

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-035
GREEN YORK HOMES - GRANELLI
HOME CORP - INDIGO 1 ELEV 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

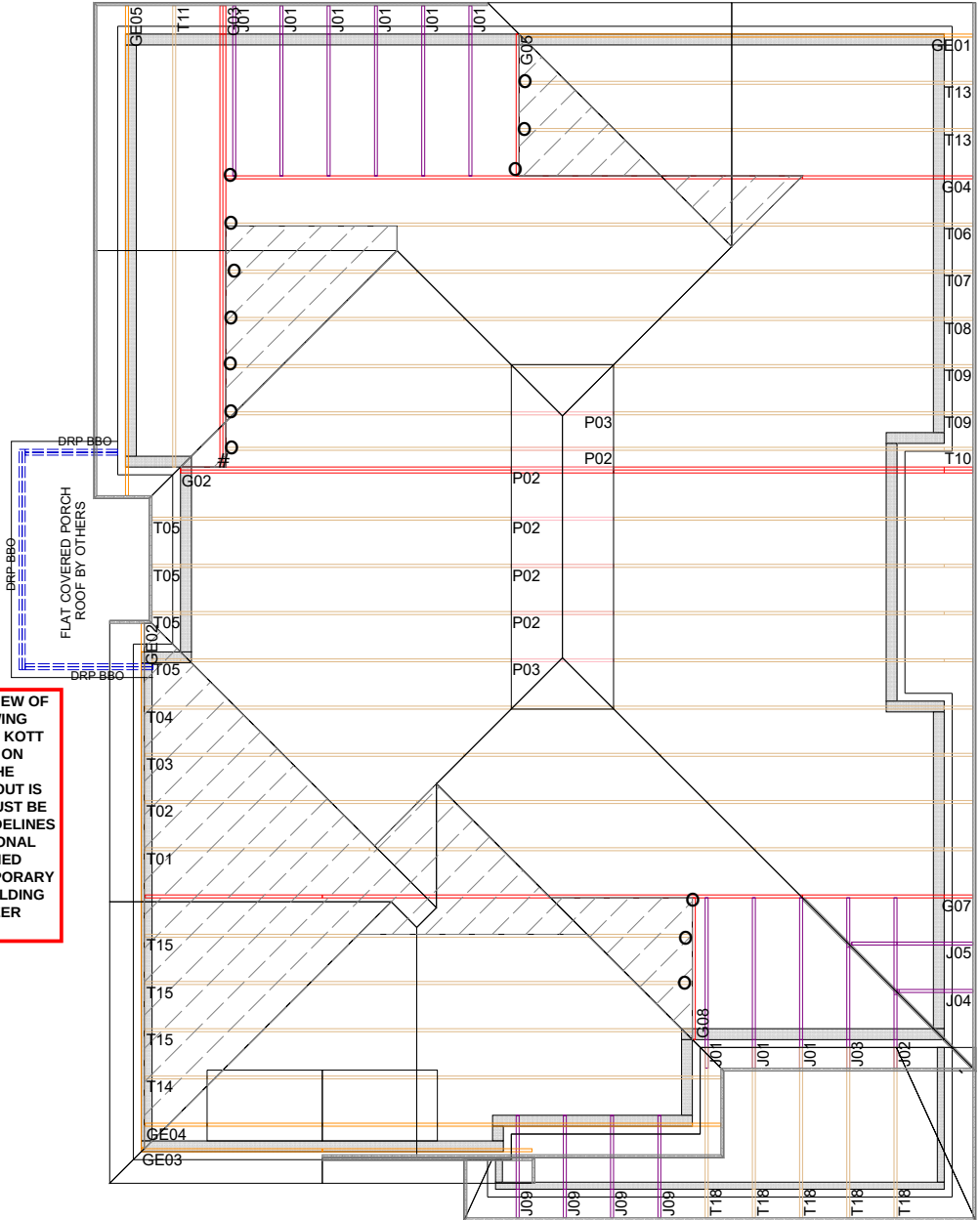
- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



GARAGE TOP OF PLATE
TO MATCH UNDERSIDE
OF SOFFIT

ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24 O - LJS26DS
- HGUS28-2 % - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

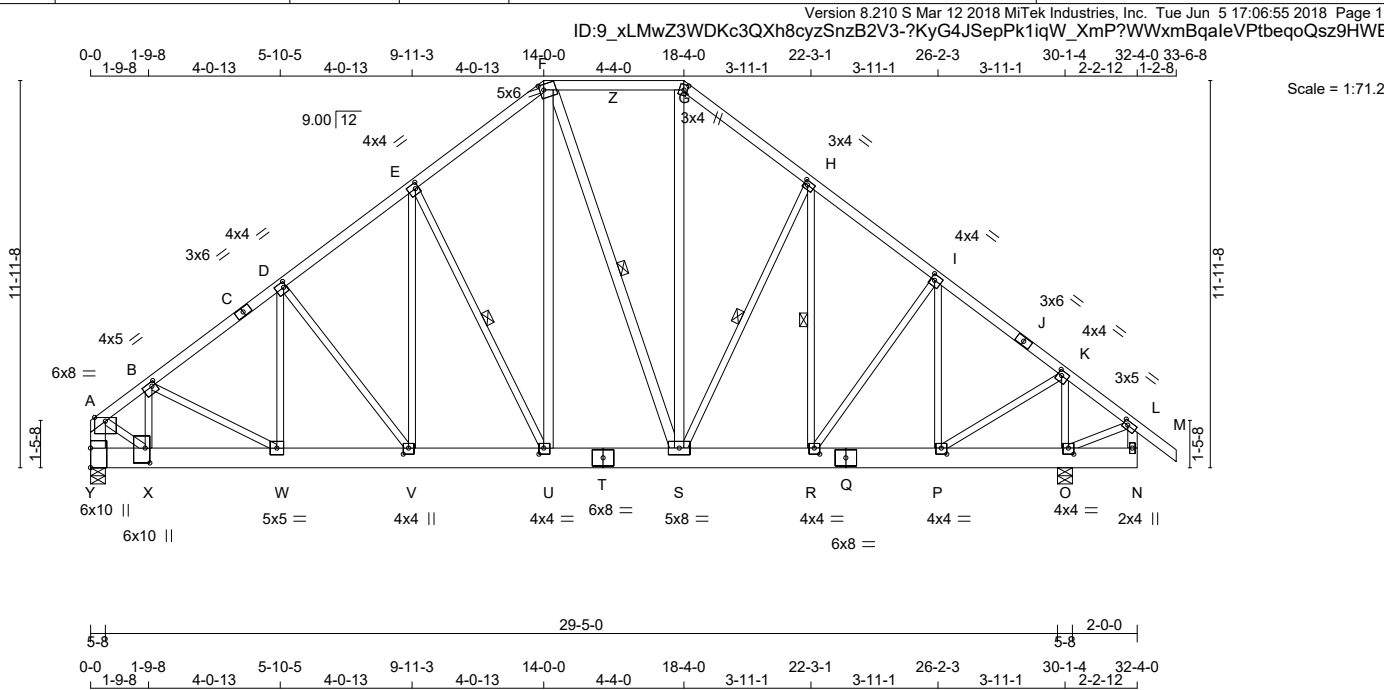
SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011
ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	INDIGO 1 ELE 2
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/31/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE TRUSS BELOW EA CROSS PT. VERT POS. THAN 6' TO HAVE A LATERAL BRACING SO THAT THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'



