		
C-D	-28/0	-84.3	-84.3	0.18 (1)	6.25	I-J	-82/0	PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)	MAX MIN MAX MIN MAX MIN		
D-L	-59/0	-84.3	-84.3	0.02 (1)	6.25	K-L	-84/0	MT20 618 354 1687 822 2284 1856	PLATE PLACEMENT TOL = 0.250 inches		
L-E	-40/5	-84.3	-84.3	0.01 (1)	6.25			PLATE ROTATION TOL = 6.0 Deg.	JSI GRIP= 0.40 (D) (INPUT = 0.90)		
E-F	0/12	-84.3	-84.3	0.01 (1)	10.00			JSI METAL= 0.04 (B) (INPUT = 1.00)			
B-I	0/49	-28.0	-28.0	0.03 (1)	10.00						
I-H	0/49	-28.0	-28.0	0.05 (2)	10.00						
H-G	0/40	-28.0	-28.0	0.05 (2)	10.00						
G-K	0/39	-28.0	-28.0	0.05 (2)	10.00						
K-E	0/39	-28.0	-28.0	0.03 (1)	10.00						



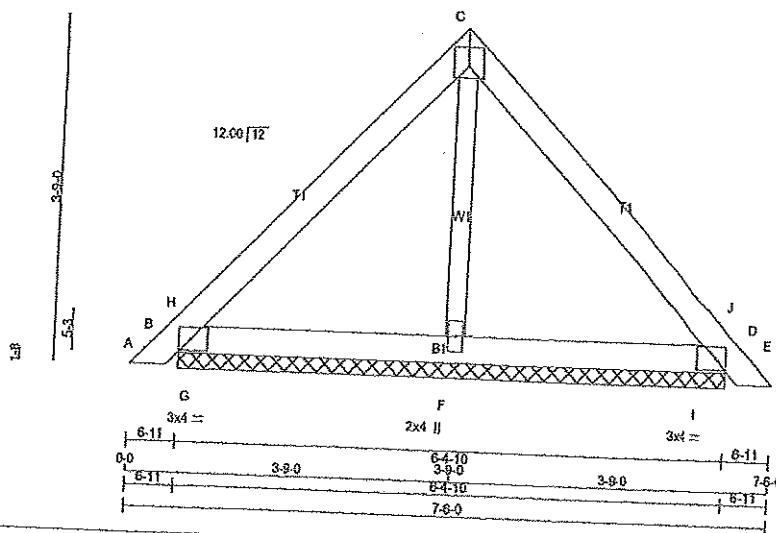
DRWG NO. TAM20109-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 240105	TRUSS NAME PB43	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamareck Roof Truss, Burlington

Version 7.610S Mar 24 2015 MTEK Industries, Inc. Fri May 28 17:38:20 2015 Page 1
ID:zxnHLeYmAmnoxP9_eC517jygfAu-kIPA49NR77Eg8M2vdSy2FgWxc7hO86q3VZ3vOEzBarr
3-9-0 3-9-0 3-9-0 7-6-0

Scale = 1:23.7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.25
F	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	281	0	281	0	6-4-10	1-8
D	281	0	281	0	6-4-10	1-8
F	230	0	230	0	6-4-10	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND PERM/LIVE	WIND DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE			
B	215	145/0	32/0	0/0	0/0	38/0	0/0
D	215	145/0	32/0	0/0	0/0	38/0	0/0
F	210	83/0	71/0	0/0	0/0	57/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0/12	-84.3 -84.3	0.01 (1)	10.00	F-C	-58/9	0.01 (1)
B-H	-73/22	-84.3 -84.3	0.08 (1)	6.25	G-H	-317/48	0.00 (1)
H-C	-127/0	-84.3 -84.3	0.10 (1)	6.25	I-J	-317/48	0.00 (1)
C-J	-127/0	-84.3 -84.3	0.10 (1)	6.25			
J-D	-73/22	-84.3 -84.3	0.08 (1)	10.00			
D-E	0/12	-84.3 -84.3	0.01 (1)	10.00			
B-G	0/83	-28.0 -28.0	0.10 (1)	10.00			
G-F	0/83	-28.0 -28.0	0.11 (1)	10.00			
F-I	0/83	-28.0 -28.0	0.11 (1)	10.00			
I-D	0/83	-28.0 -28.0	0.10 (1)	10.00			

