NE0119-095 PAGE 1 OF 29

Engineering Note Page (ENP-2)

GREEN YORK HOMES- LOT 18 (LIANA -2 EL-2)-BRAMPTON-ON **REVISION 2009-10-09**

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.

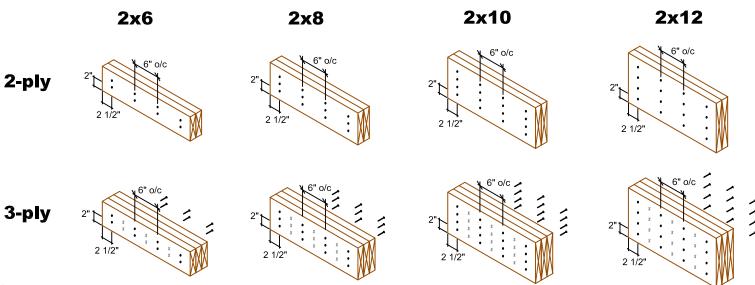


NE0119-095 PAGE 2 OF 29

FIPLE MEMBER CONNECTIONS

GREEN YORK HOMES-LOT 18 (LIANA -2 EL-2)-BRAMPTON-ON

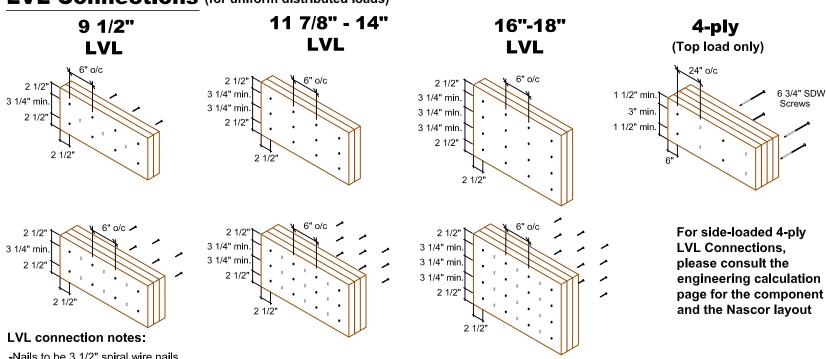
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

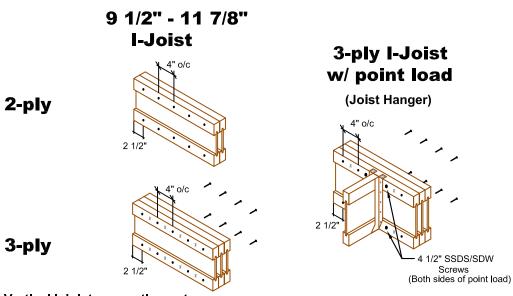
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

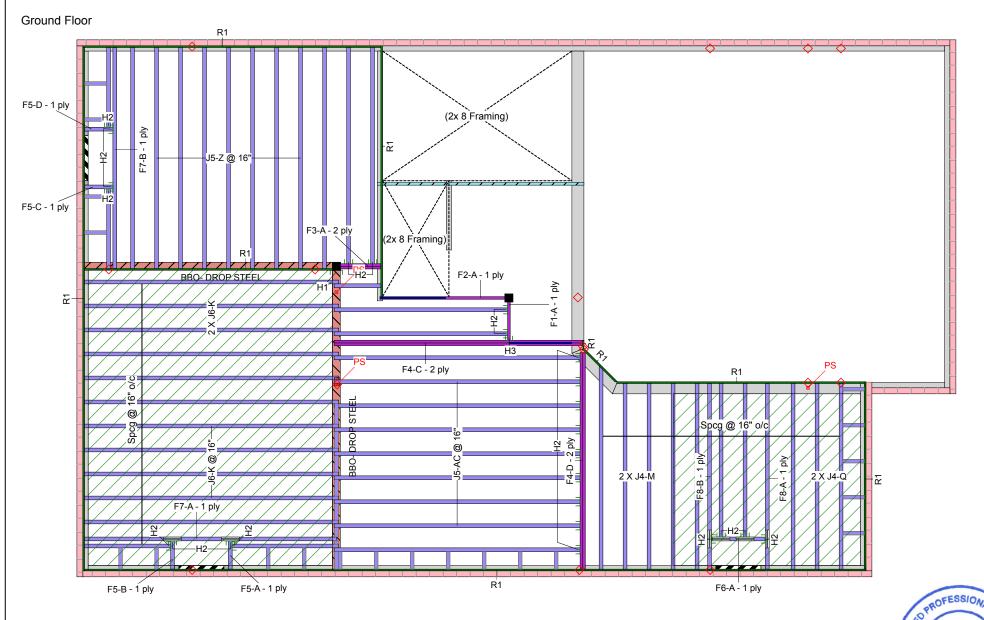


MULTI-PLY CONNECTION **DETAILS**

> Date: November 30, 2016 Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751

<u>3 OF 2</u>9 NE0119-095



ENG.JOB:NE0119-05

I.A. EL-MASRI

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.4; Dec 21,2018 Project No: 17-55 Model: lot-18 (Liana 2 el-2)

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

											F	PAGE 3
Ground Floor												
LVL/LSL (Flush)									AC			
Label	Descri	iption	Widt	n De	pth	C	Qty	Plies	Pcs	Length		AS
F4	Forex 2.0E-3	000Fb LVL	1.7	5	9.5		2	2	4	14-0-0		
F2	Forex 2.0E-3	000Fb LVL	1.7	5	9.5				1	8-0-0	, ,	t Name 8 (LIANA 2
F3	Forex 2.0E-3	000Fb LVL	1.7	5	9.5		1	2	2	4-0-0	Desigr	n Method
F1	Forex 2.0E-3	000Fb LVL	1.7	5	9.5				1	4-0-0	Descri	ption
I Joist	(Flush)			•						•		IELLI HOME
Label	Descri	iption	Widt	n De	pth	C	Qty	Plies	Pcs	Length	BRAN	IPTON, ONT
F7	LPI 20I	Plus	2.	5	9.5				2	14-0-0	Create	
F8	LPI 20I	Plus	2.	5	9.5				2	12-0-0) May 2	9, 2018
F6	LPI 20I	Plus	2.	5	9.5				1	4-0-0	Builde	r
F5	LPI 20I	Plus	2.	5	9.5				4	2-0-0		IN YORK HO
J6	LPI 20I		2.	_	9.5				11	16-0-0) Coloo	
J5	LPI 20I		2.		9.5				20	14-0-0	DM	Keh
J4	LPI 201		2.	_	9.5				8	12-0-0	<u>'</u>	
J3	LPI 201		2.		9.5				4	10-0-0		
J2	LPI 201		2.		9.5				4	6-0-0		3B
J1	LPI 20I	Plus	2.	5	9.5				1	4-0-0	Shippi	ng
Rim Bo			147 10					DI:	_		Projec	t
Label			Widt		pth	(Qty	Plies	Pcs	Length	Duilda	r's Project
R1		d Rimboard 125 X 9.5	1.12	b	9.5				13	12	•	
Blockir		125 X 3.5										Lumber
Label	-	intion	Widt	n De	pth	_	Qty	Plies	Pcs	Length		derson Blvd
BLK1	LPI 20		2.	_	9.5		nFt	1 1103	Varies	29-0-0	Stouii	ville, Ontario
Hange		1 103		<u> </u>	3.5				varios	2500	Canac	
liunge	•						Bea	am/Girde		ported	L4A 72	X4 42-4400
									Me	ember		
Label	Label Pcs Description			Skew	Slop	ре	fa	steners	fas	teners	Job Pa	
H1	1	Unknown Hanger										IELLI HOME 18 LIANA 2-2 A 2 EL-2).isl
H2	25	LT259						10dx1 1/2		dx1 1/2	<u> </u>	
H3	H3 1 HUS1.81/10 30 16d 10 16d									d Floor		
NOTES:									Method			
								Building	g Code NE			
Framer to verify dimensions on the architectural drawings.								Floor				
Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.							Loads					
		cking @ 24"					oad b	earing wal	ls.		Live	
4. Instal	l single-p	oly flush wind									1	
rimboard/rimjoist.							Dead					

