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Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

GREEN YORK HOMES-GRANELLI HOME CORP-LIANA 1 (ELEV.2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

<u>CODE</u>

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.

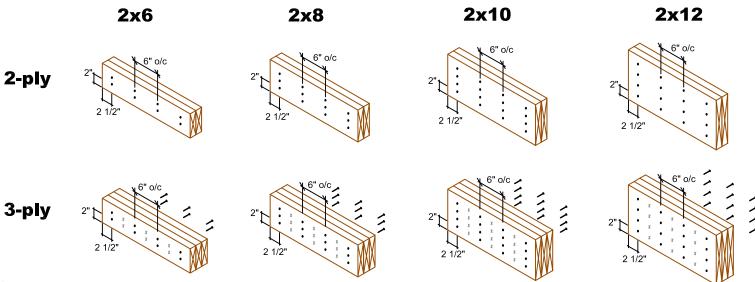


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MULTIPLE MEMBER CONNECTIONS

GREEN YORK HOMES-GRANELLI HOME CORP-LIANA 1 (ELEV.2

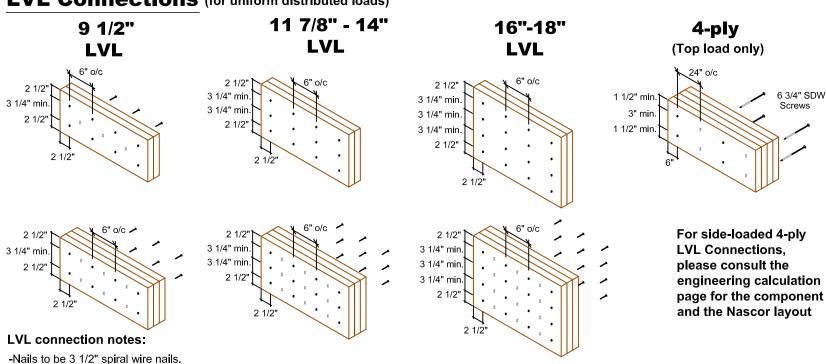
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

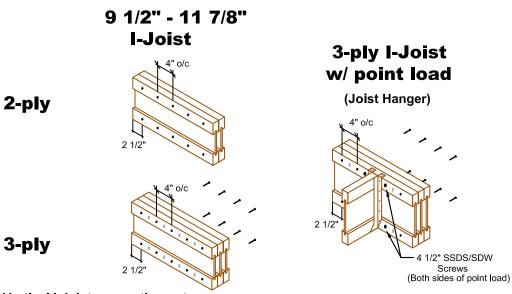
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

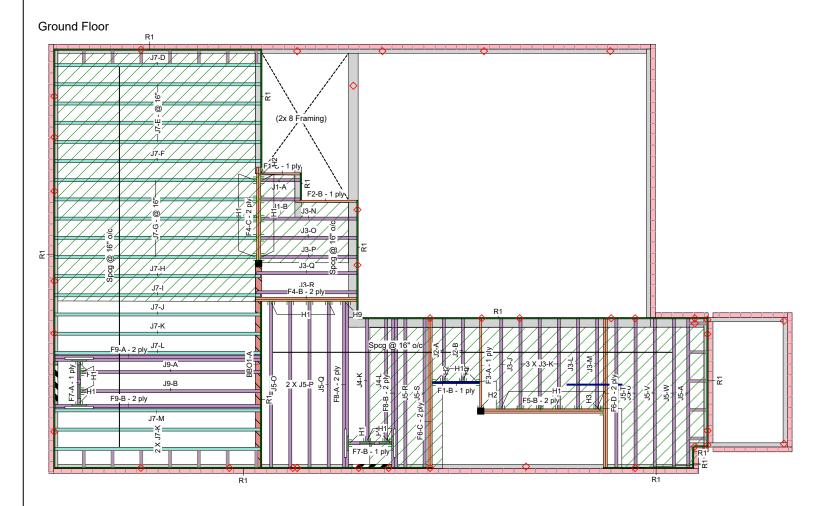
- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI -PLY CONNECTION DETAILS

> Date: November 30, 2016 Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751 NE0618-020 **PAGE 3 OF 41**



THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

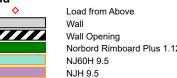




Wall Opening Norbord Rimboard Plus 1.125 X 9.5 NJ60H 9.5 NJH 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Legend



2. Nascor CCMC - 13535-R

امط	December
/LSI	L (Flush)
unu	1 1001

LVL/LS	L (Flush)							
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	
F6	Forex 2.0E-3000Fb LVL	1.75	9.5	2	2	4	12-0-0	Ļ
F5	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	10-0-0] -
F4	Forex 2.0E-3000Fb LVL	1.75	9.5	2	2	4	8-0-0	С
F3	Forex 2.0E-3000Fb LVL	1.75	9.5			1	8-0-0	Г
F2	Forex 2.0E-3000Fb LVL	1.75	9.5			1	6-0-0	
F1	Forex 2.0E-3000Fb LVL	1.75	9.5			2	4-0-0	C
I Joist (Flush)							Ļ
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	В
J7	NJ60H	2.5	9.5			19	16-0-0	
F9	NJH	2.5	9.5	2	2	4	16-0-0	S
F8	NJH	2.5	9.5	2	2	4	12-0-0	
F7	NJH	2.5	9.5			2	4-0-0	Г
J9	NJH	2.5	9.5			2	14-0-0] -
J5	NJH	2.5	9.5			11	12-0-0	_
J4	NJH	2.5	9.5			2	10-0-0	S
J3	NJH	2.5	9.5			11	8-0-0	P
J2	NJH	2.5	9.5			2	6-0-0	В
J1	NJH	2.5	9.5			2	4-0-0	
Rim Bo	ard							
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	
R1	Norbord Rimboard Plus 1.125 X 9.5	1.125	9.5			12	12	

Hanger

					Beam/Girder	Supported
						Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
H1	30	LT259			4 10dx1 1/2	2 10dx1 1/2
H2	4	HUS1.81/10			30 16d	10 16d
H3	1	HGUS410			46 16d	16 16d
H9	1	THAI-2 (Min)			2 10d	2 10dx1 1/2

- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger.
- Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- . Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018 Project No: 17-55

Model: Liana 1

12-0-0	
10-0-0	Layout Name
10-0 0	LIANA 1 (ELEV.2)
8-0-0	Design Method
8-0-0	LSD
8-0-0	Description
6-0-0	GRANELLI HOME CORP. BRAMPTON, ONT.
4-0-0	Created
	May 29, 2018
	Builder
ength	GREEN YORK HOMES
16-0-0	
16-0-0	Sales Rep
12-0-0	RM
4-0-0	Designer
14-0-0	RCO
12-0-0	
10-0-0	Shipping
8-0-0	Project
6-0-0	Builder's Project
4-0-0	Kott Lumber Company

14 Anderson Blvd Stouffville, Ontario

Canada L4A 7X4 905-642-4400

Job Path

D:\LIsers\rochavillo\WORK FROM HOME\GREEN YORK HOMES \GRANELLI HOME CORP\MODELS \LIANA 1\LIANA 1 ELEV 2\FLOOR \LIANA 1 (ELEV.2).isl **Ground Floor**

Design Method

LSD Building Code NBCC 2010 / OBC 2012 Floor

Loads Live Dead 15 **Deflection Joist** 480 LL Span L/ 360 TL Span L/ LL Cant 2L/ 480 360 TL Cant 2L/ **Deflection Girder** 360 LL Span L/ 240 TL Span L/ 480 LL Cant 2L/ TL Cant 2L/ 360 Decking OSB Deck Thickness 3/4" Fastener Nailed & Glued Vibration



NE0618-020 **PAGE 4 OF 41**

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES** Project:

Designer: RCO

Job Name: LIANA 1 (ELEV.2)

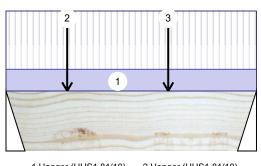
5/31/2018

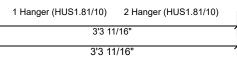
Level: Ground Floor

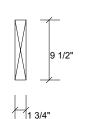
Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F1-B

Address:







Wind

Member Info	ormation			Unfacto	red Reac	tions (JNPATTERN	ED lb (Upl
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow
Plies:	1	Design Method:	LSD	1	171		80	0
Moisture Condit	tion: Dry	Building Code:	NBCC 2010 / OBC 2012	2	152		73	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions	
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total Ld.
				1 4	2 000"	00/	100 / 256	256

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	258 ft-lb	2' 7/16"	11362 ft-lb	0.023 (2%)	1.25D+1.5L	L
Unbraced	258 ft-lb	2' 7/16"	10006 ft-lb	0.026 (3%)	1.25D+1.5L	L
Shear	274 lb	11 3/4"	4638 lb	0.059 (6%)	1.25D+1.5L	L
Perm Defl in	. 0.001 (L/45769)	1'9 1/4"	0.098 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/21849)	1'9"	0.098 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.002 (L/14790)	1'9 1/16"	0.146 (L/240)	0.020 (2%)	D+L	L

(L/14/90) **Design Notes**

1 Fill all hanger nailing holes. 2 Girders are designed to be supported on the bottom edge only.

3 Top braced at bearings.

4 Bottom braced at bearings.

Unfactored	Reactions	UNPAI	I EKNED I	(Uplift)

1 2	171	80	0	0
2	152	73	0	0

Bearing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 -	3.000"	9%	100 / 256	356	L	1.25D+1.5L
Hanger						
2 -	3.000"	8%	91 / 229	320	L	1.25D+1.5L
Hanger						

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-3-11		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
2	Point	0-9-11		Far Face	41 lb	90 lb	0 lb	0 lb	J2
3	Point	2-1-11		Far Face	50 lb	101 lb	0 lb	0 lb	J2
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







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Client: **GREEN YORK HOMES** Project:

Designer:

RCO Job Name: LIANA 1 (ELEV.2)

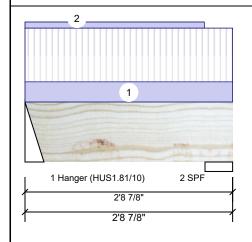
Project #:

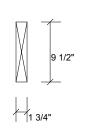
1.750" X 9.500" - PASSED F1-C Forex 2.0E-3000Fb LVL

Address:

Level: Ground Floor

5/31/2018





Member Inform	nation
Type:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Not Checked Deck: Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Wind Live Dead Brg 12 0 11 O 1 2 13 11 0 0

Floor Live: 40 PSF 15 PSF Dead:

Bearings and Factored Reactions

Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.	
•	3.000"	1%	13 / 17	31	L	1.25D+1.5L	
Hanger							
2 - SPF	4.375"	1%	14 / 19	33	L	1.25D+1.5L	

