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Engineering Note Page (ENP-2)

REVISION 2018-10-17

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with transfer blocks. Structural elements such as walls, posts, connectors, and transfer blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of floor joists is to be carried out in accordance with the current edition of the manufacturer's literature available at http://www.kottgroup.com.

<u>CODE</u>

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru transfer block framing is required at all point loads over bearings.

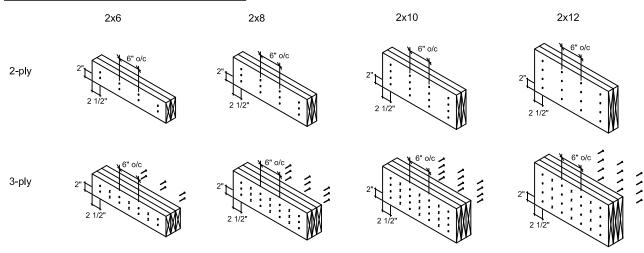
HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



W0119-092 Page 2 of 34

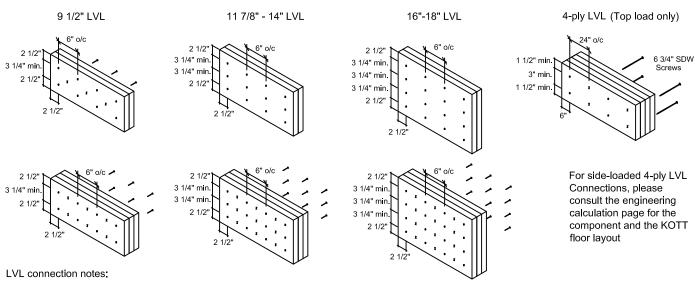
Conventional Connections



Conventional connection notes:

- -Nails to be 3" long wire nails.
- -Nails to be located 2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections



- -LVL ply width is 1-3/4"
- -Nails to be 3 1/2" common wire nails.
- -Nails to be located 2 1/2" min. from the top and bottom of the member. Start all nails 2 1/2" min. from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Multiple Member Connections

All connections are for uniformly distributed loads.

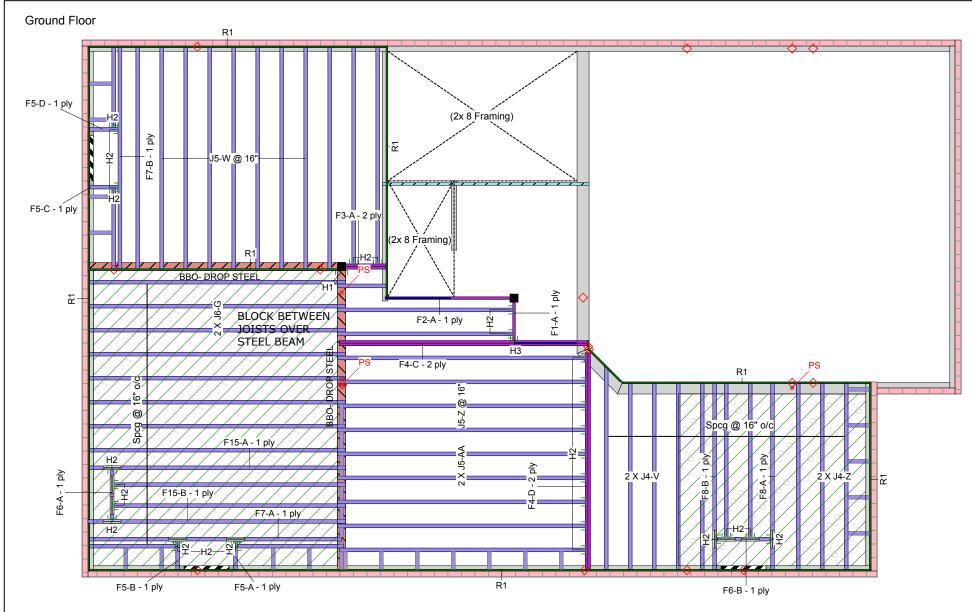
For multi-ply connections of I-joists, refer to Manufacturer's Installation Guide



KOTT Inc. 3228 Moodie Drive Ottawa, ON K2H 7V1 613-838-2775



TW0119-092



This certification is to confirm that:

components conform to the floor assembly shown on this

2. The floor joists comply with the Nascor span table for the

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and



JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.4: Dec 21.2018 Project No: 17-55 Model: lot-17 (Liana 2 el-1)

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

1. The loads used in the calculation of the attached approved layout.

loads and spacing shown on this layout.

bracing for lateral stability are the responsibility of others.



Ground Floor LVL/LSL (Flush) Label Description Width Depth Qty Plies Pcs Length F4 1.75 9.5 2 4 2.0E-3000Fb LVL F2 8-0-0 Forex 1.75 2.0E-3000Fb LVL F3 Forex 1.75 9.5 2 4-0-0 2.0E-3000Fb LVL F1 1.75 4-0-0 Forex 2.0E-3000Fb LVL Joist (Flush) Label Description Width Depth Qty Plies Pcs Length F15 LPI 20Plus 2.5 9.5 2 16-0-0 F7 LPI 20Plus 9.5 2 14-0-0 2.5 F8 LPI 20Plus 2.5 9.5 12-0-0 F6 LPI 20Plus 9.5 2 2.5 4-0-0 F5 LPI 20Plus 2.5 9.5 4 2-0-0 Sales Rep J6 LPI 20Plus 2.5 9.5 8 16-0-0 14-0-0 J5 LPI 20Plus 2.5 9.5 22 J4 LPI 20Plus 2.5 9.5 8 12-0-0 J3 LPI 20Plus 2.5 9.5 4 10-0-0 J2 LPI 20Plus 2.5 9.5 4 6-0-0 J1 LPI 20Plus 2.5 9.5 4-0-0 Project Rim Board Label Description Plies Pcs Length Width Depth Qty Norbord Rimboard 1.125 9.5 13 Plus 1.125 X 9.5 Blocking Stouffville, Ontario Label Description Width Depth Qty Plies Pcs Length BLK1 LPI 20 Plus 2.5 9.5 LinFt Varies 28-0-0 BLK2 NJH 9.5 LinFt 2.5 Varies 1-0-0 Hanger Job Path Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners H1 Unknown Hanger H2 29 LT259 4 10dx1 1/2 2 10dx1 1/2 НЗ 1 HUS1.81/10 30 16d 10 16d NOTES Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting Live another member using a face-mounted hanger. Dead . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of rimboard/rimjoist. . Refer to Nascor specifier guide for installation works. . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding Load transfer blocks to be installed under all point loads. . It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards. Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements. Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and Thickness foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others. Hatch area represents ceramic tiled floor with an addtional dead load The framing shown on this layout may deviate from the architectural

Page 3 of 34 LOT-17 (LIANA 2 EL-1)

Design Method LSD Description GRANELLI HOMES CORP. BRAMPTON, ONT. Created May 29, 2018

Builder **GREEN YORK HOMES**

Designer Shipping

Builder's Project

Kott Lumber Company 14 Anderson Blvd

Canada I 4A 7X4 905-642-4400

GREEN YORK HOMES\GRANELLI HOME CORP\MODELS\LOT 17 LIANA 2-1\LOT-17 (LIANA 2 EL-1).isl

Ground Floor Design Method

LSD Building Code NBCC 2010 / OBC 2012 Floor

Loads Deflection Joist 480 LL Span L/ 360 TL Span L/ LL Cant 2L/ TL Cant 2L/ 360 Deflection Girder 360 240

15

480

480

360

3/4"

Nailed & Glued

LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/ Decking OSB Deck

Fastener Vibration

of 5 PSF

and structural drawings. Project Engineer to review and approve the deviation prior to construction.