TOTAL WEIGHT = 2 X 22 = 44 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA C88-09
- TPC 2011

(55 % OF 31.4 P.S.F. G.S.I. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (C-H:1), BC=0.11 (F-G:1), WB=0.01 (C-F:1), SS=0.25 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.37 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

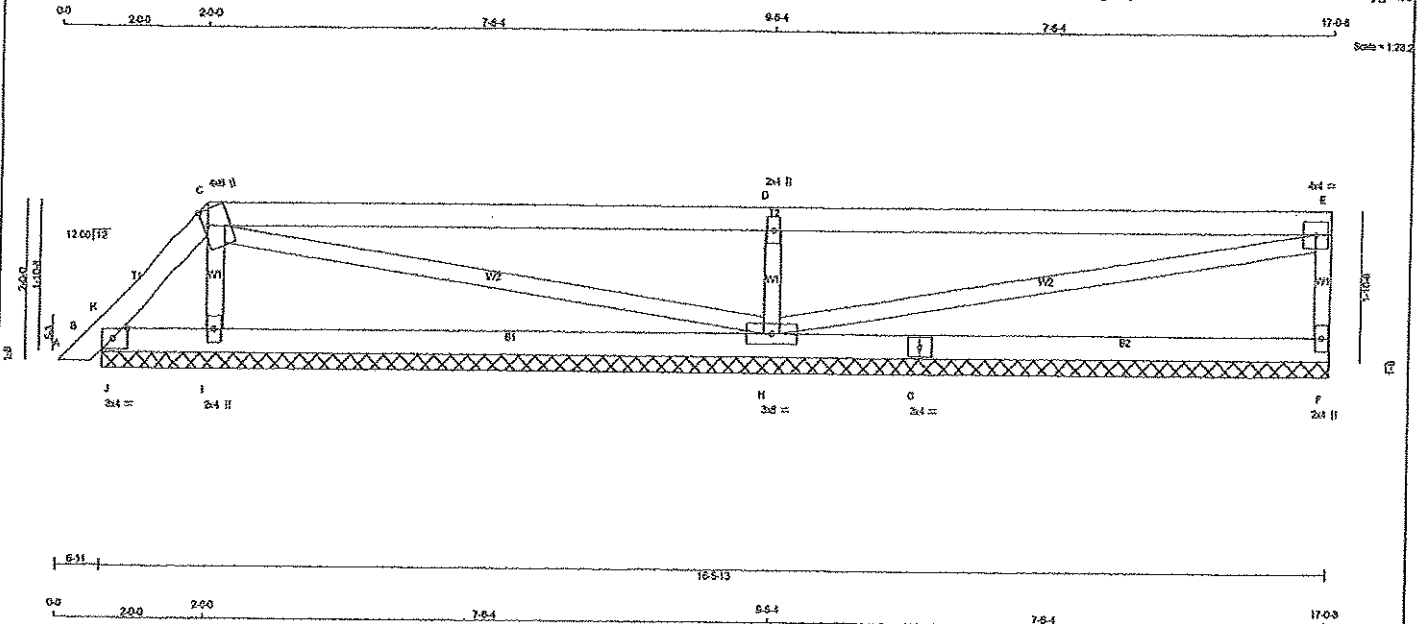


DWG NO. TAM20110-15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
240105	PB45	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 7.500 S Oct 23 2013 MiTek Industries, Inc. Fri Sep 05 16:28:59 2014 Page 1
ID:zxHLeYmAmnoxP9_eC517jyglAu-Tf5IVXgCxpTbcw2hr2trKEat3dHlx?dDWCOygeto