- . Refer to Nascor specifier guide for installation works. . Squash blocks recommended to be installed at end bearing on
- all first level joists which support loading from above exceeding
- Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction

Legend



Point Load Support Load from Above Wall Opening

Norbord Rimboard Plus 1.125 X 9.5 LPI 20Plus 9.5

Forex 2.0E-3000Fb LVL 1.75 X 9.5 0 X 0 (Dropped)

1.75 X 9.5 (Dropped) 5.25 X 8 (Dropped)

EL-2) ES CORP. OMES

Company

E CORP\MODELS -2\FLOOR\LOT-18

NBCC 2010 / OBC

Deflection Joist LL Span L/

480 TL Span L/ 360 LL Cant 2L/ TL Cant 2L/ 360 Deflection Girder LL Span L/ 360 TL Span L/ 240 LL Cant 2L/

480 TL Cant 2L/ 360 Decking Deck OSB Thickness Nailed & Glued

Fastener Vibration

PAGE 4 OF 29 NE0119-095



Client: Project:

GREEN YORK HOMES

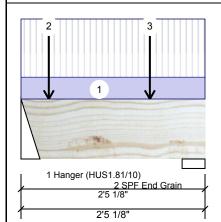
Address:

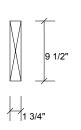
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Page 1 of 1

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	vvina
1	299	116	0	0
2	258	101	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	214 ft-lb	1'6 7/8"	11362 ft-lb	0.019 (2%)	1.25D+1.5L	L
Unbraced	214 ft-lb	1'6 7/8"	10729 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	438 lb	11 3/4"	4638 lb	0.094 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/58713)	1'4 3/8"	0.067 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/23005)	1'4 9/16"	0.067 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.001 (L/16529)	1'4 7/16"	0.100 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	3.000"	15%	145 / 449	594	L	1.25D+1.5L
2 - SPF End Grain	3.625"	11%	127 / 387	514	L	1.25D+1.5L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

Dottom brace	
)	Comments
	J3
	J3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





NE0119-095 PAGE 5 OF 29

isDesign'

Client: Project: Address:

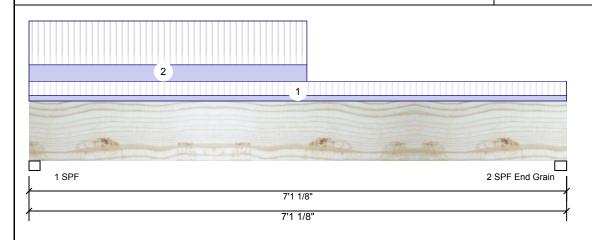
GREEN YORK HOMES

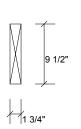
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	156	72	0	0
2	84	45	0	0

Analysis Results

Dead:

15 PSF

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	453 ft-lb	2'11 9/16"	11362 ft-lb	0.040 (4%)	1.25D+1.5L	L
Unbraced	453 ft-lb	2'11 9/16"	5389 ft-lb	0.084 (8%)	1.25D+1.5L	L
Shear	228 lb	10 1/2"	4638 lb	0.049 (5%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/20045)	3'4 1/8"	0.231 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.009 (L/9718)	3'3 1/2"	0.231 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.013 (L/6545)	3'3 11/16"	0.346 (L/240)	0.040 (4%)	D+L	L

Bearings and Factored Reactions

Bearing Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 1.750"	17%	90 / 234	324	L	1.25D+1.5L
2 - SPF 1.875" End	7%	56 / 125	182	L	1.25D+1.5L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead
1	Tie-In	0-0-0 to 7-1-2	(Span)0-7-14	Тор	15 PSF
2	Part. Uniform	0-0-0 to 3-8-0		Тор	15 PLF
	Self Weight				4 PLF

Live Snow Wind Comments 40 PSF 0 PSF 0 PSF

0 PLF

0 PLF

40 PLF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

Forex APA: PR-L318

Canada





PAGE 6 OF 29 NE0119-095

isDesign'

Client: Project: Address:

GREEN YORK HOMES

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

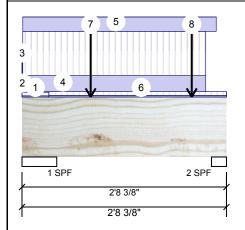
Forex 2.0E-3000Fb LVL

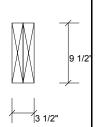
1.750" X 9.500"

2-Ply - PASSED

1

Level: Ground Floor





Wind

O

0

Page 1 of 2

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Dead Live

2	522	292	0	0

380

Bearings and Factored Reactions Can React D/L lb Rearing Length

669

Bearing Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 5.500"	12%	475 / 1003	1478	L	1.25D+1.5L
2 - SPF 2.375"	22%	365 / 783	1149	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	607 ft-lb	1'4 1/8"	22724 ft-lb	0.027 (3%)	1.25D+1.5L	L
Unbraced	607 ft-lb	1'4 1/8"	22724 ft-lb	0.027 (3%)	1.25D+1.5L	L
Shear	728 lb	1'2 1/4"	9277 lb	0.079 (8%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/32360)	1'5 1/8"	0.072 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/18075)	1'4 11/16"	0.072 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.002 (L/11600)	1'4 13/16"	0.108 (L/240)	0.020 (2%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.

15 PSF

- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6	Lateral slen	derness rat	io based	on full	section	width

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-4	(Span)0-10-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 0-0-2		Тор	46 PLF	123 PLF	0 PLF	0 PLF	J5
3	Part. Uniform	0-0-0 to 0-0-2		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Part. Uniform	0-0-2 to 2-5-2		Тор	92 PLF	246 PLF	0 PLF	0 PLF	J5
5	Part. Uniform	0-0-2 to 2-6-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

Forex APA: PR-L318





PAGE 7 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

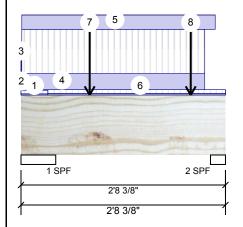
Project #:

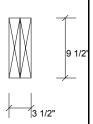
Forex 2.0E-3000Fb LVL F3-A

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Tie-In	0-4-4 to 2-8-6	(Span)1-0-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	0-10-14		Far Face	120 lb	319 lb	0 lb	0 lb	J5
8	Point	2-2-14		Far Face	83 lb	220 lb	0 lb	0 lb	J5
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