Legend

Point Load Support

Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 9.5 LPI 20Plus 9.5 Forex 2.0E-3000Fb LVL 1.75 X 9.5 0 X 0 (Dropped) 1.75 X 9.5 (Dropped) 5.25 X 8 (Dropped)

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isDesign™

Client: GR Project:

Address:

GREEN YORK HOMES

ORK HOMES

Designer: SB

Job Name: LOT-17 (LIANA 2 EL-1)

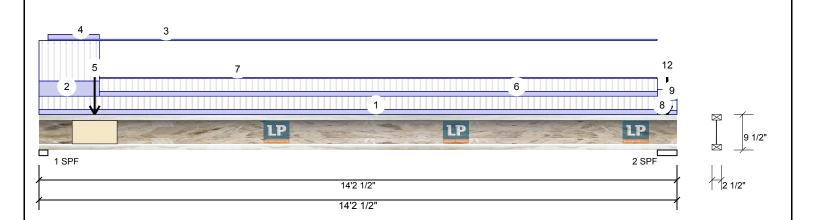
1/16/2019

Project #:

Date:

F15-A LPI 20Plus 9.500" - PASSED

Level: Ground Floor



Member Info	rmation			Unfactore	ed Reacti	ions UNPATTE	RNED Ib (U	Jplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow		Wind
Plies:	1	Design Method:	LSD	1	590	286	0		0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	480	242	0		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings a	and Fact	ored Reaction	s		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L	. lb Total I	Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	77% 358 / 8	885 1243 L	L	1.25D+1.5L
				2-SPF 5	5.250"	58% 302 / 7	'19 1022 l	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2280 ft-lb	6'3 11/16"	4670 ft-lb	0.488 (49%)	1.25D+1.5L	L
Shear	1221 lb	1 5/8"	1990 lb	0.614 (61%)	1.25D+1.5L	L
Perm Defl in.	0.109 (L/1503)	6'9 1/16"	0.457 (L/360)	0.240 (24%)	D	Uniform
LL Defl inch	0.227 (L/725)	6'9 1/8"	0.457 (L/360)	0.500 (50%)	L	L
TL Defl inch	0.336 (L/489)	6'9 1/8"	0.685 (L/240)	0.490 (49%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.109", Long Term = 0.164"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 6'1" o.c.
- 7 Bottom flange braced at bearings.

	gg									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 13-9-4	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 1-4-2	(Span) 2-11-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Part. Uniform	0-2-7 to 13-9-3		Тор	2 PLF	0 PLF	0 PLF	0 PLF		
4	Part. Uniform	0-2-7 to 1-4-2		Тор	7 PLF	0 PLF	0 PLF	0 PLF		

Continued on page 2...

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lncorp.com

www.lpcorp.com CCMC: 12412-R APA: PR-L238C



Page 1 of 2





TW0119-092 Page 5 of 34

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Client: Project: Address: **GREEN YORK HOMES**

Date: 1/16/2019

Designer:

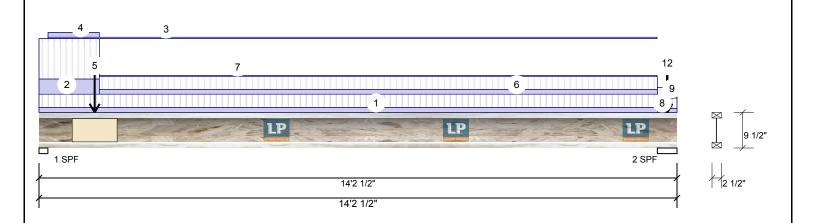
Job Name: LOT-17 (LIANA 2 EL-1)

Page 2 of 2

Project #:

9.500" - PASSED LPI 20Plus

Level: Ground Floor



Continued from page	age 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
5	Point	1-2-14		Near Face	145 lb	293 lb	0 lb	0 lb	F6
6	Tie-In	1-4-2 to 13-9-4	(Span)0-11-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-4-2 to 13-9-3		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	13-9-4 to 14-2-8	(Span)0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Tie-In	13-9-4 to 14-2-8	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Point	13-11-14		Тор	34 lb	89 lb	0 lb	0 lb	J5
	Bearing Length	0-1-8							
11	Point	13-11-14		Тор	39 lb	93 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
12	Point	13-11-14		Тор	27 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 10/31/2020



TW0119-092 Page 6 of 34

isDesign™

Client: GREEN YORK HOMES Project:

Address:

EEN YORK HOMES Date:

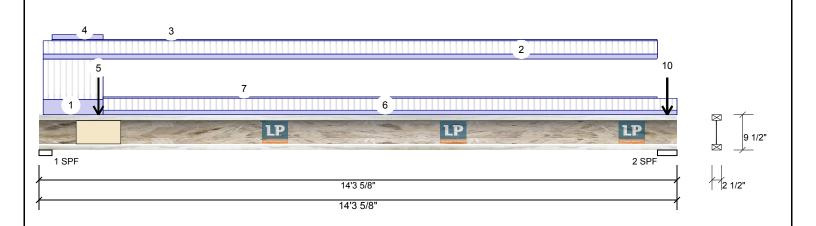
Date: 1/16/2019 Designer: SB

Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

F15-B LPI 20Plus 9.500" - PASSED

Level: Ground Floor



Member Info	rmation			Unfactor	ed React	ions UNPATTERI	NED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	591	287	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	441	223	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 3	3.500"	73% 359 / 886	1245 L	1.25D+1.5L
				2-SPF 5	5.250"	54% 278 / 661	939 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2239 ft-lb	6'4 5/16"	4670 ft-lb	0.479 (48%)	1.25D+1.5L	L
Shear	1224 lb	2 3/4"	1990 lb	0.615 (62%)	1.25D+1.5L	L
Perm Defl in.	0.108 (L/1523)	6'10 1/16"	0.457 (L/360)	0.240 (24%)	D	Uniform
LL Defl inch	0.222 (L/739)	6'10 1/8"	0.457 (L/360)	0.490 (49%)	L	L
TL Defl inch	0.330 (L/497)	6'10 1/8"	0.685 (L/240)	0.480 (48%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.108", Long Term = 0.162"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 6'2" o.c.
- 7 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-2 to 1-5-4	(Span) 2-11-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-2 to 13-10-6	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-3-8 to 13-10-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-3-8 to 1-5-4		Тор	7 PLF	0 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C



Page 1 of 2





TW0119-092 Page 7 of 34

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Client: Project:

Address:

GREEN YORK HOMES

Date: 1/16/2019

Designer:

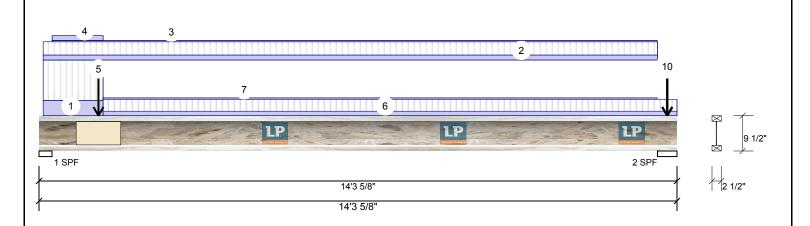
Job Name: LOT-17 (LIANA 2 EL-1)

Page 2 of 2

Project #:

9.500" - PASSED LPI 20Plus

Level: Ground Floor



Continued fro	om page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
5	Point	1-4-0		Far Face	149 lb	301 lb	0 lb	0 lb	F6
6	Tie-In	1-5-4 to 14-3-10	(Span)0-10-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-5-4 to 13-10-4		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Point	14-1-0		Тор	29 lb	75 lb	0 lb	0 lb	J5
	Bearing Length	0-1-8							
9	Point	14-1-0		Тор	34 lb	79 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
10	Point	14-1-0		Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 10/31/2020



TW0119-092 Page 8 of 34



Client: **GREEN YORK HOMES** Project:

Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

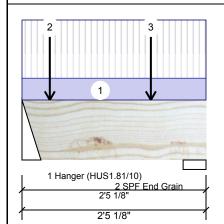
1/16/2019

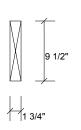
Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Address:

Level: Ground Floor





Page 1 of 1

Member Information Application: Floor (Residential) Type: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	299	116	0	0
2	258	101	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
1 -	3.000"	15%	145 / 449	594	L	1.25D+1.5L
Hanger						
2 - SPF	3.625"	11%	127 / 387	514	L	1.25D+1.5L
End						
Grain						

Wind

0 PSF

0 lb J3

Comments

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	214 ft-lb	1'6 7/8"	11362 ft-lb	0.019 (2%)	1.25D+1.5L	L
Unbraced	214 ft-lb	1'6 7/8"	10729 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	438 lb	11 3/4"	4638 lb	0.094 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/58713)	1'4 3/8"	0.067 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/23005)	1'4 9/16"	0.067 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.001 (L/16529)	1'4 7/16"	0.100 (L/240)	0.010 (1%)	D+L	L

Design Notes

1 Fill all hanger nailing holes

Point

Point

Self Weight

	are designed to be suppo	rted on the bottom ed	dge only.					
3 Top bra	aced at bearings.							
4 Bottom	braced at bearings.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	15 PSF	40 PSF	0 PSF				

