

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-009
GREEN YORK HOMES - GRANELLI HOME
CORP - CELESTIAL 1 ELEV 1

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

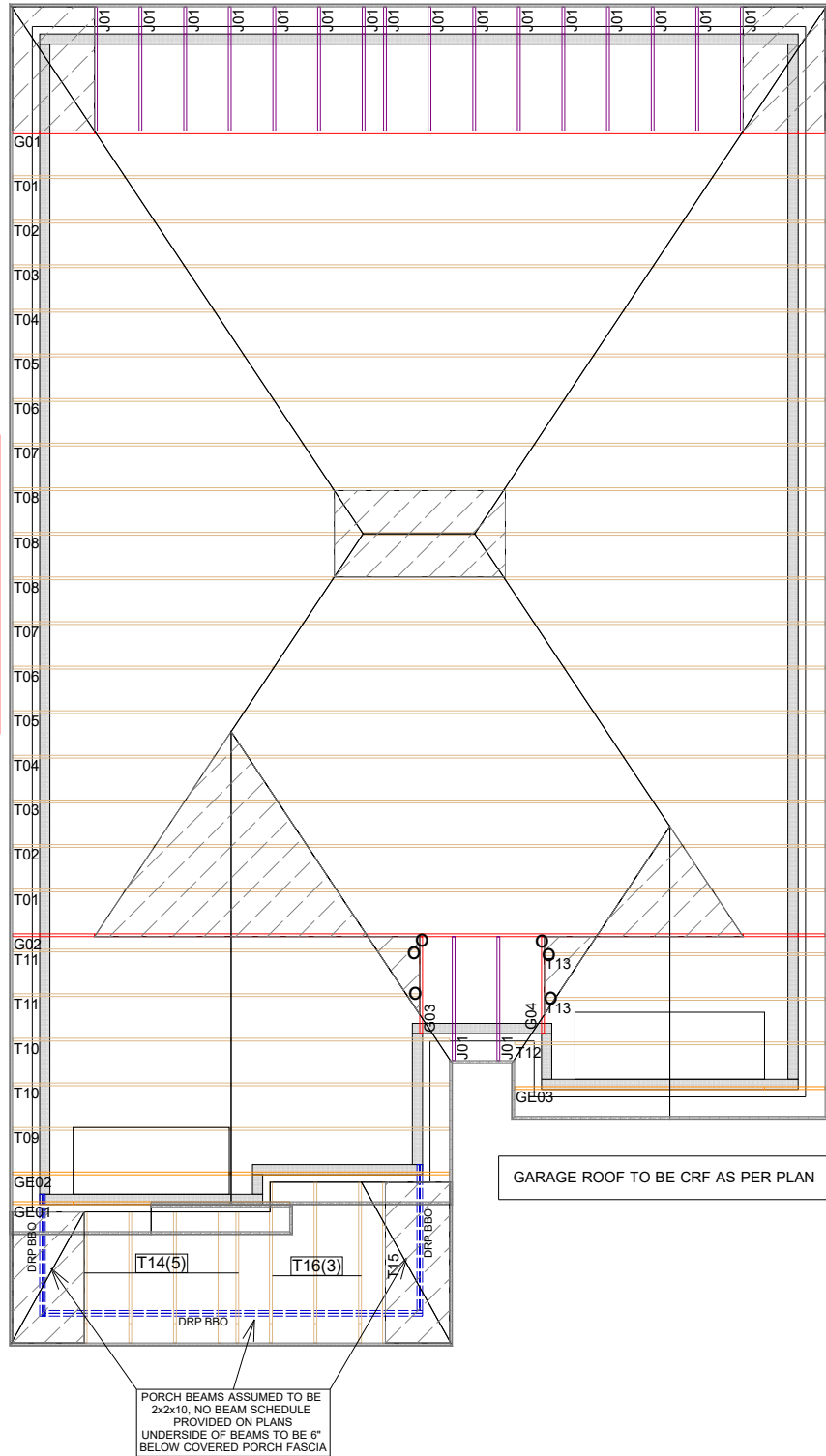
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-009



ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	CELESTIAL 1 ELE 1
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/31/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE TRUSS BELOW EA CROSS PT. VERT POS. THAN 6' TO HAVE A LATERAL BRACING SO THAT THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'

Version 8.210 S Mar 12 2018 MITek Industries, Inc. Sat Jun 2 15:38:05 2018 Page 1

ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-L5001K6G?ZkmmEnR2kZkIUvPmAohqraN0?c0tQzAI5V

1-2-8 0-0 2-5-8 2-5-8 4-11-0 7-4-8 4-9-12 12-2-4 4-9-12 17-0-0 4-9-12 21-9-12 4-9-12 26-7-8 4-11-0 31-6-8 2-5-8 34-0-0 35-2-8 1-2-8

Scale = 1:58.0

TOTAL WEIGHT = 149 lb

[M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	2100F 1.8E	SPF
S - P	2x4	DRY	2100F 1.8E	SPF
P - M	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2496	0	2496	0	0	5-8	5-8
M	2496	0	2496	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1752	1228 / 0	0 / 0	0 / 0	0 / 0	524 / 0	0 / 0
M	1752	1228 / 0	0 / 0	0 / 0	0 / 0	524 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4 0.10 (1)	10.00	V-C	-721 / 0	0.14 (1)	
B-C	-2414 / 0	-77.4 -77.4 0.17 (1)	4.25	C-U	0 / 3399	0.84 (1)	
C-D	-4829 / 0	-114.0 -114.0 0.37 (1)	3.66	U-D	-1631 / 0	0.32 (1)	
D-E	-6475 / 0	-114.0 -114.0 0.46 (1)	3.14	D-T	0 / 1927	0.48 (1)	
E-F	-7037 / 0	-114.0 -114.0 0.53 (1)	2.94	T-E	-871 / 0	0.17 (1)	
F-G	-7037 / 0	-114.0 -114.0 0.53 (1)	2.94	E-R	0 / 658	0.16 (1)	
G-H	-7037 / 0	-114.0 -114.0 0.53 (1)	2.94	R-F	-556 / 0	0.11 (1)	
H-I	-6478 / 0	-114.0 -114.0 0.46 (1)	3.14	R-H	0 / 655	0.16 (1)	
I-J	-4827 / 0	-114.0 -114.0 0.38 (1)	3.66	Q-H	-873 / 0	0.17 (1)	
J-K	-2411 / 0	-77.4 -77.4 0.17 (1)	4.25	Q-I	0 / 1932	0.48 (1)	
K-L	0 / 30	-77.4 -77.4 0.10 (1)	10.00	O-I	-1627 / 0	0.32 (1)	
W-B	-2487 / 0	0.0 0.0 0.28 (1)	5.37	O-J	0 / 3400	0.84 (1)	
M-K	-2484 / 0	0.0 0.0 0.28 (1)	5.37	N-J	-726 / 0	0.14 (1)	
				B-V	0 / 2120	0.52 (1)	
				N-K	0 / 2117	0.52 (1)	
W-V	0 / 0	-26.9 -26.9 0.08 (4)	10.00				
V-U	0 / 1914	-26.9 -26.9 0.22 (1)	10.00				
U-T	0 / 4829	-26.9 -26.9 0.46 (1)	10.00				
T-S	0 / 6475	-26.9 -26.9 0.60 (1)	10.00				
S-R	0 / 6475	-26.9 -26.9 0.60 (1)	10.00				
R-Q	0 / 6478	-26.9 -26.9 0.60 (1)	10.00				
Q-P	0 / 4827	-26.9 -26.9 0.94 (1)	10.00				
P-O	0 / 4827	-26.9 -26.9 0.94 (1)	10.00				
O-N	0 / 1912	-26.9 -26.9 0.42 (1)	10.00				
N-M	0 / 0	-26.9 -26.9 0.11 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-5-8	-90	-90	---	FRONT	VERT	TOTAL
J	31-6-8	-90	-90	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-5-8
END SETBACK = 4-4-4
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 958 (0.43")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 542 (0.75")

CSI: TC=0.53/1.00 (E-F:1), BC=0.94/1.00 (O-Q:1),
WB=0.84/1.00 (J-O:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
MT18HS 580 354 2729 1405 4191 2010

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
PROVINCIAL OF ONTARIO
Jun 02, 2018

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-009	G01	1	1	GREEN YORK HOMES - GRANELL HOME CORP - CELESTIAL 1 ELEV 1	PAGE 4 OF 32
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:38:06 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-pHaOFg7umssdOOMdbR4zrhRaVa8wZlqWEfMaPszAl5V

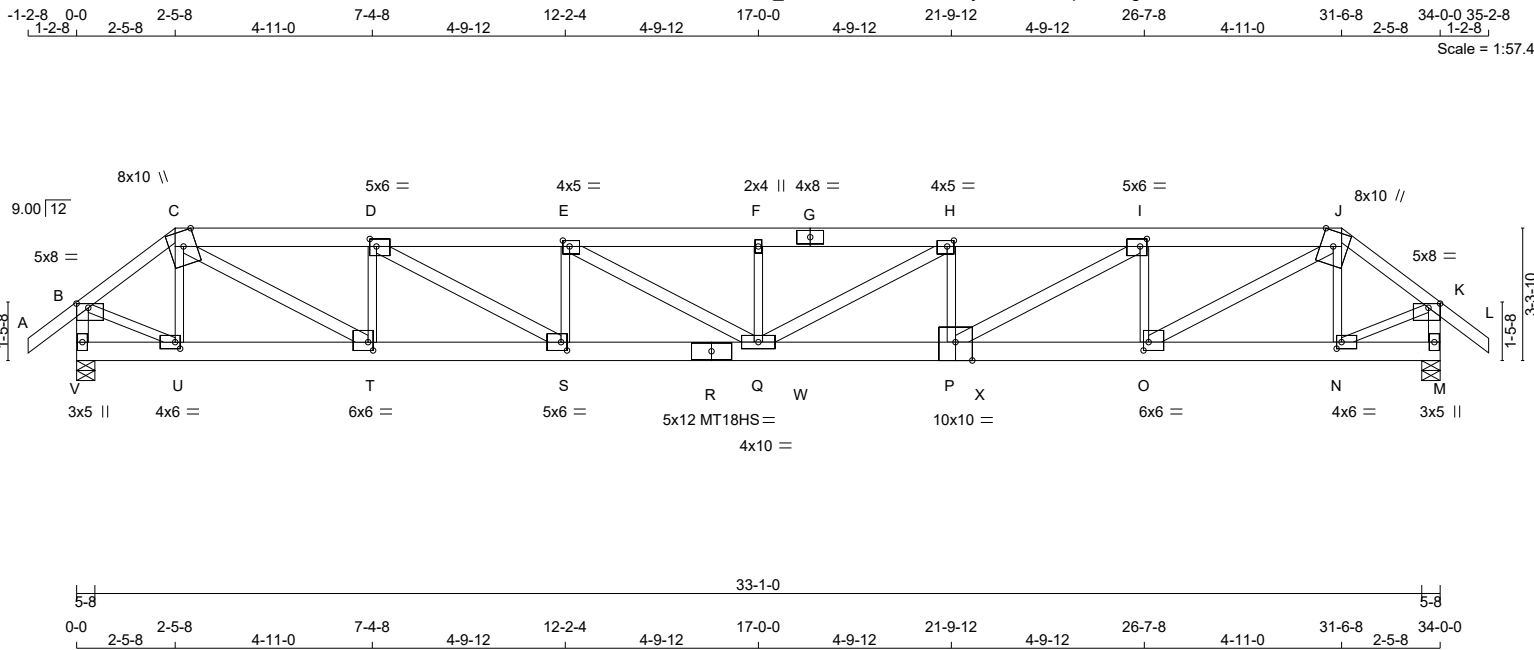
WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 90.5 lbs FACTORED DOWN AT 31-6-8,
AND 90.5 lbs FACTORED DOWN AT 2-5-8 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.94 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 179 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - G	2x6	DRY No.2	SPF
G - J	2x6	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
V - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
V - R	2x6	DRY 2100F 1.8E	SPF
R - P	2x6	DRY 2100F 1.8E	SPF
P - M	2x6	DRY 2100F 1.8E	SPF
ALL WEBS 2x3 DRY No.2 SPF			
EXCEPT			
C - T	2x4	DRY No.2	SPF
D - S	2x4	DRY No.2	SPF
E - Q	2x4	DRY No.2	SPF
Q - H	2x4	DRY No.2	SPF
P - I	2x4	DRY No.2	SPF
O - J	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 1.25 3.50
C	TTWW+m	MT20	8.0	10.0 4.50 3.75
D	TMWW-t	MT20	5.0	6.0 2.25 2.00
E	TMWW-t	MT20	4.0	5.0 1.75 2.00
F	TMW+w	MT20	2.0	4.0
G	TS-t	MT20	4.0	8.0
H	TMWW-t	MT20	4.0	5.0 1.75 2.00
I	TMWW-t	MT20	5.0	6.0 2.25 2.00
J	TTWW+m	MT20	8.0	10.0 4.50 3.75
K	TMVW-p	MT20	5.0	8.0 1.25 3.50
M	BMV1+p	MT20	3.0	5.0
N	BMWW-t	MT20	4.0	6.0 2.00 1.50
O	BMWW-t	MT20	6.0	6.0 2.50 1.50
P	BSWW-t	MT20	10.0	10.0 Edge 5.00
Q	BMWWWW-t	MT20	4.0	10.0
R	BS-t	MT18HS	5.0	12.0
S	BMWW-t	MT20	5.0	6.0 2.50 1.75
T	BMWW-t	MT20	6.0	6.0 2.50 1.50
U	BMWW-t	MT20	4.0	6.0 2.00 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
V	2500	0	2500	0	0
M	2747	0	2747	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
V	1755	1223 / 0	0 / 0
M	1930	1341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 30	U-C	-686 / 0
B-C	-2507 / 0	C-T	0 / 3850
C-D	-5316 / 0	T-D	-1894 / 0
D-E	-7780 / 0	D-S	0 / 2863
E-F	-9399 / 0	S-E	-1322 / 0
F-G	-9399 / 0	E-Q	0 / 1880
G-H	-9399 / 0	Q-F	-402 / 0
H-I	-8714 / 0	Q-H	0 / 795
I-J	-5977 / 0	P-H	-858 / 0
J-K	-2768 / 0	P-I	0 / 3179
K-L	0 / 30	O-I	-2046 / 0
V-B	-2510 / 0	O-J	0 / 4378
M-K	-2751 / 0	N-J	-798 / 0
		B-U	0 / 2176
		N-K	0 / 2403
V-U	0 / 0		
U-T	0 / 1990		
T-S	0 / 5316		
S-R	0 / 7780		
R-Q	0 / 7780		
Q-W	0 / 8714		
W-P	0 / 8714		
P-X	0 / 5977		
X-O	0 / 5977		
O-N	0 / 2195		
N-M	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)		FACE	DIR.	TYPE
JT	LOC.	LC1	MAX-	MAX+
Q	17-1-4	-966	-966	---
X	22-6-12	-672	-672	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.3 PSF
TOTAL LOAD	= 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/883 (0.46")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/494 (0.83")

