TW1218-081 Page 1 of 42

Engineering Note Page (ENP-2)

REVISION 2018-10-17

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

<u>CODE</u>

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

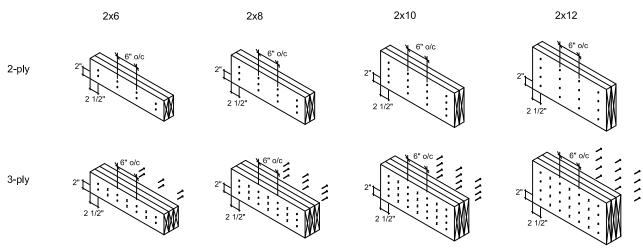
HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



W1218-081 Page 2 of 42

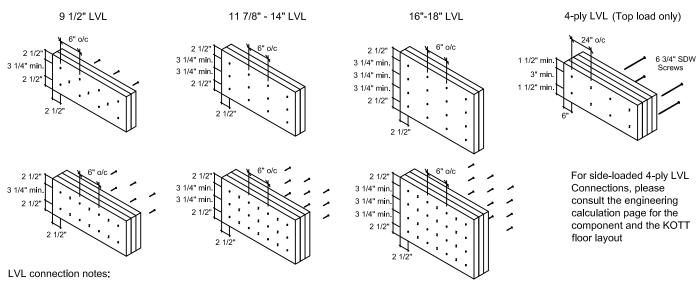
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections



- -LVL ply width is 1-3/4"
- -Nails to be 3 1/2" common wire nails.
- -Nails to be located 2 1/2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Multiple Member Connections

All connections are for uniformly distributed loads.

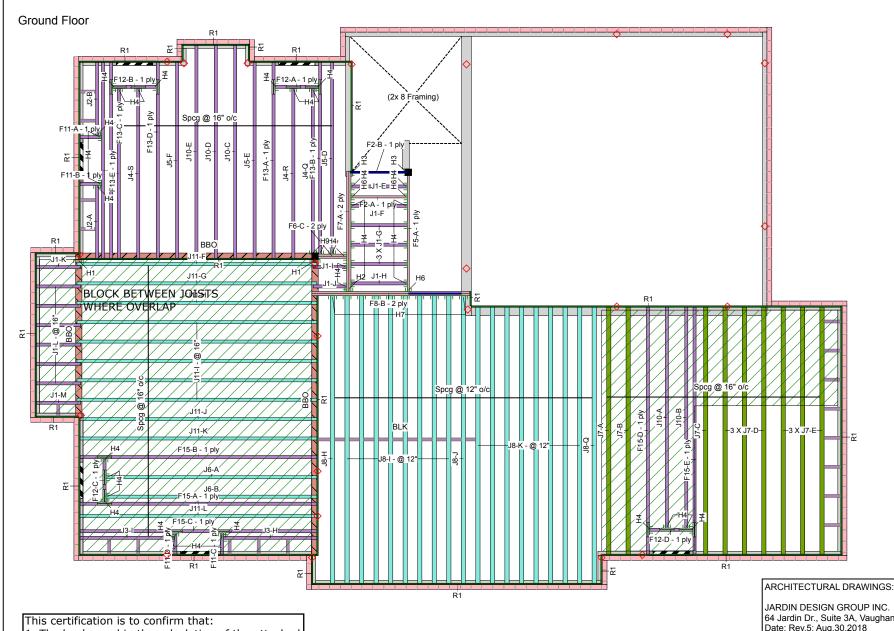
For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



TW1218-081



1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.5; Aug.30,2018 Project No: 17-55 Model: Celestial 2

Legend



Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ40U 11.875 NJ60H 11.875 NJ60U 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Page 3 of 42												
Ground												
LVL/LS	LVL/LSL (Flush)											
Label	Descri	ption	Wid	th	Dep	oth	Qty	Plies	Pcs	Length	NAS	CUR
F8	Forex 2.0E-30	000Fb LVL	1.1	75	11.8	375	1	2	2	12-0-0		
F7	Forex 2.0E-30	000Fb LVL	1.1	75	11.8	375	1	2	2	10-0-0	Layout Name LOT 1 (CELESTIAL 2 EL.1)	
F5	Forex 2.0E-30	000Fb LVL	1.1	75	11.8	375			1	10-0-0	Design Method	
F2	Forex 2.0E-30	000Fb LVL	1.1	75	11.8	375			2	4-0-0	Description	
F6	Forex 2.0E-30	000Fb LVL	1.1	75	11.8	375	1	2	2	4-0-0	GRANELLI HOME BRAMPTON, ONT	
I Joist (Flush)										Created	
Label	Descri	ption	Wid	th	Dep	oth	Qty	Plies	Pcs	Length	May 30, 2018	
F15	LPI 20F	Plus	2	2.5	11.8	375			5	18-0-0	Builder	
F13	LPI 20F			2.5	11.8				5	14-0-0	GREEN YORK HO	MES
F12	LPI 20F			2.5	11.8	-			4	4-0-0	_	,,,,,,,
F11	LPI 20F			2.5	11.8				4	2-0-0	Sales Rep	
J10	LPI 20F			2.5	11.8				5	16-0-0	RM	
J5	LPI 20F			2.5	11.8				3	14-0-0	Designer	
J4	LPI 20F			2.5	11.8				4	12-0-0	RCO	
J3	LPI 20F			2.5	11.8				2	8-0-0	Shipping	
J2	LPI 20F			2.5	11.8				2	6-0-0	Project	
J1	LPI 20F	Plus		2.5	11.8				16	4-0-0		
J7	NJ40U			3.5	11.8				9	18-0-0	Builder's Project	
J11	NJ60H			2.5	11.8				11	18-0-0	Kott Lumber	Company
J6	NJ60H			2.5	11.8				2 19	16-0-0	14 Anderson Blvd	
J8	NJ60U		3	3.5	11.8	5/5			19	20-0-0	Stouffville, Ontario	
Rim Bo		ntion	۱۸/: ما	46	Dar	a 4 la	Ohi	Dline	Daa	Lanath	Canada	
	Descri		Wid	_	Dep		Qty	Plies	Pcs	Length	L4A 7X4	
R1	Plus 1.	d Rimboard	1.12	25	11.8	3/5			17	12	905-642-4400	
	11.875	125 /									Job Path	
Blockin											D:\Users\rochavillo	WORK EDOM
	Descri	ntion	Wid	th	Der	oth	Qtv	Plies	Pcs	Length	HOME\GREEN YO	
	LPI 20			2.5	11.8	. ,			Varies 29-0-0 \GRANELLI HOME CORP\MC			
Hanger												AL 2 ELEV.1)\FLOOR
. 3.							Bea	am/Girder	Sur	ported	\LOT 1 (CELESTIA	AL 2 EL.1).isl
										ember	Ground Floor	
Label	Pcs	Descriptio	n	Sk	ew	Slope	e fa	asteners	fas	teners	Design Method	LSD
H1	2	Unknown									Building Code N	BCC 2010 / OBC
		Hanger									Floor	2012
H2	2	HGUS410						46 16d	10	5 16d		
H3	2	HUCQ1.81/ SDS	/9-								Loads	40
H4	40	LT251188					4	10dx1 1/2	2 10	dx1 1/2	Live	
H6	3	HUS1.81/1	n				+ -	30 16d		0 16d	Dead	15
H7	10	LT351188					4	10dx1 1/2		dx1 1/2	Deflection Joist	400
H9	1	LT251188					† ·			<u> </u>	LL Span L/	480
NOTES:											TL Span L/	360
110120.											LL Cant 2L/	480
1. Frame	er to ver	ify dimensio	ns on	the a	archi	tectura	l drawir	ngs.			TL Cant 2L/	360
Double joist only require filler/backer ply when supporting							Deflection Girder	•				
another member using a face-mounted hanger.							LL Span L/	360				
Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. Install single-ply flush window header along inside face of							TL Span L/	240				
rimboard/rimjoist.						LL Cant 2L/	480					
5. Refer	to Nasc	or specifier o									TL Cant 2L/	360
		recommen									Decking	
		ists which sı r or roof.	upport	ioad	uing t	ıom at	ove ex	ceeaing			Deck	SPF Plywood
		blocks to be	install	led ι	ınder	all poi	nt loads	S.			Thickness	3/4"
8. It shall	l be the	framer's res	ponsib	oility	that t	floor jo	sts and		е		Fastener	Nailed & Glued
fasten	ed as pe	er the hange	r man	ufac	turer'	's stand	dards.				Vibration	
l		Vibration										

- Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

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isDesign™

Client: Project: Address:

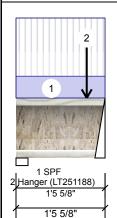
GREEN YORK HOMES

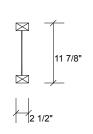
12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

11.875" - PASSED LPI 20Plus

Level: Ground Floor





Page 1 of 1

Member Information							
Type:	Girder	Application:	Floor (Residential)				
Plies:	1	Design Method:	LSD				
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012				
Deflection LL:	360	Load Sharing:	No				
Deflection TL:	240	Deck:	Not Checked				
Importance:	Normal	Vibration:	Not Checked				
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						

•	Unfactored Reactions UNPATTERNED Ib (Uplift)									
E	3rg	Live	Dead	Snow	Wind					
l	1	61	23	0	0					
l	2	114	42	0	0					

Bearings and Factored Reactions Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 7% 29 / 92 120 L 1.25D+1.5L 2 -2.000" 14% 53 / 171 224 L 1.25D+1.5L Hanger

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	42 ft-lb	11 7/16"	6250 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	211 lb	1'4 3/8"	2345 lb	0.090 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/36074)	11 5/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/26279)	11 5/16"	0.061 (L/240)	0.010 (1%)	D+L	L

1-2-0

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.