TOTAL WEIGHT = 2 X 224 = 448 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
Y - A	2x6	DRY	No.2
N - L	2x4	DRY	No.2
Y - T	2x8	DRY	No.2
T - Q	2x8	DRY	No.2
Q - N	2x8	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
U - F	2x4	DRY	No.2
F - S	2x4	DRY	No.2
S - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - G	12	TOP
G - J	12	TOP
J - M	12	TOP
N - L	12	TOP
Y - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Y - T	12	SIDE(0.0)
T - Q	12	TOP
Q - N	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	JT	HORZ	JT	UPLIFT	JT	IN-SX
Y	8153	0	8153	0	0	5-8	5-8
O	2210	0	2210	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	5730	3964 / 0	0 / 0	0 / 0	0 / 0	1767 / 0	0 / 0
O	1552	1081 / 0	0 / 0	0 / 0	0 / 0	472 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, O

BRACING

FOR SECTION F-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-U, F-S, H-S, H-R. DBS = 20-0-0. CBF = 177 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A - B	-7089 / 0	-77.4 -77.4 0.19 (1)	3.58	X - B	0 / 3020	0.37 (1)	
B - C	-4025 / 0	-77.4 -77.4 0.17 (1)	4.62	B - W	-2803 / 0	0.51 (1)	
C - D	-4025 / 0	-77.4 -77.4 0.17 (1)	4.62	W - D	0 / 1898	0.23 (1)	
D - E	-2688 / 0	-77.4 -77.4 0.13 (1)	5.45	D - V	-1792 / 0	0.73 (1)	
E - F	-1921 / 0	-77.4 -77.4 0.11 (1)	6.20	V - E	0 / 1385	0.17 (1)	
F - Z	-1343 / 0	-84.9 -84.9 0.16 (1)	2.00	E - U	-1414 / 0	0.44 (1)	
Z - G	-1343 / 0	-84.9 -84.9 0.16 (1)	2.00	U - F	0 / 1364	0.12 (1)	
G - H	-1690 / 0	-77.4 -77.4 0.10 (1)	6.25	F - S	-564 / 0	0.24 (1)	
H - I	-1689 / 0	-77.4 -77.4 0.10 (1)	6.25	S - G	0 / 701	0.06 (1)	
I - J	-1400 / 0	-77.4 -77.4 0.08 (1)	6.25	S - H	-46 / 0	0.01 (1)	
J - K	-1400 / 0	-77.4 -77.4 0.08 (1)	6.25	R - H	-289 / 0	0.07 (1)	
K - L	-5 / 57	-77.4 -77.4 0.08 (1)	10.00	R - I	0 / 408	0.05 (1)	
L - M	0 / 30	-77.4 -77.4 0.05 (1)	10.00	P - I	-806 / 0	0.21 (1)	
Y - A	-7407 / 0	0.0 0.0 0.26 (1)	5.51	P - K	0 / 1382	0.17 (1)	
N - L	-114 / 0	0.0 0.0 0.01 (1)	7.81	O - K	-1912 / 0	0.18 (1)	
				A - X	0 / 6453	0.80 (1)	
				O - L	-28 / 7	0.00 (1)	
Y - X	0 / 0	-18.2 -18.2 0.32 (1)	10.00				
X - W	0 / 5670	-18.2 -18.2 0.63 (1)	10.00				
W - V	0 / 3234	-18.2 -18.2 0.21 (1)	10.00				
V - U	0 / 2152	-18.2 -18.2 0.15 (1)	10.00				
U - T	0 / 1536	-18.2 -18.2 0.11 (1)	10.00				
T - S	0 / 1536	-18.2 -18.2 0.11 (1)	10.00				
S - R	0 / 1354	-18.2 -18.2 0.10 (1)	10.00				
R - Q	0 / 1120	-18.2 -18.2 0.08 (1)	10.00				
Q - P	0 / 1120	-18.2 -18.2 0.08 (1)	10.00				
P - O	-26 / 7	-18.2 -18.2 0.07 (1)	6.25				
O - N	0 / 0	-18.2 -18.2 0.07 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 23.3	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.3	PSF
TOTAL LOAD = 33.6 PSF			

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.26/1.00 (A-Y:1), BC=0.63/1.00 (W-X:1), WB=0.80/1.00 (A-X:1), SSI=0.22/1.00 (X-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	8.0	Edge	1.50
B	TMWW-t	MT20	4.0	5.0	1.50	1.50
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TTW+m	MT20	3.0	4.0	2.00	1.25
H	TMWW-t	MT20	3.0	4.0	1.50	1.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	4.0	4.0	1.75	1.50
L	TMVW-t	MT20	3.0	5.0	1.50	1.75
N	BMV+p	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0	2.25	2.00
P	BMWW-t	MT20	4.0	4.0	2.25	2.00
Q	BS-t	MT20	6.0	8.0		
R	BMWW-t	MT20	4.0	4.0	2.25	2.00
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	6.0	8.0		
U	BMWW-t	MT20	4.0	4.0	2.25	1.75
V	BMWW+t	MT20	4.0	4.0	2.25	2.00
W	BMWW-t	MT20	5.0	5.0		
X	BMWW+t	MT20	6.0	10.0	5.50	1.75
Y	BMV1+t	MT20	6.0	10.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 7139.4 lbs FACTORED DOWN AT 1-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	1-9-8	-7139	-7139	---	BACK	VERT	TOTAL