CSI: TC=0.86/1.00 (F-H:1), BC=0.56/1.00 (P-Q:1),
WB=0.77/1.00 (J-O:1), SSI=0.42/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES		SECTION	
PLATE	GRIP(DRY) SHEAR (PSI)	(PLI)	(PLI)
MT20	618 354 1667 822 2284 1656	MAX MIN	MAX MIN
MT18HS	580 354 2729 1405 4191 2010	MAX MIN	MAX MIN

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-009	G02	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 1	PAGE 6 OF 32
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:38:06 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-pHaOFg7umssdOOMdbR4zrhRVPaEtZJtWEfMaPszAl5V

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
V BMV1+p MT20 3.0 5.0

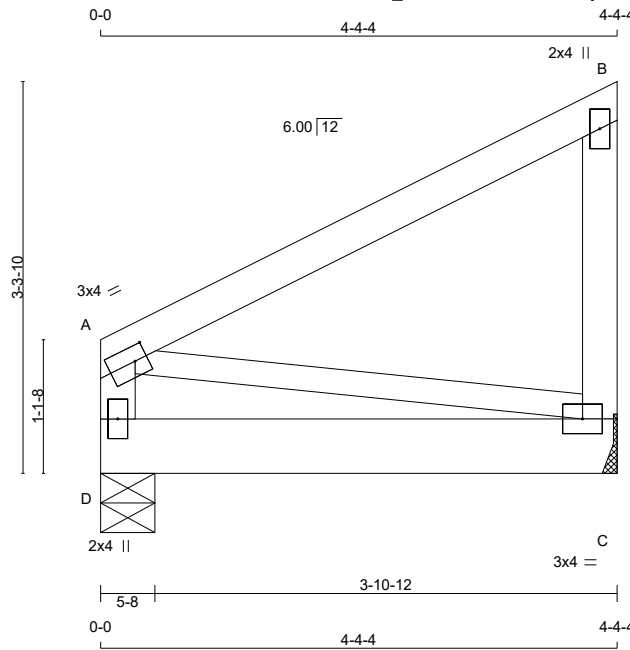
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 966.4 lbs FACTORED DOWN AT 17'-1-4,
AND 671.8 lbs FACTORED DOWN AT 22'-6-12 ON
BOTTOM CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.97 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.25
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	967	0	967	0	MECHANICAL	
D	967	0	967	0	5-8	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	679	471 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0
D	679	471 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. MEMB. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED CSI (LC)	MAX
FR-TO		FROM	TO					
A-B	0 / 0	-77.4	-77.4	0.27 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-168 / 0	0.0	0.0	0.03 (1)	7.81			
D-A	-168 / 0	0.0	0.0	0.02 (1)	7.81			
D-C	0 / 0	-366.8	-366.8	0.64 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 17-0-8
END DISTANCE = 4-4-4
END SPAN CARRIED = 17-0-8
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 586 (0.09")

CSI: TC=0.27/1.00 (A-B:1), BC=0.64/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.53/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



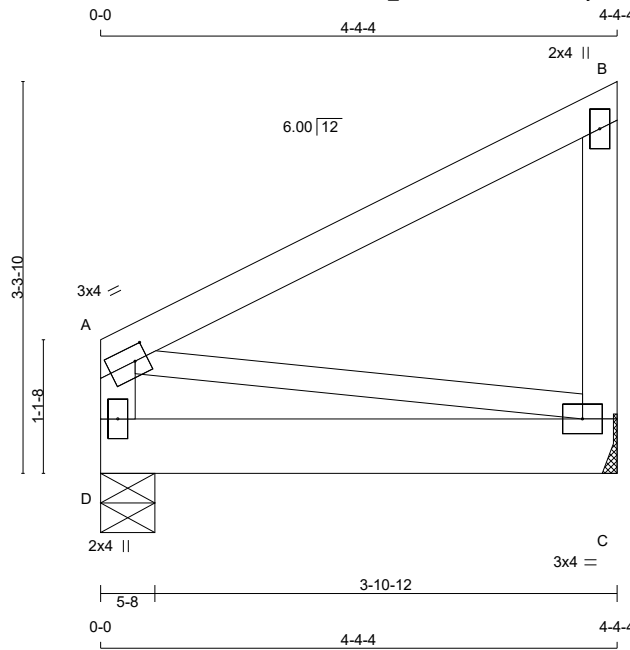
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 1	DRWG NO.
NE0618-009	G03	1	1	TRUSS DESC.		PAGE 8 OF 32

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:38:07 2018 Page 2
ID:9 xLMwZ3WDKc3QXh8cyzSnzB2V3-IU8mS08WXA U0Xxq99bCNv pl_ZrlyDgTJ57xIzAI5U

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.25
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	672	0	672	0	0	MECHANICAL
D	672	0	672	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	472	327 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
D	472	327 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LC)
FR-TO		FROM	TO					
A-B	0 / 0	-77.4	-77.4	0.27 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-168 / 0	0.0	0.0	0.03 (1)	7.81			
D-A	-168 / 0	0.0	0.0	0.02 (1)	7.81			
D-C	0 / 0	-231.4	-231.4	0.40 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 11-4-8
END DISTANCE = 4-4-4
END SPAN CARRIED = 11-4-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/931 (0.06")

CSI: TC=0.27/1.00 (A-B:1), BC=0.40/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.33/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



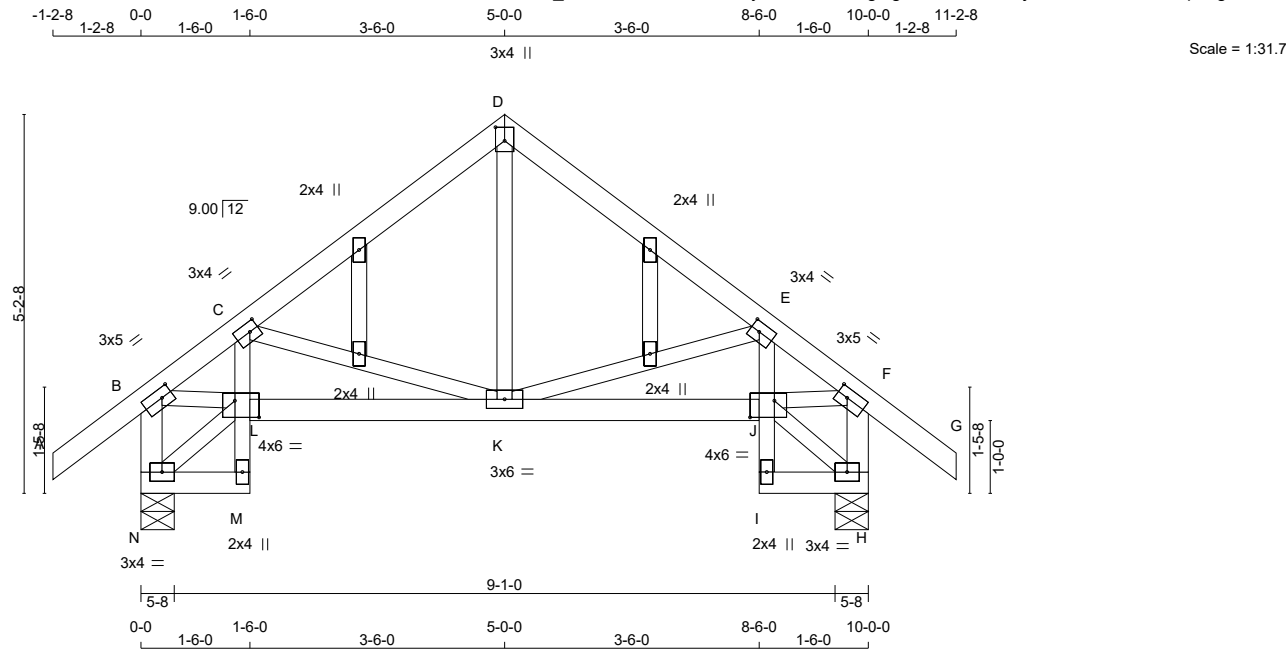
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - CELESTIAL 1 ELEV 1	DRWG NO.
NE0618-009	G04	1	1	TRUSS DESC.		PAGE 10 OF 32

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:38:07 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-IU8mS08WXA_U0Xxq99bCNv_pl_cXlyDgTJ57xIzAI5U

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x3	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x3	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	3.0	5.0	1.50	1.75
H	BMVW1-t	MT20	3.0	4.0		
I	BMV+p	MT20	2.0	4.0		
J	BVMWW-I	MT20	4.0	6.0	2.75	4.00
K	BMVWW-t	MT20	3.0	6.0		
L	BVMWW-I	MT20	4.0	6.0	2.75	4.00
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		
O, P, Q, R						
O	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	578	0	578	0	5-8	5-8
H	578	0	578	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	404	293 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
H	404	293 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-D	0 / 207
B-C	-645 / 0	-77.4	-77.4	0.08 (1)	6.25	K-E	-240 / 0
C-D	-384 / 0	-77.4	-77.4	0.12 (1)	6.25	C-K	-240 / 0
D-E	-384 / 0	-77.4	-77.4	0.12 (1)	6.25	N-L	-11 / 0
E-F	-645 / 0	-77.4	-77.4	0.08 (1)	6.25	B-L	0 / 518
F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00	J-H	-11 / 0
N-B	-559 / 0	0.0	0.0	0.06 (1)	7.81	J-F	0 / 518
H-F	-559 / 0	0.0	0.0	0.06 (1)	7.81		
N-M	0 / 9	-18.2	-18.2	0.01 (4)	10.00		
M-L	0 / 14	0.0	0.0	0.03 (1)	10.00		
L-C	0 / 38	0.0	0.0	0.03 (1)	10.00		
L-K	0 / 526	-18.3	-18.3	0.12 (1)	10.00		
K-J	0 / 526	-18.3	-18.3	0.12 (1)	10.00		
I-J	0 / 14	0.0	0.0	0.03 (1)	10.00		
J-E	0 / 38	0.0	0.0	0.03 (1)	10.00		
I-H	0 / 9	-18.2	-18.2	0.01 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	PSF
DL	=	3.0	PSF
BOT CH.	LL	=	0.0 PSF
DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.12/1.00 (C-D:1), BC=0.12/1.00 (K-L:1), WB=0.12/1.00 (B-L:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