PAGE 8 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

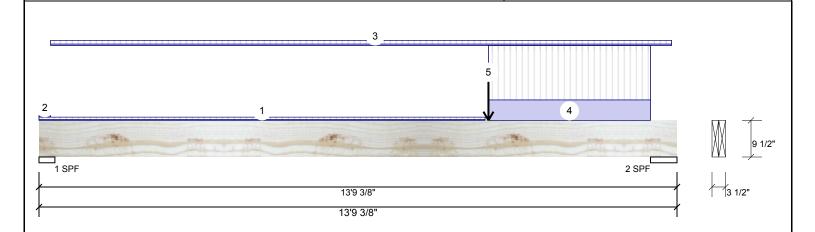
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



∕lember Info	rmation			Unfactore	ed Reacti	ions UNP	ATTERN	IED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	De	ad	Snov	v	Wind
Plies:	2	Design Method:	LSD	1	374		193		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1087	4	164		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fact	ored Read	ctions			
Dead:	15 PSF			Bearing L	_ength	Cap. Rea	ct D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF 4	1.125"	9%	241 / 560	801	L	1.25D+1.5L
				2-SPF 6	3.875"	15% 5	80 / 1631	2211	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4654 ft-lb	9'8 1/2"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
Unbraced	4654 ft-lb	9'8 1/2"	19392 ft-lb	0.240 (24%)	1.25D+1.5L	L
Shear	1816 lb	12'5 3/4"	9277 lb	0.196 (20%)	1.25D+1.5L	L
Perm Defl in.	0.058 (L/2669)	7'3 7/8"	0.433 (L/360)	0.130 (13%)	D	Uniform
LL Defl inch	0.127 (L/1225)	7'5 1/4"	0.433 (L/360)	0.290 (29%)	L	L
TL Defl inch	0.186 (L/840)	7'4 13/16"	0.649 (L/240)	0.290 (29%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Page 1 of 1

0	nacinicos ratio basca	on rail ocollon wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 9-7-10	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-3-0	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	0-3-0 to 13-7-15	(Span)0-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	9-8-8 to 13-2-8		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
5	Point	9-8-8		Far Face	116 lb	299 lb	0 lb	0 lb	F1
	Self Weight				8 PLF				
l									

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

Forex APA: PR-L318





PAGE 9 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

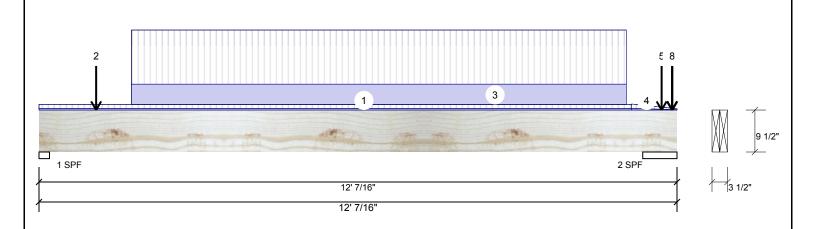
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED Level: Ground Floor



Member Info	rmation			Unfactore	ed Reacti	ions UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	1545	622	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1776	737	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Fact	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	61% 777 / 2317	3094 L	1.25D+1.5L
	-			2 - SPF 7	7.754"	21% 921 / 2664	3586 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9242 ft-lb	5'9 9/16"	22724 ft-lb	0.407 (41%)	1.25D+1.5L	L
Unbraced	9242 ft-lb	5'9 9/16"	20195 ft-lb	0.458 (46%)	1.25D+1.5L	L
Shear	3271 lb	11 1/8"	9277 lb	0.353 (35%)	1.25D+1.5L	L
Perm Defl in.	0.092 (L/1482)	5'9 9/16"	0.377 (L/360)	0.240 (24%)	D	Uniform
LL Defl inch	0.229 (L/594)	5'9 9/16"	0.377 (L/360)	0.610 (61%)	L	L
TL Defl inch	0.320 (L/424)	5'9 9/16"	0.566 (L/240)	0.570 (57%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



- Later ar didinating to factor our fair coolers within										
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 11-2-3	(Span)1-0-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	1-0-15		Far Face	124 lb	332 lb	0 lb	0 lb	J5	
3	Part. Uniform	1-8-15 to 11-0-15		Far Face	101 PLF	270 PLF	0 PLF	0 PLF		
4	Tie-In	11-2-3 to 12-0-7	(Span)1-1-0 to 0-2-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
5	Point	11-8-15		Far Face	48 lb	128 lb	0 lb	0 lb	J5	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





PAGE 10 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

1/16/2019

Designer: RCO /SB Job Name: LOT-18 (LIANA 2 EL-2)

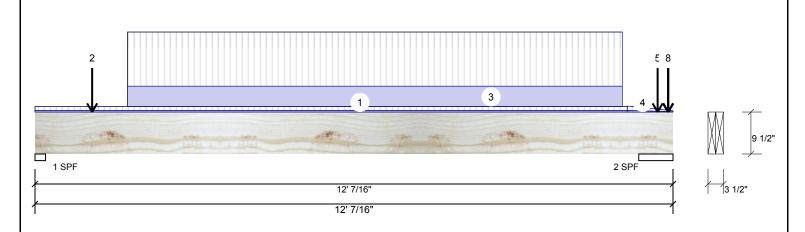
Project #:

Forex 2.0E-3000Fb LVL F4-D

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Continued	from	page	1
-----------	------	------	---

ID	Load Type	Location Trib Widt	h Side	Dead	Live	Snow	Wind	Comments
6	Point	11-11-5	Тор	16 lb	34 lb	0 lb	0 lb	J4
7	Point	11-11-5	Тор	25 lb	68 lb	0 lb	0 lb	J5
8	Point	11-11-5	Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Self Weight			8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





NE0119-095 PAGE 11 OF 29

isDesign

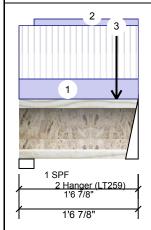
Client: Project: Address: **GREEN YORK HOMES**

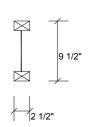
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

9.500" - PASSED LPI 20Plus

Level: Ground Floor





Member Information Type:

Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live:

15 PSF

Application: Design Method: **Building Code:** Load Sharing:

Floor (Residential) NBCC 2010 / OBC 2012

No Not Checked

Deck: Vibration: Not Checked **Unfactored Reactions UNPATTERNED Ib (Uplift)**

Brg	Live	Dead	Snow	Wind
1	65	31	0	0
2	135	67	0	0

Analysis Results

Dead:

Analysis Actual Location Allowed Comb. Case Capacity 0.011 (1%) 1.25D+1.5L L Moment 52 ft-lb 1' 1/8" 4670 ft-lb Shear 1'5 5/8" 1990 lb 0.137 (14%) 1.25D+1.5L L 11 7/8" 0.044 (L/360) 0.010 (1%) D Perm Defl in. 0.000 Uniform (L/54318) LL Defl inch 0.001 11 7/8" 0.044 (L/360) 0.010 (1%) L L (L/27008) TL Defl inch 0.001 11 7/8" 0.067 (L/240) 0.010 (1%) D+L L

0-2-6 to 1-6-14

1-3-7

Bearings and Factored Reactions

Bearing	Length	Cap. Rea	act D/L lb	Iotal	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	8%	39 / 98	136	L	1.25D+1.5L	
2 - Hanger	2.000"	18%	84 / 202	286	L	1.25D+1.5L	

