

NE0618-10**GREEN YORK HOMES - GRANELLI
HOME CORP - CELESTIAL 1 ELEV 2****RESPONSIBILITIES**

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

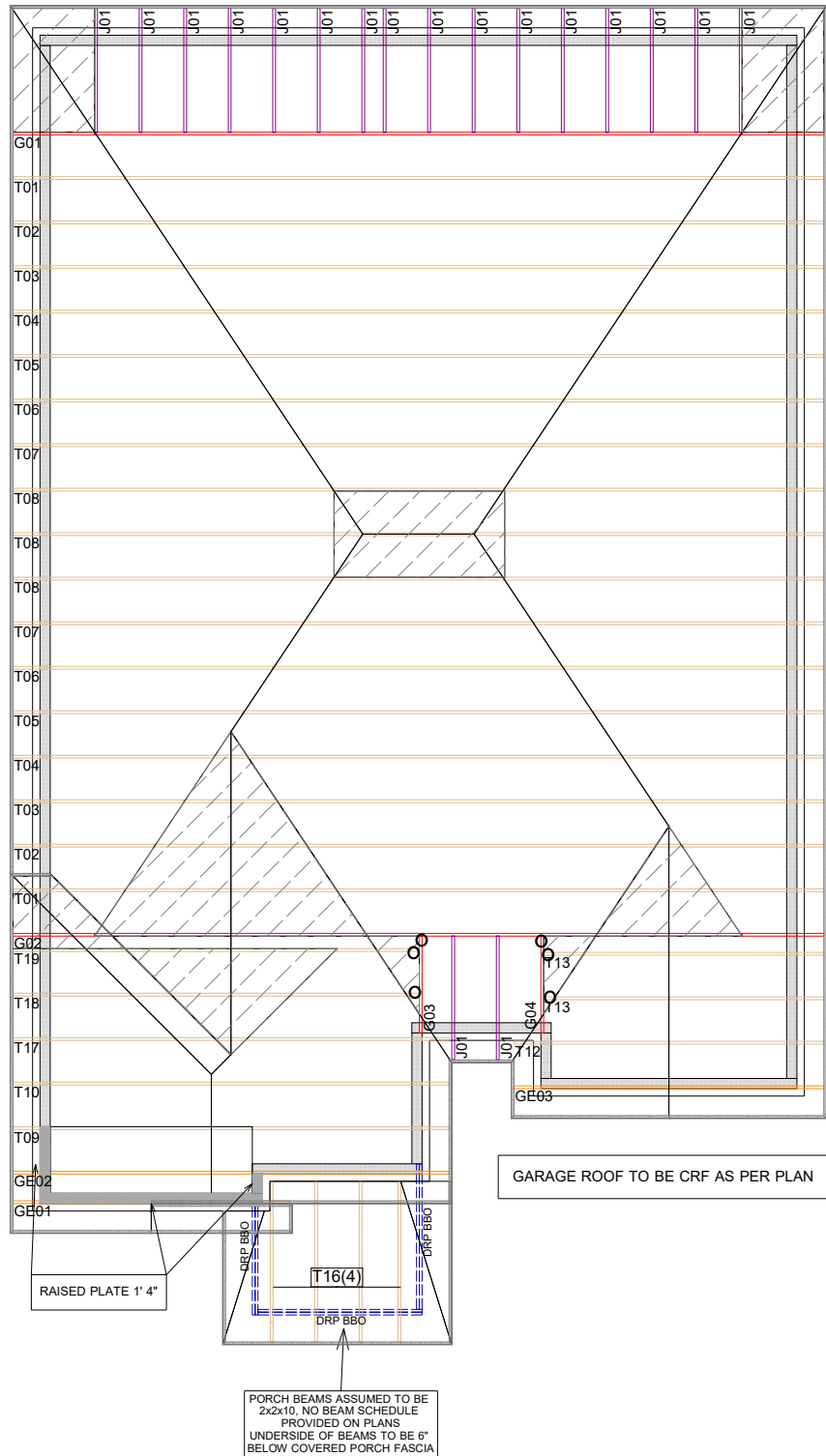
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-010



ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	CELESTIAL 1 ELE 2
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/31/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

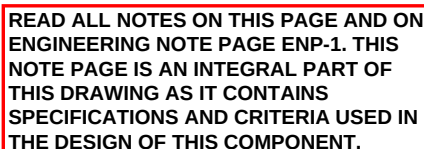
ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE TRUSS BELOW EA CROSS PT. VERT POS. THAN 6' TO HAVE A LATERAL BRACING SO THAT THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'

GREEN YORK HOMES - GRANELLI
HOME CORP - CELESTIAL 1 ELEV 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 2	DRWG NO.
NE0618-010	G01	1	1	TRUSS DESC.		PAGE 4 OF 33

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ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-SdOLRzPRqjq9lsmorFp3lk6gFpvY_G0UTEW?i9zAHuE

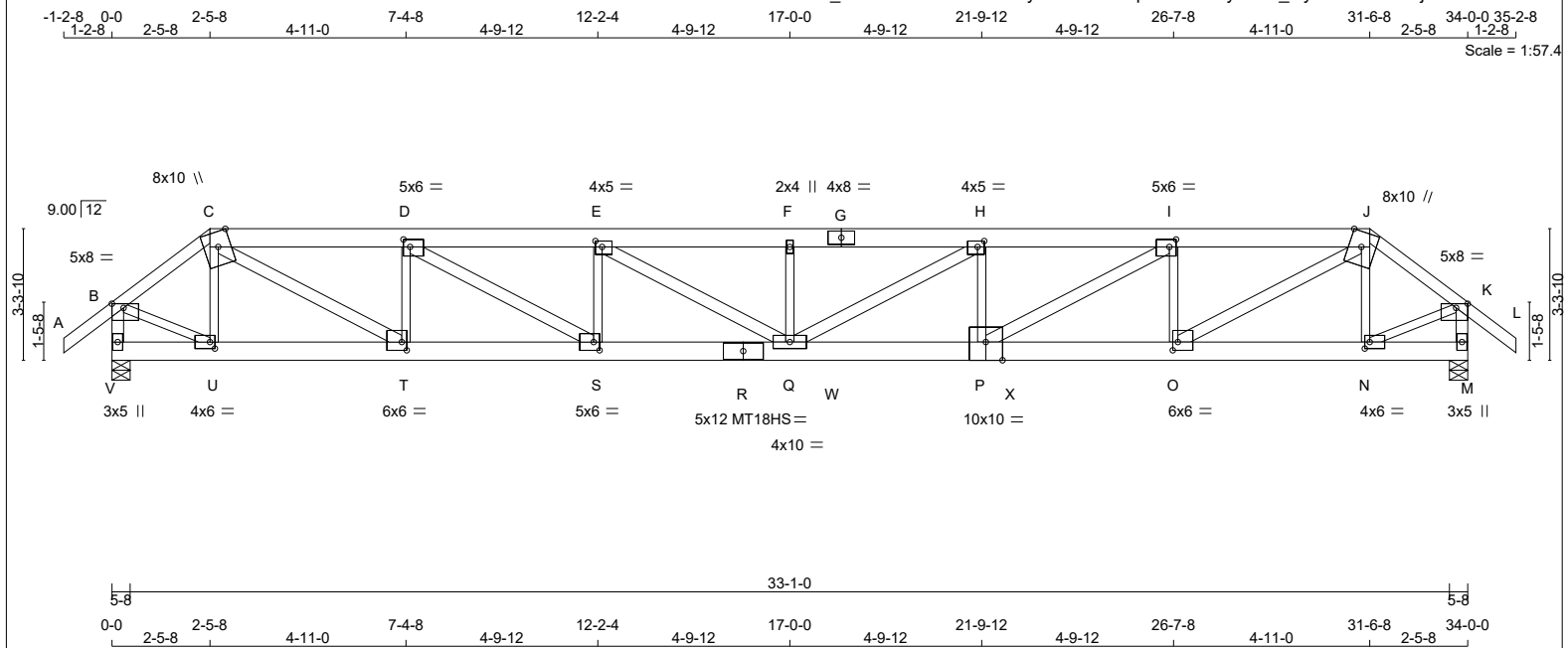
WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 90.5 lbs FACTORED DOWN AT 31-6-8,
AND 90.5 lbs FACTORED DOWN AT 2-5-8 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.94 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 179 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
V - R	2x6	DRY	2100F 1.8E	SPF
R - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - T	2x4	DRY	No.2	SPF
D - S	2x4	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	1.25 3.50
C	TTWW+m	MT20	8.0	10.0	4.50 3.75
D	TMWW-t	MT20	5.0	6.0	2.25 2.00
E	TMWW-t	MT20	4.0	5.0	1.75 2.00
F	TMW+w	MT20	2.0	4.0	
G	TS-t	MT20	4.0	8.0	
H	TMWW-t	MT20	4.0	5.0	1.75 2.00
I	TMWW-t	MT20	5.0	6.0	2.25 2.00
J	TTWW+m	MT20	8.0	10.0	4.50 3.75
K	TMVW-p	MT20	5.0	8.0	1.25 3.50
M	BMV1+p	MT20	3.0	5.0	
N	BMWW-t	MT20	4.0	6.0	2.00 1.50
O	BMWW-t	MT20	6.0	6.0	2.50 1.50
P	BSWW-t	MT20	10.0	10.0	Edge 5.00
Q	BMWWWW-t	MT20	4.0	10.0	
R	BS-t	MT18HS	5.0	12.0	
S	BMWW-t	MT20	5.0	6.0	2.50 1.75
T	BMWW-t	MT20	6.0	6.0	2.50 1.50
U	BMWW-t	MT20	4.0	6.0	2.00 1.50



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	BRG	BRG
V	2500	0	2500	0	0	0	5-8	5-8	5-8
M	2747	0	2747	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1755	1223 / 0	0 / 0	0 / 0	0 / 0	533 / 0	0 / 0
M	1930	1341 / 0	0 / 0	0 / 0	0 / 0	589 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4 0.10 (1)	10.00	U-C	-686 / 0	0.13 (1)	
B-C	-2507 / 0	-77.4 -77.4 0.17 (1)	4.18	C-T	0 / 3850	0.68 (1)	
C-D	-5316 / 0	-77.4 -77.4 0.35 (1)	3.55	T-D	-1894 / 0	0.36 (1)	
D-E	-7780 / 0	-77.4 -77.4 0.55 (1)	2.80	D-S	0 / 2863	0.51 (1)	
E-F	-9399 / 0	-77.4 -77.4 0.79 (1)	2.29	S-E	-1322 / 0	0.25 (1)	
F-G	-9399 / 0	-77.4 -77.4 0.86 (1)	2.20	E-Q	0 / 1880	0.33 (1)	
G-H	-9399 / 0	-112.8 -112.8 0.86 (1)	2.20	Q-F	-402 / 0	0.08 (1)	
H-I	-8714 / 0	-77.4 -77.4 0.67 (1)	2.53	Q-H	0 / 795	0.14 (1)	
I-J	-5977 / 0	-77.4 -77.4 0.41 (1)	3.31	P-H	-858 / 0	0.16 (1)	
J-K	-2768 / 0	-77.4 -77.4 0.19 (1)	3.99	P-I	0 / 3179	0.56 (1)	
K-L	0 / 30	-77.4 -77.4 0.10 (1)	10.00	O-I	-2046 / 0	0.39 (1)	
V-B	-2510 / 0	0.0 0.0 0.28 (1)	5.35	O-J	0 / 4378	0.77 (1)	
M-K	-2751 / 0	0.0 0.0 0.31 (1)	5.13	N-J	-798 / 0	0.15 (1)	
				B-U	0 / 2176	0.54 (1)	
				N-K	0 / 2403	0.59 (1)	
V-U	0 / 0	-18.2 -18.2 0.03 (1)	10.00				
U-T	0 / 1990	-18.2 -18.2 0.14 (1)	10.00				
T-S	0 / 5316	-18.2 -18.2 0.33 (1)	10.00				
S-R	0 / 7780	-18.2 -18.2 0.51 (1)	10.00				
R-Q	0 / 7780	-18.2 -18.2 0.51 (1)	10.00				
Q-W	0 / 8714	-18.2 -18.2 0.56 (1)	10.00				
W-P	0 / 8714	-26.9 -26.9 0.56 (1)	10.00				
P-X	0 / 5977	-18.2 -18.2 0.47 (1)	10.00				
X-O	0 / 5977	-18.2 -18.2 0.47 (1)	10.00				
O-N	0 / 2195	-18.2 -18.2 0.14 (1)	10.00				
N-M	0 / 0	-18.2 -18.2 0.03 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	TYPE
Q	17-1-4	-966	-966	---	FRONT VERT
X	22-6-12	-672	-672	---	FRONT VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 883 (0.46")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 494 (0.83")

CSI: TC=0.86/1.00 (F-H:1), BC=0.56/1.00 (P-Q:1), WB=0.77/1.00 (J-O:1), SSI=0.42/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)	(PLI)		
MAX MIN	MAX MIN	MAX MIN	MAX MIN		
MT20	618 354	1667 822	2284 1656		
MT18HS	580 354	2729 1405	4191 2010		

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-010	G02	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 2	PAGE 6 OF 33
TRUSS DESC.					

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PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
V BMV1+p MT20 3.0 5.0

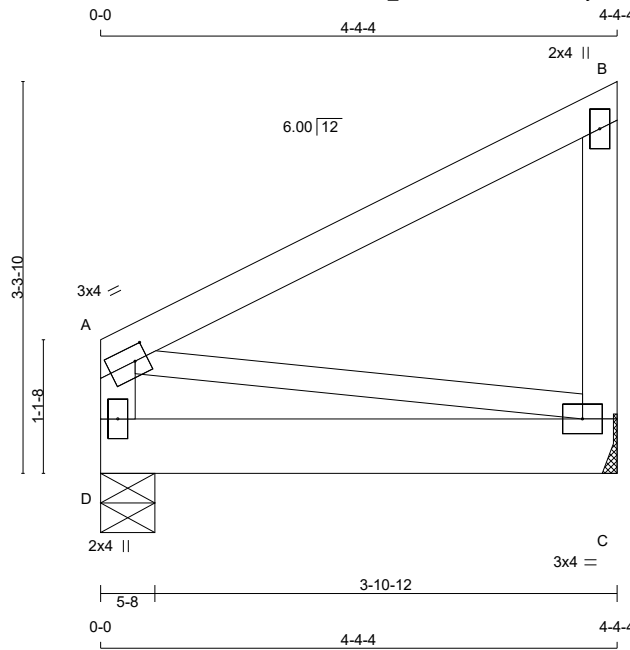
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 966.4 lbs FACTORED DOWN AT 17'-1-4,
AND 671.8 lbs FACTORED DOWN AT 22'-6-12 ON
BOTTOM CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.97 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
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SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.25
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	967	0	967	0	MECHANICAL	
D	967	0	967	0	5-8	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	679	471 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0
D	679	471 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX		MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED	
			FROM	TO			CS1 (LC)	CS1 (LC)
FR-TO								
A-B	0 / 0	-77.4	-77.4	0.27 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-168 / 0	0.0	0.0	0.03 (1)	7.81			
D-A	-168 / 0	0.0	0.0	0.02 (1)	7.81			
D-C	0 / 0	-366.8	-366.8	0.64 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 17-0-8
END DISTANCE = 4-4-4
END SPAN CARRIED = 17-0-8
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/586 (0.09")

CSI: TC=0.27/1.00 (A-B:1), BC=0.64/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.53/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



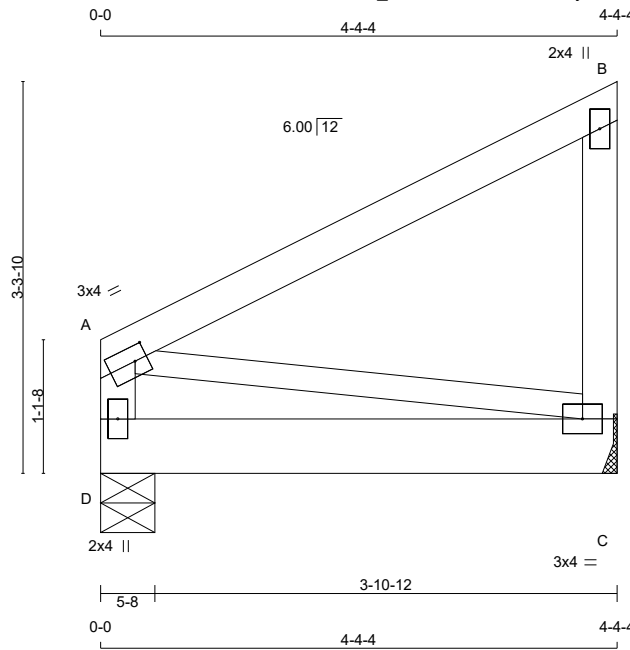
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-010	G03	1	1	GREEN YORK HOMES - GRANELL HOME CORP - CELESTIAL 1 ELEV 2	PAGE 8 OF 33
TRUSS DESC.					