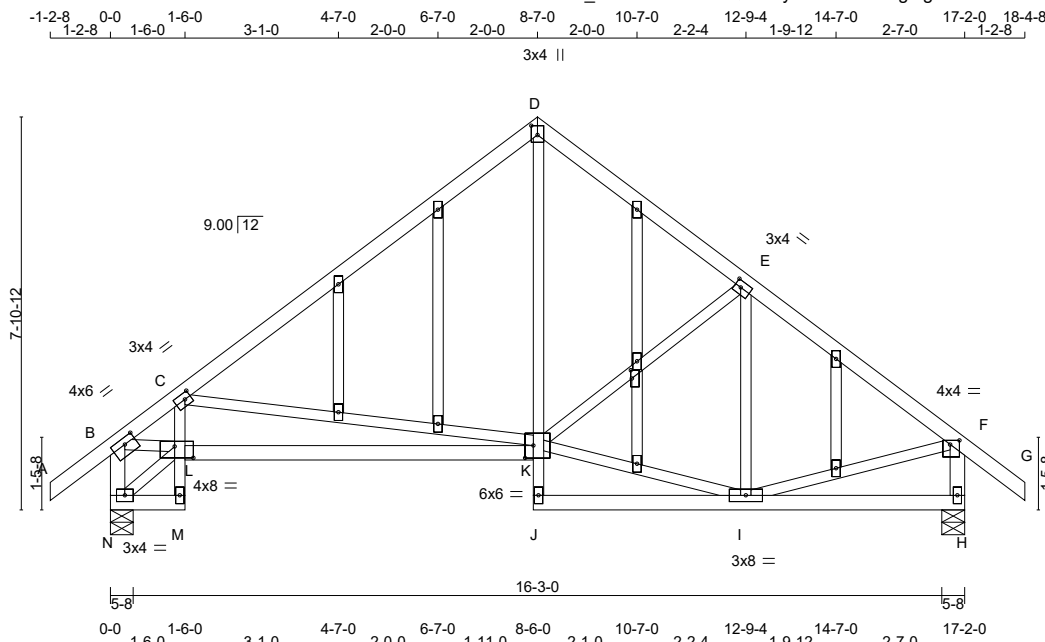
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 91 lb [M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x3	DRY	No.2
L - K	2x4	DRY	No.2
J - D	2x3	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.50 2.75
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TTV+p	MT20	3.0	4.0	2.25 1.50
E	TMWW-t	MT20	3.0	4.0	1.50 1.50
F	TMVW-p	MT20	4.0	4.0	1.00 2.25
H	BMV1+p	MT20	2.0	4.0	
I	BMVWW-t	MT20	3.0	8.0	
J	BMV+p	MT20	2.0	4.0	
K	BVMWWW-I	MT20	6.0	6.0	3.00 2.00
L	BVMWW-I	MT20	4.0	8.0	2.75 4.50
M	BMV+p	MT20	2.0	4.0	
N	BMVW1-t	MT20	3.0	4.0	
O, P, Q, R, S, T, U, V, W					
O	NP+w	MT20	2.0	4.0	
U	NP+w	MT20	2.0	4.0	2.00 0.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
H	917	0	917	0	0	5-8	5-8	5-8	
N	925	0	925	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
H	642	458 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0		
N	647	462 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	C-K	-725 / 0
B-C	-1401 / 0	-77.4	-77.4	0.41 (1)	4.99	K-I	0 / 661
C-D	-695 / 0	-77.4	-77.4	0.50 (1)	6.25	K-E	-152 / 0
D-E	-688 / 0	-77.4	-77.4	0.18 (1)	6.25	I-E	-227 / 11
E-F	-787 / 0	-77.4	-77.4	0.19 (1)	6.25	I-F	0 / 669
F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-L	-24 / 0
N-B	-898 / 0	0.0	0.0	0.09 (1)	7.81	B-L	0 / 1229
H-F	-885 / 0	0.0	0.0	0.09 (1)	7.81		
N-M	0 / 19	-18.2	-18.2	0.01 (4)	10.00		
M-L	0 / 14	0.0	0.0	0.06 (1)	10.00		
L-C	0 / 76	0.0	0.0	0.06 (1)	10.00		
L-K	0 / 1246	-18.2	-18.2	0.36 (1)	10.00		
J-K	0 / 32	0.0	0.0	0.02 (1)	10.00		
K-D	0 / 449	0.0	0.0	0.12 (1)	10.00		
J-I	0 / 5	-18.2	-18.2	0.09 (4)	10.00		
I-H	0 / 0	-18.2	-18.2	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.50/1.00 (C-D:1), BC=0.36/1.00 (K-L:1), WB=0.78/1.00 (C-K:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

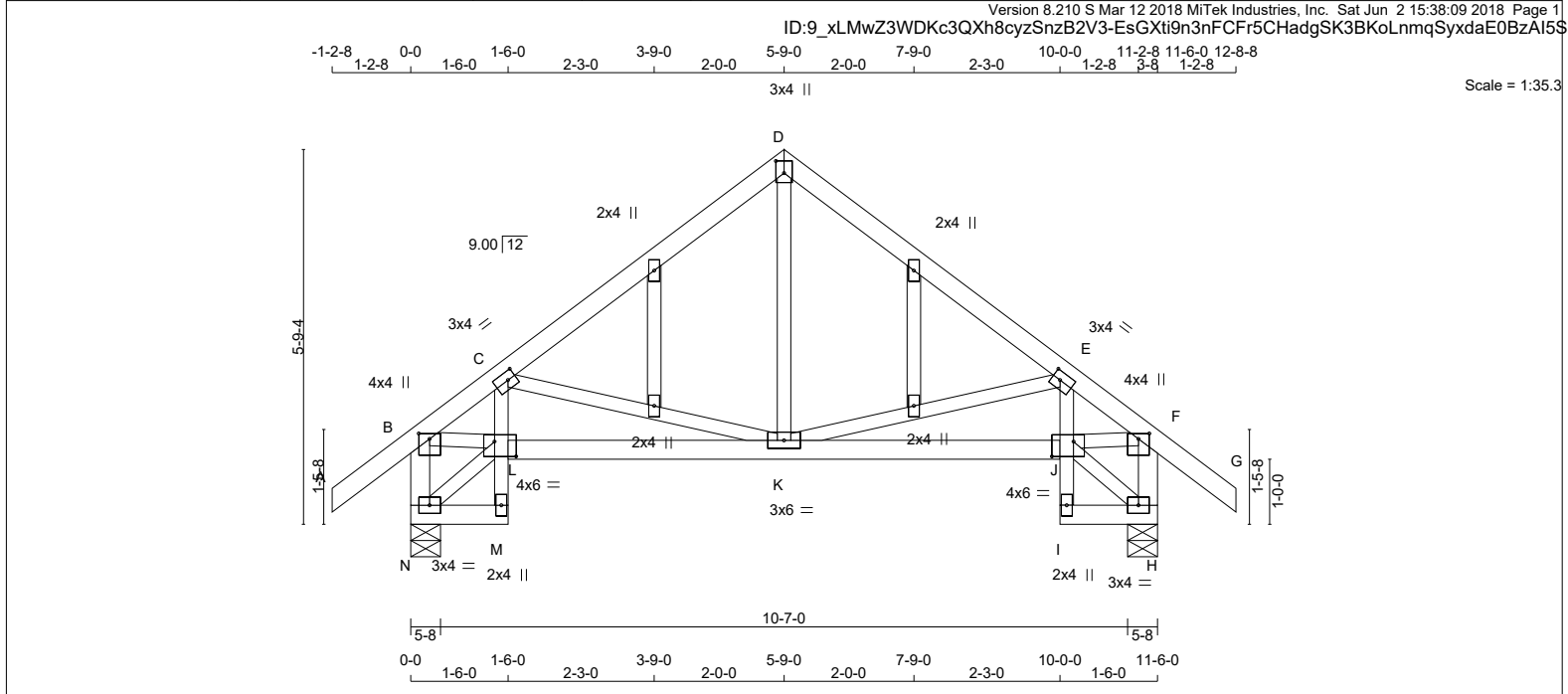
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





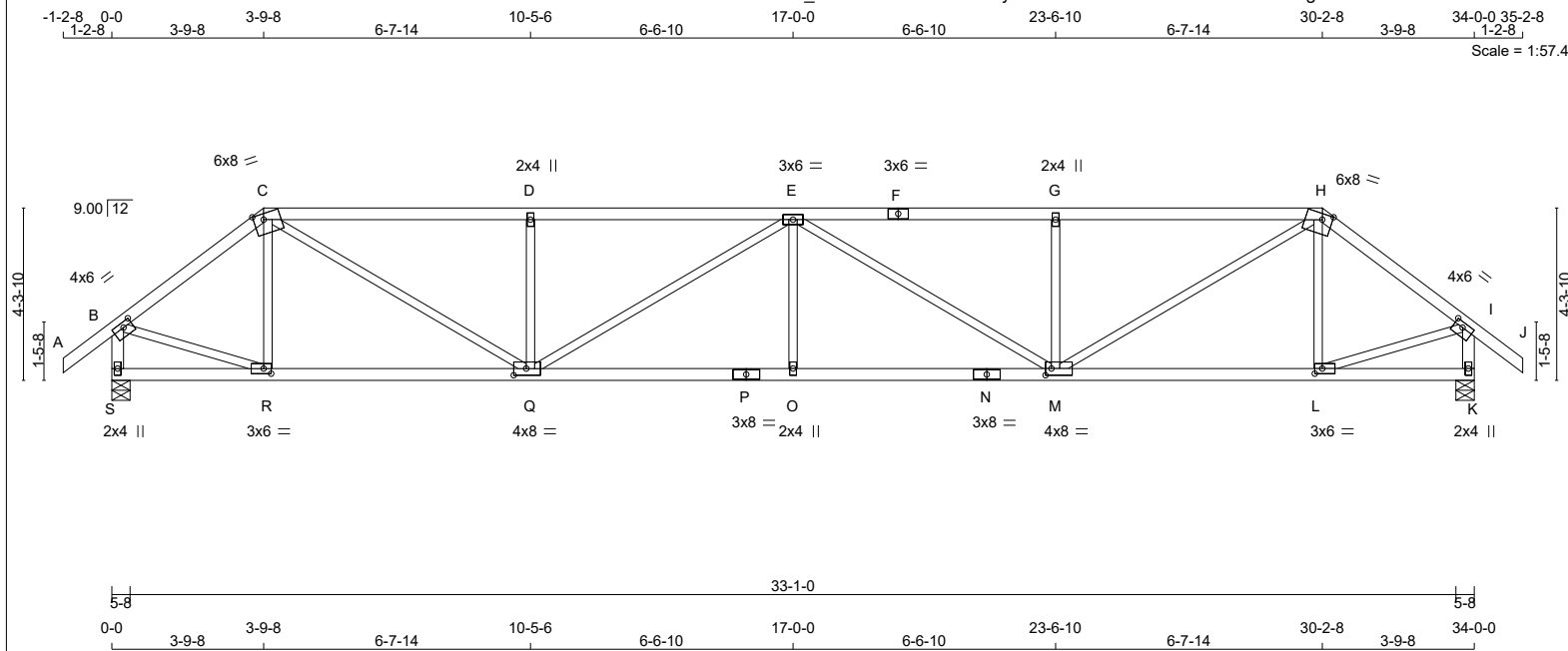
TOTAL WEIGHT = 57 lb [M][F]

N. L. G. A. RULES				BUILDING DESIGNER				DESIGN CRITERIA															
CHORDS				LUMBER				DESCR.				SPECIFIED LOADS:											
A - D				2x4				DRY				No.2				TOP CH. LL = 23.3 PSF							
D - G				2x4				DRY				No.2				DL = 3.0 PSF							
N - B				2x4				DRY				No.2				BOT CH. LL = 0.0 PSF							
H - F				2x4				DRY				No.2				DL = 7.3 PSF							
N - M				2x4				DRY				No.2				TOTAL LOAD = 33.6 PSF							
M - C				2x3				DRY				No.2				SPACING = 24.0 IN. C/C							
L - J				2x4				DRY				No.2				THIS TRUSS IS DESIGNED FOR RESIDENTIAL							
I - E				2x3				DRY				No.2				OR SMALL BUILDING REQUIREMENTS OF							
I - H				2x4				DRY				No.2				PART 9, NBCC 2010							
ALL WEBS EXCEPT				2x3				DRY				No.2				THIS DESIGN COMPLIES WITH:							
ALL GABLE WEBS EXCEPT				2x3				DRY				No.2				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014							
DRY: SEASONED LUMBER.				2x3				DRY				No.2				- CSA 086-09							
GABLE STUDS SPACED AT 2-0-0 OC.				2x3				DRY				No.2				- TPIC 2011							
PLATES (table is in inches)				2x3				DRY				No.2				DESIGN ASSUMPTIONS							
JT TYPE				PLATES				W				LEN				Y				X			
B				TMVW+p				MT20				4.0				4.0				1.00 2.00			
C				TMVW-t				MT20				3.0				4.0				1.50 1.50			
D				TTW+p				MT20				3.0				4.0				2.25 1.50			
E				TMVW-t				MT20				3.0				4.0				1.50 1.50			
F				TMVW+p				MT20				4.0				4.0				1.00 2.00			
H				BMVW1-t				MT20				3.0				4.0							
I				BMV+p				MT20				2.0				4.0							
J				BVMWW-I				MT20				4.0				6.0				2.75 4.00			
K				BMVWW-t				MT20				3.0				6.0							
L				BVMWW-I				MT20				4.0				6.0				2.75 4.00			
M				BMV+p				MT20				2.0				4.0							
N				BMVW1-t				MT20				3.0				4.0							
O, P, Q, R																							
O				NP+w				MT20				2.0				4.0							
CHORDS				FACTORED				FACTORED				WEBS				FACTORED							
MEMB.				FORCE				VERT. LOAD				MEMB.				FORCE				MAX.			
				(LBS)				(PLF)								(LBS)				CSI (LC)			
FR-TO				FROM				TO				FR-TO				FROM				TO			
A-B				0 / 30				-77.4				-77.4				0.09 (1)				10.00			
B-C				-789 / 0				-77.4				-77.4				0.13 (1)				6.25			
C-D				-454 / 0				-77.4				-77.4				0.18 (1)				6.25			
D-E				-454 / 0				-77.4				-77.4				0.18 (1)				6.25			
E-F				-789 / 0				-77.4				-77.4				0.13 (1)				6.25			
F-G				0 / 30				-77.4				-77.4				0.09 (1)				10.00			
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
M-L				0 / 14				0.0				0.0				0.03 (1)				10.00			
L-C				0 / 42				0.0				0.0				0.04 (1)				10.00			
L-K				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
K-J				0 / 658				-18.2				-18.2				0.16 (1)				10.00			
I-J				0 / 14				0.0				0.0				0.03 (1)				10.00			
J-E				0 / 42				0.0				0.0				0.04 (1)				10.00			
I-H				0 / 11				-18.2				-18.2				0.01 (4)				10.00			
J-F				0 / 648				-13 / 0				0.00 (1)				0.15 (1)							
N-B				-629 / 0				0.0				0.0				0.07 (1)				7.81			
H-F				-629 / 0				0.0				0.0				0.07 (1)				7.81			
N-M																							