Comments

(L/18039) **Design Notes**

1 Provide restraint at supports to ensure lateral stability.

Part. Uniform

Point

- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

0 PLF

0 lb

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



	e braced at bearings. ange braced at bearings.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF

Top

Near Face

2

3

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



8 PLF

49 lb

0 PLF

99 lb

Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C

0 PLF

0 lb .12





NE0119-095 PAGE 12 OF 29

isDesign™

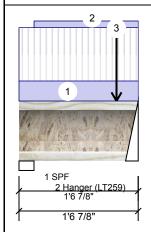
Client: Project: Address: **GREEN YORK HOMES**

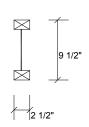
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

9.500" - PASSED LPI 20Plus

Level: Ground Floor





Member Inform	ation
Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360

360 240 Normal

General Load 40 PSF Floor Live: 15 PSF Dead:

Application: Floor (Residential) Design Method:

> **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No

Deck: Not Checked Vibration:

Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	vvina
1	63	30	0	0
2	120	59	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	8%	37 / 94	131	L	1.25D+1.5L
2 - Hanger	2.000"	16%	74 / 180	254	L	1.25D+1.5L

Analysis Results

Deflection TL:

Importance:

Analysis Actual Location Allowed Comb. Case Capacity 0.010 (1%) 1.25D+1.5L L Moment 48 ft-lb 11 11/16" 4670 ft-lb Shear 240 lb 1'5 5/8" 1990 lb 0.121 (12%) 1.25D+1.5L L 11 1/2" 0.044 (L/360) 0.010 (1%) D Perm Defl in. 0.000 Uniform (L/59285) LL Defl inch 0.001 11 1/2" 0.044 (L/360) 0.010 (1%) L L (L/29321) TL Defl inch 0.001 11 1/2" 0.067 (L/240) 0.010 (1%) D+L L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.

(L/19618)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



0 Bollom hange	braceu at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF		
3	Point	1_3_7		Far Face	40 lh	82 lh	0 lh	0.lh	12	

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 13 OF 29 NE0119-095

isDesign™

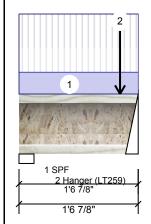
Client: Project: Address: **GREEN YORK HOMES**

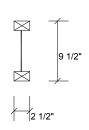
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

9.500" - PASSED LPI 20Plus

Level: Ground Floor





Wind

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Total Ld. Case

116 L

227 L

Member Inform			
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg

1	59	22	0	0
2	115	44	0	0
1 2				

Cap. React D/L lb

28 / 88

55 / 173

Bearing Length 1 - SPF 2.375" 2.000"

Anal	ysis	Results	
------	------	---------	--

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	39 ft-lb	11"	4670 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	214 lb	1'5 5/8"	1990 lb	0.108 (11%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/33835)	10 7/8"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/24562)	10 7/8"	0.067 (L/240)	0.010 (1%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

7%

14%

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Bearings and Factored Reactions

2 -

Hanger

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-4-0		Near Face	28 lb	73 lb	0 lb	0 lb	J2

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 14 OF 29 NE0119-095

isDesign™

Client: Project: Address: **GREEN YORK HOMES**

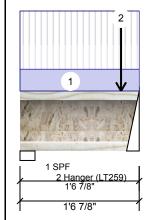
1/16/2019 Designer: RCO/SB

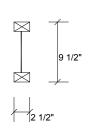
Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

9.500" - PASSED LPI 20Plus

Level: Ground Floor





Wind

0

0

1.25D+1.5L

O

231 L

Page 1 of 1

	Member Inform	mation		
	Type:	Girder	Application:	Floor (Residential)
	Plies:	1	Design Method:	LSD
	Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012
	Deflection LL:	360	Load Sharing:	No
ı	Deflection TL:	240	Deck:	Not Checked
	Importance:	Normal	Vibration:	Not Checked
	General Load			
ı	Floor Live:	40 PSF		
ı	Dead:	15 PSF		

Unfactored	Reactions	UNPATTERNED	lb (Uplift)
Brg	Live	Dead	Snow

22

44

Bearings and Factored Reactions								
Bearing Le	ngth Cap.	React D/L lb T	otal	Ld. Case	Ld. Comb.			
1 - SPF 2.3	375" 7%	28 / 89	116	L	1.25D+1.5L			

55 / 177

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	39 ft-lb	11 1/16"	4670 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	218 lb	1'5 5/8"	1990 lb	0.110 (11%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/33432)	10 15/16"	0.044 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/24349)	10 15/16"	0.067 (L/240)	0.010 (1%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

15%

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL

59

118

2.000"

2

2 -

Hanger

POINT LOADS OVER BEARINGS. READ ALL NOTES ON THIS PAGE AND ON

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



gn	NO	τes
	<u>gn</u>	gn No

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.000", Long Term = 0.000"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-4-0		Far Face	28 lb	76 lb	0 lb	0 lb	J2

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 15 OF 29 NE0119-095

isDesign™

Client: Project: Address:

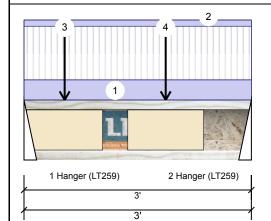
GREEN YORK HOMES

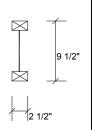
Date: 1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

9.500" - PASSED LPI 20Plus

Level: Ground Floor





Member	Information
Tunai	Cindon

Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Floor (Residential) Design Method:

NBCC 2010 / OBC 2012

Load Sharing: No

Building Code:

Deck: Not Checked Vibration: Not Checked

Analysis Results

, ,						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	407 ft-lb	1'10 7/16"	4670 ft-lb	0.087 (9%)	1.25D+1.5L	L
Shear	552 lb	1 1/4"	1990 lb	0.277 (28%)	1.25D+1.5L	L
Perm Defl in.	0.003 (L/12219)	1'10 7/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.006 (L/6031)	1'10 7/16"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.008 (L/4038)	1'10 7/16"	0.140 (L/240)	0.060 (6%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.003", Long Term = 0.004"
- 3 Fill all hanger nailing holes.
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange braced at bearings.
- 7 Bottom flange braced at bearings.

