Engineering Note Page (ENP-2)

REVISION 2018-10-17

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

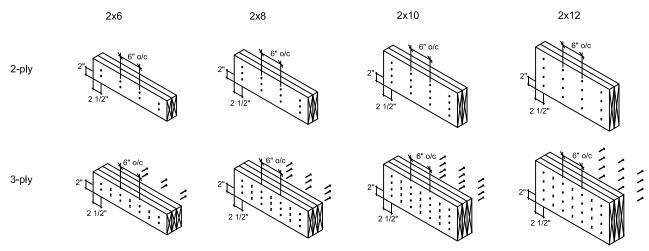
- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



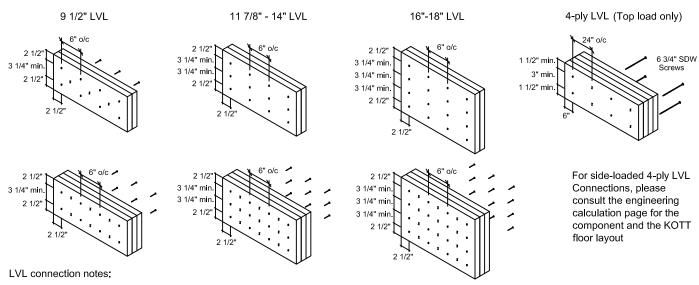
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections



- -LVL ply width is 1-3/4"
- -Nails to be 3 1/2" common wire nails.
- -Nails to be located 2 1/2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

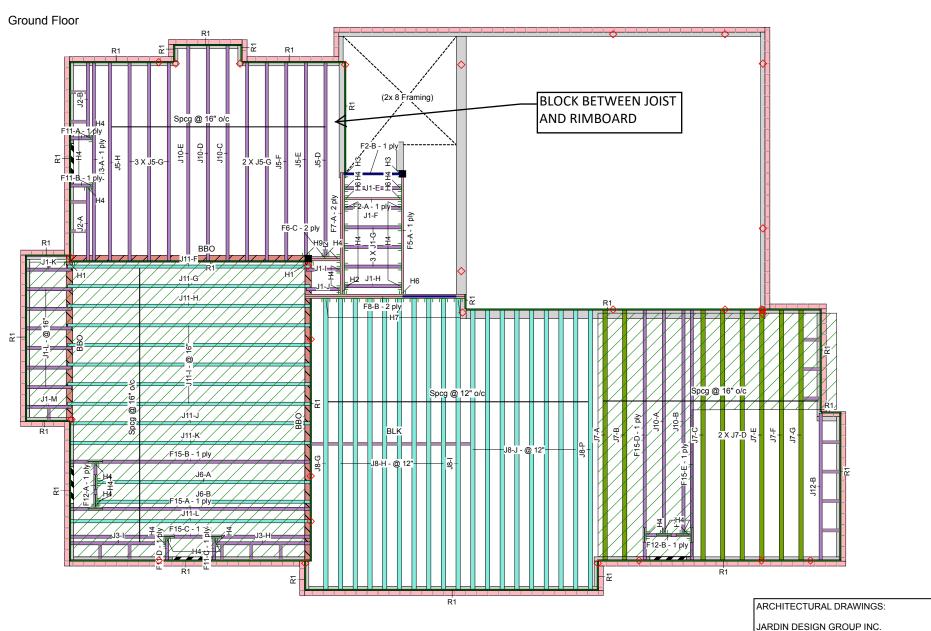
Multiple Member Connections

All connections are for uniformly distributed loads.

For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



This certification is to confirm that:

- 1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
- 2. The floor joists comply with the KOTT span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the KOTT Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.5; Aug.30,2018 Project No: 17-55 Model: Celestial 2

Legend



Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ40U 11.875 NJ60H 11.875 NJ60U 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

	Floor											
	L (Flus						. 1					_
	Descri	ption	Width	_	pth	Q	,	Plies	Pcs	3	Length	_
F8	Forex 2.0E-3	000Fb LVL	1.75	11.8	375	1	1	2	2		12-0-0	
F7	Forex 2.0E-3	000Fb LVL	1.75	11.8	375	1	1	2	2		10-0-0	Layout Name LOT 10 (CELESTIAL 2 2)
F5	Forex	000Fb LVL	1.75	11.8	375				1		10-0-0	Design Method
F2	Forex	000Fb LVL	1.75	11.8	375				2		4-0-0	LSD Revised
F6	Forex	000Fb LVL	1.75	11.8	375	1	1	2	2		4-0-0	December 14, 2018
loist (Flush)			1								 Description
	Descri		Width	De	pth	Q	tv	Plies	Pcs		Length	GRANELLI HOME CORP.
F15	LPI 201		2.5	_	375	Q	Ly	1 1103	5	_	18-0-0	BRAMPTON, ONT.
F13	LPI 201		2.5		375				1		14-0-0	- Builder
F12	LPI 201		2.5	_	375				2		4-0-0	II GREEN YORK HOMES
F11	LPI 201		2.5	-	375				4		2-0-0	
J10	LPI 201		2.5		375				5		16-0-0	_ '
J5	LPI 201		2.5	11.8					9		14-0-0	
J12	LPI 201		2.5	_	375				1		10-0-0	Designer
J3	LPI 201		2.5	11.8					2	-	8-0-0	1100
J2	LPI 201		2.5		375				2		6-0-0	-II Shinning
J1	LPI 201		2.5	_	375				16		4-0-0	- D:4
J7	NJ40U	lus	3.5		375				8		18-0-0	
J11	NJ60H		2.5	11.6					11		18-0-0	
J6	NJ60H		2.5	_	375				2	-	16-0-0	
J8	NJ60U		3.5	_	375				19		20-0-0	II 14 Anderson Blvd
			3.3	11.0	5/5				19		20-0-0	Stouffville, Ontario
im Bo			\ A /: - 4 -	Г Б-	-41-		4	Dii	D		1 41-	Canada
Label	Descri		Width	_	pth	Q	ty	Plies	Pcs		Length	- II
R1	Norbor	d Rimboard	1.125	11.8	375				17		12	905-642-4400
	11.875	120 A										
lockin				1								Job Path
Label	Descri	ntion	Width	Πρ	pth	Q	tv	Plies	Pcs		Length	 D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES
BLK1	LPI 20	•	2.5	_	375	Lin	,	1 1100	Varie		29-0-0	\GRANELLI HOME CORP\MODE
langer		1 103	2.5	1 11.0	373	LIII			vario	,3	23-0-0	\LOT 10 (CELESTIAL 2 ELEV.2)
angei							Bea	m/Girde	r S	au	ported	\FLOOR\LOT 10 (CELESTIAL 2 2
											mber	Ground Floor
Label	Pcs	Descriptio	n S	kew	Slop	oe l	fa	steners	f	ast	teners	Design Method LS
H1	2	Unknown Hanger										Building Code NBCC 2010 / OB 201
H2	2	HGUS410						46 16d		16	3 16d	Floor
НЗ	2	HUCQ1.81	/9-									Loads
114	0.4	SDS				_		10.1.4.40	+_	40	1 4 4 10	_ Live 4
H4	31	LT251188	_			_		0dx1 1/2	2		dx1 1/2	_ Dead 1
H6	3	HUS1.81/1	U			4		30 16d	+-		16d	Deflection Joist
H7	10	LT351188				+	4 1	0dx1 1/2	 2	10	dx1 1/2	LL Span L/ 48
H9	1	LT251188										TL Span L/ 36
OTES:											- 1	LL Cant 2L/ 48
Frame	er to vor	ify dimensio	ne on tho	arch	itactur	al dr	awin	ae			- 1	TL Cant 2L/ 36
		nly require fi										Deflection Girder

- another member using a face-mounted hanger.
- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- 5. Refer to Nascor specifier guide for installation works. 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- '. Load transfer blocks to be installed under all point loads. 8. It shall be the framer's responsibility that floor joists and beams are

fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

erson Blvd ille, Ontario 2-4400 rs\rochavillo\WORK FROM GREEN YORK HOMES ELLI HOME CORP\MODELS (CELESTIAL 2 ELEV.2) R\LOT 10 (CELESTIAL 2 2).is d Floor Method Code NBCC 2010 / OBC 2012 on Joist 480 L/ L/ 360 2L/ 480 360 2L/ Deflection Girder LL Span L/ 360 240 TL Span L/ LL Cant 2L/ 480 TL Cant 2L/ 360 Decking OSB Deck Thickness 3/4" Fastener Nailed & Glued Vibration KOTT



Client:

Project: Address:

GREEN YORK HOMES

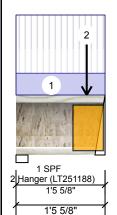
12/14/2018 Date: Designer: RCO

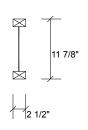
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor





Member In	formation
Type:	Girder
Plies:	1

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

General Load 40 PSF Floor Live: 15 PSF Dead:

Application: Floor (Residential)

Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	61	23	0	0
2	114	42	0	0

Bearings and Factored Reactions

Bearing Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.375"	7%	29 / 92	120	L	1.25D+1.5L
2 - 2.000"	14%	53 / 171	224	L	1.25D+1.5L
Hanger					

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	42 ft-lb	11 7/16"	6250 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	211 lb	1'4 3/8"	2345 lb	0.090 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/36074)	11 5/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/26279)	11 5/16"	0.061 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

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December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-0		Far Face	30 lb	81 lb	0 lb	0 lb	J2

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: **GREEN YORK HOMES**

Project:

Address:

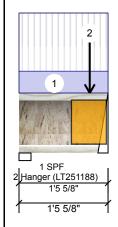
Date: 12/14/2018 Designer: RCO

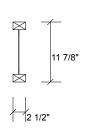
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor





Wind

0

0

0

0

Member Infori	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

23

42

61

112

1

2

Bearings	Bearings and Factored Reactions										
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	2.375"	7%	28 / 91	119	L	1.25D+1.5L					
2 - Hanger	2.000"	14%	52 / 167	219	L	1.25D+1.5L					

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	41 ft-lb	11 5/16"	6250 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	206 lb	1'4 3/8"	2345 lb	0.088 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/36734)	11 3/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/26745)	11 3/16"	0.061 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

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December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-0		Near Face	29 lb	78 lb	0 lb	0 lb	J2

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Project: Address:

GREEN YORK HOMES 12/14/2018 Date:

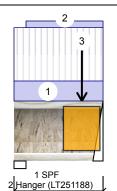
Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Level: Ground Floor

Project #:

11.875" - PASSED LPI 20Plus



11 7/8"

1'5 5/8' 1'5 5/8"

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked Vibration: Not Checked

15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	vvina	
1	68	32	0	0	
2	127	63	0	0	

Bearings and Factored Reactions

Bearing Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.375"	9%	41 / 102	143	L	1.25D+1.5L
2 - 2.000"	17%	79 / 191	269	L	1.25D+1.5L
Hanger					

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	59 ft-lb	1' 3/4"	6250 ft-lb	0.009 (1%)	1.25D+1.5L	L
Shear	255 lb	1'4 3/8"	2345 lb	0.109 (11%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/56853)	1' 3/8"	0.041 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/28228)	1' 7/16"	0.041 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/18862)	1' 3/8"	0.061 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-5-10		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-1-7		Near Face	50 lb	101 lb	0 lb	0 lb	J3

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: GREEN YORK HOMES

Project: Address: ORK HOMES Date: 12/14/2018

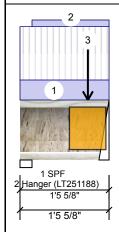
Designer: RCO

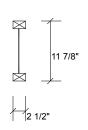
Designer: RCO
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

F11-D LPI 20Plus 11.875" - PASSED

Level: Ground Floor





Member Information						
Type:	Girder					
Plies:	1					

Plies: 1
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 240
Importance: Normal
General Load

40 PSF 15 PSF

Application: Floor (Residential) Design Method: LSD Building Code: NBCC 2010 / OBC 2012

Load Sharing: No Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	69	33	0	0
2	129	65	0	0

Bearings and Factored Reactions

Bearing Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 2.375"	9%	41 / 103	144	L	1.25D+1.5L
2 - 2.000"	17%	81 / 194	275	L	1.25D+1.5L
Hanger					

Analysis Results

Floor Live:

Dead:

-							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	60 ft-lb	1' 7/8"	6250 ft-lb	0.010 (1%)	1.25D+1.5L	L
	Shear	261 lb	1'4 3/8"	2345 lb	0.111 (11%)	1.25D+1.5L	L
	Perm Defl in.	0.000 (L/55408)	1' 1/2"	0.041 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.001 (L/27690)	1' 1/2"	0.041 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/18463)	1' 1/2"	0.061 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-10	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-5-10		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-1-7		Far Face	52 lb	104 lb	0 lb	0 lb	J3

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Notes

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.locorp.com

www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: **GREEN YORK HOMES**

Project:

Address:

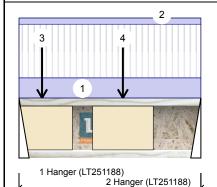
12/14/2018 Date: Designer: RCO

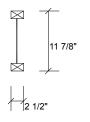
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus







1	3'	

Member Inform	ation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

	Unfactored Reactions UNPATTERNED lb (Uplift)										
	Brg	Live	Dead	Snow	Wind						
	1	451	219	0	0						
12	2	302	147	0	0						

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity 1'8 9/16" 6250 ft-lb 0.111 (11%) 1.25D+1.5L L Moment 693 ft-lb Shear 943 lb 1 1/4" 2345 lb 0.402 (40%) 1.25D+1.5L L Perm Defl in. 0.004 (L/9546) 1'8 9/16" 0.093 (L/360) 0.040 (4%) D Uniform LL Defl inch 0.007 (L/4632) 1'8 9/16" 0.093 (L/360) 0.080 (8%) L TL Defl inch 0.011 (L/3119) 1'8 9/16" 0.140 (L/240) 0.080 (8%) D+L L

Bearings and Factored Reactions

Bearing Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - 2.000"	60%	274 / 677	950	L	1.25D+1.5L	
Hanger						
2 - 2.000"	40%	184 / 453	637	L	1.25D+1.5L	
Hanger						

Design Notes

Dead:

1 Provide restraint at supports to ensure lateral stability.

15 PSF

- 2 Dead Load Deflection: Instant = 0.004", Long Term = 0.005"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-4-9		Near Face	126 lb	260 lb	0 lb	0 lb	J6
4	Point	1-8-9		Near Face	189 lb	390 lb	0 lb	0 lb	J6

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com

CCMC: 12412-R APA: PR-L238C







Client:

Project: Address: **GREEN YORK HOMES**

12/14/2018 Date: Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Level: Ground Floor

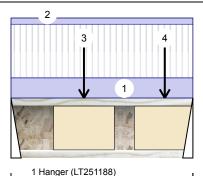
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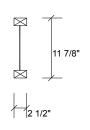
Brg

1 2

Hanger

11.875" - PASSED LPI 20Plus





Wind

0

0

0

0

	<u> </u>	2 Hanger (LT251188)	
1 1		3'	7
│ ∤ ─		3'	_

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Bearing	s and Fa	ctored R	eactions				
Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	43%	198 / 487	686	L	1.25D+1.5L	
2 -	2.000"	60%	273 / 676	948	L	1.25D+1.5L	

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

159

218

Live

325

450

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	701 ft-lb	1'2 7/16"	6250 ft-lb	0.112 (11%)	1.25D+1.5L	L
Shear	941 lb	2'10 3/4"	2345 lb	0.401 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/9387)	1'2 7/16"	0.093 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.007 (L/4578)	1'2 7/16"	0.093 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.011 (L/3077)	1'2 7/16"	0.140 (L/240)	0.080 (8%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.004", Long Term = 0.005"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.

7 Bottom flange braced at bearings.



ents

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Commen
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-2-7		Far Face	191 lb	391 lb	0 lb	0 lb	J10
4	Point	2-6-7		Far Face	135 lb	281 lb	0 lb	0 lb	J10

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: **GREEN YORK HOMES**

Project:

Address:

Date: 12/14/2018

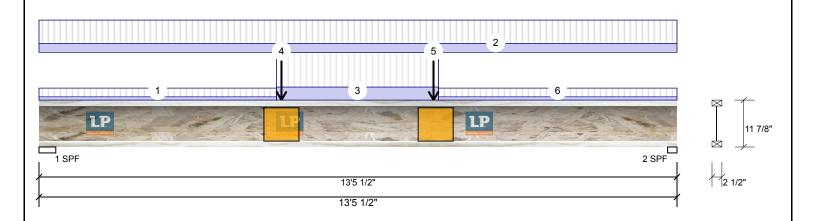
Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactore	d Reacti	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	361	135	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	352	132	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 4.	.250"	39% 169 / 541	709 L	1.25D+1.5L
				2 - SPF 2	.375"	42% 164 / 528	693 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2806 ft-lb	6'9 13/16"	6250 ft-lb	0.449 (45%)	1.25D+1.5L	L
Shear	692 lb	3 1/2"	2345 lb	0.295 (30%)	1.25D+1.5L	L
Perm Defl in.	0.060 (L/2628)	6'9 5/8"	0.434 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.159 (L/982)	6'9 11/16"	0.434 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.219 (L/715)	6'9 11/16"	0.652 (L/240)	0.340 (34%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.060", Long Term = 0.089"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top flange must be laterally braced at a maximum of 6'3" o.c.

5 Bottom flange braced at bearings

3 Bollom hange	braceu at bearings.								De
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	
1	Tie-In	0-0-0 to 5-0-2	(Span)0-4-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-5-8	(Span)1-1-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	5-0-2 to 8-5-2	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	5-1-6		Far Face	42 lb	112 lb	0 lb	0 lb	F11
5	Point	8-3-14		Far Face	42 lb	114 lb	0 lb	0 lb	F11
6	Tie-In	8-5-2 to 13-5-8	(Span)0-4-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

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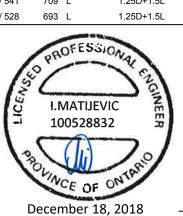
Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C









5

1

1 SPF

Client:

Project: Address:

GREEN YORK HOMES

Date: 12/14/2018 Designer:

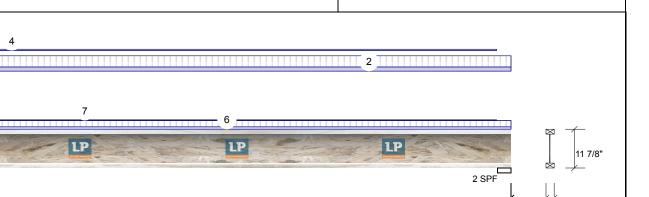
RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Level: Ground Floor

Project #:

11.875" - PASSED LPI 20Plus



Member Infor	mation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	1	Design Method:	LSD	1	715	347	0	0		
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	268	129	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Facto	ored Reactions				
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF 2	2.375"	92% 434 / 1072	1506 L	1.25D+1.5L		
				2 - SPF 4	4.125"	31% 162 / 401	563 L	1.25D+1.5L		

16'3 3/8' 16'3 3/8'

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2674 ft-lb	6'3"	6250 ft-lb	0.428 (43%)	1.25D+1.5L	L
Shear	1484 lb	1 5/8"	2345 lb	0.633 (63%)	1.25D+1.5L	L
Perm Defl in.	0.101 (L/1881)	7'6 7/8"	0.529 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.208 (L/917)	7'6 7/8"	0.529 (L/360)	0.390 (39%)	L	L
TL Defl inch	0.309 (L/616)	7'6 7/8"	0.793 (L/240)	0.390 (39%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.101", Long Term = 0.152"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'5" o.c. 6 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