Analysis Results Analysis Actual Case Location Allowed Capacity Comb. 1'3 3/4" 11362 ft-lb Moment 15 ft-lb 0.001 (0%) 1.25D+1.5L L Unbraced 15 ft-lb 1'3 3/4" 10562 ft-lb 0.001 (0%) 1.25D+1.5L L 1'7 3/4" 4638 lb Shear 0.002 (0%) 1.25D+1.5L L 8 lb Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-8-14	(Span)0-5-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 2-4-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 **PAGE 6 OF 41**

Brg

2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

5/31/2018 Designer: RCO

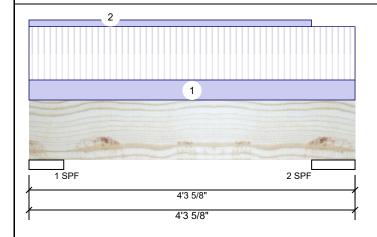
Job Name: LIANA 1 (ELEV.2)

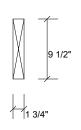
Project #:

1.750" X 9.500" - PASSED F2-B Forex 2.0E-3000Fb LVL

Address:

Level: Ground Floor





Wind

0

0

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

2	53	33	0	0

Bearings and Factored Reactions

Live

50

Bearing	Length	Cap. Read	t D/L lb	Iotal	Ld. Case	Ld. Comb.	
1 - SPF	5.500"	2%	41 / 76	117	L	1.25D+1.5L	
2 - SPF	6 875"	2%	42 / 80	121	1	1 25D+1 5I	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	81 ft-lb	2'1 1/8"	11362 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	81 ft-lb	2'1 1/8"	9540 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	51 lb	3'	4638 lb	0.011 (1%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/46449)	2'1 3/16"	0.170 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-3-10	(Span)1-2-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-12		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 **PAGE 7 OF 41**



Client:

Project: Address:

GREEN YORK HOMES

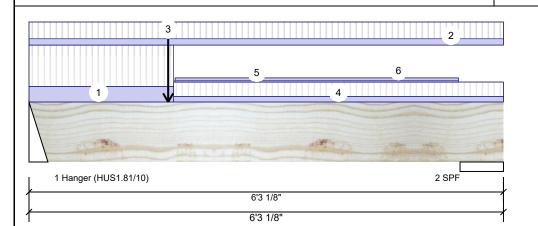
5/31/2018 Designer: RCO

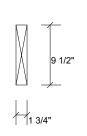
Job Name: LIANA 1 (ELEV.2)

Level: Ground Floor

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F3-A





Wind

Member Information Application: Floor (Residential) Type: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live: 15 PSF Dead:

Unfactored Reactions UNPATTERNED Ib (Uplift)

1	340	159	0	0
2	233	122	0	0

Bearings and Factored Reactions

Bearing Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - 3.000"	18%	199 / 511	710	L	1.25D+1.5L
Hanger					
2 - SPF 6.875"	7%	152 / 349	501	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	839 ft-lb	1'11 7/8"	11362 ft-lb	0.074 (7%)	1.25D+1.5L	L
Unbraced	839 ft-lb	1'11 7/8"	6701 ft-lb	0.125 (13%)	1.25D+1.5L	L
Shear	514 lb	11 3/4"	4638 lb	0.111 (11%)	1.25D+1.5L	L
Perm Defl in.	0.006 (L/11950)	2'9 1/8"	0.185 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.011 (L/6054)	2'8 9/16"	0.185 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.017 (L/4019)	2'8 13/16"	0.278 (L/240)	0.060 (6%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-10-15	(Span)3-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 6-3-2	(Span)1-5-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-10-1		Far Face	73 lb	152 lb	0 lb	0 lb	F1
4	Tie-In	1-10-15 to 6-3-2	(Span)1-2-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Part. Uniform	1-11-2 to 5-8-0		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
6	Part. Uniform	1-11-2 to 5-8-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Self Weight

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

4 PLF

Manufacturer Info

APA: PR-L318







NE0618-020 **PAGE 8 OF 41**



Client: **GREEN YORK HOMES**

Project: Address:

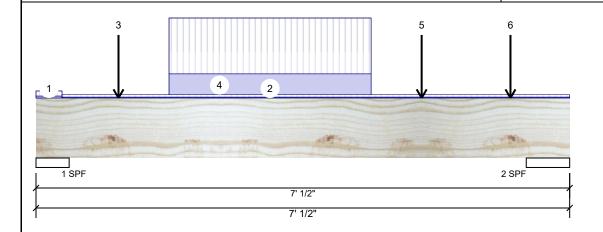
5/31/2018 Designer: RCO

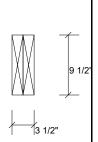
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F4-B

Level: Ground Floor





Wind

Page 1 of 1

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

1	669	277	0	0	
2	862	351	0	0	

Dead

Bearings and Factored Reactions

Live

Brg

Bearing Length	Cap. React D/L II	b lotal Ld. Case	La. Comb.
1 - SPF 5.250"	12% 346 / 100	4 1350 L	1.25D+1.5L
2 CDE 6.975"	12% /30 / 120	3 1732 I	1 25D±1 5I

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2221 ft-lb	3'5 13/16"	22724 ft-lb	0.098 (10%)	1.25D+1.5L	L
Unbraced	2221 ft-lb	3'5 13/16"	21975 ft-lb	0.101 (10%)	1.25D+1.5L	L
Shear	1694 lb	5'8 7/8"	9277 lb	0.183 (18%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/9580)	3'5 5/8"	0.205 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.019 (L/3901)	3'5 5/8"	0.205 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.027 (L/2772)	3'5 5/8"	0.308 (L/240)	0.090 (9%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)1-0-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-4-2 to 7-0-8	(Span)0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-1-1		Near Face	88 lb	234 lb	0 lb	0 lb	J5
4	Part. Uniform	1-9-1 to 4-5-1		Near Face	86 PLF	230 PLF	0 PLF	0 PLF	
5	Point	5-1-1		Near Face	108 lb	288 lb	0 lb	0 lb	J5
6	Point	6-3-2		Near Face	122 lb	323 lb	0 lb	0 lb	F8
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







PAGE 9 OF 41 NE0618-020



EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

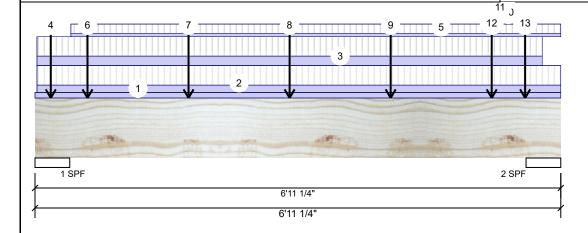
5/31/2018 Designer: RCO

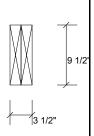
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" 2-Ply - PASSED F4-C Forex 2.0E-3000Fb LVL

Level: Ground Floor





Page 1 of 2

Member Information

Type:	Girder	Application:
Plies:	2	Design Metho
Moisture Condition:	Dry	Building Code
Deflection LL:	360	Load Sharing
Deflection TL:	240	Deck:
Importance:	Normal	Vibration:
General Load		
Floor Live:	40 PSF	

15 PSF

Floor (Residential) od: NBCC 2010 / OBC 2012 de:

g: No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	5607	2698	0	0
2	2555	1390	0	0

Bearings and Factored Reactions

Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
1 - SPF 5.500"	99% 3372 / 8411	11783 L	1.25D+1.5L	
2-SPF 5500"	47% 1737 / 3833	5570 L	1.25D+1.5L	

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7859 ft-lb	3'4 11/16"	22724 ft-lb	0.346 (35%)	1.25D+1.5L	L
Unbraced	7859 ft-lb	3'4 11/16"	21978 ft-lb	0.358 (36%)	1.25D+1.5L	L
Shear	4616 lb	1'2 1/4"	9277 lb	0.498 (50%)	1.25D+1.5L	L
Perm Defl in.	0.033 (L/2213)	3'5 7/16"	0.205 (L/360)	0.160 (16%)	D	Uniform
LL Defl inch	0.062 (L/1195)	3'5 3/8"	0.205 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.095 (L/776)	3'5 3/8"	0.307 (L/240)	0.310 (31%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID Load Type Location Trib Width Side Dead Live Snow Wind Comments Part. Uniform 0-0-0 to 6-11-4 80 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight Top 0 PLF 283 PLF 2 Part. Uniform 0-0-5 to 6-11-4 106 PLF 0 PLF Top Far Face 280 PLF 0 PLF 0 PLF Part. Uniform 0-0-5 to 6-8-5 136 PLF 3 Point 0-2-8 Top 1254 lb 2945 lb 0 lb 0 lb F11 F11 50 PLF 133 PLF 0 PLF 0 PLF 5 Part. Uniform 0-5-11 to 6-11-4 Top J3

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
 - Damaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 10 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Designer:

RCO Job Name: LIANA 1 (ELEV.2)

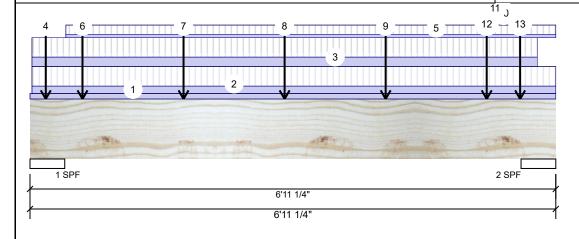
Project #:

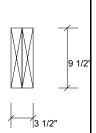
1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F4-C

Address:

Level: Ground Floor

5/31/2018





Page 2 of 2

ŀ	Continued from page 1									
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	6	Point	0-8-5		Near Face	73 lb	154 lb	0 lb	0 lb	J3
	7	Point	2-0-5		Near Face	82 lb	167 lb	0 lb	0 lb	J3
	8	Point	3-4-5		Near Face	29 lb	59 lb	0 lb	0 lb	J3
	9	Point	4-8-5		Near Face	32 lb	68 lb	0 lb	0 lb	J1
	10	Tie-In	6-0-5 to 6-6-8	(Span)2-11-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	11	Part. Uniform	6-0-5 to 6-5-1		Тор	7 PLF	0 PLF	0 PLF	0 PLF	
	12	Point	6-0-5		Near Face	22 lb	45 lb	0 lb	0 lb	J1
	13	Point	6-5-10		Near Face	11 lb	12 lb	0 lb	0 lb	F1
		Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 11 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