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x3	DRY	No.2
B - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (Table 16 in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.50
C	TTWW+m	MT20	4.0	6.0	2.25 0.75
D	TMW+w	MT20	2.0	4.0	
E	TMVW-I	MT20	4.0	4.0	
F	BMV1+p	MT20	2.0	4.0	
G	BS-I	MT20	3.0	4.0	
H	BMWWV1-I	MT20	3.0	8.0	
I	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
F	320	0	320	0	0	0	16-5-13 (10-11-82)		
B	101	0	101	0	-18	0	16-5-13 (10-11-82)		
I	429	0	429	0	0	0	16-5-13 (10-11-82)		
H	1039	0	1039	0	0	0	16-5-13 (10-11-82)		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS				
JT	COMBINED	SNOW	LIVE	WIND
F	284	145 / 0	62 / 0	0 / 0
B	44	87 / 0	0 / 32	0 / 0
I	394	152 / 0	134 / 0	0 / 0
H	844	484 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (3)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD
	(LBS)	(PLF)	LC1 MAX		(LBS)	(PLF)	LC1 MAX
FR-TO		FROM TO	CSI (LC)	FR-TO		LENGTH FR-TO	CSI (LC)
A-B	0 / 12	-84.3	-84.3 0.01 (1)	I-C	-232 / 0	0.03 (1)	
B-K	0 / 111	-84.3	-84.3 0.03 (2)	C-H	-20 / 0	0.02 (1)	
K-C	-84 / 0	-84.3	-84.3 0.04 (2)	H-D	-792 / 0	0.12 (1)	
C-D	0 / 2	-84.3	-84.3 0.01 (1)	D-E	-2 / 0	0.00 (2)	
D-E	0 / 2	-84.3	-84.3 0.01 (1)	E-F	-197 / 0	0.00 (1)	
F-E	-238 / 0	0.0	0.0 0.04 (1)				
B-J	0 / 35	-28.0	-28.0 0.04 (1)				
J-I	0 / 35	-28.0	-28.0 0.22 (2)				
I-H	0 / 20	-28.0	-28.0 0.41 (2)				
H-G	0 / 0	-28.0	-28.0 0.41 (2)				
G-F	0 / 0	-28.0	-28.0 0.41 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2012
- CSA 086-09
- TPC 2011

(55 % OF 31.4 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.81 (C-D:1), BC=0.41 (H-I:2), WB=0.12 (D-H:1), SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

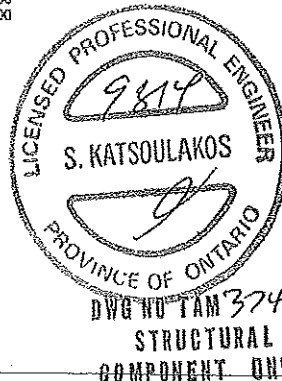
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)



STRUCTURAL
COMPONENT ONLY

Schedule 1: Designer Information

Use one form for each individual who reviews and takes responsibility for design activities with respect to the project.

A. Project Information			Application number:	
Building number, street name			Unit no.	Lot/con.
Municipality CITY OF BRAMPTON	Postal code	Plan number/ other description		
B. Individual who reviews and takes responsibility for design activities				
Name SAM KATSOULAKOS, P. ENG.		Firm MICRO CITY ENGINEERING SERVICES INC.		
Street address R.R #1, PO BOX 61			Unit no.	Lot/con.
Municipality GLENCOE	Postal code N0L 1M0	Province ONTARIO	E-mail	
Telephone number (519) 287-2242 Business		Fax number (519) 287-5750	Cell number	
C. Design activities undertaken by individual identified in Section B. [Building Code Table 3.5.2.1. of Division C]				
☐ House ☐ Small Buildings ☐ Large Buildings ☐ Complex Buildings ☐ HVAC – House ☐ Building Services ☐ Detection, Lighting and Power ☐ Fire Protection ☒ Building Structural ☐ Plumbing – House ☐ Plumbing – All Buildings ☐ On-site Sewage Systems				
Description of designer's work GREENYORK HOMES – OSTIENS – MODEL: DENTON 12 – ELEV. 1 (SCHEDULE IS NOT ISSUED AS LOT SPECIFIC) REVIEW PRE-ENGINEERED ROOF SYSTEM COMPONENT DRAWINGS AND LAYOUT PLACEMENT PLAN SUPPLIED BY TAMARACK ROOF TRUSSES INC. (SEE DWG #TAM29071-17 DATED 6-07-17). SUPPORTING STRUCTURE TO BE REVIEWED AND VERIFIED BY QUALIFIED BUILDING DESIGNER.				
D. Declaration of Designer				
I, <u>SAM KATSOULAKOS, P. ENG</u> declare that (choose one as appropriate): (print name) ☒ I review and take responsibility for the design work on behalf of a firm registered under subsection 3.2.4. of Division C, of the Building Code. I am qualified, and the firm is registered, in the appropriate classes/categories. Individual BCIN: <u>26064</u> Firm BCIN: <u>29991</u> ☐ I review and take responsibility for the design and am qualified in the appropriate category as an "other designer" under subsection 3.2.5. of Division C, of the Building Code. Individual BCIN: _____ Basis for exemption from registration: _____ ☐ The design work is exempt from the registration and qualification requirements of the Building Code. Basis for exemption from registration and qualification: _____				
I certify that: 1. The information contained in this schedule is true to the best of my knowledge. 2. I have submitted this application with the knowledge and consent of the firm.				
Date		 Signature of Designer		