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ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-P?V6sfQhML4t_AwBygrXq9B4odfiSNenwY?6n1zAHuC

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
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NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:19.4

TOTAL WEIGHT = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.25
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	672	0	672	0	0	MECHANICAL
D	672	0	672	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	472	327 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
D	472	327 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO				
A-B	0 / 0	-77.4	-77.4	0.27 (1)	10.00	A-C	0 / 0		0.00 (1)
C-B	-168 / 0	0.0	0.0	0.03 (1)	7.81				
D-A	-168 / 0	0.0	0.0	0.02 (1)	7.81				
D-C	0 / 0	-231.4	-231.4	0.40 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 11-4-8
END DISTANCE = 4-4-4
END SPAN CARRIED = 11-4-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/931 (0.06")

CSI: TC=0.27/1.00 (A-B:1), BC=0.40/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.33/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

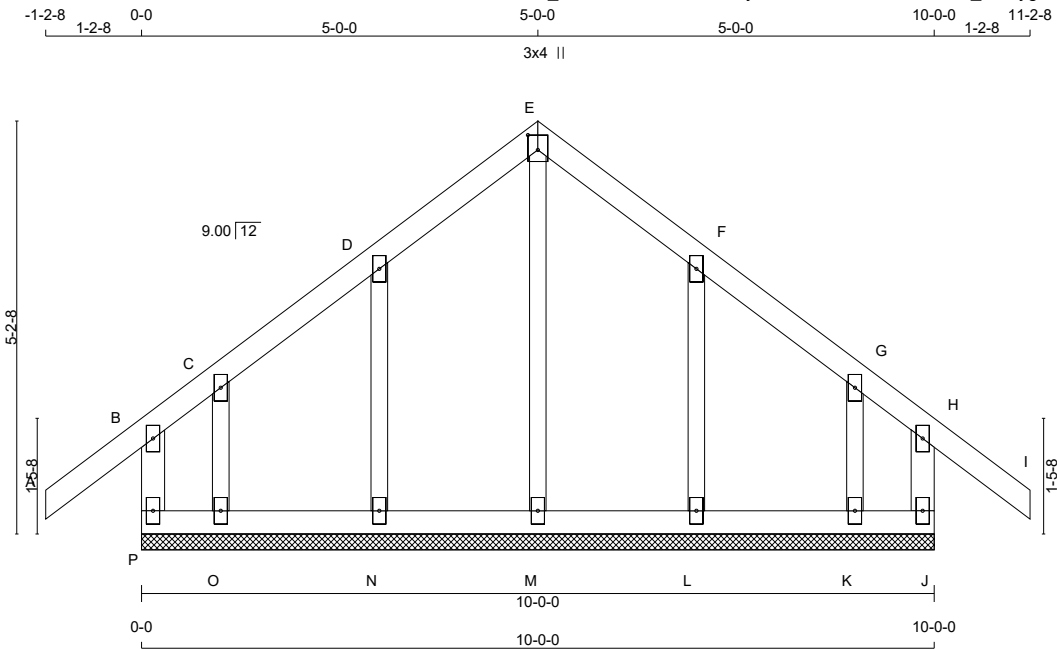


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-010	G04	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 2	PAGE 10 OF 33
Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:52:17 2018 Page 2					
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-P?V6sfQhML4t_AwBygrXq9B4odiOSNenwY?6n1zAHuC					

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:29.1

TOTAL WEIGHT = 43 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0.0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, F, G						
E	TTW+p	MT20	2.0	4.0	2.25	1.50
H	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMV1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
P-B	-158 / 0	0.0	0.0	0.03 (1)	7.81	M-E	-189 / 0	0.07 (1)
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-D	-167 / 0	0.04 (1)
B-C	-1 / 0	-77.4	-77.4	0.06 (1)	10.00	O-C	-68 / 0	0.01 (1)
C-D	0 / 28	-77.4	-77.4	0.05 (1)	10.00	L-F	-167 / 0	0.04 (1)
D-E	0 / 26	-77.4	-77.4	0.05 (1)	10.00	K-G	-68 / 0	0.01 (1)
E-F	0 / 26	-77.4	-77.4	0.05 (1)	10.00			
F-G	0 / 28	-77.4	-77.4	0.05 (1)	10.00			
G-H	-1 / 0	-77.4	-77.4	0.06 (1)	10.00			
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
J-H	-158 / 0	0.0	0.0	0.03 (1)	7.81			
P-O	-15 / 0	-18.2	-18.2	0.01 (4)	6.25			
O-N	-18 / 0	-18.2	-18.2	0.02 (4)	6.25			
N-M	-23 / 0	-18.2	-18.2	0.02 (4)	6.25			
M-L	-23 / 0	-18.2	-18.2	0.02 (4)	6.25			
L-K	-18 / 0	-18.2	-18.2	0.02 (4)	6.25			
K-J	-15 / 0	-18.2	-18.2	0.01 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (H-I:1), BC=0.02/1.00 (L-M:4), WB=0.07/1.00 (E-M:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

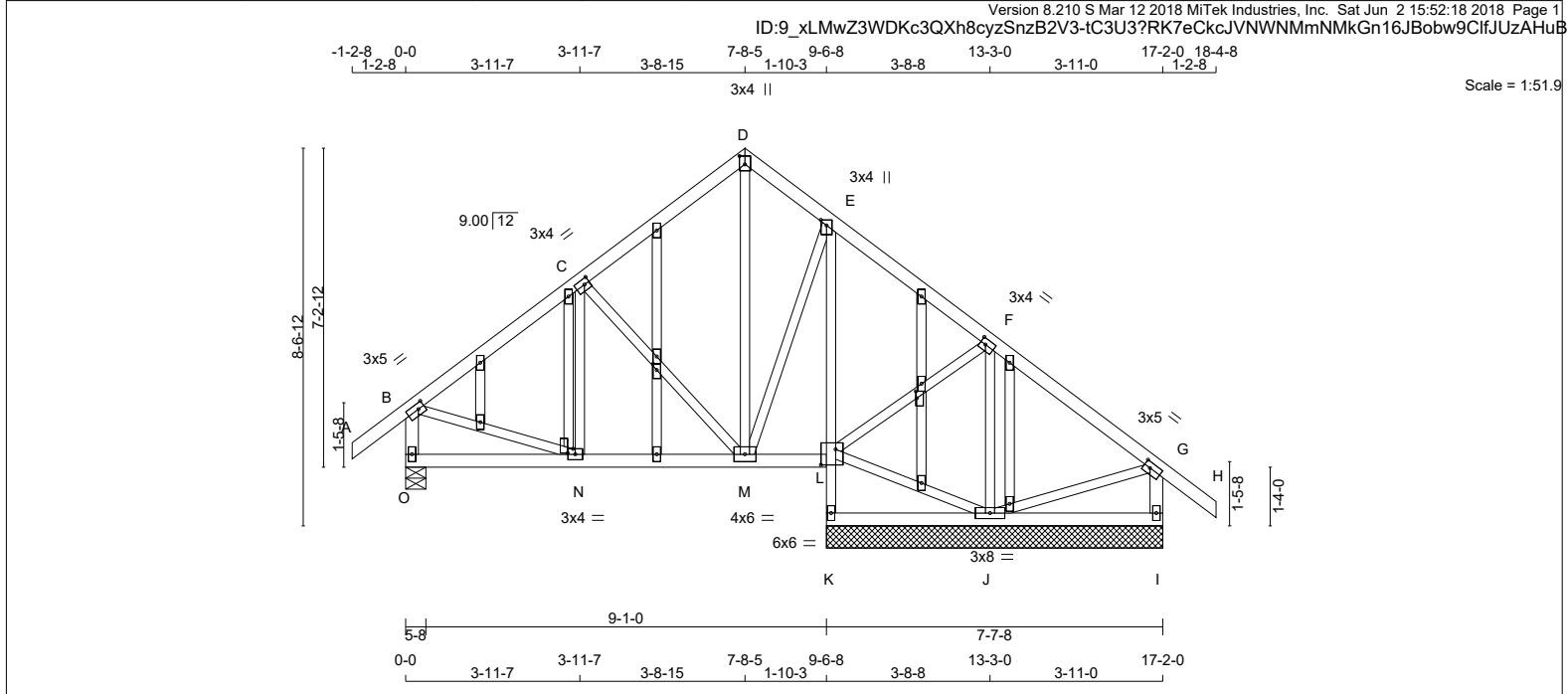
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 103 lb [M]

N. L. G. A. RULES				BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS SIZE LUMBER DESCR.				BEARINGS				SPECIFIED LOADS:			
A - D 2x4 DRY No.2 SPF				FACTORED MAXIMUM FACTORED INPUT REQRD				TOP CH. LL = 23.3 PSF			
D - H 2x4 DRY No.2 SPF				GROSS REACTION BRG BRG				DL = 3.0 PSF			
O - B 2x4 DRY No.2 SPF				JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX				BOT CH. LL = 0.0 PSF			
I - G 2x4 DRY No.2 SPF				O 601 0 601 0 0 5-8 5-8				DL = 7.3 PSF			
O - L 2x4 DRY No.2 SPF				K 531 0 531 0 0 7-7-8 7-7-8				TOTAL LOAD = 33.6 PSF			
K - E 2x3 DRY No.2 SPF				I 326 0 326 0 0 7-7-8 7-7-8				SPACING = 24.0 IN. C/C			
K - I 2x4 DRY No.2 SPF				J 384 0 384 0 0 7-7-8 7-7-8				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
ALL WEBS 2x3 DRY No.2 SPF				UNFACTORED REACTIONS				THIS DESIGN COMPLIES WITH:			
EXCEPT				1ST LCASE MAX./MIN. COMPONENT REACTIONS				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
ALL GABLE WEBS 2x3 DRY No.2 SPF				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL				- CSA 086-09			
DRY: SEASONED LUMBER.				O 419 305 / 0 0 / 0 0 / 0 0 / 0 115 / 0 0 / 0				- TPIC 2011			
GABLE STUDS SPACED AT 2'-0" OC.				K 372 265 / 0 0 / 0 0 / 0 0 / 0 107 / 0 0 / 0				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				I 225 178 / 0 0 / 0 0 / 0 0 / 0 46 / 0 0 / 0				ALLOWABLE DEFL.(LL)= L/360 (0.32")			
				J 273 172 / 0 0 / 0 0 / 0 0 / 0 101 / 0 0 / 0				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")			
				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, K, I, J				ALLOWABLE DEFL.(TL)= L/360 (0.32")			
				BRACING				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.							
				MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.							
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.							
PLATES (table is in inches)				LOADING				CSi: TC=0.19/1.00 (B-C:1) , BC=0.17/1.00 (E-L:1) , WB=0.15/1.00 (C-M:1) , SSI=0.12/1.00 (B-C:1)			
JT TYPE PLATES W LEN Y X				TOTAL LOAD CASES: (4)				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
B TMVW-t MT20 3.0 5.0 1.50 1.75				CHORDS				COMP=1.10 SHEAR=1.10 TENS= 1.10			
C TMWW-t MT20 3.0 4.0 1.50 1.50				MAX. FACTORED				COMPANION LIVE LOAD FACTOR = 0.50			
D TTW+p MT20 3.0 4.0 2.25 1.50				MEMB. FORCE (LBS)				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
E TMVW+p MT20 3.0 4.0 1.50 1.50				FACTORED				NAIL VALUES			
F TMWW-t MT20 3.0 4.0 1.50 1.50				VERT. LOAD LC1 MAX UNBRAC				PLATE GRIP(DRY) SHEAR SECTION			
G TMVW-t MT20 3.0 5.0 1.50 1.75				FR-TO FROM TO LENGTH FR-TO				(PSI) (PLI) (PLI)			
I, K, O				A- B 0 / 30 -77.4 -77.4 0.09 (1) 10.00 N- C -16 / 63 0.02 (4)				MAX MIN MAX MIN MAX MIN			
I BMV1+p MT20 2.0 4.0				B- C -405 / 0 -77.4 -77.4 0.19 (1) 6.25 C- M -285 / 0 0.15 (1)				MT20 618 354 1667 822 2284 1656			
J BMWW1-t MT20 3.0 8.0				C- D -208 / 0 -77.4 -77.4 0.19 (1) 6.25 M- D 0 / 67 0.02 (4)				PLATE PLACEMENT TOL. = 0.250 inches			
J, P, Q, R, S, T, U, V, W, X, Y				D- E -202 / 0 -77.4 -77.4 0.07 (1) 6.25 M- E 0 / 197 0.04 (1)				PLATE ROTATION TOL. = 5.0 Deg.			
L NP+w MT20 2.0 4.0				E- F -115 / 0 -77.4 -77.4 0.17 (1) 6.25 L- J 0 / 82 0.02 (1)				JSI GRIP= 0.89 (F) (INPUT = 0.90)			
L BVMWW-I MT20 6.0 6.0 4.25 4.00				F- G -75 / 0 -77.4 -77.4 0.17 (1) 6.25 L- F 0 / 12 0.00 (4)				JSI METAL= 0.17 (B) (INPUT = 1.00)			
M BMWW-t MT20 4.0 6.0				G- H 0 / 30 -77.4 -77.4 0.09 (1) 10.00 J- F -352 / 0 0.10 (1)							
N BMWW-t MT20 3.0 4.0				O- B -571 / 0 0.0 0.0 0.06 (1) 7.81 B- N 0 / 354 0.08 (1)							
N NP+w MT20 2.0 4.0 1.25 1.50				I- G -298 / 0 0.0 0.0 0.03 (1) 7.81 J- G 0 / 79 0.02 (1)							
P NP+w MT20 2.0 4.0 1.75 1.00				O- N 0 / 0 -18.2 -18.2 0.07 (4) 10.00							
X NP+w MT20 2.0 4.0 1.75 0.25				N- M 0 / 341 -18.2 -18.2 0.09 (4) 10.00							
				M- L 0 / 82 -18.2 -18.2 0.03 (4) 10.00							
				K- L -507 / 0 0.0 0.0 0.01 (1) 7.81							
				L- E -458 / 0 0.0 0.0 0.17 (1) 7.81							
				K- J 0 / 0 -18.2 -18.2 0.08 (4) 10.00							
				J- I 0 / 0 -18.2 -18.2 0.08 (4) 10.00							