Far Face

Far Face



0 lb Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation

0-4-6

1-8-6

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is

65 lb

72 lb

4 PLF

175 lb

192 lb

Manufacturer Info Forex APA: PR-L318

0 lb

0 lb

READ ALL NOTES ON THIS PAGE AND ON THE

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TW0119-092 Page 9 of 34

isDesign™

Client: **GREEN YORK HOMES**

Project:

1/16/2019 Designer:

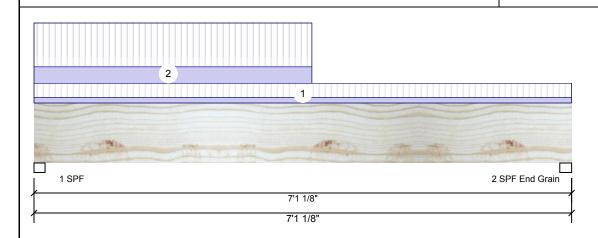
Job Name: LOT-17 (LIANA 2 EL-1)

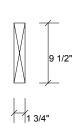
Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Address:

Level: Ground Floor





Page 1 of 1

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	156	72	0	0
2	84	45	0	0

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	453 ft-lb	2'11 9/16"	11362 ft-lb	0.040 (4%)	1.25D+1.5L	L
Unbraced	453 ft-lb	2'11 9/16"	5389 ft-lb	0.084 (8%)	1.25D+1.5L	L
Shear	228 lb	10 1/2"	4638 lb	0.049 (5%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/20045)	3'4 1/8"	0.231 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.009 (L/9718)	3'3 1/2"	0.231 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.013 (L/6545)	3'3 11/16"	0.346 (L/240)	0.040 (4%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	1.750"	17%	90 / 234	324	L	1.25D+1.5L
2 - SPF	1.875"	7%	56 / 125	182	L	1.25D+1.5L
End						

Grain

January 17, 2019

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100083566

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 7-1-2	(Span)0-7-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-0		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is



TW0119-092 Page 10 of 34



Client: **GREEN YORK HOMES** Project:

Date: 1/16/2019

Designer:

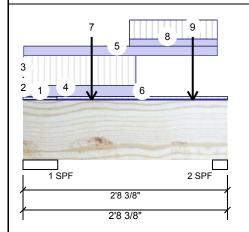
Job Name: LOT-17 (LIANA 2 EL-1)

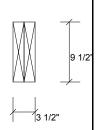
Project #:

1.750" X 9.500" 2-Ply - PASSED Forex 2.0E-3000Fb LVL

Address:

Level: Ground Floor





Page 1 of 2

Member Info	rmation			Unfactore	d Reacti	ons UNPATTERN	IED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	653	374	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	490	280	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 5	.500"	12% 467 / 979	1446 L	1.25D+1.5L
				2-SPF 2	.375"	21% 350 / 736	1086 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	579 ft-lb	1'3 9/16"	22724 ft-lb	0.025 (3%)	1.25D+1.5L	L
Unbraced	579 ft-lb	1'3 9/16"	22724 ft-lb	0.025 (3%)	1.25D+1.5L	L
Shear	713 lb	1'9 1/4"	9277 lb	0.077 (8%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/33703)	1'4 7/8"	0.072 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/19171)	1'4 1/4"	0.072 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.002 (L/12226)	1'4 1/2"	0.108 (L/240)	0.020 (2%)	D+L	L



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-5-6	(Span)0-10-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 0-0-2		Тор	46 PLF	123 PLF	0 PLF	0 PLF	J5
3	Part. Uniform	0-0-0 to 0-0-2		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Part. Uniform	0-0-2 to 1-5-14		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J5
5	Part. Uniform	0-0-2 to 2-6-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

 - Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



IN THE DESIGN OF THIS COMPONENT. This design is

TW0119-092 Page 11 of 34



Client: Project: Address:

GREEN YORK HOMES

Date: 1/16/2019

Designer:

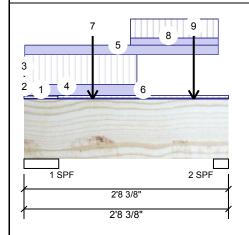
Job Name: LOT-17 (LIANA 2 EL-1)

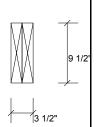
Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL

2-Ply - PASSED

Level: Ground Floor





Page 2 of 2

İ	Continued from page	age 1								
ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
ı	6	Tie-In	0-5-6 to 2-8-6	(Span)1-0-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	7	Point	0-10-14		Far Face	120 lb	319 lb	0 lb	0 lb	J5
ı	8	Part. Uniform	1-4-14 to 2-6-12		Тор	59 PLF	158 PLF	0 PLF	0 PLF	J5

Far Face

83 lb

220 lb

0 lb

0 lb J5

Self Weight 8 PLF

2-2-14

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

9

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Ianding & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

APA: PR-L318

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 10/18/2021

TW0119-092 Page 12 of 34

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Client: Project:

GREEN YORK HOMES

Date: 1/16/2019

Designer:

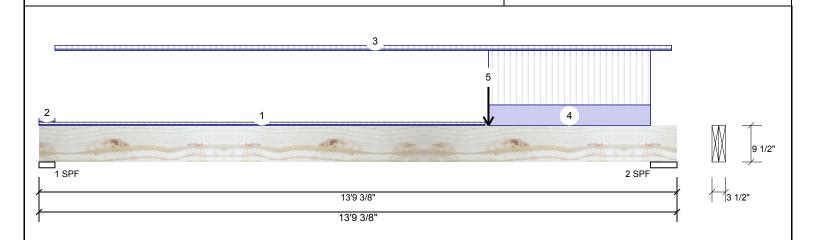
Job Name: LOT-17 (LIANA 2 EL-1)

Page 1 of 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED Level: Ground Floor



Member Info	rmation			Unfactore	ed React	ions Ul	NPATTERNI	ED lb ((Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	W	Wind
Plies:	2	Design Method:	LSD	1	373		193		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1087		464		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fact	ored R	eactions			
Dead:	15 PSF			Bearing L	_ength	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF 4	4.125"	9%	241 / 560	801	L	1.25D+1.5L
				2-SPF 6	6.875"	15%	580 / 1631	2211	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4654 ft-lb	9'8 1/2"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
Unbraced	4654 ft-lb	9'8 1/2"	19392 ft-lb	0.240 (24%)	1.25D+1.5L	L
Shear	1816 lb	12'5 3/4"	9277 lb	0.196 (20%)	1.25D+1.5L	L
Perm Defl in.	0.058 (L/2669)	7'3 7/8"	0.433 (L/360)	0.130 (13%)	D	Uniform
LL Defl inch	0.127 (L/1225)	7'5 1/4"	0.433 (L/360)	0.290 (29%)	L	L
TL Defl inch	0.186 (L/840)	7'4 13/16"	0.649 (L/240)	0.290 (29%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6 Lateral s	slenderness ratio based o	on full section width.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 9-7-10	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 0-4-2	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Tie-In	0-4-2 to 13-7-15	(Span)0-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF
4	Part. Uniform	9-8-8 to 13-2-8		Тор	90 PLF	240 PLF	0 PLF	®ass-Thru Framing Squash Block is
5	Point	9-8-8		Far Face	116 lb	299 lb	0 lb	reguired at all point loads over bearings
	Self Weight				8 PLF			Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

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January 17, 2019



This design



requirements

TW0119-092 Page 13 of 34

isDesign™

Client: Project:

GREEN YORK HOMES

Date: 1/16/2019

Designer:

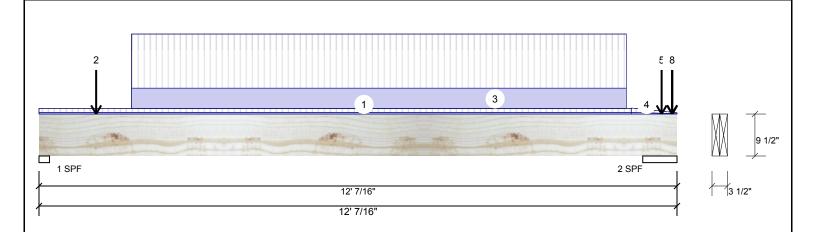
Job Name: LOT-17 (LIANA 2 EL-1)

