NE0618-038 PAGE 1 OF 39

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

GREEN YORK HOMES-GRANELLI HOME CORP-INDIGO 1 (ELEV.1&2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

<u>CODE</u>

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.

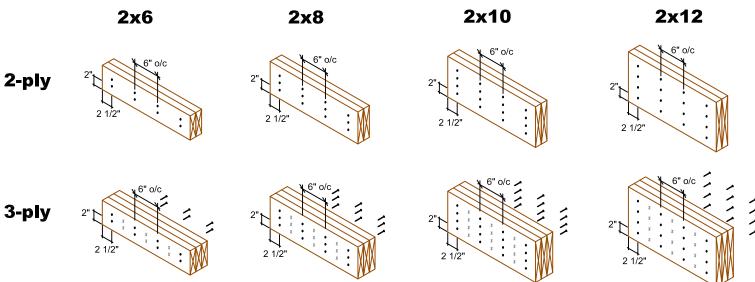


NE0618-038 **PAGE 2 OF 39**

ILTIPLE MEMBER CONNECTIONS

GREEN YORK HOMES-GRANELLI HOME CORP-INDIGO 1 (ELEV.1&2

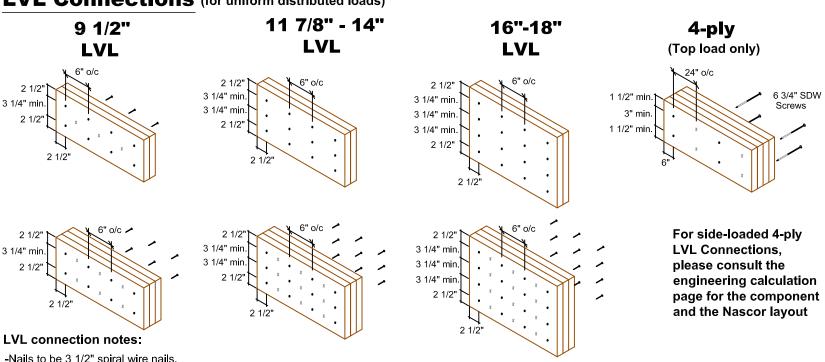
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

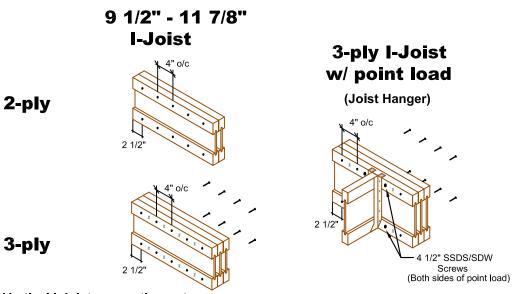
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

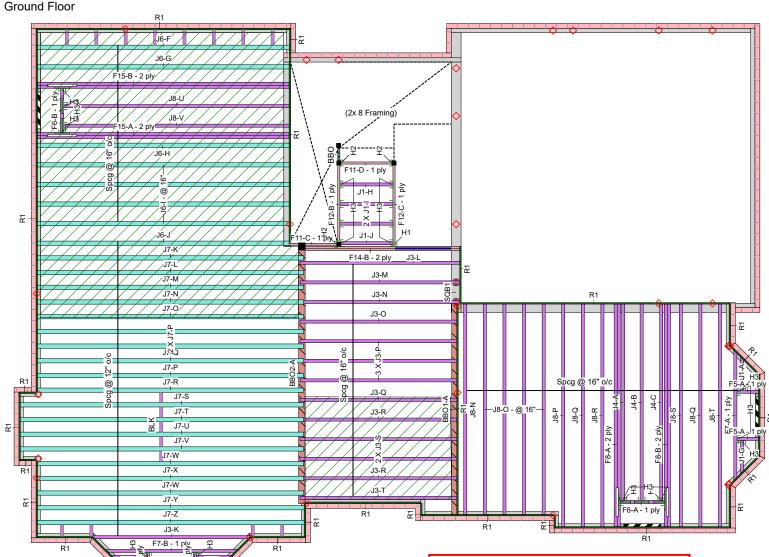
- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

> Date: November 30, 2016 Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend



Load from Above

Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

| Ground | | | | | | | | | | | | |
|---------------|----------------------|---------------------|-----|-----|------|-----|----|------|-----------|----------|-----------------|--------------------------------|
| LVL/LS | | | | 1 | | | _ | | | | I | NAS |
| Label | Descri | iption | Wic | _ | De | _ | (| Qty | Plies | Pcs | Length | IN/AS |
| F14 | Forex 2.0E-3 | 000Fb LVL | 1. | 75 | 11.8 | 375 | | 1 | 2 | 2 | 12-0-0 | |
| F12 | Forex 2.0E-3 | 000Fb LVL | 1. | 75 | 11.8 | 375 | | | | 2 | 6-0-0 | Layout Name INDIGO 1 (ELE |
| F11 | Forex 2.0E-3 | 000Fb LVL | 1. | 75 | 11.8 | 375 | | | | 2 | 4-0-0 | Design Metho |
| I Joist (| Flush) | | | | | | | | | | | |
| Label | Descri | iption | Wic | lth | De | pth | (| Qty | Plies | Pcs | Length | Description |
| J7 | NJ60U | | 3 | 3.5 | 11.8 | 375 | | | | 19 | 20-0-0 | GRANELLI HO BRAMPTON, O |
| J6 | NJ60U | | 3 | 3.5 | 11.8 | 375 | | | | 8 | 18-0-0 | |
| F15 | NJH | | 2 | 2.5 | 11.8 | 375 | | 2 | 2 | 4 | 18-0-0 | Created |
| F8 | NJH | | 2 | 2.5 | 11.8 | 375 | | 2 | 2 | 4 | 16-0-0 | May 30, 2018 |
| F7 | NJH | | (2 | 2.5 | 11.8 | 375 | | | | 2 | 10-0-0 | Builder |
| F6 | NJH | | 2 | 2.5 | 11.8 | 375 | | | | 2 | 4-0-0 | GREEN YORK |
| F5 | NJH | | | 2.5 | 11.8 | 375 | | | | 4 | 2-0-0 | Sales Rep |
| J8 | NJH | | | 2.5 | 11.8 | 375 | | | | 13 | 16-0-0 | RM |
| J4 | NJH | | | 2.5 | 11.8 | | | | | 3 | 14-0-0 | |
| J3 | NJH | | | 2.5 | 11.8 | | | | | 14 | 12-0-0 | Designer |
| J1 | NJH | | 2 | 2.5 | 11.8 | 375 | | | | 8 | 4-0-0 | RCO |
| Rim Bo | | | | | | | | | | | | Shipping |
| Label | Descri | | Wic | - | De | | | Qty | Plies | Pcs | Length | Project |
| R1 | Norbor Plus 1. | d Rimboard 125 X | 1.1 | 25 | 11.8 | 375 | | | | 15 | 12 | Builder's Proje |
| | 11.875 | | | | | | | | | | | Kott Lumb |
| Blockin | ıg | | | | | | | | | | | 14 Anderson Bl |
| Label | Descri | iption | Wic | lth | De | pth | | Qty | Plies | Pcs | Length | Stouffville, Onta |
| BLK1 | NJH | | 2 | 2.5 | 11.8 | 375 | Li | inFt | | Varies | 16-0-0 | Canada |
| Hangei | r | | | | | | | | | | | L4A 7X4 |
| | | | | | | | | Bea | am/Girde | | ported ember | 905-642-4400 |
| Label | Label Pcs Descriptio | | n | Sk | œw | Slo | ne | fs | steners | | teners | Job Path |
| H1 | 1 | HUS1.81/1 | | | | 3.0 | 77 | | 30 16d | | 0 16d | D:\Users\rochav |
| H2 | 3 | LS90 | - | | | | | | 04 | <u> </u> | | HOME\GREEN |
| H3 | 25 | LT251188 | | | | | | 4 | 10dx1 1/2 | 2 10 | dx1 1/2 | \GRANELLI HC \INDIGO 1\INDI |
| $\overline{}$ | | | | | | | | | | | | 1 (1140190 1/11401 |

NOTES:

- Framer to verify dimensions on the architectural drawings. . Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- . Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads
- It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

| | Floor L (Flush) | | | | | | | NIACC |
|------|----------------------------------|-------|--------|-------|-------|--------|--------|--------------------------------------|
| bel | Description | Width | Depth | Qty | Plies | Pcs | Length | NASC |
| 14 | Forex 2.0E-3000Fb LVL | 1.75 | 11.875 | 1 | 2 | 2 | 12-0-0 | |
| 12 | Forex 2.0E-3000Fb LVL | 1.75 | 11.875 | | | 2 | 6-0-0 | Layout Name INDIGO 1 (ELEV.1) |
| 11 | Forex 2.0E-3000Fb LVL | 1.75 | 11.875 | | | 2 | 4-0-0 | Design Method |
| st (| Flush) | | | | | | | |
| bel | Description | Width | Depth | Qty | Plies | Pcs | Length | Description |
| 7 | NJ60U | 3.5 | 11.875 | | | 19 | 20-0-0 | GRANELLI HOME CORF BRAMPTON, ONT. |
| 6 | NJ60U | 3.5 | 11.875 | | | 8 | 18-0-0 | <u>'</u> |
| 15 | NJH | 2.5 | 11.875 | 2 | 2 | 4 | 18-0-0 | Created |
| 8 | NJH | 2.5 | 11.875 | 2 | 2 | 4 | 16-0-0 | May 30, 2018 |
| 7 | NJH | 2.5 | 11.875 | | | 2 | 10-0-0 | Builder |
| 6 | NJH | 2.5 | 11.875 | | | 2 | 4-0-0 | GREEN YORK HOMES |
| 5 | NJH | 2.5 | 11.875 | | | 4 | 2-0-0 | Sales Rep |
| 8 | NJH | 2.5 | 11.875 | | | 13 | 16-0-0 | RM |
| 4 | NJH | 2.5 | 11.875 | | | 3 | 14-0-0 | |
| 3 | NJH | 2.5 | 11.875 | | | 14 | 12-0-0 | Designer |
| 1 | NJH | 2.5 | 11.875 | | | 8 | 4-0-0 | RCO |
| Во | ard | | | | | | | Shipping |
| bel | Description | Width | Depth | Qty | Plies | Pcs | Length | Project |
| .1 | Norbord Rimboard Plus 1.125 X | 1.125 | 11.875 | | | 15 | 12 | Builder's Project |
| | 11.875 | | | | | | | Kott Lumber Com |
| kin | g | | | | | | | 14 Anderson Blvd |
| bel | Description | Width | Depth | Qty | Plies | Pcs | Length | Stouffville, Ontario |
| K1 | NJH | 2.5 | 11.875 | LinFt | | Varies | 16-0-0 | Canada |
| ger | | | | | | | | L4A 7X4 |

b Path

·\LIsers\rochavillo\WORK FROM HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS \INDIGO 1\INDIGO 1 ELEV.1\FLOOR

| | (IINDIGO I (ELE | . v. i).isi |
|--|--------------------|-------------------------|
| | Ground Floo | r |
| | Design Method | LSD |
| | Building Code | NBCC 2010 / OBC 2012 |
| | Floor | |
| | Loads | |
| | Live | 40 |
| | | |

| Loads | |
|-------------------|-----|
| Live | 40 |
| Dead | 15 |
| Deflection Joist | |
| LL Span L/ | 480 |
| TL Span L/ | 360 |
| LL Cant 2L/ | 480 |
| TL Cant 2L/ | 360 |
| Deflection Girder | |
| LL Span L/ | 360 |
| TL Span L/ | 240 |
| LL Cant 2L/ | 480 |
| TL Cant 2L/ | 360 |
| | |

Thickness Fastener Nailed & Glued Vibration

OSB

3/4"

Decking

Deck

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EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

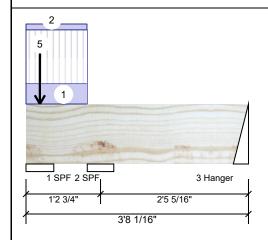
6/1/2018 Designer: RCO

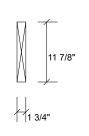
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F11-C Forex 2.0E-3000Fb LVL

Level: Ground Floor





| Member Inform | nation | | |
|---------------------|--------|----------------|----------------------|
| Туре: | Girder | Application: | Floor (Residential) |
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|-------|------|------|------|
| 1 | 104 | 57 | 0 | 0 |
| 2 | 4 | 11 | 0 | 0 |
| 3 | 0 (0) | 6 | 0 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|------|--------------|-------|----------|------------|
| 1 - SPF | 5.500" | 5% | 69 / 156 | 225 | L_ | 1.25D+1.5L |
| 2 - SPF | 5.250" | 1% | 19 / 0 | 19 | Uniform | 1.4D |
| 3 - Hanger | 3.000" | 0% | 7/0 | 7 | Uniform | 1.4D |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Analysis Results

Dead:

15 PSF

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|---------------|-----------|---------------|------------|-------|---------|
| Neg Moment | -3 ft-lb | 1'2 3/4" | 11135 ft-lb | 0.000 (0%) | 1.4D | Uniform |
| Unbraced | -3 ft-lb | 1'2 3/4" | 11135 ft-lb | 0.000 (0%) | 1.4D | Uniform |
| Pos Moment | 3 ft-lb | 2'7 1/16" | 11135 ft-lb | 0.000 (0%) | 1.4D | Uniform |
| Unbraced | 3 ft-lb | 2'7 1/16" | 10807 ft-lb | 0.000 (0%) | 1.4D | Uniform |
| Shear | 11 lb | 1'4 5/8" | 3769 lb | 0.003 (0%) | 1.4D | Uniform |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings

| o bottom bre | acca at bearings. | | | | | | | | | |
|--------------|-------------------|----------------|--------------|------|--------|--------|-------|-------|------------------|--|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | |
| 1 | Tie-In | 0-0-0 to 1-0-2 | (Span)0-11-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 2 | Part. Uniform | 0-0-0 to 1-0-1 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | | |
| 3 | Point | 0-2-12 | | Тор | 21 lb | 55 lb | 0 lb | 0 lb | J6 | |
| 4 | Point | 0-2-12 | | Тор | 13 lb | 35 lb | 0 lb | 0 lb | J3 | |
| 5 | Point | 0-2-12 | | Тор | 13 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight | |
| | Self Weight | | | | 5 PLF | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