Brg	Live	Dead	Snow	Wind
1	264	131	0	0
2	209	103	0	0

Bearings and Factored Reactions

Bearing Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - 2.000"	36%	164 / 396	559	L	1.25D+1.5L	
Hanger						
2 - 2.000"	28%	129 / 313	442	L	1.25D+1.5L	
Hanger						

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



	<u> </u>								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-6-7		Far Face	80 lb	160 lb	0 lb	0 lb	J3
4	Point	1-10-7		Far Face	103 lb	209 lb	0 lb	0 lb	J3

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 16 OF 29 NE0119-095

isDesign™

Client: Project: Address:

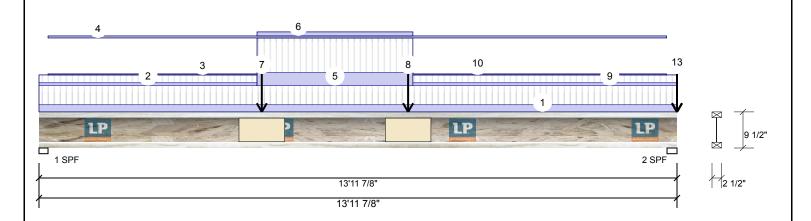
GREEN YORK HOMES

Date: 1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

LPI 20Plus 9.500" - PASSED Level: Ground Floor



Member Infor	mation			Unfactore	d Reacti	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	370	181	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	531	272	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	.ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	375"	49% 226 / 555	781 L	1.25D+1.5L
				2-SPF 2	.625"	70% 340 / 797	1137 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3313 ft-lb	7'1"	4670 ft-lb	0.709 (71%)	1.25D+1.5L	L
Shear	774 lb	1 5/8"	1990 lb	0.389 (39%)	1.25D+1.5L	L
Perm Defl in.	0.152 (L/1079)	6'11 7/16"	0.457 (L/360)	0.330 (33%)	D	Uniform
LL Defl inch	0.311 (L/529)	6'11 3/8"	0.457 (L/360)	0.680 (68%)	L	L
TL Defl inch	0.463 (L/355)	6'11 3/8"	0.685 (L/240)	0.680 (68%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Applied loads over end bearings and loads exceeding 250 lbs over intermediate bearings must be transferred directly to the support by rim board, blocking, squash blocks, or other device.
- 3 Dead Load Deflection: Instant = 0.152", Long Term = 0.228"
- 4 See manufacture installation guide note E4 for installation details
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top flange must be laterally braced at a maximum of 4'11" o.c.
- 7 Bottom flange braced at bearings

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-11-14	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 4-9-6	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 4-9-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 13-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	4-9-6 to 8-2-6	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 17 OF 29 NE0119-095

isDesign™

Project:

Client: **GREEN YORK HOMES**

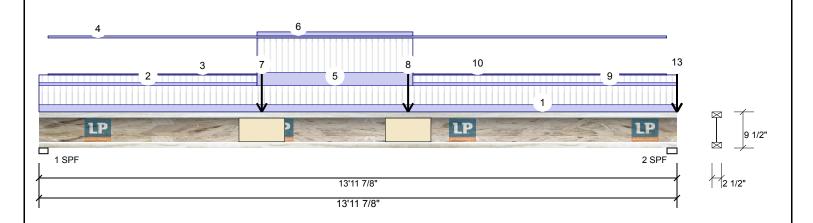
Address:

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

LPI 20Plus 9.500" - PASSED Level: Ground Floor



ŀ	Continued from p	age 1								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	6	Part. Uniform	4-9-6 to 8-2-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
	7	Point	4-10-10		Near Face	59 lb	120 lb	0 lb	0 lb	F5
	8	Point	8-1-2		Near Face	67 lb	135 lb	0 lb	0 lb	F5
	9	Tie-In	8-2-6 to 13-11-14	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	10	Part. Uniform	8-2-6 to 13-9-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	11	Point	13-11-14		Тор	34 lb	89 lb	0 lb	0 lb	J5
		Bearing Length	0-1-8							
	12	Point	13-11-14		Тор	41 lb	93 lb	0 lb	0 lb	J6
		Bearing Length	0-1-8							
	13	Point	13-11-14		Тор	27 lb	0 lb	0 lb	0 lb	Wall Self Weight
		Bearing Length	0-1-8							

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





NE0119-095 PAGE 18 OF 29

isDesign™

Client: Project: Address:

:: GREEN YORK HOMES

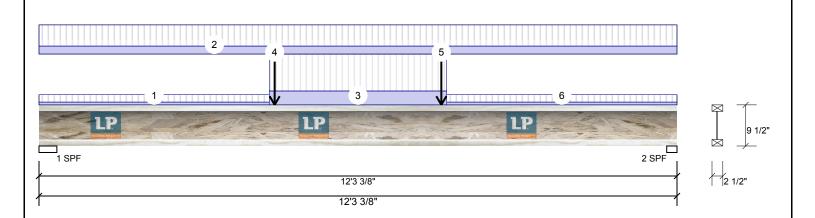
Date: 1/16/2019
Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

F7-B LPI 20Plus 9.500" - PASSED

Level: Ground Floor



rmation			Unfactore	d Reacti	ons UNPAT	TERNED II	o (Uplift)	
Girder	Application:	Floor (Residential)	Brg	Live	Dead	S	now	Wind
1	Design Method:	LSD	1	331	125		0	0
n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	324	122		0	0
360	Load Sharing:	No						
240	Deck:	Not Checked						
Normal	Vibration:	Not Checked						
40 PSF			Bearings a	and Fact	ored Reaction	ns		
15 PSF			Bearing L	.ength	Cap. React D)/L lb Tot	al Ld. Case	Ld. Comb.
			1 - SPF 4	.125"	37% 156	/ 496 6	52 L	1.25D+1.5L
			2 - SPF 2	375"	40% 152	/ 486 63	38 L	1.25D+1.5L
	1 on: Dry 360 240 Normal	Girder 1 Design Method: Design Method: Building Code: Load Sharing: Deck: Normal Vibration: 40 PSF 15 PSF	Girder 1 Design Method: LSD 2n: Dry 360 Load Sharing: No 240 Deck: Not Checked Normal 40 PSF 15 PSF	Application: Floor (Residential) Brg 1	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live Dead	Application: Floor (Residential) Brg Live Dead State	Application: Floor (Residential) Brg Live Dead Snow

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2390 ft-lb	6'2 3/4"	4670 ft-lb	0.512 (51%)	1.25D+1.5L	L
Shear	637 lb	3 3/8"	1990 lb	0.320 (32%)	1.25D+1.5L	L
Perm Defl in.	0.071 (L/2003)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.189 (L/753)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
TL Defl inch	0.260 (L/547)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.071", Long Term = 0.107"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top flange must be laterally braced at a maximum of 5'11" o.c.

5 Rottom flance braced at hearings

5 Botton	n flange braced at bearings.					CONTAINS S	SPECIFICATI	ONS AND (CRITER
ID	Load Type	Location	Trib Width	Side	Dead	USED IN THI	E DESIGN OF	THIS COM	IPONE
1	Tie-In	0-0-0 to 4-5-3	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-3-6	(Span) 0-11-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	4-5-3 to 7-10-3	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	4-6-7		Far Face	44 lb	115 lb	0 lb	0 lb	F5
5	Point	7-8-15		Far Face	44 lb	118 lb	0 lb	0 lb	F5
6	Tie-In	7-10-3 to 12-3-6	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Page 1 of 1

Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lbcorp.com

www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 19 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

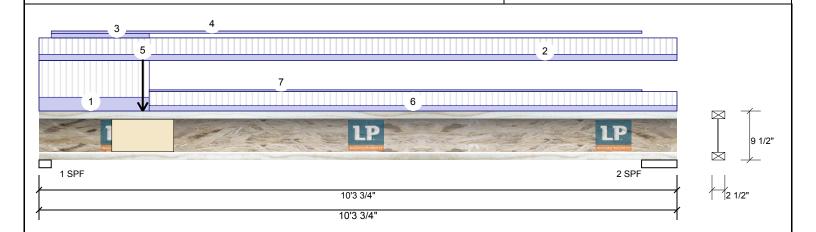
Date: 1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

LPI 20Plus 9.500" - PASSED

Level: Ground Floor



Member Info	rmation			Unfactore	d Reactio	ns UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	505	251	0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	324	160	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	375"	67% 314 / 758	1072 L	1.25D+1.5L
				2-SPF 6	.875"	39% 199 / 486	685 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1756 ft-lb	4'2 15/16"	4670 ft-lb	0.376 (38%)	1.25D+1.5L	L
Shear	1047 lb	1 5/8"	1990 lb	0.526 (53%)	1.25D+1.5L	L
Perm Defl in.	0.048 (L/2431)	4'8 3/4"	0.322 (L/360)	0.150 (15%)	D	Uniform
LL Defl inch	0.095 (L/1223)	4'8 11/16"	0.322 (L/360)	0.290 (29%)	L	L
TL Defl inch	0.143 (L/814)	4'8 3/4"	0.483 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.048", Long Term = 0.072"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 6'10" o.c.