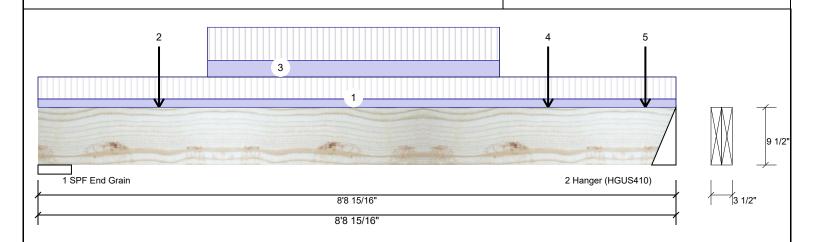
Date: 5/31/2018 Designer: RCO Project: Address:

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F5-B

Level: Ground Floor



Member Info	rmation			Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	2	Design Method:	LSD	1	795		377		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1170		525		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF	5.500"	12%	471 / 1192	1663	L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3658 ft-lb	4'7 3/4"	22724 ft-lb	0.161 (16%)	1.25D+1.5L	L
Unbraced	3658 ft-lb	4'7 3/4"	21435 ft-lb	0.171 (17%)	1.25D+1.5L	L
Shear	2240 lb	7'8 3/16"	9277 lb	0.241 (24%)	1.25D+1.5L	L
Perm Defl in.	0.022 (L/4332)	4'5 15/16"	0.269 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.048 (L/2031)	4'6 3/16"	0.269 (L/360)	0.180 (18%)	L	L
TL Defl inch	0.070 (L/1383)	4'6 1/16"	0.404 (L/240)	0.170 (17%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

Brg	Live	Dead	Snow	Wind
1	795	377	0	0
2	1170	525	0	0

4.000"

2 -

. lb Total Ld. Case Ld. Comb. 1.25D+1.5L 92 1663 L End Grain

656 / 1755

2411 L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

23%

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



1.25D+1.5L

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-8-15	(Span)3-10-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-7-14		Far Face	78 lb	162 lb	0 lb	0 lb	J3
3	Part. Uniform	2-3-14 to 6-3-14		Far Face	57 PLF	117 PLF	0 PLF	0 PLF	
4	Point	6-11-14		Far Face	162 lb	386 lb	0 lb	0 lb	J3
5	Point	8-3-14		Far Face	114 lb	275 lb	0 lb	0 lb	J3
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 12 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Date: 5/31/2018 Designer: RCO

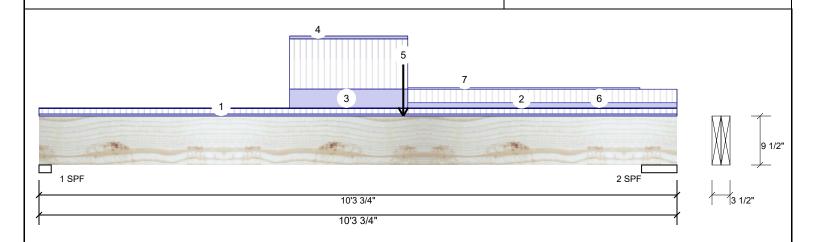
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F6-C

Address:

Level: Ground Floor



Member Infor	rmation			Unfactore	ed Reacti	ons UN	PATTERNI	ED lb (U	plift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow		Wind
Plies:	2	Design Method:	LSD	1	188		125	0		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	279		169	0		0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Facto	ored Re	eactions			
Dead:	15 PSF			Bearing L	ength	Cap. R	eact D/L lb	Total L	d. Case	Ld. Comb.
				1-SPF 2	2.375"	9%	156 / 282	439 L		1.25D+1.5L
				2-SPF 6	3.875"	4%	212 / 418	629 L		1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1837 ft-lb	5'10 11/16"	22724 ft-lb	0.081 (8%)	1.25D+1.5L	L
Unbraced	1837 ft-lb	5'10 11/16"	20878 ft-lb	0.088 (9%)	1.25D+1.5L	L
Shear	546 lb	9' 1/8"	9277 lb	0.059 (6%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/7265)	5'2 1/8"	0.322 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.027 (L/4220)	5'2 11/16"	0.322 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.043 (L/2670)	5'2 7/16"	0.483 (L/240)	0.090 (9%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 1 of 2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 9-8-9		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Tie-In	4-0-10 to 5-11-9	(Span)3-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	4-0-10 to 5-11-9		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
5	Point	5-10-11		Near Face	80 lb	171 lb	0 lb	0 lb	F1
6	Tie-In	5-11-9 to 10-3-12	(Span)0-11-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 13 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Date: 5/31/2018 Project: Designer: RCO

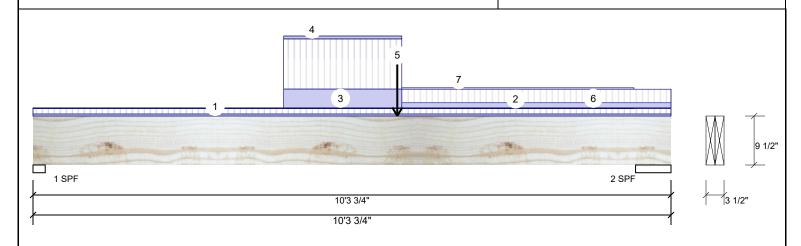
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F6-C

Address:

Level: Ground Floor



.Continued from page 1

ID Load Type Location Trib Width Side Wind Comments Dead Live Snow Part. Uniform 5-11-9 to 9-8-9 Тор 2 PLF 0 PLF 0 PLF 0 PLF 7

> Self Weight 8 PLF

> > READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 2 of 2

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 14 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Designer:

5/31/2018 RCO

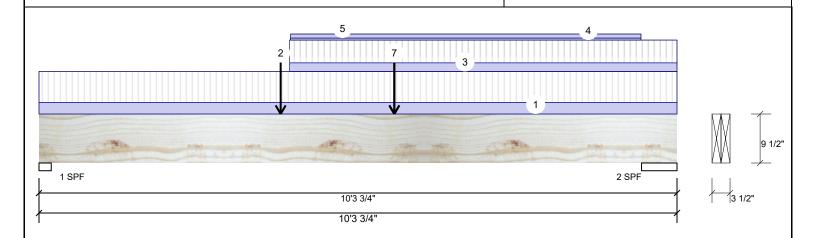
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F6-D

Address:

Level: Ground Floor



Member Info	rmation			Unfactore	ed Reacti	ons UNPATTERNI	D lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	876	424	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	683	342	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	_ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	36% 530 / 1315	1845 L	1.25D+1.5L
				2-SPF 6	3.875"	10% 428 / 1024	1452 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6654 ft-lb	3'10 7/8"	22724 ft-lb	0.293 (29%)	1.25D+1.5L	L
Unbraced	6654 ft-lb	3'10 7/8"	20878 ft-lb	0.319 (32%)	1.25D+1.5L	L
Shear	1808 lb	11 1/8"	9277 lb	0.195 (19%)	1.25D+1.5L	L
Perm Defl in.	0.048 (L/2440)	4'7 3/4"	0.322 (L/360)	0.150 (15%)	D	Uniform
LL Defl inch	0.100 (L/1158)	4'7 9/16"	0.322 (L/360)	0.310 (31%)	L	L
TL Defl inch	0.148 (L/785)	4'7 11/16"	0.483 (L/240)	0.310 (31%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-9-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	3-10-14		Far Face	525 lb	1170 lb	0 lb	0 lb	F5
3	Tie-In	4-0-10 to 10-3-12	(Span)0-6-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	4-0-13 to 9-8-12		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
5	Part. Uniform	4-0-13 to 9-8-12		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
6	Point	5-8-15		Тор	25 lb	68 lb	0 lb	0 lb	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 15 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Date: 5/31/2018 Designer: RCO

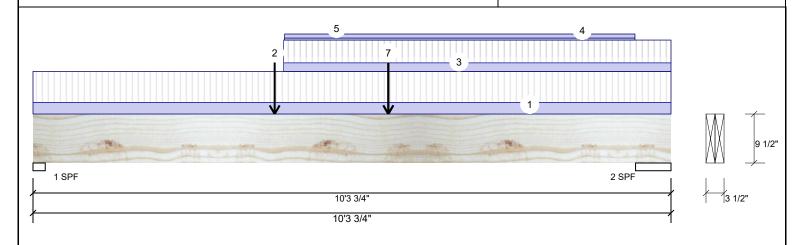
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F6-D

Address:

Level: Ground Floor



.Continued from page 1

ID Location Trib Width Side Wind Comments Load Type Dead Live Snow Point 5-8-15 Тор 35 lb 92 lb 0 lb 7 Self Weight 8 PLF

> READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 2 of 2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 16 OF 41



Client: **GREEN YORK HOMES**

Project:

Address:

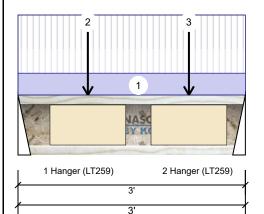
5/31/2018 Date: Designer: RCO

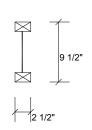
Job Name: LIANA 1 (ELEV.2)

Level: Ground Floor

Project #:

9.500" - PASSED F7-A NJH





Member Information Application: Type: Floor (Residential) Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live:

Unfactored Reactions UNPATTERNED Ib (Uplift) Wind Brg Live Dead 331 0 124 0 1 2 358 134 0 0

Dead: 15 PSF Analysis Results

Bearings and Factored Reactions

Bearing	Length	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	41%	155 / 497	652	L	1.25D+1.5L
2 - Hanger	2.000"	45%	168 / 537	705	L	1.25D+1.5L

Analysis Actual Location Allowed Comb. Case Capacity 11 1/16" 3830 ft-lb 0.132 (13%) 1.25D+1.5L L Moment 505 ft-lb Unbraced 505 ft-lb 11 1/16" 3411 ft-lb 0.148 (15%) 1.25D+1.5L L 0.442 (44%) 1.25D+1.5L L Shear 698 lh 2'10 3/4" 1580 lb Perm Defl in. 0.003 1'3 15/16" 0.093 (L/360) 0.030 (3%) D Uniform (L/12051) LL Defl inch 0.007 (L/4523) 1'3 15/16" 0.093 (L/360) 0.080 (8%) L L TL Defl inch 0.010 (L/3288) 1'3 15/16" 0.140 (L/240) 0.070 (7%) D+L L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

+ Dottom hange	bracca at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-11-1		Near Face	114 lb	304 lb	0 lb	0 lb	J9	
3	Point	2-3-1		Near Face	106 lb	282 lb	0 lb	0 lb	J9	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 Libel flagges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, suffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-020 PAGE 17 OF 41

2



Client: **GREEN YORK HOMES** Project:

Address:

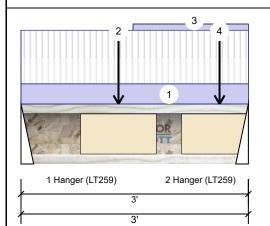
5/31/2018 Designer: RCO

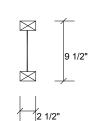
Job Name: LIANA 1 (ELEV.2)