GREEN YORK HOMES

Designer:

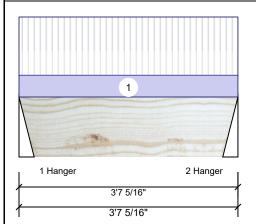
RCO Job Name: INDIGO 1 (ELEV.1)

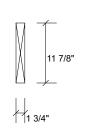
Project #:

1.750" X 11.875" - PASSED F11-D Forex 2.0E-3000Fb LVL

Level: Ground Floor

6/1/2018





Member Information Type: Plies: Moisture Condition: Dry Deflection LL:

360 Deflection TL: 240 Importance: Normal

General Load Floor Live: 40 PSF 15 PSF Dead:

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Not Checked Deck: Vibration:

Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Live | Dead | Snow | Wind |
|------|------|-------|---------|
| 53 | 28 | 0 | 0 |
| 53 | 28 | 0 | 0 |
| | | 53 28 | 53 28 0 |

Bearings and Factored Reactions

Hanger

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|---------|--------|---------|------------|-------|----------|------------|
| 1 - | 3.000" | 3% | 35 / 79 | 114 | L | 1.25D+1.5L |
| Hanger | | | | | | |
| 2 - | 3.000" | 3% | 35 / 79 | 114 | L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|---------------|------------|---------------|------------|------------|------|
| Moment | 83 ft-lb | 1'9 11/16" | 17130 ft-lb | 0.005 (0%) | 1.25D+1.5L | L |
| Unbraced | 83 ft-lb | 1'9 11/16" | 13233 ft-lb | 0.006 (1%) | 1.25D+1.5L | L |
| Shear | 40 lb | 1'2 1/8" | 5798 lb | 0.007 (1%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-------------|----------------|-------------|------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 3-7-5 | (Span)1-5-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | Self Weight | | | | 5 PLF | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318









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Client: **GREEN YORK HOMES**

Project: Address:

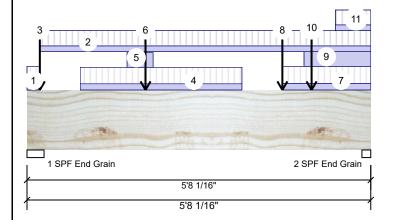
6/1/2018 Designer: RCO

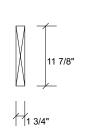
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F12-B Forex 2.0E-3000Fb LVL

Level: Ground Floor





Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |
| | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 798 | 349 | 0 | 0 |
| 2 | 933 | 449 | 0 | 0 |
| | | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 2299 ft-lb | 2'4 7/16" | 17130 ft-lb | 0.134 (13%) | 1.25D+1.5L | L |
| Unbraced | 2299 ft-lb | 2'4 7/16" | 8377 ft-lb | 0.274 (27%) | 1.25D+1.5L | L |
| Shear | 1466 lb | 4'7 3/16" | 5798 lb | 0.253 (25%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.008 (L/7964) | 2'9 5/8" | 0.179 (L/360) | 0.050 (5%) | D | Uniform |
| LL Defl inch | 0.018 (L/3518) | 2'9 5/8" | 0.179 (L/360) | 0.100 (10%) | L | L |
| TL Defl inch | 0.026 (L/2440) | 2'9 5/8" | 0.268 (L/240) | 0.100 (10%) | D+L | L |

Bearings and Factored Reactions

| Bearing | Length | Cap. R | eact D/L lb | Total | Ld. Case | Ld. Comb. |
|-------------------------|--------|--------|-------------|-------|----------|------------|
| 1 - SPF End Grain | 3.375" | 37% | 436 / 1197 | 1633 | L | 1.25D+1.5L |
| 2 - SPF End Grain | 1.750" | 86% | 562 / 1399 | 1961 | L | 1.25D+1.5L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

| CONNECTION DETAIL FOR PLY TO PLY | | | | | | | | |
|----------------------------------|---------|----------|---|-----|----|--|--|--|
| NAILING OR BO | | | | ENT | S. | | | |
| PASS THRU FR | AMING | SQUASH | 1 | | | | | |
| BLOCK IS REQ | UIRED / | AT ALL | ı | | | | | |
| POINT LOADS | OVER B | EARINGS. | | | | | | |
| | | | _ | _ | | | | |

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | | |
|-------------------|---------------------|------------------|-------------|-----------|--------|--------|-------|-------|------------------|--|--|
| 1 | Tie-In | 0-0-0 to 0-2-9 | (Span)3-9-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | | |
| 2 | Tie-In | 0-2-8 to 5-8-1 | (Span)3-4-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | | |
| 3 | Point | 0-2-9 | | Near Face | 23 lb | 61 lb | 0 lb | 0 lb | J1 | | |
| 4 | Part. Uniform | 0-10-9 to 3-6-9 | | Near Face | 27 PLF | 72 PLF | 0 PLF | 0 PLF | | | |
| 5 | Part. Uniform | 1-7-13 to 2-0-15 | | Тор | 64 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight | | |
| 6 | Point | 1-11-7 | | Тор | 188 lb | 468 lb | 0 lb | 0 lb | F11 F11 | | |
| 7 | Tie-In | 4-2-9 to 5-8-1 | (Span)3-9-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | | |
| Continued on page | Continued on page 2 | | | | | | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





NE0618-038 **PAGE 7 OF 39**

EWP Studio Simpson Strong-Tie® Component Solutions™

Continued from page 1

Self Weight

Client: **GREEN YORK HOMES**

Project: Designer:

RCO Job Name: INDIGO 1 (ELEV.1)

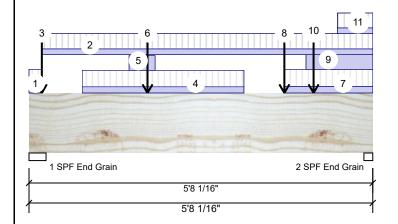
Project #:

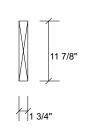
1.750" X 11.875" - PASSED F12-B Forex 2.0E-3000Fb LVL

Address:

Level: Ground Floor

6/1/2018





| ı | Continued from p | onlined from page 1 | | | | | | | | | | |
|---|------------------|---------------------|-----------------|------------|-----------|--------|--------|-------|-------|------------------|--|--|
| | ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | | |
| | 8 | Point | 4-2-9 | | Near Face | 38 lb | 101 lb | 0 lb | 0 lb | J1 | | |
| | 9 | Part. Uniform | 4-6-13 to 5-8-1 | | Тор | 64 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight | | |
| | 10 | Point | 4-8-5 | | Тор | 154 lb | 381 lb | 0 lb | 0 lb | F11 F11 | | |
| | 11 | Part. Uniform | 5-1-1 to 5-8-1 | | Тор | 25 PLF | 65 PLF | 0 PLF | 0 PLF | J1 | | |

5 PLF

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

2 Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info





PAGE 8 OF 39 NE0618-038

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

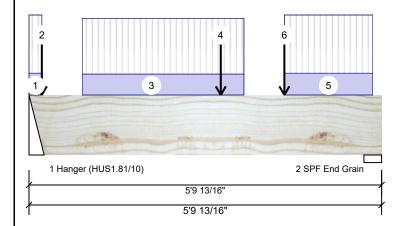
6/1/2018 Designer: RCO

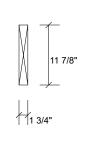
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F12-C Forex 2.0E-3000Fb LVL

Level: Ground Floor





\\/ind

Page 1 of 1

Member Information

| Туре: | Girder | Application: | Floor (Residential) |
|--------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition | : Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| ыу | LIVE | Dead | SHOW | VVIIIU |
|----|------|------|------|--------|
| 1 | 537 | 221 | 0 | 0 |
| 2 | 613 | 251 | 0 | 0 |
| | | | | |

Dead

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-------------|------------|---------|
| Moment | 2367 ft-lb | 3'1 15/16" | 17130 ft-lb | 0.138 (14%) | 1.25D+1.5L | L |
| Unbraced | 2367 ft-lb | 3'1 15/16" | 8328 ft-lb | 0.284 (28%) | 1.25D+1.5L | L |
| Shear | 1110 lb | 4'7 3/16" | 5798 lb | 0.191 (19%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.007 (L/9287) | 3'1 15/16" | 0.180 (L/360) | 0.040 (4%) | D | Uniform |
| LL Defl inch | 0.017 (L/3759) | 3'1 15/16" | 0.180 (L/360) | 0.100 (10%) | L | L |
| TL Defl inch | 0.024 (L/2676) | 3'1 15/16" | 0.270 (L/240) | 0.090 (9%) | D+L | L |

Bearings and Factored Reactions

Livo

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|-------------------------|--------|---------|------------|-------|----------|------------|
| 1 - | 3.000" | 28% | 276 / 805 | 1081 | L | 1.25D+1.5L |
| Hanger | | | | | | |
| 2 - SPF End Grain | 3.500" | 27% | 314 / 920 | 1234 | L | 1.25D+1.5L |

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|-------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 0-2-9 | (Span)3-9-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Point | 0-2-9 | | Far Face | 23 lb | 61 lb | 0 lb | 0 lb | J1 |
| 3 | Part. Uniform | 0-10-9 to 3-6-9 | | Far Face | 27 PLF | 72 PLF | 0 PLF | 0 PLF | |
| 4 | Point | 3-1-15 | | Тор | 264 lb | 671 lb | 0 lb | 0 lb | C4 |
| 5 | Tie-In | 4-2-9 to 5-8-1 | (Span)3-9-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 6 | Point | 4-2-9 | | Far Face | 38 lb | 101 lb | 0 lb | 0 lb | J1 |
| | Self Weight | | | | 5 PLF | | | | |
| | | | | | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





PAGE 9 OF 39 NE0618-038

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES** Project:

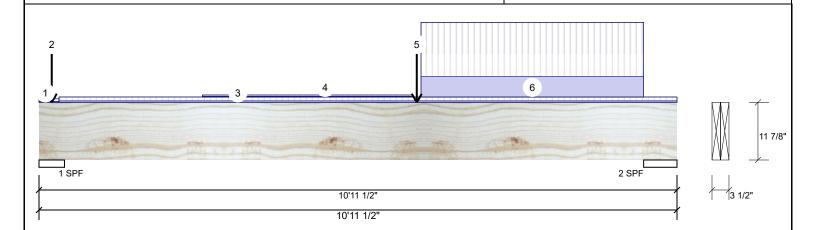
6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #

2-Ply - PASSED Level: Ground Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F14-B

Address:



| Member Info | rmation | | | Unfactore | d Reacti | ons UNPATTERI | NED lb (Uplift) | |
|--------------------|---------|----------------|----------------------|------------|-----------|-------------------|-----------------|------------|
| Туре: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind |
| Plies: | 2 | Design Method: | LSD | 1 | 532 | 295 | 0 | 0 |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 1275 | 542 | 0 | 0 |
| Deflection LL: | 360 | Load Sharing: | No | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | |
| General Load | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | |
| Dead: | 15 PSF | | | Bearing L | .ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |
| | | | | 1 - SPF 5 | .250" | 10% 369 / 798 | 1167 L | 1.25D+1.5L |
| | | | | 2-SPF 6 | 3.875" | 17% 678 / 1913 | 2590 L | 1.25D+1.5L |
| l!- D | | | | | | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 5728 ft-lb | 6'5 7/8" | 34261 ft-lb | 0.167 (17%) | 1.25D+1.5L | L |
| Unbraced | 5728 ft-lb | 6'5 7/8" | 29540 ft-lb | 0.194 (19%) | 1.25D+1.5L | L |
| Shear | 2022 lb | 9'5 1/2" | 11596 lb | 0.174 (17%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.023 (L/5246) | 5'11 1/2" | 0.336 (L/360) | 0.070 (7%) | D | Uniform |
| LL Defl inch | 0.053 (L/2301) | 6' 3/8" | 0.336 (L/360) | 0.160 (16%) | L | L |
| TL Defl inch | 0.076 (L/1600) | 6' 1/16" | 0.504 (L/240) | 0.150 (15%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

ID Load Type Location Trib Width Side Dead Live Snow Wind Comments 0-0-0 to 0-4-2 15 PSF 40 PSF 0 PSF 0 PSF Tie-In (Span)0-7-11 Top 0 lb 2 Point 0-2-10 37 lb 0 lb 0 lb Wall Self Weight Top (Span)0-11-11 Top 40 PSF 0 PSF 0-4-2 to 10-11-8 **15 PSF** 0 PSF 3 Tie-In to 0-11-11 40 PSF 0 PSF Tie-In 2-9-11 to 6-5-0 (Span)0-4-5 Top 15 PSF 0 PSF to 0-4-5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info





NE0618-038 **PAGE 10 OF 39**

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

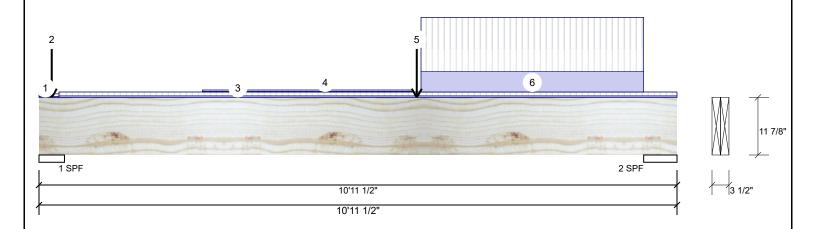
Address:

Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED Level: Ground Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F14-B