LICENSED PROFESSIONAL ENGINEER

N.A. EL-MASRI

N.A. El-Masri

PROVINCIAL OF ONTARIO

Jun 02, 2018

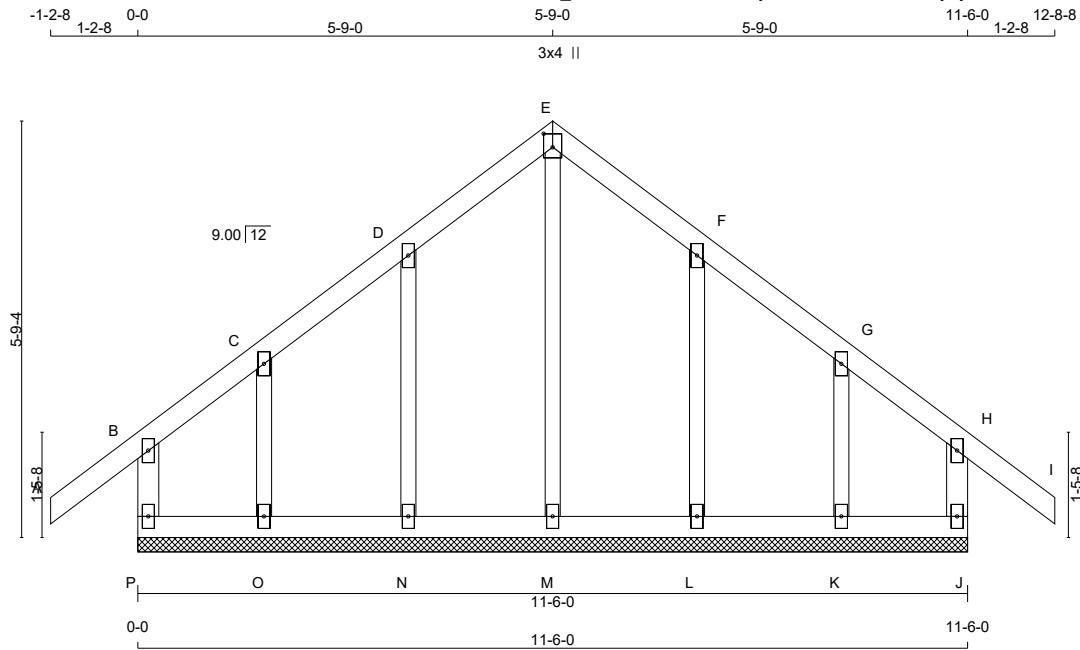
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale: 3/8"=1'

TOTAL WEIGHT = 50 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, F, G					
C	TMW+w	MT20	2.0	4.0	
E	TTW+p	MT20	3.0	4.0	2.25 1.50
H	TMV+p	MT20	2.0	4.0	
J	BMV1+p	MT20	2.0	4.0	
K, L, M, N, O					
K	BMW1+w	MT20	2.0	4.0	
P	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)
FR-TO		FROM TO			FR-TO				
P-B	-172 / 0	0.0	0.0	0.03 (1)	7.81	M-E	-187 / 0		0.09 (1)
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-D	-165 / 0		0.05 (1)
B-C	-2 / 5	-77.4	-77.4	0.06 (1)	10.00	O-C	-114 / 0		0.02 (1)
C-D	0 / 25	-77.4	-77.4	0.05 (1)	10.00	L-F	-165 / 0		0.05 (1)
D-E	0 / 24	-77.4	-77.4	0.05 (1)	10.00	K-G	-114 / 0		0.02 (1)
E-F	0 / 24	-77.4	-77.4	0.05 (1)	10.00				
F-G	0 / 25	-77.4	-77.4	0.05 (1)	10.00				
G-H	-2 / 5	-77.4	-77.4	0.06 (1)	10.00				
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00				
J-H	-172 / 0	0.0	0.0	0.03 (1)	7.81				
P-O	-13 / 0	-18.2	-18.2	0.01 (4)	6.25				
O-N	-17 / 0	-18.2	-18.2	0.01 (4)	6.25				
N-M	-21 / 0	-18.2	-18.2	0.01 (4)	6.25				
M-L	-21 / 0	-18.2	-18.2	0.01 (4)	6.25				
L-K	-17 / 0	-18.2	-18.2	0.01 (4)	6.25				
K-J	-13 / 0	-18.2	-18.2	0.01 (4)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1), BC=0.01/1.00 (N-O:4), WB=0.09/1.00 (E-M:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

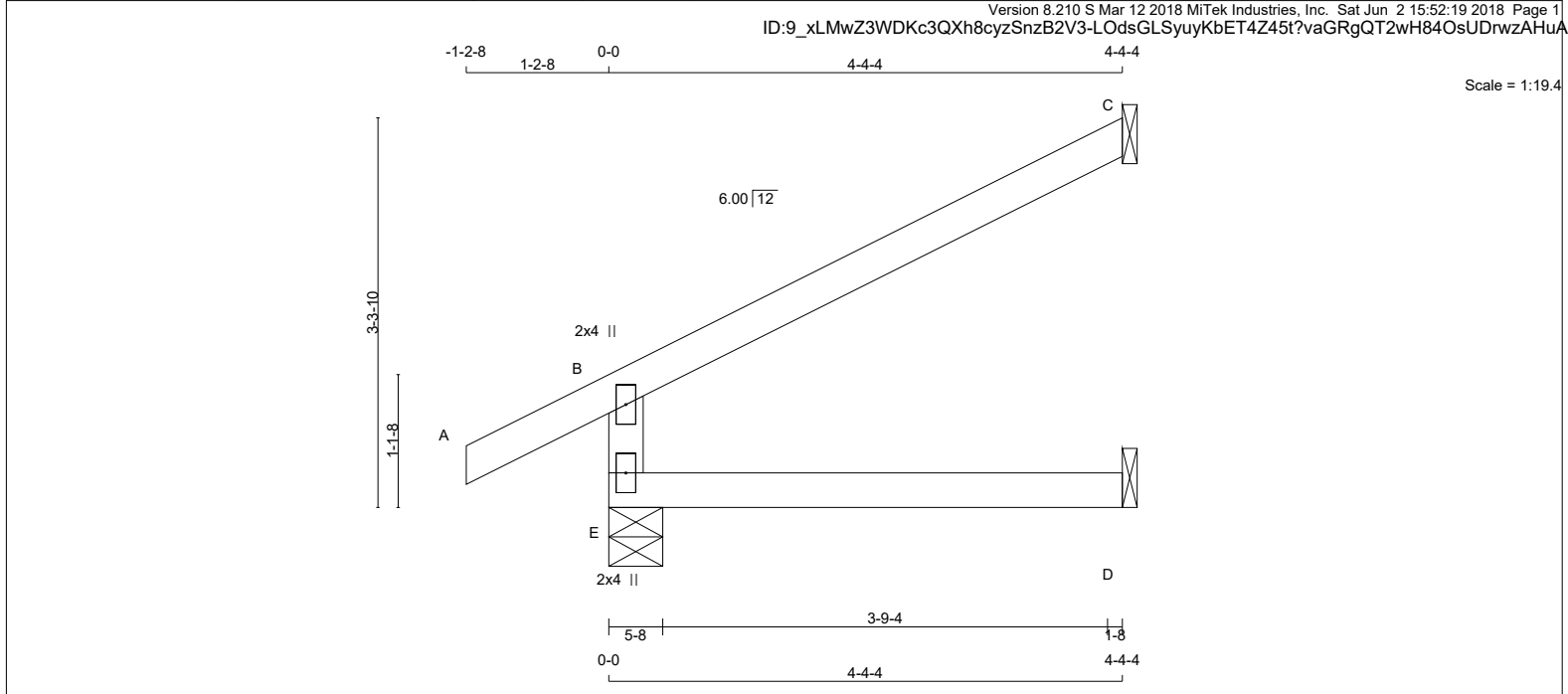
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

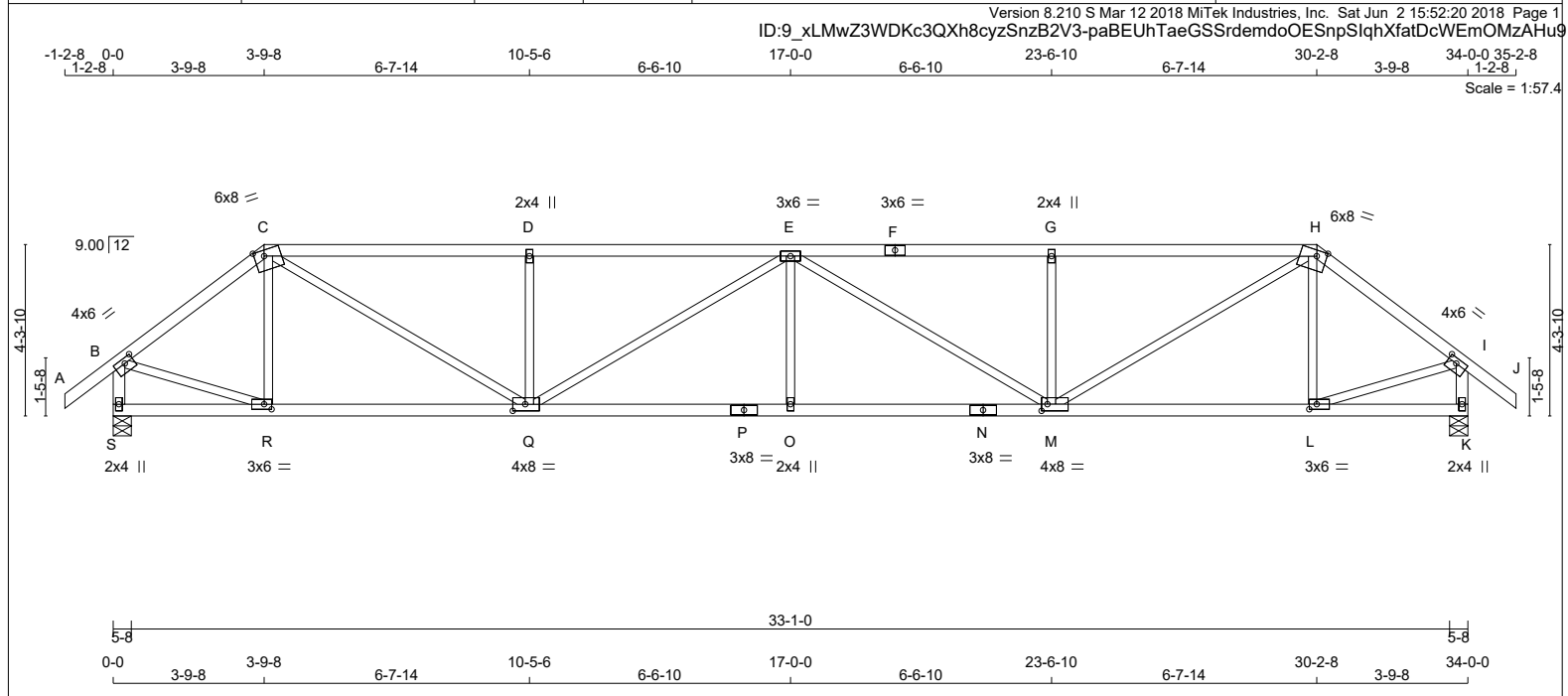




										TOTAL WEIGHT = 18 X 13 = 233 lb										[M]																																												
LUMBER N. L. G. A. RULES CHORDS SIZE E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				LUMBER				DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td></td><td colspan="2">FACTORED GROSS REACTION</td><td colspan="2">MAXIMUM FACTORED GROSS REACTION</td><td>INPUT BRG</td><td>REQRD BRG</td></tr><tr><td>JT</td><td>VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td><td>UPLIFT</td><td>IN-SX</td><td>IN-SX</td></tr><tr><td>E</td><td>355</td><td>0</td><td>355</td><td>0</td><td>0</td><td>5-8</td><td>5-8</td></tr><tr><td>C</td><td>126</td><td>0</td><td>126</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>33</td><td>0</td><td>37</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr></table>											FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	E	355	0	355	0	0	5-8	5-8	C	126	0	126	0	0	1-8	1-8	D	33	0	37	0	0	1-8	1-8	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF			
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG																																																										
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX																																																									
E	355	0	355	0	0	5-8	5-8																																																									
C	126	0	126	0	0	1-8	1-8																																																									
D	33	0	37	0	0	1-8	1-8																																																									

PLATES (table is in inches)							SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D							SPACING = 24.0 IN. C/C		
JT	TYPE	PLATES	W	LEN	Y	X	UNFACTORED REACTIONS							THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010		
B	TMV+p	MT20	2.0	4.0			JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS			THIS DESIGN COMPLIES WITH:				
E	BMV1+p	MT20	2.0	4.0			C	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014	
							E	247	186 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0	- CSA 086-09	
							C	86	76 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0	- TPIC 2011	
							D	27	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0		
							BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C							DESIGN ASSUMPTIONS		
							BRACING							-OVERHANG NOT TO BE ALTERED OR CUT OFF.		
							TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.							(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD		
							MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.							ALLOWABLE DEFL.(LL)= L/360 (0.19")		
							ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.							CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")		
							LOADING							ALLOWABLE DEFL.(TL)= L/360 (0.19")		
							TOTAL LOAD CASES: (4)							CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")		
							CHORDS							WEBS		
							MAX. FACTORED FORCE (LBS)							MAX. FACTORED FORCE (LBS)		
							VERT. LOAD (PLF)							MAX. UNBRACED LENGTH		
							LC1 MAX CSI (LC)							MEMB. FR-TO		
							FROM TO									
							E-B							7.81		
							A-B							10.00		
							B-C							6.25		
							E-D							10.00		