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TTVW-m	MT20	6.0	8.0	1.75	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	3.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTVW-m	MT20	6.0	8.0	1.75	3.00
I	TMVW-t	MT20	4.0	6.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	3.0	6.0	1.50	2.25
M	BMWWW-t	MT20	4.0	8.0	2.00	1.75
N	BS-t	MT20	3.0	8.0		
O	BMW+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	8.0	2.00	3.75
R	BMWW-t	MT20	3.0	6.0	1.50	2.25
S	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
S	1726	0	1726	0	5-8	5-8
K	1726	0	1726	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
K	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED VERT. LOAD (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	R-C	-293 / 0	0.08 (1)
B-C	-1748 / 0	-77.4	-77.4	0.25 (1)	4.80	C-Q	0 / 1916	0.43 (1)
C-D	-3030 / 0	-77.4	-77.4	0.83 (1)	3.29	Q-D	-573 / 0	0.16 (1)
D-E	-3031 / 0	-77.4	-77.4	0.82 (1)	3.29	Q-E	-565 / 0	0.67 (1)
E-F	-3031 / 0	-77.4	-77.4	0.82 (1)	3.29	O-E	0 / 132	0.05 (4)
F-G	-3031 / 0	-77.4	-77.4	0.82 (1)	3.29	E-M	-565 / 0	0.67 (1)
G-H	-3030 / 0	-77.4	-77.4	0.83 (1)	3.29	M-G	-573 / 0	0.16 (1)
H-I	-1748 / 0	-77.4	-77.4	0.25 (1)	4.80	M-H	0 / 1916	0.43 (1)
I-J	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-H	-293 / 0	0.08 (1)
S-B	-1705 / 0	0.0	0.0	0.18 (1)	6.36	B-R	0 / 1453	0.33 (1)
K-I	-1705 / 0	0.0	0.0	0.18 (1)	6.36	L-I	0 / 1453	0.33 (1)
S-R	0 / 0	-18.2	-18.2	0.13 (4)	10.00			
R-Q	0 / 1388	-18.2	-18.2	0.30 (1)	10.00			
Q-P	0 / 3512	-18.2	-18.2	0.63 (1)	10.00			
P-O	0 / 3512	-18.2	-18.2	0.63 (1)	10.00			
O-N	0 / 3512	-18.2	-18.2	0.63 (1)	10.00			
N-M	0 / 3512	-18.2	-18.2	0.63 (1)	10.00			
M-L	0 / 1388	-18.2	-18.2	0.30 (1)	10.00			
L-K	0 / 0	-18.2	-18.2	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/990 (0.41")

CSI: TC=0.83/1.00 (G-H:1), BC=0.63/1.00 (O-Q:1), WB=0.67/1.00 (E-Q:1), SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

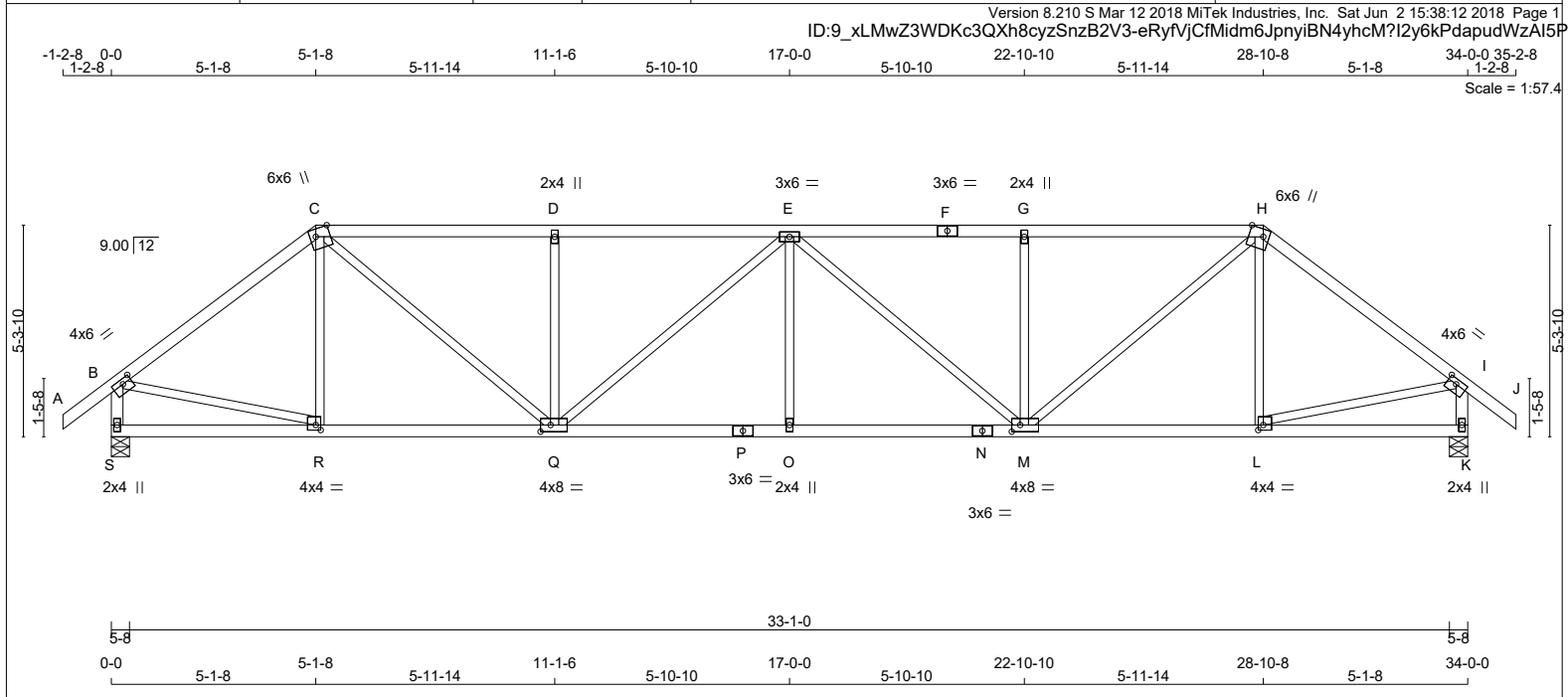
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.93 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 138 = 276 lb [M]

N. L. G. A. RULES				BUILDING DESIGNER							DESIGN CRITERIA				
CHORDS		SIZE	LUMBER	DESCR.	BEARINGS		FACTORED		MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:			
					GROSS REACTION		GROSS REACTION			BRG	BRG				
					JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX				
A - C	2x4	DRY	No.2	SPF	S	1726	0	1726	0	0	5-8	5-8	TOP CH.	LL	= 23.3 PSF
C - F	2x4	DRY	No.2	SPF	K	1726	0	1726	0	0	5-8	5-8	DL	= 3.0 PSF	
F - H	2x4	DRY	No.2	SPF								BOT CH.	LL	= 0.0 PSF	
H - J	2x4	DRY	No.2	SPF								DL	= 7.3 PSF		
S - B	2x4	DRY	No.2	SPF								TOTAL LOAD	= 33.6 PSF		
K - I	2x4	DRY	No.2	SPF											
S - P	2x4	DRY	No.2	SPF											
P - N	2x4	DRY	No.2	SPF											
N - K	2x4	DRY	No.2	SPF											
					UNFACTORED REACTIONS							SPACING = 24.0 IN. C/C			
					1ST LCASE		MAX./MIN. COMPONENT REACTIONS								
					JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
ALL WEBS	2x3	DRY	No.2	SPF	S	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0	LOADING IN FLAT SECTION BASED ON A		
EXCEPT					K	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0	SLOPE OF 2.00/12 MINIMUM		

DRY: SEASONED LUMBER.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	R-C	-195 / 32
B-C	-1795 / 0	-77.4	-77.4	0.46 (1)	4.51	C-Q	0 / 1381
C-D	-2490 / 0	-77.4	-77.4	0.57 (1)	3.83	Q-D	-515 / 0
D-E	-2490 / 0	-77.4	-77.4	0.57 (1)	3.83	Q-E	-410 / 0
E-F	-2490 / 0	-77.4	-77.4	0.57 (1)	3.83	O-E	0 / 121
F-G	-2490 / 0	-77.4	-77.4	0.57 (1)	3.83	E-M	-410 / 0
G-H	-2490 / 0	-77.4	-77.4	0.57 (1)	3.83	M-G	-515 / 0
H-I	-1795 / 0	-77.4	-77.4	0.46 (1)	4.51	M-H	0 / 1381
I-J	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-H	-195 / 32
S-B	-1689 / 0	0.0	0.0	0.18 (1)	6.39	B-R	0 / 1467
K-I	-1689 / 0	0.0	0.0	0.18 (1)	6.39	L-I	0 / 1467
S-R	0 / 0	-18.2	-18.2	0.13 (4)	10.00		
R-Q	0 / 1430	-18.2	-18.2	0.31 (1)	10.00		
Q-P	0 / 2802	-18.2	-18.2	0.51 (1)	10.00		
P-O	0 / 2802	-18.2	-18.2	0.51 (1)	10.00		
O-N	0 / 2802	-18.2	-18.2	0.51 (1)	10.00		
N-M	0 / 2802	-18.2	-18.2	0.51 (1)	10.00		
M-L	0 / 1430	-18.2	-18.2	0.31 (1)	10.00		
L-K	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.28")

CSI: TC=0.57/1.00 (C-D:1), BC=0.51/1.00 (O-Q:1), WB=0.49/1.00 (E-Q:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

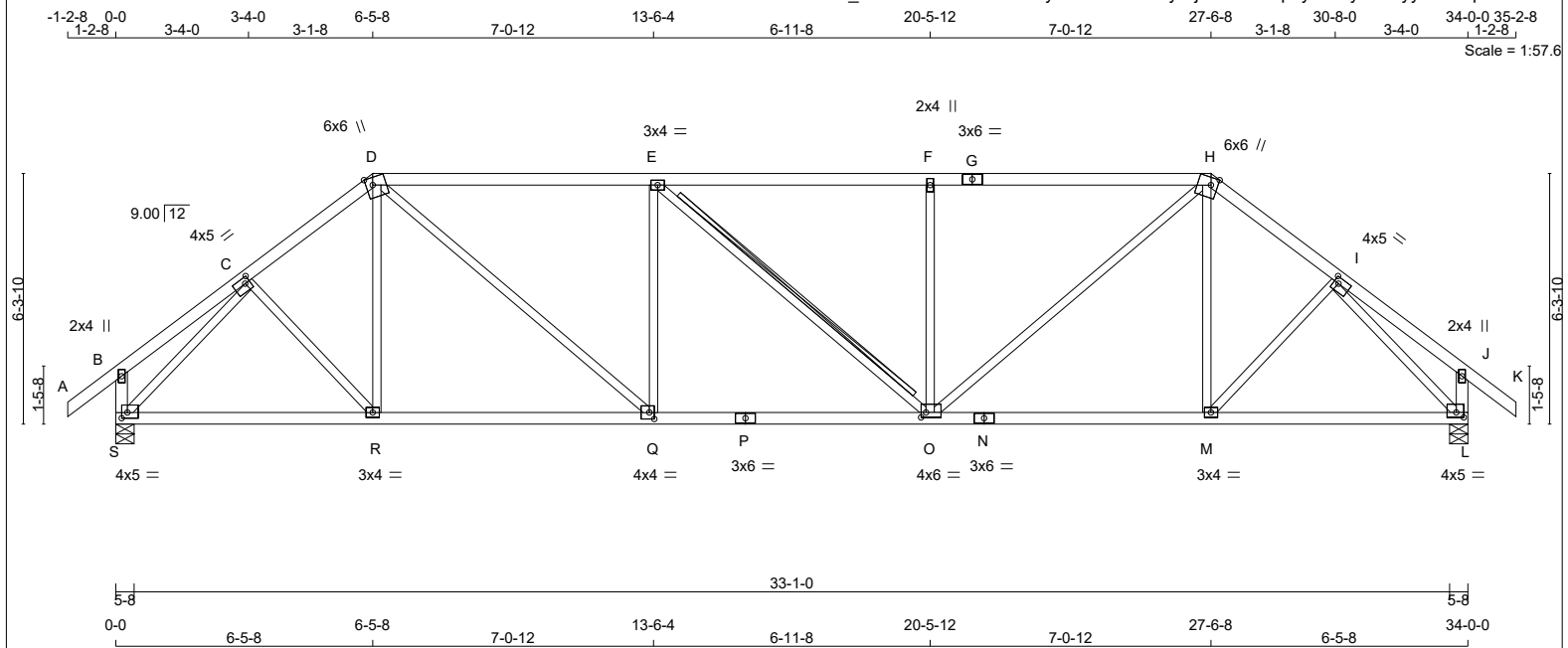
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