.Continued from page 1

Location Trib Width Side Wind Comments ID Load Type Dead Live Snow Far Face 221 lb 537 lb 0 lb 0 lb 5 Point 6-5-14 6 Part. Uniform 6-6-12 to 10-4-10 Тор 101 PLF 270 PLF 0 PLF 0 PLF Self Weight 10 PLF

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-038 **PAGE 11 OF 39**

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

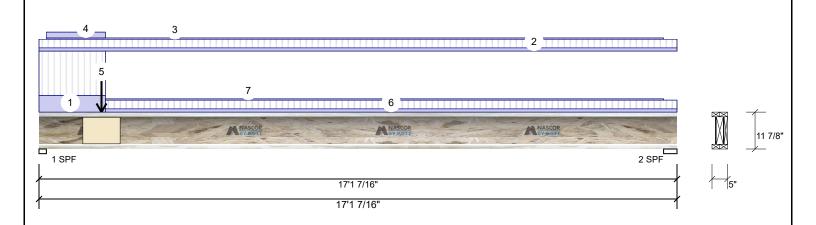
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

11.875" 2-Ply - PASSED F15-A NJH

Level: Ground Floor



| Member Info | Member Information | | | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | |
|--------------------|--------------------|----------------|----------------------|------------|-----------|--|----------------|------------|--|--|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | | |
| Plies: | 2 | Design Method: | LSD | 1 | 755 | 381 | 0 | 0 | | | |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 279 | 144 | 0 | 0 | | | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | | |
| General Load | | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | | | | |
| Dead: | 15 PSF | | | Bearing L | ength. | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | | |
| | | | | 1 - SPF 2 | .375" | 49% 477 / 1133 | 1609 L | 1.25D+1.5L | | | |
| | | | | 2-SPF 4 | .319" | 17% 180 / 419 | 599 L | 1.25D+1.5L | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-------------|---------------|-----------------|------------|---------|
| Moment | 2960 ft-lb | 6'8 1/8" | 10780 ft-lb | 0.275 (27%) | 1.25D+1.5L | L |
| Unbraced | 2960 ft-lb | 6'8 1/8" | 2970 ft-lb | 0.997 (100%) | 1.25D+1.5L | L |
| Shear | 1588 lb | 1 5/8" | 3620 lb | 0.439 (44%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.065 (L/3060) | 8' 1/16" | 0.556 (L/360) | 0.120 (12%) | D | Uniform |
| LL Defl inch | 0.127 (L/1581) | 7'11 15/16" | 0.556 (L/360) | 0.230 (23%) | L | L |
| TL Defl inch | 0.192 (L/1042) | 8' | 0.834 (L/240) | 0.230 (23%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|--------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-9-6 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 17-1-7 | (Span)0-7-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-2-5 to 16-9-2 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-2-5 to 1-9-6 | | Тор | 9 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-8-2 | | Far Face | 240 lb | 480 lb | 0 lb | 0 lb | F6 |
| 6 | Tie-In | 1-9-6 to 17-1-7 | (Span)0-8-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-9-6 to 16-9-1 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthp=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-038 PAGE 12 OF 39

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

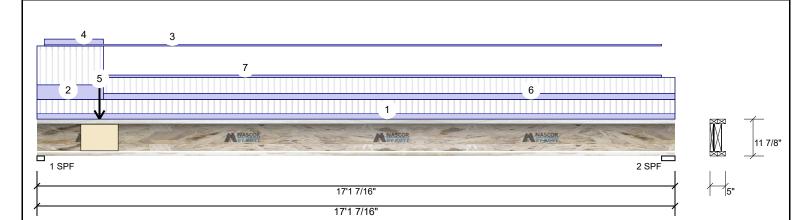
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

2-Ply - PASSED F15-B 11.875" NJH



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Wind Type: Application: Floor (Residential) Brg Live Dead Plies: 2 Design Method: 832 417 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 496 248 0 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 521 / 1248 53% 1769 I 1.25D+1.5L 2 - SPF 4.319" 29% 310 / 744 1054 L 1.25D+1.5L

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-----------------|------------|---------|
| Moment | 4604 ft-lb | 7'9 15/16" | 10780 ft-lb | 0.427 (43%) | 1.25D+1.5L | L |
| Unbraced | 4604 ft-lb | 7'9 15/16" | 4611 ft-lb | 0.998 (100%) | 1.25D+1.5L | L |
| Shear | 1744 lb | 1 5/8" | 3620 lb | 0.482 (48%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.100 (L/2004) | 8'3 3/16" | 0.556 (L/360) | 0.180 (18%) | D | Uniform |
| LL Defl inch | 0.198 (L/1013) | 8'3 3/16" | 0.556 (L/360) | 0.360 (36%) | L | L |
| TL Defl inch | 0.298 (L/673) | 8'3 3/16" | 0.834 (L/240) | 0.360 (36%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'2" o.c.

5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 17-1-7 | (Span)1-3-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-9-6 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-2-6 to 16-9-1 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-2-6 to 1-9-6 | | Тор | 9 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-8-2 | | Near Face | 172 lb | 343 lb | 0 lb | 0 lb | F6 |
| 6 | Tie-In | 1-9-6 to 17-1-7 | (Span)1-4-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-9-6 to 16-9-1 | | Тор | 4 PLF | 0 PLF | 0 PLF | 0 PLF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 Libel flagges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, suffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







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Brg

1

2



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

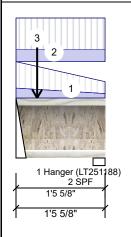
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

NJH 11.875" - PASSED F5-A



11 7/8"

O

0

Wind

0

0

| Member | Information |
|--------|-------------|
|--------|-------------|

| Type: | Girder | Ap |
|---------------------|--------|-----|
| Plies: | 1 | De |
| Moisture Condition: | Dry | Bu |
| Deflection LL: | 360 | Lo |
| Deflection TL: | 240 | De |
| Importance: | Normal | Vil |
| General Load | | |
| Floor Live: | 40 PSF | |

Application: Floor (Residential) esign Method: uilding Code: NBCC 2010 / OBC 2012

oad Sharing: No eck: Not Checked

ibration: Not Checked

15 PSF

Bearings and Factored Reactions

Live

104

79

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

39

30

| Bearing Length | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | |
|----------------|-------------------|----------------|------------|--|
| 1 - 2.000" | 13% 49 / 156 | 204 L | 1.25D+1.5L | |
| Hanger | | | | |
| 2 - SPF 2.375" | 9% 37 / 119 | 156 L | 1.25D+1.5L | |

Analysis Results

Dead:

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|-------------|------------|------|
| Moment | 47 ft-lb | 7 3/4" | 5390 ft-lb | 0.009 (1%) | 1.25D+1.5L | L |
| Unbraced | 47 ft-lb | 7 3/4" | 5334 ft-lb | 0.009 (1%) | 1.25D+1.5L | L |
| Shear | 180 lb | 1 1/4" | 1810 lb | 0.100 (10%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| LL Defl inch | 0.000 (L/34775) | 7 13/16" | 0.041 (L/360) | 0.010 (1%) | L | L |
| TL Defl inch | 0.001 | 7 13/16" | 0.061 (L/240) | 0.010 (1%) | D+L | L |

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|-----------------|-------------------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-5-10 | (Span)2-8-1 to 1-2-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-5-10 | (Span)3-2-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 0-4-3 | | Far Face | 12 lb | 32 lb | 0 lb | 0 lb | J1 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- IARIGHING & INSEGUATION

 Lodist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, suffiener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid

alteral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







PAGE 14 OF 39 NE0618-038



EWP Studio Simpson Strong-Tie®

Client: **GREEN YORK HOMES**

Project: Address:

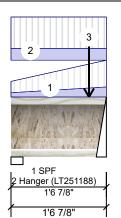
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

11.875" - PASSED F5-B NJH



11 7/8"

0

0

Wind

0

0

Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead

31

43

83

114

1

2

| L | | | | | | | | |
|---|---------------------------------|--------|------|--------------|-------|----------|------------|--|
| | Bearings and Factored Reactions | | | | | | | |
| I | Bearing | Length | Сар. | React D/L lb | Total | Ld. Case | Ld. Comb. | |
| l | 1 - SPF | 2.375" | 10% | 39 / 124 | 163 | L | 1.25D+1.5L | |
| 1 | 2 - Hanger | 2.000" | 14% | 53 / 172 | 225 | L | 1.25D+1.5L | |

Analysis Results

Dead:

15 PSF

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|-------------|------------|------|
| Moment | 52 ft-lb | 10 3/8" | 5390 ft-lb | 0.010 (1%) | 1.25D+1.5L | L |
| Unbraced | 52 ft-lb | 10 3/8" | 5319 ft-lb | 0.010 (1%) | 1.25D+1.5L | L |
| Shear | 201 lb | 1'5 5/8" | 1810 lb | 0.111 (11%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| LL Defl inch | 0.000 (L/33591) | 10 5/16" | 0.044 (L/360) | 0.010 (1%) | L | L |
| TL Defl inch | 0.001 (L/24438) | 10 5/16" | 0.067 (L/240) | 0.010 (1%) | D+L | L |

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|-----------------|---------------------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-6-14 | (Span)1-1-15 to 2-8-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-6-14 | (Span)3-2-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-3-7 | | Far Face | 13 lb | 35 lb | 0 lb | 0 lb | J1 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum
 point load bearing length ≥ 3.5 inches
 7. For flat roofs provide proper drainage to prevent
 ponding

Manufacturer Info

Nascor by Kott







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2

Hanger



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

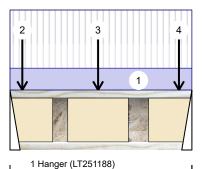
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

11.875" - PASSED **NJH** F6-A



11 7/8"

Wind

0

0

0

| | 2 Hanger (LT251188) |
|--|---------------------|
| I 1 | 3' |
| │ | 31 |
| ' | 3 |

15 PSF

Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead 441 166 0

165

439

| ı | | | | | | | | | |
|---|---------------------------------|--------|--------|-------------|-------|----------|------------|--|--|
| | Bearings and Factored Reactions | | | | | | | | |
| | Bearing | Length | Cap. R | eact D/L lb | Total | Ld. Case | Ld. Comb. | | |
| | 1 - Hanger | 2.000" | 54% | 207 / 662 | 869 | L | 1.25D+1.5L | | |
| | 2 - | 2.000" | 53% | 206 / 659 | 865 | L | 1.25D+1.5L | | |

Analysis Results

Dead:

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|-----------|---------------|-------------|------------|---------|
| Moment | 571 ft-lb | 1'5 7/16" | 5390 ft-lb | 0.106 (11%) | 1.25D+1.5L | L |
| Unbraced | 571 ft-lb | 1'5 7/16" | 4800 ft-lb | 0.119 (12%) | 1.25D+1.5L | L |
| Shear | 862 lb | 1 1/4" | 1810 lb | 0.476 (48%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.002 (L/14698) | 1'5 7/16" | 0.093 (L/360) | 0.020 (2%) | D | Uniform |
| LL Defl inch | 0.006 (L/5525) | 1'5 7/16" | 0.093 (L/360) | 0.070 (7%) | L | L |
| TI Deflinch | 0.008 (L/4016) | 1'5 7/16" | 0.140 (L/240) | 0.060 (6%) | D+L | L |

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

| + Dottom nange | bracea at bearings. | | | | | P. | | | |
|----------------|---------------------|----------------|--------------|----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 3-0-0 | (Span)1-8-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Point | 0-2-7 | | Far Face | 81 lb | 215 lb | 0 lb | 0 lb | J4 |
| 3 | Point | 1-5-7 | | Far Face | 126 lb | 335 lb | 0 lb | 0 lb | J4 |
| 4 | Point | 2-9-7 | | Far Face | 85 lb | 227 lb | 0 lb | 0 lb | J4 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- IARIGHING & INSEGUATION

 Lodist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, suffiener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum
 point load bearing length ≥ 3.5 inches
 7. For flat roofs provide proper drainage to prevent
 ponding

Manufacturer Info

Nascor by Kott







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EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

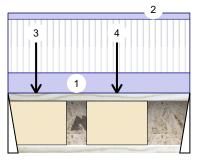
Date: 6/1/2018 Designer: RCO

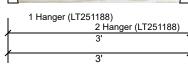
Job Name: INDIGO 1 (ELEV.1)

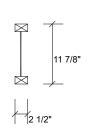
Level: Ground Floor

Project #:

11.875" - PASSED F6-B NJH







| wember | intormation |
|--------|-------------|
| Type: | Girder |

| Type: | Girder |
|---------------------|--------|
| Plies: | 1 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 No

Load Sharing: Not Checked Deck:

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 480 | 240 | 0 | 0 |
| 2 | 343 | 172 | 0 | 0 |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 749 ft-lb | 1'9 9/16" | 5390 ft-lb | 0.139 (14%) | 1.25D+1.5L | L |
| Unbraced | 749 ft-lb | 1'9 9/16" | 4800 ft-lb | 0.156 (16%) | 1.25D+1.5L | L |
| Shear | 1013 lb | 1 1/4" | 1810 lb | 0.560 (56%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.004 (L/9092) | 1'9 9/16" | 0.093 (L/360) | 0.040 (4%) | D | Uniform |
| LL Defl inch | 0.007 (L/4555) | 1'9 9/16" | 0.093 (L/360) | 0.080 (8%) | L | L |
| TL Defl inch | 0.011 (L/3035) | 1'9 9/16" | 0.140 (L/240) | 0.080 (8%) | D+L | L |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. | |
|---------------|--------|---------|------------|-------|----------|------------|--|
| 1 - Hanger | 2.000" | 63% | 300 / 721 | 1021 | L | 1.25D+1.5L | |
| 2 - Hanger | 2.000" | 45% | 215 / 515 | 729 | L | 1.25D+1.5L | |

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|----------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 3-0-0 | (Span)1-8-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 3-0-0 | | Тор | 4 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 3 | Point | 0-5-9 | | Near Face | 152 lb | 304 lb | 0 lb | 0 lb | J8 |
| 4 | Point | 1-9-9 | | Near Face | 209 lb | 416 lb | 0 lb | 0 lb | J8 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Handling & Installation