NOTE:

- For the purposes of this form, "individual" means the "person" referred to in Clause 3.2.4.7(1) d) of Division C, Article 3.2.5.1. of Division C, and all other persons who are exempt from qualification under Subsections 3.2.4. and 3.2.5. of Division C.
- Schedule 1 is not required to be completed by a holder of a license, temporary license, or a certificate of authorization, issued by the Ontario Association of Architects. Schedule 1 is also not required to be completed by a holder of a license to practise, a limited license to practise, or a certificate of authorization, issued by the Association of Professional Engineers of Ontario.

DWG#TAM 29071-17-S
 DWG#TAM 29072-17-S

6777

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

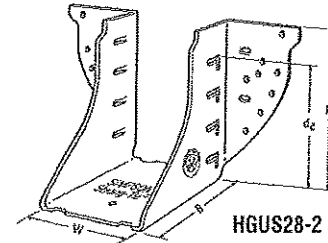
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

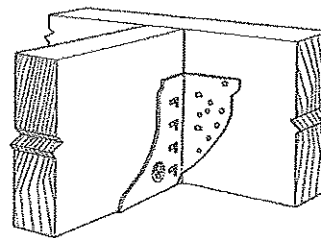
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

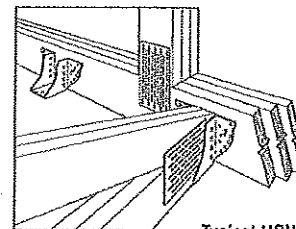
- See current catalogue for options



HGUS28-2



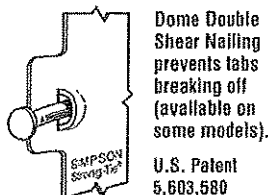
Typical HGUS
Installation



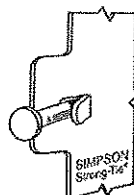
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₁ ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1½	5½	5	4½	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5½	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4½	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7½	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6½	36-16d	12-16d	6070	12980	4310	9215
HGUS210-2	12	3⅞	9⅞	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4⅞	9½	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8½	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10½	4	10½	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12½	4	11½	66-16d	22-16d	10130	16400	7195	11645

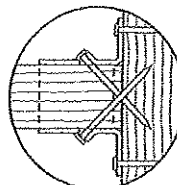
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



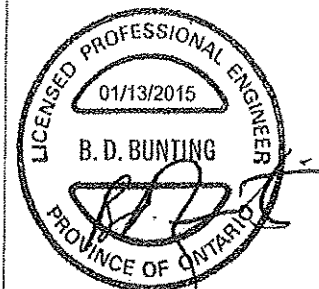
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

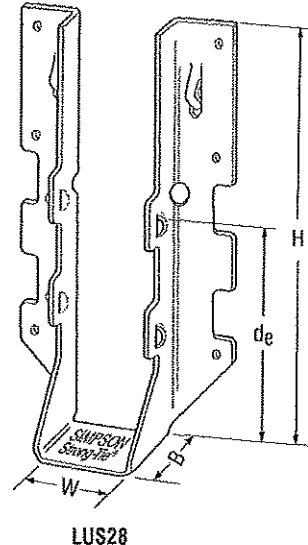
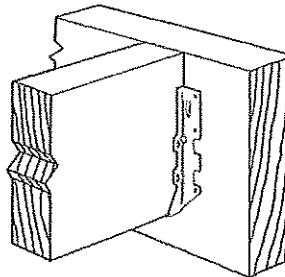
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

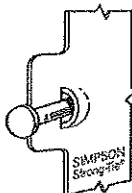
- These hangers cannot be modified.