Level: Ground Floor

Project #:

9.500" - PASSED F7-B NJH





Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead

Brg	Live	Dead	Snow	Wind
1	200	82	0	0
2	290	131	0	0

Analysis Results

15 PSF

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	420 ft-lb	1'3 7/16"	3830 ft-lb	0.110 (11%)	1.25D+1.5L	L
Unbraced	420 ft-lb	1'3 7/16"	3411 ft-lb	0.123 (12%)	1.25D+1.5L	L
Shear	591 lb	2'10 3/4"	1580 lb	0.374 (37%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/13745)	1'3 7/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.006 (L/5760)	1'3 7/16"	0.093 (L/360)	0.060 (6%)	L	L
TL Defl inch	0.008 (L/4059)	1'3 7/16"	0.140 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	26%	103 / 300	403	L	1.25D+1.5L
2 - Hanger	2.000"	38%	164 / 435	599	L	1.25D+1.5L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-3-7		Far Face	94 lb	230 lb	0 lb	0 lb	J4
3	Part. Uniform	1-5-12 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
4	Point	2-7-7		Far Face	75 lb	156 lb	0 lb	0 lb	J4

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. Noist flanges must not be out or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid

lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum
point load bearing length ≥ 3.5 inches
7. For flat roofs provide proper drainage to prevent
ponding

Manufacturer Info

Nascor by Kott







PAGE 18 OF 41 NE0618-020

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

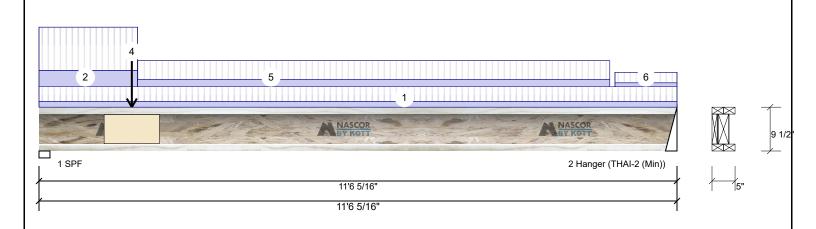
5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED 9.500" F8-A NJH

Level: Ground Floor



Member Infor	mation			Unfactored Reactions UNPATTERNED lb (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	2	Design Method:	LSD	1	543		210		0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	323		122		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings a	and Fact	ored F	Reactions		
Dead:	15 PSF			Bearing L	ength.	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF 2	.375"	34%	262 / 814	1076	L

Analysis Results

,						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2033 ft-lb	5'1 9/16"	7660 ft-lb	0.265 (27%)	1.25D+1.5L	L
Unbraced	2033 ft-lb	5'1 9/16"	2038 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1052 lb	1 5/8"	3160 lb	0.333 (33%)	1.25D+1.5L	L
Perm Defl in.	0.029 (L/4604)	5'6 1/2"	0.375 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.077 (L/1754)	5'6 5/8"	0.375 (L/360)	0.210 (21%)	L	L
TL Defl inch	0.106 (L/1270)	5'6 5/8"	0.562 (L/240)	0.190 (19%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 7'10" o.c.

6 Bottom flange braced at bearings.

2 -2.500" 20% 153 / 484 637 L Hanger

READ ALL NOTES ON THIS PAGE AND ON

ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Wind 0 0

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-6-5	(Span)1-2-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	1-8-2		Near Face	82 lb	200 lb	0 lb	0 lb	F7
5	Tie-In	1-9-6 to 10-3-12	(Span)1-5-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	10-4-14 to 11-6-5	(Span)0-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-020 PAGE 19 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

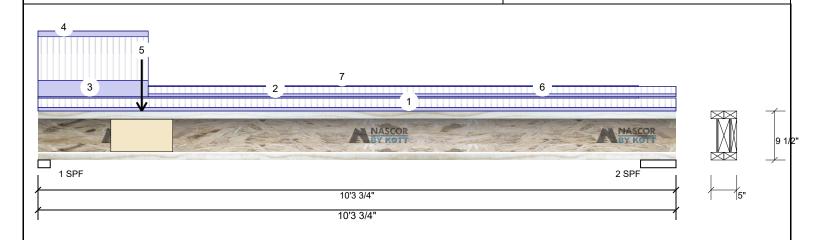
Date: 5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED F8-B NJH 9.500"

Level: Ground Floor



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Type: Application: Floor (Residential) Brg Live Dead Plies: Design Method: 469 223 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 197 93 0 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case 278 / 704

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1313 ft-lb	2'11 5/8"	7660 ft-lb	0.171 (17%)	1.25D+1.5L	L
Unbraced	1313 ft-lb	2'11 5/8"	1322 ft-lb	0.993 (99%)	1.25D+1.5L	L
Shear	958 lb	1 5/8"	3160 lb	0.303 (30%)	1.25D+1.5L	L
Perm Defl in.	0.017 (L/6969)	4'6"	0.322 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.035 (L/3306)	4'5 7/8"	0.322 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.052 (L/2242)	4'5 7/8"	0.483 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'6" o.c.

5 Bottom flange braced at bearings

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

31%

13%

116 / 295

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

1 - SPF 2.375"

2 - SPF 6.875"



Wind

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

982 I

411 L

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-3-12	(Span)0-8-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 9-8-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
3	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	0-0-0 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	131 lb	290 lb	0 lb	0 lb	F7
6	Tie-In	1-9-6 to 10-3-12	(Span)0-7-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 9-8-8		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation
 Libel flagges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, suffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details
 Damaged IJoists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







NE0618-020 PAGE 20 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

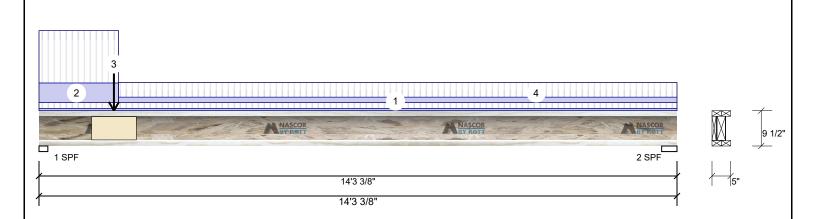
Date: 5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED **NJH** 9.500" F9-A

Level: Ground Floor



Member Info	rmation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	[Dead	Snow		Wind
Plies:	2	Design Method:	LSD	1	590		221	0		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	237		89	0		0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Fact	ored Re	actions			
Dead:	15 PSF			Bearing L	.ength	Cap. Re	eact D/L lb	Total Ld.	Case	Ld. Comb.
				1-SPF 2	.375"	37%	276 / 884	1161 L		1.25D+1.5L
				2-SPF 4	.125"	15%	111 / 355	467 L		1.25D+1.5L
A l D	It -									

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1944 ft-lb	5'4 3/4"	7660 ft-lb	0.254 (25%)	1.25D+1.5L	L
Unbraced	1944 ft-lb	5'4 3/4"	1951 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1140 lb	1 5/8"	3160 lb	0.361 (36%)	1.25D+1.5L	L
Perm Defl in.	0.040 (L/4158)	6'7 5/8"	0.462 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.107 (L/1558)	6'7 5/8"	0.462 (L/360)	0.230 (23%)	L	L
TL Defl inch	0.147 (L/1134)	6'7 5/8"	0.693 (L/240)	0.210 (21%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8' o.c.
- 5 Bottom flange braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-3-6	(Span)0-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-2		Near Face	134 lb	358 lb	0 lb	0 lb	F7
4	Tie-In	1-9-6 to 14-3-6	(Span)0-11-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

Manufacturer Info

Nascor by Kott







NE0618-020 PAGE 21 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Address:

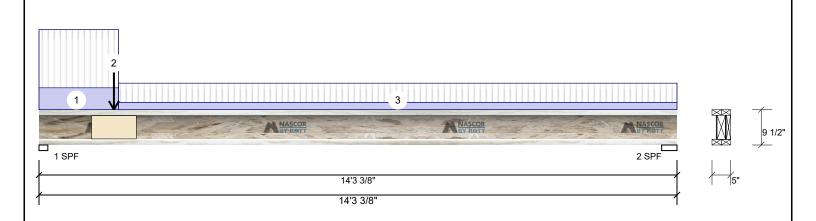
5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

2-Ply - PASSED 9.500" F9-B NJH

Level: Ground Floor



Member Info	rmation			Unfactore	ed Reaction	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	531	199	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	204	77	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	red Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	33% 249 / 796	1045 L	1.25D+1.5L
				2-SPF 4	1.125"	13% 96 / 306	402 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1707 ft-lb	5'2 15/16"	7660 ft-lb	0.223 (22%)	1.25D+1.5L	L
Unbraced	1707 ft-lb	5'2 15/16"	1714 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1027 lb	1 5/8"	3160 lb	0.325 (32%)	1.25D+1.5L	L
Perm Defl in.	0.035 (L/4746)	6'7 5/16"	0.462 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.094 (L/1779)	6'7 5/16"	0.462 (L/360)	0.200 (20%)	L	L
TL Defl inch	0.129 (L/1294)	6'7 5/16"	0.693 (L/240)	0.190 (19%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'6" o.c.
- 5 Bottom flange braced at bearings.

IE) L	₋oad Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Т	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	F	Point	1-8-2		Far Face	124 lb	331 lb	0 lb	0 lb	F7
3	Т	Гie-In 1	I-9-6 to 14-3-6	(Span)1-1-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- anoling & installation
 Lioist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information
 details for framing details, sulffener tables, web hole
 chart, bridging details, multi-hyl fastening details and
 handling/erection details
 Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

Manufacturer Info

Nascor by Kott







LIANA 1 (ELEV.2)

Design Method

GRANELLI HOME CORP.

GREEN YORK HOMES

BRAMPTON, ONT.