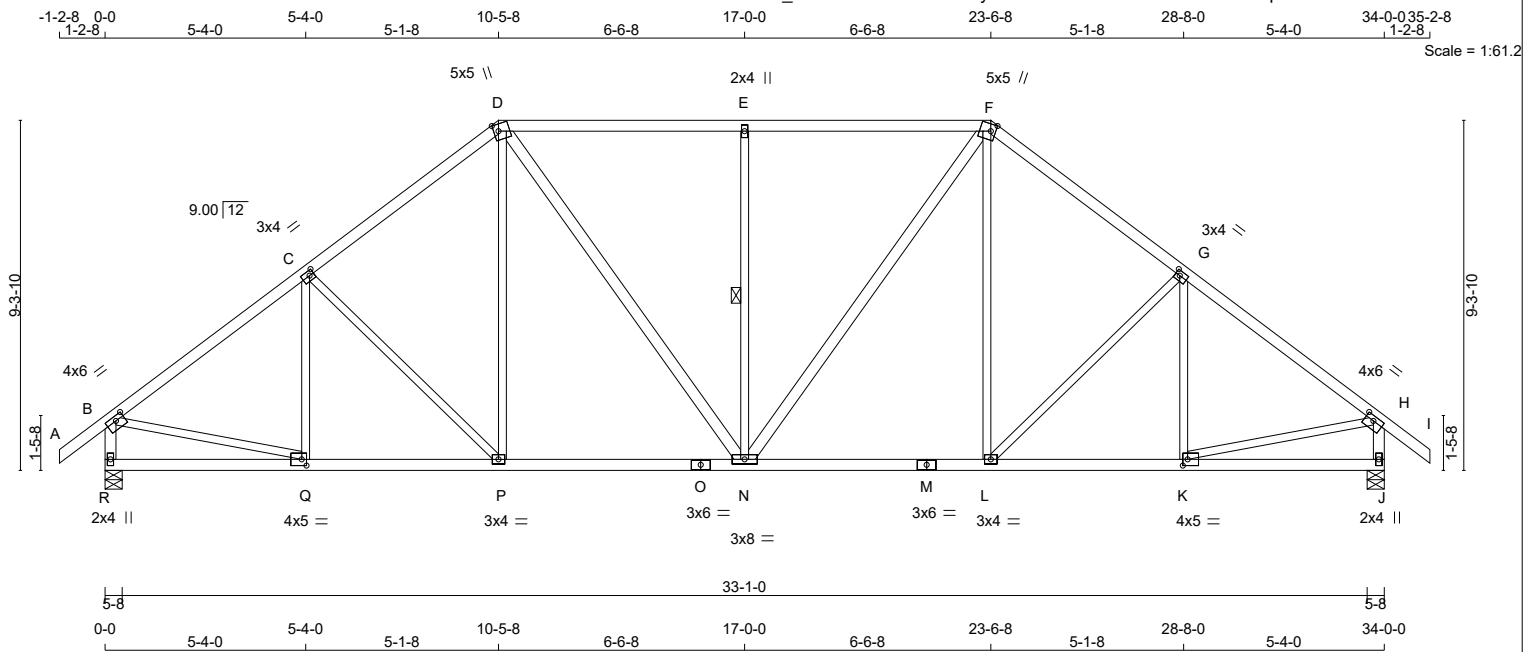
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
 JSI METAL= 0.71 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS	SIZE	LUMBER	DESCR.
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.25	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	5.0	2.00	1.50
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	3.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	4.0		
Q	BMWW-t	MT20	4.0	5.0	2.00	1.50
R	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1726	0	1726	0	0	5-8	5-8
J	1726	0	1726	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
J	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20-0-0. CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	Q-C	-213 / 23	0.10 (1)
B-C	-1823 / 0	-77.4 -77.4	0.34 (1)	4.65	C-P	-237 / 0	0.25 (1)
C-D	-1672 / 0	-77.4 -77.4	0.32 (1)	4.83	P-D	0 / 273	0.06 (1)
D-E	-1585 / 0	-77.4 -77.4	0.47 (1)	4.70	D-N	0 / 459	0.07 (1)
E-F	-1585 / 0	-77.4 -77.4	0.47 (1)	4.70	N-E	-620 / 0	0.35 (1)
F-G	-1672 / 0	-77.4 -77.4	0.32 (1)	4.83	N-F	0 / 459	0.07 (1)
G-H	-1823 / 0	-77.4 -77.4	0.34 (1)	4.65	L-F	0 / 273	0.06 (1)
H-I	0 / 30	-77.4 -77.4	0.09 (1)	10.00	L-G	-237 / 0	0.25 (1)
R-B	-1684 / 0	0.0 0.0	0.17 (1)	6.40	K-G	-213 / 23	0.10 (1)
J-H	-1684 / 0	0.0 0.0	0.17 (1)	6.40	B-Q	0 / 1514	0.34 (1)
					K-H	0 / 1514	0.34 (1)
R-Q	0 / 0	-18.2 -18.2	0.11 (4)	10.00			
Q-P	0 / 1481	-18.2 -18.2	0.31 (1)	10.00			
P-O	0 / 1315	-18.2 -18.2	0.30 (1)	10.00			
O-N	0 / 1315	-18.2 -18.2	0.30 (1)	10.00			
N-M	0 / 1315	-18.2 -18.2	0.30 (1)	10.00			
M-L	0 / 1315	-18.2 -18.2	0.30 (1)	10.00			
L-K	0 / 1481	-18.2 -18.2	0.31 (1)	10.00			
K-J	0 / 0	-18.2 -18.2	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.47/1.00 (D-E:1), BC=0.31/1.00 (P-Q:1), WB=0.35/1.00 (E-N:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

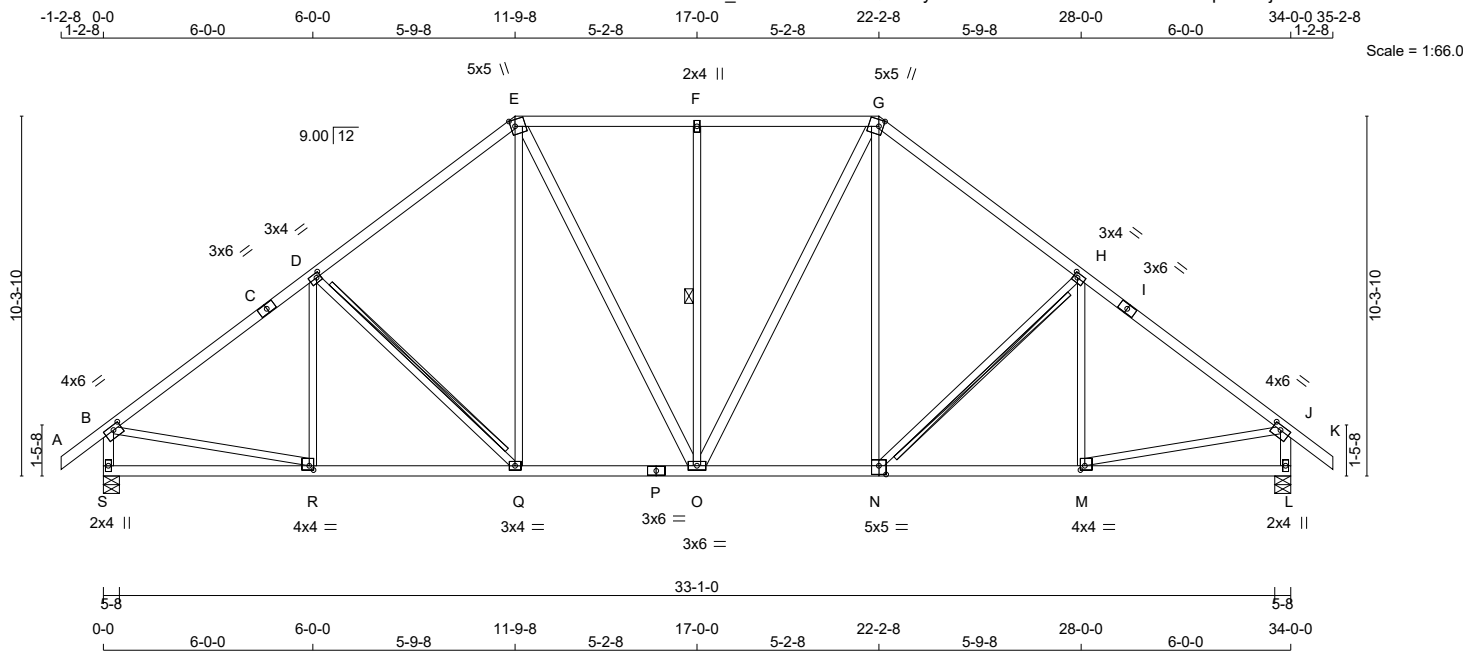
PLATE ROTATION TOL. = 5.0 Deg.

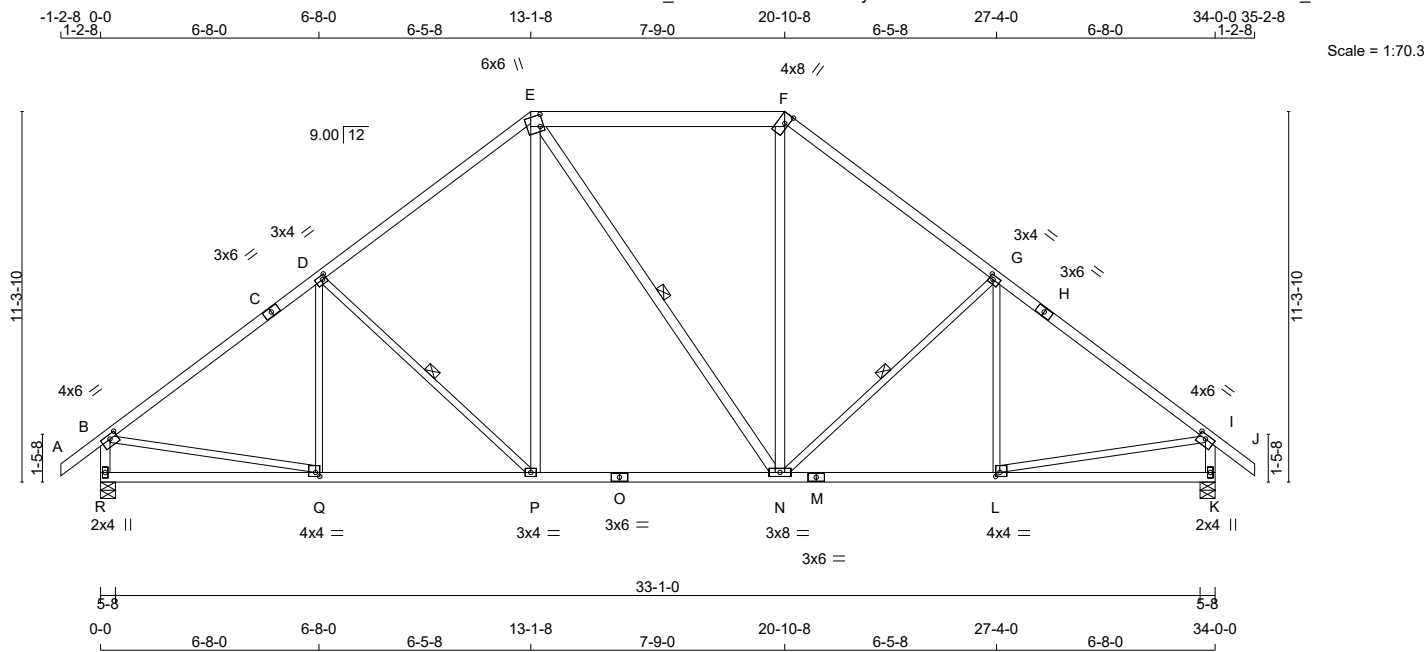
JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.







TOTAL WEIGHT = 3 X 169 = 507 lb
[M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - F	2x6	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
P - E	2x4	DRY No.2	SPF
E - N	2x4	DRY No.2	SPF
N - F	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.50 2.75
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	3.0	4.0	1.50 1.50
E	TTWW+m	MT20	6.0	6.0	4.25 1.25
F	TTW+h	MT20	4.0	8.0	Edge 1.50
G	TMWW-t	MT20	3.0	4.0	1.50 1.50
H	TS-t	MT20	3.0	6.0	
I	TMVW-t	MT20	4.0	6.0	1.50 2.75
K	BMV1+p	MT20	2.0	4.0	
L	BMWW-t	MT20	4.0	4.0	1.50 1.50
M	BS-t	MT20	3.0	6.0	
N	BMWWW-t	MT20	3.0	8.0	
O	BS-t	MT20	3.0	6.0	
P	BMWW-t	MT20	3.0	4.0	
Q	BMWW-t	MT20	4.0	4.0	1.50 1.50
R	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1726	0	1726	0	5-8
K	1726	0	1726	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0
K	1210	852 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-P, E-N, G-N. DBS = 20-0-0. CBF = 51 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	Q-D	-123 / 73	
B-C	-1837 / 0	-77.4 -77.4	0.56 (1)	4.38	D-P	-409 / 0	
C-D	-1837 / 0	-77.4 -77.4	0.56 (1)	4.38	P-E	0 / 407	
D-E	-1533 / 0	-77.4 -77.4	0.51 (1)	4.75	E-N	0 / 0	
E-F	-1204 / 0	-77.4 -77.4	0.30 (1)	6.25	N-F	0 / 408	
F-G	-1533 / 0	-77.4 -77.4	0.51 (1)	4.75	N-G	-409 / 0	
G-H	-1836 / 0	-77.4 -77.4	0.56 (1)	4.38	L-G	-124 / 73	
H-I	-1836 / 0	-77.4 -77.4	0.56 (1)	4.38	B-Q	0 / 1520	
I-J	0 / 30	-77.4 -77.4	0.09 (1)	10.00	L-I	0 / 1520	
R-B	-1675 / 0	0.0 0.0	0.17 (1)	6.41			
K-I	-1675 / 0	0.0 0.0	0.17 (1)	6.41			
R-Q	0 / 0	-18.2 -18.2	0.18 (4)	10.00			
Q-P	0 / 1498	-18.2 -18.2	0.34 (1)	10.00			
P-O	0 / 1204	-18.2 -18.2	0.30 (1)	10.00			
O-N	0 / 1204	-18.2 -18.2	0.30 (1)	10.00			
N-M	0 / 1498	-18.2 -18.2	0.35 (1)	10.00			
M-L	0 / 1498	-18.2 -18.2	0.35 (1)	10.00			
L-K	0 / 0	-18.2 -18.2	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.56/1.00 (B-D:1), BC=0.35/1.00 (L-N:1), WB=0.34/1.00 (B-Q:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