Typical LUS Installation



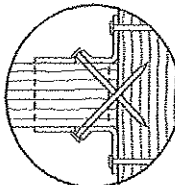
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 3/4	3 3/4	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/4	1 3/4	3 3/4	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/4	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

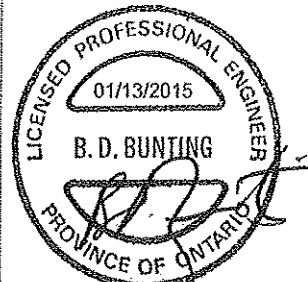


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC LUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

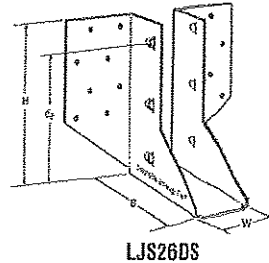
- Factored resistances are in accordance with CSA D86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

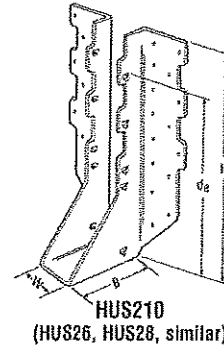
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

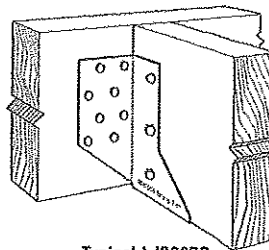
- See current catalogue for options



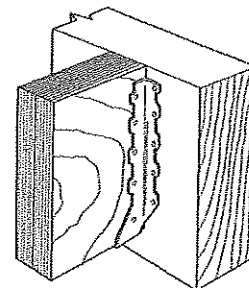
LJS26DS



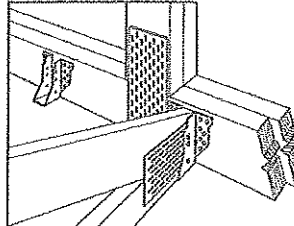
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



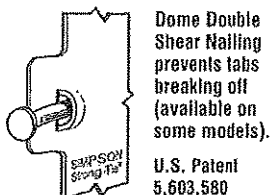
Typical HUS
Installation



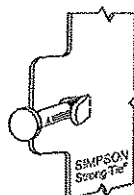
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)			Fasteners			Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/4	5	3 1/2	4 1/4	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/4	5 3/4	3	3 1/4	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/4	7 3/4	3	6 3/4	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/4	9 3/4	3	7 3/4	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/4	9	3	8	30-16d	10-16d	4505	6450	4010	5200

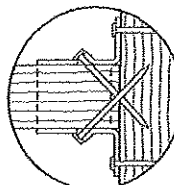
1. d_e is the distance from the seat of the hanger to the highest joist nail.



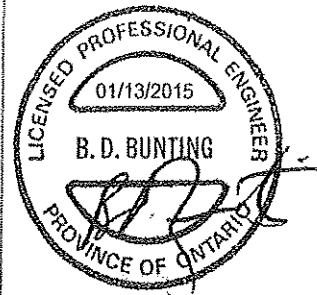
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

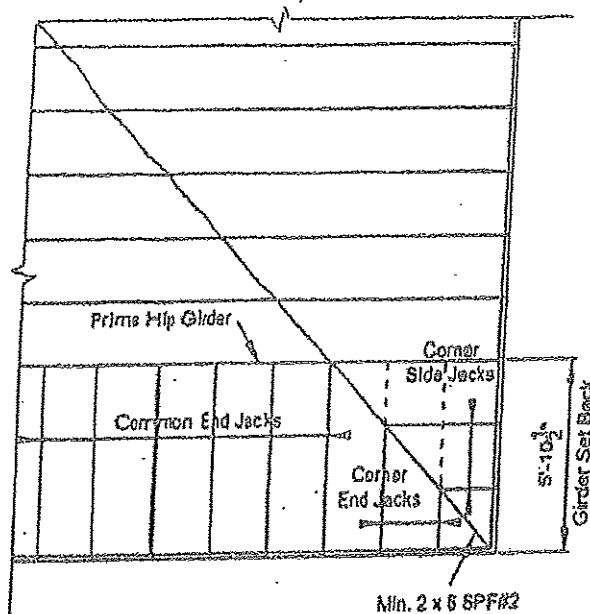
© 2015 Simpson Strong-Tie Company Inc.