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

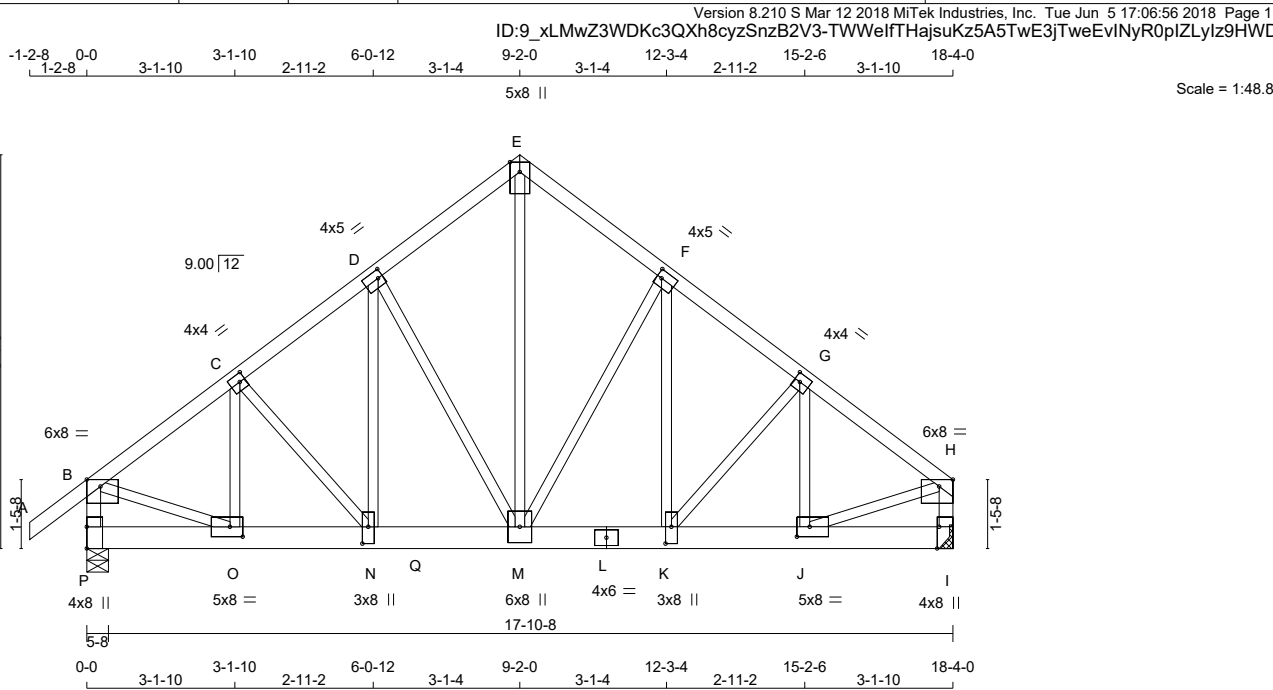
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (W) (INPUT = 0.90)

JSI METAL= 0.62 (X) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 104 = 208 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - E	12	TOP
E - H	12	TOP
P - B	12	TOP
I - H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - L	8	SIDE(334.1)
L - I	8	SIDE(334.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	5403	0	5403	0	5-8	5-8
I	7140	0	7140	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	3794	2643 / 0	0 / 0	0 / 0	0 / 0	1150 / 0	0 / 0
I	5016	3478 / 0	0 / 0	0 / 0	0 / 0	1538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A - B	0 / 30	-77.4 -77.4	0.05 (1)	10.00	M - E	0 / 6221	0.77 (1)
B - C	-5910 / 0	-77.4 -77.4	0.19 (1)	3.89	M - F	-1970 / 0	0.72 (1)
C - D	-6744 / 0	-77.4 -77.4	0.22 (1)	3.64	K - F	0 / 2209	0.27 (1)
D - E	-5385 / 0	-77.4 -77.4	0.18 (1)	4.06	K - G	-509 / 0	0.09 (1)
E - F	-5387 / 0	-77.4 -77.4	0.18 (1)	4.06	J - G	0 / 394	0.05 (1)
F - G	-6584 / 0	-77.4 -77.4	0.21 (1)	3.68	D - M	-2223 / 0	0.81 (1)
G - H	-6996 / 0	-77.4 -77.4	0.25 (1)	3.55	N - D	0 / 2537	0.31 (1)
P - B	-5342 / 0	0.0 0.0	0.30 (1)	5.20	C - N	0 / 1006	0.12 (1)
I - H	-6186 / 0	0.0 0.0	0.35 (1)	4.84	O - C	-1511 / 0	0.17 (1)
					B - O	0 / 4989	0.62 (1)
					J - H	0 / 5902	0.73 (1)
P - O	0 / 0	-18.2 -18.2	0.04 (1)	10.00			
O - N	0 / 4732	-18.2 -18.2	0.39 (1)	10.00			
N - Q	0 / 5389	-18.2 -18.2	0.64 (1)	10.00			
Q - M	0 / 5389	-686.5 -686.5	0.64 (1)	10.00			
M - L	0 / 5266	-686.5 -686.5	0.63 (1)	10.00			
L - K	0 / 5266	-686.5 -686.5	0.63 (1)	10.00			
K - J	0 / 5598	-686.5 -686.5	0.53 (1)	10.00			
J - I	0 / 0	-686.5 -686.5	0.24 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	6-0-12	-3116	-3116	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 7-0-0
START SPAN CARRIED = 30-5-0
END DISTANCE = 18-4-0
END SPAN CARRIED = 30-5-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.35/1.00 (H-I:1), BC=0.64/1.00 (M-N:1), WB=0.81/1.00 (D-M:1), SS=0.43/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW-t	MT20	4.0	5.0	2.00	1.25
E	TTW+p	MT20	5.0	8.0	Edge	
F	TMWW-t	MT20	4.0	5.0	2.00	1.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	6.0	8.0	Edge	3.50
I	BMV1+t	MT20	4.0	8.0	Edge	0.50
J	BMWW-t	MT20	5.0	8.0	2.50	3.25
K	BMWW+t	MT20	3.0	8.0	4.25	1.50
L	BS-t	MT20	4.0	6.0		
M	BMWWWW+t	MT20	6.0	8.0		
N	BMWW+t	MT20	3.0	8.0	4.25	1.50
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BMV1+t	MT20	4.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3115.7 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

NAIL VALUES

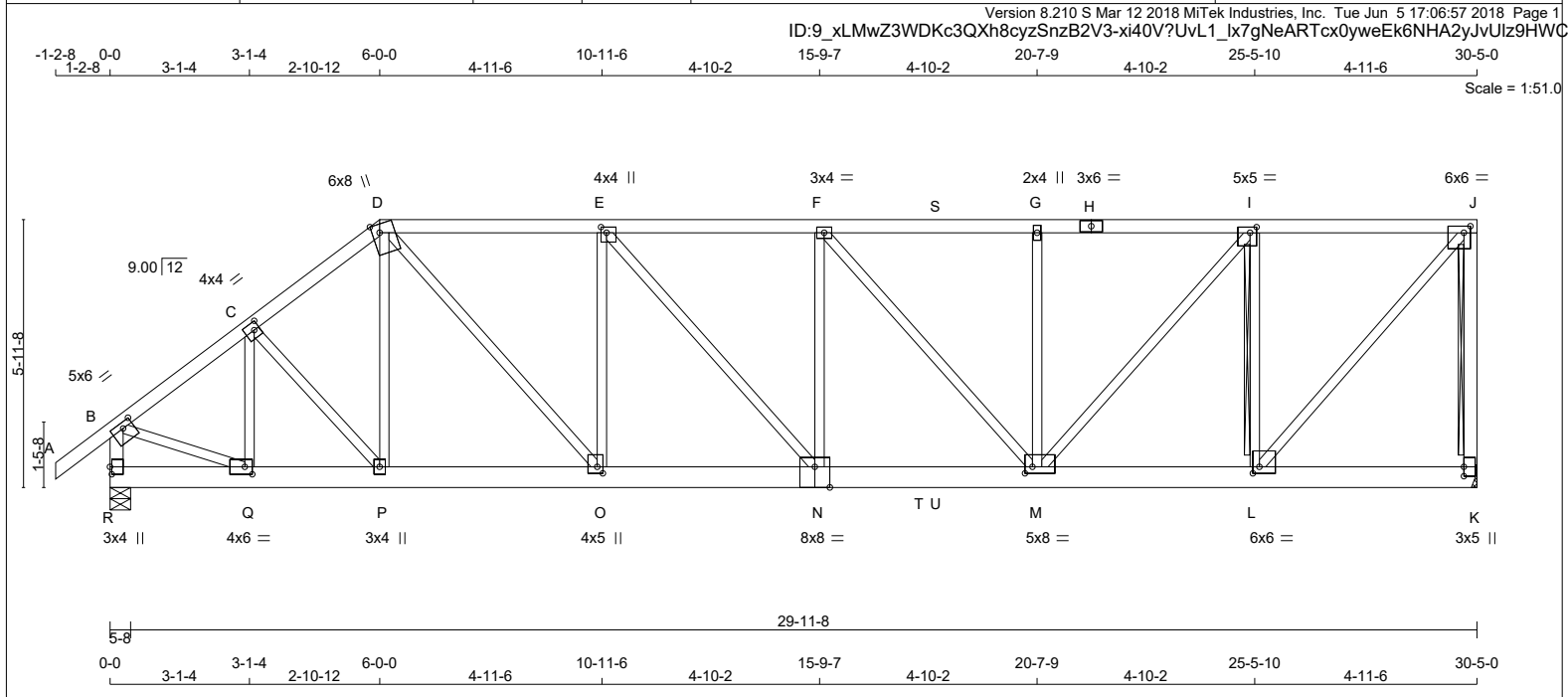
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.79 (L) (INPUT = 1.00)