Description

May 29, 2018

Sales Rep

Designer RCO

Shipping

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

\LIANA 1 (ELEV.2).isl

Second Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceiling:

Deck

Deflection Girder

Floor

oads

Dead

Kott Lumber Company

\GRANELLI HOME CORP\MODELS

LSD

2012

15

480

360

480

360

360

240

480

360

OSB

5/8"

Gypsum 1/2"

\LIANA 1\LIANA 1 ELEV 2\FLOOR

Building Code NBCC 2010 / OBC

Project

LSD

Created

Builder

Pcs Length

12-0-0

12-0-0

10-0-0

10-0-0

8-0-0

6-0-0

4-0-0

4-0-0

Pcs Length

11 16-0-0

18 14-0-0

13 10-0-0

13 8-0-0

1 2-0-0

12-0-0

15

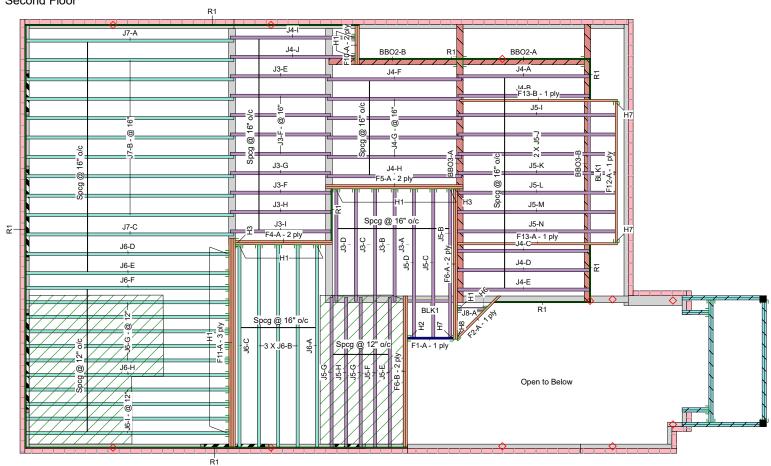
3

2

2

2

Second Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS** INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Legend

Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 9.5 NJ60H 9.5 NJH 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 5. CCMC -12787-R APA PR-L310(C)



- 4. CAN/CSA-O86-09

Label	Description	Width	Depth	Qty	Plies	Pcs	Length	Canada
R1	Norbord Rimboard	1.125	9.5			10	12	L4A 7X4
	Plus 1.125 X 9.5							905-642-4400
Blockin	g							
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	Job Path
BLK1	NJH	2.5	9.5	LinFt		Varies	11-0-0	D:\Users\rochavillo\WORK FROM HOMF\GREEN YORK HOMES
								I DUNIENSKEEN YUKK DUNIES

Qty

Beam/Girder Supported Member Pcs Description Skew Slope fasteners fasteners

Label H1 28 LT259 4 10dx1 1/2 2 10dx1 1/2 H2 1 HUS1.81/10 30 16d 10 16d H3 2 HGUS410 46 16d 16 16d H4 Unknowr Hanger 14 10dx1 1/2 2 10dx1 1/2 H6 1 SUR2.56/9 (Min) Right HUCQ1.81/9-SDS

Right

H8 NOTES

Second Floor LVL/LSL (Flush)

F11

F6

F13

F5

F12

F4

F2

F10

Label Description

Forex

Forex

Forex

Forex

Forex

Forex

Forex

Forex

Label Description

Joist (Flush)

J7 NJ60H

J6 NJ60H

J5 NJH

J4 NJH

J3 NJH

J8 NJH

Rim Board

Label

Hanger

2.0E-3000Fb LVL

Width Depth

9.5

9.5

9.5

9.5

1.75

1.75

1.75

1.75

1.75

1.75

1.75

1.75

1.75

2.5

2.5

2.5

2.5

2.5

2.5

Width | Depth |

9.5

9.5

9.5

9.5

9.5

9.5

Qty

Plies

2

Plies

- Framer to verify dimensions on the architectural drawings.
- . Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

1 LSSUI25

- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls . Install single-ply flush window header along inside face of rimboard/rimioist
- . Refer to Nascor specifier guide for installation works. Squash blocks recommended to be installed at end bearing on
- all first level joists which support loading from above exceeding two levels floor or roof. . Load transfer blocks to be installed under all point loads.
- . It shall be the framer's responsibility that floor joists and beams are
- fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls. columns. and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

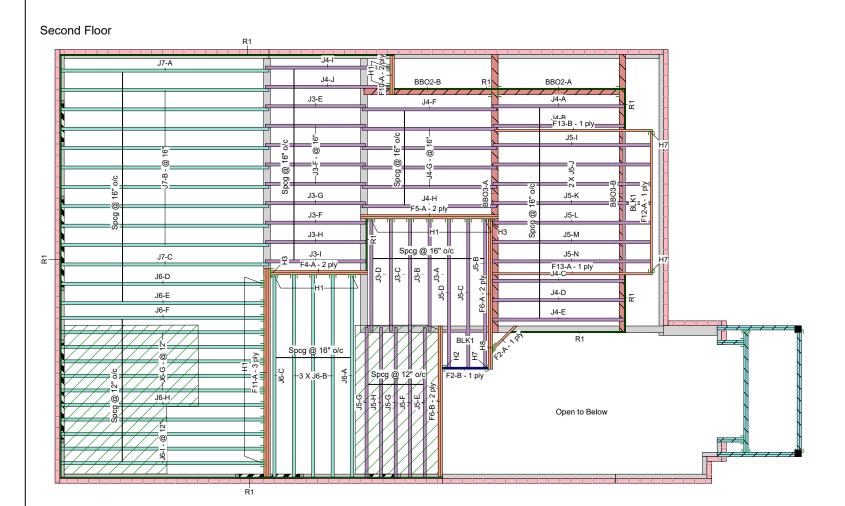
The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior

ARCHITECTURAL DRAWINGS:

Model: Liana 1

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2: May 22.2018 Project No: 17-55

EWP Studio Simpson Strong-Tie® Component Solutions™ NE0618-020



THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL

POINT LOADS OVER BEARINGS.



Legend



Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 9.5 NJ60H 9.5 NJH 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 5. CCMC -12787-R APA PR-L310(C)



- 4. CAN/CSA-O86-09

Second Floor												
LVL/LS	L (Flus	sh)										RIACCOD
Label	Descri	ption	Widt	th De	pth	Qt	y	Plies	Pcs	S	Length	NASCOR
F11	Forex 2.0E-3	000Fb LVL	1.7	75	9.5	1		3	3		16-0-0	
F6	Forex 2.0E-3	000Fb LVL	1.7	75	9.5	2		2	4		12-0-0	Layout Name LIANA 1 (ELEV.2) 4 BEDROOM
F13	Forex 2.0E-3	000Fb LVL	1.7	75	9.5				2		12-0-0	Design Method
F5	Forex 2.0E-3	000Fb LVL	1.7	75	9.5	1		2	2		10-0-0	Description
F12	Forex 2.0E-3	000Fb LVL	1.7	75	9.5				1		10-0-0	GRANELLI HOME CORP. BRAMPTON, ONT.
F4	-	000Fb LVL	1.7		9.5	1		2	2		8-0-0	Created May 29, 2018
F2	Forex 2.0E-3	000Fb LVL	1.7		9.5				2		4-0-0	Builder
F10	2.0E-3000Fb LVL		1.7	75	9.5	1		2	2		4-0-0	GREEN YORK HOMES Sales Rep
I Joist (Flush)											RM
Label	Descri	ption	Widt	th De	pth	Qt	y	Plies	Pcs	S	Length	
J7	NJ60H			.5	9.5				11		16-0-0	Designer RCO
J6	NJ60H			.5	9.5				18		14-0-0	
J5	J4 NJH			.5	9.5				15		12-0-0	Shipping
				.5	9.5				13		10-0-0	Project
J3	NJH		2	.5	9.5				13		8-0-0	Builder's Project
Rim Bo			\ A /: -14	u. D.	41.		1	Dii	D-			Kott Lumber Company
	Label Description R1 Norbord Rimboard Plus 1.125 X 9.5		Widt		pth 9.5	Qt	y	Plies	Pcs 10		Length 12	14 Anderson Blvd
KI			1.12	25	9.5				10		12	Stouffville, Ontario
Blockin		120710.0										Canada
-	γ	Description		th De	pth	Qt	v	Plies	Pcs	3	Length	L4A 7X4
BLK1			2	.5	9.5	Lin	,		Varie	es	11-0-0	905-642-4400
Hanger	-											Job Path
							Bea	am/Girde			ported ember	D:\Users\rochavillo\WORK FROM HOME\GREEN YORK HOMES
Label	Label Pcs Description		n	Skew	Slo	ре	fa	steners	f	as	teners	\GRANELLI HOME CORP\MODELS
H1	27	27 LT259 4 10dx1 1/2 2		10	dx1 1/2	\LIANA 1\LIANA 1 ELEV 2\FLOOR\4						
H2	1	HUS1.81/1	0					30 16d) 16d	BEDRM\LIANA 1 (ELEV.2).isl
Н3	2	HGUS410						46 16d		16	6 16d	Second Floor
H4 9 Unknown Hanger		-										Design Method LSD
		/0			-			-			Building Code NBCC 2010 / OBC	

NOTES:

. Framer to verify dimensions on the architectural drawings.

Right

- . Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger.

HUCQ1.81/9-

SDS

LSSUI25

- . Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls. . Install single-ply flush window header along inside face of rimboard/rimjoist.
- . Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.2; May 22,2018

Project No: 17-55 Model: Liana 1

EWP Studio Simpson Strong-Tie® Component Solutions™ 2012

15

480

360

480

360

360

240

480

360

OSB

5/8"

Nailed & Glued

Gypsum 1/2"

Floor

Loads

I ive

Dead

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceiling:

Deck

Deflection Girder

PAGE 23 OF 41

PAGE 24 OF 41 NE0618-020

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project:

Address:

GREEN YORK HOMES

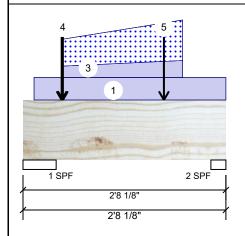
5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

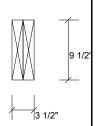
Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F10-A

Level: Second Floor



15 PSF



Wind

0

0

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow

661

198

1056

134

272

143

2

ı	Bearings and Factored Reactions												
	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.						
	1 - SPF	5.250"	25%	826 / 1720	2546	L	1.25D+1.5S +0.5L						
	2 - SPF	2.375"	11%	248 / 214	462	L	1.25D+1.5L						

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	395 ft-lb	1'1 9/16"	22724 ft-lb	0.017 (2%)	1.25D+1.5S +0.5L	L
Unbraced	395 ft-lb	1'1 9/16"	22724 ft-lb	0.017 (2%)	1.25D+1.5S +0.5L	L
Shear	638 lb	1'2"	7792 lb	0.082 (8%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/39438)	1'4 5/8"	0.072 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/29384)	1'3"	0.072 (L/360)	0.010 (1%)	S+0.5L	L
TL Defl inch	0.002 (L/16882)	1'3 3/4"	0.108 (L/240)	0.010 (1%)	D+S+0.5L	L
· ·			· · · · · · · · · · · · · · · · · · ·	·	·	

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

Jun 04, 2018

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 25 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES** Project:

5/31/2018 Designer: RCO

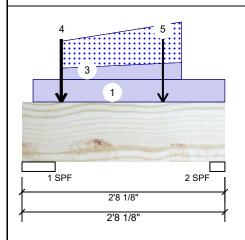
Job Name: LIANA 1 (ELEV.2)