L										
ſ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 16-3-6	(Span)0-10-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
l	4	Part. Uniform	0-2-6 to 15-10-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
l	5	Point	1-8-2		Far Face	219 lb	451 lb	0 lb	0 lb	F12
l	6	Tie-In	1-9-6 to 16-3-6	(Span)0-5-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	7	Part. Uniform	1-9-6 to 15-10-15		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

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Pass-Thru Framing Squash Block is required at all point loads over bearings

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: **GREEN YORK HOMES**

Project: Address:

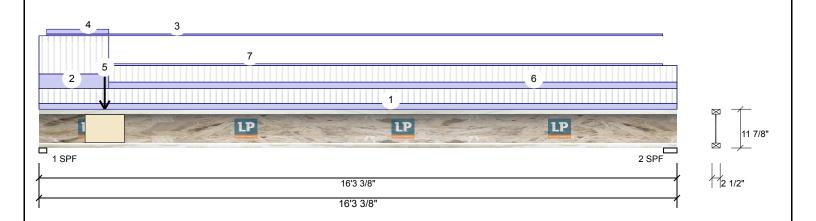
12/14/2018

Designer: RCO Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	1	Design Method:	LSD	1	765	372	0	0		
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	471	227	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions				
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF 2	2.375"	99% 465 / 1147	1612 L	1.25D+1.5L		
				2-SPF 4	4.125"	54% 284 / 706	990 L	1.25D+1.5L		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4104 ft-lb	7'5 1/2"	6250 ft-lb	0.657 (66%)	1.25D+1.5L	L
Shear	1588 lb	1 5/8"	2345 lb	0.677 (68%)	1.25D+1.5L	L
Perm Defl in.	0.155 (L/1227)	7'10 5/16"	0.529 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.318 (L/598)	7'10 5/16"	0.529 (L/360)	0.600 (60%)	L	L
TL Defl inch	0.473 (L/402)	7'10 5/16"	0.793 (L/240)	0.600 (60%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.155", Long Term = 0.233"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 5'1" o.c.

6 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-3-6	(Span)1-3-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 15-11-0		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	147 lb	302 lb	0 lb	0 lb	F12
6	Tie-In	1-9-6 to 16-3-6	(Span)1-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 15-11-0		Тор	3 PLF	0 PLF	0 PLF	0 PLF	

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Client:

Project: Address:

GREEN YORK HOMES

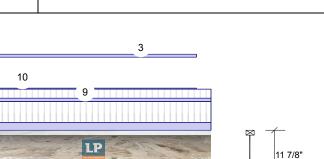
12/14/2018 Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Level: Ground Floor

Project #:

11.875" - PASSED LPI 20Plus



1	,	
LP LP	L	LP
1 SPF		2 SPF
	16'3 3/8"	
	16'3 3/8"	1

Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Application: Floor (Residential) Live Dead Snow Wind Type: Brg Plies: Design Method: 408 206 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 416 209 0 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 53% 257 / 612 1.25D+1.5L 869 I 2 - SPF 4.125" 48% 261 / 624 885 1.25D+1.5L

8

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4338 ft-lb	8' 5/8"	6250 ft-lb	0.694 (69%)	1.25D+1.5L	L
Shear	868 lb	16'	2345 lb	0.370 (37%)	1.25D+1.5L	L
Perm Defl in.	0.159 (L/1194)	8' 7/8"	0.529 (L/360)	0.300 (30%)	D	Uniform
LL Defl inch	0.316 (L/603)	8' 15/16"	0.529 (L/360)	0.600 (60%)	L	L
TL Defl inch	0.475 (L/400)	8' 15/16"	0.793 (L/240)	0.600 (60%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.159", Long Term = 0.239"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 4'11" o.c.

6 Bottom flange braced at bearings.

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December 18, 2018

	, ,								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-3-6	(Span)1-0-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-5-6	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 15-10-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 6-5-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	6-5-6 to 9-10-6	(Span)1-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	6-5-6 to 9-10-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
7	Point	6-6-10		Near Face	65 lb	129 lb	0 lb	0 lb	F11

Continued on page 2...

Notes

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CCMC: 12412-R APA: PR-L238C





Page 2 of 2



Client: Address:

Project:

GREEN YORK HOMES Date:

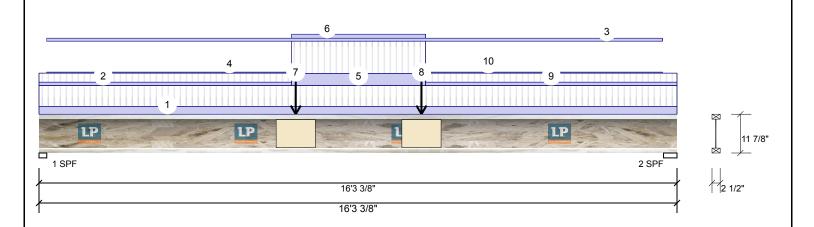
12/14/2018 Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Continued from page	e 1
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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	9-9-2		Near Face	63 lb	127 lb	0 lb	0 lb	F11
9	Tie-In	9-10-6 to 16-3-6	(Span)0-5-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	9-10-6 to 15-10-14		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Address:

Project:

GREEN YORK HOMES

12/14/2018

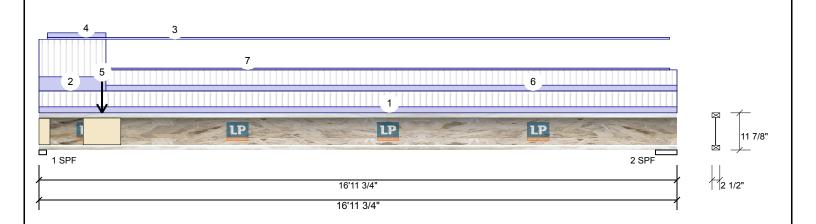
Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED F15-D LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	1	Design Method:	LSD	1	802	390	0	0		
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	497	241	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions				
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF 2	2.375"	79% 488 / 1203	1690 L	1.25D+1.5L		
				2-SPF 6	3.875"	57% 301 / 745	1046 L	1.25D+1.5L		

Analysis Results

Comb.	Case
) 1.25D+1.5L	L
) 1.25D+1.5L	L
) D	Uniform
) L	L
) D+L	L
6	Comb. 6) 1.25D+1.5L 6) 1.25D+1.5L 6) D 6) L 6) D+L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.174", Long Term = 0.260"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only. 5 Top flange must be laterally braced at a maximum of 4'10" o.c.
- 6 Bottom flange braced at bearings.

7 Web stiffeners required at Bearing 1

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

7 Web stilleners required at bearing 1.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-11-12	(Span)1-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-10 to 16-9-6		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-12 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Near Face	159 lb	325 lb	0 lb	0 lb	F12
6	Tie-In	1-9-6 to 16-11-12	(Span)1-3-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 16-9-6		Тор	3 PLF	0 PLF	0 PLF	0 PLF	

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client:

Address:

Project:

GREEN YORK HOMES

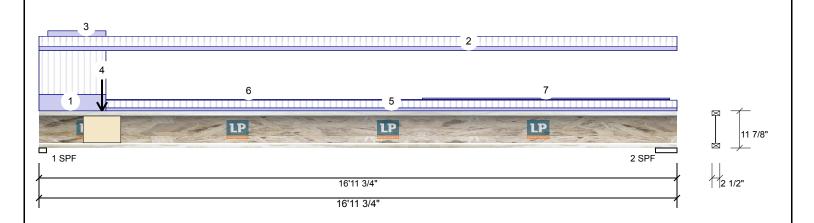
12/14/2018 Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

11.875" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactored	l Reactio	ns UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	719	335	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	278	129	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF			Bearing Le	ngth	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2.3	375"	91% 418 / 1078	1496 L	1.25D+1.5L
				2 - SPF 6.8	375"	32% 161 / 417	578 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2709 ft-lb	6'6 1/4"	6250 ft-lb	0.433 (43%)	1.25D+1.5L	L
Shear	1475 lb	1 5/8"	2345 lb	0.629 (63%)	1.25D+1.5L	L
Perm Defl in.	0.103 (L/1907)	7'9 7/8"	0.544 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.227 (L/864)	7'9 7/8"	0.544 (L/360)	0.420 (42%)	L	L
TL Defl inch	0.330 (L/594)	7'9 7/8"	0.817 (L/240)	0.400 (40%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.103", Long Term = 0.154"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'4" o.c.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 16-11-12	(Span)0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-13 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
4	Point	1-8-2		Far Face	218 lb	450 lb	0 lb	0 lb	F12
5	Tie-In	1-9-6 to 16-11-12	(Span)0-6-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	1-9-6 to 16-9-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
7	Part. Uniform	10-2-6 to 16-9-6		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

Pass-Thru Framing Squash Block is

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6 Bottom flange braced at bearings

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required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C







Client: Project: Address:

GREEN YORK HOMES

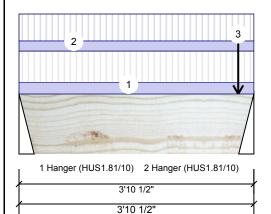
12/14/2018 Designer: RCO

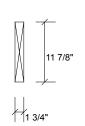
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





\\/ind

Member Inforn	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

lnia .	LIVC	Dead	GIIOW	vviila	
1	62	33	0	0	
2	580	266	0	0	

Dead

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	133 ft-lb	2'3 13/16"	17130 ft-lb	0.008 (1%)	1.25D+1.5L	L
Unbraced	133 ft-lb	2'3 13/16"	12574 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	101 lb	2'8 3/8"	5798 lb	0.017 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/66968)	2'1 7/8"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/44165)	2'1 3/4"	0.175 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