										TOTAL WEIGHT = 157 lb					
										[M]					
LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							DESIGN CRITERIA			
N. L. G. A. RULES															
CHORDS		SIZE		LUMBER	DESCR.	BEARINGS							*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE		
A - D		2x4	DRY	No.2	SPF	FACTORED		MAXIMUM FACTORED		INPUT	REQRD				
D - H		2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG	BRG				
H - J		2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX			IN-SX
K - J		2x4	DRY	No.2	SPF	K	3116	0	3116	0	0	MECHANICAL			
R - B		2x4	DRY	No.2	SPF	R	2333	0	2333	0	0	5-8	5-8		
R - N		2x6	DRY	2100F 1.8E	SPF	A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.								SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF	
N - K		2x6	DRY	2100F 1.8E	SPF										
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF										

DRY: SEASONED LUMBER.

		MAX./MIN. COMPONENT REACTIONS						SPACING =	24.0	IN. C/C
1ST LCASE										
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
K	2189	1518 / 0	0 / 0	0 / 0	0 / 0	671 / 0	0 / 0			
R	1637	1148 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT J-K, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

HANGERS NOTES	CHORDS				WEBS				THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.01") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23") ALLOWABLE DEFL.(TL)= L/360 (1.01") CALCULATED VERT. DEFL.(TL)= L/ 908 (0.40") CSI: TC=0.95/1.00 (G-I:1) , BC=0.76/1.00 (M-N:1) , WB=0.90/1.00 (J-L:1) , SSI=0.55/1.00 (M-N:1)
	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.	FR-TO		FROM	TO		FR-TO			
	A- B	0 / 30	-77.4	-77.4	0.10 (1)	10.00	Q- C	-690 / 0	0.16 (1)
	B- C	-2373 / 0	-77.4	-77.4	0.19 (1)	4.27	C- P	0 / 320	0.08 (1)
	C- D	-2674 / 0	-77.4	-77.4	0.21 (1)	4.03	P- D	-45 / 40	0.02 (1)
	D- E	-3359 / 0	-77.4	-77.4	0.53 (1)	3.36	D- O	0 / 1865	0.46 (1)
	E- F	-4296 / 0	-77.4	-77.4	0.67 (1)	2.86	O- E	-1490 / 0	0.79 (1)
	F- S	-4029 / 0	-77.4	-77.4	0.69 (1)	2.89	E- N	0 / 1430	0.35 (1)
	S- G	-4029 / 0	-145.9	-145.9	0.69 (1)	2.89	N- F	-78 / 50	0.04 (1)
	G- H	-4029 / 0	-145.9	-145.9	0.95 (1)	2.42	F- M	-408 / 0	0.43 (1)
	H- I	-4029 / 0	-145.9	-145.9	0.95 (1)	2.42	M- G	-643 / 0	0.34 (1)
	I- J	-2419 / 0	-145.9	-145.9	0.72 (1)	3.49	M- I	0 / 2458	0.61 (1)
	K- J	-3018 / 0	0.0	0.0	0.60 (1)	7.81	L- I	-2674 / 0	0.70 (1)
	R- B	-2274 / 0	0.0	0.0	0.25 (1)	5.60	L- J	0 / 3648	0.90 (1)
							B- Q	0 / 2014	0.50 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	18-0-12	-1425	-1425	---	FRONT	VERT	TOTAL

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	G04	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 8 OF 47

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ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-xi40V?UvL1_lx7gNeARTcx0yweEk6NHA2yJvUlz9HWC

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1424.8 lbs FACTORED DOWN AT 18'-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

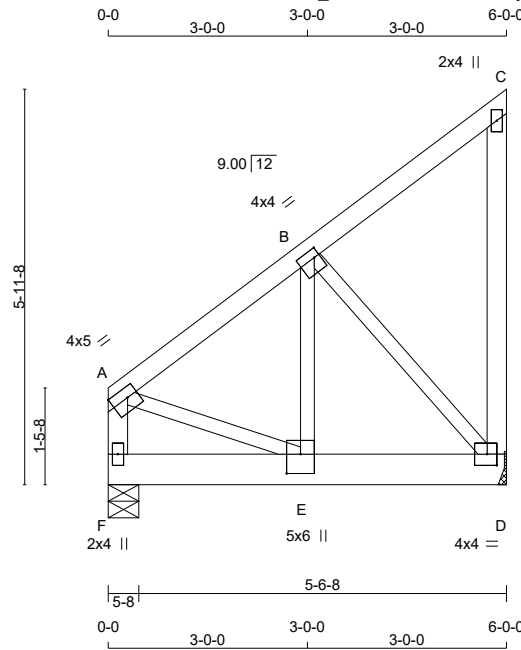
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.69 (N) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	G05	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 9 OF 47

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Scale = 1:34.7

TOTAL WEIGHT = 34 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	5.0	1.75 Edge
B	TMWW-t	MT20	4.0	4.0	1.50 1.00
C	TMV-p	MT20	2.0	4.0	
D	BMVW1-t	MT20	4.0	4.0	2.00 1.75
E	BMWW-t	MT20	5.0	6.0	3.50 2.50
F	BMV1-p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
D	1425	0	1425	0	0	MECHANICAL
F	1425	0	1425	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	1001	694 / 0	0 / 0	0 / 0	0 / 0	0 / 0	307 / 0
F	1001	694 / 0	0 / 0	0 / 0	0 / 0	0 / 0	307 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO	
A-B	-944 / 0	-77.4 -77.4 0.13 (1)	6.21	E-B 0 / 1137
B-C	-15 / 0	-77.4 -77.4 0.11 (1)	6.25	B-D -1140 / 0
D-C	-92 / 0	0.0 0.0 0.06 (1)	7.81	A-E 0 / 813
F-A	-935 / 0	0.0 0.0 0.10 (1)	7.81	
F-E	0 / 0	-397.7 -397.7 0.23 (1)	10.00	
E-D	0 / 767	-397.7 -397.7 0.33 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 18-0-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 18-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.13/1.00 (A-B:1), BC=0.33/1.00 (D-E:1), WB=0.36/1.00 (B-D:1), SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	G05	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 10 OF 47
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 17:06:58 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-PvePJLVX6K6cZHFZCuyi88ZKa1gZrz?JHc2S1Bz9HWB