Point

									Decen
4 Girders	are designed to be suppor	ted on the bottom ed				Decen			
5 Top flang	ge braced at bearings.								
6 Bottom f	flange braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Far Face

30 lb

81 lb



required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Pass-Thru Framing Squash Block is

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is valid until 10/31/2020

Manufacturer Info

0 lb

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0 lb





TW1218-081 Page 5 of 42

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Client: **GREEN YORK HOMES** Project:

Address:

12/14/2018

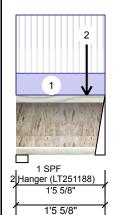
Designer: RCO

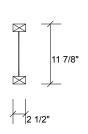
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor





Page 1 of 1

wember II	ntormation
Type:	Girder
Plies:	1

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load

40 PSF Floor Live: 15 PSF Dead:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	61	23	0	0
2	112	42	0	0

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Vibration: Not Checked

Application:

Design Method:

Bearings and Factored Reactions

Bearing Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.375"	7%	28 / 91	119	L	1.25D+1.5L
2 - 2.000"	14%	52 / 167	219	L	1.25D+1.5L
Hanger					

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	41 ft-lb	11 5/16"	6250 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	206 lb	1'4 3/8"	2345 lb	0.088 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/36734)	11 3/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/26745)	11 3/16"	0.061 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

T.L. WISE 100083566 VCE OF ON

December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-0		Near Face	29 lb	78 lb	0 lb	0 lb	J2

Floor (Residential)

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

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Client: Project:

Address:

GREEN YORK HOMES

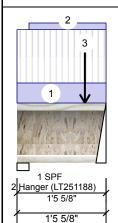
12/14/2018 Date: Designer: RCO

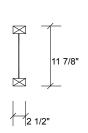
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor





Page 1 of 1

Member In	formation
Type:	Girder
Plies:	1
Moisture Con	dition: Dry

Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load

40 PSF Floor Live: 15 PSF Dead:

Application: Floor (Residential)

Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	68	32	0	0
2	127	63	0	0

Bearings and Factored Reactions

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 41 / 102 9% 143 L 1.25D+1.5L 2.000" 2 -17% 79 / 191 269 L 1.25D+1.5L Hanger

Analysis Results

Analysis Actual Comb. Case Location Allowed Capacity Moment 59 ft-lb 1' 3/4" 6250 ft-lb 0.009 (1%) 1.25D+1.5L L Shear 1'4 3/8" 2345 lb 0.109 (11%) 1.25D+1.5L L Perm Defl in. 0.000 1' 3/8" 0.041 (L/360) 0.010 (1%) D Uniform (L/56853) LL Defl inch 0.001 1' 7/16" 0.041 (L/360) 0.010 (1%) L (L/28228) TL Defl inch 0.001 1' 3/8" 0.061 (L/240) 0.010 (1%) D+L L (L/18862)



- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

T.L. WISE 100083566 VCE OF ON

December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF
2	Part. Uniform	0-2-6 to 1-5-10		Тор	8 PLF	0 PLF	0 PLF
3	Point	1-1-7		Near Face	50 lb	101 lb	0 lb

0 PLF Pass-Theu Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

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CCMC: 12412-R APA: PR-L238C

Wind

0 PSF

Comments





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Client: **GREEN YORK HOMES** Project:

Address:

Date: 12/14/2018

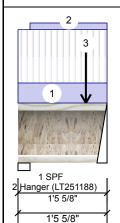
Designer: RCO

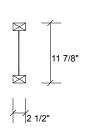
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

LPI 20Plus 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Information					
Type:	Girder				
Plies:	1				
Majatura Canditianu	Dm.				

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load

40 PSF Floor Live: 15 PSF Dead:

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	69	33	0	0
2	129	65	0	0

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Vibration: Not Checked

Application:

Design Method:

Floor (Residential)

Bearings and Factored Reactions

Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	9%	41 / 103	144	L	1.25D+1.5L
2 - Hanger	2.000"	17%	81 / 194	275	L	1.25D+1.5L

Analysis Results

Capacity Analysis Actual Location Allowed Comb. Case Moment 60 ft-lb 1' 7/8" 6250 ft-lb 0.010 (1%) 1.25D+1.5L L Shear 1'4 3/8" 2345 lb 0.111 (11%) 1.25D+1.5L L Perm Defl in. 0.000 1' 1/2" 0.041 (L/360) 0.010 (1%) D Uniform (L/55408) 1' 1/2" 0.041 (L/360) 0.010 (1%) L LL Defl inch 0.001 (L/27690) TL Defl inch 0.001 1' 1/2" 0.061 (L/240) 0.010 (1%) D+L L





December 17, 2018

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF
2	Part. Uniform	0-2-6 to 1-5-10		Тор	8 PLF	0 PLF	0 PLF
3	Point	1-1-7		Far Face	52 lb	104 lb	0 lb

0 PLF Pass-Theu Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

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Wind

0 PSF

Comments





TW1218-081 Page 8 of 42

isDesign™

Client: Project: Address:

GREEN YORK HOMES

12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

96

137

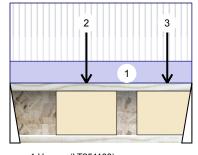
Live

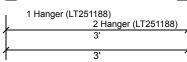
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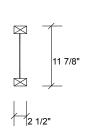
11.875" - PASSED LPI 20Plus

Level: Ground Floor





15 PSF



Wind

0

0

Ld. Comb.

1.25D+1.5L

Page 1 of 1

Wichinger zimonn	1411011		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Bearings and Factored Reactions Bearing Length Cap. React D/L lb

Hanger

Hanger

2 -

2.000"

2.000"

Brg

1

2

Total Ld. Case 120 / 384 504 L 45% 171 / 550

721 L 1.25D+1.5L

0

0

Analysis Results

Dead:

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	529 ft-lb	1'3 1/8"	6250 ft-lb	0.085 (8%)	1.25D+1.5L	L
Shear	714 lb	2'10 3/4"	2345 lb	0.304 (30%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/15127)	1'3 1/8"	0.093 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.006 (L/5660)	1'3 1/8"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.008 (L/4119)	1'3 1/8"	0.140 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings

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December 17, 2018	

ID Load Type Location Trib Width Side	Dead	Live	Snow
1 Tie-In 0-0-0 to 3-0-0 (Span)1-8-11 Top	15 PSF	40 PSF	0 PSF
2 Point 1-3-2 Near Face	116 lb	310 lb	0 lb
3 Point 2-7-2 Near Face	78 lb	209 lb	0 lb

required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Canada

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF 0 lb Comments



Kott Lumber Company 14 Anderson Blvd, Ontario



905-642-4400

TW1218-081 Page 9 of 42

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Client: Project: Address:

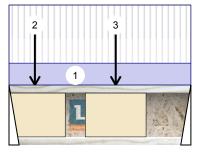
GREEN YORK HOMES

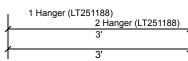
Date: 12/14/2018 Designer: RCO

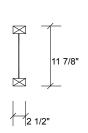
Job Name: LOT 1 (CELESTIAL 2 EL.1)

LPI 20Plus 11.875" - PASSED

Level: Ground Floor







Page 1 of 1

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240

Member Information

240 Importance: Normal General Load 40 PSF

15 PSF

Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked

Floor (Residential)

Application:

Vibration: Not Checked Brg 1 2

Unfactored Reactions UNPATTERNED Ib (Uplift) Live Dead 366 137 O 259 97 0

Wind 0 0

Analysis Results

Floor Live:

Dead:

Analysis Actual Location Allowed Capacity Capacity <t< th=""><th>•</th><th></th><th></th><th></th><th></th><th></th><th></th></t<>	•						
Shear 713 lb 1 1/4" 2345 lb 0.304 (30%) 1 Perm Defl in. 0.002 (L/15183) 1'9 1/8" 0.093 (L/360) 0.020 (2%) D	nalysis Ac	Actual L	ocation	Allowed	Capacity	Comb.	Case
Perm Defl in. 0.002 1'9 1/8" 0.093 (L/360) 0.020 (2%) D	loment 52	27 ft-lb	1'9 1/8"	6250 ft-lb	0.084 (8%)	1.25D+1.5L	L
(L/15183)	hear 71	13 lb	1 1/4"	2345 lb	0.304 (30%)	1.25D+1.5L	L
LL Defl inch 0.006 (L/5673) 1'9 1/8" 0.093 (L/360) 0.060 (6%) L			1'9 1/8"	0.093 (L/360)	0.020 (2%)	D	Uniform
	L Defl inch 0.0	.006 (L/5673)	1'9 1/8"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch 0.008 (L/4130) 1'9 1/8" 0.140 (L/240) 0.060 (6%) D	L Defl inch 0.0	.008 (L/4130)	1'9 1/8"	0.140 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.002", Long Term = 0.003"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings.

Bearings	Bearings and Factored Reactions									
Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.				
1 - Hanger	2.000"	45%	171 / 549	720	L	1.25D+1.5L				
2 - Hanger	2.000"	32%	121 / 389	510	L	1.25D+1.5L				



December 17, 2018

7 Bollom hange	braceu at bearings.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF
2	Point	0-5-2		Near Face	80 lb	214 lb	0 lb
3	Point	1-9-2		Near Face	115 lb	308 lb	0 lb

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF 0 lb Comments





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Client: Project: Address:

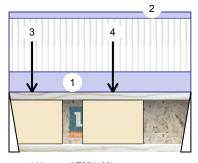
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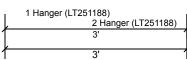
12/14/2018 Designer: RCO

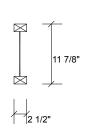
Job Name: LOT 1 (CELESTIAL 2 EL.1)

11.875" - PASSED LPI 20Plus

Level: Ground Floor







Page 1 of 1

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Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	451	219	0	0
2	302	147	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	693 ft-lb	1'8 9/16"	6250 ft-lb	0.111 (11%)	1.25D+1.5L	L
Shear	943 lb	1 1/4"	2345 lb	0.402 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9546)	1'8 9/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4632)	1'8 9/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3119)	1'8 9/16"	0.140 (L/240)	0.080 (8%)	D+L	L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	693 ft-lb	1'8 9/16"	6250 ft-lb	0.111 (11%)	1.25D+1.5L	L
Shear	943 lb	1 1/4"	2345 lb	0.402 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9546)	1'8 9/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4632)	1'8 9/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3119)	1'8 9/16"	0.140 (L/240)	0.080 (8%)	D+L	L

Bearings and Factored Reactions

ш								
Γ	Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.	
	1 -	2.000"	60%	274 / 677	950	L	1.25D+1.5L	
	Hanger							
	2 -	2.000"	40%	184 / 453	637	L	1.25D+1.5L	
ı	Hanger							

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.004", Long Term = 0.005"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings.



December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In (0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-4-9		Near Face	126 lb	260 lb	0 lb	0 lb	J6
4	Point	1-8-9		Near Face	189 lb	390 lb	0 lb	Pas s - \ ₱h	rป6Framing Sc

Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

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www.lpcorp.com CCMC: 12412-R APA: PR-L238C



TW1218-081 Page 11 of 42



Client: Project:

Address:

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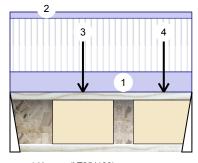
12/14/2018 Designer: RCO

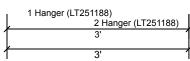
Job Name: LOT 1 (CELESTIAL 2 EL.1)

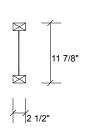
Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor







Page 1 of 1

Member Information

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dood:	15 DSE

Application: Floor (Residential) Design Method:

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No

Deck: Not Checked Vibration: Not Checked

Dead: **15 PSF**

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	325	159	0	0
2	450	218	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	701 ft-lb	1'2 7/16"	6250 ft-lb	0.112 (11%)	1.25D+1.5L	L
Shear	941 lb	2'10 3/4"	2345 lb	0.401 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9387)	1'2 7/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4578)	1'2 7/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3077)	1'2 7/16"	0.140 (L/240)	0.080 (8%)	D+L	L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	701 ft-lb	1'2 7/16"	6250 ft-lb	0.112 (11%)	1.25D+1.5L	L
Shear	941 lb	2'10 3/4"	2345 lb	0.401 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9387)	1'2 7/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4578)	1'2 7/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3077)	1'2 7/16"	0.140 (L/240)	0.080 (8%)	D+L	L

Bearings and Factored Reactions

ш								
	Bearing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.	
	1 -	2.000"	43%	198 / 487	686	L	1.25D+1.5L	
	Hanger							
	2 -	2.000"	60%	273 / 676	948	L	1.25D+1.5L	
l	Hanger							

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.004", Long Term = 0.005"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings.