T-SPEC-HUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

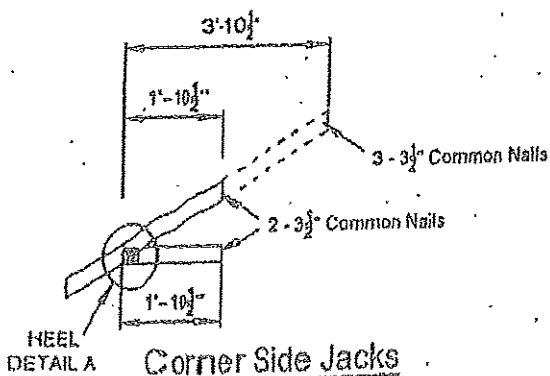
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

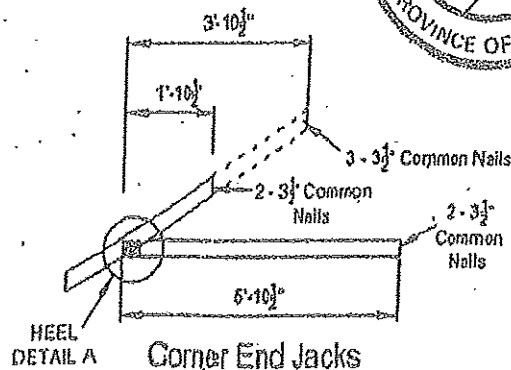
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

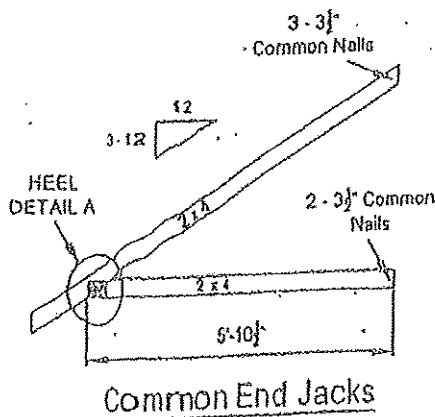
DWG NO T&M 3495.14
STRUCTURAL
COMPONENT ONLY



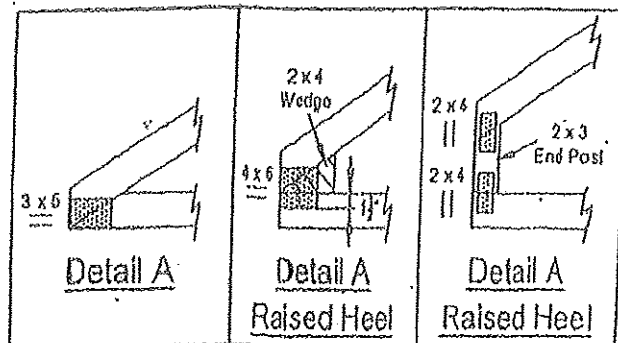
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

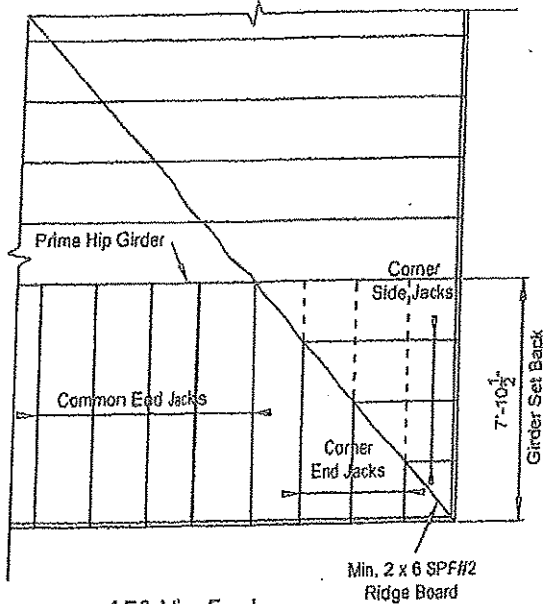
MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