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.30 (A) (INPUT = 1.00)

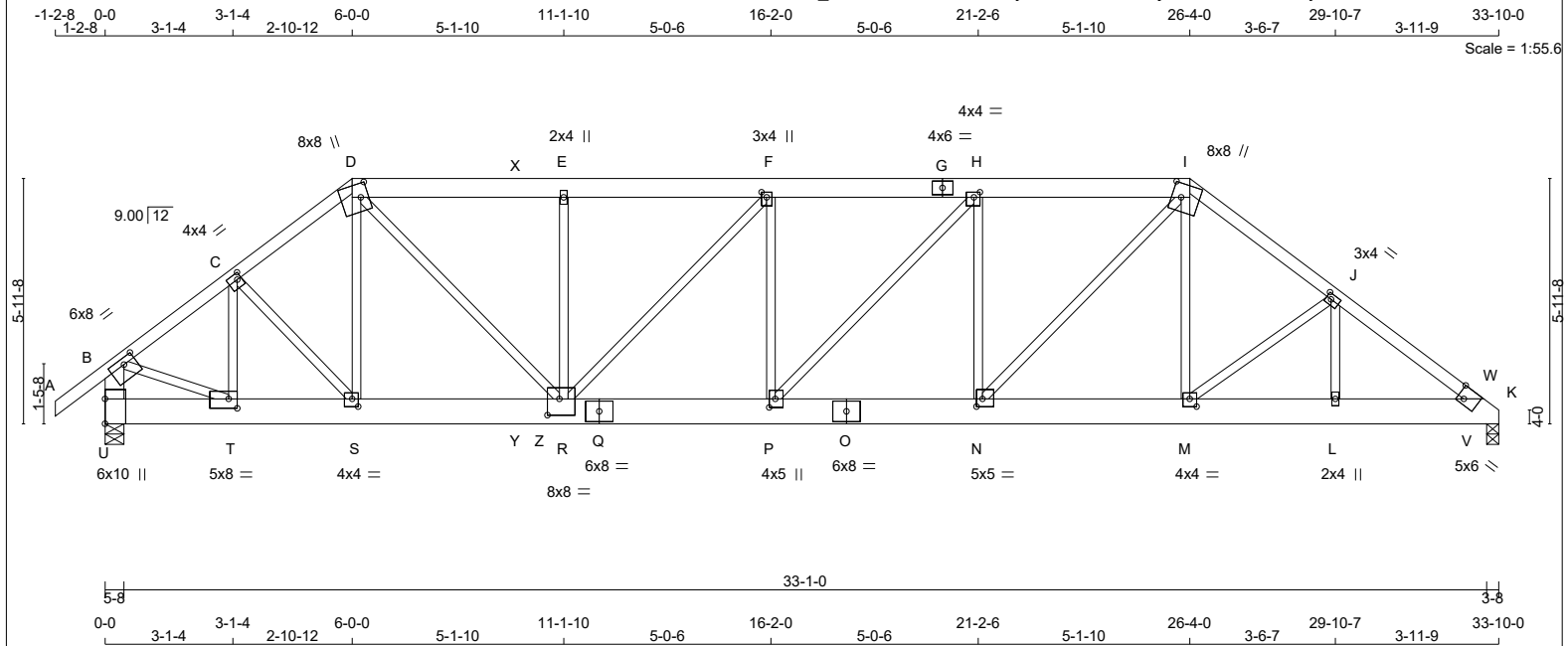
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NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	G07	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 11 OF 47

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ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-PvePJLVX6K6cZHFZCuyi88ZDr1ZcrrOJHc2S1Bz9HWB



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x6	DRY No.2	SPF
G - I	2x6	DRY No.2	SPF
I - K	2x4	DRY No.2	SPF
U - B	2x6	DRY No.2	SPF
U - Q	2x8	DRY No.2	SPF
Q - O	2x8	DRY No.2	SPF
O - K	2x8	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	6.0	8.0	1.75 3.50
C	TMWW-t	MT20	4.0	4.0	1.75 1.25
D	TTWW+m	MT20	8.0	8.0	4.00 2.25
E	TMW+w	MT20	2.0	4.0	
F	TMWW+t	MT20	3.0	4.0	1.50 1.50
G	TS-t	MT20	4.0	6.0	
H	TMWW-t	MT20	4.0	4.0	1.50 1.75
I	TTWW+m	MT20	8.0	8.0	4.00 2.75
J	TMWW-t	MT20	3.0	4.0	1.50 1.50
K	TBM1-h	MT20	5.0	6.0	Edge 2.00
L	BMW+w	MT20	2.0	4.0	
M	BMWW-t	MT20	4.0	4.0	2.25 2.00
N	BMWW-t	MT20	5.0	5.0	2.25 1.75
O	BS-t	MT20	6.0	8.0	
P	BMWW+t	MT20	4.0	5.0	2.50 1.75
Q	BS-t	MT20	6.0	8.0	
R	BMWWWW-t	MT20	8.0	8.0	4.75 3.50
S	BMWW-t	MT20	4.0	4.0	2.25 1.75
T	BMWW-t	MT20	5.0	8.0	2.75 2.50
U	BMV1+t	MT20	6.0	10.0	7.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	JT	HORZ	JT	UPLIFT	JT	IN-SX
K	2389	0	2389	0	0	3-8	3-8
U	3834	0	3834	0	0	5-8	5-8

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	JT	PERM.LIVE
K	1678	1167 / 0	0 / 0
U	2695	1860 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, U

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 30	T-C	-1206 / 0
B-C	-4073 / 0	C-S	0 / 778
C-D	-4730 / 0	S-D	0 / 184
D-X	-5515 / 0	D-R	0 / 2514
X-E	-5515 / 0	R-E	-647 / 0
E-F	-5515 / 0	F-F	0 / 720
F-G	-5025 / 0	P-F	-874 / 0
G-H	-5025 / 0	P-H	0 / 1238
H-I	-4184 / 0	N-H	-1369 / 0
I-J	-3512 / 0	N-I	0 / 1997
J-W	-3794 / 0	M-I	0 / 374
W-K	-3813 / 0	M-J	-304 / 0
U-B	-3678 / 0	L-J	0 / 73
		B-T	0 / 3424
		V-W	-121 / 1
U-T	0 / 0		
T-S	0 / 3267		
S-Y	0 / 3791		
Y-Z	0 / 3791		
Z-R	0 / 3791		
R-Q	0 / 5025		
Q-P	0 / 5025		
P-O	0 / 4184		
O-N	0 / 4184		
N-M	0 / 2815		
M-L	0 / 3048		
L-V	0 / 3048		
V-K	0 / 2971		