December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-7		Far Face	191 lb	391 lb	0 lb	0 lb	J10
4	Point	2-6-7		Far Face	135 lb	281 lb	0 lb	Pass9-l⊽h	ru¹Framing S

Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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Manufacturer Info

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Client: Project: Address:

GREEN YORK HOMES

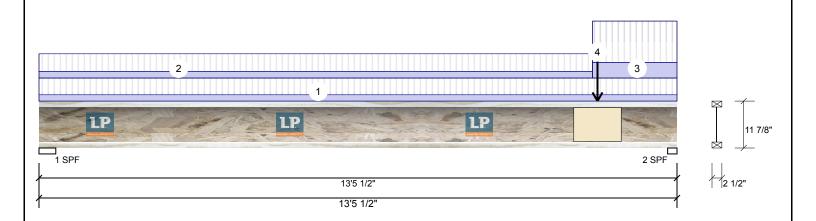
Date: 12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactore	ed Reaction	ons UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	397	149	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	642	241	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 4	.250"	43% 186 / 596	782 L	1.25D+1.5L
				2 - SPF 2	2.375"	77% 301 / 963	1265 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2688 ft-lb	7'5 3/8"	6250 ft-lb	0.430 (43%)	1.25D+1.5L	L
Shear	1240 lb	13'3 7/8"	2345 lb	0.529 (53%)	1.25D+1.5L	L
Perm Defl in.	0.060 (L/2623)	7' 3/8"	0.434 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.159 (L/984)	7' 3/8"	0.434 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.219 (L/715)	7' 3/8"	0.652 (L/240)	0.340 (34%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.060", Long Term = 0.089"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'4" o.c.

6 Bottom flange braced at bearings.

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December 17, 2018

Page 1 of 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-5-8	(Span)1-3-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-8-2	(Span)1-4-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	11-8-2 to 13-5-8	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	11-9-6		Near Face	96 lb	256 lb	0 lb	0 lb	F12

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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TW1218-081 Page 13 of 42

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Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018 Designer: RCO

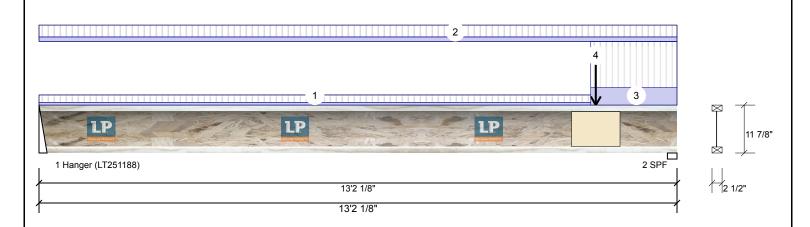
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Page 1 of 1

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Infor	mation		Unfactored Reactions UNPATTERNED lb (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	225		84	0	0	
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	589		221	0	0	
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored R	eactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total Ld. Case	Ld. Comb.	
				1 -	2.000"	28%	105 / 337	442 L	1.25D+1.5L	
				Hanger						
Analysis Resul	ts			2 - SPF	2.375"	71%	276 / 883	1159 L	1.25D+1.5L	

Analysis Actual Location Allowed Capacity Comb. Case Moment 0.290 (29%) 1.25D+1.5L L 1815 ft-lb 8'5 1/16" 6250 ft-lb Shear 1138 lb 13' 1/2" 2345 lb 0.485 (49%) 1.25D+1.5L L Perm Defl in. 0.039 (L/3941) 7' 15/16" 0.431 (L/360) 0.090 (9%) D Uniform LL Defl inch 0.105 (L/1477) 7' 15/16" 0.431 (L/360) 0.240 (24%) L TL Defl inch 0.145 (L/1074) 7' 15/16" 0.647 (L/240) 0.220 (22%) D+L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.039", Long Term = 0.059"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6. Top flange must be laterally braced at a maximum of 7'7" o c

o Top hange me	ist be laterally brace	a at a maximum or r	7 0.0.		I				
7 Bottom flange	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-4-12	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-2-2	(Span)0-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	11-4-12 to 13-2-2	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	11-6-0		Far Face	137 lb	366 lb	0 lb		rயிPraming Squash Block is at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com

CCMC: 12412-R APA: PR-L238C

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

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December 17, 2018





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Client: **GREEN YORK HOMES** Project:

Address:

Date: 12/14/2018

Designer: RCO

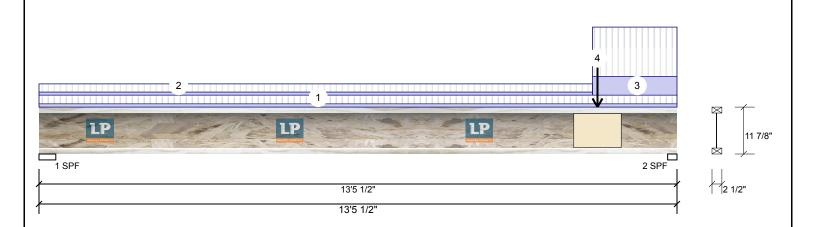
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Page 1 of 1

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation		Unfactored Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	198	74	0	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	558	209	0	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions			
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
				1-SPF 4	.250"	21% 93 / 297	389 L	1.25D+1.5L	
				2-SPF 2	375"	67% 261 / 837	1098 L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1648 ft-lb	9' 1/2"	6250 ft-lb	0.264 (26%)	1.25D+1.5L	L
Shear	1078 lb	13'3 7/8"	2345 lb	0.460 (46%)	1.25D+1.5L	L
Perm Defl in.	0.036 (L/4358)	7'4 1/2"	0.434 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.096 (L/1633)	7'4 1/2"	0.434 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.132 (L/1188)	7'4 1/2"	0.652 (L/240)	0.200 (20%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.036", Long Term = 0.054"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'11" o.c.

6 Bottom flange braced at bearings.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-5-8	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-8-2	(Span)0-6-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	11-8-2 to 13-5-8	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	11-9-6		Near Face	137 lb	366 lb	0 lb	0 lb	F12

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





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Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018 Designer: RCO

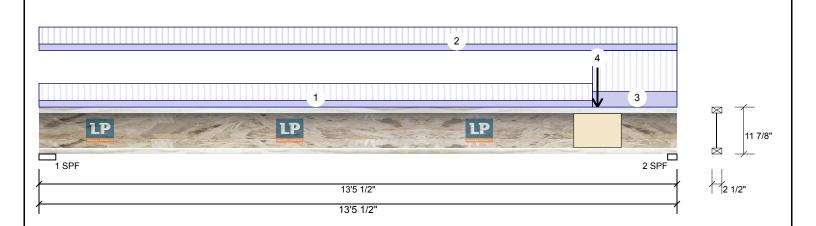
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Page 1 of 1

Project #:

11.875" - PASSED LPI 20Plus F13-D

Level: Ground Floor



Member Info	rmation			Unfactore	d Reaction	is UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	398	149	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	646	242	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Factor	ed Reactions		
Dead:	15 PSF			Bearing Le	ength (Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 4.	250"	43% 186 / 596	783 L	1.25D+1.5L
				2 - SPF 2.	375"	78% 303 / 968	1271 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2693 ft-lb	7'5 7/16"	6250 ft-lb	0.431 (43%)	1.25D+1.5L	L
Shear	1247 lb	13'3 7/8"	2345 lb	0.532 (53%)	1.25D+1.5L	L
Perm Defl in.	0.060 (L/2618)	7' 3/8"	0.434 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.159 (L/982)	7' 3/8"	0.434 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.219 (L/714)	7' 3/8"	0.652 (L/240)	0.340 (34%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.060", Long Term = 0.090"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'4" o.c.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-8-2	(Span)1-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-5-8	(Span)1-3-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	11-8-2 to 13-5-8	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	11-9-6		Far Face	97 lb	259 lb	0 lb	0 lb	F12

Pass-Thru Framing Squash Block is required at all point loads over bearings

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December 17, 2018

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





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Client: Project:

GREEN YORK HOMES

Address:

Date: 12/14/2018 RCO

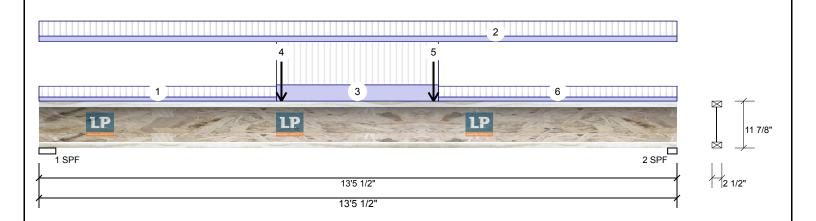
Designer:

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactored	d Reactio	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	288	108	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	281	105	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF			Bearing Le	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 4.	250"	31% 135 / 432	567 L	1.25D+1.5L
				2 - SPF 2.	375"	34% 131 / 422	554 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2362 ft-lb	6'9 13/16"	6250 ft-lb	0.378 (38%)	1.25D+1.5L	L
Shear	556 lb	3 1/2"	2345 lb	0.237 (24%)	1.25D+1.5L	L
Perm Defl in.	0.050 (L/3138)	6'9 5/8"	0.434 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.133 (L/1172)	6'9 5/8"	0.434 (L/360)	0.310 (31%)	L	L
TL Defl inch	0.183 (L/853)	6'9 5/8"	0.652 (L/240)	0.280 (28%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.050", Long Term = 0.075"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top flange must be laterally braced at a maximum of 6'9" o.c.

5 Bottom flange braced at bearings

o Bottom nango	Dracoa at Doaringo.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 5-0-2	(Span)0-4-14	Тор	15 PSF	40 PSF	0 PSF
2	Tie-In	0-0-0 to 13-5-8	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF
3	Tie-In	5-0-2 to 8-5-2	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF
4	Point	5-1-6		Far Face	42 lb	112 lb	0 lb
5	Point	8-3-14		Far Face	42 lb	114 lb	0 lb
6	Tie-In	8-5-2 to 13-5-8	(Snan)0-4-14	Ton	15 PSF	40 PSF	0 PSF

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Page 1 of 1

December 17, 2018

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF 0 PSF 0 PSF 0 lb F11

Comments





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Client: **GREEN YORK HOMES** Project:

Date: 12/14/2018 Designer: RCO

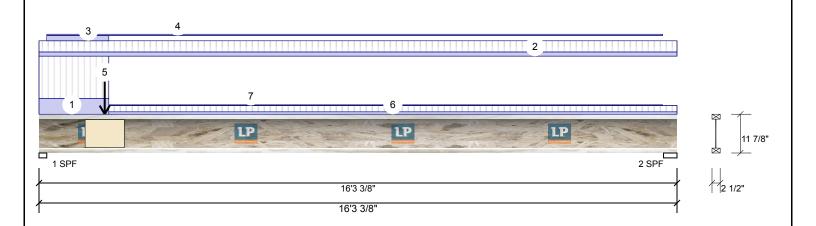
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Address:

Level: Ground Floor



Member Info	rmation			Unfactore	d Reacti	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	715	347	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	268	129	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	ored Reactions		
Dead:	15 PSF			Bearing Lo	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2.	.375"	92% 434 / 1072	1506 L	1.25D+1.5L
				2 - SPF 4.	.125"	31% 162 / 401	563 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2674 ft-lb	6'3"	6250 ft-lb	0.428 (43%)	1.25D+1.5L	L
Shear	1484 lb	1 5/8"	2345 lb	0.633 (63%)	1.25D+1.5L	L
Perm Defl in.	0.101 (L/1881)	7'6 7/8"	0.529 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.208 (L/917)	7'6 7/8"	0.529 (L/360)	0.390 (39%)	L	L
TL Defl inch	0.309 (L/616)	7'6 7/8"	0.793 (L/240)	0.390 (39%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.101", Long Term = 0.152"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'5" o.c.