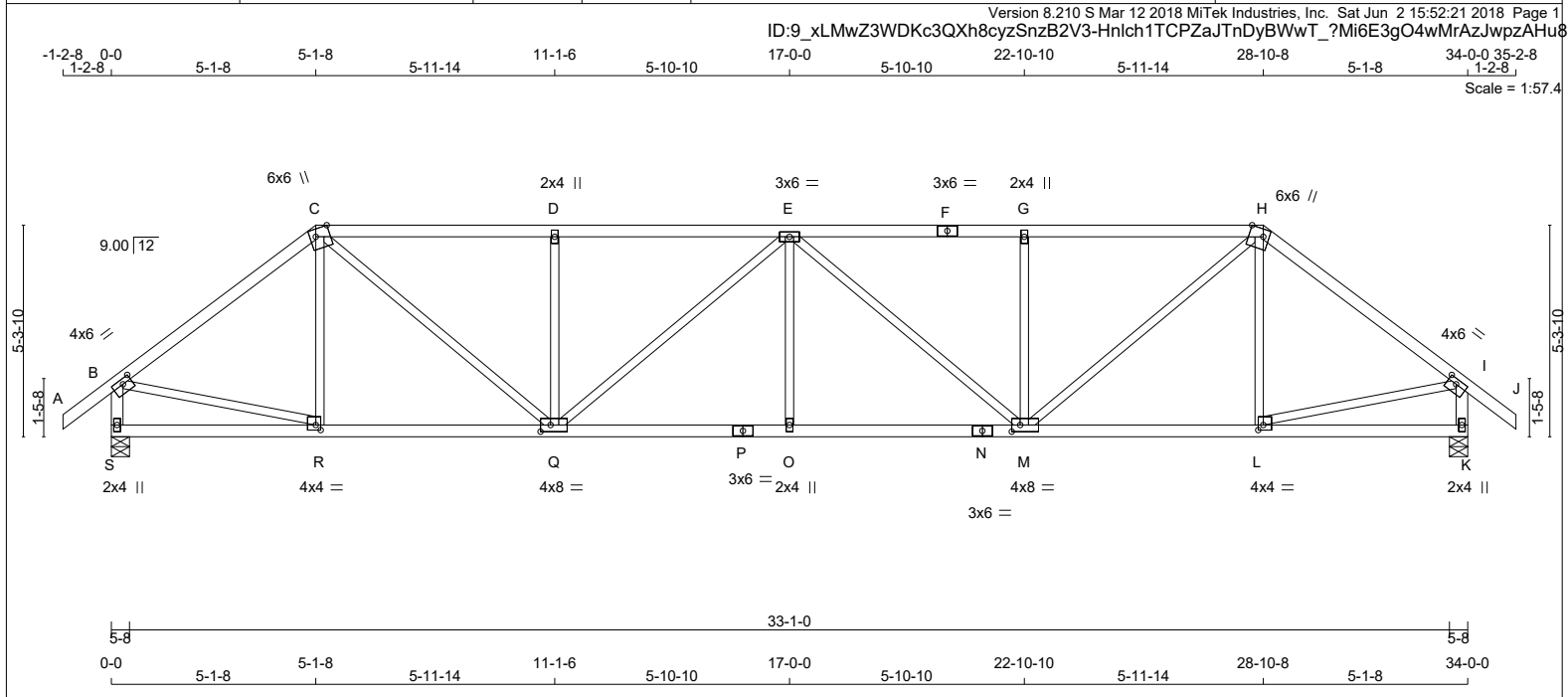
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



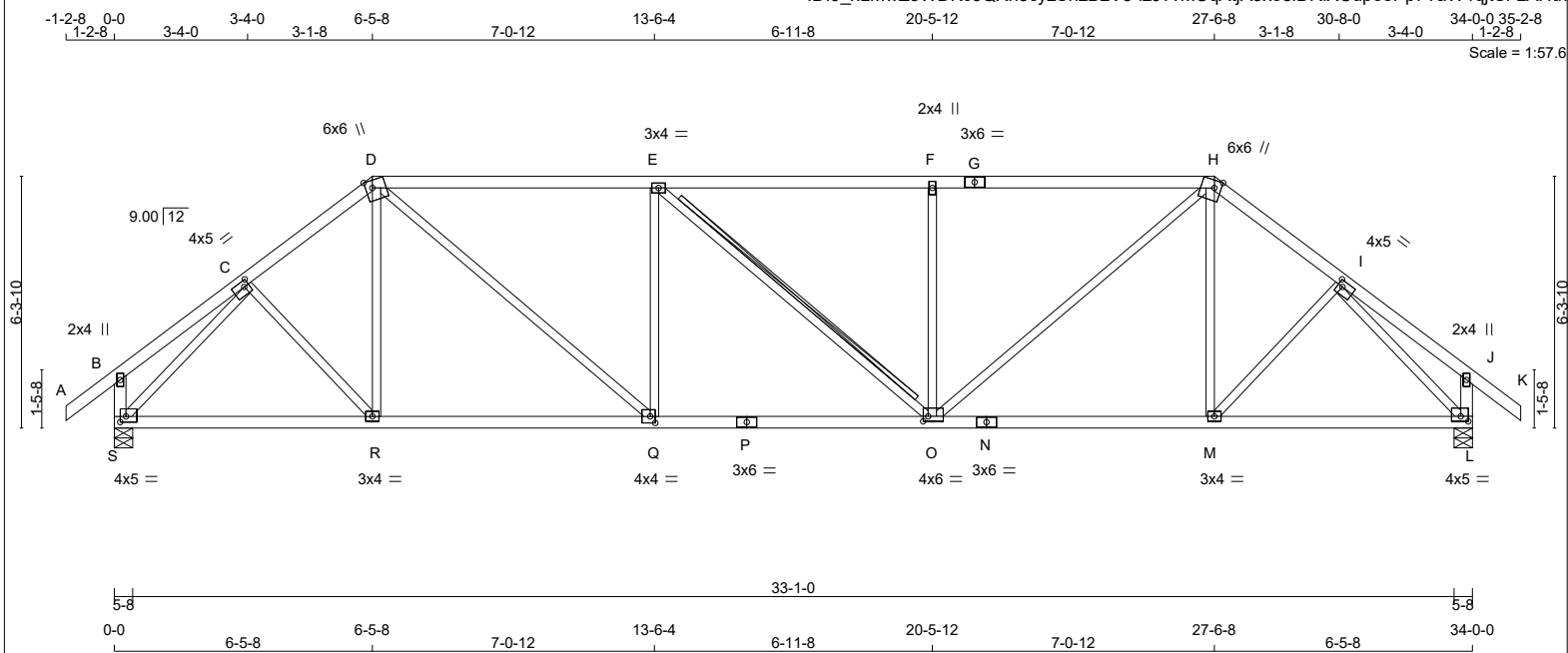
LUMBER N. L. G. A. RULES					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA	
CHORDS SIZE LUMBER DESCR.					BEARINGS										SPECIFIED LOADS:	
A - C	2x4	DRY	No.2	SPF	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		TOP CH. LL = 23.3 PSF			
C - F	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 3.0 PSF			
F - H	2x4	DRY	No.2	SPF	S	1726	0	1726	0	0	5-8	5-8	BOT CH. LL = 0.0 PSF			
H - J	2x4	DRY	No.2	SPF	K	1726	0	1726	0	0	5-8	5-8	DL = 7.3 PSF			
S - B	2x4	DRY	No.2	SPF											TOTAL LOAD = 33.6 PSF	
K - I	2x4	DRY	No.2	SPF											SPACING = 24.0 IN. C/C	
S - P	2x4	DRY	No.2	SPF											LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM	
P - N	2x4	DRY	No.2	SPF											THIS TRUSS IS DESIGNED FOR RESIDENTIAL	
N - K	2x4	DRY	No.2	SPF												
ALL WEBS 2x3 DRY No.2 SPF					UNFACTORED REACTIONS											
EXCEPT					1ST LCASE MAX./MIN. COMPONENT REACTIONS											
					JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
					S	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0				
					K	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0				
DRY: SEASONED LUMBER.					BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K											

PLATES (table is in inches)							BRACING							OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010													
JT	TYPE	PLATES	W	LEN	Y	X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.														THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011						
B	TMVW-t	MT20	4.0	6.0	1.50	2.75	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.														(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD						
C	TTWW-m	MT20	6.0	8.0	1.75	3.00	LOADING TOTAL LOAD CASES: (4)														ALLOWABLE DEFL.(LL)= L/360 (1.13") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23") ALLOWABLE DEFL.(TL)= L/360 (1.13") CALCULATED VERT. DEFL.(TL) = L/ 990 (0.41")						
D	TMW+w	MT20	2.0	4.0			C H O R D S														W E B S						
E	TMWWW-t	MT20	3.0	6.0			MEMB.														MAX. FACTORED						
F	TS-t	MT20	3.0	6.0			FORCE														MAX. FACTORED						
G	TMW+w	MT20	2.0	4.0			(LBS)														(LBS)						
H	TTWW-m	MT20	6.0	8.0	1.75	3.00	VERT. LOAD														MAX. FACTORED						
I	TMVW-t	MT20	4.0	6.0	1.50	2.75	LC1 (PLF)														MAX. UNBRACED						
K	BMV1+p	MT20	2.0	4.0			CS1 (LC)														LENGTH						
L	BMWW-t	MT20	3.0	6.0	1.50	2.25	FR-TO														FR-TO						
M	BMWWW-t	MT20	4.0	8.0	2.00	1.75	FROM														TO						
N	BS-t	MT20	3.0	8.0			A-B														R-C						
O	BMW+w	MT20	2.0	4.0			0 / 30														-293 / 0						
P	BS-t	MT20	3.0	8.0			-77.4														C-Q						
Q	BMWWW-t	MT20	4.0	8.0	2.00	3.75	-1748 / 0														0 / 1916						
R	BMWW-t	MT20	3.0	6.0	1.50	2.25	-3030 / 0														Q-D						
S	BMV1+p	MT20	2.0	4.0			-77.4														-573 / 0						
							-3031 / 0														Q-E						
							-77.4														-565 / 0						
							-77.4														O-E						
							-77.4														0 / 132						
							-77.4														E-M						
							-77.4														-565 / 0						
							-77.4														M-G						
							-77.4														-573 / 0						
							-1748 / 0														M-H						
							0 / 30														0 / 1916						
							S-B														L-H						
							-1705 / 0														-293 / 0						
							K-I														B-R						
							0.0														0 / 1453						
							0.0														L-I						
							0.0														0 / 1453						
							0 / 0														-18.2						
							R-Q														-18.2						
							0 / 1388														-18.2						
							Q-P														-18.2						
							0 / 3512														-18.2						
							0.13 (4)														10.00						
							0.30 (1)														10.00						
							0.63 (1)														10.00						

[illegible]



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TOTAL WEIGHT = 2 X 142 = 285 lb

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
--------------------	-----	-----	------	-----

DRY: SEASONED LUMBER.

JT TYPE	PLATES
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
20	20
21	21
22	22
23	23
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84	84
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86	86
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88	88
89	89
90	90
91	91
92	92
93	93
94	94
95	95
96	96
97	97
98	98
99	99
100	100

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TMV+p	MT20	2.0	4.0		
C	T	TMVW-t	MT20	4.0	5.0	1.75	1.50
D	T	TTWW+m	MT20	6.0	6.0	Edge	2.00
E	T	TMVW-t	MT20	3.0	4.0		
F	T	TMV+w	MT20	2.0	4.0		
G	T	TS-t	MT20	3.0	6.0		
H	T	TTWW+m	MT20	6.0	6.0	Edge	2.00
I	T	TMVW-t	MT20	4.0	5.0	1.75	1.50
J	T	TMV+p	MT20	2.0	4.0		
L	B	BMVW-1-t	MT20	4.0	5.0	1.50	2.25
M	B	BMVW-t	MT20	3.0	4.0		
N	B	BS-t	MT20	3.0	6.0		
O	B	BMVWVW-t	MT20	4.0	6.0	1.50	1.50
P	B	BS-t	MT20	3.0	6.0		
Q	B	BMVW-t	MT20	4.0	4.0	2.00	1.50
R	B	BMVW-t	MT20	3.0	4.0		
S	B	BMVW-1-t	MT20	4.0	5.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACT

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1726	0	1726	0	0	5-8	5-8
L	1726	0	1726	0	0	5-8	5-8

1ST LCASE		MAX	
1	1	1	1
2	2	2	2
3	3	3	3
4	4	4	4
5	5	5	5
6	6	6	6
7	7	7	7
8	8	8	8
9	9	9	9
10	10	10	10
11	11	11	11
12	12	12	12
13	13	13	13
14	14	14	14
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90	90	90	90
91	91	91	91
92	92	92	92
93	93	93	93
94	94	94	94
95	95	95	95
96	96	96	96
97	97	97	97
98	98	98	98
99	99	99	99
100	100	100	100

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
L	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S. L

BRACING
TOP CHOICE

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LC

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (IN)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	C-R	0 / 134	0.03 (1)	
B-C	0 / 14	-77.4	-77.4	0.11 (1)	10.00	D-R	0 / 94	0.03 (4)	
C-D	-1811 / 0	-77.4	-77.4	0.15 (1)	4.85	D-Q	0 / 1103	0.25 (1)	
D-E	-2275 / 0	-77.4	-77.4	0.77 (1)	3.75	Q-E	-589 / 0	0.37 (1)	
E-F	-2273 / 0	-77.4	-77.4	0.76 (1)	3.75	E-O	-2 / 0	0.00 (1)	
F-G	-2273 / 0	-77.4	-77.4	0.76 (1)	3.77	O-F	-587 / 0	0.37 (1)	
G-H	-2273 / 0	-77.4	-77.4	0.76 (1)	3.77	H-O	0 / 1100	0.25 (1)	
H-I	-1811 / 0	-77.4	-77.4	0.15 (1)	4.85	M-H	0 / 94	0.03 (4)	
I-J	0 / 14	-77.4	-77.4	0.11 (1)	10.00	M-I	0 / 134	0.03 (1)	
J-K	0 / 30	-77.4	-77.4	0.09 (1)	10.00	S-C	-1995 / 0	0.72 (1)	
S-B	-200 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-1996 / 0	0.72 (1)	
L-J	-200 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 1341	-18.2	-18.2	0.33 (1)	10.00				
R-Q	0 / 1434	-18.2	-18.2	0.34 (1)	10.00				
Q-P	0 / 2275	-18.2	-18.2	0.45 (1)	10.00				
P-O	0 / 2275	-18.2	-18.2	0.45 (1)	10.00				
O-N	0 / 1434	-18.2	-18.2	0.34 (1)	10.00				
N-M	0 / 1434	-18.2	-18.2	0.34 (1)	10.00				
M-L	0 / 1341	-18.2	-18.2	0.33 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.77/1.00 (D-E:1) , BC=0.45/1.00 (O-Q:1) ,
WB=0.72/1.00 (I-L:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

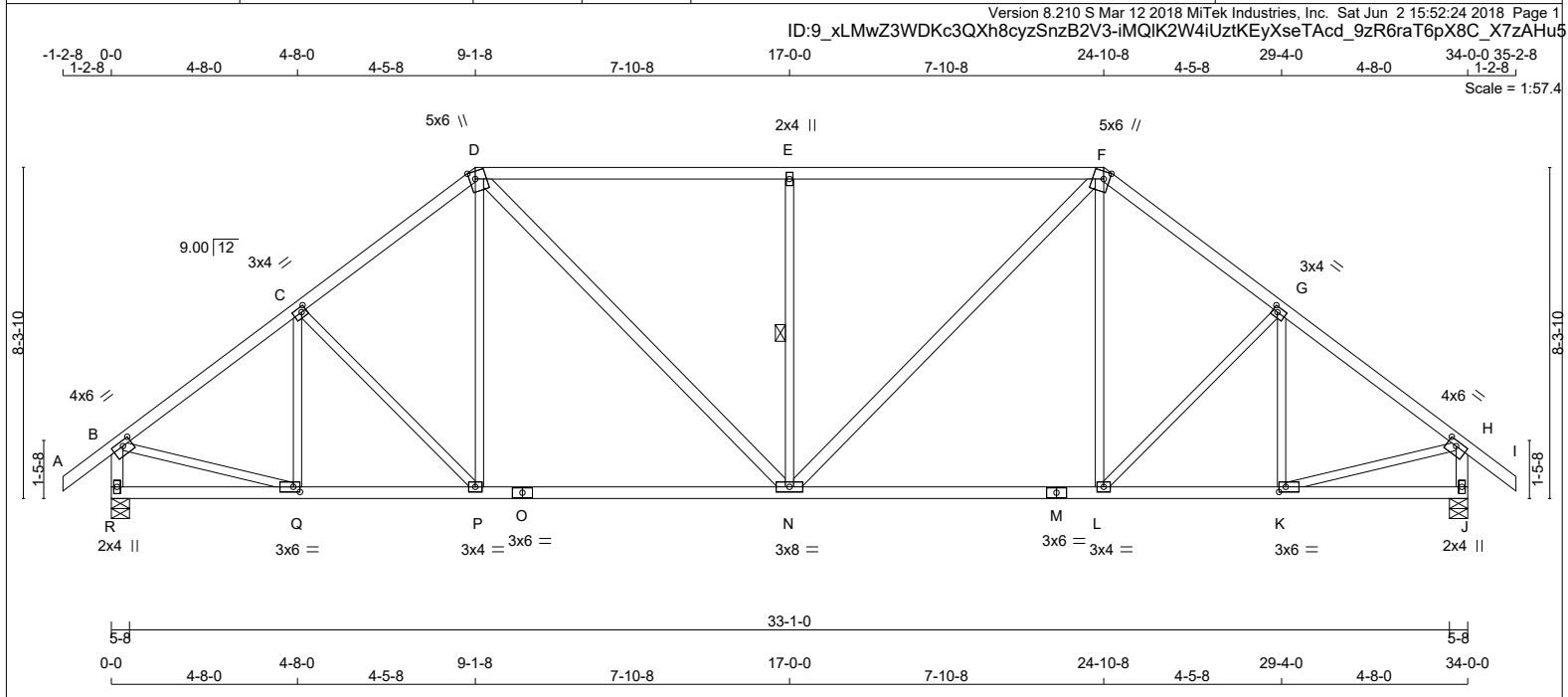
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.64 (P) (INPUT = 1.00)