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott





NE0618-038 PAGE 17 OF 39



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

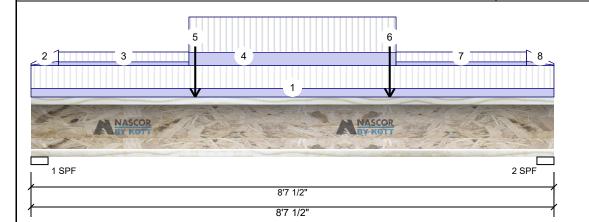
Date: 6/1/2018 Designer: RCO

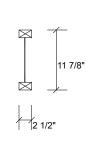
Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

NJH 11.875" - PASSED F7-A





Wind

O

0

0

0

| wember | intormation |
|--------|-------------|
| Type: | Girder |

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live: 15 PSF Dead:

Application: Floor (Residential) Design Method:

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No

Not Checked Deck: Vibration: Not Checked

Bearings and Factored Reactions

Live

272

272

Brg

1

2

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.375" 31% 127 / 408 535 I 1.25D+1.5L 2 - SPF 3.375" 31% 127 / 408 535 L 1.25D+1.5L

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

102

102

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-------------|------------|---------|
| Moment | 1259 ft-lb | 4'3 3/4" | 5390 ft-lb | 0.234 (23%) | 1.25D+1.5L | L |
| Unbraced | 1259 ft-lb | 4'3 3/4" | 1315 ft-lb | 0.958 (96%) | 1.25D+1.5L | L |
| Shear | 525 lb | 8'4 7/8" | 1810 lb | 0.290 (29%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.014 (L/7253) | 4'3 13/16" | 0.273 (L/360) | 0.050 (5%) | D | Uniform |
| LL Defl inch | 0.036 (L/2720) | 4'3 13/16" | 0.273 (L/360) | 0.130 (13%) | L | L |
| TL Defl inch | 0.050 (L/1978) | 4'3 13/16" | 0.409 (L/240) | 0.120 (12%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange unbraced.
- 3 Bottom flange braced at bearings

| o bottom nange | bracea at bearings. | | | | I . | | | | |
|----------------|---------------------|----------------|--------------------------|-----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 8-7-8 | (Span)1-0-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 0-5-7 | (Span)0-0-13 to 0-6-4 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Tie-In | 0-5-7 to 2-7-4 | (Span)0-5-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 4 | Tie-In | 2-7-4 to 6-0-4 | (Span)1-7-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 5 | Point | 2-8-8 | | Near Face | 39 lb | 104 lb | 0 lb | 0 lb | F5 |
| 6 | Point | 5-11-0 | | Near Face | 39 lb | 104 lb | 0 lb | 0 lb | F5 |
| 7 | Tie-In | 6-0-4 to 8-2-1 | (Span)0-5-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 8 | Tie-In | 8-2-1 to 8-7-8 | (Span)0-6-4 to 0-0-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | | | | | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





PAGE 18 OF 39 NE0618-038



Client: **GREEN YORK HOMES**

Project: Address:

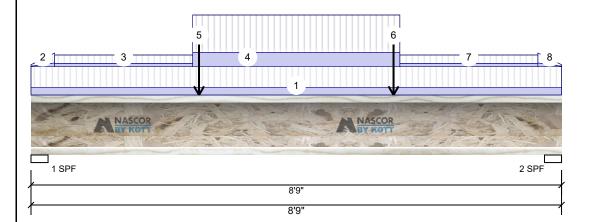
Date: 6/1/2018 Designer: RCO

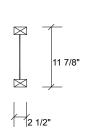
Job Name: INDIGO 1 (ELEV.1)

Level: Ground Floor

Project #:

11.875" - PASSED F7-B NJH





Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |
| | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 275 | 103 | 0 | 0 |
| 2 | 275 | 103 | 0 | 0 |
| | | | | |

Bearings and Factored Reactions

| Bearing Length | Cap. R | eact D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|--------|-------------|-------|----------|------------|--|
| 1 - SPF 3.375" | 31% | 129 / 413 | 542 | L | 1.25D+1.5L | |
| 2 - SPF 3.375" | 31% | 129 / 413 | 542 | L | 1.25D+1.5L | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-----------------|------------|---------|
| Moment | 1323 ft-lb | 4'4 1/2" | 5390 ft-lb | 0.246 (25%) | 1.25D+1.5L | L |
| Unbraced | 1323 ft-lb | 4'4 1/2" | 1326 ft-lb | 0.998 (100%) | 1.25D+1.5L | L |
| Shear | 533 lb | 2 5/8" | 1810 lb | 0.294 (29%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.015 (L/6852) | 4'4 9/16" | 0.277 (L/360) | 0.050 (5%) | D | Uniform |
| LL Defl inch | 0.039 (L/2576) | 4'4 9/16" | 0.277 (L/360) | 0.140 (14%) | L | L |
| TL Defl inch | 0.053 (L/1872) | 4'4 9/16" | 0.416 (L/240) | 0.130 (13%) | D+L | L |
| | | | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 8'2" o.c.

3 Bottom flange braced at bearings

| 3 DOLLOTT | nange braced at bearings |). | | | | | | | |
|-----------|--------------------------|-----------------|--------------------------|-----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 8-9-0 | (Span)0-11-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 0-4-11 | (Span)0-0-13 to 0-5-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Tie-In | 0-4-11 to 2-8-0 | (Span)0-4-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 4 | Tie-In | 2-8-0 to 6-1-0 | (Span)1-8-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 5 | Point | 2-9-4 | | Near Face | 43 lb | 114 lb | 0 lb | 0 lb | F5 |
| 6 | Point | 5-11-12 | | Near Face | 43 lb | 114 lb | 0 lb | 0 lb | F5 |
| 7 | Tie-In | 6-1-0 to 8-4-5 | (Span)0-4-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 8 | Tie-In | 8-4-5 to 8-9-0 | (Span)0-5-8 to 0-0-13 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- IARIGHING & INSEGUATION

 Lodist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, suffiener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid

lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent populing.

Manufacturer Info

Nascor by Kott







NE0618-038 **PAGE 19 OF 39**

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

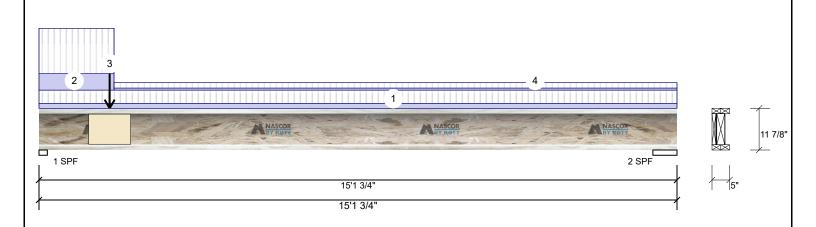
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED **NJH** 11.875" F8-A

Level: Ground Floor



| Member Info | rmation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | |
|--------------------|---------|----------------|----------------------|--|-----------|-------------------|----------------|------------|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | |
| Plies: | 2 | Design Method: | LSD | 1 | 705 | 265 | 0 | 0 | |
| Moisture Condition | n: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 272 | 102 | 0 | 0 | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | |
| General Load | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | | |
| Dead: | 15 PSF | | | Bearing L | ength. | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | |
| | | | | 1-SPF 2 | .375" | 42% 331 / 1057 | 1388 L | 1.25D+1.5L | |
| | | | | 2-SPF 6 | .875" | 15% 128 / 409 | 536 L | 1.25D+1.5L | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 2312 ft-lb | 5'6 3/8" | 10780 ft-lb | 0.214 (21%) | 1.25D+1.5L | L |
| Unbraced | 2312 ft-lb | 5'6 3/8" | 2331 ft-lb | 0.992 (99%) | 1.25D+1.5L | L |
| Shear | 1365 lb | 1 5/8" | 3620 lb | 0.377 (38%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.031 (L/5557) | 6'10 3/4" | 0.483 (L/360) | 0.060 (6%) | D | Uniform |
| LL Defl inch | 0.083 (L/2086) | 6'10 3/4" | 0.483 (L/360) | 0.170 (17%) | L | L |
| TL Defl inch | 0.115 (L/1517) | 6'10 3/4" | 0.725 (L/240) | 0.160 (16%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'8" o.c.

5 Bottom flange braced at bearings

| - | | | | | | | | | | |
|----|-----------|------------------|--------------|-----------|--------|--------|-------|-------|----------|--|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments | |
| 1 | Tie-In | 0-0-0 to 15-1-12 | (Span)1-0-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 2 | Tie-In | 0-0-0 to 1-9-6 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |
| 3 | Point | 1-8-2 | | Near Face | 166 lb | 441 lb | 0 lb | 0 lb | F6 | |
| 4 | Tie-In | 1-9-6 to 15-1-12 | (Span)0-4-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | | |

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

Nascor by Kott



Manufacturer Info



PAGE 20 OF 39 NE0618-038

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

Date: 6/1/2018

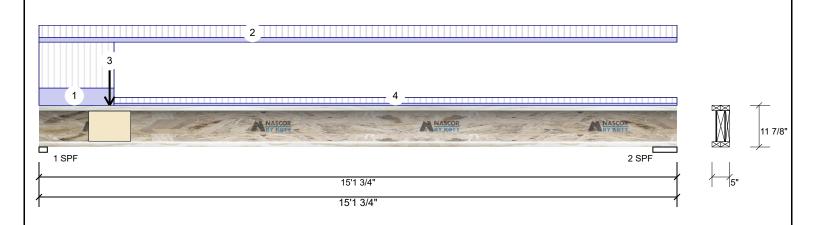
Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED 11.875" F8-B NJH

Level: Ground Floor



| Member Info | rmation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | | |
|--------------------|---------|----------------|----------------------|--|----------|-------------------|----------------|------------|--|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | |
| Plies: | 2 | Design Method: | LSD | 1 | 690 | 259 | 0 | 0 | | |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 259 | 97 | 0 | 0 | | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | |
| General Load | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | nd Facto | red Reactions | | | | |
| Dead: | 15 PSF | | | Bearing L | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | |
| | | | | 1 - SPF 2 | .375" | 41% 324 / 1036 | 1360 L | 1.25D+1.5L | | |
| | | | | 2-SPF 6 | .875" | 14% 122 / 389 | 510 L | 1.25D+1.5L | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-----------------|------------|---------|
| Moment | 2227 ft-lb | 5'5 1/16" | 10780 ft-lb | 0.207 (21%) | 1.25D+1.5L | L |
| Unbraced | 2227 ft-lb | 5'5 1/16" | 2234 ft-lb | 0.997 (100%) | 1.25D+1.5L | L |
| Shear | 1336 lb | 1 5/8" | 3620 lb | 0.369 (37%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.030 (L/5780) | 6'10 9/16" | 0.483 (L/360) | 0.060 (6%) | D | Uniform |
| LL Defl inch | 0.080 (L/2169) | 6'10 9/16" | 0.483 (L/360) | 0.170 (17%) | L | L |
| TL Defl inch | 0.110 (L/1577) | 6'10 9/16" | 0.725 (L/240) | 0.150 (15%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY**

Manufacturer Info

Nascor by Kott

NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'9" o.c.

5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|------------------|-------------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-9-6 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 15-1-12 | (Span) 0-10-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-8-2 | | Far Face | 165 lb | 439 lb | 0 lb | 0 lb | F6 |
| 4 | Tie-In | 1-9-6 to 15-1-12 | (Span)0-5-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

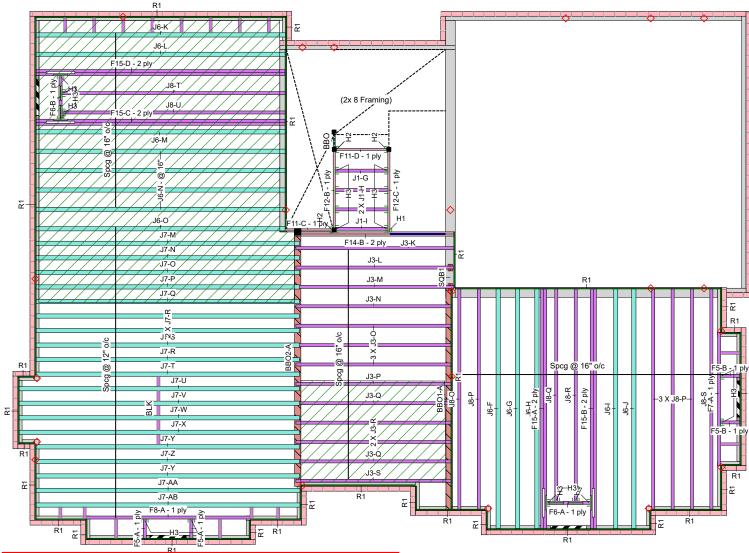
- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
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 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthp=3.5 inches
 For flat roofs provide proper drainage to prevent





Ground Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.**

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend



Load from Above Wall Wall Opening NJ60U 11.875 NJH 11.875

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09



Norbord Rimboard Plus 1.125 X 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