6 Bottom flange braced at bearings.



Page 1 of 1

December 17, 2018

l	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
l	1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF
l	2	Tie-In	0-0-0 to 16-3-6	(Span)0-10-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF
l	3	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF
l	4	Part. Uniform	0-2-6 to 15-10-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF
l	5	Point	1-8-2		Far Face	219 lb	451 lb	0 lb	Pass Thru Framing Squash Block is required at all point loads over bearings
l	6	Tie-In	1-9-6 to 16-3-6	(Span)0-5-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF
	7	Part. Uniform	1-9-6 to 15-10-15		Тор	1 PLF	0 PLF		Referto Multiple Member Connection Detail for ply to ply nailing or bolting

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C

requirements





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Client: Project:

Address:

GREEN YORK HOMES

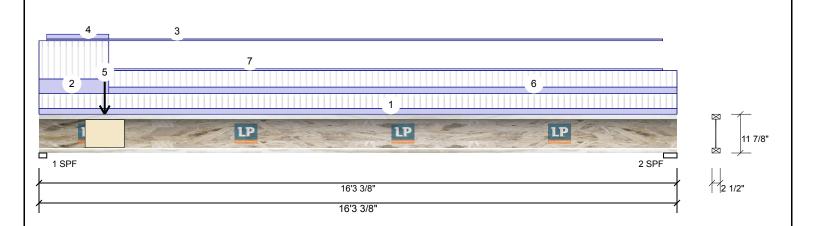
Date: 12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactore	d Reaction	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	765	372	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	471	227	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	.375"	99% 465 / 1147	1612 L	1.25D+1.5L
				2 - SPF 4	.125"	54% 284 / 706	990 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4104 ft-lb	7'5 1/2"	6250 ft-lb	0.657 (66%)	1.25D+1.5L	L
Shear	1588 lb	1 5/8"	2345 lb	0.677 (68%)	1.25D+1.5L	L
Perm Defl in.	0.155 (L/1227)	7'10 5/16"	0.529 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.318 (L/598)	7'10 5/16"	0.529 (L/360)	0.600 (60%)	L	L
TL Defl inch	0.473 (L/402)	7'10 5/16"	0.793 (L/240)	0.600 (60%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.155", Long Term = 0.233"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 5'1" o.c.

6 Bottom flange braced at bearings.



Page 1 of 1

December 17, 2018

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 16-3-6	(Span)1-3-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Part. Uniform	0-2-6 to 15-11-0		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
l	4	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
l	5	Point	1-8-2		Near Face	147 lb	302 lb	0 lb	Pass-Thr	મૃ ⊱raming Squash Block is at all point loads over bearings
l	6	Tie-In	1-9-6 to 16-3-6	(Span)1-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	at all pollit loads over bearings
	7	Part. Uniform	1-9-6 to 15-11-0		Тор	3 PLF	0 PLF			Multiple Member Connection ply to ply nailing or bolting

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C

requirements





TW1218-081 Page 19 of 42

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Client: Project:

Address:

GREEN YORK HOMES

Designer:

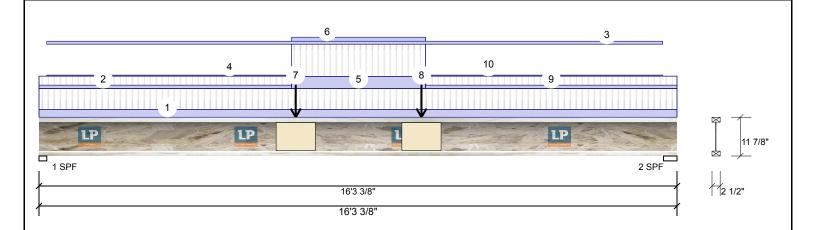
Date: 12/14/2018 RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactore	d Reacti	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	408	206	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	416	209	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	.375"	53% 257 / 612	869 L	1.25D+1.5L
				2-SPF 4	.125"	48% 261 / 624	885 L	1.25D+1.5L

Analysis Results

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	,				. ,		
	Moment	4338 ft-lb	8' 5/8"	6250 ft-lb	0.694 (69%)	1.25D+1.5L	L
	Shear	868 lb	16'	2345 lb	0.370 (37%)	1.25D+1.5L	L
	Perm Defl in.	0.159 (L/1194)	8' 7/8"	0.529 (L/360)	0.300 (30%)	D	Uniform
	LL Defl inch	0.316 (L/603)	8' 15/16"	0.529 (L/360)	0.600 (60%)	L	L
	TL Defl inch	0.475 (L/400)	8' 15/16"	0.793 (L/240)	0.600 (60%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.159", Long Term = 0.239"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 4'11" o.c.

6 Bottom flange braced at bearings.



Page 1 of 2

December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-3-6	(Span)1-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-5-6	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 15-10-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 6-5-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	6-5-6 to 9-10-6	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	6-5-6 to 9-10-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
7	Point	6-6-10		Near Face	65 lb	129 lb	0 lb	0 lb	F11

Notes

Continued on page 2...

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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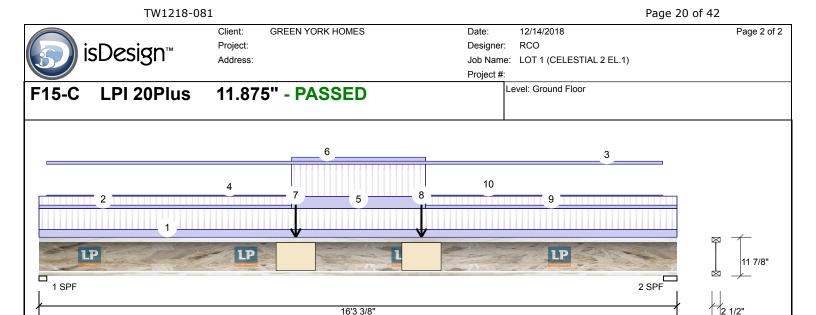
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Manufacturer Info

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Continued	from	page	1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	9-9-2		Near Face	63 lb	127 lb	0 lb	0 lb	F11
9	Tie-In	9-10-6 to 16-3-6	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	9-10-6 to 15-10-14		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

16'3 3/8'

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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TW1218-081 Page 21 of 42

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Client: GREEN YOR
Project:

Address:

GREEN YORK HOMES

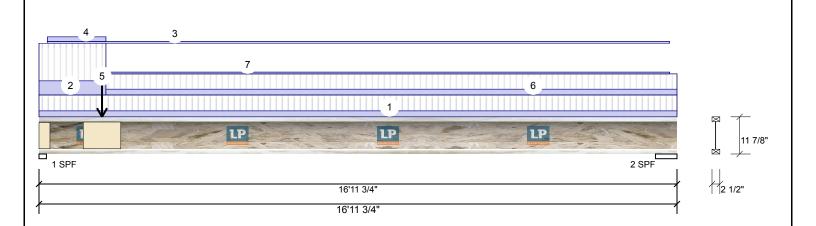
Date: 12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

F15-D LPI 20Plus 11.875" - PASSED

Level: Ground Floor



Member Info	mation			Unfactore	ed Reacti	ons UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	802	390	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	497	241	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	_ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	79% 488 / 1203	1690 L	1.25D+1.5L
				2-SPF 6	3.875"	57% 301 / 745	1046 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4358 ft-lb	7'8"	6250 ft-lb	0.697 (70%)	1.25D+1.5L	L
Shear	1666 lb	1 5/8"	2345 lb	0.710 (71%)	1.25D+1.5L	L
Perm Defl in.	0.174 (L/1129)	8'1 1/16"	0.544 (L/360)	0.320 (32%)	D	Uniform
LL Defl inch	0.356 (L/551)	8'1 1/16"	0.544 (L/360)	0.650 (65%)	L	L
TL Defl inch	0.530 (L/370)	8'1 1/16"	0.817 (L/240)	0.650 (65%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.174", Long Term = 0.260"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 4'10" o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 1.



Page 1 of 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 16-11-12	(Span)1-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Part. Uniform	0-2-10 to 16-9-6		Тор	3 PLF	0 PLF	0 PLF	0 PLF
4	Part. Uniform	0-2-12 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	Pa®8LTh
5	Point	1-8-2		Near Face	159 lb	325 lb	0 lb	required
6	Tie-In	1-9-6 to 16-11-12	(Span)1-3-11	Тор	15 PSF	40 PSF	0 PSF	Referto
7	Part. Uniform	1-9-6 to 16-9-6		Тор	3 PLF	0 PLF	0 PLF	Detailfo

Pa&BLFhru Framing Squash Block is equired ર્મુગ્રી point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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Client: Project: Address:

GREEN YORK HOMES

Date: 12/14/2018 Designer: RCO

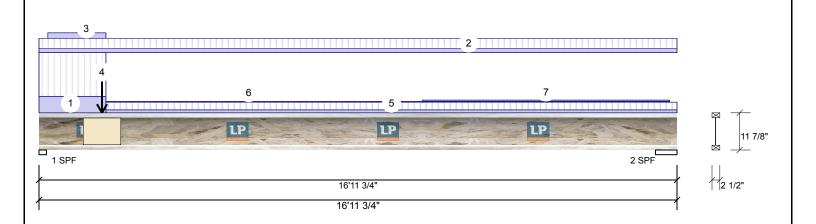
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Page 1 of 1

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Infor	mation			Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	719	335	0	0	
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	278	129	0	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions			
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
				1 - SPF 2	2.375"	91% 418 / 1078	1496 L	1.25D+1.5L	
				2-SPF 6	6.875"	32% 161 / 417	578 L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2709 ft-lb	6'6 1/4"	6250 ft-lb	0.433 (43%)	1.25D+1.5L	L
Shear	1475 lb	1 5/8"	2345 lb	0.629 (63%)	1.25D+1.5L	L
Perm Defl in.	0.103 (L/1907)	7'9 7/8"	0.544 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.227 (L/864)	7'9 7/8"	0.544 (L/360)	0.420 (42%)	L	L
TL Defl inch	0.330 (L/594)	7'9 7/8"	0.817 (L/240)	0.400 (40%)	D+L	L

10-2-6 to 16-9-6

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.103", Long Term = 0.154"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.