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 2 X 157 = 314 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.50 2.75
C	TMWW-t	MT20	3.0	4.0	1.50 1.50
D	TTWW+m	MT20	5.0	6.0	2.25 1.75
E	TMW+w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	2.25 1.75
G	TMWW-t	MT20	3.0	4.0	1.50 1.50
H	TMVW-t	MT20	4.0	6.0	1.50 2.75
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	3.0	6.0	1.50 2.00
L	BMWW-t	MT20	3.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMWW-t	MT20	3.0	8.0	
O	BS-t	MT20	3.0	6.0	
P	BMWW-t	MT20	3.0	4.0	
Q	BMWW-t	MT20	3.0	6.0	1.50 2.00
R	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1726	0	1726	0	0
J	1726	0	1726	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
J	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20'-0" . CBF = 94 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	Q-C	-276 / 0	0.10 (1)
B-C	-1798 / 0	-77.4 -77.4	0.25 (1)	4.77	C-P	-133 / 0	0.10 (1)
C-D	-1734 / 0	-77.4 -77.4	0.24 (1)	4.84	P-D	0 / 214	0.05 (4)
D-E	-1810 / 0	-77.4 -77.4	0.73 (1)	4.03	D-N	0 / 632	0.10 (1)
E-F	-1810 / 0	-77.4 -77.4	0.73 (1)	4.03	N-E	-750 / 0	0.33 (1)
F-G	-1734 / 0	-77.4 -77.4	0.24 (1)	4.84	N-F	0 / 632	0.10 (1)
G-H	-1798 / 0	-77.4 -77.4	0.25 (1)	4.77	L-F	0 / 214	0.05 (4)
H-I	0 / 30	-77.4 -77.4	0.09 (1)	10.00	L-G	-133 / 0	0.10 (1)
R-B	-1687 / 0	0.0 0.0	0.17 (1)	6.39	K-G	-276 / 0	0.10 (1)
J-H	-1687 / 0	0.0 0.0	0.17 (1)	6.39	B-Q	0 / 1500	0.34 (1)
					K-H	0 / 1500	0.34 (1)
R-Q	0 / 0	-18.2 -18.2	0.09 (4)	10.00			
Q-P	0 / 1458	-18.2 -18.2	0.33 (1)	10.00			
P-O	0 / 1367	-18.2 -18.2	0.35 (1)	10.00			
O-N	0 / 1367	-18.2 -18.2	0.35 (1)	10.00			
N-M	0 / 1367	-18.2 -18.2	0.35 (1)	10.00			
M-L	0 / 1367	-18.2 -18.2	0.35 (1)	10.00			
L-K	0 / 1458	-18.2 -18.2	0.33 (1)	10.00			
K-J	0 / 0	-18.2 -18.2	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.73/1.00 (D-E:1), BC=0.35/1.00 (L-N:1), WB=0.34/1.00 (H-K:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

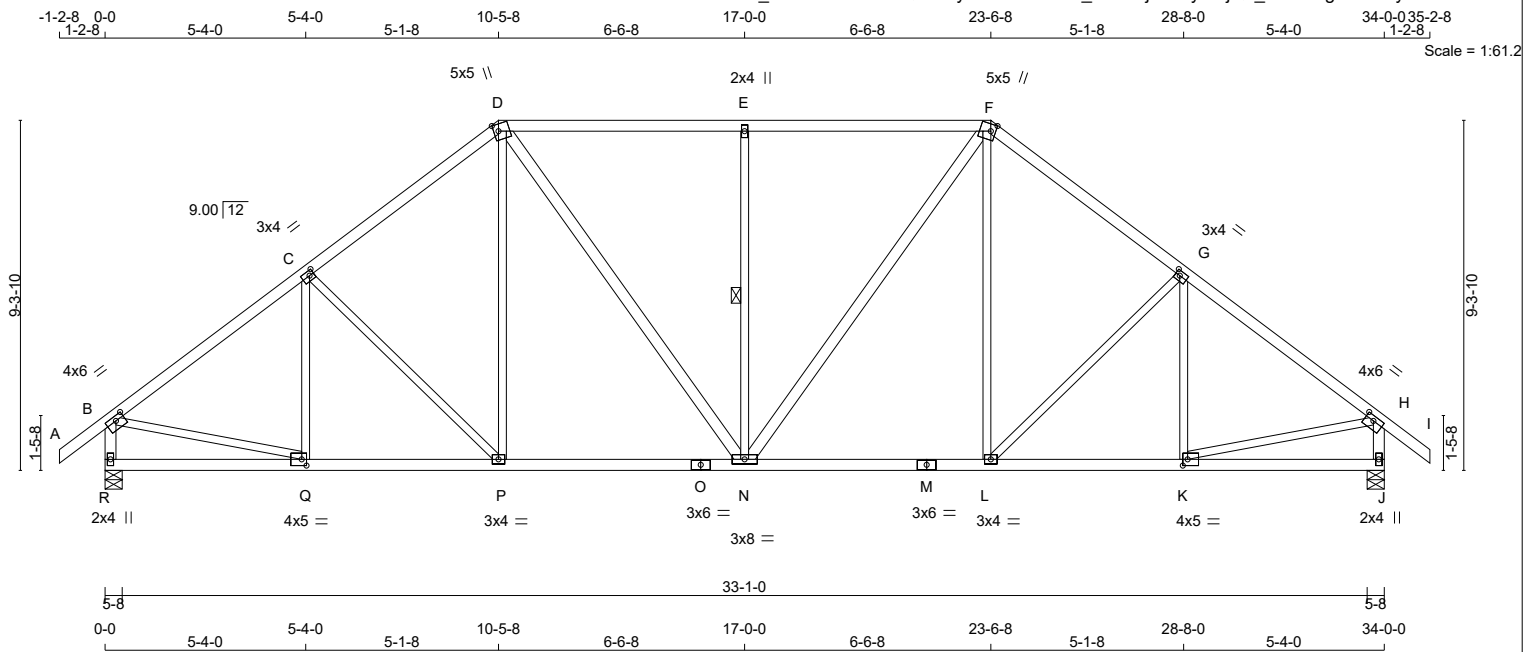
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.25	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	5.0	2.00	1.50
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	3.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	4.0		
Q	BMWW-t	MT20	4.0	5.0	2.00	1.50
R	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
R	1726	0	1726	0	5-8	5-8
J	1726	0	1726	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	
R	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
J	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20-0-0. CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS			MAX. UNBRACED LENGTH	MEMB.	WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)			MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	Q-C	-213 / 23	0.10 (1)
B-C	-1823 / 0	-77.4	-77.4	0.34 (1)	4.65	C-P	-237 / 0	0.25 (1)
C-D	-1672 / 0	-77.4	-77.4	0.32 (1)	4.83	P-D	0 / 273	0.06 (1)
D-E	-1585 / 0	-77.4	-77.4	0.47 (1)	4.70	D-N	0 / 459	0.07 (1)
E-F	-1585 / 0	-77.4	-77.4	0.47 (1)	4.70	N-E	-620 / 0	0.35 (1)
F-G	-1672 / 0	-77.4	-77.4	0.32 (1)	4.83	N-F	0 / 459	0.07 (1)
G-H	-1823 / 0	-77.4	-77.4	0.34 (1)	4.65	L-F	0 / 273	0.06 (1)
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-G	-237 / 0	0.25 (1)
R-B	-1684 / 0	0.0	0.0	0.17 (1)	6.40	K-G	-213 / 23	0.10 (1)
J-H	-1684 / 0	0.0	0.0	0.17 (1)	6.40	B-Q	0 / 1514	0.34 (1)
						K-H	0 / 1514	0.34 (1)
R-Q	0 / 0	-18.2	-18.2	0.11 (4)	10.00			
Q-P	0 / 1481	-18.2	-18.2	0.31 (1)	10.00			
P-O	0 / 1315	-18.2	-18.2	0.30 (1)	10.00			
O-N	0 / 1315	-18.2	-18.2	0.30 (1)	10.00			
N-M	0 / 1315	-18.2	-18.2	0.30 (1)	10.00			
M-L	0 / 1315	-18.2	-18.2	0.30 (1)	10.00			
L-K	0 / 1481	-18.2	-18.2	0.31 (1)	10.00			
K-J	0 / 0	-18.2	-18.2	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.47/1.00 (D-E:1), BC=0.31/1.00 (P-Q:1), WB=0.35/1.00 (E-N:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

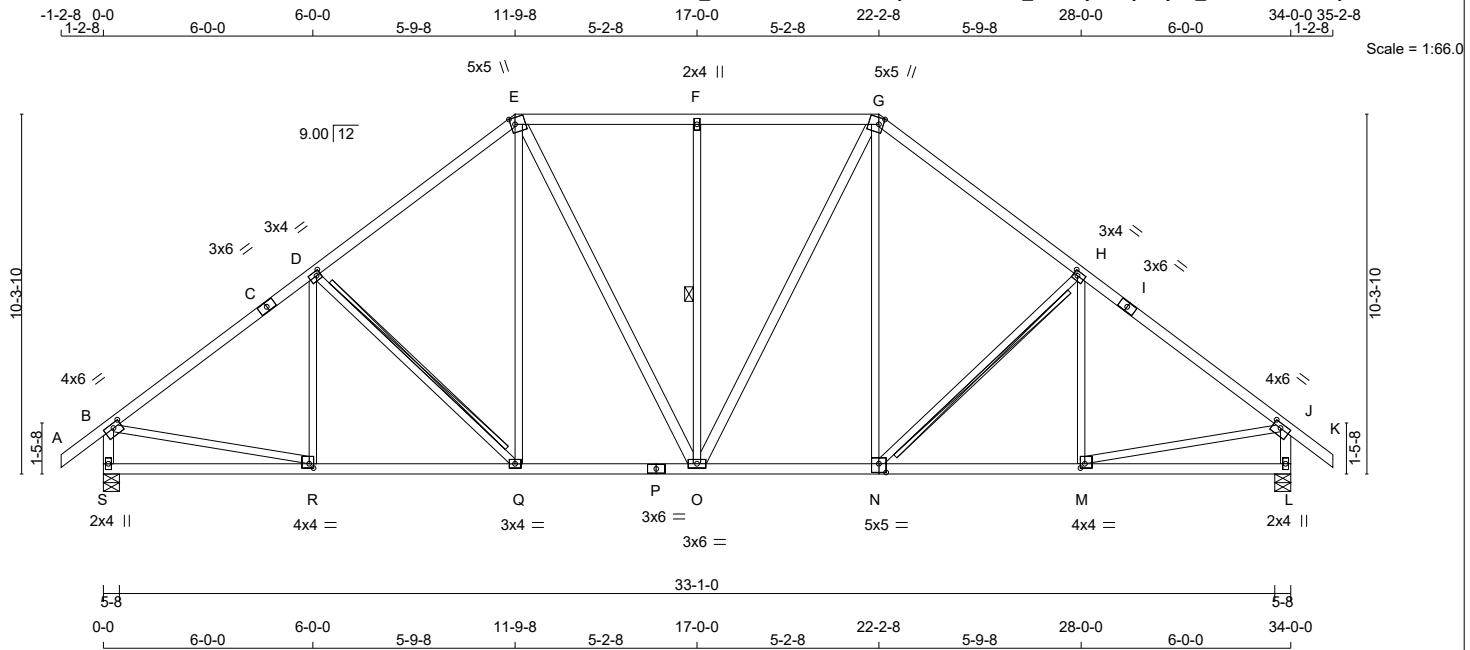
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 170 = 340 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	5.0	2.25	1.50
H	TMWW-t	MT20	3.0	4.0	1.50	1.50
I	TS-t	MT20	3.0	6.0		
J	TMVW-t	MT20	4.0	6.0	1.50	2.75
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	1.50	1.50
N	BSWW-t	MT20	5.0	5.0	3.00	2.50
O	BMWW-t	MT20	3.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	3.0	4.0		
R	BMWW-t	MT20	4.0	4.0	1.50	1.50
S	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1726	0	1726	0	0	5-8	5-8
L	1726	0	1726	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
L	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-O. DBS = 20-0-0 . CBF = 61 LBS.
2x4 DRY SPF No.2 T-BRACE AT D-Q, H-N

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	R-D	-161 / 54	0.09 (1)
B-C	-1835 / 0	-77.4 -77.4	0.44 (1)	4.52	D-Q	-332 / 0	0.17 (1)
C-D	-1835 / 0	-77.4 -77.4	0.44 (1)	4.52	Q-E	0 / 328	0.07 (1)
D-E	-1604 / 0	-77.4 -77.4	0.40 (1)	4.80	E-O	0 / 327	0.05 (1)
E-F	-1409 / 0	-77.4 -77.4	0.29 (1)	5.18	O-F	-489 / 0	0.36 (1)
F-G	-1409 / 0	-77.4 -77.4	0.29 (1)	5.18	O-G	0 / 327	0.05 (1)
G-H	-1604 / 0	-77.4 -77.4	0.40 (1)	4.80	N-G	0 / 328	0.07 (1)
H-I	-1835 / 0	-77.4 -77.4	0.44 (1)	4.52	N-H	-332 / 0	0.17 (1)
I-J	-1835 / 0	-77.4 -77.4	0.44 (1)	4.52	M-H	-161 / 54	0.09 (1)
J-K	0 / 30	-77.4 -77.4	0.09 (1)	10.00	B-R	0 / 1520	0.34 (1)
S-B	-1680 / 0	0.0 0.0	0.17 (1)	6.41	M-J	0 / 1520	0.34 (1)
L-J	-1680 / 0	0.0 0.0	0.17 (1)	6.41			
S-R	0 / 0	-18.2 -18.2	0.16 (4)	10.00			
R-Q	0 / 1494	-18.2 -18.2	0.32 (1)	10.00			
Q-P	0 / 1258	-18.2 -18.2	0.26 (1)	10.00			
P-O	0 / 1258	-18.2 -18.2	0.26 (1)	10.00			
O-N	0 / 1258	-18.2 -18.2	0.26 (1)	10.00			
N-M	0 / 1494	-18.2 -18.2	0.32 (1)	10.00			
M-L	0 / 0	-18.2 -18.2	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.44/1.00 (B-D:1), BC=0.32/1.00 (M-N:1), WB=0.36/1.00 (F-O:1), SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