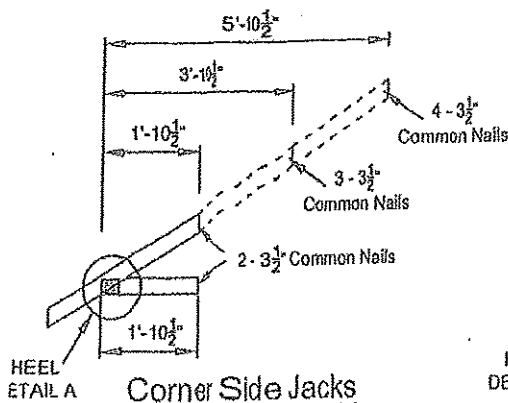
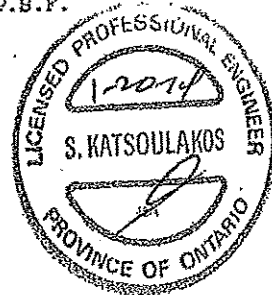
45° Hip End

DESIGN LOAD:

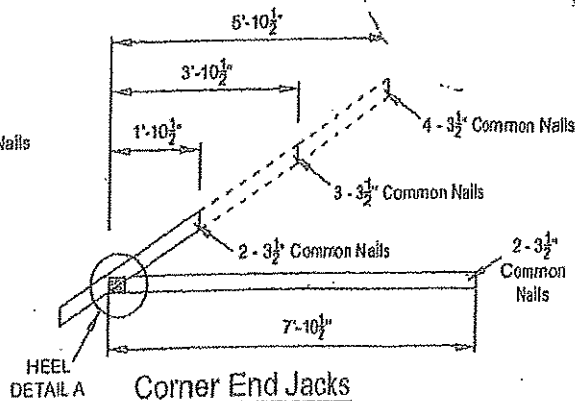
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

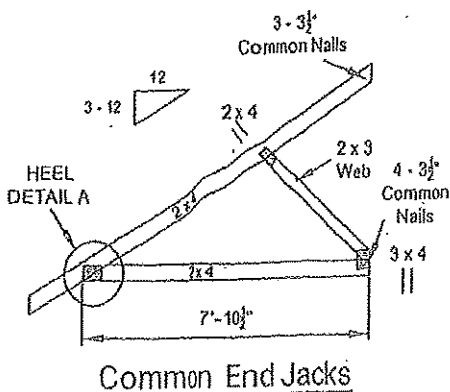
DWG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY



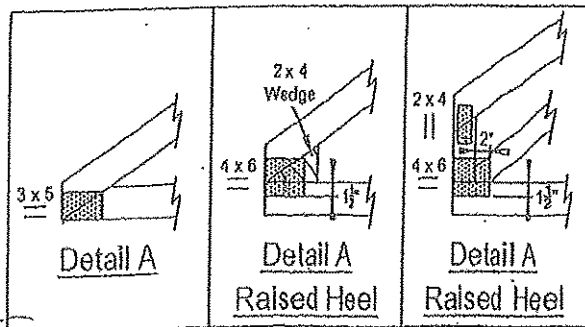
Corner Side Jacks



Corner End Jacks



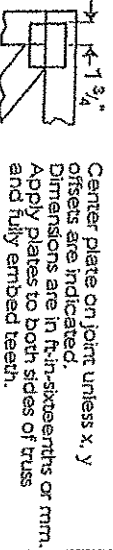
Common End Jacks



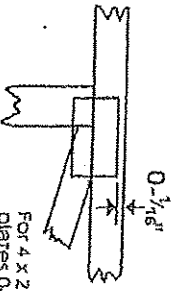
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-3/4\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

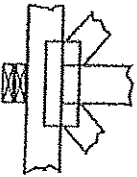
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

BEARING

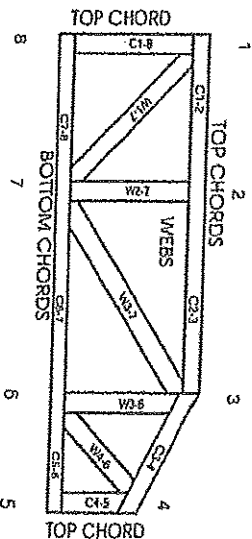


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DS9-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

8-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

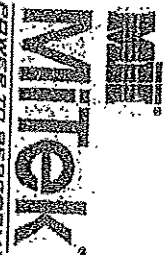
PRODUCT CODE APPROVALS

CCMC Reports:

11998-L, 10319-L, 13270-L, 12591-R

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Mitek Engineering Reference Sheet MTK-7473C Rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

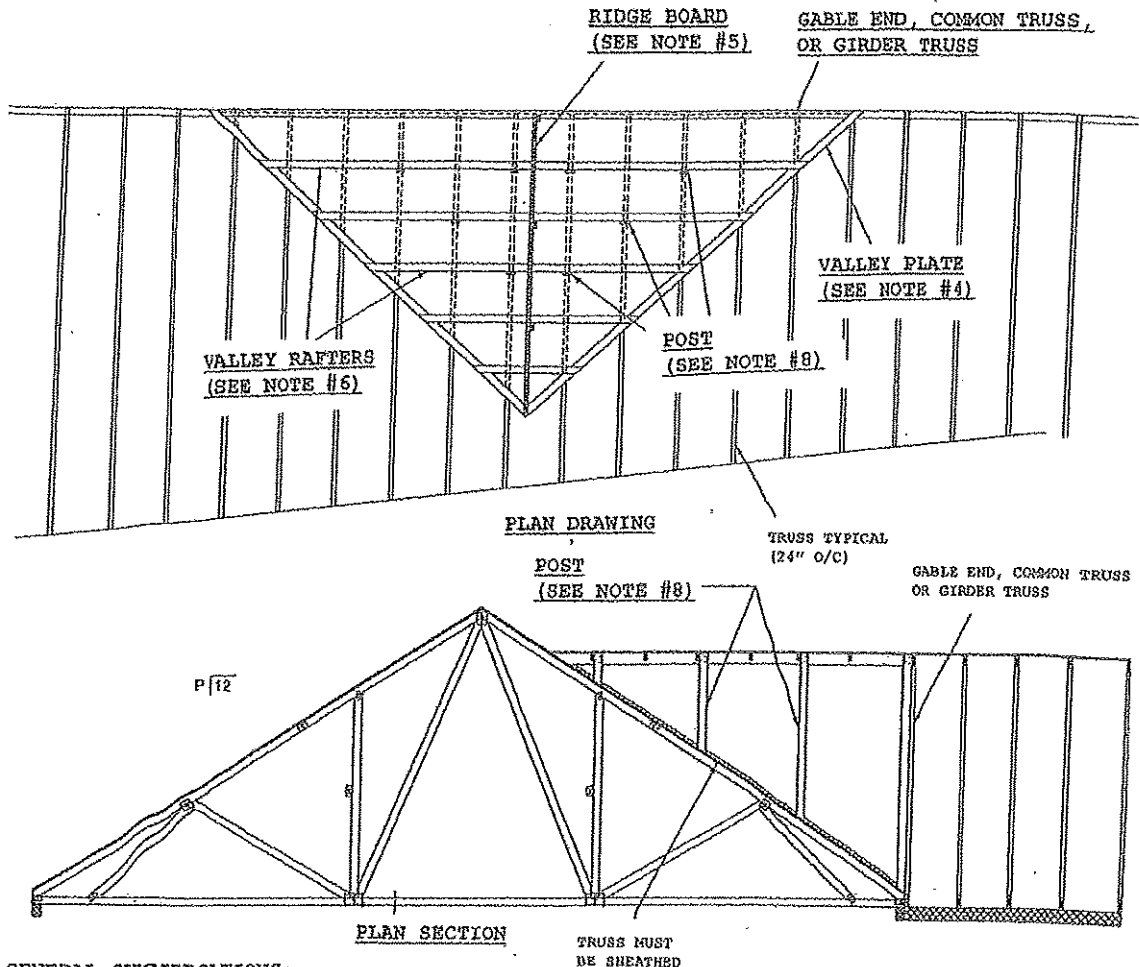
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 15% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C, BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: $P = 4$ (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: $P = 4$ (4/12) - MINIMUM.



DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

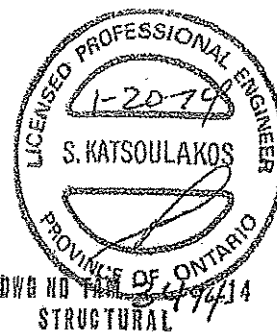
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tplic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.