5 Top flange i	must be laterally brac	ced at a maximum of 6	'4" o.c.						
6 Bottom flan	ge braced at bearing	S.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 16-11-12	(Span)0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-13 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
4	Point	1-8-2		Far Face	218 lb	450 lb	0 lb	0 lb	F12
5	Tie-In	1-9-6 to 16-11-12	(Span)0-6-13	Тор	15 PSF	40 PSF	0 PSF	Pass Th	ru Framing Squash Block is I at all point loads over bearings
6	Part. Uniform	1-9-6 to 16-9-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	i at all pollit loads over bealings

2 PLF

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

requirements

Notes

7

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Part. Uniform

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Top

This design is valid until 10/31/2020

0 PLF

Manufacturer Info

0 PLF

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T.L. WISE 100083566

100083566

December 17, 2018





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Client: Project: Address:

GREEN YORK HOMES

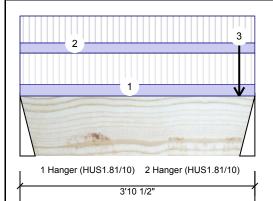
12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

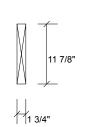
Project #

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor



3'10 1/2"



Page 1 of 1

Mem	hor	Info	rm-	tion

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift) Bra Dead

Brg	Live	Dead	Snow	Wind
1	62	33	0	0
2	580	266	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	133 ft-lb	2'3 13/16"	17130 ft-lb	0.008 (1%)	1.25D+1.5L	L
Unbraced	133 ft-lb	2'3 13/16"	12574 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	101 lb	2'8 3/8"	5798 lb	0.017 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/66968)	2'1 7/8"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/44165)	2'1 3/4"	0.175 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

ш								
Γ	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	3%	42 / 94	136	L	1.25D+1.5L	
	2 -	3.000"	64%	332 / 870	1202	L	1.25D+1.5L	
ı	Hanger							

Design Notes

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.

Location

3-7-6

0-0-0 to 3-10-8

0-0-0 to 3-10-8

Trib Width

(Span)0-8-9

(Span)0-7-7

Side

Тор

Top

Тор

- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings

Load Type

Self Weight

Tie-In

Tie-In

Point



0 PSF Pass-Thruc Framing Squash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

ID

2

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Dead

15 PSF

15 PSF

242 lb

5 PLF

Live

40 PSF

40 PSF

539 lb

Manufacturer Info Forex APA: PR-L318

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Snow

0 PSF

0 PSF

0 lb

Wind

0 PSF

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This design is va



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Client: Project: Address:

GREEN YORK HOMES

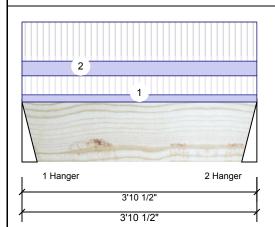
12/14/2018 Designer: RCO

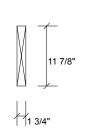
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Wind

0

0

1.25D+1.5L

0

0

238 L

Page 1 of 1

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

6%

52

52

115

115

3.000"

2

2 -

Hanger

L								
	Bearing:	s and Fa	actored I	Reactions				
Γ	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	6%	66 / 173	238	L	1.25D+1.5L	

66 / 173

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	188 ft-lb	1'11 1/4"	17130 ft-lb	0.011 (1%)	1.25D+1.5L	L
Unbraced	188 ft-lb	1'11 1/4"	12574 ft-lb	0.015 (1%)	1.25D+1.5L	L
Shear	93 lb	1'2 1/8"	5798 lb	0.016 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/45885)	1'11 1/4"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/31535)	1'11 1/4"	0.175 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-10-8	(Span)0-11-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-10-8		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

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This design



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Client: Project: Address:

GREEN YORK HOMES

Date: 12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Application:

Design Method:

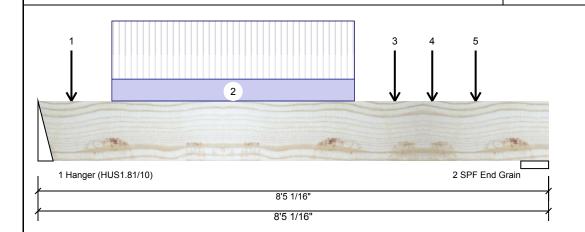
Building Code:

Load Sharing:

Deck:

Vibration:

Level: Ground Floor



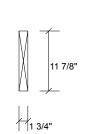
Floor (Residential)

Not Checked

Not Checked

No

NBCC 2010 / OBC 2012



Page 1 of 1

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load

Floor Live: 40 PSF 15 PSF Dead:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	404	181	0	0
2	704	324	0	0

Bearings and Factored Reactions

Grain

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 227 / 607 1.25D+1.5L 1 -3.000" 833 L Hanger 2 - SPF 5.500" 20% 405 / 1055 1460 L 1.25D+1.5L End

> Wind 0 lb 0 PLF 0 lb J1

> > 0 lb

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2193 ft-lb	5'10 5/8"	17130 ft-lb	0.128 (13%)	1.25D+1.5L	L
Unbraced	2193 ft-lb	5'10 5/8"	5736 ft-lb	0.382 (38%)	1.25D+1.5L	L
Shear	1452 lb	7' 7/16"	5798 lb	0.250 (25%)	1.25D+1.5L	L
Perm Defl in.	0.013 (L/7131)	4'5 9/16"	0.261 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.029 (L/3217)	4'5 7/16"	0.261 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.042 (L/2217)	4'5 1/2"	0.392 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Point	0-6-10		Far Face	30 lb	79 lb	0 lb
2	Part. Uniform	1-2-10 to 5-2-10		Far Face	29 PLF	77 PLF	0 PLF
3	Point	5-10-10		Far Face	28 lb	76 lb	0 lb
4	Point	6-6-0		Far Face	266 lb	580 lb	0 lb
5	Point	7-2-10		Far Face	25 lb	65 lb	0 lb
	Self Weight				5 PLF		



December 17, 2018

Pass-Thru 1 Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

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Client: **GREEN YORK HOMES** Project:

12/14/2018

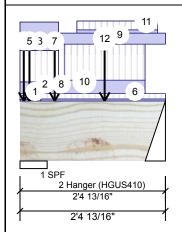
Designer: RCO

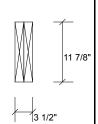
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

1.750" X 11.875" 2-Ply - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Wind

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Page 1 of 2

Member Info	rmation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

1207

159

2791

299

1 2

Bearing:	Bearings and Factored Reactions								
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - SPF	5.375"	52%	1509 / 4187	5695	L	1.25D+1.5L			
2 - Hanger	4.000"	6%	199 / 449	648	L	1.25D+1.5L			

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	375 ft-lb	1'4 3/4"	34261 ft-lb	0.011 (1%)	1.25D+1.5L	L
Unbraced	375 ft-lb	1'4 3/4"	34261 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	582 lb	1'4 1/2"	11596 lb	0.050 (5%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/35461)	1'4 3/4"	0.058 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/24245)	1'4 3/4"	0.087 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.

15 PSF

- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width.

ĺ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 0-4-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-0-0 to 0-7-10		Тор	33 PLF	88 PLF	0 PLF	0 PLF	J5
	3	Part. Uniform	0-0-0 to 0-7-10		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

TI WISE PR

100083566

December 17, 2018





TW1218-081 Page 27 of 42

isDesign™

Client: Project:

GREEN YORK HOMES

Date: 12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

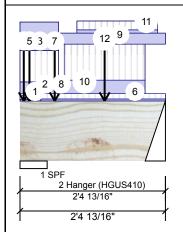
Project #:

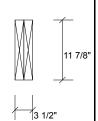
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Page 2 of 2

Continued fr	om page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Point	0-0-12		Far Face	55 lb	147 lb	0 lb	0 lb	J4
5	Point	0-1-14		Тор	901 lb	2190 lb	0 lb	0 lb	F9 F9
6	Tie-In	0-4-2 to 2-4-13	(Span)0-8-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	0-6-14		Far Face	84 lb	225 lb	0 lb	0 lb	F13
8	Part. Uniform	0-7-10 to 0-8-12		Тор	13 PLF	34 PLF	0 PLF	0 PLF	J5
9	Part. Uniform	0-7-10 to 2-4-13		Тор	31 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Part. Uniform	0-8-12 to 2-0-12		Тор	38 PLF	101 PLF	0 PLF	0 PLF	J5
11	Part. Uniform	0-11-4 to 2-3-4		Тор	9 PLF	25 PLF	0 PLF	0 PLF	J1
12	Point	1-4-12		Far Face	100 lb	267 lb	0 lb	0 lb	J5
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info





TW1218-081 Page 28 of 42

isDesign™

Client: Project:

GREEN YORK HOMES

12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

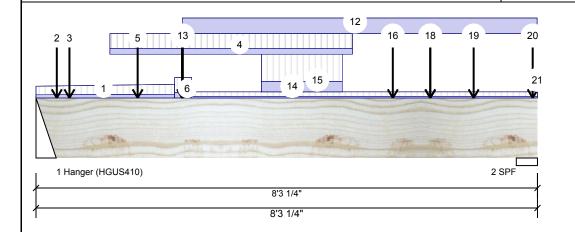
Project #:

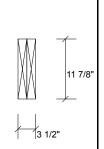
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Page 1 of 2

Member	Information
Type:	Girder
Plies:	2

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

General Load 40 PSF Floor Live: 15 PSF Dead:

Application: Floor (Residential)

Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1093	723	0	0
2	992	790	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.	
1 -	4.000"	24%	903 / 1640	2543	L	1.25D+1.5L	
Hanger							
2 - SPF	4.192"	27%	988 / 1487	2475	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4881 ft-lb	3'9 7/8"	34261 ft-lb	0.142 (14%)	1.25D+1.5L	L
Unbraced	4881 ft-lb	3'9 7/8"	31493 ft-lb	0.155 (15%)	1.25D+1.5L	L
Shear	2408 lb	1'3 1/8"	11596 lb	0.208 (21%)	1.25D+1.5L	L
Perm Defl in.	0.021 (L/4444)	4' 11/16"	0.257 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.028 (L/3322)	4' 1/8"	0.257 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.049 (L/1901)	4' 5/16"	0.386 (L/240)	0.130 (13%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.