6 Bottom flange braced at bearings.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

PROFESSIONAL	
\$ 'S	
N.A. EL-MASRI	
Was Emast	1
The state of the s	
Jan 22, 2019	
Jan 22, 2019	

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Commen
1	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-3-12	(Span)1-5-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 9-8-15		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	103 lb	209 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 10-3-12	(Span)1-2-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 9-8-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325

www.lpcorp.com CCMC: 12412-R APA: PR-L238C





PAGE 20 OF 29 NE0119-095

isDesign™

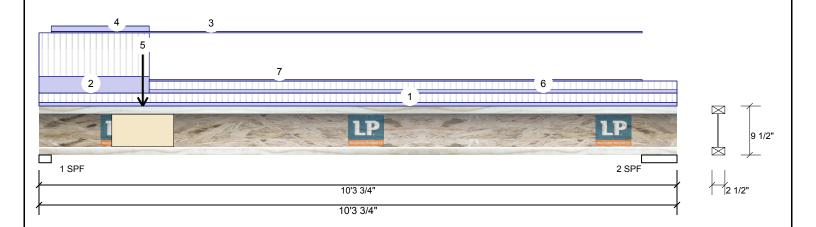
Client: Project: Address: **GREEN YORK HOMES**

Date: 1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

9.500" - PASSED LPI 20Plus

Level: Ground Floor



Member Info	rmation			Unfactore	ed Reactio	ns UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	439	219	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	192	97	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	red Reactions		
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	375"	58% 274 / 658	932 L	1.25D+1.5L
				2-SPF 6	5.875"	23% 121 / 288	409 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1272 ft-lb	3'1 13/16"	4670 ft-lb	0.272 (27%)	1.25D+1.5L	L
Shear	911 lb	1 5/8"	1990 lb	0.458 (46%)	1.25D+1.5L	L
Perm Defl in.	0.034 (L/3418)	4'6 3/16"	0.322 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.066 (L/1747)	4'6"	0.322 (L/360)	0.210 (21%)	L	L
TL Defl inch	0.100 (L/1156)	4'6 1/16"	0.483 (L/240)	0.210 (21%)	D+L	L

Design Notes

- 1 Provide restraint at supports to ensure lateral stability.
- 2 Dead Load Deflection: Instant = 0.034", Long Term = 0.051"
- 3 See manufacture installation guide note E4 for installation details
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top flange must be laterally braced at a maximum of 7'11" o.c.

6 Bottom flange braced at bearings.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS

CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.								
	Live	Snow	Wind	Comments				
	40 PSF	0 PSF	0 PSF					
	40 PSF	0 PSF	0 PSF					
	0 DI E	0 DI E	0 DI E					



I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comment
ı	1	Tie-In	0-0-0 to 10-3-12	(Span)0-8-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Tie-In	0-0-0 to 1-9-6	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	3	Part. Uniform	0-2-6 to 9-9-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
ı	4	Part. Uniform	0-2-6 to 1-9-6		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
ı	5	Point	1-8-2		Near Face	131 lb	264 lb	0 lb	0 lb	F6
ı	6	Tie-In	1-9-6 to 10-3-12	(Span)0-7-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	7	Part. Uniform	1-9-6 to 9-9-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

Copyright 2016 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219



Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com CCMC: 12412-R APA: PR-L238C





Second Floor R1 F9-B - 1 ply –J5-l @ 1 2 X J5-H `R1 (9) DROP BM4-A F9-A - 1 ply F4-B - 2 ply ~ 듄 R1

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Second Floor VL/LSL (Flush) Pcs Length Label Description Width Depth Qty Plies F4 1.75 9.5 4 2.0E-3000Fb LVL 6-0-0 F10 Forex 1.75 2 2.0E-3000Fb LVL F9 Forex 1.75 9.5 2-0-0 2.0E-3000Fb LVL LVL/LSL (Dropped) Description Label Description Width Depth Qty Plies Pcs Length BM4 Forex 1.75 9.5 2 12-0-0 2.0E-3000Fb LVL Created Joist (Flush) Label Description Width | Depth | Qty | Plies Pcs Length 2.5 J6 LPI 20Plus 16 16-0-0 9.5 Builder LPI 20Plus 2.5 9.5 41 14-0-0 J4 LPI 20Plus 2.5 9.5 20 | 12-0-0 Sales Rep J1 LPI 20Plus 2.5 4-0-0 9.5 Rim Board Pcs Length Designer Label Description Width Depth Qty Plies RCO/SB R1 Norbord Rimboard 1.125 9.5 Plus 1.125 X 9.5 Shipping Blocking Proiect Label Description Width Depth Qty Plies Pcs Length Builder's Project BLK2 NJH 2.5 9.5 LinFt Varies 7-0-0 Hanger Beam/Girder Supported Member Pcs Description fasteners Label Skew | Slope fasteners Canada H2 21 LT259 4 10dx1 1/2 2 10dx1 1/2 I 4A 7X4 1 HGUS410 H4 46 16d 16 16d 905-642-4400 NOTES Job Path

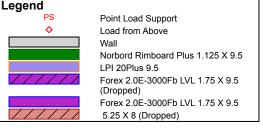
- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- . Refer to Nascor specifier guide for installation works.
- . Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- . It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior





LOT-18 (LIANA 2 EL-2) Design Method GRANELLI HOMES CORP. BRAMPTON, ONT. May 29, 2018 **GREEN YORK HOMES**

> **Kott Lumber Company** 14 Anderson Blvd Stouffville, Ontario

GRANELLI HOME CORP\MODELS \LOT 18 LIANA 2-2\FLOOR\LOT-18 (LIANA 2 EL-2).isl

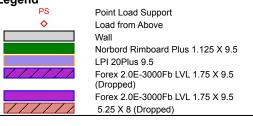
Second Floor Design Method LSD Building Code NBCC 2010 / OBC 2012

Floor Loads Live 15 Dead Deflection Joist LL Span L/ 480 360 TL Span L/ 480 LL Cant 2L/ 360 TL Cant 2L/ Deflection Girder LL Span L/ 360 TL Span L/ 240 480 LL Cant 2L/ 360 TL Cant 2L/ Decking OSB Deck

5/8"

Nailed & Glued

Gypsum 1/2"







Thickness

Fastener

Vibration

Ceiling:

Version 18.80.219 Powered by iStruct™

5. CCMC -12787-R APA PR-L310(C)

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC.