Livo

Bra

L								
I	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	3%	42 / 94	136	L	1.25D+1.5L	
	2 - Hanger	3.000"	64%	332 / 870	1202	L	1.25D+1.5L	

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 3-10-8	(Span)0-8-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 3-10-8	(Span)0-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	3-7-6		Тор	242 lb	539 lb	0 lb	0 lb	C3	
	Self Weight	READ ALL NOTES O	EAD ALL NOTES ON THIS PAGE AND ON THE							

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

Detail for ply 6. For flat roofs provide proper drainage to prevent ponding requirements

Forex

to ply nailing or bolting Kott Lumber Company 14 Anderson Blvd, Ontario Canada APA: PR-L318 905-642-4400





This design is valid until 7/10/2021



Client: Project: Address:

GREEN YORK HOMES

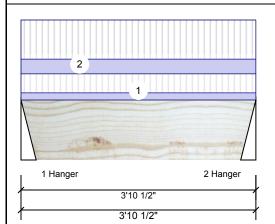
12/14/2018 Date:

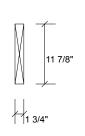
Designer: RCO Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Wind

0

0

0

0

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

52

52

115

115

2

Bearing	s and Fa	ctored l					
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	6%	66 / 173	238	L	1.25D+1.5L	
2 - Hanger	3.000"	6%	66 / 173	238	L	1.25D+1.5L	

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	188 ft-lb	1'11 1/4"	17130 ft-lb	0.011 (1%)	1.25D+1.5L	L
Unbraced	188 ft-lb	1'11 1/4"	12574 ft-lb	0.015 (1%)	1.25D+1.5L	L
Shear	93 lb	1'2 1/8"	5798 lb	0.016 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/45885)	1'11 1/4"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/31535)	1'11 1/4"	0.175 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 3-10-8	(Span)0-11-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Part. Uniform	0-0-0 to 3-10-8		Тор	15 PLF	40 PLF	0 PLF	0 PLF
	Self Weight				5 PLF			

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info







Client: Address:

Project:

GREEN YORK HOMES

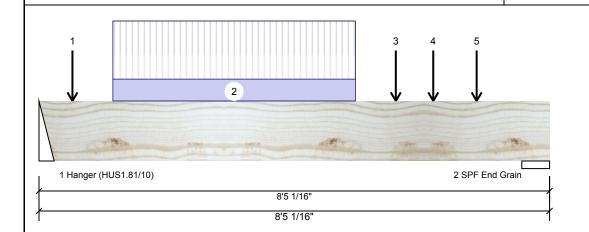
12/14/2018 Date:

Designer: RCO Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Member Into	ormation
-------------	----------

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	404	181	0	0
2	704	324	0	0

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2193 ft-lb	5'10 5/8"	17130 ft-lb	0.128 (13%)	1.25D+1.5L	L
Unbraced	2193 ft-lb	5'10 5/8"	5736 ft-lb	0.382 (38%)	1.25D+1.5L	L
Shear	1452 lb	7' 7/16"	5798 lb	0.250 (25%)	1.25D+1.5L	L
Perm Defl in.	0.013 (L/7131)	4'5 9/16"	0.261 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.029 (L/3217)	4'5 7/16"	0.261 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.042 (L/2217)	4'5 1/2"	0.392 (L/240)	0.110 (11%)	D+L	L

Bearings and Factored Reactions

Bearing Length	Cap. R	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - 3.000" Hanger	21%	227 / 607	833	L	1.25D+1.5L
2 - SPF 5.500" End Grain	20%	405 / 1055	1460	L	1.25D+1.5L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

	Self Weight	READ ALL NOTES ON THIS PAGE AND ON THE		5 PLF	Pass-Thru Framing Squash Block is				
5	Point	7-2-10		Far Face	25 lb	65 lb	0 lb	0 lb	J1
4	Point	6-6-0		Far Face	266 lb	580 lb	0 lb	0 lb	F2
3	Point	5-10-10		Far Face	28 lb	76 lb	0 lb	0 lb	J1
2	Part. Uniform	1-2-10 to 5-2-10		Far Face	29 PLF	77 PLF	0 PLF	0 PLF	
1	Point	0-6-10		Far Face	30 lb	79 lb	0 lb	0 lb	
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements Manufacturer Info

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
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- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
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 fastening details, beam strength values, and code
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

Damaged Beams must not be used

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

PROFESSIONAL

I.MATIJEVIC 100528832

SHOWINGE OF ONTARIO

December 18, 2018







Client: **GREEN YORK HOMES**

Project:

12/14/2018 Date: Designer: RCO

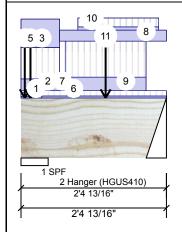
Job Name: LOT 10 (CELESTIAL 2 2)

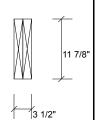
Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Address:

Level: Ground Floor





Wind 0 0

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

Member Info	rmation			Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	w
Plies:	2	Design Method:	LSD	1	2716		1180		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	314		165		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions		
Dead:	15 PSF			Bearing L	_ength	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF 5	5.375"	52%	1474 / 4075	5549	L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	396 ft-lb	1'4 3/4"	34261 ft-lb	0.012 (1%)	1.25D+1.5L	L
Unbraced	396 ft-lb	1'4 3/4"	34261 ft-lb	0.012 (1%)	1.25D+1.5L	L
Shear	337 lb	1'1 11/16"	11596 lb	0.029 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/33790)	1'4 3/4"	0.058 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/23170)	1'4 3/4"	0.087 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

8 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

2 -

Hanger

4.000"

7%

206 / 471



677 L

December 18, 2018

	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 0-4-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-0-0 to 0-7-10		Тор	33 PLF	88 PLF	0 PLF	0 PLF	J5
	3	Part. Uniform	0-0-0 to 0-7-10		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
								Framing Squ	uash Rior	rk is

Continued on page 2...

Pass-Thru Framing Squash Block is required at all point loads over bearings

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide propressing to Multiple Member Connection ponding Detail for ply topy nailing or bolting
APA: PR-L318 requirements

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400







This design is valid until 7/10/2021

Page 2 of 2



Client: Project: Address:

GREEN YORK HOMES

12/14/2018

Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

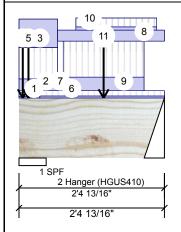
Project #:

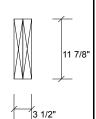
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Continued from	Continued from page 1										
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments		
4	Point	0-0-12		Far Face	92 lb	245 lb	0 lb	0 lb	J5		
5	Point	0-1-14		Тор	901 lb	2190 lb	0 lb	0 lb	F9 F9		
6	Tie-In	0-4-2 to 2-4-13	(Span)0-8-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF			
7	Part. Uniform	0-7-10 to 0-8-12		Тор	13 PLF	34 PLF	0 PLF	0 PLF	J5		
8	Part. Uniform	0-7-10 to 2-4-13		Тор	31 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight		
9	Part. Uniform	0-8-12 to 2-0-12		Тор	38 PLF	101 PLF	0 PLF	0 PLF	J5		
10	Part. Uniform	0-11-4 to 2-3-4		Тор	9 PLF	25 PLF	0 PLF	0 PLF	J1		
11	Point	1-4-12		Far Face	125 lb	334 lb	0 lb	0 lb	J5		
	Self Weight				10 PLF						



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Manufacturer Info

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 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318







Client:

Project: Address: **GREEN YORK HOMES** Date:

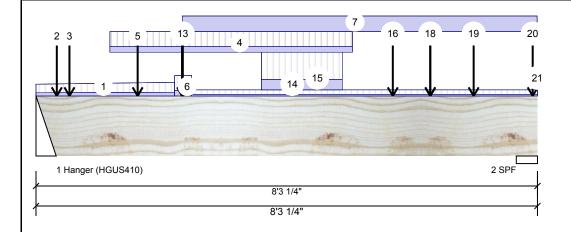
12/14/2018 Designer: RCO

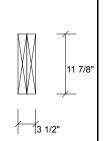
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Ground Floor





1.25D+1.5L

Member Info	rmation			Unfactored Reactions UNPATTERNED lb (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dea	ıd	Snow		Wind	
Plies:	2	Design Method:	LSD	1	1104	72	27	0		0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	996	79)2	0		0	
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearings	s and Fac	tored React	ions				
Dead:	15 PSF			Bearing	Length	Cap. Reac	t D/L lb	Total L	d. Case	Ld. Comb.	
				1 -	4.000"	25% 909	9 / 1656	2565 L	_	1.25D+1.5L	
				Hanger							

Case

J							
ı	Analysis	Actual	Location	Allowed	Capacity	Comb.	Ca
ı	Moment	4916 ft-lb	3'9 11/16"	34261 ft-lb	0.143 (14%)	1.25D+1.5L	L
ı	Unbraced	4916 ft-lb	3'9 11/16"	31493 ft-lb	0.156 (16%)	1.25D+1.5L	L
I	Shear	2429 lb	1'3 1/8"	11596 lb	0.209 (21%)	1.25D+1.5L	L

Perm Defl in. 0.021 (L/4424) 4' 5/8" 0.257 (L/360) 0.080 (8%) D LL Defl inch 0.028 (L/3294) 4' 1/16" 0.257 (L/360) 0.110 (11%) L L TL Defl inch 0.049 (L/1888) 4' 5/16" 0.386 (L/240) 0.130 (13%) D+L

)+1.5L L)+1.5L L Uniform

I.MATIJEVIC 100528832 SHOVINCE OF ONTRHI

2484 L

December 18, 2018

Design Notes

Analysis Results

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

2 - SPF 4.192"

28%

990 / 1494

I Lateral Sieriue	illess latio based oil	iuli section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-3-7	(Span)2-2-7 to 2-6-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-4-2		Far Face	15 lb	39 lb	0 lb	0 lb	J1
3	Point	0-6-10		Near Face	30 lb	79 lb	0 lb	0 lb	J1
4	Part. Uniform	1-2-10 to 5-2-10		Near Face	29 PLF	77 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	17 lb	44 lb	0 lb	0 lb	J1

Continued on page 2...