Page 1 of 2

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED Level: Ground Floor



Member Info	rmation			Unfactore	ed Reacti	ions UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	1545	622	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1769	735	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	61% 777 / 2317	3094 L	1.25D+1.5L
				2-SPF 7	7.754"	21% 919 / 2654	3573 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9242 ft-lb	5'9 9/16"	22724 ft-lb	0.407 (41%)	1.25D+1.5L	L
Unbraced	9242 ft-lb	5'9 9/16"	20195 ft-lb	0.458 (46%)	1.25D+1.5L	L
Shear	3271 lb	11 1/8"	9277 lb	0.353 (35%)	1.25D+1.5L	L
Perm Defl in.	0.092 (L/1482)	5'9 9/16"	0.377 (L/360)	0.240 (24%)	D	Uniform
LL Defl inch	0.229 (L/594)	5'9 9/16"	0.377 (L/360)	0.610 (61%)	L	L
TL Defl inch	0.320 (L/424)	5'9 9/16"	0.566 (L/240)	0.570 (57%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

/ Lateral slende	erness ratio based o	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-2-3	(Span)1-0-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-0-15		Far Face	124 lb	332 lb	0 lb	0 lb	J5
3	Part. Uniform	1-8-15 to 11-0-15		Far Face	101 PLF	270 PLF	0 PLF	0 PLF	
4	Tie-In	11-2-3 to 12-0-7	(Span)1-1-0 to 0-2-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	11-8-15		Far Face	48 lb	128 lb	0 lb	0 lb	J5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED**

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

T.L. WISE

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January 17, 2019



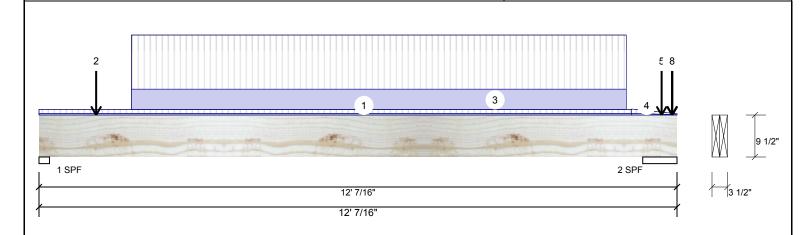
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TW0119-092 Page 14 of 34

Client: **GREEN YORK HOMES** Date: 1/16/2019 Project: Designer: isDesign™ Job Name: LOT-17 (LIANA 2 EL-1) Project #: Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Page 2 of 2

Level: Ground Floor



.Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	11-11-5		Тор	16 lb	34 lb	0 lb	0 lb	J4
7	Point	11-11-5		Тор	23 lb	61 lb	0 lb	0 lb	J5
8	Point	11-11-5		Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Self Weight				8 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 10/18/2021

APA: PR-L318

Manufacturer Info



TW0119-092 Page 15 of 34



Client: Project: Address:

GREEN YORK HOMES

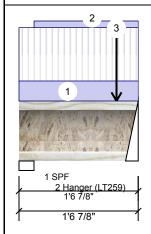
Date: 1/16/2019 Designer:

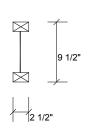
Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

LPI 20Plus 9.500" - PASSED

Level: Ground Floor





Page 1 of 1

wember	Intormation
Type:	Girder

Type.	Olluci
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method: **Building Code:**

Load Sharing: Deck: Not Checked Vibration: Not Checked

NBCC 2010 / OBC 2012 No

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg 1	Live	Dead	Snow	Wind
1	65	31	0	0
2	135	67	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	8%	39 / 98	136	L	1.25D+1.5L
2 - Hanger	2.000"	18%	84 / 202	286	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	52 ft-lb	1' 1/8"	4670 ft-lb	0.011 (1%)	1.25D+1.5L	L
Shear	272 lb	1'5 5/8"	1990 lb	0.137 (14%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/54318)	11 7/8"	0.044 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/27008)	11 7/8"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/18039)	11 7/8"	0.067 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

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January 17, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	
3	Point	1-3-7		Near Face	49 lb	99 lb	0 lb	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF 0 PLF Comments





TW0119-092 Page 16 of 34

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Client: Project: Address:

GREEN YORK HOMES

Date: 1/16/2019 Designer: SB

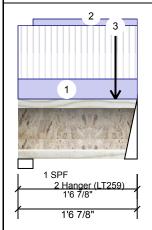
Designer: SB

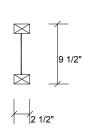
Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

F5-B LPI 20Plus 9.500" - PASSED

Level: Ground Floor





Page 1 of 1

Member I	nformatio
Type:	Girder

Type:	Giraei
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

Application: Floor (Residential)
Design Method: LSD
Building Code: NBCC 2010 / OBC 2012
Load Sharing: No

Deck: Not Checked
Vibration: Not Checked

That Visitati

40 PSF 15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg 1	Live	Dead	Snow	Wind
1	63	30	0	0
2	120	59	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	8%	37 / 94	131	L	1.25D+1.5L
2 - Hanger	2.000"	16%	74 / 180	254	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	48 ft-lb	11 11/16"	4670 ft-lb	0.010 (1%)	1.25D+1.5L	L
Shear	240 lb	1'5 5/8"	1990 lb	0.121 (12%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/59285)	11 1/2"	0.044 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/29321)	11 1/2"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001	11 1/2"	0.067 (L/240)	0.010 (1%)	D+L	L



- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.



January 17, 2019

-	g						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF
3	Point	1-3-7		Far Face	40 lb	82 lb	0 lb

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.locorp.com

www.lpcorp.com CCMC: 12412-R APA: PR-L238C

Wind

0 PSF 0 PLF Comments





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Client: Project: Address:

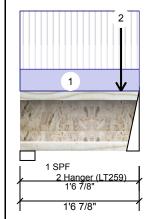
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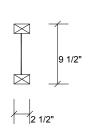
Date: 1/16/2019 Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

LPI 20Plus 9.500" - PASSED Level: Ground Floor





Wind

0 0

0

0

Page 1 of 1

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

22

44

59

115

2

Hanger

Bearings and Factored Reactions										
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.				
1 - SPF	2.375"	7%	28 / 88	116	L	1.25D+1.5L				
2 -	2.000"	14%	55 / 173	227	L	1.25D+1.5L				

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	39 ft-lb	11"	4670 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	214 lb	1'5 5/8"	1990 lb	0.108 (11%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/33835)	10 7/8"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/24562)	10 7/8"	0.067 (L/240)	0.010 (1%)	D+L	L

Design Notes

1 Provide restraint at supports to ensure lateral stability.

15 PSF

- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge of
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

0" only.						Janua	ary 17, 2019
ib Width	Side	Dead	Live	Snow	Wind	Comments	

Load Type Location Tie-In 0-0-0 to 1-6-14 (Span)3-2-8 Top 15 PSF 40 PSF 0 PSF 0 PSF 2 Point 1-4-0 Near Face 28 lb 73 lb 0 lb 0 lb

> Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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Manufacturer Info

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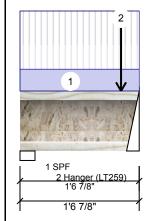
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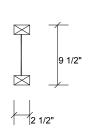
1/16/2019 Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

LPI 20Plus 9.500" - PASSED Level: Ground Floor





Wind

0

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Page 1 of 1

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

2	118	44	0	0

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	39 ft-lb	11 1/16"	4670 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	218 lb	1'5 5/8"	1990 lb	0.110 (11%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/33432)	10 15/16"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/24349)	10 15/16"	0.067 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

59

1

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	7%	28 / 89	116	L	1.25D+1.5L
2 - Hanger	2.000"	15%	55 / 177	231	L	1.25D+1.5L

Design Notes

1 Provide restraint at supports to ensure lateral stability.

15 PSF

- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

T.L. WISE 100083566 100083566 NCE OF ONT

January 17, 2019

	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
:	2	Point	1-4-0		Far Face	28 lb	76 lb	0 lb	0 lb	J2

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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Brg



Client: Project: Address:

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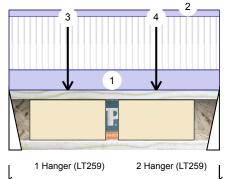
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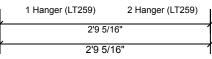
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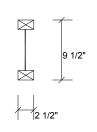
Project #:

LPI 20Plus 9.500" - PASSED









Wind

Page 1 of 1

nation		
Girder	Application:	Floor (Residential)
1	Design Method:	LSD
Dry	Building Code:	NBCC 2010 / OBC 2012
360	Load Sharing:	No
240	Deck:	Not Checked
Normal	Vibration:	Not Checked
40 PSF		
	Girder 1 Dry 360 240 Normal	Girder Application: Design Method: Dry Building Code: 360 Load Sharing: 240 Deck: Normal Vibration:

15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

1 2	301	149	0	0
2	293	145	0	0

Bearings and Factored Reactions

ſ	Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
	1 - Hanger	2.000"	41%	187 / 452	639	L	1.25D+1.5L
$\frac{1}{1}$	2 - Hanger	2.000"	39%	181 / 439	621	L	1.25D+1.5L

Analysis Results

Dead:

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	436 ft-lb	1'8 1/4"	4670 ft-lb	0.093 (9%)	1.25D+1.5L	L
	Shear	633 lb	1 1/4"	1990 lb	0.318 (32%)	1.25D+1.5L	L
l	Perm Defl in.	0.003 (L/10410)	1'5 1/2"	0.086 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.006 (L/5151)	1'5 9/16"	0.086 (L/360)	0.070 (7%)	L	L
	TL Defl inch	0.009 (L/3446)	1'5 1/2"	0.128 (L/240)	0.070 (7%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.003", Long Term = 0.004"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings



January 17, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 2-9-5	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Part. Uniform	0-0-0 to 2-9-5		Тор	3 PLF	0 PLF	0 PLF	0 PLF
3	Point	0-9-5		Near Face	128 lb	258 lb	0 lb	0 lbJ5
4	Point	1-11-5		Near Face	131 lb	265 lb	0 lb	Pass-Thru Framing Squash Block is required at all point loads over bearings
								redance at an penit leade ever bearinge

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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Client: Project: Address:

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Date: 1/16/2019

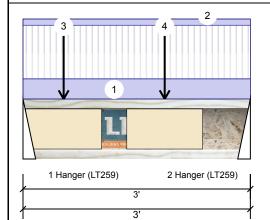
Designer:

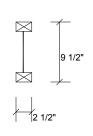
Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

9.500" - PASSED LPI 20Plus

Level: Ground Floor





Wind

0

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Page 1 of 1

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 DQE		

	Reactions	UNPATTER	NED lb (Uplift)
Brg	Live	Dead	Snow

	_			
1 2	209	103	U	U

131

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	407 ft-lb	1'10 7/16"	4670 ft-lb	0.087 (9%)	1.25D+1.5L	L
Shear	552 lb	1 1/4"	1990 lb	0.277 (28%)	1.25D+1.5L	L
Perm Defl in.	0.003 (L/12219)	1'10 7/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.006 (L/6031)	1'10 7/16"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.008 (L/4038)	1'10 7/16"	0.140 (L/240)	0.060 (6%)	D+L	L

Location

0-6-7

1-10-7

0-0-0 to 3-0-0

0-0-0 to 3-0-0

Trib Width

(Span)1-8-11

Side

Far Face

Far Face

Top

Bearings and Factored Reactions

264

1

ш								
Γ	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 -	2.000"	36%	164 / 396	559	L	1.25D+1.5L	
l	Hanger 2 -	2.000"	28%	129 / 313	442	L	1.25D+1.5L	
ı	Hanger							

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.003", Long Term = 0.004"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings

Load Type

Part. Uniform

Tie-In

Point

Point



lb	J3		0	Dis de la	
F					
F					
ıd	Cor	nments			

0 PLF 0 PL 0 lb Pass-Thru Framing Squash Block is 0 lb required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

2

3

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Dead

15 PSF

4 PLF

80 lb

103 lb

This design is valid until 10/31/2020

Live

40 PSF

0 PLF

160 lb

209 lb

Manufacturer Info

Snow

0 PSF

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Win

0 PS





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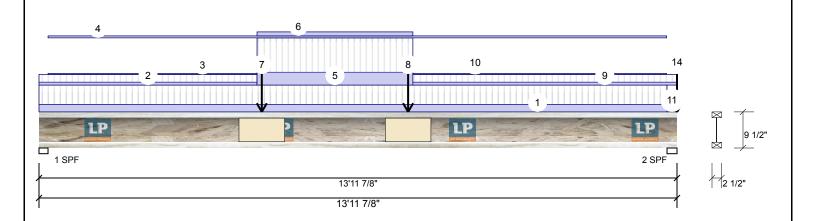
DRK HOMES Date:

Date: 1/16/2019
Designer: SB
Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

F7-A LPI 20Plus 9.500" - PASSED

Level: Ground Floor



Member Info	rmation			Unfactor	ed Reacti	ons UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	375	183	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	507	257	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	2.375"	49% 228 / 563	791 L	1.25D+1.5L
				2 - SPF	2.625"	66% 321 / 761	1082 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3347 ft-lb	7'1"	4670 ft-lb	0.717 (72%)	1.25D+1.5L	L
Shear	784 lb	1 5/8"	1990 lb	0.394 (39%)	1.25D+1.5L	L
Perm Defl in.	0.154 (L/1070)	6'11 7/16"	0.457 (L/360)	0.340 (34%)	D	Uniform
LL Defl inch	0.314 (L/523)	6'11 3/8"	0.457 (L/360)	0.690 (69%)	L	L
TL Defl inch	0.468 (L/351)	6'11 3/8"	0.685 (L/240)	0.680 (68%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.154", Long Term = 0.230"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 4'11" o.c.
- 7 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-9-4	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 4-9-6	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 4-9-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 13-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes

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Client: Project: Address:

: GREEN YORK HOMES

Date: 1/16/2019
Designer: SB

Designer: SB

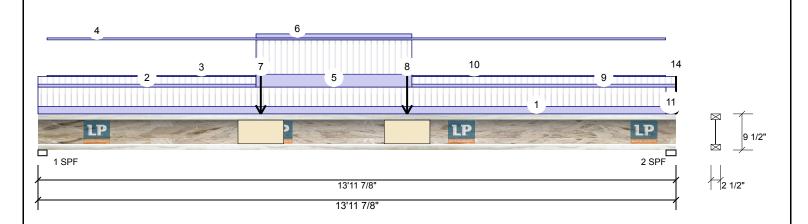
Job Name: LOT-17 (LIANA 2 EL-1)

Page 2 of 2

Project #:

F7-A LPI 20Plus 9.500" - PASSED

Level: Ground Floor



Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
5	Tie-In	4-9-6 to 8-2-6	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	4-9-6 to 8-2-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
7	Point	4-10-10		Near Face	59 lb	120 lb	0 lb	0 lb	F5
8	Point	8-1-2		Near Face	67 lb	135 lb	0 lb	0 lb	F5
9	Tie-In	8-2-6 to 13-11-14	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	8-2-6 to 13-9-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
11	Tie-In	13-9-4 to 13-11-14	(Span)0-8-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
12	Point	13-11-14		Тор	29 lb	75 lb	0 lb	0 lb	J5
	Bearing Length	0-1-8							
13	Point	13-11-14		Тор	34 lb	79 lb	0 lb	0 lb	J6
	Bearing Length	0-1-8							
14	Point	13-11-14		Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Bearing Length	0-1-8							

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com

www.lpcorp.com CCMC: 12412-R APA: PR-L238C Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



This design is valid until 10/31/2020

TW0119-092 Page 23 of 34

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Client: Project: Address:

GREEN YORK HOMES

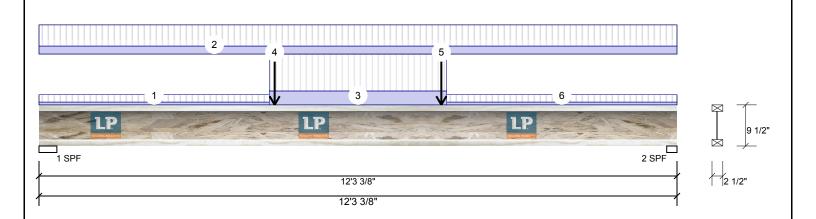
Date: 1/16/2019 Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

Page 1 of 1

Project #

LPI 20Plus 9.500" - PASSED Level: Ground Floor



Member Info	rmation			Unfactore	d Reactio	ns UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	331	125	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	324	122	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 4	.125"	37% 156 / 496	652 L	1.25D+1.5L
				2-SPF 2	.375"	40% 152 / 486	638 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2390 ft-lb	6'2 3/4"	4670 ft-lb	0.512 (51%)	1.25D+1.5L	L
Shear	637 lb	3 3/8"	1990 lb	0.320 (32%)	1.25D+1.5L	L
Perm Defl in.	0.071 (L/2003)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.189 (L/753)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
TL Defl inch	0.260 (L/547)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L