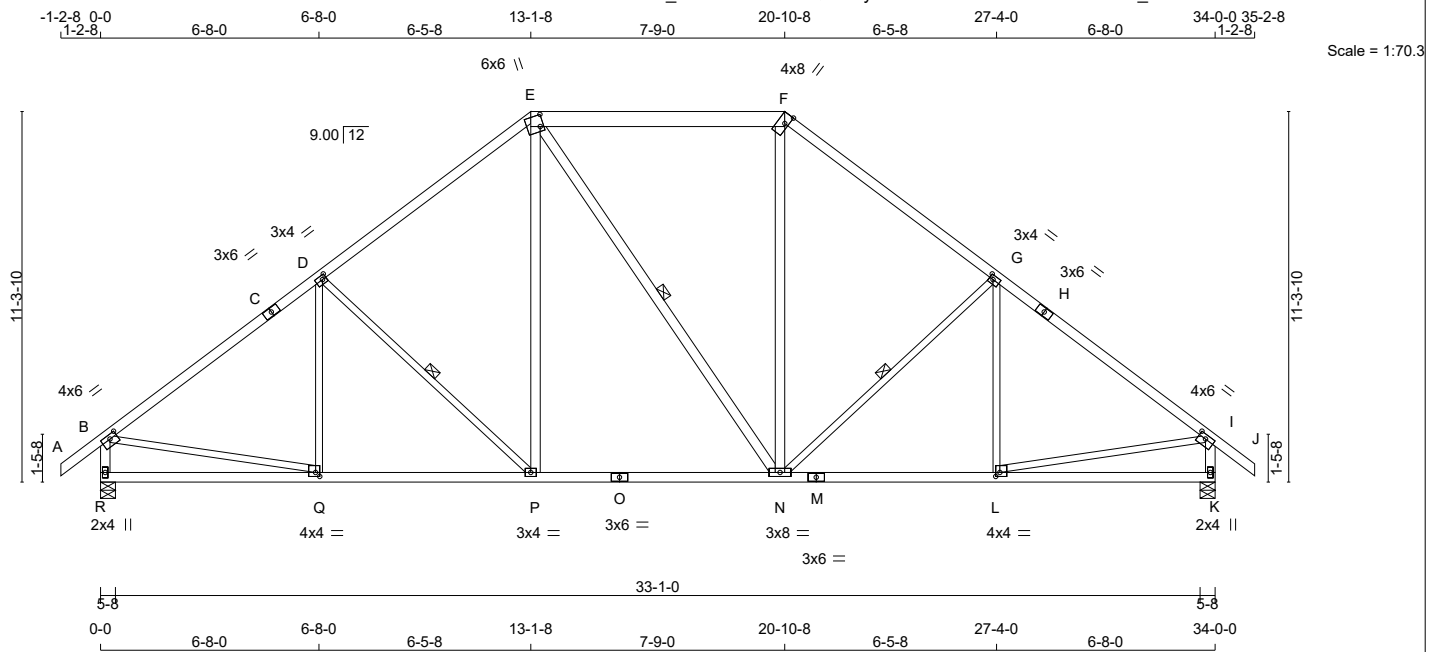
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.56 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 169 = 507 lb
[M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x6	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
P - E	2x4	DRY No.2	SPF
E - N	2x4	DRY No.2	SPF
N - F	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW+m	MT20	6.0	6.0	4.25	1.25
F	TTW+h	MT20	4.0	8.0	Edge	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	6.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	3.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	4.0		
Q	BMWW-t	MT20	4.0	4.0	1.50	1.50
R	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1726	0	1726	0	0
K	1726	0	1726	0	0

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1210	852	0	0	0	0	358	0
K	1210	852	0	0	0	0	358	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-P, E-N, G-N. DBS = 20-0-0. CBF = 51 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	FORCE (LBS)	MAX LC1 (LC)	MAX UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0/30	-77.4	-77.4 0.09 (1)	Q-D	-123/73	0.08 (1)	
B-C	-1837/0	-77.4	-77.4 0.56 (1)	D-P	-409/0	0.23 (1)	
C-D	-1837/0	-77.4	-77.4 0.56 (1)	P-E	0/407	0.07 (1)	
D-E	-1533/0	-77.4	-77.4 0.51 (1)	E-N	0/0	0.00 (1)	
E-F	-1204/0	-77.4	-77.4 0.30 (1)	N-F	0/408	0.07 (1)	
F-G	-1533/0	-77.4	-77.4 0.51 (1)	N-G	-409/0	0.23 (1)	
G-H	-1836/0	-77.4	-77.4 0.56 (1)	L-G	-124/73	0.08 (1)	
H-I	-1836/0	-77.4	-77.4 0.56 (1)	B-Q	0/1520	0.34 (1)	
I-J	0/30	-77.4	-77.4 0.09 (1)	L-I	0/1520	0.34 (1)	
R-B	-1675/0	0.0	0.0 0.17 (1)				
K-I	-1675/0	0.0	0.0 0.17 (1)				
R-Q	0/0	-18.2	-18.2 0.18 (4)				
Q-P	0/1498	-18.2	-18.2 0.34 (1)				
P-O	0/1204	-18.2	-18.2 0.30 (1)				
O-N	0/1204	-18.2	-18.2 0.30 (1)				
N-M	0/1498	-18.2	-18.2 0.35 (1)				
M-L	0/1498	-18.2	-18.2 0.35 (1)				
L-K	0/0	-18.2	-18.2 0.18 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.56/1.00 (B-D:1), BC=0.35/1.00 (L-N:1), WB=0.34/1.00 (B-Q:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.56 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



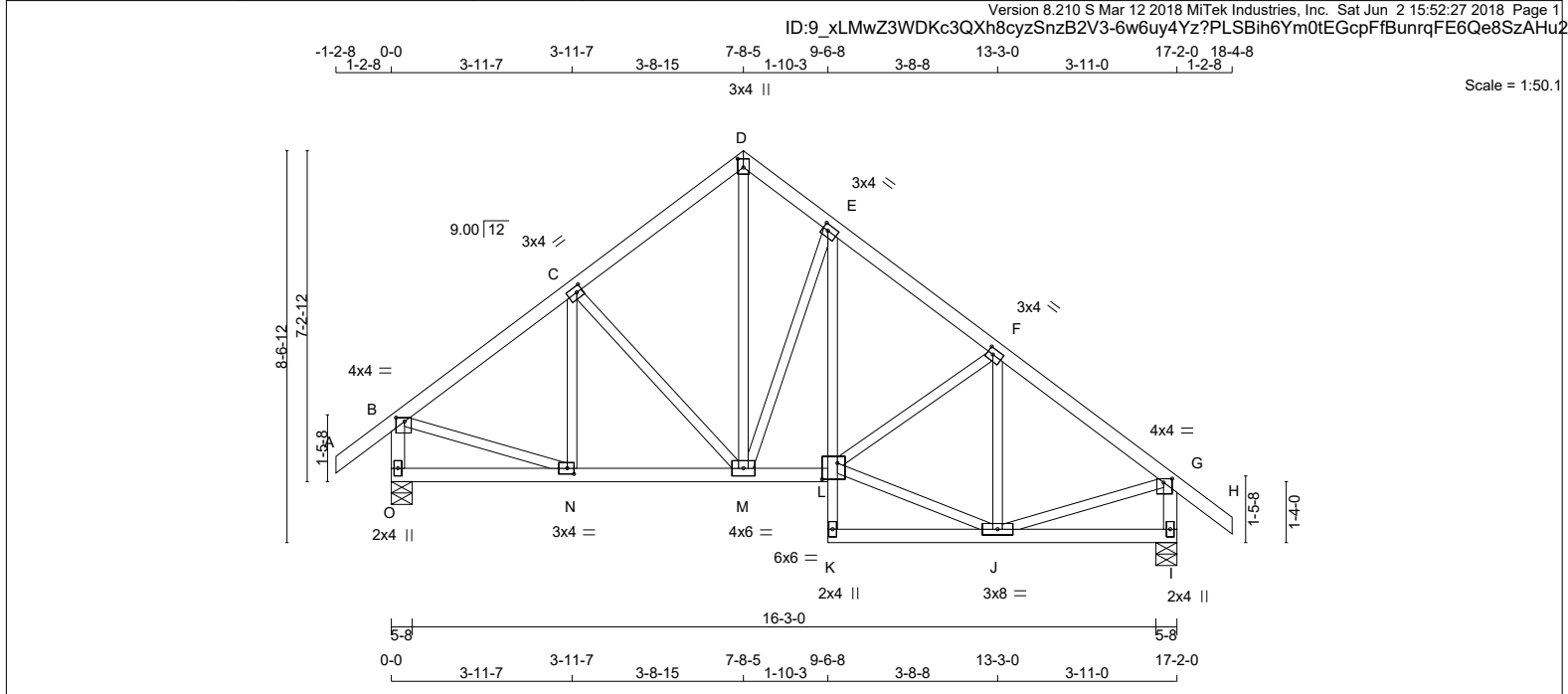
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-010	T08	3	1	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 2	PAGE 23 OF 33

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:52:26 2018 Page 2
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READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 88 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - E	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.00	2.25
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-p	MT20	4.0	4.0	1.00	2.25
I	BMV1+p	MT20	2.0	4.0		
J	BMVWW-t	MT20	3.0	8.0		
K	BMV+p	MT20	2.0	4.0		
L	BVMWW-I	MT20	6.0	6.0	4.25	4.00
M	BMVWW-t	MT20	4.0	6.0		
N	BMVWW-t	MT20	3.0	4.0	1.50	1.75
O	BMV1+p	MT20	2.0	4.0		

Jun 02, 2018

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	921	0	921	0	5-8	5-8
I	921	0	921	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
I	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)		
FR-TO		FROM TO	CSI (LC)		FR-TO				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-C	-111 / 34	0.03 (1)	
B-C	-789 / 0	-77.4	-77.4	0.15 (1)	6.25	C-M	-206 / 0	0.11 (1)	
C-D	-656 / 0	-77.4	-77.4	0.14 (1)	6.25	M-D	0 / 597	0.13 (1)	
D-E	-647 / 0	-77.4	-77.4	0.04 (1)	6.25	M-E	-473 / 0	0.28 (1)	
E-F	-847 / 0	-77.4	-77.4	0.13 (1)	6.25	L-J	0 / 696	0.16 (1)	
F-G	-788 / 0	-77.4	-77.4	0.14 (1)	6.25	L-F	0 / 26	0.01 (1)	
G-H	0 / 30	-77.4	-77.4	0.09 (1)	10.00	J-F	-375 / 0	0.11 (1)	
O-B	-890 / 0	0.0	0.0	0.09 (1)	7.81	B-N	0 / 673	0.15 (1)	
I-G	-892 / 0	0.0	0.0	0.09 (1)	7.81	J-G	0 / 672	0.15 (1)	
O-N	0 / 0	-18.2	-18.2	0.06 (4)	10.00				
N-M	0 / 647	-18.2	-18.2	0.13 (1)	10.00				
M-L	0 / 670	-18.2	-18.2	0.14 (1)	10.00				
K-L	0 / 27	0.0	0.0	0.02 (1)	10.00				
L-E	0 / 352	0.0	0.0	0.09 (1)	10.00				
K-J	0 / 3	-18.2	-18.2	0.07 (4)	10.00				
J-I	0 / 0	-18.2	-18.2	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	DL	PSF
	23.3	3.0	PSF
BOT CH.	LL	DL	PSF
	0.0	7.3	PSF
TOTAL LOAD	33.6		PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.15/1.00 (B-C:1), BC=0.14/1.00 (L-M:1), WB=0.28/1.00 (E-M:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

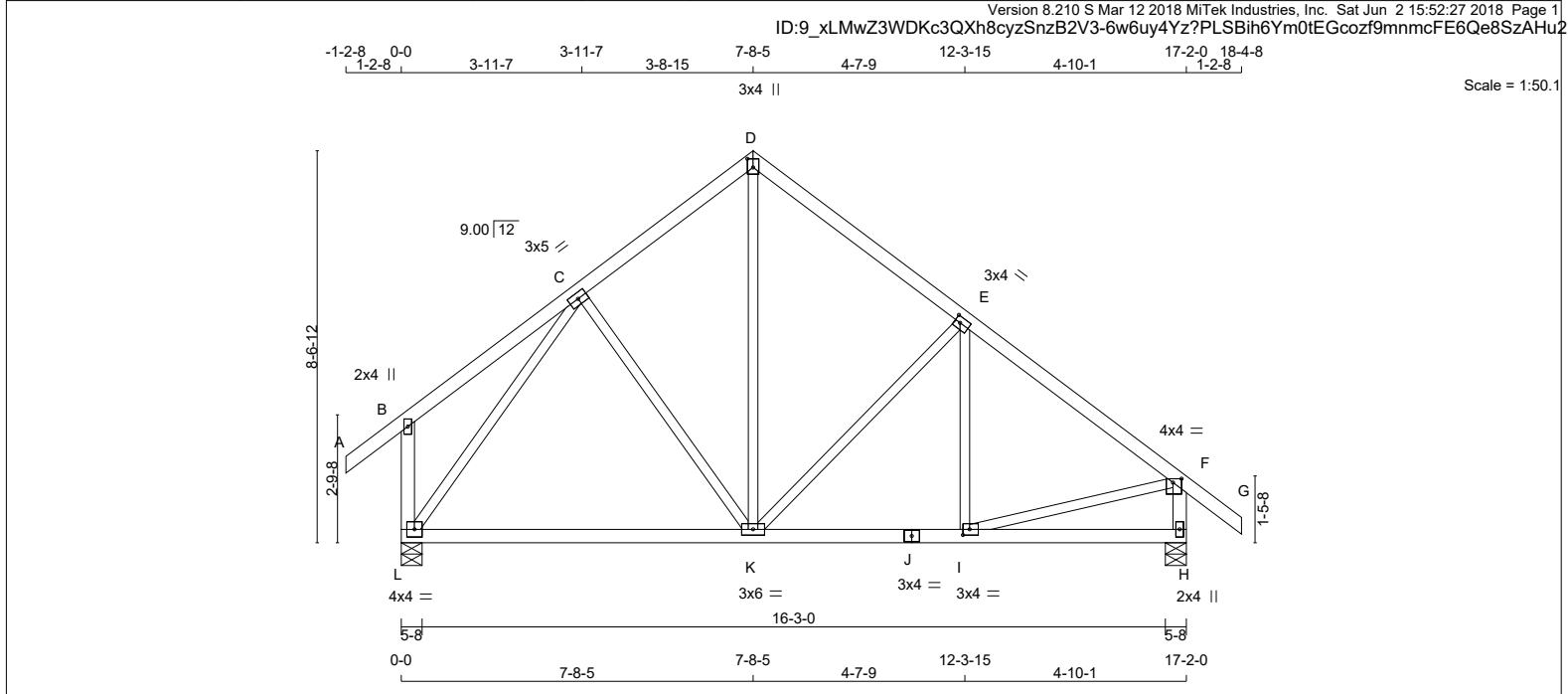
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.24 (N) (INPUT = 1.00)