Pass-Thru Framing Squash Block is required at all point loads over bearings

Notes

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 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proponding

Manufacturer Info Refer to Multiple Member Connection Detail for ply tenely mailing or bolting requirements

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 7/10/2021

Page 2 of 2



Continued from page 1

Client: **GREEN YORK HOMES**

Address:

Project:

Date: 12/14/2018

Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

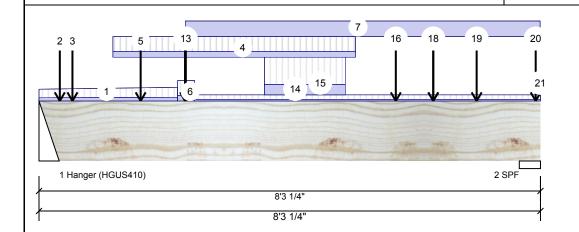
Project #:

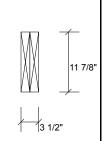
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





ı	Continued from pa	age 1								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	6	Tie-In	2-3-7 to 2-6-13	(Span)3-9-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	7	Part. Uniform	2-4-15 to 8-3-3		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	8	Point	2-4-15		Тор	79 lb	211 lb	0 lb	0 lb	J5
	9	Point	2-4-15		Тор	20 lb	52 lb	0 lb	0 lb	J1
	10	Point	2-4-15		Тор	2 lb	5 lb	0 lb	0 lb	J5
	11	Point	2-4-15		Тор	86 lb	0 lb	0 lb	0 lb	Wall Self Weight
	12	Point	2-4-15		Тор	12 lb	0 lb	0 lb	0 lb	Wall Self Weight
	13	Point	2-5-1		Far Face	165 lb	314 lb	0 lb	0 lb	F6
	14	Tie-In	2-6-13 to 8-3-4	(Span)1-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	15	Part. Uniform	3-8-10 to 5-0-10		Тор	53 PLF	140 PLF	0 PLF	0 PLF	J3
	16	Point	5-10-10		Near Face	28 lb	76 lb	0 lb	0 lb	J1
	17	Point	6-6-0		Тор	141 lb	322 lb	0 lb	0 lb	F10 F10
	18	Point	6-6-0		Near Face	33 lb	62 lb	0 lb	0 lb	F2
	19	Point	7-2-10		Near Face	25 lb	65 lb	0 lb	0 lb	J1
	20	Point	8-2-5		Near Face	34 lb	75 lb	0 lb	0 lb	F2
	21	Part. Uniform	8-3-3 to 8-3-4		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
		Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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December 18, 2018

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6. For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400





This design is valid until 7/10/2021



Client: **GREEN YORK HOMES**

Address:

Project:

12/14/2018 Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

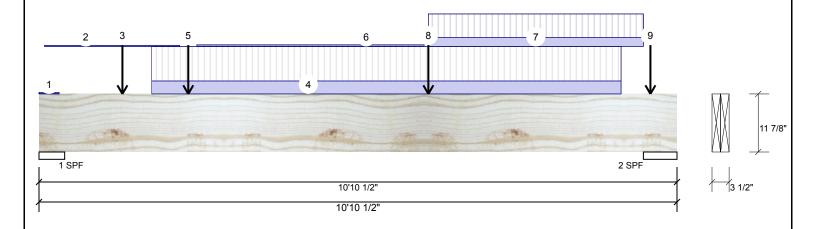
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor



mation			Unfactored Reactions UNPATTERNED lb (Uplift)							
Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind			
2	Design Method:	LSD	1	3024	1441	0	0			
n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	3159	1322	0	0			
360	Load Sharing:	No								
240	Deck:	Not Checked								
Normal	Vibration:	Not Checked								
40 PSF			Bearings a	and Facto	red Reactions					
15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.			
			1 - SPF 5.	.250"	56% 1801 / 4536	6337 L	1.25D+1.5L			
			2 - SPF 6	.875"	43% 1653 / 4738	6391 L	1.25D+1.5L			
	2 in: Dry 360 240 Normal	Girder 2 Design Method: Building Code: 100 Load Sharing: 100 Deck: 100 Vibration: 100 Vibration: 100 Vibration:	Girder 2 Design Method: LSD Building Code: NBCC 2010 / OBC 2012 Load Sharing: No 240 Normal Vibration: Not Checked 40 PSF 15 PSF	Girder Application: Floor (Residential) Design Method: LSD In: Dry Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Normal Vibration: Not Checked Bearings a Bearings L 1 - SPF 5 2 - SPF 6	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live Dead	Application: Floor (Residential) Brg Live Dead Snow			

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16170 ft-lb	5'6 1/16"	34261 ft-lb	0.472 (47%)	1.25D+1.5L	L
Unbraced	16170 ft-lb	5'6 1/16"	29618 ft-lb	0.546 (55%)	1.25D+1.5L	L
Shear	6713 lb	1'4 3/8"	11596 lb	0.579 (58%)	1.25D+1.5L	L
Perm Defl in	. 0.076 (L/1587)	5'3 11/16"	0.333 (L/360)	0.230 (23%)	D	Uniform
LL Defl inch	0.171 (L/703)	5'4 5/8"	0.333 (L/360)	0.510 (51%)	L	L
TL Defl inch	0.246 (L/487)	5'4 3/8"	0.499 (L/240)	0.490 (49%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
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- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral slenderness ratio based on full section width

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December 18, 2018

o Lateral Sieride	illess ratio based off it	ili section width.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 0-4-2	(Span)0-7-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-1-2 to 2-4-13	(Span)0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-5-1		Near Face	151 lb	402 lb	0 lb	0 lb	J8	
4	Part. Uniform	1-11-1 to 9-11-1		Near Face	148 PLF	395 PLF	0 PLF	0 PLF		
5	Point	2-6-9		Far Face	727 lb	1104 lb	0 lb	0 lb	F7	
6	Tie-In	2-8-5 to 6-6-12	(Span)0-8-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF		

Continued on page 2...

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raming Squash Block is 6. For flat roofs provide proper drainage to prevent ponding required at a il-point loads over bearing

APA: PR-L318

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirement

This design is valid until 7/10/2021





Page 2 of 2

isDesign™

Client: Project:

Address:

GREEN YORK HOMES

12/14/2018 Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

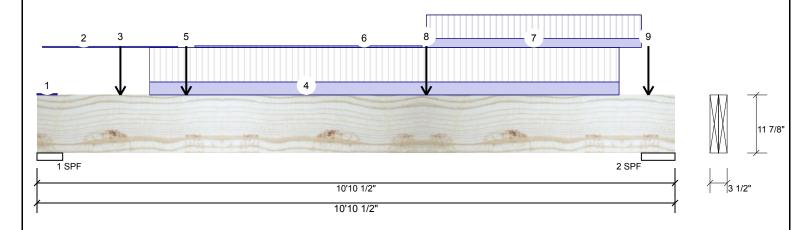
Project #:

Forex 2.0E-3000Fb LVL F8-B

1.750" X 11.875"

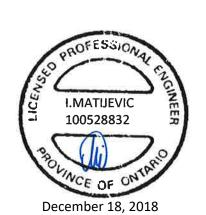
2-Ply - PASSED

Level: Ground Floor



Continued	from	page	1
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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Part. Uniform	6-7-10 to 10-3-10		Тор	101 PLF	270 PLF	0 PLF	0 PLF	
8	Point	6-7-10		Far Face	181 lb	404 lb	0 lb	0 lb	F5
9	Point	10-5-1		Near Face	16 lb	42 lb	0 lb	0 lb	J8
	Self Weight				10 PLF				



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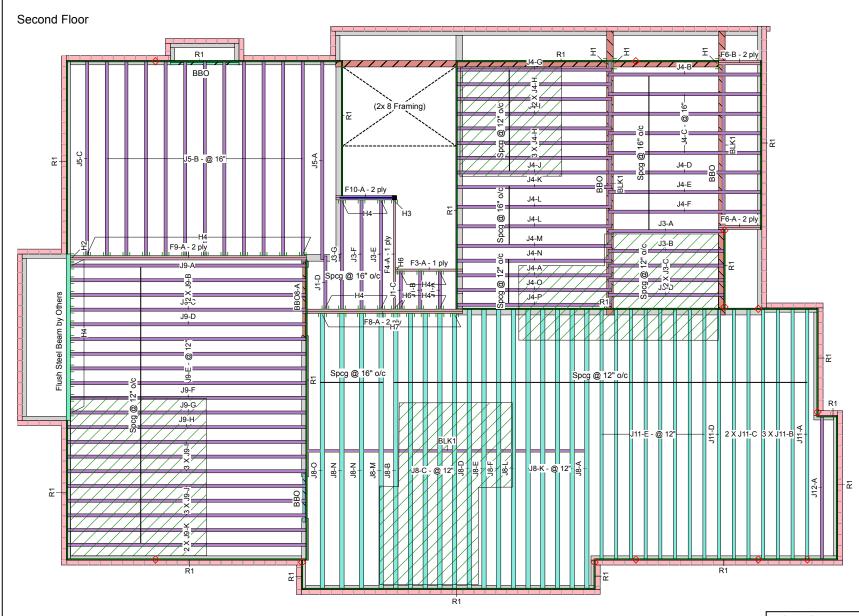
For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318







This certification is to confirm that:

- 1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
- 2. The floor joists comply with the KOTT span table for the loads and spacing shown on this layout. The floor system must be assembled in accordance to the KOTT Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral

stability are the responsibility of others.