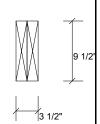
Project #:

1.750" X 9.500" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F10-A

Address:

Level: Second Floor





Page 2 of 2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-1-12 to 2-8-2		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Point	0-6-2		Тор	458 lb	0 lb	1038 lb	0 lb	F14 F14
3	Tapered Start	0-6-2		Тор	32 PLF	0 PLF	76 PLF	0 PLF	
	End	2-1-4			48 PLF	0 PLF	115 PLF	0 PLF	
4	Point	0-6-5		Far Face	86 lb	230 lb	0 lb	0 lb	J4
5	Point	1-10-5		Far Face	69 lb	185 lb	0 lb	0 lb	J4
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







NE0618-020 PAGE 26 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES** Project:

5/31/2018 Date: Designer: RCO

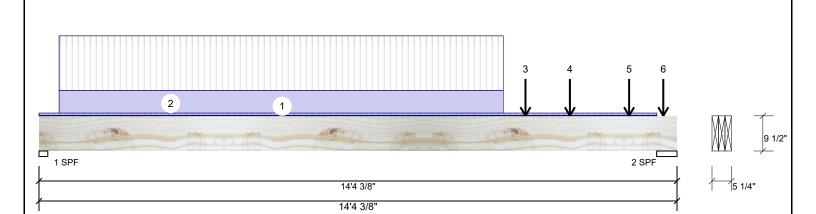
Job Name: LIANA 1 (ELEV.2)

Level: Second Floor

Project #

Forex 2.0E-3000Fb LVL 1.750" X 9.500" 3-Ply - PASSED F11-A

Address:



Member Information **Unfactored Reactions UNPATTERNED Ib (Uplift)** Wind Type: Application: Floor (Residential) Brg Live Dead Plies: 3 Design Method: 1909 863 0 O 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 2945 1254 0 0 Deflection LL: 360 Load Sharing: Yes Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 51% 1079 / 2864 3943 I 1.25D+1.5L 2 - SPF 5.500" 34% 1567 / 4417 5984 I 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	14206 ft-lb	7' 5/8"	35449 ft-lb	0.401 (40%)	1.25D+1.5L	L
Unbraced	14206 ft-lb	7' 5/8"	34190 ft-lb	0.416 (42%)	1.25D+1.5L	L
Shear	4299 lb	13'2 1/8"	13915 lb	0.309 (31%)	1.25D+1.5L	L
Perm Defl in.	0.149 (L/1117)	7' 1/2"	0.461 (L/360)	0.320 (32%)	D	Uniform
LL Defl inch	0.333 (L/499)	7' 11/16"	0.461 (L/360)	0.720 (72%)	L	L
TL Defl inch	0.481 (L/345)	7' 11/16"	0.692 (L/240)	0.700 (70%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

6 Lateral s	sienderness ratio based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 13-10-15	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-5-7 to 10-5-7		Far Face	115 PLF	278 PLF	0 PLF	0 PLF	
3	Point	10-11-7		Far Face	104 lb	278 lb	0 lb	0 lb	J6
4	Point	11-11-7		Far Face	122 lb	324 lb	0 lb	0 lb	J6
5	Point	13-3-7		Far Face	139 lb	371 lb	0 lb	0 lb	J6
6	Point	14-0-11		Near Face	384 lb	959 lb	0 lb	0 lb	F4 (
	Self Weight				11 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



Jun 04, 2018



NE0618-020 PAGE 27 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Date: 5/31/2018 Designer: RCO

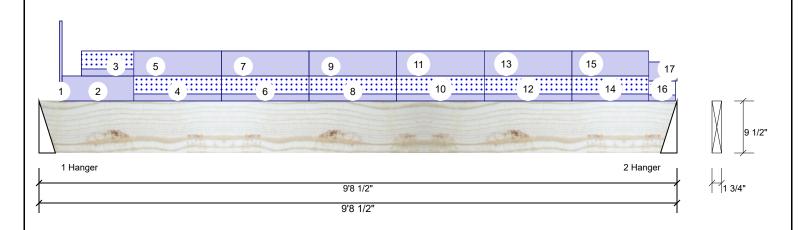
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED F12-A Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor



Member Information U Floor (Residential) Bro Live Dead Snow Type: Application: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: 15 PSF Dead:

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	779 ft-lb	4'10 5/16"	9544 ft-lb	0.082 (8%)	1.25D+1.5S	L
Unbraced	779 ft-lb	4'10 5/16"	3994 ft-lb	0.195 (20%)	1.25D+1.5S	L
Shear	278 lb	11 3/4"	3896 lb	0.071 (7%)	1.25D+1.5S	L
Perm Defl in.	0.027 (L/4130)	4'10 1/4"	0.311 (L/360)	0.090 (9%)	D	Uniform
LL Defl inch	0.014 (L/8256)	4'10 5/16"	0.311 (L/360)	0.040 (4%)	S	L
TL Defl inch	0.041 (L/2753)	4'10 1/4"	0.467 (L/240)	0.090 (9%)	D+S	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

1 0 164 76 2 0 171 85	ıu
2 0 171 85	0
	0

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	10%	205 / 114	319	L	1.25D+1.5S	
2 - Hanger	3.000"	10%	213 / 128	341	L	1.25D+1.5S	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



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Page 1 of 2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-12 to 0-4-4		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-4-4 to 1-5-5		Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Part. Uniform	0-7-12 to 1-5-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF	
4	Part. Uniform	1-5-5 to 2-9-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF	
5	Part. Uniform	1-5-5 to 2-9-5		Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Part. Uniform	2-9-5 to 4-1-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF	
7	Part. Uniform	2-9-5 to 4-1-5		Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
8	Part. Uniform	4-1-5 to 5-5-5		Тор	7 PLF	0 PLF	18 PLF	0 PLF	
Continued or	n page 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







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EWP Studio Simpson Strong-Tie® Component Solutions™

Continued from page 1

Client: **GREEN YORK HOMES**

Project: Address:

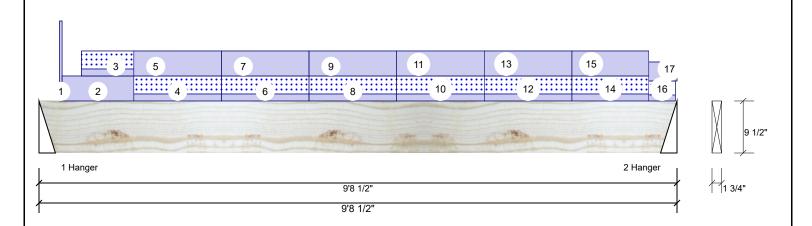
5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED F12-A Forex 2.0E-3000Fb LVL

Level: Second Floor



Continued t	rom page 1							
ID	Load Type	Location Trib Width	Side	Dead	Live	Snow	Wind	Comments
9	Part. Uniform	4-1-5 to 5-5-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Part. Uniform	5-5-5 to 6-9-5	Тор	7 PLF	0 PLF	18 PLF	0 PLF	
11	Part. Uniform	5-5-5 to 6-9-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
12	Part. Uniform	6-9-5 to 8-1-5	Тор	7 PLF	0 PLF	18 PLF	0 PLF	
13	Part. Uniform	6-9-5 to 8-1-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
14	Part. Uniform	8-1-5 to 9-3-5	Тор	7 PLF	0 PLF	18 PLF	0 PLF	
15	Part. Uniform	8-1-5 to 9-3-5	Тор	25 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
16	Part. Uniform	9-3-5 to 9-8-3	Тор	6 PLF	0 PLF	14 PLF	0 PLF	
17	Part. Uniform	9-3-5 to 9-8-8	Тор	19 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	Self Weight			4 PLF				
I								

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 2 of 2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







PAGE 29 OF 41 NE0618-020

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project: Address:

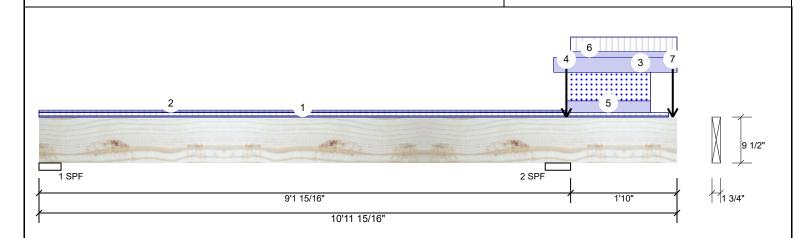
5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED F13-A Forex 2.0E-3000Fb LVL

Level: Second Floor



Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg 1	Live	Dead	Snow	Wind
1	105	(-1)	0 (-33)	0
2	310	826	719	0

Bearings and Factored Reactions

Bearing Leng	th Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 4.563	" 5%	-1 / 185	185 (-55)	L_	0.9D+1.5L
2 - SPF 5.250	" 40%	1032 / 1233	2265	LL	1.25D+1.5S +0.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-930 ft-lb	9'1 15/16"	7726 ft-lb	0.120 (12%)	1.25D+1.5L	_L
Unbraced	-1198 ft-lb	9'1 15/16"	4322 ft-lb	0.277 (28%)	1.25D+1.5S +0.5L	_L
Pos Moment	269 ft-lb	3'6 1/4"	8294 ft-lb	0.032 (3%)	0.9D+1.5L	L_
Unbraced	269 ft-lb	3'6 1/4"	5773 ft-lb	0.047 (5%)	0.9D+1.5L	L_
Shear	701 lb	9'11 7/16"	4638 lb	0.151 (15%)	1.25D+1.5S +0.5L	_L
Perm Defl in.	0.011 (L/9417)	5'9 3/16"	0.287 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.015 (L/6903)	4'7 9/16"	0.287 (L/360)	0.050 (5%)	L	L_
TL Defl inch	0.023 (L/4543)	5'6 3/8"	0.431 (L/240)	0.050 (5%)	D+S+0.5L	_L
LL Cant	0.018 (2L/2401)	Rt Cant	0.200 (2L/480)	0.092 (9%)	S+0.5L	_L
TL Cant	0.042 (2L/1038)	Rt Cant	0.300 (2L/360)	0.141 (14%)	D+S+0.5L	_L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Tie-down connection required at bearing 1 for uplift 55 lb (Combination 1.25D+1.5S+0.5L, Load Case _L).
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

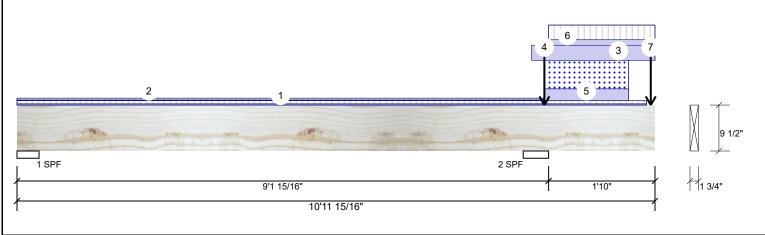
Project: Address: Date: 5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F13-A