TOTAL WEIGHT = 81 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0		
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	3.0	4.0	1.50	1.75
J	BS-t	MT20	3.0	4.0		
K	BMWWW-t	MT20	3.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	921	0	921	0	5-8	5-8
H	921	0	921	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	LC1 MAX (LC)	
FR-TO		FROM TO			FR-TO				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	C-K	-55 / 31	0.04 (1)	
B-C	0 / 21	-77.4	-77.4	0.18 (1)	10.00	K-D	0 / 404	0.09 (1)	
C-D	-564 / 0	-77.4	-77.4	0.14 (1)	6.25	K-E	-305 / 0	0.25 (1)	
D-E	-570 / 0	-77.4	-77.4	0.22 (1)	6.25	I-E	-77 / 41	0.03 (1)	
E-F	-783 / 0	-77.4	-77.4	0.23 (1)	6.25	L-C	-798 / 0	0.61 (1)	
F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00	I-F	0 / 665	0.15 (1)	
L-B	-215 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-881 / 0	0.0	0.0	0.09 (1)	7.81				
L-K	0 / 467	-18.2	-18.2	0.26 (4)	10.00				
K-J	0 / 647	-18.2	-18.2	0.27 (4)	10.00				
J-I	0 / 647	-18.2	-18.2	0.27 (4)	10.00				
I-H	0 / 0	-18.2	-18.2	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.23/1.00 (E-F:1), BC=0.27/1.00 (I-K:4), WB=0.61/1.00 (C-L:1), SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

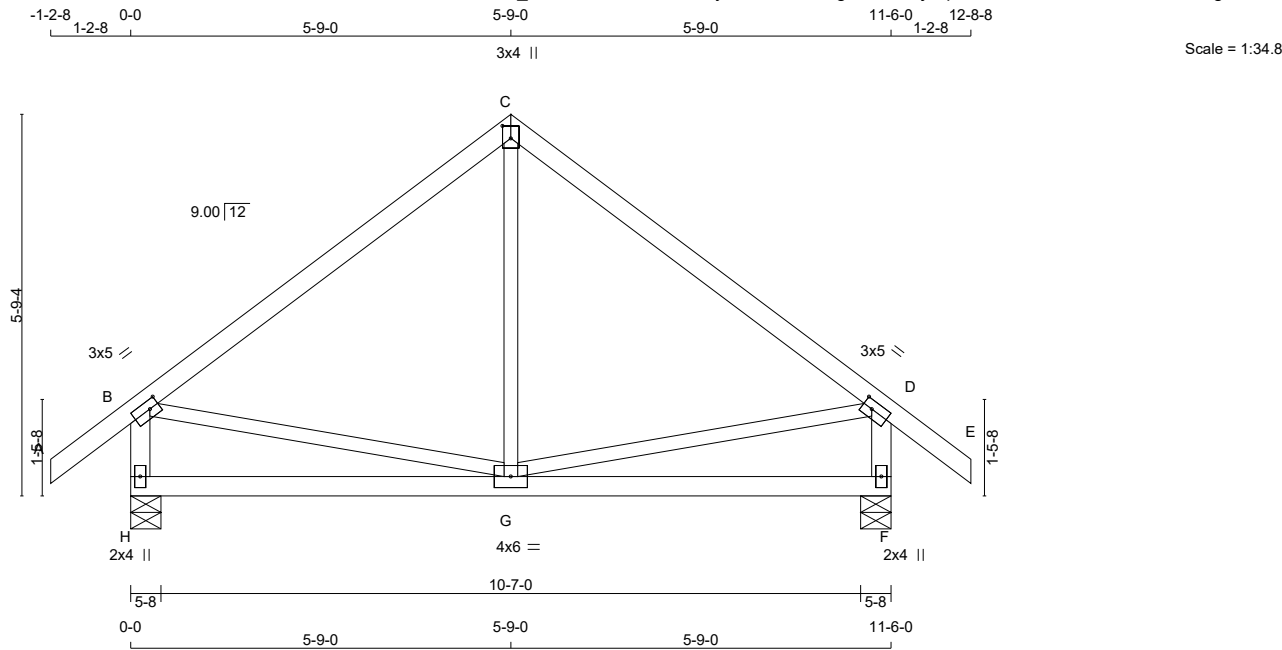
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.25 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 49 lb [M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	650	0	650	0
F	650	0	650	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	454	328 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
F	454	328 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	G-C	0 / 106	0.04 (4)
B-C	-379 / 0	-77.4	-77.4 0.33 (1)	6.25	B-G	0 / 309	0.07 (1)
C-D	-379 / 0	-77.4	-77.4 0.33 (1)	6.25	G-D	0 / 309	0.07 (1)
D-E	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
H-B	-610 / 0	0.0	0.0 0.06 (1)	7.81			
F-D	-610 / 0	0.0	0.0 0.06 (1)	7.81			
H-G	0 / 0	-18.2	-18.2 0.17 (4)	10.00			
G-F	0 / 0	-18.2	-18.2 0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (B-C:1), BC=0.17/1.00 (F-G:4), WB=0.07/1.00 (B-G:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

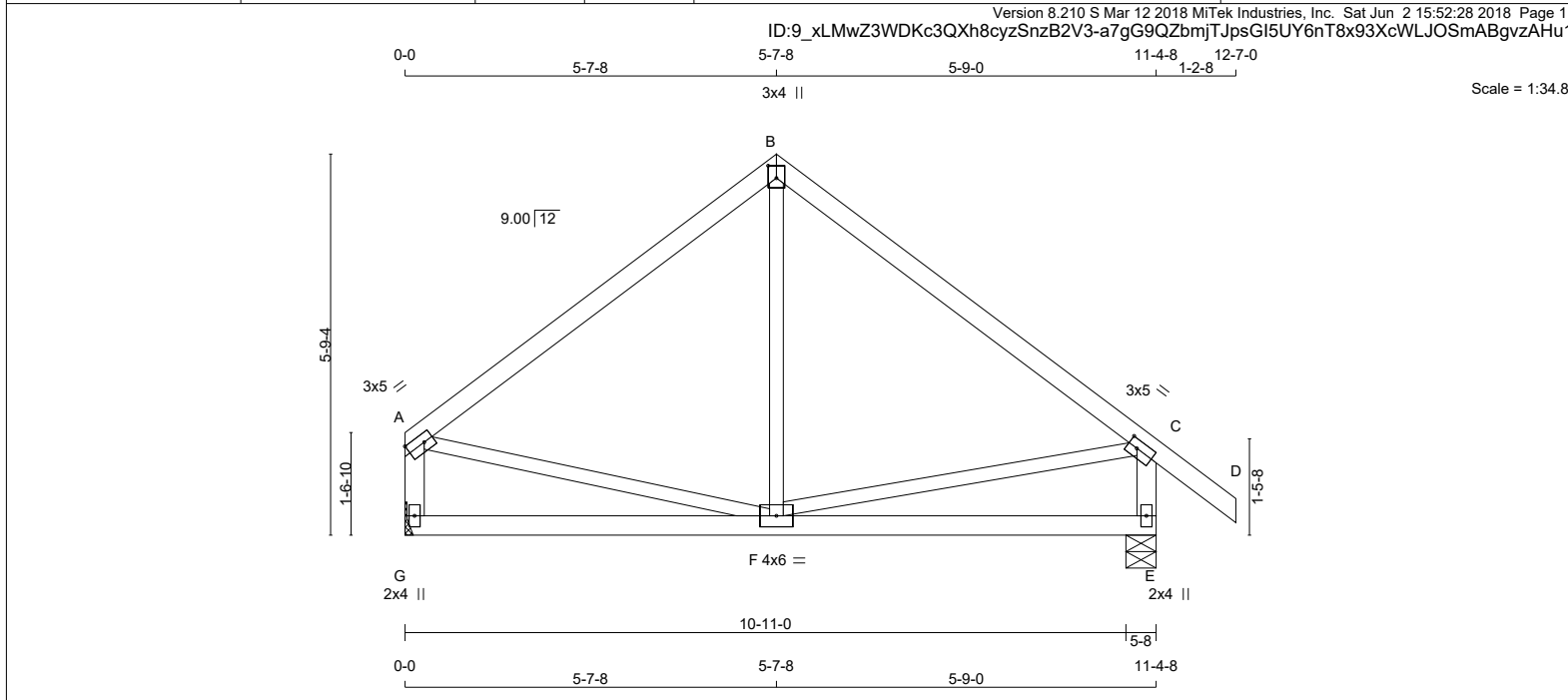
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 47 = 94 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
G - A	2x4	DRY	No.2
E - C	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	Edge
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	544	0	544	0	MECHANICAL	
E	644	0	644	0	5-8	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	382	265 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
E	450	325 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS				MEMB.	WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CSI (LC)		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CSI (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO	
A-B	-371 / 0	-77.4	-77.4	0.32 (1)	6.25	F-B	0 / 103	0.04 (4)	
B-C	-371 / 0	-77.4	-77.4	0.33 (1)	6.25	A-F	0 / 304	0.07 (1)	
C-D	0 / 30	-77.4	-77.4	0.09 (1)	10.00	F-C	0 / 302	0.07 (1)	
G-A	-508 / 0	0.0	0.0	0.05 (1)	7.81				
E-C	-601 / 0	0.0	0.0	0.06 (1)	7.81				
G-F	0 / 0	-18.2	-18.2	0.17 (4)	10.00				
F-E	0 / 0	-18.2	-18.2	0.17 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.17/1.00 (F-G:4) , WB=0.07/1.00 (A-F:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

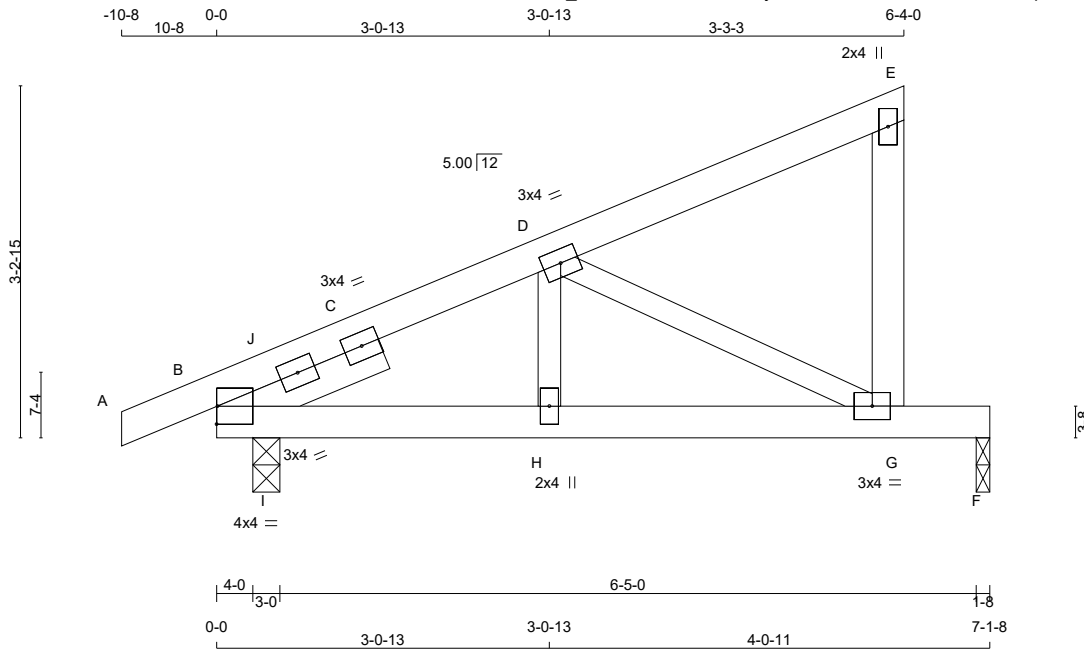
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 4 X 26 = 103 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY: SEASONED LUMBER.				
	2x3	DRY	No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBR1-I	MT20	4.0	4.0	
B	RT-t	MT20	3.0	4.0	
B	RT-t	MT20	3.0	4.0	
D	TMWW-t	MT20	3.0	4.0	
E	TMV+p	MT20	2.0	4.0	
G	BMVW-t	MT20	3.0	4.0	
H	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	283	0	283	0	0
B	409	0	409	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	200	131 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
B	286	207 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS		WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 3	-77.4	-77.4 0.05 (1)	10.00	H-D	0 / 170	0.04 (1)
B-J	-415 / 0	-77.4	-77.4 0.01 (1)	6.25	D-G	-496 / 0	0.11 (1)
J-C	-384 / 0	-77.4	-77.4 0.06 (1)	6.25	C-I	-230 / 0	0.02 (1)
C-D	-384 / 0	-77.4	-77.4 0.06 (1)	6.25	I-J	0 / 153	0.00 (1)
D-E	-6 / 0	-77.4	-77.4 0.10 (1)	10.00			
G-E	-110 / 0	0.0	0.0 0.02 (1)	7.81			
B-I	0 / 297	-18.2	-18.2 0.06 (1)	10.00			
I-H	0 / 447	-18.2	-18.2 0.16 (1)	10.00			
H-G	0 / 447	-18.2	-18.2 0.37 (1)	10.00			
G-F	0 / 0	-18.2	-18.2 0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.10/1.00 (D-E:1), BC=0.37/1.00 (G-H:1), WB=0.11/1.00 (D-G:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

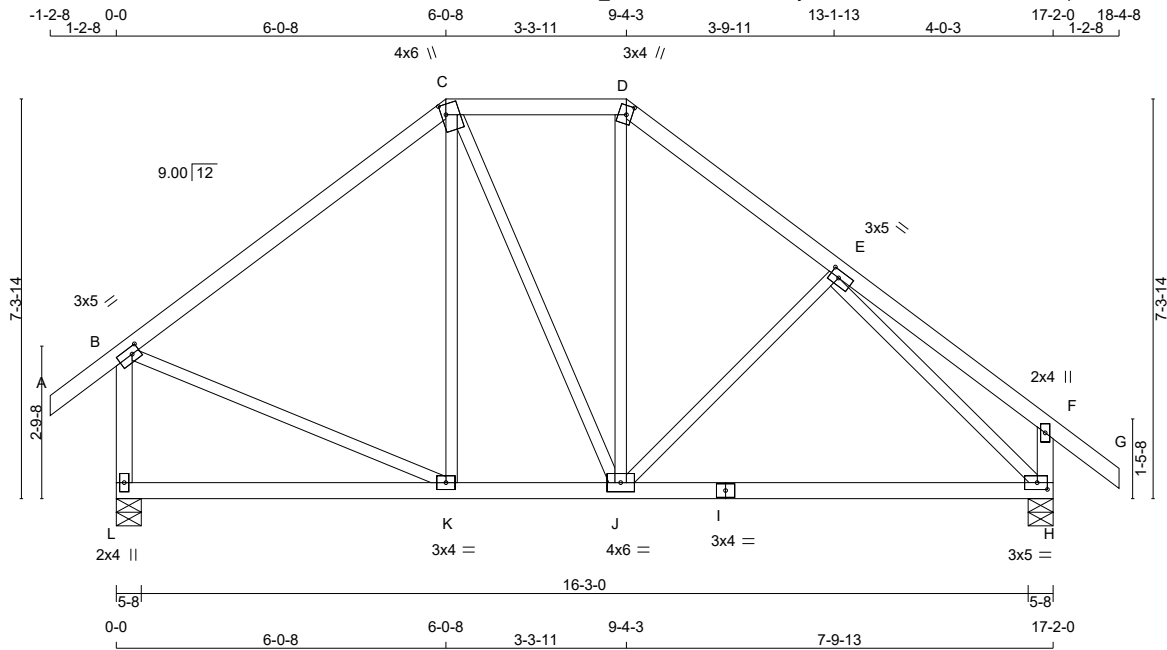
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:42.2