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.5; Aug.30,2018 Project No: 17-55 Model: Celestial 2

Legend

Load from Above Norbord Rimboard Plus 1.125 X 11.875 LPI 20Plus 11.875 NJ60H 11.875 NJ60U 11.875

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Secon	Floor										
	L (Flus										
Label	Descri	iption	Wic	lth	De	pth	(Qty	Plies	Pcs	Le
F9	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375		1	2	2	18
F8	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375		1	2	2	12
F4	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375				1	8
F10	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375		1	2	2	(
F3	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375				1	(
F6	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375		2	2	4	4
_VL/LS	L (Dro	pped)									
Label	Descri	iption	Wic	lth	De	pth	(Qty	Plies	Pcs	Le
BBO8	Forex 2.0E-3000Fb LVL		1.	75		9.5		1	2	2	•
Joist ((Flush)										
Label	Descr	iption	Wic	lth	De	pth	(Qty	Plies	Pcs	Le
J9	LPI 20Plus		2	2.5	11.8	375				20	18
J5	LPI 20		2	2.5		375				13	14
J4	LPI 20	Plus	2	2.5	11.8	375				24	12
J12	LPI 20		2	2.5	11.8	375				1	10
J3	LPI 201			2.5	11.8					8	1
J1	LPI 20			2.5	_	375				4	4
J11	NJ60H			2.5		375				15	18
J8	NJ60U		3	3.5	11.8	375				18	20
Rim Bo							_			_	1.
Label	Descr		Wic	_	De		(Qty	Plies	Pcs	Le
R1	Norbor Plus 1. 11.875		1.1	25	11.8	375				19	
Blockir	ig										
Label	Descri	iption	Wic	lth	De	pth	(Qty	Plies	Pcs	Le
BLK1	LPI 20	Plus	2	2.5	11.8		L	inFt		Varies	33
Hange	r						•	Bea	am/Girde		
		l				0'		_			emb
Label				SI	kew	Sic	pe	ta	steners	ras	ten
HT	H1 3 Unknown										
Hanger H2 1 HGUS410									46 16d	1	6 16
H3	1	HUCQ1.81	/9-						100	1	- 10
	•	ene	-								

					Deall/Gildel	Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
H1	3	Unknown Hanger				
H2	1	HGUS410			46 16d	16 16d
НЗ	1	HUCQ1.81/9- SDS				
H4	36	LT251188			4 10dx1 1/2	2 10dx1 1/2
H6	2	HUS1.81/10			30 10dx1 1/2	10 16d
H7	9	LT351188			4 10dx1 1/2	2 10dx1 1/2

NOTES

- 2. Doub
- anoth
- 3. Instal
- 4 Instal rimbo
- 5. Refer 6. Squa two le
- . Load
- 8. It sha

Refer to

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

	Hoor											
	L (Flus											
<u> </u>	Descri	ption	Widt	_	epth	(Qty	Plies	Pcs	Length		
	Forex 2 0F-30	000Fb LVL	1.7	75 11	.875		1	2	2	18-0-0		
	Forex		1.7	'5 11	.875		1	2	2	12-0-0	Layout Name	TIAL 0.0)
	2.0E-30 Forex	000Fb LVL	1.7	'5 11	.875				1	8-0-0	LOT 10 (CELES	<u> </u>
_	2.0E-30	000Fb LVL									Design Metho	u
	Forex	000Fb LVL	1.7	'5 11	.875		1	2	2	6-0-0	Revised	
	Forex		1.7	'5 11	.875				1	6-0-0	December 14, 2	2018
	2.0E-30 Forex	000Fb LVL	1.7	'E 44	.875		2	2	4	4-0-0	Description	
		000Fb LVL	1.7	3 11	.075		2	2	7	4-0-0	GRANELLI HOI BRAMPTON, O	
S	L (Drop	oped)										141.
	Descri	ption	Widt	h De	epth	(Qty	Plies	Pcs	Length	Builder	LIOMES
В	Forex	000Fb LVL	1.7	75	9.5		1	2	2	6-0-0	GREEN YORK	HOMES
(Flush)	JOOF D LVL								1	Sales Rep	
	Descri	ption	Widt	h De	epth		Qty	Plies	Pcs	Length	Designer	
	LPI 20F	Plus	2.	.5 11	.875				20	18-0-0	RCO	
	LPI 20F	Plus	2.	.5 11	.875				13	14-0-0		
	LPI 20F		2.	.5 11	.875				24	12-0-0	Shipping	
_	LPI 20F		2.		.875				1	10-0-0	Project	
_	LPI 20F		2.		.875				8	8-0-0	Builder's Proje	ect
_	LPI 20F	Plus	2.	_	.875				4	4-0-0	Kott Lumbe	er Company
	NJ60H		2.		.875				15	18-0-0	14 Anderson Bl	
,_	NJ60U ard		3.	.5 11	.875				18	20-0-0	Stouffville, Onta	
el El	Descri	ntion	Widt	h D	epth		Qty	Plies	Pcs	Length	Canada	
71		d Rimboard	1.12	_	.875	<u> </u>	IJιy	Files	19	12	L4A 7X4	
	Plus 1.		1.12	''	.070				13	12	905-642-4400	
_	11.875										Job Path	
<u>in</u>	Υ						~.			I	D:\Users\rocha\	/illo\WORK FROM
<u>!</u>	Descri	•	Widt	_	pth	_	Qty	Plies	Pcs	Length	HOME\GREEN	
<u> </u>	LPI 20	Plus	2.	.5 11	.875	L	inFt		Varies	33-0-0		ME CORP\MODELS STIAL 2 ELEV.2)
er							Res	am/Girdei	Sur	ported		0 (CELESTIAL 2 2).is
							DC	arri/Orraci		ember		
ıΤ	Pcs	Descriptio	n T	Skew	Slo	ре	fa	steners	_	teners	Second Floo	r
+	3	Unknown				η			1		Design Method	LSD
1		Hanger									Building Code	NBCC 2010 / OBC
+	1	HGUS410 HUCQ1.81	/O					46 16d	1	6 16d	Floor	2012
1	'	SDS	9-								Loads	
1	36	LT251188					4 1	10dx1 1/2	2 10	dx1 1/2	Live	40
1	2	HUS1.81/1	0				30	10dx1 1/2	1	0 16d	Dead	15
I	9	LT351188					4 1	10dx1 1/2	2 10	dx1 1/2	Deflection Jois	
S:											LL Span L/	360
		if. dimensia		ماد مدا	ita atı	مامد					TL Span L/	360
		ify dimensio nly require fi									LL Cant 2L/	480
		per using a fa						.5			TL Cant 2L/	360
		cking @ 24"							ls.		Deflection Gird	ler
	sıngle-p ard/rimjo	oly flush wind	dow he	ader al	ong ir	iside	tace	of			LL Span L/	360
		or specifier g	guide fo	or insta	lation	wor	ks.				TL Span L/	240
		s recommen									LL Cant 2L/	480
		ists which so	upport l	loading	from	abo	ve ex	ceeding			TL Cant 2L/	360
		r or roof. blocks to be	installe	ed und	er all r	ooint	loads	3 .			Decking	
al	l be the	framer's res	ponsibi	ility tha	floor	joist	ts and		е		Deck	SPF Plywood
en	ed as pe	er the hange	r manu	ufacture	r's sta	anda	ırds.				Thickness	5/8"
n	Multinle	Member Co	nnectic	on Deta	il to r	lv t∩	plv n	ailing or			Fastener	Nailed & Glued
	equireme			J., DOIG	to p	., .	P.y 11	g 01			Vibration	
										- 1	I	



Ceiling:

Gypsum 1/2"

Wind

0

0

1.25D+1.5L

1.25D+1.5L

Page 1 of 1



Client: **GREEN YORK HOMES**

Project:

12/14/2018 Date:

Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Level: Second Floor

Project #:

Forex 2.0E-3000Fb LVL BBO8-A

1.750" X 9.500"

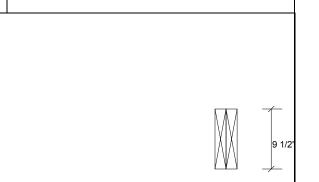
2-Ply - PASSED

1

2

1 - SPF 4.000"

2 - SPF 4.000"

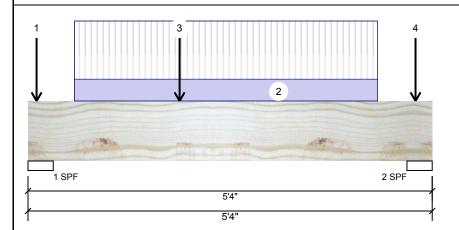


O

0

4650 L

3194 L



Address:

Member Information Type:

Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Floor (Residential) Application: Design Method:

Building Code: NBCC 2010 / OBC 2012 No

Load Sharing: Deck: Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

54%

37%

2317

1592

Bearings and Fact	tored Reactions		
Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

1174 / 3476

807 / 2388

939

645

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6964 ft-lb	2'	22724 ft-lb	0.306 (31%)	1.25D+1.5L	L
Unbraced	6964 ft-lb	2'	22724 ft-lb	0.306 (31%)	1.25D+1.5L	L
Shear	4095 lb	1' 3/4"	9277 lb	0.441 (44%)	1.25D+1.5L	L
Perm Defl in.	0.014 (L/4009)	2'3 9/16"	0.160 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.035 (L/1625)	2'3 9/16"	0.160 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.050 (L/1156)	2'3 9/16"	0.240 (L/240)	0.210 (21%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

Load Type

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.