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE	
JT	LOC.	JT	MAX+	JT	VERT	JT	TOTAL
D	6-0-0	-464	-464	---	BACK	VERT	TOTAL
Z	10-7-4	-1783	-1783	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH.	LL = 23.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.3 PSF
TOTAL LOAD	= 33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-0-0
RIGHT SETBACK = 7-6-0
END SETBACK = 7-6-0
END WALL WIDTH = 3-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO BACK SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-0-0 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.56/1.00 (D-E:1), BC=0.78/1.00 (R-S:1), WB=0.85/1.00 (B-T:1), SSI=0.95/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	G07	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 12 OF 47

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 17:06:58 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSznB2V3-PvePJLVX6K6cZHFZCuyi88ZDr1ZcrrOJHc2S1Bz9HWB

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 464.2 lbs FACTORED DOWN AT 6-0-0
ON TOP CHORD, AND 1783.4 lbs FACTORED
DOWN AT 10-7-4 ON BOTTOM CHORD. DESIGN
FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354 1667	822 2284 1656

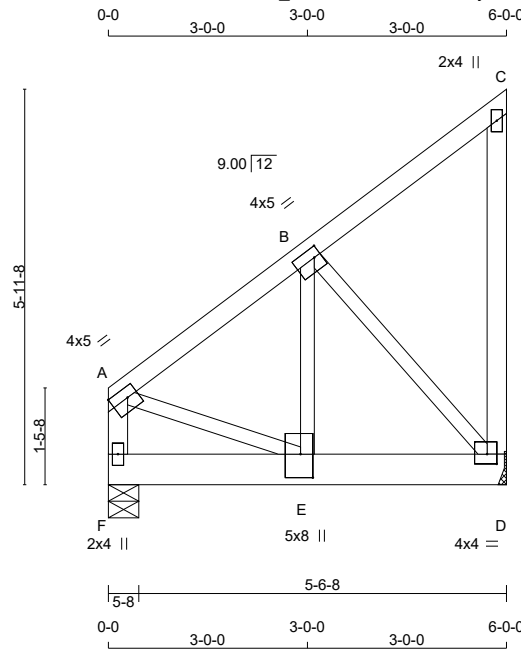
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.84 (K) (INPUT = 1.00)



ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-u5CnwhV9teETBRqlmbTxxM5UHR?GaOqSWGo?Zdz9HWA



Scale = 1:34.7

TOTAL WEIGHT = 34 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	5.0	1.75 Edge
B	TMWW-t	MT20	4.0	5.0	1.75 1.25
C	TMV-p	MT20	2.0	4.0	
D	BMVW1-t	MT20	4.0	4.0	1.75 1.75
E	BMWW-t	MT20	5.0	8.0	4.25 2.25
F	BMV1-p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	1784	0	1784	0	0	MECHANICAL
F	1784	0	1784	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1253	869 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0
F	1253	869 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	-1185 / 0	-77.4	-77.4	0.13 (1)	5.70	E-B	0 / 1495	
B-C	-14 / 0	-77.4	-77.4	0.11 (1)	6.25	B-D	-1426 / 0	
D-C	-93 / 0	0.0	0.0	0.06 (1)	7.81	A-E	0 / 1016	
F-A	-1147 / 0	0.0	0.0	0.13 (1)	7.39			
F-E	0 / 0	-517.2	-517.2	0.31 (1)	10.00			
E-D	0 / 959	-517.2	-517.2	0.43 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 23-2-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 23-2-0
END WALL WIDTH = 3-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.13/1.00 (A-F:1), BC=0.43/1.00 (D-E:1), WB=0.45/1.00 (B-D:1), SSI=0.61/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



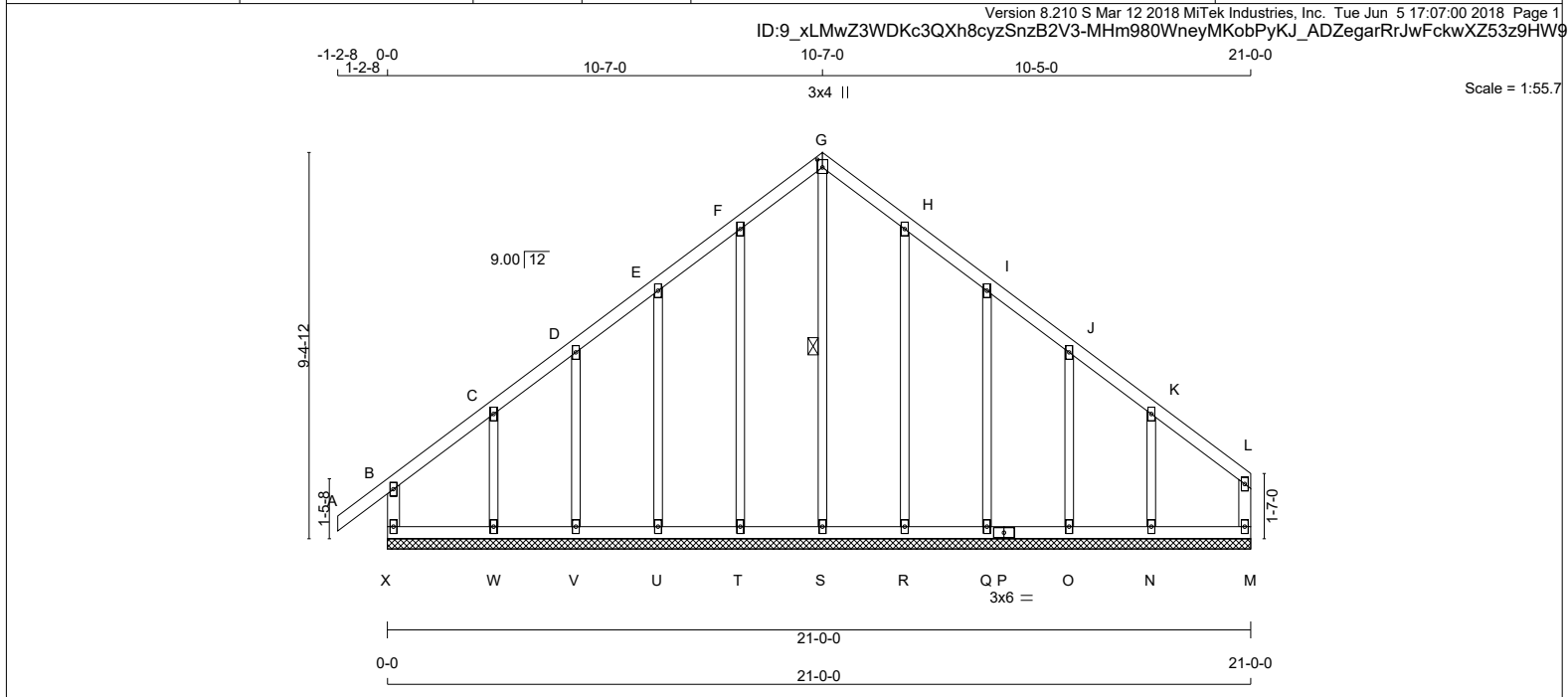
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	DRWG NO.
NE0618-035	G08	1	1	TRUSS DESC.		PAGE 14 OF 47