Location

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.071", Long Term = 0.107"
- 3 Girders are designed to be supported on the bottom edge only
- 4 Top flange must be laterally braced at a maximum of 5'11" o.c

5 Bottom flange braced at bearings.

0.107" dge only. 5'11" o.c.						January 17, 2019	
Trib Width	Side	Dead	Live	Snow	Wind	Сопшнения	
(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
(Span) 0-11-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		

Load Type Tie-In 0-0-0 to 4-5-3 (Spar 1 2 0-0-0 to 12-3-6 Tie-In (Spar 0-11-3 Tie-In 4-5-3 to 7-10-3 (Spar Point 4-6-7 Far Face 44 lb 115 lb 0 lb 0 lb Far Face 5 Point 7-8-15 44 lb 118 lb 0 lb Past-Thru Framing Squash Block is текцитеd at all point loads over bearings 15 PSF 40 PSF 0 PSF 6 Tie-In 7-10-3 to 12-3-6 (Span)0-4-2 Top

> **Refer to Multiple Member Connection** Detail for ply to ply nailing or bolting requirements

Notes

ID

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com

CCMC: 12412-R APA: PR-L238C

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

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100083566





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Client: Project: Address:

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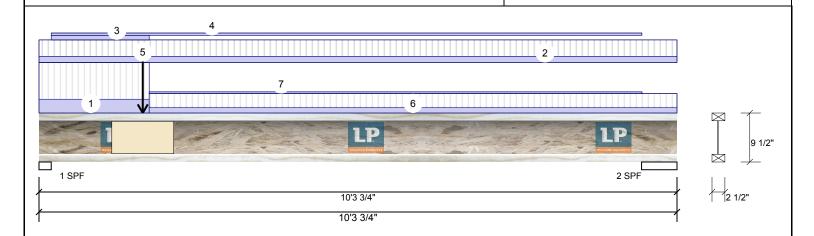
Date: 1/16/2019

Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

LPI 20Plus 9.500" - PASSED Level: Ground Floor



Member Info	rmation			Unfactor	ed Reacti	ions UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	505	251	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	324	160	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	67% 314 / 758	1072 L	1.25D+1.5L
				2 - SPF 6	6.875"	39% 199 / 486	685 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1756 ft-lb	4'2 15/16"	4670 ft-lb	0.376 (38%)	1.25D+1.5L	L
Shear	1047 lb	1 5/8"	1990 lb	0.526 (53%)	1.25D+1.5L	L
Perm Defl in.	0.048 (L/2431)	4'8 3/4"	0.322 (L/360)	0.150 (15%)	D	Uniform
LL Defl inch	0.095 (L/1223)	4'8 11/16"	0.322 (L/360)	0.290 (29%)	L	L
TL Defl inch	0.143 (L/814)	4'8 3/4"	0.483 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.048", Long Term = 0.072"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'10" o.c.

6 Bottom flange braced at bearings.



Page 1 of 1

January 17, 2019

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
	1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF
ı	2	Tie-In	0-0-0 to 10-3-12	(Span)1-5-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF
ı	3	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF
ı	4	Part. Uniform	0-2-6 to 9-8-15		Тор	4 PLF	0 PLF	0 PLF	0 PLF
ı	5	Point	1-8-2		Far Face	103 lb	209 lb	0 lb	Pass-Thru Framing Squash Block is
ı	6	Tie-In	1-9-6 to 10-3-12	(Span)1-2-13	Тор	15 PSF	40 PSF	0 PSF	required at all point loads over bearings
	7	Part. Uniform	1-9-6 to 9-8-15		Тор	3 PLF	0 PLF	0 PLF	Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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This design is valid until 10/31/2020

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C

requirements





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Client: Project: Address:

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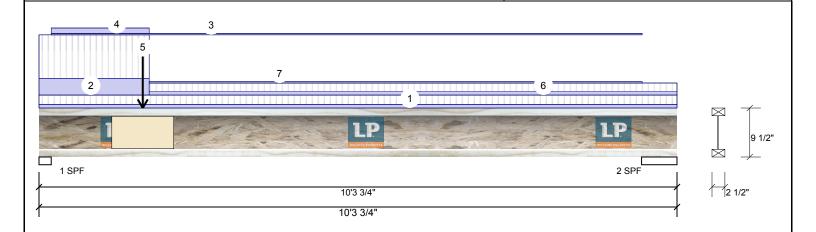
Date: 1/16/2019 Designer: SB

Job Name: LOT-17 (LIANA 2 EL-1)

Project #

F8-B LPI 20Plus 9.500" - PASSED

Level: Ground Floor



Member Info	rmation			Unfactore	d Reacti	ons UNPA	TTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dea	d	Snov	v	Wind
Plies:	1	Design Method:	LSD	1	439	21	9		0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	192	9	7	(0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Fact	ored React	ions			
Dead:	15 PSF			Bearing L	ength.	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF 2	.375"	58% 27	4 / 658	932	L	1.25D+1.5L
				2-SPF 6	.875"	23% 12	1 / 288	409	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1272 ft-lb	3'1 13/16"	4670 ft-lb	0.272 (27%)	1.25D+1.5L	L
Shear	911 lb	1 5/8"	1990 lb	0.458 (46%)	1.25D+1.5L	L
Perm Defl in.	0.034 (L/3418)	4'6 3/16"	0.322 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.066 (L/1747)	4'6"	0.322 (L/360)	0.210 (21%)	L	L
TL Defl inch	0.100 (L/1156)	4'6 1/16"	0.483 (L/240)	0.210 (21%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.034", Long Term = 0.051"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'11" o.c.

6 Bottom flange braced at bearings.



Page 1 of 1

ID Load Type Location Trib Width Side Dead Live Wind Comments Snow Tie-In 0-0-0 to 10-3-12 (Span)0-8-5 Top 15 PSF 40 PSF 0 PSF 0 PSF 2 Tie-In 0-0-0 to 1-9-6 (Span)3-2-8 Top 15 PSF 40 PSF 0 PSF 0 PSF 0 PLF 3 Part. Uniform 0-2-6 to 9-9-0 Top 2 PLF 0 PLF 0 PLF Part. Uniform 0-2-6 to 1-9-6 Top 8 PI F 0 PI F 0 PI F 0 PLF Pass-Throu Framing Squash Block is 5 Point 1-8-2 Near Face 131 lb 264 lb 0 lb required at all point loads over bearings 15 PSF 40 PSF 0 PSF 6 Tie-In 1-9-6 to 10-3-12 (Span)0-7-11 Top Part. Uniform 2 PLF 0 PLF 0 PLF Refer to Multiple Member Connection 1-9-6 to 9-9-0 Top

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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CCMC: 12412-R APA: PR-L238C

requirements

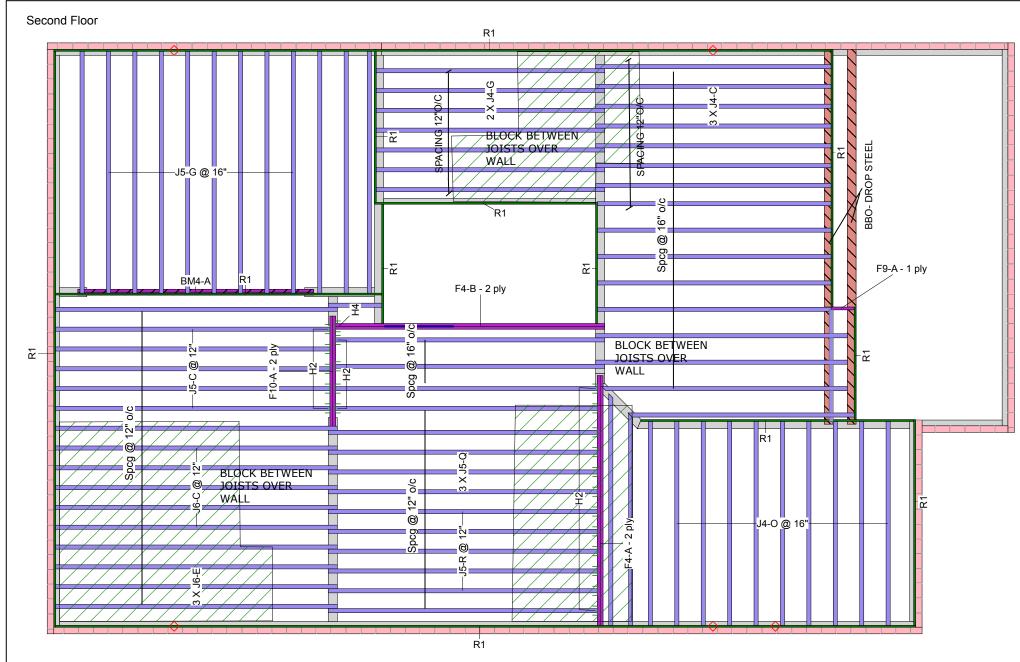
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Detail for ply to ply nailing or bolting