December 17, 2018

1 Lateral Sicriae	incoo ratio basca on r	un scollon widin.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-3-7	(Span)2-2-7 to 2-6-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-4-2		Far Face	15 lb	39 lb	0 lb	0 lb	J1
3	Point	0-6-10		Near Face	30 lb	79 lb	0 lb	0 lb	J1
4	Part. Uniform	1-2-10 to 5-2-10		Near Face	29 PLF	77 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	17 lb	44 lb	0 lb	0 lb	J1

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is

TW1218-081 Page 29 of 42

isDesign™

Client: Project:

GREEN YORK HOMES

Date: 12/14/2018

Designer: RCO

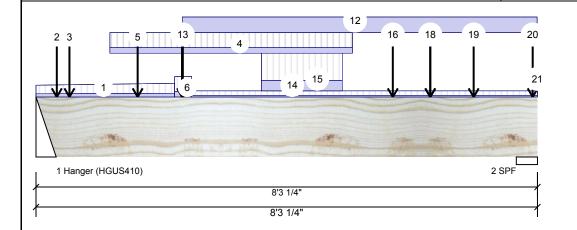
Job Name: LOT 1 (CELESTIAL 2 EL.1)

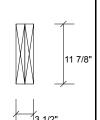
Project #:

Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Page 2 of 2

Continued from p	page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Tie-In	2-3-7 to 2-6-13	(Span)3-9-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	2-4-15		Тор	79 lb	211 lb	0 lb	0 lb	J5
8	Point	2-4-15		Тор	20 lb	52 lb	0 lb	0 lb	J1
9	Point	2-4-15		Тор	2 lb	5 lb	0 lb	0 lb	J5
10	Point	2-4-15		Тор	86 lb	0 lb	0 lb	0 lb	Wall Self Weight
11	Point	2-4-15		Тор	12 lb	0 lb	0 lb	0 lb	Wall Self Weight
12	Part. Uniform	2-4-15 to 8-3-3		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
13	Point	2-5-1		Far Face	159 lb	299 lb	0 lb	0 lb	F6
14	Tie-In	2-6-13 to 8-3-4	(Span)1-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
15	Part. Uniform	3-8-10 to 5-0-10		Тор	53 PLF	140 PLF	0 PLF	0 PLF	J3
16	Point	5-10-10		Near Face	28 lb	76 lb	0 lb	0 lb	J1
17	Point	6-6-0		Near Face	33 lb	62 lb	0 lb	0 lb	F2
18	Point	6-6-0		Тор	141 lb	322 lb	0 lb	0 lb	F10 F10
19	Point	7-2-10		Near Face	25 lb	65 lb	0 lb	0 lb	J1
20	Point	8-2-5		Near Face	34 lb	75 lb	0 lb	0 lb	F2
21	Part. Uniform	8-3-3 to 8-3-4		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Design assumes to be used

 Design assumes top edge is laterally restrained

 Forvoire lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info



TW1218-081 Page 30 of 42

isDesign™

Client: Project:

GREEN YORK HOMES

Date: 12/14/2018

Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

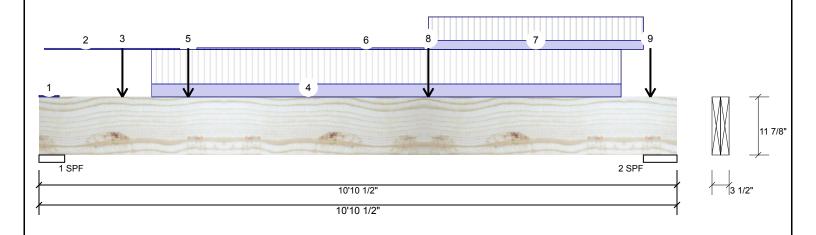
Page 1 of 2

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Member Info	rmation			Unfactore	d Reaction	D lb (Uplift)		
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	3015	1438	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	3156	1322	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Factor	ed Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 5	.250"	56% 1797 / 4523	6320 L	1.25D+1.5L
				2-SPF 6	.875"	43% 1652 / 4734	6386 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16147 ft-lb	5'6 1/8"	34261 ft-lb	0.471 (47%)	1.25D+1.5L	L
Unbraced	16147 ft-lb	5'6 1/8"	29618 ft-lb	0.545 (55%)	1.25D+1.5L	L
Shear	6696 lb	1'4 3/8"	11596 lb	0.577 (58%)	1.25D+1.5L	L
Perm Defl in	. 0.075 (L/1589)	5'3 11/16"	0.333 (L/360)	0.230 (23%)	D	Uniform
LL Defl inch	0.170 (L/704)	5'4 11/16"	0.333 (L/360)	0.510 (51%)	L	L
TL Defl inch	0.246 (L/488)	5'4 3/8"	0.499 (L/240)	0.490 (49%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

U Lateral Sieridei	ness ratio based on id	ii section width.							_
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-7-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 2-4-13	(Span)0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-1		Near Face	151 lb	402 lb	0 lb	0 lb	J8
4	Part. Uniform	1-11-1 to 9-11-1		Near Face	148 PLF	395 PLF	0 PLF	0 PLF	
5	Point	2-6-9		Far Face	723 lb	1093 lb	0 lb	0 lb	F7
6	Tie-In	2-8-5 to 6-6-12	(Span)0-8-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

TI WISE PR

100083566

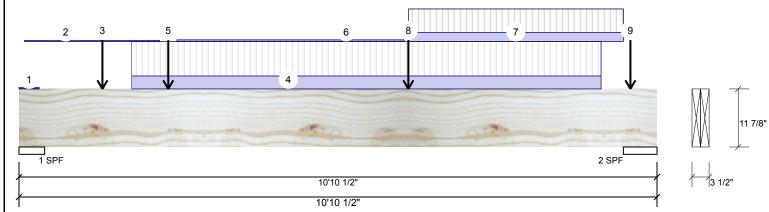
December 17, 2018





TW1218-081 Page 31 of 42

Client: **GREEN YORK HOMES** Date: 12/14/2018 Page 2 of 2 Project: Designer: RCO isDesign™ Address: Job Name: LOT 1 (CELESTIAL 2 EL.1) Project #: Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED Level: Ground Floor F8-B



.Continued from p	page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Part. Uniform	6-7-10 to 10-3-10		Тор	101 PLF	270 PLF	0 PLF	0 PLF	
8	Point	6-7-10		Far Face	181 lb	404 lb	0 lb	0 lb	F5
9	Point	10-5-1		Near Face	16 lb	42 lb	0 lb	0 lb	J8
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

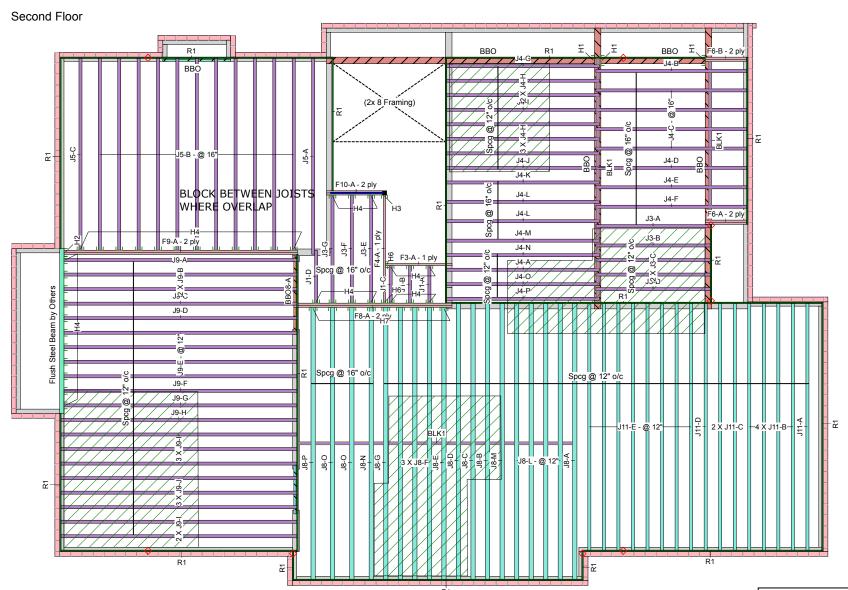
6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info APA: PR-L318



TW1218-081



This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings

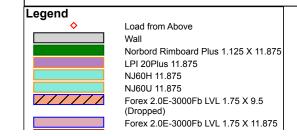
including anchorage of components and bracing for

lateral stability are the responsibility of others.



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.5; Aug.30,2018 Project No: 17-55 Model: Celestial 2



- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Second LVL/LS		·h)										
Label			Wic	lth	DΔ	pth	(Qty	Plies	Pcs	Length	$oldsymbol{\Lambda}$
F9	Forex	ption		75	_	875		بري 1	2	2	18-0-0	
13		000Fb LVL	١.	13	11.0	373		'		2	10-0-0	
F8	Forex	000Fb LVL	1.	75	11.8	875		1	2	2	12-0-0	Layout Nai
F4	Forex 2.0E-3	000Fb LVL	1.	75	11.8	875				1	8-0-0	Design Me
F10	Forex 2.0E-3	000Fb LVL	1.	75	11.8	875		1	2	2	6-0-0	Description
F3	Forex 2.0E-3	000Fb LVL	1.	75	11.8	875				1	6-0-0	GRANELLI BRAMPTOI
F6	Forex 2.0E-3	000Fb LVL	1.	75	11.8	875		2	2	4	4-0-0	Created May 30, 20
LVL/LS	L (Dro	pped)									•	
Label	Descri	ption	Wic	dth	De	pth	(Qty	Plies	Pcs	Length	Builder
BBO8	Forex 2.0E-3	000Fb LVL	1.	75		9.5		1	2	2	6-0-0	GREEN YO Sales Rep
I Joist (l						1	RM
	Descri	ption	Wic	dth	De	pth	(Qty	Plies	Pcs	Length	Designer
J9	LPI 20I	Plus	2	2.5	11.8	875				20	18-0-0	RCO
J5	LPI 20I	Plus	2	2.5	11.8	875				13	14-0-0	
J4	LPI 20I	Plus	2	2.5	11.8	875				24	12-0-0	Shipping
J3	LPI 20I	Plus	12	2.5	11.8	875				8	8-0-0	Project
J1	LPI 20I			2.5		875				4	4-0-0	Builder's P
J11	NJ60H			2.5	_	875				16	18-0-0	Kott Lur
J8	NJ60U		:	3.5	11.8	875				18	20-0-0	14 Anderso
Rim Bo					_						T	Stouffville, (
-	Descri		Wic		_	pth	(Qty	Plies	Pcs	Length	Canada
R1	Norbor Plus 1.	d Rimboard	1.1	25	11.8	875				19	12	L4A 7X4
	11.875	123 /										905-642-44
Blockin												
	γ	ption	Wic	dth	De	pth	(Qty	Plies	Pcs	Length	Job Path
BLK1	LPI 20			2.5	_	875		inFt		Varies	33-0-0	D:\Users\ro HOME\GRE
Hanger	-							Bea	am/Girdei		oported ember	\GRANELLI \LOT 1 (CE \LOT 1 (CE
Label	Pcs	Descriptio	n	S	kew	Slo	ре	fa	steners	fas	teners	0000
H1	3	Unknown										Second F
		Hanger										Design Meth
H2	1	HGUS410	10						46 16d	1	6 16d	Building Cod
H3	1	HUCQ1.81. SDS	/9-									Floor
H4	36	LT251188							10dx1 1/2		0dx1 1/2	Loads
H6	2	HUS1.81/1	0						10dx1 1/2		0 16d	Live
H7	9	LT351188						4 ′	10dx1 1/2	2 10	Odx1 1/2	Dead
NOTES:												Deflection .
		ify dimensio										LL Span L/
Doubl	e joist o	nly require fi	ller/ba	acke	r ply '	wher	sup	portir	ng			L Span L

- Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of
- 5. Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- 7. Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