Date: Rev.4; Dec 21,2018

Project No: 17-55 Model: lot-18 (Liana 2 El-2)

2. Nascor CCMC - 13535-R

3. LVL CCMC -12904-R

4. CAN/CSA-O86-09

64 Jardin Dr., Suite 3A, Vaughan, ON

1. OBC 2012 O.Reg 332/12 as amended

PAGE 22 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

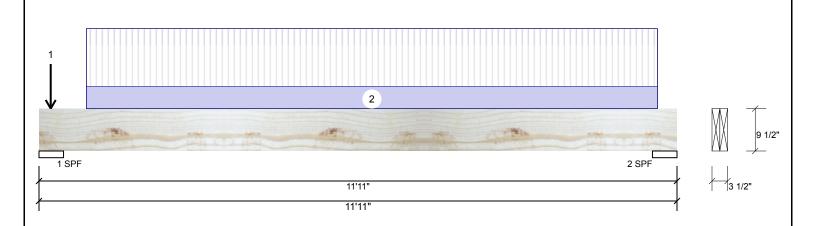
Project #

Forex 2.0E-3000Fb LVL BM4-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Info	ember Information			Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	2	Design Method:	LSD	1	1378	565	0	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1362	559	0	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings a	and Facto	red Reactions			
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
				1 - SPF 5	.500"	23% 706 / 2068	2774 L	1.25D+1.5L	
				2-SPF 5	.500"	23% 699 / 2043	2742 L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7560 ft-lb	5'11 5/8"	22724 ft-lb	0.333 (33%)	1.25D+1.5L	L
Unbraced	7560 ft-lb	5'11 5/8"	20280 ft-lb	0.373 (37%)	1.25D+1.5L	L
Shear	2345 lb	1'2 1/4"	9277 lb	0.253 (25%)	1.25D+1.5L	L
Perm Defl in.	0.074 (L/1812)	5'11 9/16"	0.371 (L/360)	0.200 (20%)	D	Uniform
LL Defl inch	0.180 (L/740)	5'11 9/16"	0.371 (L/360)	0.490 (49%)	L	L
TL Defl inch	0.254 (L/525)	5'11 9/16"	0.556 (L/240)	0.460 (46%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



, Eatoral olollar	orridad ratio bacca								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-2-10		Тор	52 lb	138 lb	0 lb	0 lb	J5
2	Part. Uniform	0-10-10 to 11-6-10		Тор	92 PLF	244 PLF	0 PLF	0 PLF	
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





NE0119-095 PAGE 23 OF 29

isDesign

Client: Project: Address:

GREEN YORK HOMES

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Level: Second Floor

Project #:

Forex 2.0E-3000Fb LVL

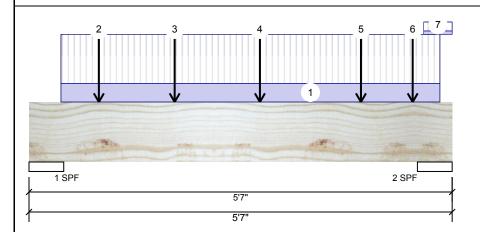
1.750" X 9.500"

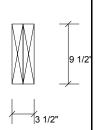
2-Ply - PASSED

Brg

1

2





Wind

O

0

0

0

wember	Intor	mation

Type:	Girder	Application:
Plies:	2	Design Method
Moisture Condition:	Dry	Building Code:
Deflection LL:	360	Load Sharing:
Deflection TL:	240	Deck:
Importance:	Normal	Vibration:
General Load		
Floor Live:	40 PSF	

15 PSF

Floor (Residential) Application: Design Method:

Load Sharing: No Deck: Not Checked Vibration:

Not Checked

NBCC 2010 / OBC 2012

Bearings and Factored Reactions

Live

1315

1964

Bearing L	ength	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 5	.500"	22%	649 / 1973	2622	L	1.25D+1.5L
2 - SPF 5	.500"	33%	1012 / 2947	3959	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3183 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L
Unbraced	3183 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L
Shear	3768 lb	4'4 3/4"	9277 lb	0.406 (41%)	1.25D+1.5L	L
Perm Defl in.	0.007 (L/7855)	2'10 1/8"	0.160 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.019 (L/3082)	2'10 1/8"	0.160 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.026 (L/2214)	2'10 1/8"	0.240 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings
- 6 Lateral slenderness ratio based on full section width.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

519

809

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



-									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-5-1 to 5-5-1		Far Face	104 PLF	276 PLF	0 PLF	0 PLF	
2	Point	0-11-1		Near Face	112 lb	292 lb	0 lb	0 lb	J5
3	Point	1-11-1		Near Face	105 lb	281 lb	0 lb	0 lb	J5
4	Point	3-0-9		Near Face	123 lb	327 lb	0 lb	0 lb	J5
5	Point	4-4-9		Near Face	100 lb	268 lb	0 lb	0 lb	J5
6	Point	5-0-12		Near Face	319 lb	713 lb	0 lb	0 lb	F4
Continued or	n page 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

Forex APA: PR-L318





PAGE 24 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

1/16/2019

Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

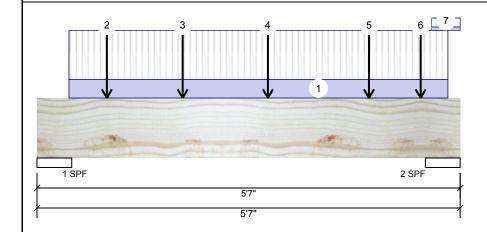
Project #:

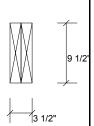
Forex 2.0E-3000Fb LVL F10-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





.Continued from page 1

7

ID Load Type

Self Weight

Tie-In

Location Trib Width

Side 5-2-8 to 5-7-0 (Span)2-6-3 Top

Dead 15 PSF

Live 40 PSF Snow 0 PSF Wind Comments

0 PSF

8 PLF

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





PAGE 25 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

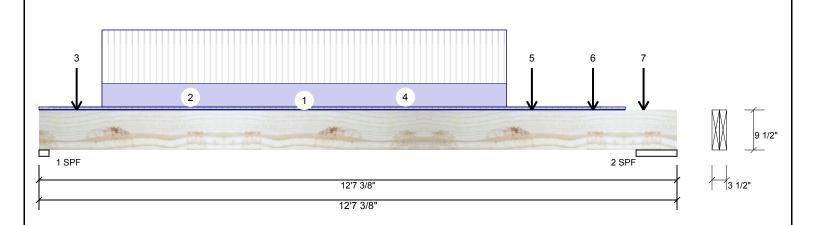
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Inforn	nation			Unfactore	d Reacti	ons UNPATTERNE	D lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	1567	722	0	0
Moisture Condition:	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1790	804	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	.375"	64% 902 / 2351	3253 L	1.25D+1.5L
				2-SPF 9	.714"	18% 1005 / 2685	3690 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9822 ft-lb	6'	22724 ft-lb	0.432 (43%)	1.25D+1.5L	L
Unbraced	9822 ft-lb	6'	20006 ft-lb	0.491 (49%)	1.25D+1.5L	L
Shear	3223 lb	11 1/8"	9277 lb	0.347 (35%)	1.25D+1.5L	L
Perm Defl in.	0.115 (L/1224)	6' 1/16"	0.391 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.251 (L/561)	6'	0.391 (L/360)	0.640 (64%)	L	L
TL Defl inch	0.366 (L/385)	6' 1/16"	0.587 (L/240)	0.620 (62%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



o Lateral oldi	acinicoo ratio bacca	on rail ocollon wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-7-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-7 to 11-1-5		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-8-15		Far Face	104 lb	238 lb	0 lb	0 lb	J5
4	Part. Uniform	1-2-15 to 9-2-15		Far Face	114 PLF	266 PLF	0 PLF	0 PLF	
5	Point	9-8-15		Far Face	128 lb	294 lb	0 lb	0 lb	J5
6	Point	10-11-7		Far Face	123 lb	292 lb	0 lb	0 lb	J5
Continued on pa	ge 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