December 18, 2018

		IS AN INTEGRAL PART OF T		1	Dace-Thru	Framina So	uiach Blo
	Self Weight	READ ALL NOTES ON THIS I ENGINEERING NOTE PAGE I	ENP-2. THE NOTE PAGE	8 PLF			
4	Point	5-1-6	Ton	38 lb	102 lb	0 lb	0 lb
3	Point	2-0-0	Тор	976 lb	2394 lb	0 lb	0 lb
2	Part. Uniform	0-7-6 to 4-7-6	Тор	120 PLF	320 PLF	0 PLF	0 PLF
1	Point	0-1-6	Тор	50 lb	133 lb	0 lb	0 lb

Trib Width

ass-Thru Framing Squash Block is CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. required at all point loads over bearings **Manufacturer Info**

Side

ID

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation

Location

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Dead

pleeMember Connection **Refer to Mult** toppipriating or bolting **Detail for ply** requirement

Snow

Wind

0 lb .19

Comments

F8 0 lb 0 lb .19

Live







Client: Project:

Address:

GREEN YORK HOMES

12/14/2018 Date:

Designer: RCO

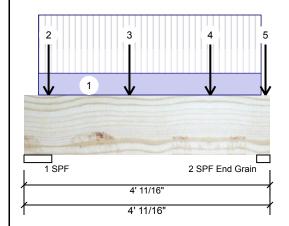
Job Name: LOT 10 (CELESTIAL 2 2)

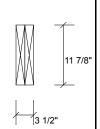
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Wind

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		
	Type: Plies: Moisture Condition: Deflection LL: Deflection TL: Importance: General Load Floor Live:	Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load Floor Live: 40 PSF	Type: Girder Application: Plies: 2 Design Method: Moisture Condition: Dry Building Code: Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Importance: Normal Vibration: General Load Floor Live: 40 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg

1	322	141	0	0
2	539	242	0	0

Bearings and Factored Reactions

Bearing Length	Cap. React	D/L lb Total	Ld. Case	Ld. Comb.
1 - SPF 5.500"	6% 17	6 / 483 660	L	1.25D+1.5L
2 - SPF 2.632" End	16% 30	2 / 808 1110	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	673 ft-lb	1'8 7/8"	34261 ft-lb	0.020 (2%)	1.25D+1.5L	L
Unbraced	673 ft-lb	1'8 7/8"	34261 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	530 lb	2'10 15/16"	11596 lb	0.046 (5%)	1.25D+1.5L	L
Perm Defl in	. 0.001 (L/60821)	2' 1/2"	0.117 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/25507)	2' 1/4"	0.117 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.002 (L/17971)	2' 3/8"	0.175 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-2-14 to 3-10-15		Тор	30 PLF	80 PLF	0 PLF	0 PLF	
2	Point	0-4-14		Near Face	3 lb	8 lb	0 lb	0 lb	J3
3	Point	1-8-14		Near Face	74 lb	198 lb	0 lb	0 lb	J3
4	Point	3-0-14		Near Face	63 lb	167 lb	0 lb	0 lb	J3
5	Point	3-11-13		Near Face	94 lb	194 lb	0 lb	0 lb	F4
	Self Weight				10 PLF	Dace-Thru	Framing Sc	wach Blo	ock ie

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation

LVI beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

required at all point loads over bearing:
6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirement







Client: **GREEN YORK HOMES**

Project:

12/14/2018 Date: Designer: RCO

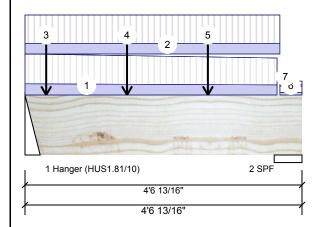
Job Name: LOT 10 (CELESTIAL 2 2)

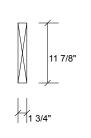
Project #

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor





Member Info	rmation			Unfactor	ed React	ions UNPATTERNI	D lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	351	142	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	303	125	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fac	tored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - Hanger	3.000"	18% 178 / 527	705 L	1.25D+1.5L
Analysis Resu	ılts			2 - SPF	5.500"	10% 156 / 454	610 L	1.25D+1.5L
Analysis A	Actual Loc	eation Allowed Canac	ity Comb Case					

READ ALL NOTES ON THIS PAGE AND ON THE

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	632 ft-lb	2'1 1/8"	17130 ft-lb	0.037 (4%)	1.25D+1.5L	L
Unbraced	632 ft-lb	2'1 1/8"	11283 ft-lb	0.056 (6%)	1.25D+1.5L	L
Shear	433 lb	1'2 1/8"	5798 lb	0.075 (7%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/32806)	2'1 13/16"	0.133 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/13295)	2'1 3/4"	0.133 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/9461)	2'1 3/4"	0.199 (L/240)	0.030 (3%)	D+L	L



ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

1 Fill all hanger nailing holes. 2 Girders are designed to be supported on the bottom edge only.

Design Notes

3 Top braced at bearings

4 Bottom braced at bearings.

ID Load Type Trib Width Side Dead I ive Wind Comments Location Snow (Span)2-11-6 Top 40 PSF Tie-In 0-0-0 to 4-1-14 15 PSF 0 PSF 0 PSF 1 to 2-7-14 Tie-In 0-0-0 to 4-2-7 (Span)2-9-10 Top 15 PSF 40 PSF 0 PSF 0 PSF 3 Point 0-4-3 Near Face 17 lb 45 lb 0 lb 0 lb J1 Point 1-8-3 Near Face 26 lb 69 lb 0 lb 0 lb J1 5 Point 3-0-3 Near Face 24 lb 64 lb 0 lb 0 lb J1 6 Tie-In 4-2-7 to 4-6-13 (Span)0-7-11 Top 15 PSF 40 PSF 0 PSF 0 PSF

Continued on page 2...

Pass-Thru Framing Squash Block is required at all point loads over bearing

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Notes

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Refer to Multiple Member Connection Detail for ply to pigh alling or bolting requirement

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 7/10/2021



Page 2 of 2



Client: Project: Address:

GREEN YORK HOMES

Date: 12/14/2018 Designer:

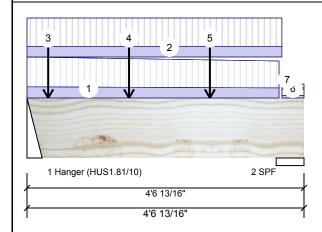
RCO

Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor



.Continued from page 1

Tie-In

7

ID Trib Width Side Live Wind Comments Load Type Location Dead Snow 4-2-7 to 4-6-13 (Span)0-4-5 15 PSF 0 PSF

Top

Self Weight 5 PLF

40 PSF 0 PSF

> PROFESSIONAL LICENSES **I.MATIJEVIC** 100528832 SHOVINCE OF ONTARIO December 18, 2018

READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
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- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info







Client: **GREEN YORK HOMES**

Project:

12/14/2018 Date:

Designer: RCO

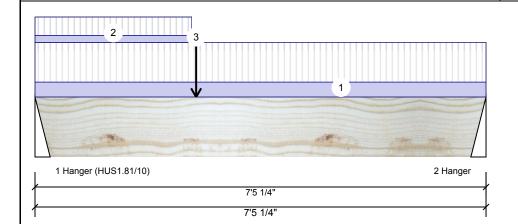
Job Name: LOT 10 (CELESTIAL 2 2)

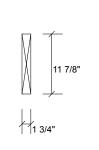
Project #:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor





|--|

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	314	142	0	0
2	194	94	0	0

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1399 ft-lb	2'7 7/8"	17130 ft-lb	0.082 (8%)	1.25D+1.5L	<u>L</u>
Unbraced	1399 ft-lb	2'7 7/8"	6365 ft-lb	0.220 (22%)	1.25D+1.5L	<u>L</u>
Shear	581 lb	1'2 1/8"	5798 lb	0.100 (10%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/13727)	3'2 7/8"	0.235 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.014 (L/6157)	3'2 3/16"	0.235 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.020 (L/4251)	3'2 7/16"	0.353 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.	_
1 - Hanger	3.000"	17%	178 / 472	650	L	1.25D+1.5L	
2 - Hanger	3.000"	10%	118 / 291	408	L	1.25D+1.5L	

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	
1	Tie-In	0-0-0 to 7-5-4	(Span) 0-10-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 2-7-0	(Span)0-5-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	2-7-14		Near Face	142 lb	351 lb	0 lb	0 lb	F3
	Self Weight				5 PLF	Pag	se Thru Era	mina Sau	ach

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Forex APA: PR-L318

Manufacturer Info







Client:

GREEN YORK HOMES Project:

12/14/2018 Date: Designer: RCO

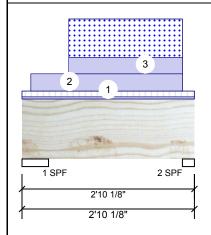
Job Name: LOT 10 (CELESTIAL 2 2)

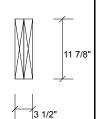
Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Address:

Level: Second Floor





Member Info	rmation			Unfactore	ed React	ions U	NPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	33		165	114	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	28		159	152	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings a	and Fac	tored F	Reactions		
Dead:	15 PSF			Bearing L	ength	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 5	5.250"	4%	207 / 188	394 L	1.25D+1.5S +0.5L
Analysis Resu	ılts			2 - SPF 2	2.375"	9%	199 / 242	441 L	1.25D+1.5S +0.5L
Analysis A	Actual	Location Allowed Capac	ity Comb. Case						

1'6 13/16" 33575 ft-lb 1.25D+1.5S L Moment 262 ft-lb 0.008 (1%) +0.5L Unbraced 262 ft-lb 1'6 13/16" 33575 ft-lb 0.008 (1%) 1.25D+1.5S L +0.5L Shear 81 lb 1'4 3/8" 11364 lb 0.007 (1%) 1.25D+1.5S L +0.5L Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) 1'6 3/4" 0.117 (L/240) 0.010 (1%) D+S+0.5L TL Defl inch 0.001 (L/38560)

I.MATIJEVIC 100528832 POVINCE OF ONTOR

December 18, 2018

1 Girders are designed to be supported on the bottom edge only.

- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.