TOTAL WEIGHT = 82 lb [M][F]

LUMBER					DESCR.
CHORDS	SIZE	DRY	LUMBER	No.2	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTWW+m	MT20	4.0	6.0	2.25	1.00
D	TTW+m	MT20	3.0	4.0	2.00	1.25
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	5.0	1.50	2.25
I	BS-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	3.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	921	0	921	0	5-8	5-8
H	921	0	921	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
L	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	K-C	-111 / 25	0.11 (1)
B-C	-584 / 0	-77.4	-77.4 0.37 (1)	6.25	C-J	0 / 111	0.03 (1)
C-D	-511 / 0	-77.4	-77.4 0.11 (1)	6.25	J-D	0 / 139	0.03 (4)
D-E	-659 / 0	-77.4	-77.4 0.15 (1)	6.25	J-E	-157 / 5	0.09 (1)
E-F	0 / 21	-77.4	-77.4 0.19 (1)	10.00	B-K	0 / 503	0.11 (1)
F-G	0 / 30	-77.4	-77.4 0.09 (1)	10.00	E-H	-895 / 0	0.46 (1)
L-B	-872 / 0	0.0	0.0 0.13 (1)	7.81			
H-F	-216 / 0	0.0	0.0 0.02 (1)	7.81			
L-K	0 / 0	-18.2	-18.2 0.15 (4)	10.00			
K-J	0 / 465	-18.2	-18.2 0.27 (4)	10.00			
J-I	0 / 620	-18.2	-18.2 0.28 (4)	10.00			
I-H	0 / 620	-18.2	-18.2 0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.12")

CSI: TC=0.37/1.00 (B-C:1), BC=0.28/1.00 (H-J:4), WB=0.46/1.00 (E-H:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

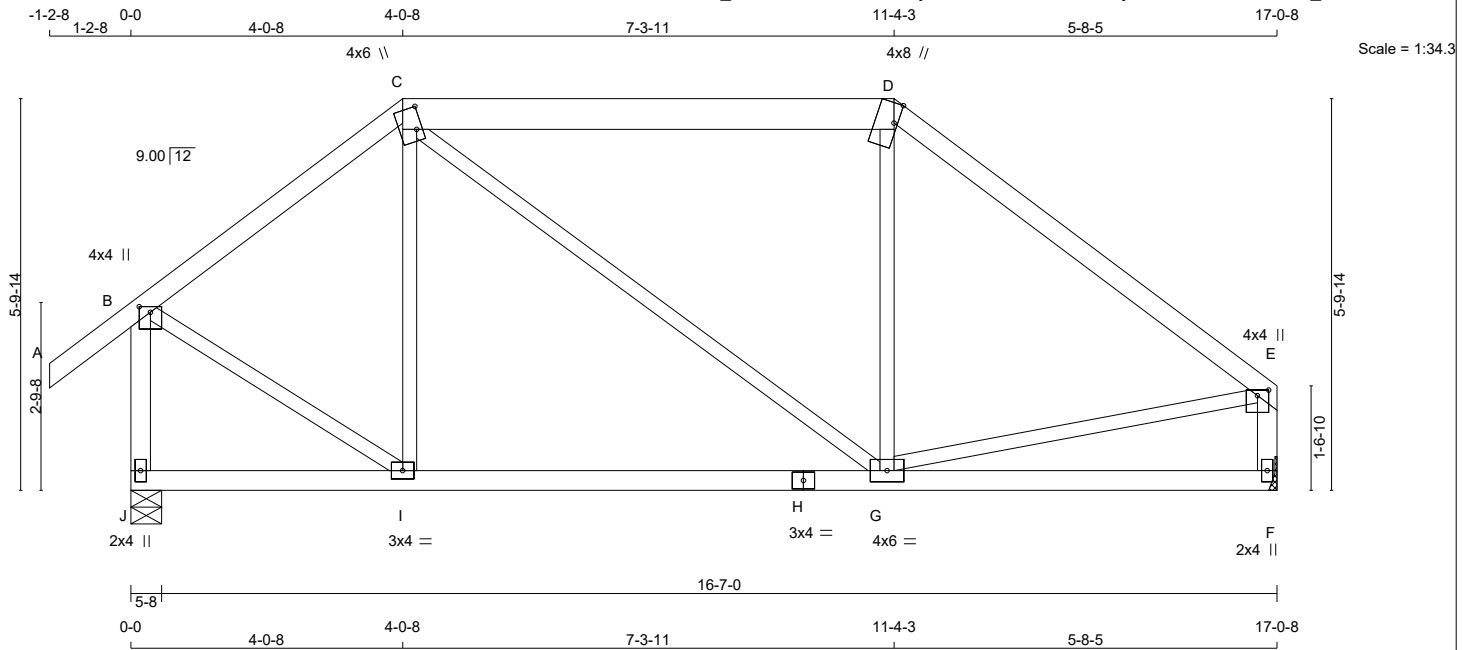
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 76 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	4.0	6.0	4.00	1.00
D	TTW+m	MT20	4.0	8.0	Edge	0.50
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	4.0		
I	BMWW-t	MT20	3.0	4.0		
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	915	0	915	0	5-8	5-8
F	815	0	815	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	641	457 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
F	572	397 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX. UNBRACED LENGTH			
FR-TO		FROM	TO		FR-TO				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	I-C	-169 / 37	0.08 (1)	
B-C	-591 / 0	-77.4	-77.4	0.17 (1)	6.25	C-G	0 / 135	0.03 (1)	
C-D	-578 / 0	-77.4	-77.4	0.26 (1)	6.25	G-D	-70 / 84	0.03 (1)	
D-E	-724 / 0	-77.4	-77.4	0.33 (1)	6.25	B-I	0 / 551	0.12 (1)	
J-B	-891 / 0	0.0	0.0	0.13 (1)	7.81	G-E	0 / 593	0.13 (1)	
F-E	-774 / 0	0.0	0.0	0.08 (1)	7.81				
J-I	0 / 0	-18.2	-18.2	0.14 (4)	10.00				
I-H	0 / 471	-18.2	-18.2	0.22 (4)	10.00				
H-G	0 / 471	-18.2	-18.2	0.22 (4)	10.00				
G-F	0 / 0	-18.2	-18.2	0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL		LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.05")

CSI: TC=0.33/1.00 (D-E:1), BC=0.22/1.00 (G-I:4), WB=0.13/1.00 (E-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

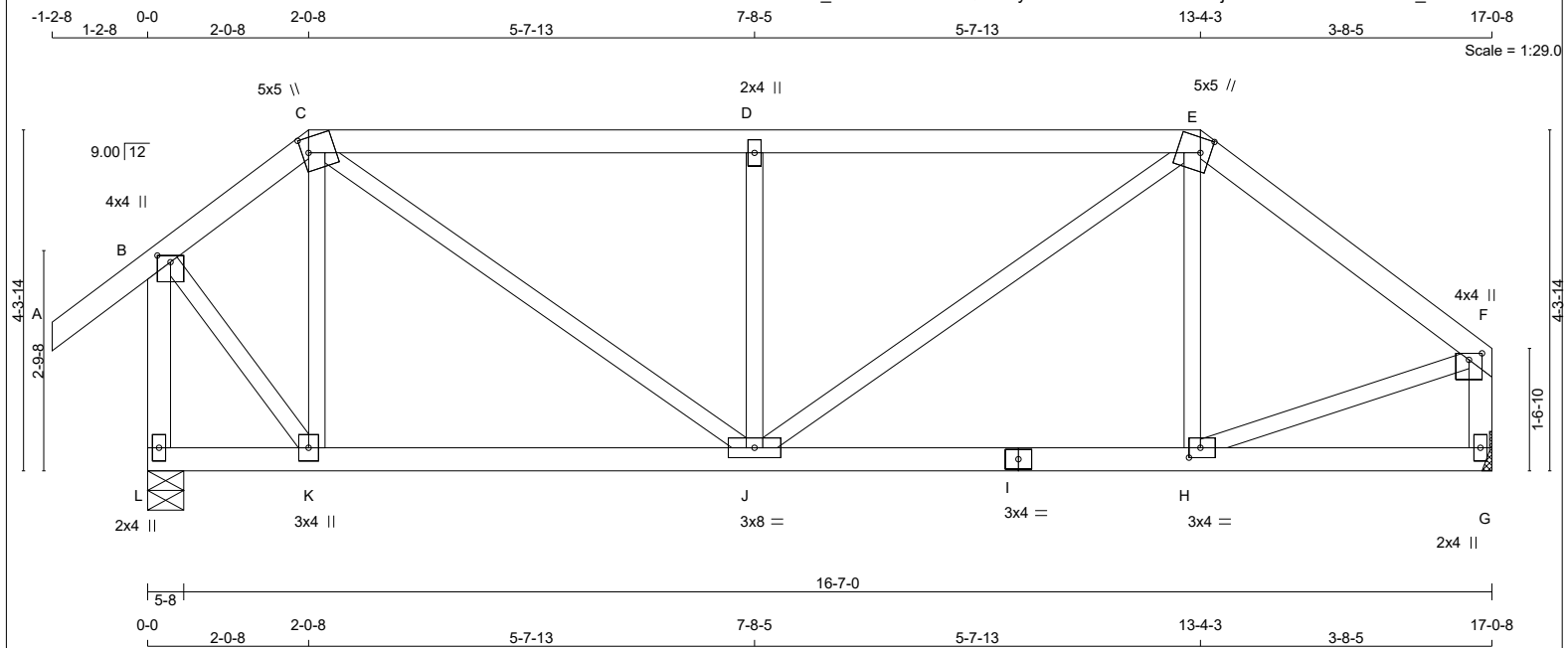
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.23 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 72 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	5.0	2.25	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	3.0	4.0	1.50	1.75
I	BS-t	MT20	3.0	4.0		
J	BMWW-t	MT20	3.0	8.0		
K	BMWW-t	MT20	3.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	915	0	915	0	5-8	5-8
G	815	0	815	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	641	457 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
G	572	397 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-C	-354 / 0	0.10 (1)
B-C	-484 / 0	-77.4	-77.4	0.06 (1)	6.25	C-J	0 / 687	0.15 (1)
C-D	-936 / 0	-77.4	-77.4	0.44 (1)	5.75	J-D	-536 / 0	0.15 (1)
D-E	-936 / 0	-77.4	-77.4	0.44 (1)	5.75	J-E	0 / 413	0.09 (1)
E-F	-753 / 0	-77.4	-77.4	0.19 (1)	6.25	H-E	-98 / 44	0.03 (1)
L-B	-914 / 0	0.0	0.0	0.14 (1)	7.81	B-K	0 / 586	0.13 (1)
G-F	-791 / 0	0.0	0.0	0.08 (1)	7.81	H-F	0 / 632	0.14 (1)
L-K	0 / 0	-18.2	-18.2	0.08 (4)	10.00			
K-J	0 / 376	-18.2	-18.2	0.14 (4)	10.00			
J-I	0 / 599	-18.2	-18.2	0.16 (4)	10.00			
I-H	0 / 599	-18.2	-18.2	0.16 (4)	10.00			
H-G	0 / 0	-18.2	-18.2	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.44/1.00 (D-E:1), BC=0.16/1.00 (H-J:4), WB=0.15/1.00 (C-J:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.22 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



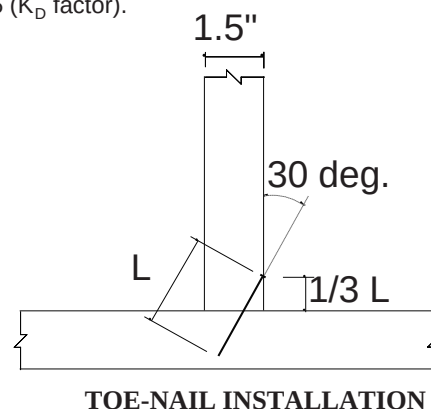
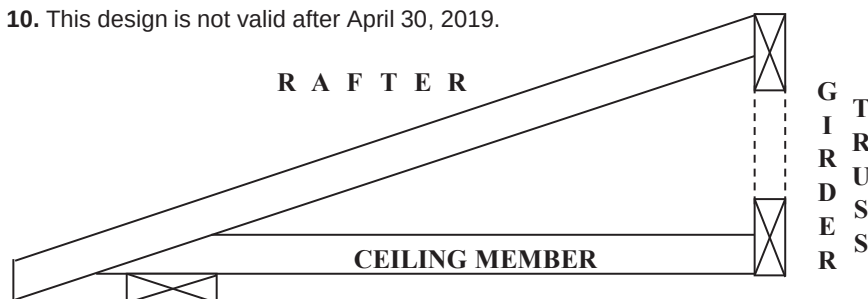
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

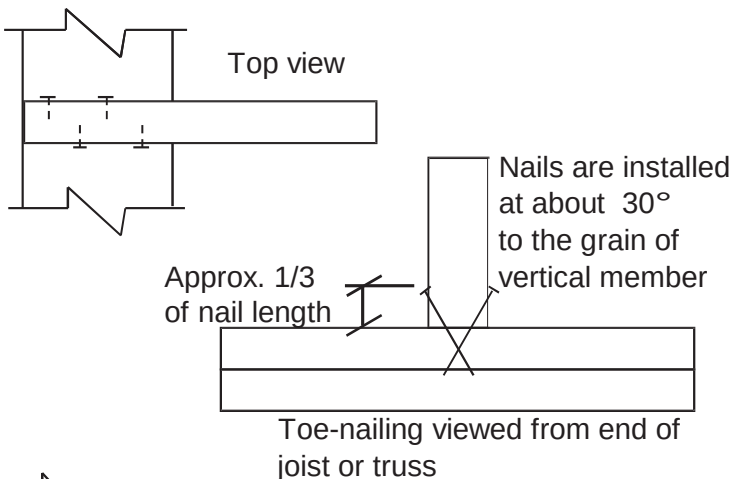
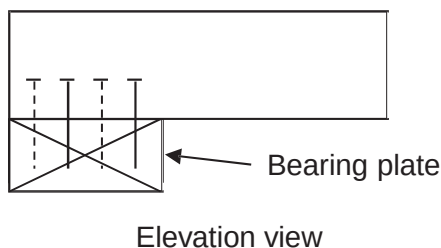
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

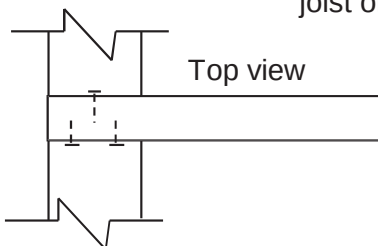
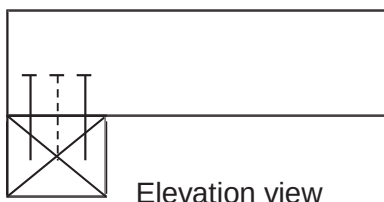
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017