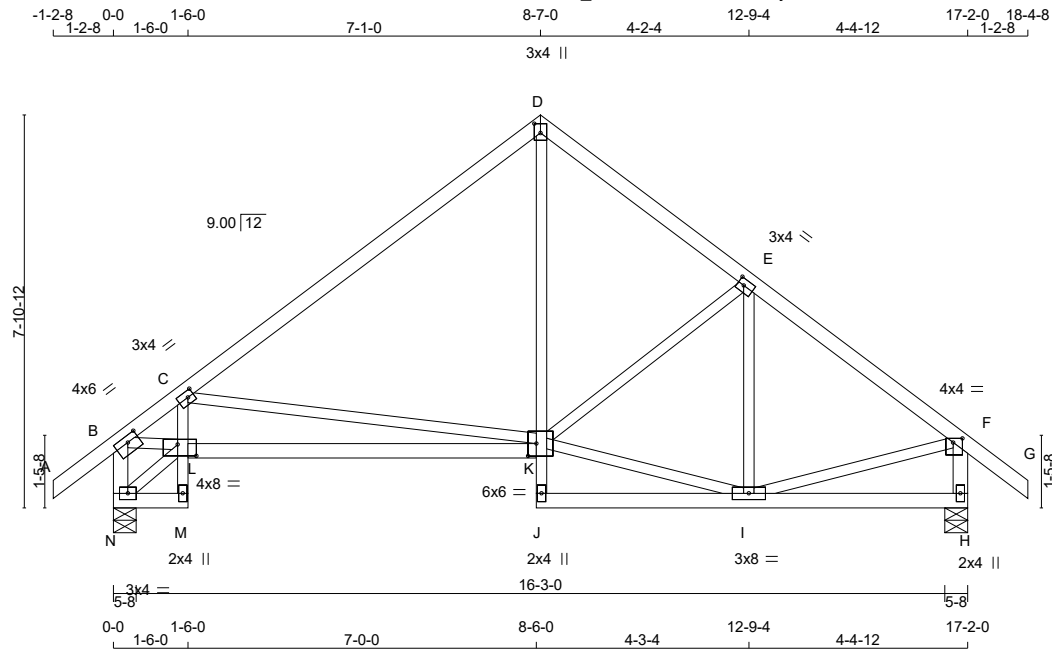
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Q) (INPUT = 0.90)
JSI METAL= 0.56 (Q) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

CONTINUED ON PAGE 2





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
J - D	2x3	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTV+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	2.0	4.0		
I	BMVWW-t	MT20	3.0	8.0		
J	BMV+p	MT20	2.0	4.0		
K	BVMVWW-I	MT20	6.0	6.0	3.00	2.00
L	BVMVWW-I	MT20	4.0	8.0	2.75	4.50
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	917	0	917	0	5-8	5-8
N	925	0	925	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	642	458 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
N	647	462 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	C-K	-725 / 0	0.78 (1)
B-C	-1401 / 0	-77.4	-77.4 0.41 (1)	4.99	K-I	0 / 661	0.15 (1)
C-D	-695 / 0	-77.4	-77.4 0.50 (1)	6.25	K-E	-152 / 0	0.07 (1)
D-E	-688 / 0	-77.4	-77.4 0.18 (1)	6.25	I-E	-227 / 11	0.08 (1)
E-F	-787 / 0	-77.4	-77.4 0.19 (1)	6.25	I-F	0 / 669	0.15 (1)
F-G	0 / 30	-77.4	-77.4 0.09 (1)	10.00	N-L	-24 / 0	0.00 (1)
N-B	-898 / 0	0.0	0.0 0.09 (1)	7.81	B-L	0 / 1229	0.28 (1)
H-F	-885 / 0	0.0	0.0 0.09 (1)	7.81			
N-M	0 / 19	-18.2	-18.2 0.01 (4)	10.00			
M-L	0 / 14	0.0	0.0 0.06 (1)	10.00			
L-C	0 / 76	0.0	0.0 0.06 (1)	10.00			
L-K	0 / 1246	-18.2	-18.2 0.36 (1)	10.00			
J-K	0 / 32	0.0	0.0 0.02 (1)	10.00			
K-D	0 / 449	0.0	0.0 0.12 (1)	10.00			
J-I	0 / 5	-18.2	-18.2 0.09 (4)	10.00			
I-H	0 / 0	-18.2	-18.2 0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.50/1.00 (C-D:1), BC=0.36/1.00 (K-L:1), WB=0.78/1.00 (C-K:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

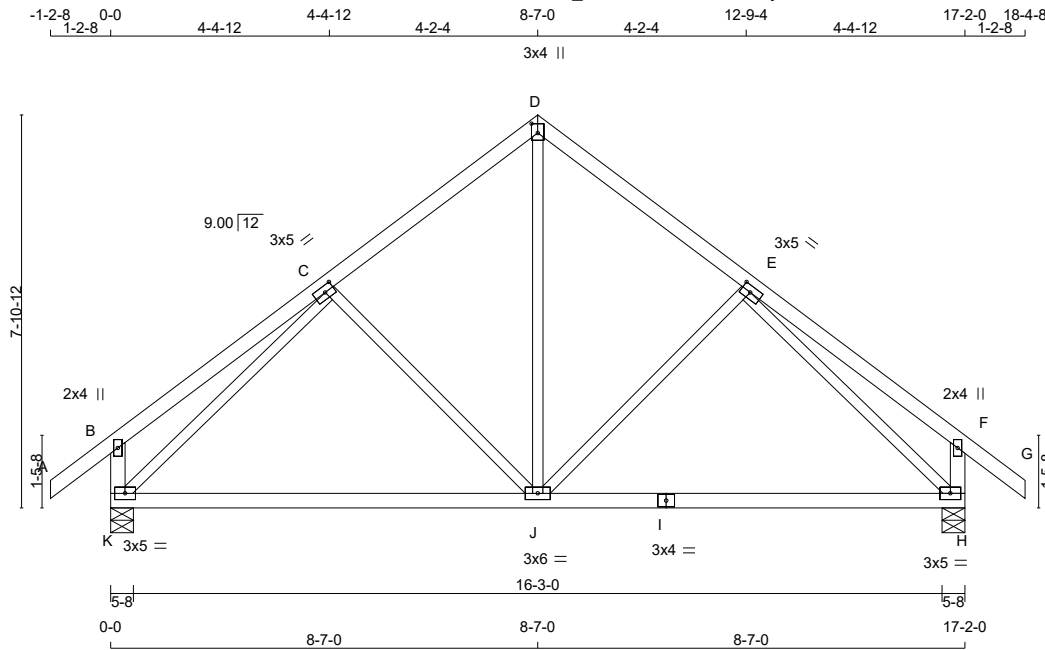
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:46.3

TOTAL WEIGHT = 2 X 75 = 150 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.25
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	5.0	1.50	2.25
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	5.0		
I	BS-t	MT20	3.0	4.0		
J	BMVW1-t	MT20	3.0	6.0		
K	BMVW1-t	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
K	921	0	921	0	5-8	5-8
H	921	0	921	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	645	460 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB.	WEBS		MAX. UNBRAC LENGTH
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	CSi (LC)			MAX. FACTORED FORCE (LBS)	MAX. CSi (LC)	
FR-TO			FROM	TO		FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	J-D	0 / 474	0.11 (1)	
B-C	0 / 23	-77.4	-77.4	0.23 (1)	10.00	J-E	-197 / 0	0.13 (1)	
C-D	-632 / 0	-77.4	-77.4	0.18 (1)	6.25	C-J	-197 / 0	0.13 (1)	
D-E	-632 / 0	-77.4	-77.4	0.18 (1)	6.25	K-C	-891 / 0	0.56 (1)	
E-F	0 / 23	-77.4	-77.4	0.23 (1)	10.00	E-H	-891 / 0	0.56 (1)	
F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00				
K-B	-227 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-227 / 0	0.0	0.0	0.02 (1)	7.81				
K-J	0 / 626	-18.2	-18.2	0.43 (4)	10.00				
J-I	0 / 626	-18.2	-18.2	0.43 (4)	10.00				
I-H	0 / 626	-18.2	-18.2	0.43 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.23/1.00 (B-C:1), BC=0.43/1.00 (H-J:4), WB=0.56/1.00 (C-K:1), SSI=0.13/1.00 (J-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

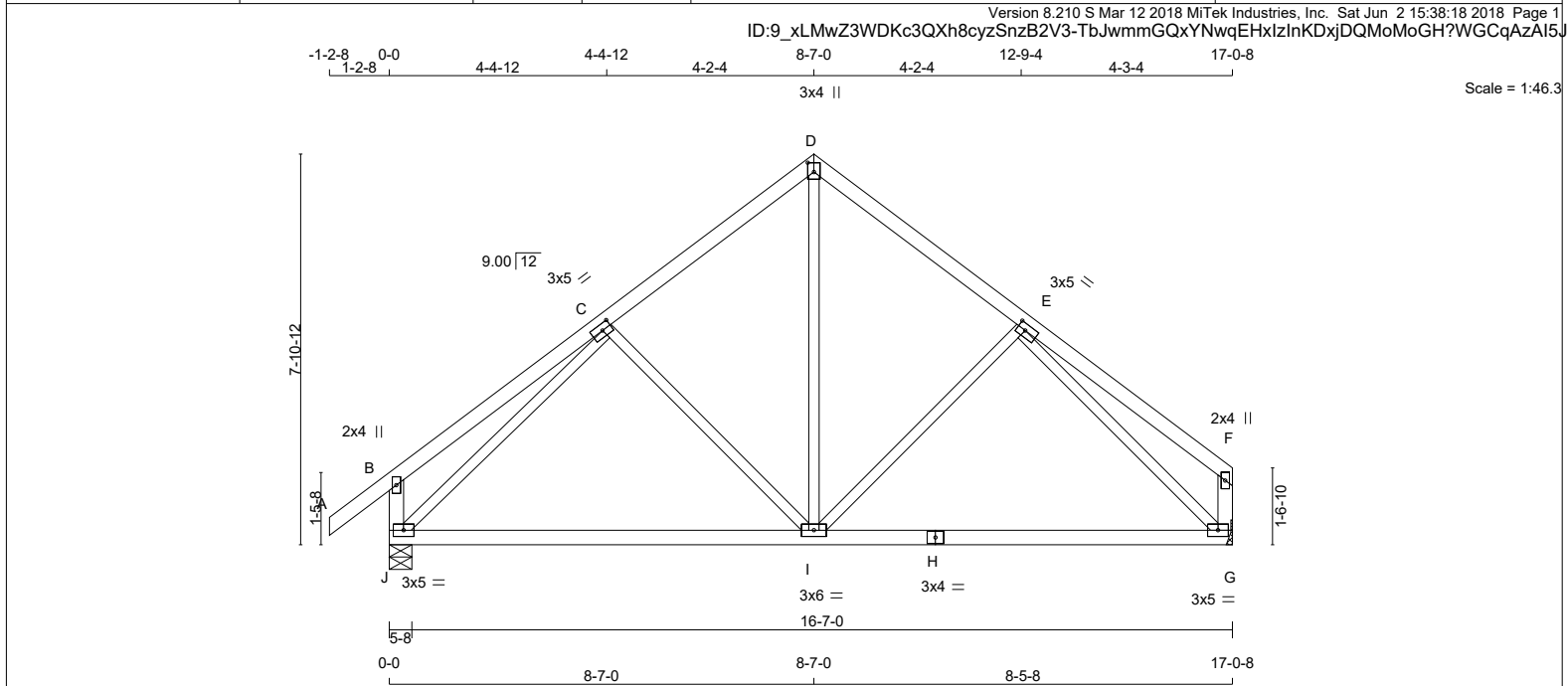
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 73 = 146 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.25
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	5.0		
H	BS-t	MT20	3.0	4.0		
I	BMVWW-t	MT20	3.0	6.0		
J	BMVW1-t	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UP/LIFT	IN-SX
JT	915	0	0	5-8
J	815	0	0	5-8
G	815	0	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	641	457 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
G	572	397 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 30	-77.4	-77.4 0.09 (1)	C-I	-199 / 0	0.13	(1)
B-C	0 / 23	-77.4	-77.4 0.23 (1)	I-D	0 / 462	0.10	(1)
C-D	-623 / 0	-77.4	-77.4 0.18 (1)	I-E	-181 / 4	0.12	(1)
D-E	-622 / 0	-77.4	-77.4 0.18 (1)	J-C	-883 / 0	0.56	(1)
E-F	0 / 23	-77.4	-77.4 0.22 (1)	E-G	-879 / 0	0.53	(1)
J-B	-227 / 0	0.0	0.0 0.02 (1)				
G-F	-121 / 0	0.0	0.0 0.01 (1)				
J-I	0 / 620	-18.2	-18.2 0.42 (4)				
I-H	0 / 607	-18.2	-18.2 0.42 (4)				
H-G	0 / 607	-18.2	-18.2 0.42 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23/1.00 (B-C:1), BC=0.42/1.00 (I-J:4), WB=0.56/1.00 (C-J:1), SSI=0.13/1.00 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