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 17:06:59 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-u5CnwhV9teETBRqlmbTxbM5UHR?GaOqSWGGo?Zdz9HWA

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.38 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
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THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 99 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
X - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, F, H, I, J, K					
C	TMW+w	MT20	2.0	4.0	
G	TTW+p	MT20	3.0	4.0	2.25 1.50
L	TMV+p	MT20	2.0	4.0	
M	BMV1+p	MT20	2.0	4.0	
N, O, Q, R, S, T, U, V, W					
N	BMW1+w	MT20	2.0	4.0	
P	BS-t	MT20	3.0	6.0	
X	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-S. DBS = 20-0-0 . CBF = 17 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
X-B	-225 / 0	0.0 0.0	0.03 (1)	7.81	S-G	-135 / 0	0.08 (1)
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	T-F	-156 / 0	0.18 (1)
B-C	-42 / 0	-77.4 -77.4	0.06 (1)	6.25	U-E	-154 / 0	0.10 (1)
C-D	-35 / 0	-77.4 -77.4	0.04 (1)	6.25	V-D	-148 / 0	0.05 (1)
D-E	-29 / 0	-77.4 -77.4	0.04 (1)	6.25	W-C	-169 / 0	0.03 (1)
E-F	-26 / 0	-77.4 -77.4	0.04 (1)	6.25	R-H	-156 / 0	0.18 (1)
F-G	-23 / 0	-77.4 -77.4	0.04 (1)	6.25	Q-I	-154 / 0	0.10 (1)
G-H	-23 / 0	-77.4 -77.4	0.04 (1)	6.25	O-J	-148 / 0	0.05 (1)
H-I	-26 / 0	-77.4 -77.4	0.04 (1)	6.25	N-K	-170 / 0	0.03 (1)
I-J	-29 / 0	-77.4 -77.4	0.04 (1)	6.25			
J-K	-35 / 0	-77.4 -77.4	0.04 (1)	6.25			
K-L	-38 / 0	-77.4 -77.4	0.04 (1)	6.25			
M-L	-112 / 0	0.0 0.0	0.04 (1)	7.81			

X-W	0 / 31	-18.2 -18.2	0.03 (1)	10.00
W-V	0 / 26	-18.2 -18.2	0.02 (4)	10.00
V-U	0 / 23	-18.2 -18.2	0.02 (4)	10.00
U-T	0 / 21	-18.2 -18.2	0.02 (4)	10.00
T-S	0 / 19	-18.2 -18.2	0.02 (4)	10.00
S-R	0 / 19	-18.2 -18.2	0.02 (4)	10.00
R-Q	0 / 21	-18.2 -18.2	0.02 (4)	10.00
Q-P	0 / 23	-18.2 -18.2	0.02 (4)	10.00
P-O	0 / 23	-18.2 -18.2	0.02 (4)	10.00
O-N	0 / 26	-18.2 -18.2	0.02 (4)	10.00
N-M	0 / 31	-18.2 -18.2	0.02 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1) , BC=0.03/1.00 (W-X:1) , WB=0.18/1.00 (H-R:1) , SSI=0.06/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

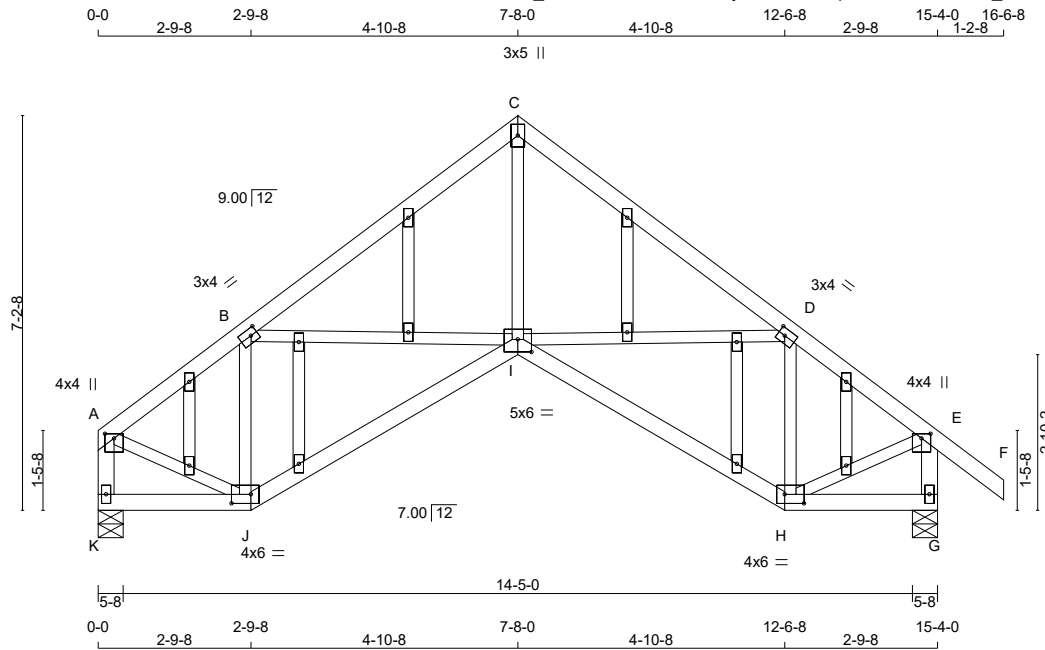
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (G) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 76 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BBWW-p	MT20	4.0	6.0	2.00	4.25
I	BBWW+p	MT20	5.0	6.0	2.75	3.00
J	BBWW-p	MT20	4.0	6.0	2.00	4.25
K	BMV1+p	MT20	2.0	4.0		
L	M, N, O, P, Q, R, S, T, U, V, W					
L	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UP	BRG	BRG
K	733	0	733	0	0	5-8	5-8	5-8	5-8
G	833	0	833	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	515	357 / 0	0 / 0	0 / 0	0 / 0	158 / 0	0 / 0
G	583	417 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-886 / 0	-77.4 -77.4	0.18 (1)	6.25	I-C	0 / 733	0.17 (1)
B-C	-896 / 0	-77.4 -77.4	0.23 (1)	6.23	I-D	0 / 137	0.03 (1)
C-D	-896 / 0	-77.4 -77.4	0.23 (1)	6.23	H-D	-492 / 0	0.10 (1)
D-E	-886 / 0	-77.4 -77.4	0.18 (1)	6.25	B-I	0 / 137	0.03 (1)
E-F	0 / 30	-77.4 -77.4	0.09 (1)	10.00	J-B	-492 / 0	0.10 (1)
K-A	-708 / 0	0.0 0.0	0.07 (1)	7.81	A-J	0 / 624	0.14 (1)
G-E	-808 / 0	0.0 0.0	0.08 (1)	7.81	H-E	0 / 624	0.14 (1)
K-J	0 / 0	-18.2 -18.2	0.04 (4)	10.00			
J-I	0 / 653	-18.3 -18.3	0.18 (1)	10.00			
I-H	0 / 653	-18.2 -18.2	0.18 (1)	10.00			
H-G	0 / 0	-18.2 -18.2	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL		LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.23/1.00 (C-D:1), BC=0.18/1.00 (I-J:1), WB=0.17/1.00 (C-I:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