PAGE 26 OF 29 NE0119-095

isDesign™

Client: Project: Address:

GREEN YORK HOMES

Date: 1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Page 2 of 2

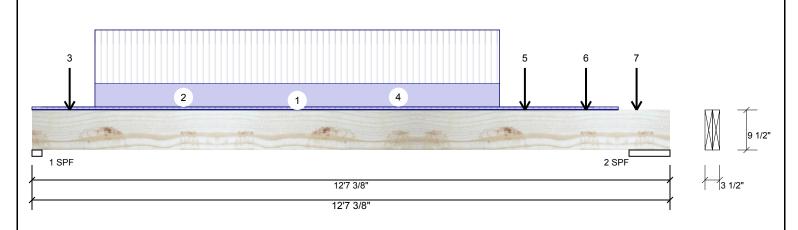
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



.Continued from page 1

ID Load Type Point 7

Self Weight

11-11-7

Location Trib Width

Side Far Face Dead 105 lb

Live 281 lb Snow 0 lb Wind Comments 0 lb

8 PLF

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

Forex APA: PR-L318





NE0119-095 PAGE 27 OF 29

isDesign™

Project: Address:

Client: **GREEN YORK HOMES** 1/16/2019

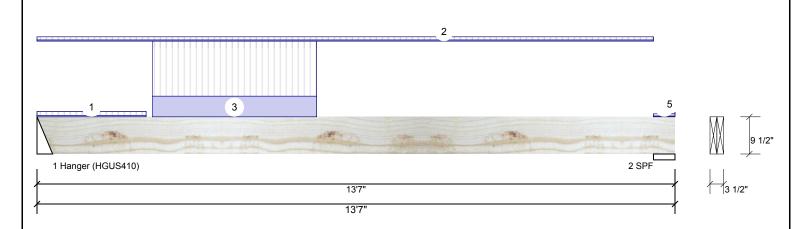
Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500" 2-Ply - PASSED Level: Second Floor



Member Infor	Member Information						Unfactored Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind				
Plies:	2	Design Method:	LSD	1	713		319	0	0				
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	356		186	0	0				
Deflection LL:	360	Load Sharing:	No										
Deflection TL:	240	Deck:	Not Checked										
Importance:	Normal	Vibration:	Not Checked										
General Load													
Floor Live:	40 PSF			Bearing	s and Fac	tored R	Reactions						
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total Ld. Cas	e Ld. Comb.				
				1 -	4.000"	14%	398 / 1070	1468 L	1.25D+1.5L				
				Hanger									
Analysis Resul	ts			2 - SPF	5.500"	6%	232 / 535	767 L	1.25D+1.5L				

ľ	,						
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	4658 ft-lb	5' 1/8"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
	Unbraced	4658 ft-lb	5' 1/8"	19429 ft-lb	0.240 (24%)	1.25D+1.5L	L
	Shear	1393 lb	1' 3/4"	9277 lb	0.150 (15%)	1.25D+1.5L	L
	Perm Defl in.	0.058 (L/2674)	6'2 5/8"	0.431 (L/360)	0.130 (13%)	D	Uniform
	LL Defl inch	0.128 (L/1212)	6'1 5/16"	0.431 (L/360)	0.300 (30%)	L	L
	TL Defl inch	0.186 (L/834)	6'1 3/4"	0.646 (L/240)	0.290 (29%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

/ Lateral Sieric	delliess ratio baseu t	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-3-14	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 13-1-8	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	2-5-7 to 5-11-7		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
4	Tie-In	13-1-8 to 13-7-0	(Span)0-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	13-2-10 to 13-7-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				8 PLF				

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





PAGE 28 OF 29 NE0119-095

isDesign™

Client: Project: Address:

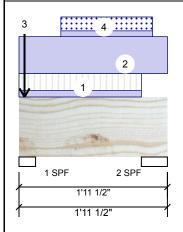
GREEN YORK HOMES

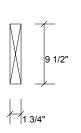
1/16/2019 Designer: RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Member Info	rmation			Unfactor	ed React	ions UNF	PATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	184		324	36	5	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	21		86	1	9	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load				-						
Floor Live:	40 PSF			Bearings	and Fac	tored Rea	actions			
Dead:	15 PSF			Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.625"	41%	406 / 639	1045	L	1.25D+1.5S +0.5L
Analysis Resu	lts			2 - SPF	4.125"	4%	121 / 0	121	Uniform	1.4D
Analysis A	otual Loo	ation Allowed Canac	ity Comb Caso				-		-	

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	35 ft-lb	11 1/16"	7385 ft-lb	0.005 (0%)	1.4D	Uniform
Unbraced	35 ft-lb	11 1/16"	7285 ft-lb	0.005 (0%)	1.4D	Uniform
Shear	8 lb	10 5/8"	4638 lb	0.002 (0%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5. 2 Girders are designed to be supported on the bottom edge only.

3 Top braced at bearings.

Design Notes

4 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

PROFESSIONAL FILE	
N.A. EL-MASRIER	
Xe mar 5 Mas	U
Jan 22, 2019	

omments
/all Self Weight
11 F11
/a

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manufacturer Info

APA: PR-L318





PAGE 29 OF 29 NE0119-095

isDesign™

Client: Project: Address: **GREEN YORK HOMES**

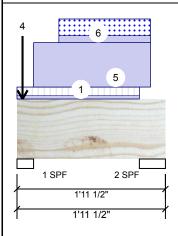
1/16/2019 Designer:

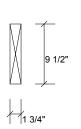
RCO/SB

Job Name: LOT-18 (LIANA 2 EL-2)

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL

Level: Second Floor





Member Info	Unfactored Reactions UNPATTERNED Ib (Uplift)									
Type:	Girder	Application:	Floor (Residential)	Brg	Live	[Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	168		317	37	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	9		69	1	9	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored Rea	actions			
Dead:	15 PSF			Bearing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.625"	41%	396 / 639	1035	L	1.25D+1.5S +0.5L
Analysis Resu	ılts			2 - SPF	4.125"	3%	97 / 0	97	Uniform	1.4D
Analysis A	Actual Loc	eation Allowed Canac	ity Comb Coso			-	-		-	

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	33 ft-lb	11 1/16"	7385 ft-lb	0.004 (0%)	1.4D	Uniform
Unbraced	33 ft-lb	11 1/16"	7285 ft-lb	0.004 (0%)	1.4D	Uniform
Shear	7 lb	10 5/8"	4638 lb	0.002 (0%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-6	(Span)0-7-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-0-14		Тор	250 lb	157 lb	360 lb	0 lb	F11 F11
3	Point	0-0-14		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	0-0-14		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight
5	Part. Uniform	0-2-10 to 1-9-2		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	0-6-10 to 1-9-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding



Manu	facturer	Info

APA: PR-L318