		Page 32 of 42
		NASCOR
cs	Length	NASCUR
2	18-0-0	
2	12-0-0	Layout Name
2	12-0-0	LOT 1 (CELESTIAL 2 EL.1)
1	8-0-0	Design Method
		LSD
2	6-0-0	
		Description
1	6-0-0	GRANELLI HOME CORP. BRAMPTON, ONT.
4	4-0-0	Created
•		May 30, 2018
cs	Length	Builder
2	6-0-0	GREEN YORK HOMES
		Sales Rep
	I	RM
cs	Length	Designer
20	18-0-0	RCO
13 24	14-0-0	Shipping
24 8	12-0-0 8-0-0	Project
<u> </u>	4-0-0	
16	18-0-0	Builder's Project
18	20-0-0	Kott Lumber Company
		14 Anderson Blvd
cs	Length	Stouffville, Ontario
9	12	Canada
		L4A 7X4
		905-642-4400
	Longth	Job Path
cs ries	Length 33-0-0	D:\Users\rochavillo\WORK FROM
nes	33-0-0	HOME\GREEN YORK HOMES
Sur	ported	\GRANELLI HOME CORP\MODELS \LOT 1 (CELESTIAL 2 ELEV.1)\FLOO
	ember	\LOT 1 (CELESTIAL 2 EL.1).isl
	teners	
		Second Floor
		Design Method LSD
1	6 16d	Building Code NBCC 2010 / OBC
		2012
0.40)-l4 4/C	Floor
	0dx1 1/2 0 16d	Loads
	0 100 0dx1 1/2	Live 40

	2012
Floor	
Loads	
Live	40
Dead	15
Deflection Joist	
LL Span L/	360
TL Span L/	360
LL Cant 2L/	480
TL Cant 2L/	360
Deflection Girder	
LL Span L/	360
TL Span L/	240
LL Cant 2L/	480
TL Cant 2L/	360
Decking	
Deck	SPF Plywood
Thickness	5/8"
Fastener	Nailed & Glued
Vibration	

Gypsum 1/2"



Ceiling:

TW1218-081 Page 33 of 42

Brg

1

2

2 - SPF 4.000"

isDesign™

Client: Project: Address:

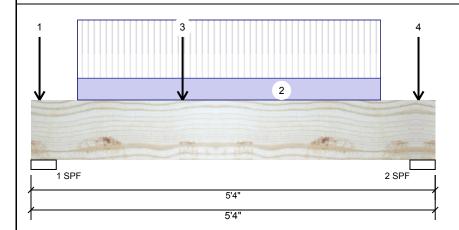
GREEN YORK HOMES

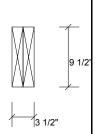
12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #

BBO8-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED Level: Second Floor





Wind

0

0

1.25D+1.5L

O

0

3194 L

Page 1 of 1

Memi	ber	Info	rma	atior

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condit	tion: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Deflection LL: Deflection TL: Importance: General Load	360 240 Normal	Load Sharing: Deck:	No Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

37%

Dead

939

645

Live

2317

1592

Bearings and Factored Reactions									
Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.						
1 SDE 4.000"	54% 1173 / 3476	4649 I	1 25D+1 5I						

806 / 2388

Wind

0 lb

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6963 ft-lb	2'	22724 ft-lb	0.306 (31%)	1.25D+1.5L	L
Unbraced	6963 ft-lb	2'	22724 ft-lb	0.306 (31%)	1.25D+1.5L	L
Shear	4094 lb	1' 3/4"	9277 lb	0.441 (44%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/4012)	2'3 9/16"	0.160 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.035 (L/1625)	2'3 9/16"	0.160 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.050 (L/1157)	2'3 9/16"	0.240 (L/240)	0.210 (21%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies.

15 PSF

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

PROFESSIONAL CHONNERS 100083566 WCE OF ONT

December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Point	0-1-6		Тор	50 lb	133 lb	0 lb
2	Part. Uniform	0-7-6 to 4-7-6		Тор	120 PLF	320 PLF	0 PLF
3	Point	2-0-0		Тор	975 lb	2394 lb	0 lb
4	Point	5-1-6		Тор	38 lb	102 lb	0 lb
	Self Weight				8 PLF		

0 PLF PassoThrupEraming Squash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is

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Client: Project: Address:

GREEN YORK HOMES

12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

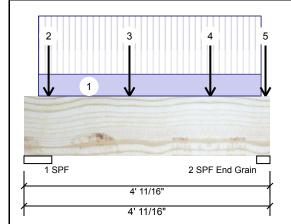
Project #:

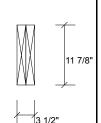
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor

Brg





Wind

Page 1 of 1

Member Information							
Type:	Girder	Application:	Floor (Residential)				
Plies:	2	Design Meth	od: LSD				
Moisture Conditio	n: Dry	Building Cod	le: NBCC 2010 / OBC 2012				
Deflection LL:	360	Load Sharing	g: No				
Deflection TL:	240	Deck:	Not Checked				
Importance:	Normal	Vibration:	Not Checked				
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						

Unfactored Reactions UNPATTERNED Ib (Uplift)

	322	141	U	U
2	539	242	0	0

Dead

Bearings and Factored Reactions

Live

Bearing Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 5.500"	6%	176 / 483	660	L	1.25D+1.5L
2 - SPF 2.632" End Grain	16%	302 / 808	1110	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	673 ft-lb	1'8 7/8"	34261 ft-lb	0.020 (2%)	1.25D+1.5L	L
Unbraced	673 ft-lb	1'8 7/8"	34261 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	530 lb	2'10 15/16"	11596 lb	0.046 (5%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/60821)	2' 1/2"	0.117 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/25507)	2' 1/4"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.002 (L/17971)	2' 3/8"	0.175 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral clanderness ratio based on full section width



December 17, 2018

ı	U Lateral Si	enderness ratio based							
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	,
	1	Part. Uniform	0-2-14 to 3-10-15		Тор	30 PLF	80 PLF	0 PLF	(
	2	Point	0-4-14		Near Face	3 lb	8 lb	0 lb	Doo
	3	Point	1-8-14		Near Face	74 lb	198 lb	0 lb	Pas requ
	4	Point	3-0-14		Near Face	63 lb	167 lb	0 lb	
	5	Point	3-11-13		Near Face	94 lb	194 lb	0 lb	Refe Deta
		Self Weight				10 PLF			real

0 lb J3 ss-Thru Framing Squash Block is quired at all point loads over bearings

Comments

0 lb J3 fer to Multiple Member Connection tail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



Wind

0 PLF

TW1218-081 Page 35 of 42



Client: Project:

Address:

GREEN YORK HOMES

12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

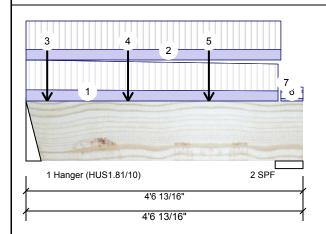
2 - SPF 5.500"

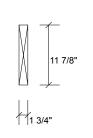
10%

156 / 454

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Wind 0 0

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

Page 1 of 2

Member Infor	rmation			Unfacto	red React	ions UN	NPATTERN	ED lb (Upli	ft)
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	
Plies:	1	Design Method:	LSD	1	351		142	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	303		125	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored R	eactions		
Dead:	15 PSF			Bearing	Length	Cap. I	React D/L lb	Total Ld. 0	ase
				1 - Hanger	3.000"	18%	178 / 527	705 L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	632 ft-lb	2'1 1/8"	17130 ft-lb	0.037 (4%)	1.25D+1.5L	L
Unbraced	632 ft-lb	2'1 1/8"	11283 ft-lb	0.056 (6%)	1.25D+1.5L	L
Shear	433 lb	1'2 1/8"	5798 lb	0.075 (7%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/32806)	2'1 13/16"	0.133 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/13295)	2'1 3/4"	0.133 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/9461)	2'1 3/4"	0.199 (L/240)	0.030 (3%)	D+L	L



610 L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

December 17, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-1-14	(Span)2-11-6 to 2-7-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 4-2-7	(Span)2-9-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-4-3		Near Face	17 lb	45 lb	0 lb	0 lb	J1
4	Point	1-8-3		Near Face	26 lb	69 lb	0 lb	0 lb	J1
5	Point	3-0-3		Near Face	24 lb	64 lb	0 lb	0 lb	J1
6	Tie-In	4-2-7 to 4-6-13	(Span)0-7-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Ianding & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design



TW1218-081 Page 36 of 42



Client: **GREEN YORK HOMES** Project:

Date: 12/14/2018

Designer: RCO

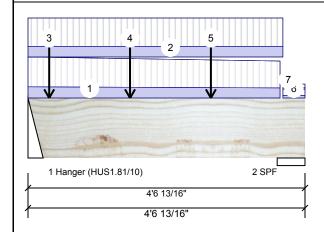
Job Name: LOT 1 (CELESTIAL 2 EL.1)

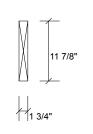
Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor





Page 2 of 2

.Continued from page 1

ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 7 Tie-In 4-2-7 to 4-6-13 (Span)0-4-5 Top 15 PSF 40 PSF 0 PSF 0 PSF

> Self Weight 5 PLF

> > Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Ianding & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info

TW1218-081 Page 37 of 42



Client: **GREEN YORK HOMES**

Project:

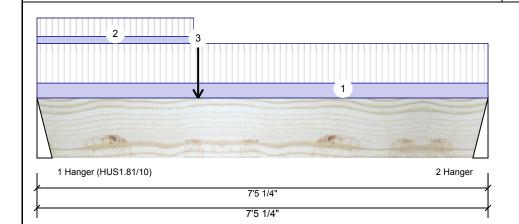
12/14/2018 Designer: RCO

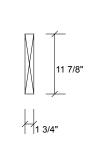
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Wind

0

O

Page 1 of 1

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

2	2	194	94	0	0

142

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1399 ft-lb	2'7 7/8"	17130 ft-lb	0.082 (8%)	1.25D+1.5L	L
Unbraced	1399 ft-lb	2'7 7/8"	6365 ft-lb	0.220 (22%)	1.25D+1.5L	L
Shear	581 lb	1'2 1/8"	5798 lb	0.100 (10%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/13727)	3'2 7/8"	0.235 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.014 (L/6157)	3'2 3/16"	0.235 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.020 (L/4251)	3'2 7/16"	0.353 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

314

1

L								
Γ	Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.	
l	1 -	3.000"	17%	178 / 472	650	L	1.25D+1.5L	
l	Hanger							
	2 -	3.000"	10%	118 / 291	408	L	1.25D+1.5L	
ı	Hanger							

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings



December 17, 2018

									_
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 7-5-4	(Span) 0-10-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 2-7-0	(Span)0-5-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	2-7-14		Near Face	142 lb	351 lb	0 lb	0 lb	F3
	Self Weight				5 PLF			Pass-Thr	u Framing S

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TW1218-081 Page 38 of 42

Brg

1 2

isDesign™

Client: Project:

GREEN YORK HOMES

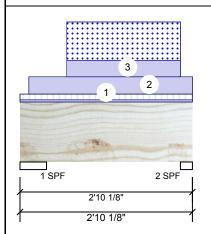
12/14/2018 Designer: RCO

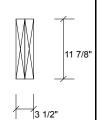
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Second Floor





Wind

+0.5L

0

0

Page 1 of 1

Member Infor	mation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		
		1	

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

165

172

Live

33

28

ı								
	Bearings							
	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - SPF	5.250"	4%	207 / 188	394	L	1.25D+1.5S +0.5L	
	2 SDE	2 375"	9%	214 / 242	457	1	1 25D+1 5S	