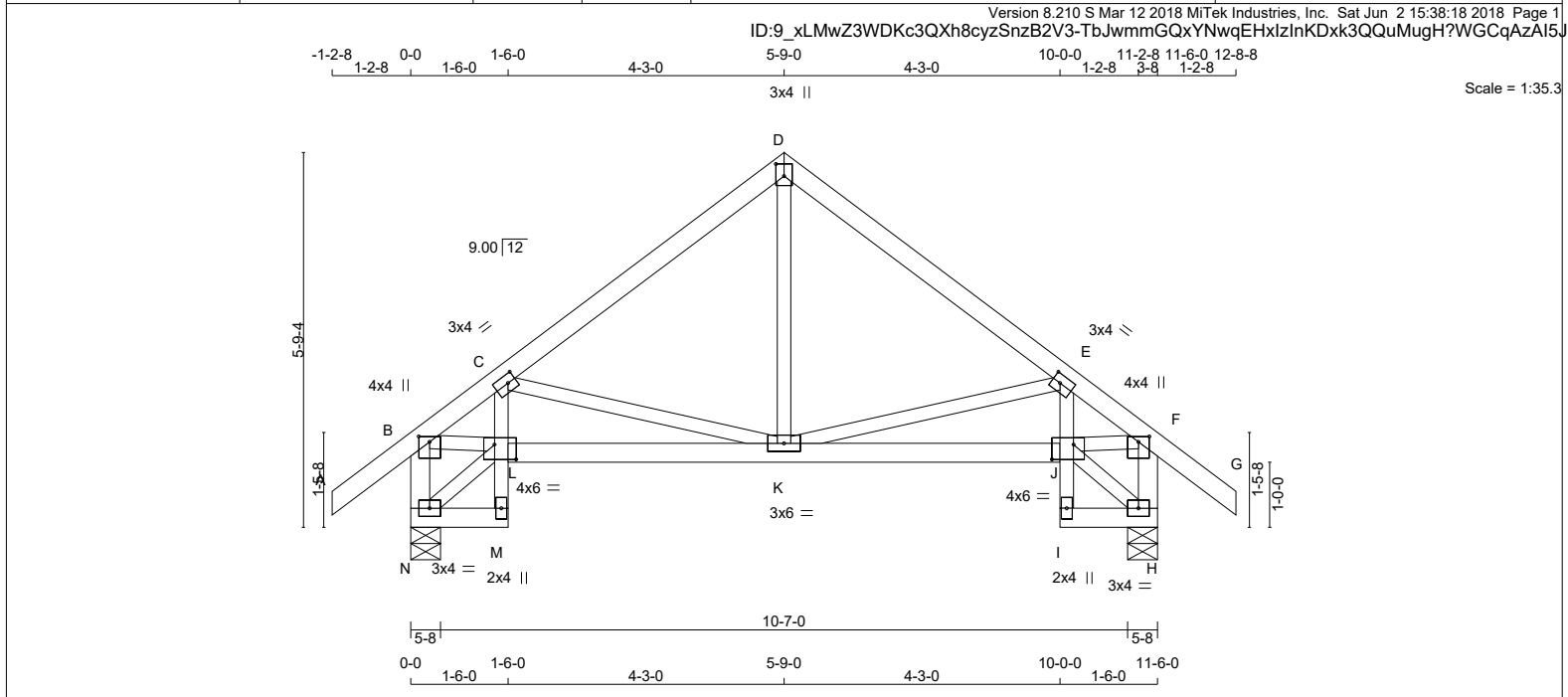
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 53 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMVW1-t	MT20	3.0	4.0		
I	BMV+p	MT20	2.0	4.0		
J	BVMWW-I	MT20	4.0	6.0	2.75	4.00
K	BMVWW-t	MT20	3.0	6.0		
L	BVMWW-I	MT20	4.0	6.0	2.75	4.00
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	650	0	650	0	5-8	5-8
H	650	0	650	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	454	328 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
H	454	328 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	K-D	0 / 241	0.05 (1)
B-C	-789 / 0	-77.4 -77.4	0.13 (1)	6.25	K-E	-319 / 0	0.11 (1)
C-D	-454 / 0	-77.4 -77.4	0.18 (1)	6.25	C-K	-319 / 0	0.11 (1)
D-E	-454 / 0	-77.4 -77.4	0.18 (1)	6.25	N-L	-13 / 0	0.00 (1)
E-F	-789 / 0	-77.4 -77.4	0.13 (1)	6.25	B-L	0 / 648	0.15 (1)
F-G	0 / 30	-77.4 -77.4	0.09 (1)	10.00	J-H	-13 / 0	0.00 (1)
N-B	-629 / 0	0.0 0.0	0.07 (1)	7.81	J-F	0 / 648	0.15 (1)
H-F	-629 / 0	0.0 0.0	0.07 (1)	7.81			
N-M	0 / 11	-18.2 -18.2	0.01 (4)	10.00			
M-L	0 / 14	0.0 0.0	0.03 (1)	10.00			
L-C	0 / 42	0.0 0.0	0.04 (1)	10.00			
L-K	0 / 658	-18.2 -18.2	0.16 (1)	10.00			
K-J	0 / 658	-18.2 -18.2	0.16 (1)	10.00			
I-J	0 / 14	0.0 0.0	0.03 (1)	10.00			
J-E	0 / 42	0.0 0.0	0.04 (1)	10.00			
I-H	0 / 11	-18.2 -18.2	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL	=	23.3 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.3 PSF
TOTAL LOAD			=	33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.18/1.00 (C-D:1), BC=0.16/1.00 (J-K:1), WB=0.15/1.00 (F-J:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

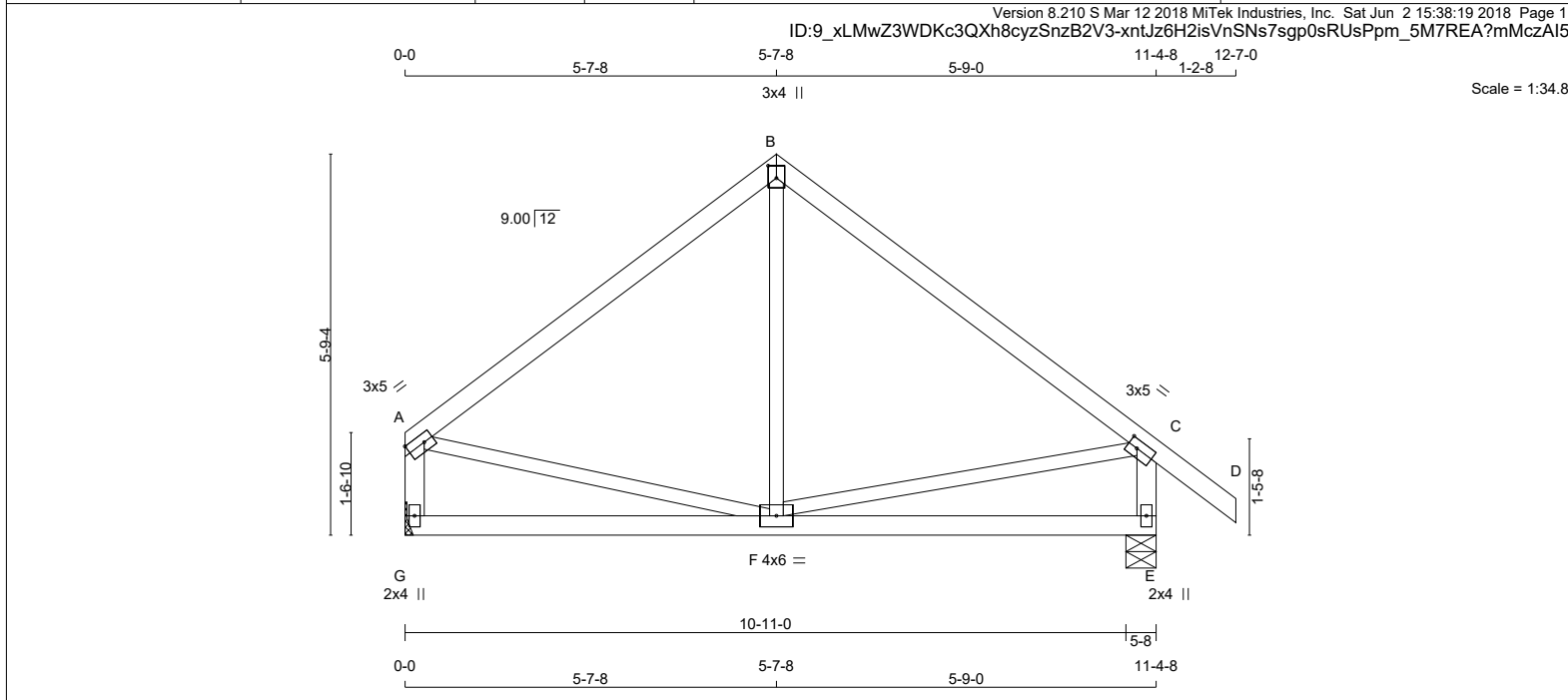
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 47 = 94 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
G - A	2x4	DRY	No.2
E - C	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DESCR.
SPF
SPF
SPF
SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	Edge
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	544	0	544	0	MECHANICAL	
E	644	0	644	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	382	265 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
E	450	325 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED VERT. LOAD (PLF)	
FR-TO		FROM	TO		FR-TO				
A-B	-371 / 0	-77.4	-77.4	0.32 (1)	6.25	F-B	0 / 103	0.04 (4)	
B-C	-371 / 0	-77.4	-77.4	0.33 (1)	6.25	A-F	0 / 304	0.07 (1)	
C-D	0 / 30	-77.4	-77.4	0.09 (1)	10.00	F-C	0 / 302	0.07 (1)	
G-A	-508 / 0	0.0	0.0	0.05 (1)	7.81				
E-C	-601 / 0	0.0	0.0	0.06 (1)	7.81				
G-F	0 / 0	-18.2	-18.2	0.17 (4)	10.00				
F-E	0 / 0	-18.2	-18.2	0.17 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.33/1.00 (B-C:1), BC=0.17/1.00 (F-G:4), WB=0.07/1.00 (A-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

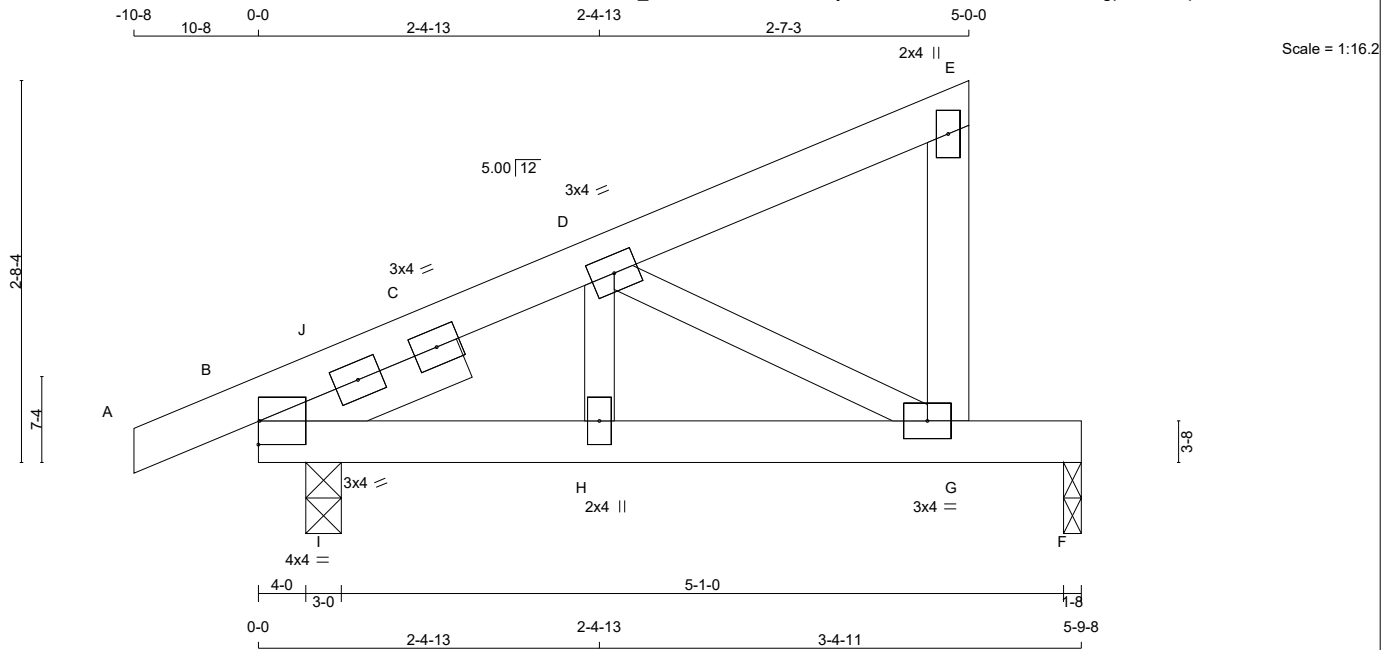
JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-xntJz6H2isVnSNs7sgp0sRUxapkC5M6REA?mMczAl5I