1. OBC 2012 O.Reg 332/12 as amended

- 5. CCMC -12787-R APA PR-L310(C)

| | | | | | | | | 17101 01 00 | | | |
|-----------|----------------------------------|-------|--------|-------|-------|--------|-----------------|---------------------------------------|--|--|--|
| - | Ground Floor LVL/LSL (Flush) | | | | | | | | | | |
| | Description | Width | Depth | Qty | Plies | Pcs | Length | NASCOR | | | |
| F14 | Forex 2.0E-3000Fb LVL | 1.75 | 11.875 | 1 | 2 | 2 | 12-0-0 | INASCON | | | |
| F12 | Forex 2.0E-3000Fb LVL | 1.75 | 11.875 | | | 2 | 6-0-0 | Layout Name INDIGO 1 (ELEV.2) | | | |
| F11 | Forex 2.0E-3000Fb LVL | 1.75 | 11.875 | | | 2 | 4-0-0 | Design Method | | | |
| I Joist (| Flush) | | | | | | | | | | |
| Label | Description | Width | Depth | Qty | Plies | Pcs | Length | Description | | | |
| J7 | NJ60U | 3.5 | 11.875 | | | 19 | 20-0-0 | GRANELLI HOME CORP. BRAMPTON, ONT. | | | |
| J6 | NJ60U | 3.5 | 11.875 | | | 13 | 18-0-0 | , | | | |
| F15 | NJH | 2.5 | 11.875 | 4 | 2 | 8 | 18-0-0 | Created | | | |
| F8 | NJH | 2.5 | 11.875 | | | 1 | 12-0-0 | May 30, 2018 | | | |
| F7 | NJH | 2.5 | 11.875 | | | 1 | 10-0-0 | Builder | | | |
| F6 | NJH | 2.5 | 11.875 | | | 2 | 4-0-0 | GREEN YORK HOMES | | | |
| F5 | NJH | 2.5 | 11.875 | | | 4 | 2-0-0 | Sales Rep | | | |
| J8 | NJH | 2.5 | 11.875 | | | 10 | 16-0-0 | RM | | | |
| J3 | NJH | 2.5 | 11.875 | | | 13 | 12-0-0 | | | | |
| J1 | NJH | 2.5 | 11.875 | | | 4 | 4-0-0 | Designer | | | |
| Rim Bo | ard | | | | | | | RCO | | | |
| Label | Description | Width | Depth | Qty | Plies | Pcs | Length | Shipping | | | |
| R1 | Norbord Rimboard Plus 1.125 X | 1.125 | 11.875 | | | 15 | 12 | Project | | | |
| | 11.875 | | | | | | | Builder's Project | | | |
| Blockin | g | | | | | | | Kott Lumber Company | | | |
| Label | Description | Width | Depth | Qty | Plies | Pcs | Length | 14 Anderson Blvd | | | |
| BLK1 | NJH | 2.5 | 11.875 | LinFt | | Varies | 21-0-0 | Stouffville, Ontario | | | |
| Hanger | | | | | | | | Canada | | | |
| | | | | | | | ported ember | L4A 7X4 905-642-4400 | | | |
| | | | | | | | | | | | |

fasteners

4 10dx1 1/2

NOTES:

Label

H2

Framer to verify dimensions on the architectural drawings.

Pcs Description

1 HUS1.81/10

3 LS90

20 LT251188

- . Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of rimboard/rimjoist.

Skew Slope

- . Refer to Nascor specifier guide for installation works.
- i. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads.
- 3. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction

905-642-4400 fasteners Job Path D:\Llsers\rochavillo\WORK FROM HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS

\INDIGO 1\INDIGO 1 ELEV.2\FLOOR \INDIGO 1 (ELEV.2).isl **Ground Floor** Design Method

2012

OSB

3/4"

Nailed & Glued

Building Code NBCC 2010 / OBC

Deck

Thickness

Fastener

Vibration

Floor Loads Live Dead 15 **Deflection Joist** 480 LL Span L/ 360 TL Span L/ LL Cant 2L/ 480 360 TL Cant 2L/ Deflection Girder 360 LL Span L/ TL Span L/ 240 480 LL Cant 2L/ TL Cant 2L/ 360 Decking



EWP Studio

Simpson Strong-Tie® Component Solutions™

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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

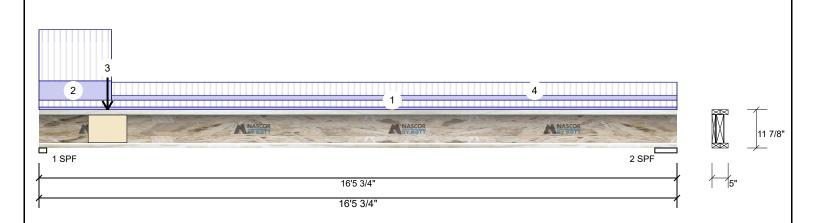
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" 2-Ply - PASSED F15-A NJH

Level: Ground Floor



| Member Info | rmation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | | |
|--------------------|---------|----------------|----------------------|--|-----------|-------------------|----------------|------------|--|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | |
| Plies: | 2 | Design Method: | LSD | 1 | 684 | 256 | 0 | 0 | | |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 273 | 102 | 0 | 0 | | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | |
| General Load | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | | | |
| Dead: | 15 PSF | | | Bearing L | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | |
| | | | | 1-SPF 2 | .375" | 41% 320 / 1026 | 1346 L | 1.25D+1.5L | | |
| | | | | 2-SPF 6 | .875" | 15% 128 / 410 | 538 L | 1.25D+1.5L | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-----------------|------------|---------|
| Moment | 2487 ft-lb | 6'2 13/16" | 10780 ft-lb | 0.231 (23%) | 1.25D+1.5L | L |
| Unbraced | 2487 ft-lb | 6'2 13/16" | 2497 ft-lb | 0.996 (100%) | 1.25D+1.5L | L |
| Shear | 1325 lb | 1 5/8" | 3620 lb | 0.366 (37%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.039 (L/4815) | 7'6 7/8" | 0.528 (L/360) | 0.070 (7%) | D | Uniform |
| LL Defl inch | 0.105 (L/1805) | 7'6 7/8" | 0.528 (L/360) | 0.200 (20%) | L | L |
| TL Defl inch | 0.145 (L/1313) | 7'6 7/8" | 0.792 (L/240) | 0.180 (18%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY**

NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'4" o.c.

5 Bottom flange braced at bearings

| o Bottom namge | Diadea at Dealinge. | | | | | | | | |
|----------------|---------------------|-------------------|--------------|-----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 16-5-12 | (Span)0-5-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-10-8 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-9-4 | | Near Face | 158 lb | 422 lb | 0 lb | 0 lb | F6 |
| 4 | Tie-In | 1-10-8 to 16-5-12 | (Span)0-10-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

Manufacturer Info

Nascor by Kott





NE0618-038 PAGE 23 OF 39

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

Date:

Designer: RCO

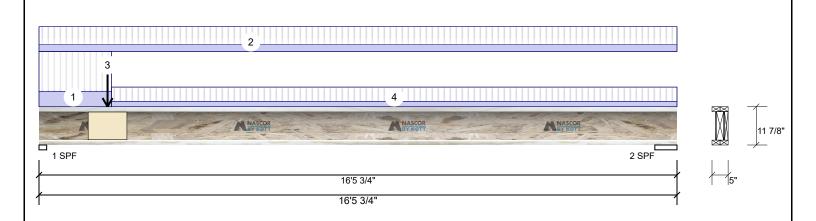
Job Name: INDIGO 1 (ELEV.2)

6/1/2018

Project #:

2-Ply - PASSED F15-B NJH 11.875"

Level: Ground Floor



| Member Info | rmation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | | |
|--------------------|---------|----------------|----------------------|--|-----------|-------------------|----------------|------------|--|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | | |
| Plies: | 2 | Design Method: | LSD | 1 | 846 | 317 | 0 | 0 | | |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 505 | 189 | 0 | 0 | | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | | |
| General Load | | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | | | |
| Dead: | 15 PSF | | | Bearing L | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | | |
| | | | | 1 - SPF 2 | .375" | 50% 397 / 1269 | 1666 L | 1.25D+1.5L | | |
| | | 1 | | 2-SPF 6 | .875" | 27% 237 / 758 | 994 L | 1.25D+1.5L | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-----------------|------------|---------|
| Moment | 4073 ft-lb | 7'3 9/16" | 10780 ft-lb | 0.378 (38%) | 1.25D+1.5L | L |
| Unbraced | 4073 ft-lb | 7'3 9/16" | 4080 ft-lb | 0.998 (100%) | 1.25D+1.5L | L |
| Shear | 1639 lb | 1 5/8" | 3620 lb | 0.453 (45%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.065 (L/2937) | 7'9 11/16" | 0.528 (L/360) | 0.120 (12%) | D | Uniform |
| LL Defl inch | 0.172 (L/1102) | 7'9 11/16" | 0.528 (L/360) | 0.330 (33%) | L | L |
| TL Defl inch | 0.237 (L/801) | 7'9 11/16" | 0.792 (L/240) | 0.300 (30%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'8" o.c.
- 5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|-------------------|-------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-10-8 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 16-5-12 | (Span)1-6-7 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-9-4 | | Far Face | 136 lb | 362 lb | 0 lb | 0 lb | F6 |
| 4 | Tie-In | 1-10-8 to 16-5-12 | (Span)1-2-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

Nascor by Kott

Manufacturer Info





NE0618-038 PAGE 24 OF 39

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

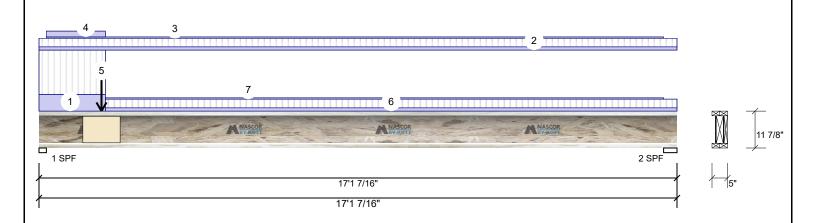
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Project #:

2-Ply - PASSED F15-C NJH 11.875"

Level: Ground Floor



| Member Infor | mation | | | Unfactored Reactions UNPATTERNED Ib (Uplift) | | | | | |
|--------------------|--------|----------------|----------------------|--|-----------|-------------------|----------------|------------|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | |
| Plies: | 2 | Design Method: | LSD | 1 | 755 | 381 | 0 | 0 | |
| Moisture Condition | : Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 279 | 144 | 0 | 0 | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | |
| General Load | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | | |
| Dead: | 15 PSF | | | Bearing L | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | |
| | | | | 1 - SPF 2 | .375" | 49% 477 / 1133 | 1609 L | 1.25D+1.5L | |
| | | | | 2-SPF 4 | .319" | 17% 180 / 419 | 599 L | 1.25D+1.5L | |

Analysis Results

| - | | | | | | | |
|---|---------------|----------------|-------------|---------------|-----------------|------------|---------|
| | Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
| | Moment | 2960 ft-lb | 6'8 1/8" | 10780 ft-lb | 0.275 (27%) | 1.25D+1.5L | L |
| | Unbraced | 2960 ft-lb | 6'8 1/8" | 2970 ft-lb | 0.997 (100%) | 1.25D+1.5L | L |
| | Shear | 1588 lb | 1 5/8" | 3620 lb | 0.439 (44%) | 1.25D+1.5L | L |
| | Perm Defl in. | 0.065 (L/3060) | 8' 1/16" | 0.556 (L/360) | 0.120 (12%) | D | Uniform |
| | LL Defl inch | 0.127 (L/1581) | 7'11 15/16" | 0.556 (L/360) | 0.230 (23%) | L | L |
| | TL Defl inch | 0.192 (L/1042) | 8' | 0.834 (L/240) | 0.230 (23%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'9" o.c.

5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|--------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-9-6 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 17-1-7 | (Span)0-7-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-2-5 to 16-9-2 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-2-5 to 1-9-6 | | Тор | 9 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-8-2 | | Far Face | 240 lb | 480 lb | 0 lb | 0 lb | F6 |
| 6 | Tie-In | 1-9-6 to 17-1-7 | (Span)0-8-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-9-6 to 16-9-1 | | Тор | 2 PLF | 0 PLF | 0 PLF | 0 PLF | |

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthp=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-038 PAGE 25 OF 39

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

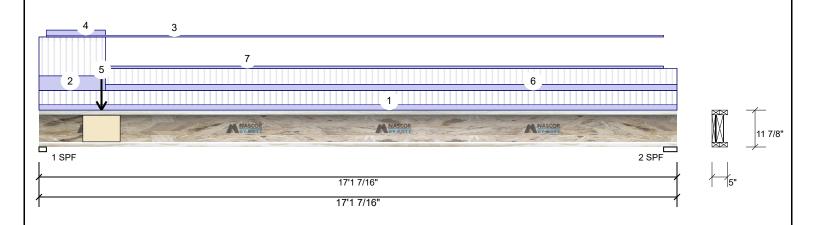
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Project #:

2-Ply - PASSED F15-D NJH 11.875"

Level: Ground Floor



| Member Info | rmation | | | Unfactore | d Reacti | ions UNPATTERN | ED lb (Uplift) | |
|--------------------|---------|----------------|----------------------|------------|----------|-------------------|----------------|------------|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind |
| Plies: | 2 | Design Method: | LSD | 1 | 832 | 417 | 0 | 0 |
| Moisture Condition | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 496 | 248 | 0 | 0 |
| Deflection LL: | 360 | Load Sharing: | No | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | |
| General Load | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Fact | ored Reactions | | |
| Dead: | 15 PSF | | | Bearing L | ength. | Cap. React D/L lb | Total Ld. Case | Ld. Comb. |
| | | | | 1 - SPF 2 | 375" | 53% 521 / 1248 | 1769 L | 1.25D+1.5L |
| | • | | | 2-SPF 4 | .319" | 29% 310 / 744 | 1054 L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|------------|---------------|-----------------|------------|---------|
| Moment | 4604 ft-lb | 7'9 15/16" | 10780 ft-lb | 0.427 (43%) | 1.25D+1.5L | L |
| Unbraced | 4604 ft-lb | 7'9 15/16" | 4611 ft-lb | 0.998 (100%) | 1.25D+1.5L | L |
| Shear | 1744 lb | 1 5/8" | 3620 lb | 0.482 (48%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.100 (L/2004) | 8'3 3/16" | 0.556 (L/360) | 0.180 (18%) | D | Uniform |
| LL Defl inch | 0.198 (L/1013) | 8'3 3/16" | 0.556 (L/360) | 0.360 (36%) | L | L |
| TL Defl inch | 0.298 (L/673) | 8'3 3/16" | 0.834 (L/240) | 0.360 (36%) | D+L | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 6'2" o.c.