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-10-3	(Span)0-10-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 9-0-13	(Span)0-5-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	8-10-7 to 10-11-15		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	9-1-1		Тор	202 lb	0 lb	390 lb	0 lb	F15 F15
5	Part. Uniform	9-1-1 to 10-6-7		Тор	63 PLF	0 PLF	152 PLF	0 PLF	
6	Tie-In	9-1-15 to 10-11-15	(Span) 3-10-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	10-11-1		Far Face	164 lb	0 lb	76 lb	0 lb	F12
	Self Weight				4 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

NE0618-020 PAGE 31 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Date: 5/31/2018 Designer: RCO

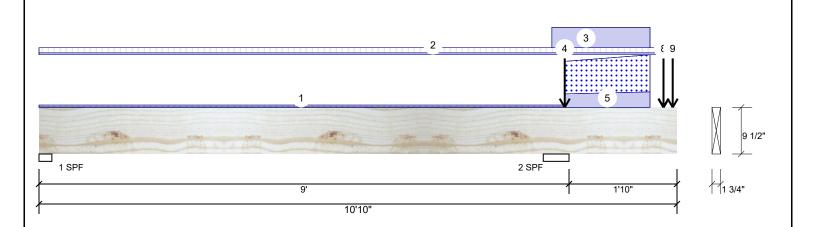
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED F13-B Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor



Member Information Type: Application: Floor (Residential) Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	116	1	0 (-37)	0
2	157	801	797	0

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.625"	9%	1 / 179	180 (-54)	L_	1.25D+1.5L
2 - SPF	5.250"	40%	1001 / 1274	2275	LL	1.25D+1.5S +0.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-1144 ft-lb	9'	11248 ft-lb	0.102 (10%)	1.25D+1.5S +0.5L	_L
Unbraced	-1144 ft-lb	9'	4322 ft-lb	0.265 (26%)	1.25D+1.5S +0.5L	LL
Pos Moment	280 ft-lb	3'5 1/8"	8408 ft-lb	0.033 (3%)	0.9D+1.5L	L_
Unbraced	280 ft-lb	3'5 1/8"	5672 ft-lb	0.049 (5%)	0.9D+1.5L	L_
Shear	664 lb	9'9 1/2"	4638 lb	0.143 (14%)	1.25D+1.5S +0.5L	_L
Perm Defl in.	0.010 (L/10431)	5'7 13/16"	0.288 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.015 (L/6902)	4'5 5/8"	0.288 (L/360)	0.050 (5%)	L	L_
TL Defl inch	0.021 (L/4995)	5'4 3/4"	0.431 (L/240)	0.050 (5%)	D+S+0.5L	_L
LL Cant	0.017 (2L/2601)	Rt Cant	0.200 (2L/480)	0.085 (8%)	S+0.5L	_L
TL Cant	0.039 (2L/1118)	Rt Cant	0.300 (2L/360)	0.131 (13%)	D+S+0.5L	_L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Tie-down connection required at bearing 1 for uplift 54 lb (Combination 0.9D+1.5S, Load Case _L).
- 4 Top braced at bearings.
- 5 Bottom braced at bearings

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 32 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

5/31/2018 Designer: RCO

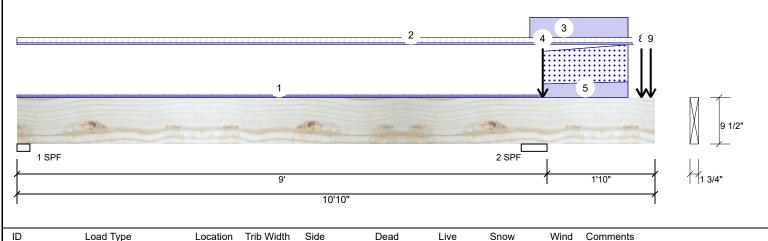
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" - PASSED F13-B Forex 2.0E-3000Fb LVL

Address:

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-10-14	(Span)0-3-15 to 0-3-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-8-4	(Span)1-0-1 to 1-0-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	8-8-8 to 10-4-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	8-11-2		Тор	232 lb	0 lb	460 lb	0 lb	F15 F15
5	Tapered Start	8-11-2		Тор	53 PLF	0 PLF	128 PLF	0 PLF	
	End	10-4-8			61 PLF	0 PLF	146 PLF	0 PLF	
6	Point	10-7-4		Тор	6 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	Point	10-7-4		Тор	7 lb	0 lb	17 lb	0 lb	
8	Point	10-7-4		Тор	26 lb	0 lb	0 lb	0 lb	Wall Self Weight
9	Point	10-9-2		Near Face	171 lb	0 lb	85 lb	0 lb	F12
	Self Weight				4 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

NE0618-020 PAGE 33 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

5/31/2018 Designer: RCO

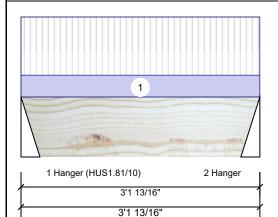
Job Name: LIANA 1 (ELEV.2)

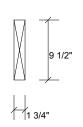
Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F1-A

Address:

Level: Second Floor





Member Information Application: Floor (Residential) Type: Plies: Design Method: Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 Deflection LL: 360 Load Sharing: No Deflection TL: 240 Not Checked Deck: Importance: Normal Vibration: Not Checked General Load 40 PSF Floor Live: 15 PSF Dead:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	vvina
1	378	148	0	0
2	378	148	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	19%	185 / 567	752	L	1.25D+1.5L	
2 - Hanger	3.000"	19%	185 / 567	752	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	460 ft-lb	1'6 15/16"	11362 ft-lb	0.040 (4%)	1.25D+1.5L	L
Unbraced	460 ft-lb	1'6 15/16"	10144 ft-lb	0.045 (5%)	1.25D+1.5L	L
Shear	285 lb	2'2 1/16"	4638 lb	0.061 (6%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/29538)	1'6 15/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/11546)	1'6 15/16"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/8301)	1'6 15/16"	0.139 (L/240)	0.030 (3%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-1-13		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 34 OF 41



Client: **GREEN YORK HOMES**

Project:

Address:

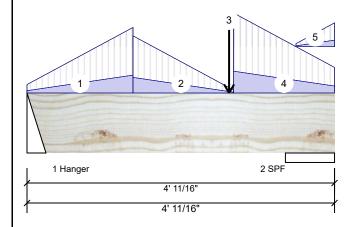
5/31/2018 Designer: RCO

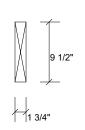
Job Name: LIANA 1 (ELEV.2)

Project #

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F2-A

Level: Second Floor





Wind

0

Member Info	rmation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

	Reactions	UNPATTERNED	lb (Uplift)
Brg	Live	Dead	Snow

23

2	80	39	0	0

n

Bearings and Factored Reactions

42

Bearing Length	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
1 - 3.000"	2%	29 / 63	92	L	1.25D+1.5L
Hanger					
2 - SPF 7.778"	2%	48 / 121	169	L	1.25D+1.5L

Analysis Results Analysis Capacity Actual Comb. Case Location Allowed Moment 94 ft-lb 2'2 7/16" 11362 ft-lb 0.008 (1%) 1.25D+1.5L L Unbraced 94 ft-lb 2'2 7/16" 9657 ft-lb 0.010 (1%) 1.25D+1.5L L 88 lb 2'8 3/16" 4638 lb 0.019 (2%) 1.25D+1.5L L Shear Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.001 1'11 13/16" 0.109 (L/360) 0.010 (1%) L L (L/60406) TL Defl inch 0.001 1'11 5/8" 0.164 (L/240) 0.010 (1%) D+L L (L/40550)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

4 BOROTT	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-13	(Span)0-2-8 to 1-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	1-4-13 to 2-8-11	(Span)1-4-13 to 0-0-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	2-8-0		Far Face	15 lb	40 lb	0 lb	0 lb	J8
4	Tie-In	2-8-11 to 4-0-11	(Span)1-11-8 to 0-7-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	3-6-9 to 4-0-11	(Span)0-0-14 to 0-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 35 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

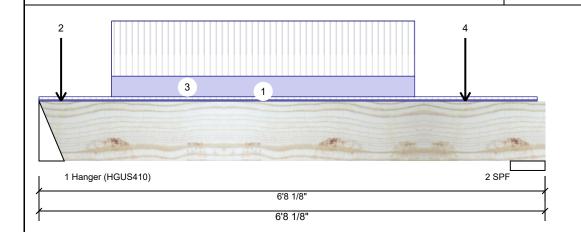
Project: Address: Date: 5/31/2018 Designer: RCO

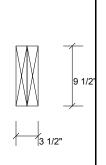
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F4-A

Level: Second Floor





Member Inform	nation
Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Not Checked Deck: Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Wind Brg Live Dead 959 384 0 O 1 2 855 349 0 0

Bearings and Factored Reactions Bearing Length Cap. React D/L lb

Total Ld. Case Ld. Comb. 481 / 1439 1.25D+1.5L 4.000" 18% 1919 L Hanger 2 - SPF 5.500" 15% 437 / 1283 1720 L 1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2571 ft-lb	3'3 5/8"	22724 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	2571 ft-lb	3'3 5/8"	22010 ft-lb	0.117 (12%)	1.25D+1.5L	L
Shear	1733 lb	5'5 7/8"	9277 lb	0.187 (19%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/8511)	3'3 1/2"	0.200 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.021 (L/3423)	3'3 1/2"	0.200 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.030 (L/2441)	3'3 1/2"	0.301 (L/240)	0.100 (10%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 6-6-14	(Span)0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-3-8		Near Face	96 lb	256 lb	0 lb	0 lb	J6
3	Part. Uniform	0-11-8 to 4-11-8		Near Face	104 PLF	278 PLF	0 PLF	0 PLF	
4	Point	5-7-8		Near Face	133 lb	345 lb	0 lb	0 lb	J6
	Self Weight				8 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 36 OF 41

2-Ply - PASSED

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

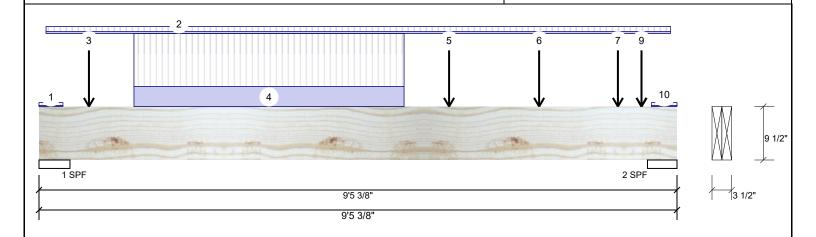
Project: Address: Date: 5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL F5-A