TOTAL WEIGHT = 5 X 21 = 105 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY: SEASONED LUMBER.				
	2x3	DRY	No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBR1-I	MT20	4.0	4.0	
B	RT-t	MT20	3.0	4.0	
B	RT-t	MT20	3.0	4.0	
D	TMWW-t	MT20	3.0	4.0	
E	TMV+p	MT20	2.0	4.0	
G	BMVW-t	MT20	3.0	4.0	
H	BMVW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
F	220	0	220	0	1-8	1-8	
B	345	0	345	0	3-0	3-0	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	156	101 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
B	241	176 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 3	-77.4 -77.4	0.05 (1)	10.00	H-D	0 / 152	0.03 (1)
B-J	-330 / 0	-77.4 -77.4	0.01 (1)	6.25	D-G	-388 / 0	0.07 (1)
J-C	-303 / 0	-77.4 -77.4	0.03 (1)	6.25	C-I	-179 / 0	0.02 (1)
C-D	-303 / 0	-77.4 -77.4	0.03 (1)	6.25	I-J	0 / 119	0.00 (1)
D-E	-4 / 0	-77.4 -77.4	0.06 (1)	10.00			
G-E	-90 / 0	0.0 0.0	0.01 (1)	7.81			
B-I	0 / 236	-18.2 -18.2	0.05 (1)	10.00			
I-H	0 / 347	-18.2 -18.2	0.12 (1)	10.00			
H-G	0 / 347	-18.2 -18.2	0.28 (1)	10.00			
G-F	0 / 0	-18.2 -18.2	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.06/1.00 (D-E:1), BC=0.28/1.00 (G-H:1), WB=0.07/1.00 (D-G:1), SSI=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

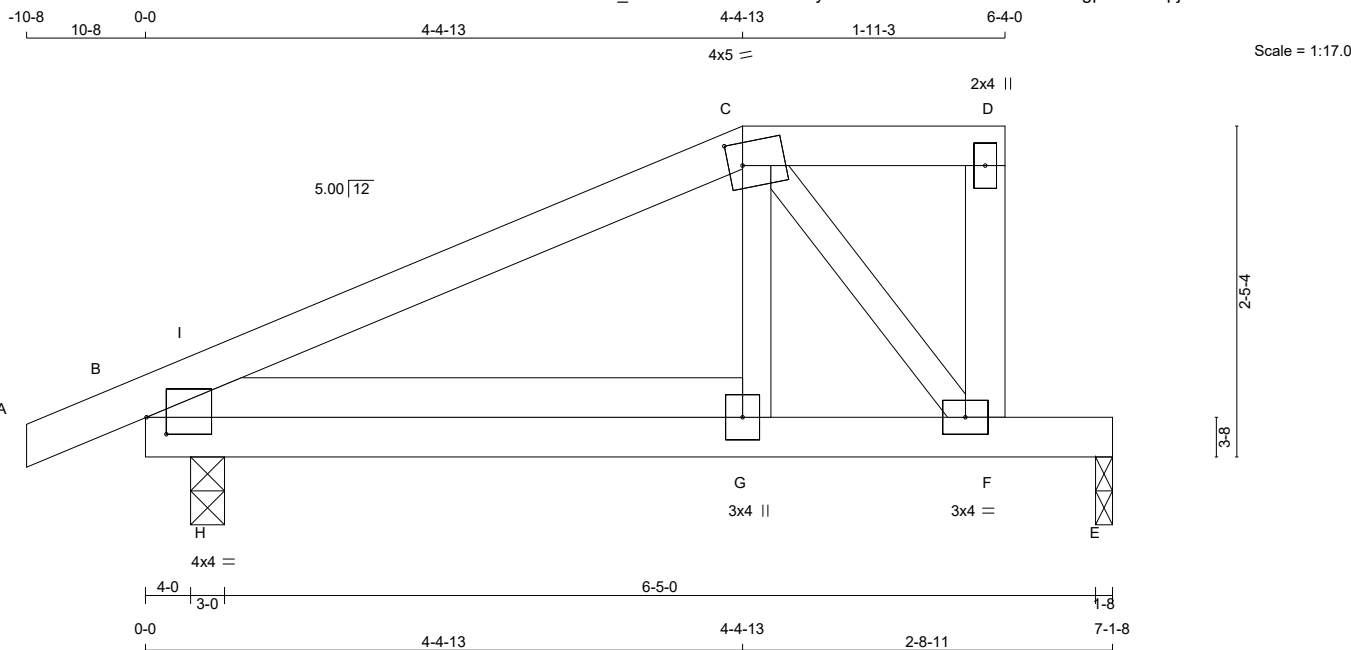
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (G) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 28 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBW1-I	MT20	4.0	4.0	1.50 1.75
C	TTWW-m	MT20	4.0	5.0	2.00 1.25
D	TMV+p	MT20	2.0	4.0	
F	BMVW-t	MT20	3.0	4.0	
G	BMWW+t	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	307	0	307	0	0	1-8	1-8		
H(B)	446	0	446	0	0	3-0	3-0		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
E	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E	216	149 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0	
H(B)	311	226 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H(B)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 3	-77.4	-77.4 0.05 (1)	10.00	G-C	0 / 193	0.04 (1)
B-I	-52 / 0	-77.4	-77.4 0.16 (1)	6.25	C-F	-381 / 0	0.07 (1)
I-C	-279 / 0	-77.4	-77.4 0.17 (1)	6.25	G-I	0 / 98	0.02 (1)
C-D	0 / 0	-77.4	-77.4 0.05 (1)	10.00	H-I	-396 / 0	0.00 (1)
F-D	-75 / 0	0.0	0.0 0.01 (1)	7.81			
B-H	0 / 149	-18.2	-18.2 0.06 (4)	10.00			
H-G	0 / 149	-18.2	-18.2 0.09 (1)	10.00			
G-F	0 / 256	-18.2	-18.2 0.33 (1)	10.00			
F-E	0 / 0	-95.6	-95.6 0.29 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17/1.00 (C-I:1), BC=0.33/1.00 (F-G:1), WB=0.07/1.00 (C-F:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)			
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

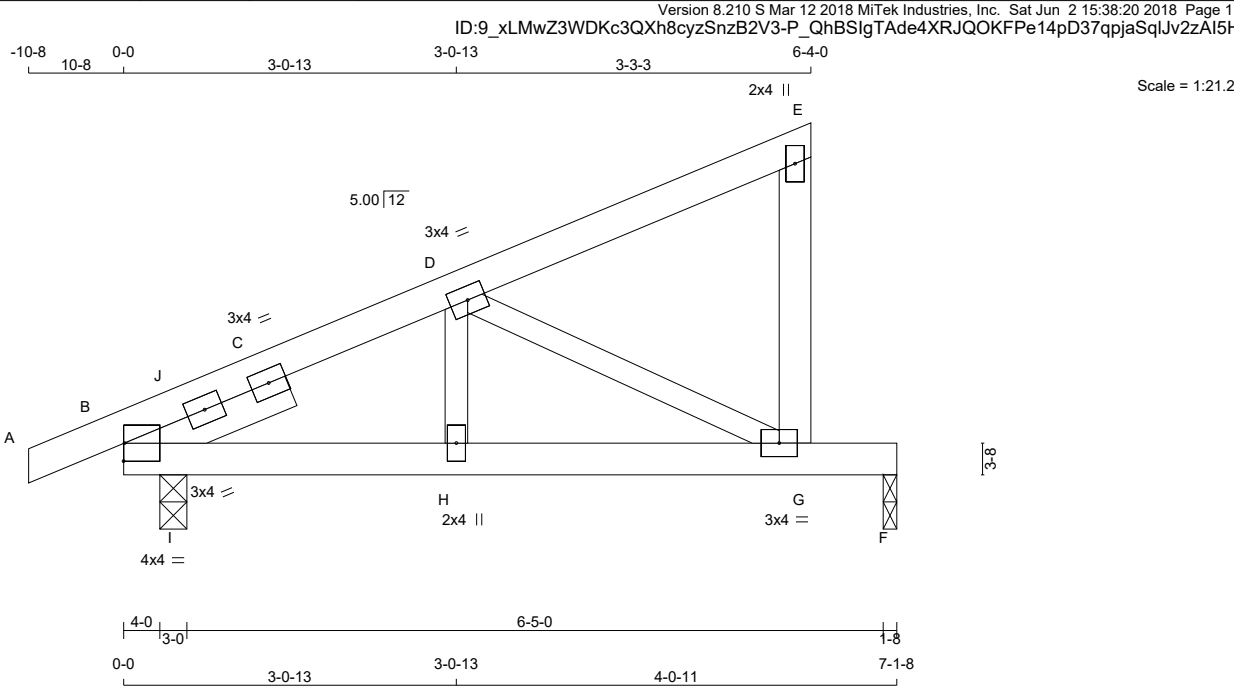
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (C) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 26 = 77 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY: SEASONED LUMBER.				
	2x3	DRY	No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBR1-I	MT20	4.0	4.0	
B	RT-t	MT20	3.0	4.0	
B	RT-t	MT20	3.0	4.0	
D	TMWW-t	MT20	3.0	4.0	
E	TMV+p	MT20	2.0	4.0	
G	BMVW-t	MT20	3.0	4.0	
H	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	UPLIFT	IN-SX
F		283	0	283	0
B		409	0	409	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	COMBINED		131 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
B			207 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 3	-77.4	-77.4 0.05 (1)	10.00	H-D	0 / 170	0.04 (1)
B-J	-415 / 0	-77.4	-77.4 0.01 (1)	6.25	D-G	-496 / 0	0.11 (1)
J-C	-384 / 0	-77.4	-77.4 0.06 (1)	6.25	C-I	-230 / 0	0.02 (1)
C-D	-384 / 0	-77.4	-77.4 0.06 (1)	6.25	I-J	0 / 153	0.00 (1)
D-E	-6 / 0	-77.4	-77.4 0.10 (1)	10.00			
G-E	-110 / 0	0.0	0.0 0.02 (1)	7.81			
B-I	0 / 297	-18.2	-18.2 0.06 (1)	10.00			
I-H	0 / 447	-18.2	-18.2 0.16 (1)	10.00			
H-G	0 / 447	-18.2	-18.2 0.37 (1)	10.00			
G-F	0 / 0	-18.2	-18.2 0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.10/1.00 (D-E:1), BC=0.37/1.00 (G-H:1), WB=0.11/1.00 (D-G:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



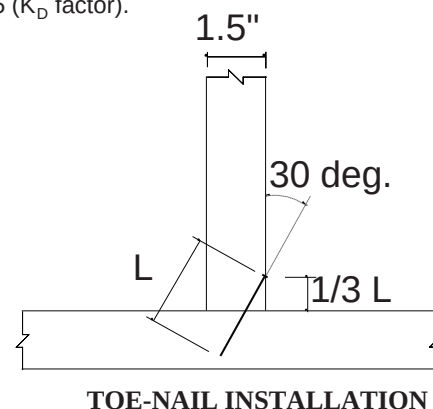
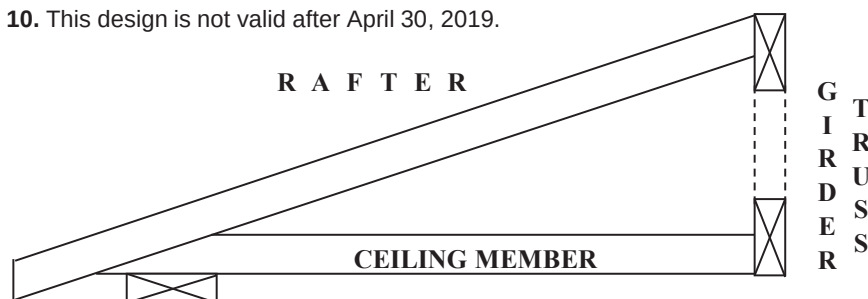
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

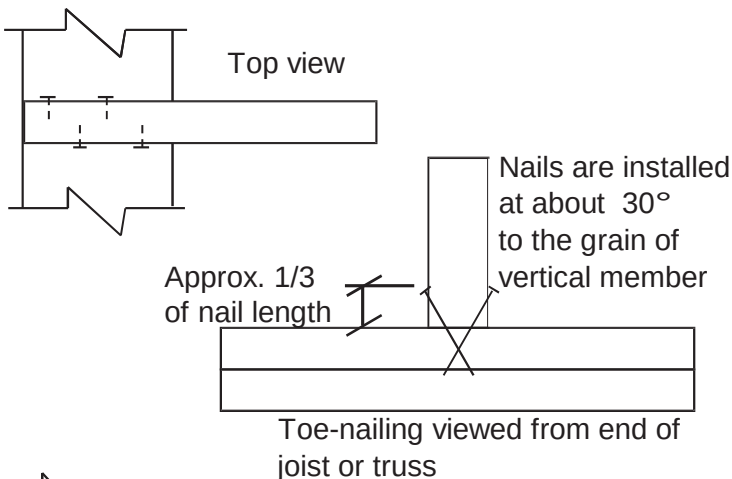
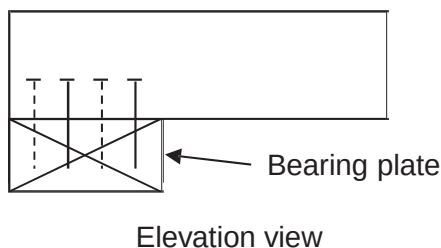
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

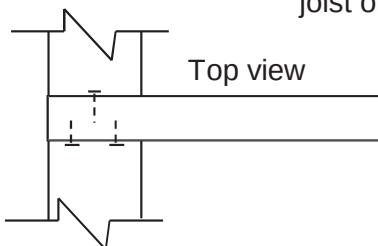
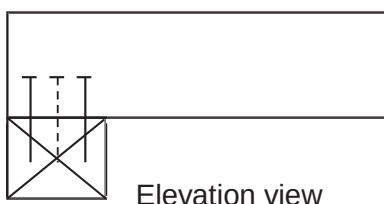
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017