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JOISTS SPACING 16"O/C UNI ESS NOTED OTHERWISE

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.4: Dec 21.2018 Project No: 17-55 Model: LOT-17 (Liana 2 EL-1)

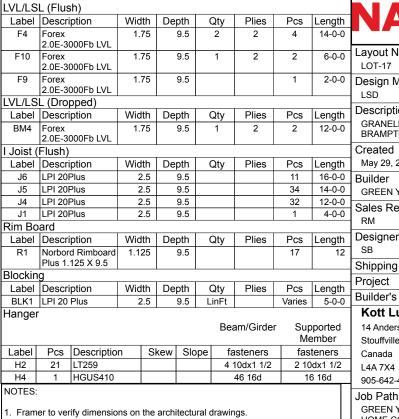
- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

This certification is to confirm that:

- 1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
- 2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.





- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Second Floor

- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- . Refer to Nascor specifier guide for installation works.
- Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- . It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction

Legend	
PS	Point Load Support
♦	Load from Above
	Wall
	Norbord Rimboard Plus 1.125 X 9.5
	LPI 20Plus 9.5
	Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped)
	Forex 2.0E-3000Fb LVL 1.75 X 9.5
	5.25 X 8 (Dropped)
•	

LOT-17 (LIANA 2 EL-1) Design Method Description GRANELLI HOMES CORP. BRAMPTON, ONT. Created May 29, 2018 Builder **GREEN YORK HOMES** Sales Rep

Builder's Project **Kott Lumber Company** 14 Anderson Blvd

SB

Stouffville, Ontario Canada I 4A 7X4 905-642-4400

Job Path GREEN YORK HOMES\GRANELLI HOME CORP\MODELS\LOT 17 LIANA

2-1\LOT-17 (LIANA 2 EL-1).isl Second Floor

LSD Design Method Building Code NBCC 2010 / OBC 2012

Floor Loads Live 15 Dead **Deflection Joist** 480 LL Span L/ TL Span L/ 360 480 LL Cant 2L/ TL Cant 2L/ 360 Deflection Girder LL Span L/ 360 TL Span L/ 240 480 LL Cant 2L/ TL Cant 2L/ 360 Decking

OSB Deck Thickness 5/8" Fastener Nailed & Glued Vibration Ceiling: Gypsum 1/2"



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Client: Project: Address:

GREEN YORK HOMES

Date: 1/16/2019 Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

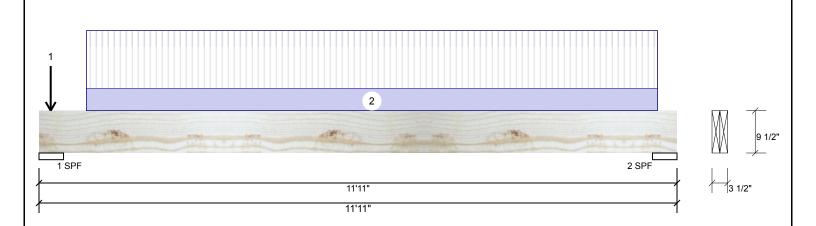
Project #

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Info	rmation			Unfactored	l Reactio	ns UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	1378	565	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1362	559	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF			Bearing Le	ngth	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 5.5	500"	23% 706 / 2068	2774 L	1.25D+1.5L
				2 - SPF 5.5	500"	23% 699 / 2043	2742 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7560 ft-lb	5'11 5/8"	22724 ft-lb	0.333 (33%)	1.25D+1.5L	L
Unbraced	7560 ft-lb	5'11 5/8"	20280 ft-lb	0.373 (37%)	1.25D+1.5L	L
Shear	2345 lb	1'2 1/4"	9277 lb	0.253 (25%)	1.25D+1.5L	L
Perm Defl in	. 0.074 (L/1812)	5'11 9/16"	0.371 (L/360)	0.200 (20%)	D	Uniform
LL Defl inch	0.180 (L/740)	5'11 9/16"	0.371 (L/360)	0.490 (49%)	L	L
TL Defl inch	0.254 (L/525)	5'11 9/16"	0.556 (L/240)	0.460 (46%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slende	rness ratio based	on full section width.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Point	0-2-10		Тор	52 lb	138 lb	0 lb	0 lb
2	Part. Uniform	0-10-10 to 11-6-10		Тор	92 PLF	244 PLF	0 PLF	0 PLF Pass-1
	Self Weight				8 PLF			require

T.L. WISE 100083566 100083566 VCE OF ONTARY January 17, 2019

Page 1 of 1

s-Thru Framing Squash Block is required at all point loads over bearings

Comments

J5

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE

905-642-4400

Canada



Kott Lumber Company 14 Anderson Blvd, Ontario

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design



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Client: Project:

GREEN YORK HOMES

1/16/2019

Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

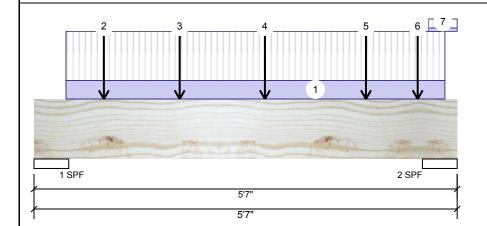
Project #:

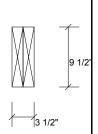
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Page 1 of 2

Member	Information
Type:	Girder
Plies:	2

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

General Load 40 PSF Floor Live: 15 PSF Dead:

Floor (Residential) Application:

Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1316	519	0	0
2	1973	812	0	0

Bearings and Factored Reactions

Bearing Len	igth Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 5.50	00" 22%	649 / 1973	2622	L	1.25D+1.5L
2 - SPF 5.50	00" 34%	1016 / 2960	3975	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3184 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L
Unbraced	3184 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L
Shear	3785 lb	4'4 3/4"	9277 lb	0.408 (41%)	1.25D+1.5L	L
Perm Defl in.	0.007 (L/7853)	2'10 1/8"	0.160 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.019 (L/3081)	2'10 1/8"	0.160 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.026 (L/2213)	2'10 1/8"	0.240 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings
- 6 Lateral slenderness ratio based on full section width.



January 17, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-5-1 to 5-5-1		Far Face	104 PLF	276 PLF	0 PLF	0 PLF	
2	Point	0-11-1		Near Face	112 lb	292 lb	0 lb	0 lb	J5
3	Point	1-11-1		Near Face	105 lb	281 lb	0 lb	0 lb	J5
4	Point	3-0-9		Near Face	123 lb	327 lb	0 lb	0 lb	J5
5	Point	4-4-9		Near Face	100 lb	268 lb	0 lb	0 lb	J5
6	Point	5-0-12		Near Face	322 lb	722 lb	0 lb	0 lb	F4

Continued on page 2...

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is



TW0119-092 Page 29 of 34

isDesign™

Client: Project: Address:

GREEN YORK HOMES

Date: 1/16/2019

Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

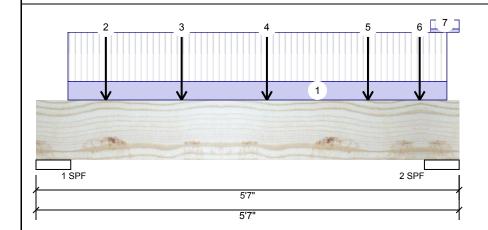
Project #:

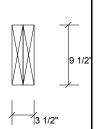
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Page 2 of 2

.Continued from page 1

Tie-In

7

Load Type ID Location Trib Width Side Dead Live Wind Comments Snow Top

5-2-8 to 5-7-0 (Span)2-6-3 Self Weight

15 PSF 8 PLF 40 PSF 0 PSF 0 PSF

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Ianding & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

APA: PR-L318

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design is valid until 10/18/2021

TW0119-092 Page 30 of 34

isDesign™

Client: Project: Address:

GREEN YORK HOMES

Date: 1/16/2019

Designer:

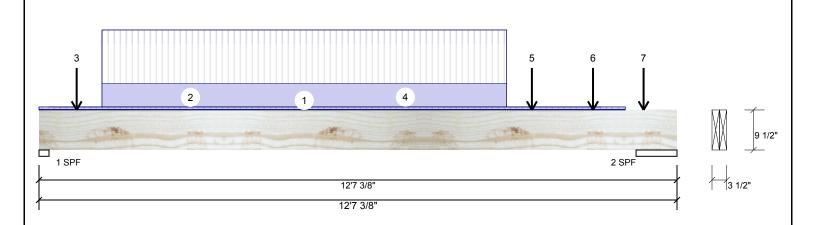
Job Name: LOT-17 (LIANA 2 EL-1)

Page 1 of 2

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED Level: Second Floor



Member Info	rmation			Unfactor	ed Reacti	ions UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	1567	722	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1790	804	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	64% 902 / 2351	3253 L	1.25D+1.5L
		1		2-SPF 9	9.714"	18% 1005 / 2685	3690 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9822 ft-lb	6'	22724 ft-lb	0.432 (43%)	1.25D+1.5L	L
Unbraced	9822 ft-lb	6'	20006 ft-lb	0.491 (49%)	1.25D+1.5L	L
Shear	3223 lb	11 1/8"	9277 lb	0.347 (35%)	1.25D+1.5L	L
Perm Defl in.	0.115 (L/1224)	6' 1/16"	0.391 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.251 (L/561)	6'	0.391 (L/360)	0.640 (64%)	L	L
TL Defl inch	0.366 (L/385)	6' 1/16"	0.587 (L/240)	0.620 (62%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 11-7-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Part. Uniform	0-2-7 to 11-1-5		Тор	1 PLF	0 PLF	0 PLF	0 PLF		
3	Point	0-8-15		Far Face	104 lb	238 lb	0 lb	0 lb	J5	
4	Part. Uniform	1-2-15 to 9-2-15		Far Face	114 PLF	266 PLF	0 PLF	0 PLF		
5	Point	9-8-15		Far Face	128 lb	294 lb	0 lb	0 lb	J5	
6	Point	10-11-7		Far Face	123 lb	292 lb	0 lb	0 lb	J5	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

T.L. WISE 100083566

100083566

January 17, 2019



ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED This design



TW0119-092 Page 31 of 34

Client: **GREEN YORK HOMES** Date: 1/16/2019 Page 2 of 2 Project: Designer: isDesign™ Address: Job Name: LOT-17 (LIANA 2 EL-1) Project #: 1.750" X 9.500" 2-Ply - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL 3 5 6 2 4 1 1 SPF 2 SPF 12'7 3/8' 12'7 3/8' .Continued from page 1

Load Type ID Location Trib Width Side Live Wind Comments Dead Snow 7 Point 11-11-7 Far Face 105 lb 281 lb 0 lb 0 lb Self Weight 8 PLF

> Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Design assumes to be used

 Design assumes top edge is laterally restrained

 Forvoire lateral support at bearing points to avoid lateral displacement and rotation

- This design is valid until 10/18/2021

For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info



TW0119-092 Page 32 of 34

isDesign™

Client: Project:

GREEN YORK HOMES

Date: 1/16/2019

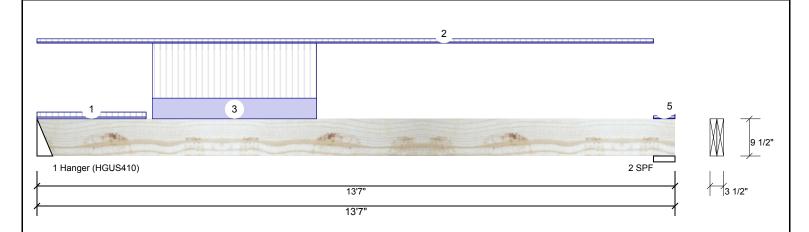
Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED Level: Second Floor



Member Info	Unfactored Reactions UNPATTERNED Ib (Uplift)									
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	2	Design Method:	LSD	1	722		322		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	357		186		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored R	eactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-	4.000"	14%	403 / 1083	1486	L	1.25D+1.5L
				Hanger						
Analysis Resu	ılts			2 - SPF	5.500"	6%	233 / 536	768	L	1.25D+1.5L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4669 ft-lb	5' 1/16"	22724 ft-lb	0.205 (21%)	1.25D+1.5L	L
Unbraced	4669 ft-lb	5' 1/16"	19429 ft-lb	0.240 (24%)	1.25D+1.5L	L
Shear	1402 lb	1' 3/4"	9277 lb	0.151 (15%)	1.25D+1.5L	L
Perm Defl in.	0.058 (L/2668)	6'2 5/8"	0.431 (L/360)	0.130 (13%)	D	Uniform
LL Defl inch	0.128 (L/1209)	6'1 5/16"	0.431 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.186 (L/832)	6'1 3/4"	0.646 (L/240)	0.290 (29%)	D+L	L

T.L. WISE 100083566 VCE OF OF January 17, 2019

Page 1 of 1

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

Self Weight

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 2-3-14	(Span)1-0-13	Тор	15 PSF	40 PSF	0 PSF
2	Tie-In	0-0-0 to 13-1-8	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF
3	Part. Uniform	2-5-7 to 5-11-7		Тор	90 PLF	240 PLF	0 PLF
4	Tie-In	13-1-8 to 13-7-0	(Span)0-5-11	Тор	15 PSF	40 PSF	0 PSF
5	Tie-In	13-2-10 to 13-7-0	(Span)0-10-5	Top	15 PSF	40 PSF	0 PSF

0 PSF Pass-Thru Framing Squash Block is required at all point loads over bearings

Comments

Wind

0 PSF

0 PSF Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

8 PLF

This design is

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Forex

Manufacturer Info

APA: PR-L318



TW0119-092 Page 33 of 34

isDesign™

Client: **GREEN YORK HOMES**

Designer:

Job Name: LOT-17 (LIANA 2 EL-1)

1/16/2019

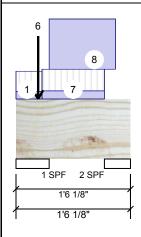
Project #:

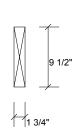
1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Project:

Address:

Level: Second Floor





Page 1 of 2

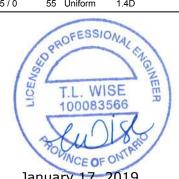
Member Information					Unfactored Reactions UNPATTERNED lb (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	V	Wind			
Plies:	1	Design Method:	LSD	1	283	457	566	3	0			
Moisture Conditi	ion: Dry	Building Code:	NBCC 2010 / OBC 2012	2	10	39	()	0			
Deflection LL:	360	Load Sharing:	No									
Deflection TL:	240	Deck:	Not Checked									
Importance:	Normal	Vibration:	Not Checked									
General Load												
Floor Live:	40 PSF			Bearings	s and Fac	tored Reactions						
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.			
				1 - SPF	5.250"	37% 571 / 991	1561	L	1.25D+1.5S +0.5L			
Analysis Resu	ılts		2 - SPF	4.125"	2% 55 / 0	55	Uniform	1.4D				

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10 ft-lb	9 5/8"	7385 ft-lb	0.001 (0%)	1.4D	Uniform
Unbraced	10 ft-lb	9 5/8"	7385 ft-lb	0.001 (0%)	1.4D	Uniform
Shear	40 lb	5 1/4"	3015 lb	0.013 (1%)	1.4D	Uniform
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



January 17, 2019

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-3-8		Тор	10 lb	0 lb	23 lb	0 lb	
3	Point	0-3-8		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-3-8		Тор	386 lb	262 lb	537 lb	0 lb	F12 F12
5	Point	0-3-8		Тор	2 lb	0 lb	6 lb	0 lb	
6	Point	0-3-8		Тор	6 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	Tie-In	0-4-2 to 1-2-0	(Span)1-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Notes

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6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE This design

TW0119-092 Page 34 of 34

isDesign™

Client: Project:

Address:

GREEN YORK HOMES

Date: 1/16/2019

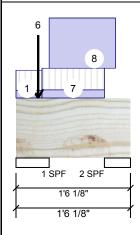
Designer:

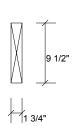
Job Name: LOT-17 (LIANA 2 EL-1)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Page 2 of 2

.Continued from page 1

Location Trib Width Comments ID Load Type Side Dead Live Wind Snow 8 Part. Uniform 0-5-4 to 1-3-12 Тор 64 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight

> Self Weight 4 PLF

> > Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

NOtes

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Manufacturer Info Forex

APA: PR-L318