Level: Second Floor



Member Info	rmation			Unfacto	red React	tions UI	NPATTERN	ED lb (U	plift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow		Wind
Plies:	2	Design Method:	LSD	1	708 (-2)		303	0		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	649 (-115)		259	0		0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fact	tored R	eactions			
Dead:	15 PSF			Bearing	Length	Cap. I	React D/L lb	Total L	d. Case	Ld. Comb.
				1 - SPF	5.500"	12%	379 / 1063	1441 L	-	1.25D+1.5L
A 1 1 - D				2-SPF	5.250"	11%	324 / 974	1297 L	-	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3030 ft-lb	4'8 3/16"	22724 ft-lb	0.133 (13%)	1.25D+1.5L	L
Unbraced	3030 ft-lb	4'8 3/16"	21237 ft-lb	0.143 (14%)	1.25D+1.5L	L
Shear	1394 lb	1'2 1/4"	9277 lb	0.150 (15%)	1.25D+1.5L	L
Perm Defl in.	0.019 (L/5399)	4'8 11/16"	0.289 (L/360)	0.070 (7%)	D	Uniform
LL Defl inch	0.046 (L/2287)	4'8 11/16"	0.289 (L/360)	0.160 (16%)	L	L
TL Defl inch	0.065 (L/1607)	4'8 11/16"	0.434 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 1 of 2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-4	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-4 to 9-4-4	(Span)0-8-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-8-14		Near Face	55 lb	145 lb	0 lb	0 lb	J3
4	Part. Uniform	1-4-14 to 5-4-14		Near Face	57 PLF	151 PLF	0 PLF	0 PLF	
5	Point	6-0-14		Near Face	72 lb	191 lb	0 lb	0 lb	J5
6	Point	7-4-14		Near Face	65 lb	174 lb	0 lb	0 lb	J5

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







NE0618-020 PAGE 37 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™

Continued from page 1

Client: **GREEN YORK HOMES**

Project:

Date: 5/31/2018 Designer: RCO

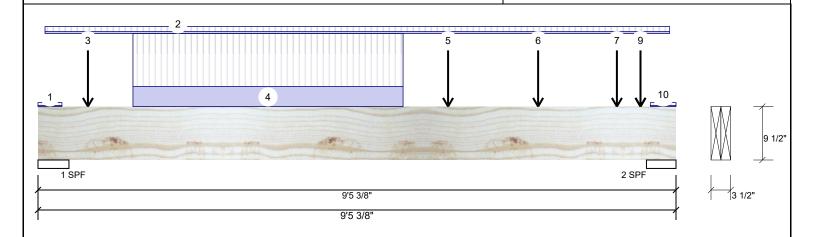
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F5-A

Address:

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	8-6-14		Near Face	40 lb	105 lb	0 lb	0 lb	J5
8	Point	8-11-1		Near Face	-22 lb	0 lb	0 lb	0 lb	F6
9	Point	8-11-1		Near Face	0 lb	-117 lb	0 lb	0 lb	F6
10	Tie-In	9-0-13 to 9-5-6	(Span)0-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

NE0618-020 PAGE 38 OF 41

EWP Studio Simpson Strong-Tie® Component Solutions™ Client:

Project: Address:

GREEN YORK HOMES

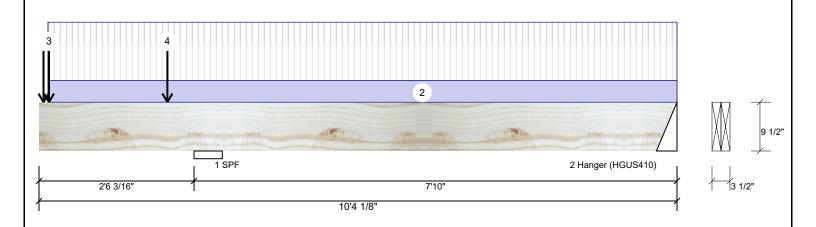
Date: 5/31/2018 Designer: RCO

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F6-A

Level: Second Floor



Member Infor	rmation			Unfacto	red React	ions L	INPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	
Plies:	2	Design Method:	LSD	1	658		316	0	
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	0 (-117)		(-22)	0	
Deflection LL:	360	Load Sharing:	No		, ,		,		
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fact	ored	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total Ld. Cas	e e
				1 - SPF	5.500"	12%	395 / 987	1382 LL	
				2 -	4.000"	0%	-20 / 41 2	22 (-242) _L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-2162 ft-lb	2'8 15/16"	22724 ft-lb	0.095 (10%)	1.25D+1.5L	L_
Unbraced	-2162 ft-lb	2'8 15/16"	21662 ft-lb	0.100 (10%)	1.25D+1.5L	L_
Pos Moment	7 ft-lb	9'2 15/16"	14770 ft-lb	0.000 (0%)	0.9D+1.5L	_L
Unbraced	7 ft-lb	9'2 15/16"	14770 ft-lb	0.000 (0%)	0.9D+1.5L	_L
Shear	978 lb	1'8 11/16"	9277 lb	0.105 (11%)	1.25D+1.5L	L_
Perm Defl in.	0.004 (L/22511)	5'7 5/16"	0.244 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.013 (L/6952)	5'10 1/8"	0.244 (L/360)	0.050 (5%)	L	L_
TL Defl inch	0.017 (L/5316)	5'9 7/16"	0.367 (L/240)	0.050 (5%)	D+L	L_
LL Cant	0.035 (2L/1727)	Lt Cant	0.200 (2L/480)	0.175 (17%)	L	L_
TL Cant	0.048	Lt Cant	0.300	0.161 (16%)	D+L	L_

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Hanger



Wind

0

0

Ld. Comb.

1.25D+1.5L

0.9D+1.5L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Tie-down connection required at bearing 2 for uplift 242 lb (Combination 1.25D+1.5L, Load Case L_).
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

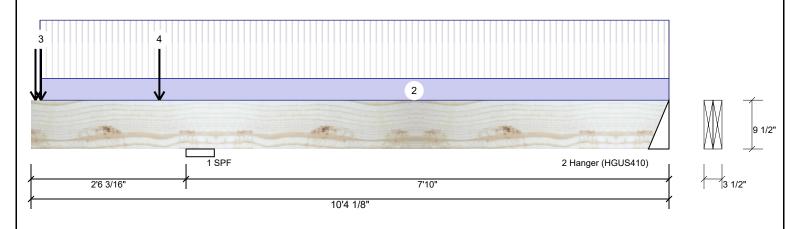
Date: 5/31/2018 Project: Designer: RCO Address:

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" 2-Ply - PASSED Forex 2.0E-3000Fb LVL F6-A

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-0-14		Far Face	148 lb	378 lb	0 lb	0 lb	F1
2	Tie-In	0-1-12 to 10-4-2	(Span)0-4-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-1-15		Near Face	23 lb	42 lb	0 lb	0 lb	F2
4	Point	2-1-0		Near Face	18 lb	49 lb	0 lb	0 lb	J8
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





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EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES**

Project:

Date: 5/31/2018 Designer: RCO

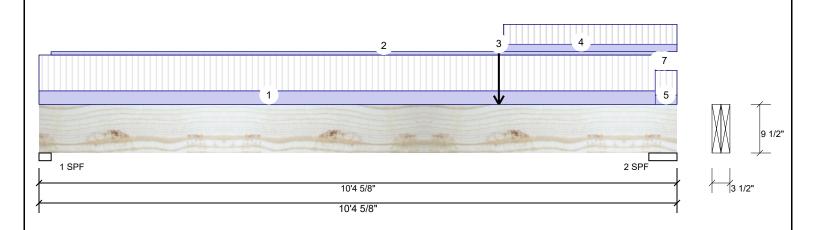
Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL 2-Ply - PASSED F6-B

Address:

Level: Second Floor



Member Infor	mation		Unfactored Reactions UNPATTERNED lb (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	2	Design Method:	LSD	1	208		128		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	424		214		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Fact	ored Re	eactions			
Dead:	15 PSF			Bearing L	ength	Cap. R	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF 2	2.375"	9%	160 / 312	472	L	1.25D+1.5L
A				2-SPF 5	5.500"	8%	268 / 636	904	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1955 ft-lb	7'5 13/16"	22724 ft-lb	0.086 (9%)	1.25D+1.5L	L
Unbraced	1955 ft-lb	7'5 13/16"	20806 ft-lb	0.094 (9%)	1.25D+1.5L	L
Shear	817 lb	9'2 3/8"	9277 lb	0.088 (9%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/7225)	5'5 1/8"	0.328 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.031 (L/3827)	5'6 3/4"	0.328 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.047 (L/2502)	5'6 1/4"	0.493 (L/240)	0.100 (10%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER **CONNECTION DETAIL FOR PLY TO PLY** NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Page 1 of 2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-0-6	(Span)1-0-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 10-0-6		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
3	Point	7-5-13		Near Face	148 lb	378 lb	0 lb	0 lb	F1
4	Tie-In	7-6-11 to 10-4-10	(Span)0-7-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	10-0-6 to 10-4-10	(Span)0-8-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Part. Uniform	10-0-6 to 10-1-14		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
Continued on page	Continued on page 2								

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Handling & Installation

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 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

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- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318







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2-Ply - PASSED

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: **GREEN YORK HOMES** Project:

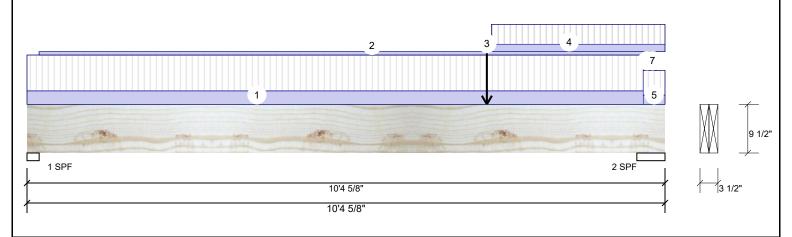
Date: 5/31/2018 Designer: RCO Address:

Job Name: LIANA 1 (ELEV.2)

Project #:

1.750" X 9.500" Forex 2.0E-3000Fb LVL F6-B

Level: Second Floor



Continued	from	page	1
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ID Load Type Trib Width Location Side Dead Live Snow Wind Comments 7 Tapered Start 10-1-14 Тор 1 PLF 0 PLF 0 PLF 0 PLF End 10-2-14 0 PLF 0 PLF 0 PLF 0 PLF 8 PLF Self Weight

Notes

NOtes
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Manufacturer Info Forex

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





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