Design Notes

- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

	Self Weight	READ ALL NOTES ON ENGINEERING NOTE			10 PLF
3	Part. Uniform	0-9-4 to 2-7-12		Тор	61 PLF
2	Part. Uniform	0-1-12 to 2-7-12		Тор	64 PLF
1	Tie-In	0-0-0 to 2-10-2	(Span)1-0-13	Тор	15 PSF
ID	Load Type	Location	Trib Width	Side	Dead

0 PLF 142 PLF 0 PLF Pass-Thru Framing Squash Block is required at all point loads over bearings

Wind

0 PSF

0 PLF

Comments

Wall Self Weight

Refer to Multiple Member Connection Detail for ply to ply nalling or bolt

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

Damaged Beams must not be used

IN THE DESIGN OF THIS COMPONENT.

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Snow

0 PSF

0 PLF

Live

40 PSF

0 PI F

APA: PR-L318







Client: **GREEN YORK HOMES**

Project:

12/14/2018 Date: Designer: RCO

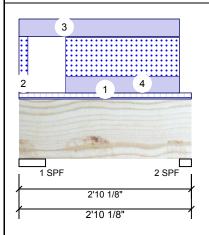
Job Name: LOT 10 (CELESTIAL 2 2)

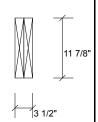
Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Address:

Level: Second Floor





Member Info	rmation			Unfacto	red Reac	tions UN	PATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	25		181	135	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	21		156	152	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored Re	actions		
Dead:	15 PSF			Bearing	Length	Cap. R	eact D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	5.250"	4%	226 / 215	441 L	1.25D+1.5S +0.5L
Analysis Resu	lts			2 - SPF	2.375"	9%	195 / 239	434 L	1.25D+1.5S +0.5l

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	258 ft-lb	1'6 13/16"	33575 ft-lb	0.008 (1%)	1.25D+1.5S +0.5L	L
Unbraced	258 ft-lb	1'6 13/16"	33575 ft-lb	0.008 (1%)	1.25D+1.5S +0.5L	L
Shear	80 lb	1'4 3/8"	11364 lb	0.007 (1%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/39192)	1'6 3/4"	0.117 (L/240)	0.010 (1%)	D+S+0.5L	L

Location

0-0-0 to 2-10-2

0-0-0 to 0-1-12

0-0-0 to 2-7-12

Handling & Installation



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

Load Type

Part. Uniform

Part. Uniform

Tie-In

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

> Wind Comments Live Snow 40 PSF 0 PSF 0 PSF 0 PI F 144 Pass-Thru Framing Squash Block is required atom/L pointalosalisve yer bearings 0 PLF

61 PLF Part. Uniform 0-9-4 to 2-7-12 Тор 0 PLF 142 PLF Refer to Multiple Member Connection Self Weight 10 PLF Detail for ply to ply nailing or bolting requirements Manufacturer Info

ID

2

3

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Damaged Beams must not be used

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation This design is valid until 7/10/2021

Trib Width

(Span)0-9-11 Top

Side

Top

Top

6. For flat roofs provide proper drainage to prevent ponding

Dead

15 PSF

62 PLF

64 PLF

Forex APA: PR-L318







Client: **GREEN YORK HOMES**

Project:

Address:

12/14/2018

Designer: RCO

Job Name: LOT 10 (CELESTIAL 2 2)

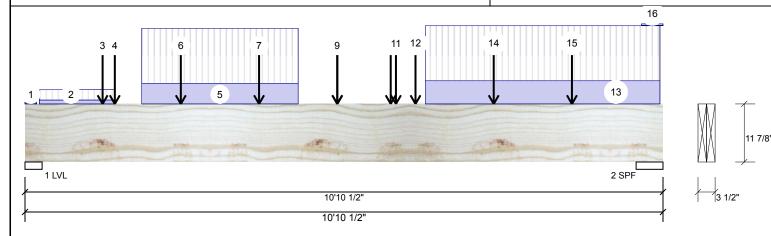
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Application: Floor (Residential) Dead Wind Type: Brg Live Plies: 2 Design Method: 2394 976 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 2576 1098 0 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: 15 PSF Dead: Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1219 / 3591 1 - LVL 3.500" 4810 L 1.25D+1.5L 2 - SPF 5.500" 44% 1372 / 3864 5236 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13924 ft-lb	5'3 7/8"	34261 ft-lb	0.406 (41%)	1.25D+1.5L	L
Unbraced	13924 ft-lb	5'3 7/8"	29373 ft-lb	0.474 (47%)	1.25D+1.5L	L
Shear	5402 lb	1'2 5/8"	11596 lb	0.466 (47%)	1.25D+1.5L	L
Perm Defl in.	0.062 (L/1983)	5'4 1/16"	0.342 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.150 (L/820)	5'3 7/8"	0.342 (L/360)	0.440 (44%)	L	L
TL Defl inch	0.212 (L/580)	5'3 7/8"	0.512 (L/240)	0.410 (41%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



December 18, 2018

0 2010.0.0.	0.140111000 1440 24004	0			I				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-2-6	(Span) 0-10-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-2-15 to 1-6-6	(Span)3-5-15 to 3-6-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-3-14		Near Face	169 lb	Pas ^{451 lb} requ	s-Thru Frar uired at all	ning Squa	ash Block is Is over bearings
4	Point	1-6-6		Far Face	31 lb	83 lb	0 lb	0 lb	J1
5	Part. Uniform	1-11-14 to 4-7-14		Near Face	140 PLF	373 PLE Refe	er to <mark>M</mark> ultip ail for ply to	le Membe ply naili	r Connection ng or bolting
Continued on	page 2					requ	uirements		

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info Forex APA: PR-L318





Page 2 of 2



Client:

Address:

GREEN YORK HOMES Project:

12/14/2018

Designer: RCO

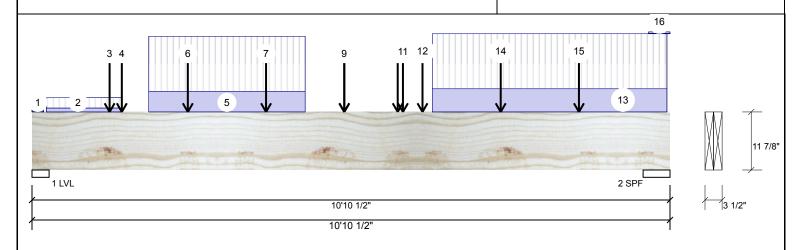
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Project #:

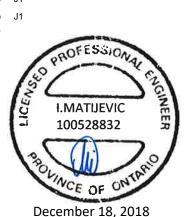
Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Second Floor



Continued	from page 1								
ID	Load Type	Location T	rib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	2-7-14		Far Face	70 lb	187 lb	0 lb	0 lb	J3
7	Point	3-11-14		Far Face	74 lb	198 lb	0 lb	0 lb	J3
8	Point	5-3-14		Far Face	63 lb	167 lb	0 lb	0 lb	J3
9	Point	5-3-14		Near Face	167 lb	435 lb	0 lb	0 lb	J8
10	Point	6-2-13		Far Face	142 lb	314 lb	0 lb	0 lb	F4
11	Point	6-3-14		Near Face	149 lb	373 lb	0 lb	0 lb	J8
12	Point	6-7-14		Far Face	17 lb	45 lb	0 lb	0 lb	J1
13	Part. Uniform	6-9-14 to 10-9-14		Near Face	157 PLF	373 PLF	0 PLF	0 PLF	
14	Point	7-11-14		Far Face	26 lb	69 lb	0 lb	0 lb	J1
15	Point	9-3-14		Far Face	24 lb	64 lb	0 lb	0 lb	J1
16	Tie-In	10-6-2 to 10-10-8 (S	Span)0-5-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				200



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

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 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- I. LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info







Client:

Project:

GREEN YORK HOMES

12/14/2018

Designer: RCO

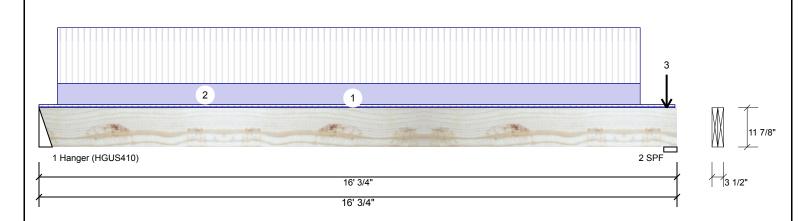
Job Name: LOT 10 (CELESTIAL 2 2)

Project #:

1.750" X 11.875" Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Address:

Level: Second Floor



Member Information				Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	2	Design Method:	LSD	1	2070	856	0	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	2190	901	0	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings and Factored Reactions					
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
				1 -	4.000"	40% 1070 / 3106	4175 L	1.25D+1.5L	
				Hanger					
Analysis Results				2 - SPF	4.000"	51% 1126 / 3285	4411 L	1.25D+1.5L	
A I i- A	atrial lass	4: All O	t. 0b 0						

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16529 ft-lb	8' 1/4"	34261 ft-lb	0.482 (48%)	1.25D+1.5L	L
Unbraced	16529 ft-lb	8' 1/4"	23053 ft-lb	0.717 (72%)	1.25D+1.5L	L
Shear	4147 lb	14'9 5/8"	11596 lb	0.358 (36%)	1.25D+1.5L	L
Perm Defl in.	0.159 (L/1171)	8' 5/16"	0.517 (L/360)	0.310 (31%)	D	Uniform
LL Defl inch	0.387 (L/482)	8' 5/16"	0.517 (L/360)	0.750 (75%)	L	L
TL Defl inch	0.546 (L/341)	8' 5/16"	0.776 (L/240)	0.700 (70%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

Self Weight



December 18, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-0-2	(Span)0-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-5-10 to 15-1-10		Far Face	99 PLF	263 PLF	0 PLF	0 PLF	
3	Point	15-9-10		Far Face	88 lb	234 lb	0 lb	0 lb	J5
	Self Weight				10 PLF	Pass-Thru Framing Squash Block is			

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info APA: PR-L318