Snow

114

152

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	262 ft-lb	1'6 13/16"	33233 ft-lb	0.008 (1%)	1.25D+1.5S +0.5L	L
Unbraced	262 ft-lb	1'6 13/16"	33233 ft-lb	0.008 (1%)	1.25D+1.5S +0.5L	L
Shear	81 lb	1'4 3/8"	11248 lb	0.007 (1%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/38547)	1'6 3/4"	0.117 (L/240)	0.010 (1%)	D+S+0.5L	L



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

Self Weight

derness ratio based	on full section width.					
Load Type	Location	Trib Width	Side	Dead	Live	Snow
Tie-In	0-0-0 to 2-10-2	(Span)1-0-13	Тор	15 PSF	40 PSF	0 PSF
Part. Uniform	0-1-12 to 2-10-2		Тор	64 PLF	0 PLF	0 PLF
Part. Uniform	0-9-4 to 2-7-12		Тор	61 PLF	0 PLF	142 PLF
	Load Type Tie-In Part. Uniform	Load Type Location Tie-In 0-0-0 to 2-10-2 Part. Uniform 0-1-12 to 2-10-2	Load Type Location Trib Width Tie-In 0-0-0 to 2-10-2 (Span)1-0-13 Part. Uniform 0-1-12 to 2-10-2	Tie-In 0-0-0 to 2-10-2 (Span)1-0-13 Top Part. Uniform 0-1-12 to 2-10-2 Top	Load Type Location Trib Width Side Dead Tie-In 0-0-0 to 2-10-2 (Span)1-0-13 Top 15 PSF Part. Uniform 0-1-12 to 2-10-2 Top 64 PLF	Load Type Location Trib Width Side Dead Live Tie-In 0-0-0 to 2-10-2 (Span)1-0-13 Top 15 PSF 40 PSF Part. Uniform 0-1-12 to 2-10-2 Top 64 PLF 0 PLF



Comments

required at all point loads over bearings **Refer to Multiple Member Connection**

Passp Thru V Fata Stelling e Stoptuash Block is

Detail for ply to ply nailing or bolting requirements

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

10 PLF

Manufacturer Info

APA: PR-L318

Wind 0 PSF

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is



TW1218-081 Page 39 of 42



Client: **GREEN YORK HOMES** Project:

Date: 12/14/2018

Designer: RCO

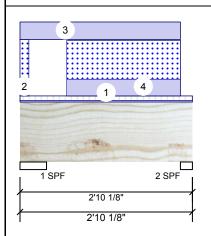
Job Name: LOT 1 (CELESTIAL 2 EL.1)

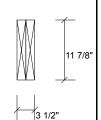
Project #:

2 - SPF 2.375"

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Second Floor





Wind 0 0

Ld. Comb. 1.25D+1.5S +0.5L

1.25D+1.5S

+0.5L

Page 1 of 1

Member Infor	mation			Unfacto	red React	tions L	JNPATTERN	ED lb (U	plift)
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	
Plies:	2	Design Method:	LSD	1	25		181	135	
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	21		156	152	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total L	d. Case
				1 - SPF	5.250"	4%	226 / 215	441 L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	258 ft-lb	1'6 13/16"	33575 ft-lb	0.008 (1%)	1.25D+1.5S +0.5L	L
Unbraced	258 ft-lb	1'6 13/16"	33575 ft-lb	0.008 (1%)	1.25D+1.5S +0.5L	L
Shear	80 lb	1'4 3/8"	11364 lb	0.007 (1%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/39192)	1'6 3/4"	0.117 (L/240)	0.010 (1%)	D+S+0.5L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral slenderness ratio based on full section width

U Lateral Sieriue	iness ratio based on it	ili sectioni widin.					
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 2-10-2	(Span)0-9-11	Тор	15 PSF	40 PSF	0 PSF
2	Part. Uniform	0-0-0 to 0-1-12		Тор	62 PLF	0 PLF	144 PLF
3	Part. Uniform	0-0-0 to 2-7-12		Тор	64 PLF	0 PLF	0 PLF
4	Part. Uniform	0-9-4 to 2-7-12		Тор	61 PLF	0 PLF	142 PLF
	Self Weight				10 PLF		

NCE OF ON December 17, 2018

T.L. WISE 100083566

100083566

PassPThru Framing Squash Block is required at all point loads over bearings

Comments

Wind 0 PSF

195 / 239

434 L

Referto Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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isDesign™

Client: Project:

GREEN YORK HOMES

12/14/2018 Designer: RCO

Job Name: LOT 1 (CELESTIAL 2 EL.1)

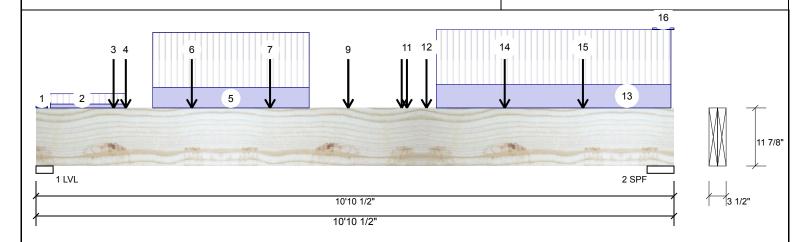
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



Member Info	rmation			Unfactor	ed Reacti	ions UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	2394	975	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	2576	1100	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - LVL	3.500"	53% 1219 / 3591	4810 L	1.25D+1.5L
				2 - SPF	5.500"	44% 1375 / 3864	5239 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13922 ft-lb	5'3 7/8"	34261 ft-lb	0.406 (41%)	1.25D+1.5L	L
Unbraced	13922 ft-lb	5'3 7/8"	29373 ft-lb	0.474 (47%)	1.25D+1.5L	L
Shear	5403 lb	1'2 5/8"	11596 lb	0.466 (47%)	1.25D+1.5L	L
Perm Defl in	. 0.062 (L/1983)	5'4 1/8"	0.342 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.150 (L/820)	5'3 7/8"	0.342 (L/360)	0.440 (44%)	L	L
TL Defl inch	0.212 (L/580)	5'3 7/8"	0.512 (L/240)	0.410 (41%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

o Lateral Sicridei	ness ratio basea on	idii 300tion widii.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 0-2-6	(Span) 0-10-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-2-15 to 1-6-6	(Span)3-5-15 to 3-6-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-3-14		Near Face	169 lb	451 lb	0 lb	0 lb	J8	
4	Point	1-6-6		Far Face	31 lb	83 lb	0 lb	0 lb	J1	
5	Part. Uniform	1-11-14 to 4-7-14		Near Face	140 PLF	373 PLF	0 PLF	0 PLF		

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE IN THE DESIGN OF THIS COMPONENT.

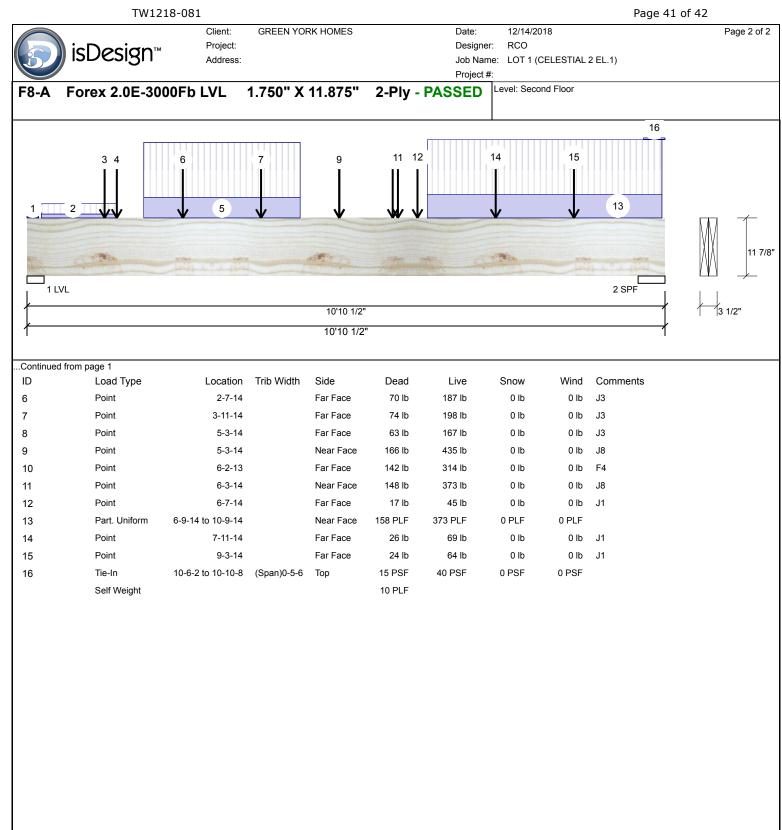


Page 1 of 2

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes
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- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 7/10/2021



TW1218-081 Page 42 of 42

isDesign™

Client: Project:

Address:

GREEN YORK HOMES

Date: 12/14/2018

Designer: RCO

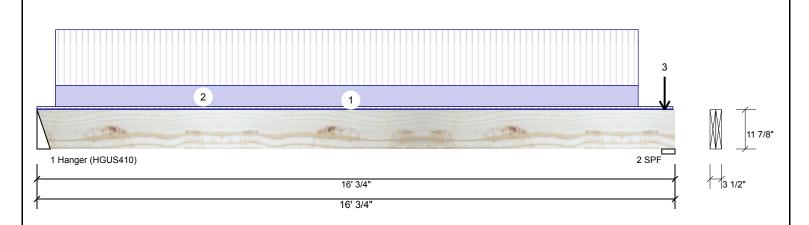
Job Name: LOT 1 (CELESTIAL 2 EL.1)

Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Second Floor



Member Inform	mation			Unfacto	red React	tions UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	2070	856	0	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	2190	901	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fac	tored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 -	4.000"	40% 1070 / 3106	4175 L	1.25D+1.5L
		ļ		Hanger				
Analysis Result				2 - SPF	4.000"	51% 1126 / 3285	4411 L	1.25D+1.5L

Analysis Actual Location Allowed Capacity Comb. Case Moment 8' 1/4" 34261 ft-lb 0.482 (48%) 1.25D+1.5L L 16529 ft-lb Unbraced 16529 ft-lb 8' 1/4" 23053 ft-lb 0.717 (72%) 1.25D+1.5L L 4147 lb 14'9 5/8" 11596 lb 0.358 (36%) 1.25D+1.5L L Shear Perm Defl in. 0.159 (L/1171) 8' 5/16" 0.517 (L/360) 0.310 (31%) D Uniform LL Defl inch 0.387 (L/482) 8' 5/16" 0.517 (L/360) 0.750 (75%) L L TL Defl inch 0.546 (L/341) 8' 5/16" 0.776 (L/240) 0.700 (70%) D+L L



Page 1 of 1

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

ID Load Type Location Trib Width Side Dead Live Snow 0-0-0 to 16-0-2 15 PSF 40 PSF 0 PSF 1 Tie-In (Span)0-6-5 Top 0 PLF Part. Uniform 0-5-10 to 15-1-10 Far Face 99 PLF 263 PLF 2

Point 15-9-10 Far Face 88 lb Self Weight 10 PLF

Pass-Thru Framing Squash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

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 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

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6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Wind

0 PSF

0 PLF

Forex APA: PR-L318

0 lb

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234 lb