5 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|-----------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 17-1-7 | (Span)1-3-1 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 1-9-6 | (Span)3-5-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Part. Uniform | 0-2-6 to 16-9-1 | | Тор | 3 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Part. Uniform | 0-2-6 to 1-9-6 | | Тор | 9 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 5 | Point | 1-8-2 | | Near Face | 172 lb | 343 lb | 0 lb | 0 lb | F6 |
| 6 | Tie-In | 1-9-6 to 17-1-7 | (Span)1-4-15 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 7 | Part. Uniform | 1-9-6 to 16-9-1 | | Тор | 4 PLF | 0 PLF | 0 PLF | 0 PLF | |

NOtes
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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthp=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







PAGE 26 OF 39 NE0618-038



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

Project #:

NJH 11.875" - PASSED F5-A



1 SPF

Hanger (LT251188) 1'3 7/8" 1'3 7/8"

11 7/8"

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

| Member | Information |
|--------|-------------|
| Member | miormation |

| Type: | Girder |
|---------------------|--------|
| Plies: | 1 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Not Checked Deck:

Vibration: Not Checked

| Brg | J Live | Dead | Snow | vvina | |
|-----|--------|------|------|-------|--|
| 1 | 43 | 16 | 0 | 0 | |
| 2 | 41 | 16 | 0 | 0 | |
| 1 | | | | | |

Cap. React D/L lb

5%

5%

20 / 65

19 / 62

Unfactored Reactions UNPATTERNED Ib (Uplift)

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|---------------|----------|---------------|------------|------------|------|
| Moment | 19 ft-lb | 8 1/8" | 5390 ft-lb | 0.003 (0%) | 1.25D+1.5L | L |
| Unbraced | 19 ft-lb | 8 1/8" | 5352 ft-lb | 0.003 (0%) | 1.25D+1.5L | L |
| Shear | 68 lb | 1 5/8" | 1810 lb | 0.038 (4%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Bearings and Factored Reactions

Bearing Length

1 - SPF 2.375"

2 -

Hanger

2.000"

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Total Ld. Case

86 I

82 L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|-----------------|-------------|------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-3-14 | (Span)3-2-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- IARIGHING & INSEGUATION

 Lodist flanges must not be cut or drilled

 Refer to latest copy of the IJoist product information details for framing details, suffiener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged IJoists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- alteral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info Nascor by Kott







PAGE 27 OF 39 NE0618-038



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

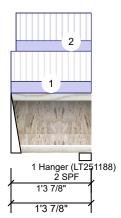
6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

Project #:

11.875" - PASSED F5-B NJH



11 7/8"

Wind 0

0

| Member In | formation | | | Unfacto | red Reactions | UNPATTER | NED lb (Uplift) |) |
|-----------|-----------|----------------|---------------------|---------|---------------|----------|-----------------|---|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | |
| Plies. | 1 | Design Method: | LSD | 1 4 | 74 | 20 | 0 | |

Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load

40 PSF Floor Live: 15 PSF Dead: Analysis Results

| Bearings | Bearings and Factored Reactions | | | | | | | | | | |
|---------------|---------------------------------|----------|-----------|-------|----------|------------|--|--|--|--|--|
| Bearing | Length | Cap. Rea | ct D/L lb | Total | Ld. Case | Ld. Comb. | | | | | |
| 1 - Hanger | 2.000" | 9% | 35 / 112 | 147 | L | 1.25D+1.5L | | | | | |
| 2 - SPF | 2.375" | 10% | 39 / 125 | 164 | L | 1.25D+1.5L | | | | | |

Capacity Actual Location Allowed Comb. Case Analysis Moment 35 ft-lb 7 3/4" 5390 ft-lb 0.007 (1%) 1.25D+1.5L L Unbraced 35 ft-lb 7 3/4" 5352 ft-lb 0.007 (1%) 1.25D+1.5L L 131 lb 1 1/4" 1810 lb 0.072 (7%) 1.25D+1.5L L Shear Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 7 3/4" 0.036 (L/360) 0.010 (1%) L L (L/40749) TL Defl inch 0.000 7 3/4" 0.054 (L/240) 0.010 (1%) D+L L (L/29636)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|-----------------|--------------|------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 1-3-14 | (Span)3-2-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-1-0 to 1-3-14 | (Snan)2-11-3 | Ton | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid

lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







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2



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

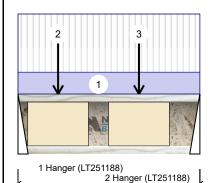
6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

Project #:

11.875" - PASSED F6-A NJH



3'

11 7/8"

| ٨ | /lom | hor | Inforr | nation |
|----|------|-----|--------|--------|
| ı١ | иem | ber | intorr | nation |

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 422 | 158 | 0 | 0 |
| 2 | 362 | 136 | 0 | 0 |
| | | | | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|-------------|---------------|-------------|------------|---------|
| Moment | 607 ft-lb | 1'11 15/16" | 5390 ft-lb | 0.113 (11%) | 1.25D+1.5L | L |
| Unbraced | 607 ft-lb | 1'11 15/16" | 4800 ft-lb | 0.126 (13%) | 1.25D+1.5L | L |
| Shear | 823 lb | 1 1/4" | 1810 lb | 0.455 (45%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.002 (L/13854) | 1'11 1/16" | 0.093 (L/360) | 0.030 (3%) | D | Uniform |
| LL Defl inch | 0.006 (L/5198) | 1'11 1/16" | 0.093 (L/360) | 0.070 (7%) | L | L |
| TL Defl inch | 0.009 (L/3780) | 1'11 1/16" | 0.140 (L/240) | 0.060 (6%) | D+L | L |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|---------|------------|-------|----------|------------|
| 1 - Hanger | 2.000" | 51% | 197 / 633 | 830 | L | 1.25D+1.5L |
| 2 - Hanger | 2.000" | 44% | 170 / 543 | 713 | L | 1.25D+1.5L |

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



| ID | Load Type | Location | Trib Width Side | Dead | Live | Snow | Wind | Comments |
|----|-----------|----------------|------------------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 3-0-0 | (Span)1-9-13 Top | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Point | 0-7-15 | Far Face | 117 lb | 313 lb | 0 lb | 0 lb | J8 |
| 3 | Point | 1-11-15 | Far Face | 136 lb | 362 lb | 0 lb | 0 lb | J8 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







PAGE 29 OF 39 NE0618-038

Brg

1

2

1 -

2 -

Hanger

Hanger

Bearing Length

2.000"

2.000"



EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

240

172

Cap. React D/L lb

63%

45%

300 / 721

215 / 515

Live

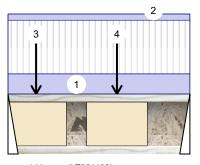
480

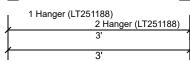
343

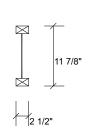
Bearings and Factored Reactions

Project #:

11.875" - PASSED F6-B NJH







Wind

O

0

Total Ld. Case

729 L

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

| Member In | formation |
|-----------|-----------|
|-----------|-----------|

| Type: | Girder |
|---------------------|--------|
| Plies: | 1 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Not Checked Deck:

Vibration: Not Checked

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 749 ft-lb | 1'9 9/16" | 5390 ft-lb | 0.139 (14%) | 1.25D+1.5L | L |
| Unbraced | 749 ft-lb | 1'9 9/16" | 4800 ft-lb | 0.156 (16%) | 1.25D+1.5L | L |
| Shear | 1013 lb | 1 1/4" | 1810 lb | 0.560 (56%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.004 (L/9092) | 1'9 9/16" | 0.093 (L/360) | 0.040 (4%) | D | Uniform |
| LL Defl inch | 0.007 (L/4555) | 1'9 9/16" | 0.093 (L/360) | 0.080 (8%) | L | L |
| TL Defl inch | 0.011 (L/3035) | 1'9 9/16" | 0.140 (L/240) | 0.080 (8%) | D+L | L |

CALCULATION SUMMARY PAGE AS IT

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL

POINT LOADS OVER BEARINGS



| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|----------------|--------------|-----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 3-0-0 | (Span)1-8-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 3-0-0 | | Тор | 4 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 3 | Point | 0-5-9 | | Near Face | 152 lb | 304 lb | 0 lb | 0 lb | J8 |
| 4 | Point | 1-9-9 | | Near Face | 209 lb | 416 lb | 0 lb | 0 lb | J8 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- alteral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott





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Client: **GREEN YORK HOMES**

Project:

Address:

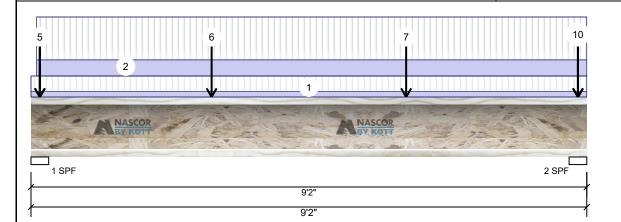
Date: 6/1/2018 Designer: RCO

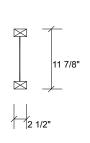
Job Name: INDIGO 1 (ELEV.2)

Level: Ground Floor

Project #:

11.875" - PASSED NJH F7-A





Member Information

| 71 | |
|---------------------|--------|
| Plies: | 1 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |
| | |

Floor (Residential) Application:

Design Method: **Building Code:**

NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Deck:

Vibration: Not Checked

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 1148 ft-lb | 4'7" | 5174 ft-lb | 0.222 (22%) | 1.25D+1.5L | L |
| Unbraced | 1148 ft-lb | 4'7" | 1155 ft-lb | 0.994 (99%) | 1.25D+1.5L | L |
| Shear | 489 lb | 8'11 1/4" | 1738 lb | 0.281 (28%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.014 (L/7693) | 4'7" | 0.290 (L/360) | 0.050 (5%) | D | Uniform |
| LL Defl inch | 0.036 (L/2894) | 4'7" | 0.290 (L/360) | 0.120 (12%) | L+0.5S | L |
| TL Defl inch | 0.050 (L/2103) | 4'7" | 0.435 (L/240) | 0.110 (11%) | D+L+0.5S | L |

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 8'8" o.c.
- 3 Bottom flange braced at bearings.

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 326 | 394 | 69 | 0 |
| 2 | 328 | 395 | 69 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. I | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------|--------|--------|--------------|-------|----------|---------------------|
| 1 - SPF | 3.500" | 59% | 492 / 523 | 1015 | L | 1.25D+1.5L +0.5S |
| 2 - SPF | 3.500" | 59% | 493 / 527 | 1020 | L | 1.25D+1.5L +0.5S |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



| | gg- | | | | | | | | |
|----|-----------|----------------|--------------|-----------|--------|--------|-------|-------|------------------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 9-2-0 | (Span)0-6-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-1-2 to 9-2-0 | (Span)1-5-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 0-1-12 | | Тор | 18 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 4 | Point | 0-1-12 | | Тор | 261 lb | 71 lb | 69 lb | 0 lb | F3 F3 |
| 5 | Point | 0-1-12 | | Тор | 19 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 6 | Point | 2-11-12 | | Near Face | 28 lb | 74 lb | 0 lb | 0 lb | F5 |
| 7 | Point | 6-2-4 | | Near Face | 28 lb | 74 lb | 0 lb | 0 lb | F5 |
| 8 | Point | 9-0-4 | | Тор | 261 lb | 71 lb | 69 lb | 0 lb | F3 F3 |
| 9 | Point | 9-0-4 | | Тор | 19 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 10 | Point | 9-0-4 | | Тор | 18 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengthp=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

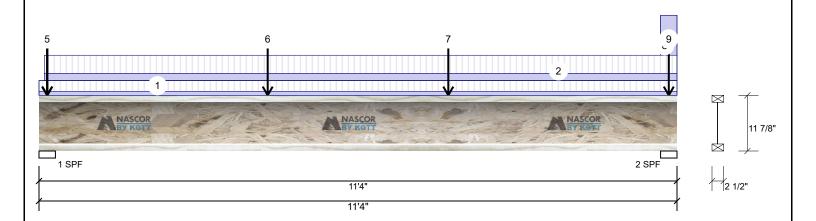
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.2)

Project #:

11.875" - PASSED **NJH** F8-A

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Application: Floor (Residential) Type: Brg Live Dead Snow Plies: Design Method: 405 660 136 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 407 644 136 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case 1-SPF 3.500" 825 / 675 92% 1500 L

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|----------------|-----------|---------------|-------------|------------|---------|
| Moment | 1671 ft-lb | 5'8" | 4851 ft-lb | 0.344 (34%) | 1.25D+1.5L | L |
| Unbraced | 1671 ft-lb | 5'8" | 1680 ft-lb | 0.994 (99%) | 1.25D+1.5L | L |
| Shear | 586 lb | 11'1 1/4" | 1629 lb | 0.360 (36%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.028 (L/4732) | 5'8" | 0.362 (L/360) | 0.080 (8%) | D | Uniform |
| LL Defl inch | 0.073 (L/1788) | 5'8" | 0.362 (L/360) | 0.200 (20%) | L+0.5S | L |
| TL Defl inch | 0.101 (L/1298) | 5'8" | 0.544 (L/240) | 0.180 (18%) | D+L+0.5S | L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

91%

805 / 679

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL

2 - SPF 3.500"



Wind

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

+0.5S

+0.5S

1484 L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 7'2" o.c.

| 3 Bottom flange braced at bearings. | | | | | | POINT LOAD | S OVER BEAR | NGS. | Juli 05, 2010 |
|-------------------------------------|---------------|------------------|--------------|-----------|--------|------------|-------------|-------|------------------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 11-4-0 | (Span)0-10-5 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-1-2 to 11-4-0 | (Span)1-5-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 0-1-12 | | Тор | 21 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 4 | Point | 0-1-12 | | Тор | 512 lb | 102 lb | 136 lb | 0 lb | F4 F4 |
| 5 | Point | 0-1-12 | | Тор | 13 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 6 | Point | 4-0-12 | | Near Face | 16 lb | 41 lb | 0 lb | 0 lb | F5 |
| 7 | Point | 7-3-4 | | Near Face | 16 lb | 41 lb | 0 lb | 0 lb | F5 |
| 8 | Part. Uniform | 11-0-8 to 11-4-0 | | Тор | 64 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| 9 | Point | 11-2-4 | | Тор | 510 lb | 102 lb | 136 lb | 0 lb | F4 F4 |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

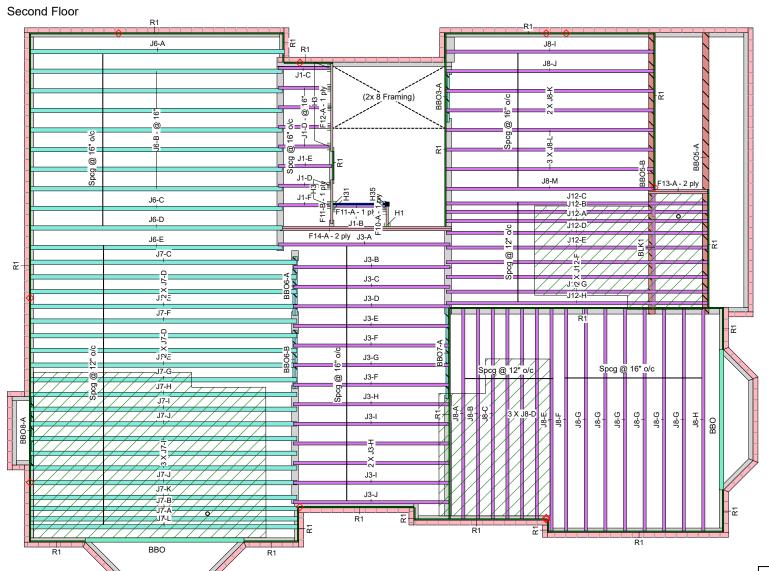
- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info Nascor by Kott











THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

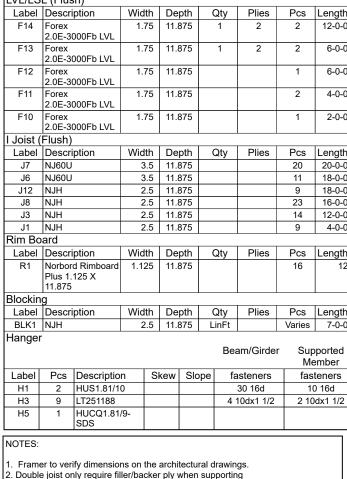
Legend



Load from Above Wall

Norbord Rimboard Plus 1.125 X 11.875 NJ60U 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)



- another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 1. Install single-ply flush window header along inside face of rimboard/rimjoist. . Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on
- all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- 3. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or boltina requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

Second Floor LVL/LSL (Flush) Pcs Length 6-0-0 INDIGO 1 (ELEV.1) 6-0-0 Design Method LSD 4-0-0 Description GRANELLI HOME CORP. 2-0-0 BRAMPTON, ONT. Created Pcs Length May 30, 2018 20 20-0-0 Builder 11 18-0-0 **GREEN YORK HOMES** 9 18-0-0 Sales Rep 16-0-0 RM 14 12-0-0 9 4-0-0 Designer RCO Pcs Length Shipping Project Builder's Project **Kott Lumber Company** Pcs Length 14 Anderson Blvd Varies 7-0-0 Stouffville, Ontario Canada Supported I 4A 7X4 Member 905-642-4400 fasteners Job Path

HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS \INDIGO 1\INDIGO 1 ELEV.1\FLOOR \INDIGO 1 (ELEV.1).isl

D:\LIsers\rochavillo\WORK FROM

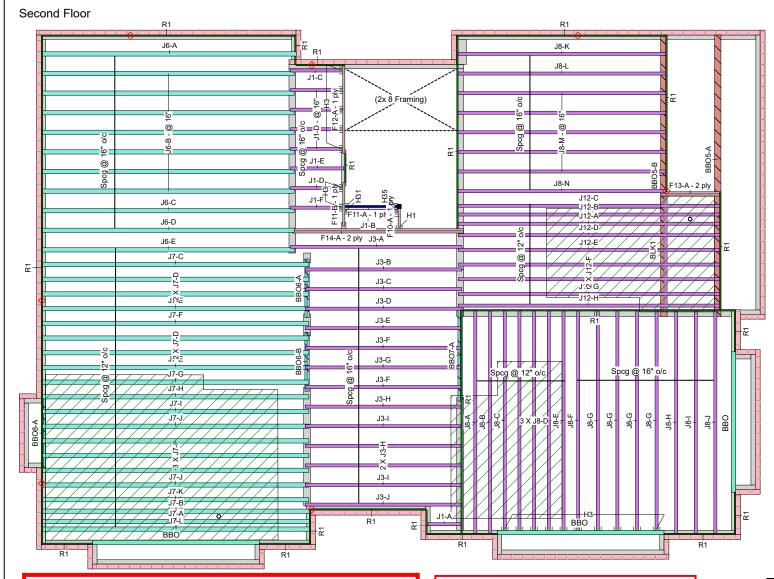
Second Floor

| Second i loo | • |
|-----------------|-----------------|
| Design Method | LSD |
| Building Code | NBCC 2010 / OBC |
| | 2012 |
| Floor | |
| Loads | |
| Live | 40 |
| Dead | 15 |
| Deflection Jois | t |
| LL Span L/ | 480 |
| TL Span L/ | 360 |
| LL Cant 2L/ | 480 |
| TL Cant 2L/ | 360 |
| Deflection Gird | ler |
| LL Span L/ | 360 |
| TL Span L/ | 240 |
| LL Cant 2L/ | 480 |
| TL Cant 2L/ | 360 |
| Decking | |

| TL Cant 2L/ | 360 |
|-------------|----------------|
| Decking | |
| Deck | OSB |
| Thickness | 5/8" |
| Fastener | Nailed & Glued |
| Vibration | |







THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY **NAILING OR BOLTING REQUIREMENTS.**

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL **POINT LOADS OVER BEARINGS.**



ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55 Model: Indigo 1

Legend

Load from Above NJ60U 11.875 NJH 11.875

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Norbord Rimboard Plus 1.125 X 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

1. OBC 2012 O.Reg 332/12 as amended

| | | | | | | | | | | | PAGE 33 OF 39 |
|---------|------------------|------------------|-----|-------------------|---------|----|--|-----------|--------|---------|---------------------------------------|
| Second | Floor | | | | | | | | | | |
| VL/LS | L (Flus | sh) | | | | | | | | | ALACCOD. |
| Label | Descri | ption | Wid | th D | epth | (| Qty | Plies | Pcs | Length | NASCOR |
| F14 | Forex 2.0E-30 | 000Fb LVL | 1. | 75 1 | 1.875 | | 1 | 2 | 2 | 12-0-0 | |
| F13 | Forex 2.0E-30 | 000Fb LVL | 1. | 75 1 ⁻ | 1.875 | | 1 | 2 | 2 | 6-0-0 | Layout Name INDIGO 1 (ELEV.2) |
| F12 | Forex 2.0E-30 | 000Fb LVL | 1. | 75 1 ⁻ | 1.875 | | | | 1 | 6-0-0 | Design Method |
| F11 | Forex 2.0E-30 | 000Fb LVL | 1. | 75 1 | 1.875 | | | | 2 | 4-0-0 | Description |
| F10 | Forex 2.0E-30 | 000Fb LVL | 1. | 75 1 ⁻ | 1.875 | | | | 1 | 2-0-0 | GRANELLI HOME CORP. BRAMPTON, ONT. |
| Joist (| Flush) | | | • | | | | | | | Created |
| Label | Descri | ption | Wid | th D | epth | (| Qty | Plies | Pcs | Length | May 30, 2018 |
| J7 | NJ60U | | 3 | 3.5 1 | 1.875 | | | | 20 | 20-0-0 | Builder |
| J6 | NJ60U | | 3 | 3.5 1 | 1.875 | | | | 11 | 18-0-0 | GREEN YORK HOMES |
| J12 | NJH | | 2 | 2.5 1 | 1.875 | | | | 9 | 18-0-0 | |
| J8 | NJH | | 2 | 2.5 1 | 1.875 | | | | 23 | 16-0-0 | Sales Rep |
| J3 | NJH | | | _ | 1.875 | | | | 14 | 12-0-0 | RM |
| J1 | NJH | | 2 | 2.5 1 | 1.875 | | | | 10 | 4-0-0 | Designer |
| Rim Bo | ard | | | | | | | | | | RCO |
| Label | Descri | | Wid | | epth | (| Qty | Plies | Pcs | Length | Shipping |
| R1 | Norbor | d Rimboard | 1.1 | 25 1 | 1.875 | | | | 15 | 12 | Project |
| | 11.875 | 125 X | | | | | | | | | Builder's Project |
| Blockin | | | | | | | | | | | |
| Label | Descri | ption | Wid | th D | epth | (| Qty | Plies | Pcs | Length | Kott Lumber Company |
| BLK1 | NJH | F | | | 1.875 | | inFt | | Varies | 6-0-0 | 14 Anderson Blvd |
| langer | | | | | | | | | | | Stouffville, Ontario |
| langor | | | | | | | Bea | am/Girder | Sur | ported | Canada |
| | | Member | | | L4A 7X4 | | | | | | |
| Label | Pcs | Description | n | Skev | v Slo | ре | fa | steners | | teners | 905-642-4400 |
| H1 | 2 | HUS1.81/10 |) | | | | | 30 16d | 10 |) 16d | Job Path |
| НЗ | 19 | LT251188 | | | | | 4 1 | 10dx1 1/2 | 2 10 | dx1 1/2 | D:\Users\rochavillo\WORK FROM |
| H5 | 1 | HUCQ1.81/ SDS | 9- | | | | HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS | | | | |
| IOTES: | | | | | | | | | | | \INDIGO 1\INDIGO 1 ELEV.2\FLOOR |

- Framer to verify dimensions on the architectural drawings.
- . Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of
- Refer to Nascor specifier guide for installation works. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- . It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

\INDIGO 1 (ELEV.2).isl Second Floor

Building Code NBCC 2010 / OBC

2012

15

480

360

480

360

360

240

480

360

OSB

Gypsum 1/2"

Design Method

Deflection Joist

Deflection Girder

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceiling:

Deck

Floor

Loads

Live

Dead



NE0618-038 PAGE 34 OF 39

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Address:

Project:

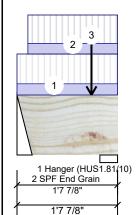
6/1/2018 Designer: RCO

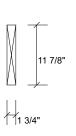
Job Name: INDIGO 1 (ELEV.1)

Project #:

1.750" X 11.875" - PASSED F10-A Forex 2.0E-3000Fb LVL

Level: Second Floor





Member Information

| Type: | Girder | Application: | Floor (Residential) |
|---------------------|--------|----------------|----------------------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBCC 2010 / OBC 2012 |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not Checked |
| Importance: | Normal | Vibration: | Not Checked |
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |
| | | | |

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 121 | 49 | 0 | 0 |
| 2 | 182 | 73 | 0 | 0 |
| | | | | |

Analysis Results

| , | | | | | | |
|---------------|--------------------|----------|---------------|------------|------------|------|
| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
| Moment | 72 ft-lb | 10 7/16" | 17130 ft-lb | 0.004 (0%) | 1.25D+1.5L | L |
| Unbraced | 72 ft-lb | 10 7/16" | 16556 ft-lb | 0.004 (0%) | 1.25D+1.5L | L |
| Shear | 95 lb | 1'2 1/8" | 5798 lb | 0.016 (2%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| LL Defl inch | 0.000 (L/67338) | 10 3/8" | 0.041 (L/360) | 0.010 (1%) | L | L |
| TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| | | | | | | |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|-------------------------|--------|---------|------------|-------|----------|------------|
| 1 - Hanger | 3.000" | 6% | 62 / 182 | 244 | L | 1.25D+1.5L |
| 2 - SPF End Grain | 3.500" | 8% | 91 / 273 | 363 | L | 1.25D+1.5L |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Rottom braced at bearings

| 4 001101 | ii braced at bearings. | | | | | | | | |
|----------|------------------------|-----------------|--------------|----------|--------|--------|-------|-------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 1-7-14 | (Span)3-11-9 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-2-2 to 1-7-14 | (Span)3-9-4 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Point | 1-2-12 | | Far Face | 23 lb | 60 lb | 0 lb | 0 lb | J1 |
| | Self Weight | | | | 5 PLF | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

L. UV. beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Date: Project: Designer:

RCO Job Name: INDIGO 1 (ELEV.1)

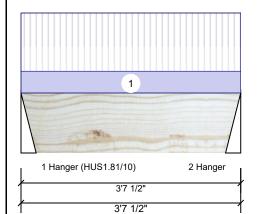
Project #:

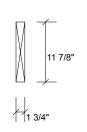
1.750" X 11.875" - PASSED F11-A Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor

6/1/2018





| Member | Information |
|--------|-------------|
| Type: | Girder |
| Plies: | 1 |

Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal

General Load 40 PSF Floor Live: 15 PSF Dead:

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Not Checked Deck:

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 489 | 192 | 0 | 0 |
| 2 | 489 | 192 | 0 | 0 |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. |
|---------------|--------|---------|------------|-------|----------|------------|
| 1 - Hanger | 3.000" | 25% | 240 / 734 | 973 | L | 1.25D+1.5L |
| 2 - Hanger | 3.000" | 25% | 240 / 734 | 973 | L | 1.25D+1.5L |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|------------|------------|---------|
| Moment | 709 ft-lb | 1'9 3/4" | 17130 ft-lb | 0.041 (4%) | 1.25D+1.5L | L |
| Unbraced | 709 ft-lb | 1'9 3/4" | 13201 ft-lb | 0.054 (5%) | 1.25D+1.5L | L |
| Shear | 341 lb | 1'2 1/8" | 5798 lb | 0.059 (6%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.001 (L/29614) | 1'9 3/4" | 0.108 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.003 (L/11600) | 1'9 3/4" | 0.108 (L/360) | 0.030 (3%) | L | L |
| TI Deflinch | 0.005 (L/8335) | 1'9 3/4" | 0.162 (L/240) | 0.030 (3%) | D+L | L |

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

| ID | Load Type | Location Tril | b Width Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|----------------|--------------|---------|---------|-------|-------|----------|
| 1 | Part. Uniform | 0-0-0 to 3-7-8 | Тор | 101 PLF | 270 PLF | 0 PLF | 0 PLF | |
| | Self Weight | | | 5 PLF | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

6/1/2018 Designer: RCO

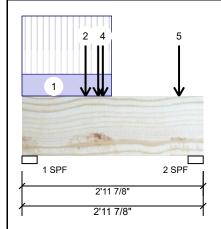
Job Name: INDIGO 1 (ELEV.1)

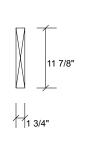
Project #:

1.750" X 11.875" - PASSED F11-B Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor





Member Information

| Туре: | Girder |
|---------------------|--------|
| Plies: | 1 |
| Moisture Condition: | Dry |
| Deflection LL: | 360 |
| Deflection TL: | 240 |
| Importance: | Normal |
| General Load | |
| Floor Live: | 40 PSF |
| Dead: | 15 PSF |
| | |

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: No Deck: Not Checked

Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 468 | 188 | 0 | 0 |
| 2 | 381 | 154 | 0 | 0 |
| | | | | |
| 1 | | | | |

Bearings and Factored Reactions

| Bearing | Length | Cap. Re | act D/L lb | Total | Ld. Case | Ld. Comb. | |
|---------|--------|---------|------------|-------|----------|------------|--|
| 1 - SPF | 3.000" | 29% | 235 / 701 | 936 | L | 1.25D+1.5L | |
| 2 - SPF | 3 000" | 24% | 193 / 571 | 764 | 1 | 1 25D+1 5I | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|-------------|------------|---------|
| Moment | 889 ft-lb | 1'4" | 17130 ft-lb | 0.052 (5%) | 1.25D+1.5L | L |
| Unbraced | 889 ft-lb | 1'4" | 14588 ft-lb | 0.061 (6%) | 1.25D+1.5L | L |
| Shear | 757 lb | 1'9 3/4" | 5798 lb | 0.131 (13%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.001 (L/23259) | 1'4" | 0.087 (L/360) | 0.020 (2%) | D | Uniform |
| LL Defl inch | 0.003 (L/9252) | 1'4" | 0.087 (L/360) | 0.040 (4%) | L | L |
| TL Defl inch | 0.005 (L/6619) | 1'4" | 0.131 (L/240) | 0.040 (4%) | D+L | L |
| | | | | | | |

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Live

Snow

0 PSF

0 lb

0 lb

0 lb

0 lb



Design Notes

ID

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

Load Type

| 1 | Tie-In | 0-0-0 to 1-5-12 | (Span)3-9-4 | Тор | 15 PSF | 40 PSF |
|---|-------------|-----------------|-------------|-----------|--------|--------|
| 2 | Point | 1-0-9 | | Near Face | 23 lb | 60 lb |
| 3 | Point | 1-3-1 | | Far Face | 37 lb | 98 lb |
| 4 | Point | 1-4-0 | | Near Face | 192 lb | 489 lb |
| 5 | Point | 2-7-1 | | Far Face | 34 lb | 90 lb |
| | Self Weight | | | | 5 PLF | |

Location

Trib Width

Side

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Dead

Manufacturer Info

APA: PR-L318



Wind

0 PSF

0 lb J1

0 lb

0 lb F11

0 lb

Comments



NE0618-038 PAGE 37 OF 39

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

Designer: RCO

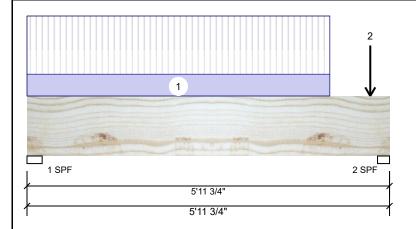
Job Name: INDIGO 1 (ELEV.1)

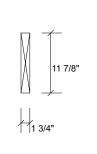
6/1/2018

Project #:

1.750" X 11.875" - PASSED F12-A Forex 2.0E-3000Fb LVL

Level: Second Floor





Member Information

| Type: | Girder | Application: | Floo |
|---------------------|--------|----------------|-------|
| Plies: | 1 | Design Method: | LSD |
| Moisture Condition: | Dry | Building Code: | NBC |
| Deflection LL: | 360 | Load Sharing: | No |
| Deflection TL: | 240 | Deck: | Not 0 |
| Importance: | Normal | Vibration: | Not (|
| General Load | | | |
| Floor Live: | 40 PSF | | |
| Dead: | 15 PSF | | |

or (Residential)

CC 2010 / OBC 2012

Checked Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

| Brg | Live | Dead | Snow | Wind |
|-----|------|------|------|------|
| 1 | 214 | 95 | 0 | 0 |
| 2 | 202 | 90 | 0 | 0 |
| | | | | |

Bearings and Factored Reactions

| Bearing Length | Cap. Re | eact D/L lb | Total | Ld. Case | Ld. Comb. | |
|----------------|---------|-------------|-------|----------|------------|--|
| 1 - SPF 3.000" | 14% | 118 / 322 | 440 | L | 1.25D+1.5L | |
| 2 - SPF 2375" | 16% | 112 / 303 | 415 | 1 | 1 25D+1 5I | |

Analysis Results

| Analysis | Case |
|--------------|---------|
| Moment | L |
| Unbraced | L |
| Shear | L |
| Perm Defl in | Uniform |
| LL Defl inch | L |
| TL Defl inch | L |
| TL Defl inch | |

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|---------------|------------------|------------|----------|--------|--------|-------|-------|----------|
| 1 | Part. Uniform | 0-0-0 to 4-11-15 | | Far Face | 27 PLF | 72 PLF | 0 PLF | 0 PLF | |
| 2 | Point | 5-7-15 | | Far Face | 21 lb | 57 lb | 0 lb | 0 lb | J1 |
| | Self Weight | | | | 5 PLF | | | | |

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318





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EWP Studio

Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

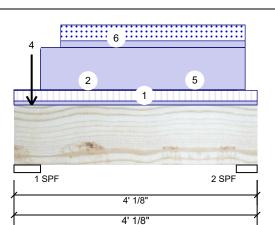
6/1/2018 Designer: RCO

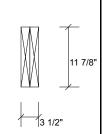
Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED Level: Second Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F13-A

Address:





| nation | | | Unfactored Reactions UNPATTERNED lb (Uplift) | | | | | | |
|--------|----------------------------------|---|--|--|---|--|--|---|---|
| Girder | Application: | Floor (Residential) | Brg | Live | Dea | d | Snov | V | Wind |
| 2 | Design Method: | LSD | 1 | 33 | 15 | 8 | 3 | 1 | 0 |
| Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 32 | 16 | 2 | 4: | 2 | 0 |
| 360 | Load Sharing: | No | | | | | | | |
| 240 | Deck: | Not Checked | | | | | | | |
| Normal | Vibration: | Not Checked | | | | | | | |
| | | | | | | | | | |
| 40 PSF | | | Bearings a | and Facto | ored React | ions | | | |
| 15 PSF | | | Bearing Lo | ength | Cap. Reac | D/L lb | Total | Ld. Case | Ld. Comb. |
| | | | 1 - SPF 5. | .250" | 3% | 98 / 50 | 248 | L | 1.25D+1.5L |
| | | | 2 - SPF 4. | .125" | 4% 2 | 203 / 63 | 265 | L | 1.25D+1.5S |
| | 2 Dry 360 240 Normal | Design Method: Building Code: | Design Method: LSD Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Normal Vibration: Not Checked | Application: Floor (Residential) Brg 1 | Application: Floor (Residential) Brg Live | Application: Floor (Residential) Brg Live Deal | Application: Floor (Residential) Brg Live Dead | Application: Floor (Residential) Brg Live Dead Snow | Application: Floor (Residential) Brg Live Dead Snow |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|-----------|---------------|------------|------------|---------|
| Moment | 208 ft-lb | 2' 3/4" | 23297 ft-lb | 0.009 (1%) | 1.25D+1.5S | L |
| Unbraced | 208 ft-lb | 2' 3/4" | 23297 ft-lb | 0.009 (1%) | 1.25D+1.5S | L |
| Shear | 104 lb | 1'4 3/8" | 7885 lb | 0.013 (1%) | 1.25D+1.5S | L |
| Perm Defl in. | 0.001 (L/65472) | 2' 11/16" | 0.112 (L/360) | 0.010 (1%) | D | Uniform |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

| 7 Lateral slen | 7 Lateral slenderness ratio based on full section width. | | | | | | | | |
|----------------|--|-----------------|--------------|------|--------|--------|--------|-------|------------------|
| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
| 1 | Tie-In | 0-0-0 to 4-0-2 | (Span)0-9-11 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Part. Uniform | 0-0-0 to 3-8-2 | | Тор | 1 PLF | 0 PLF | 0 PLF | 0 PLF | |
| 4 | Point | 0-3-8 | | Тор | 8 lb | 0 lb | 0 lb | 0 lb | Wall Self Weight |
| 5 | Part. Uniform | 0-5-4 to 3-9-12 | | Тор | 64 PLF | 0 PLF | 0 PLF | 0 PLF | Wall Self Weight |
| 6 | Part. Uniform | 0-9-4 to 3-9-12 | | Тор | 10 PLF | 0 PLF | 24 PLF | 0 PLF | |
| | Self Weight | | | | 10 PLF | | | | |

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info





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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

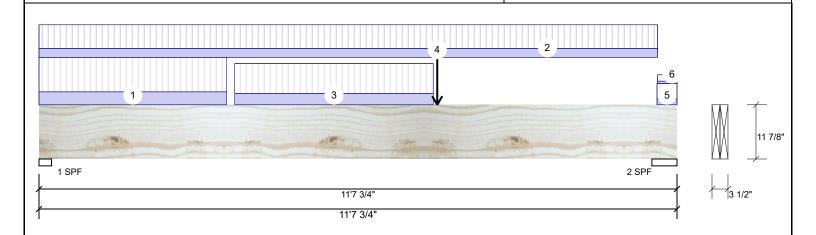
Date: 6/1/2018 Designer: RCO

Job Name: INDIGO 1 (ELEV.1)

Project #:

2-Ply - PASSED Level: Second Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL F14-A

Address:



| Member Info | rmation | | | Unfactore | d Reacti | ons UNPATTERN | ED lb (Uplift) | :) | |
|------------------|---------|----------------|----------------------|------------|-----------|-------------------|----------------|------------|--|
| Type: | Girder | Application: | Floor (Residential) | Brg | Live | Dead | Snow | Wind | |
| Plies: | 2 | Design Method: | LSD | 1 | 313 | 173 | 0 | 0 | |
| Moisture Conditi | on: Dry | Building Code: | NBCC 2010 / OBC 2012 | 2 | 268 | 159 | 0 | 0 | |
| Deflection LL: | 360 | Load Sharing: | No | | | | | | |
| Deflection TL: | 240 | Deck: | Not Checked | | | | | | |
| Importance: | Normal | Vibration: | Not Checked | | | | | | |
| General Load | | | | | | | | | |
| Floor Live: | 40 PSF | | | Bearings a | and Facto | ored Reactions | | | |
| Dead: | 15 PSF | | | Bearing L | ength | Cap. React D/L lb | Total Ld. Case | Ld. Comb. | |
| | | | | 1 - SPF 2 | .750" | 12% 216 / 470 | 686 L | 1.25D+1.5L | |
| | | | | 2 - SPF 5 | .500" | 5% 199 / 402 | 601 L | 1.25D+1.5L | |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|---------------|--------------------|----------|---------------|------------|------------|---------|
| Moment | 1948 ft-lb | 6'1 1/2" | 34261 ft-lb | 0.057 (6%) | 1.25D+1.5L | L |
| Unbraced | 1948 ft-lb | 6'1 1/2" | 28546 ft-lb | 0.068 (7%) | 1.25D+1.5L | L |
| Shear | 551 lb | 1'1 7/8" | 11596 lb | 0.048 (5%) | 1.25D+1.5L | L |
| Perm Defl in. | 0.012 (L/10848) | 5'9 1/8" | 0.369 (L/360) | 0.030 (3%) | D | Uniform |
| LL Defl inch | 0.022 (L/5964) | 5'9 1/4" | 0.369 (L/360) | 0.060 (6%) | L | L |
| TI Deflinch | 0.035 (L/3848) | 5'9 1/4" | 0.554 (L/240) | 0.060 (6%) | D+L | L |

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- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

| ID | Load Type | Location | Trib Width | Side | Dead | Live | Snow | Wind | Comments |
|----|-------------|-------------------|-------------|----------|--------|--------|-------|-------|----------|
| 1 | Tie-In | 0-0-0 to 3-5-2 | (Span)1-7-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 2 | Tie-In | 0-0-0 to 11-3-8 | (Span)1-1-0 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 3 | Tie-In | 3-6-14 to 7-2-6 | (Span)1-4-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 4 | Point | 7-3-4 | | Far Face | 49 lb | 121 lb | 0 lb | 0 lb | F10 |
| 5 | Tie-In | 11-3-6 to 11-7-12 | (Span)0-8-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| 6 | Tie-In | 11-3-8 to 11-7-12 | (Span)0-3-8 | Тор | 15 PSF | 40 PSF | 0 PSF | 0 PSF | |
| | Self Weight | | | | 10 PLF | | | | |

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