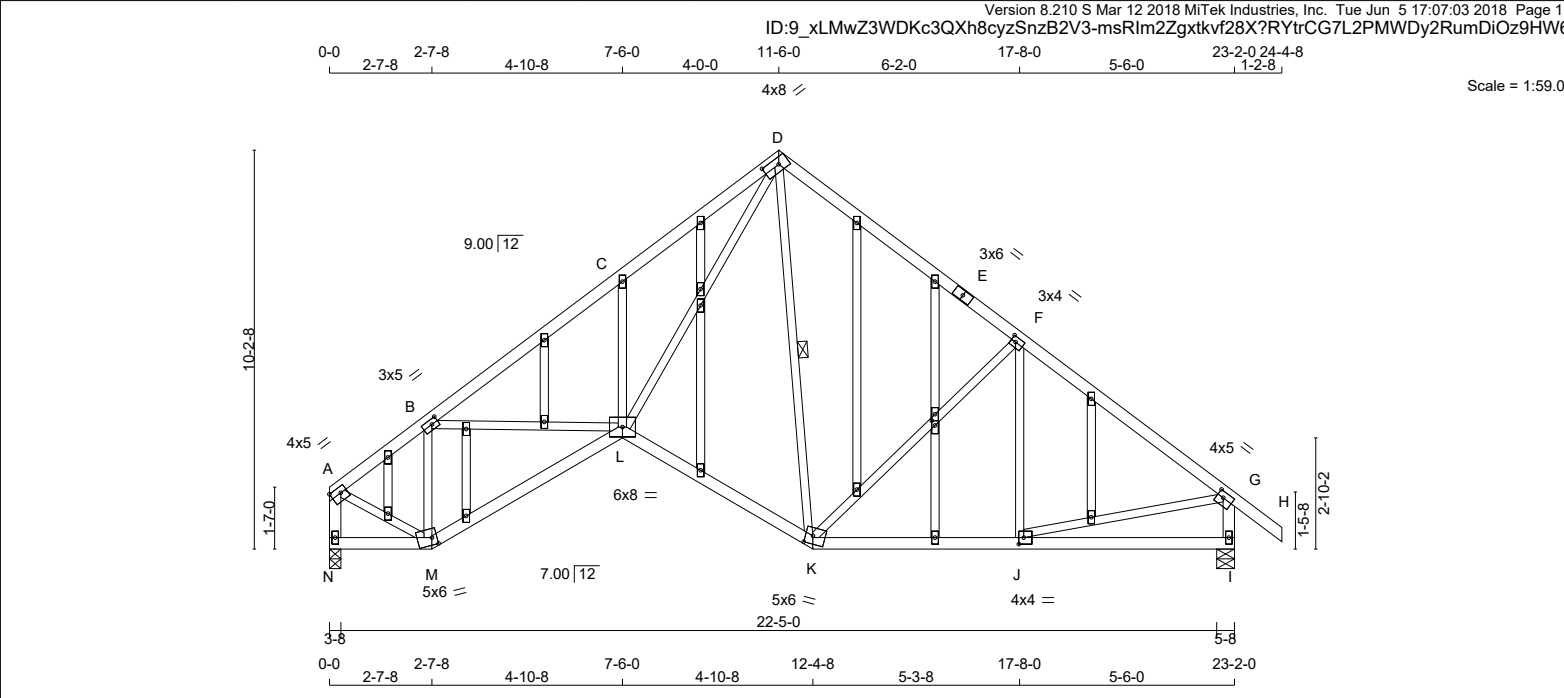
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





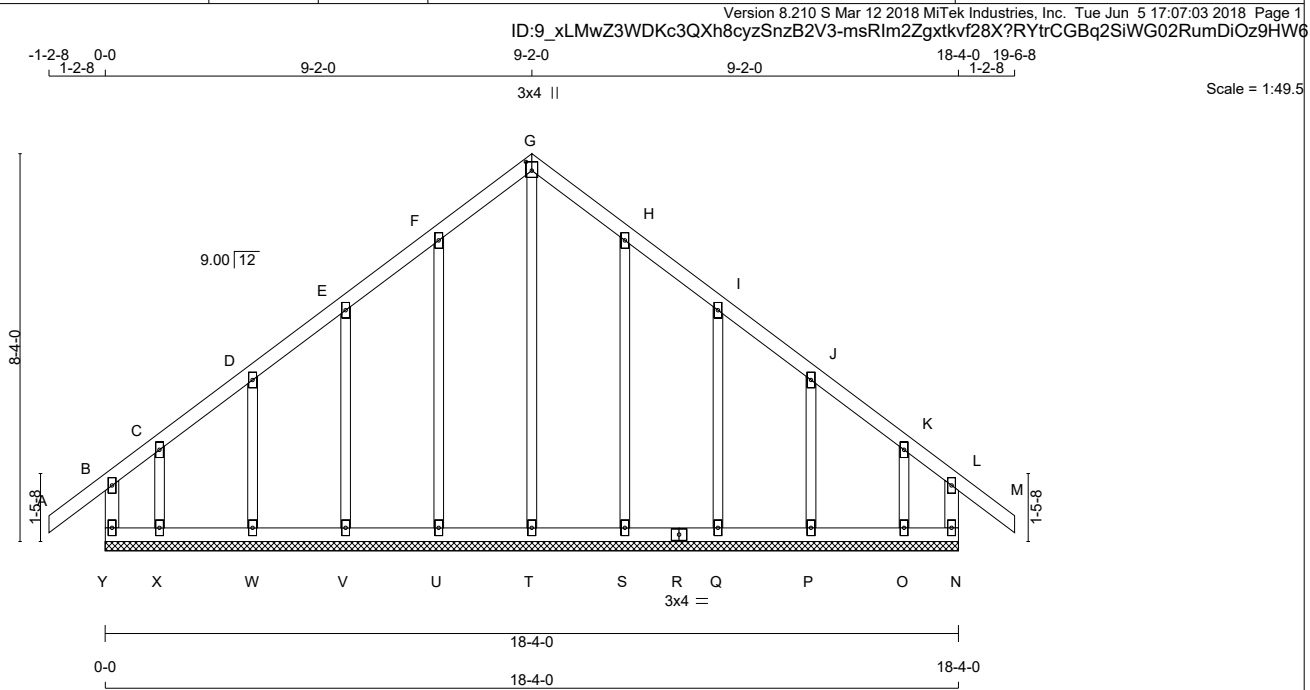
LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					TOTAL WEIGHT = 129 LBS				
N. L. G. A. RULES					BEARINGS					[M][F]				
CHORDS SIZE					FACTORED					DESIGN CRITERIA				
LUMBER					MAXIMUM FACTORED					SPECIFIED LOADS:				
DESCR.					INPUT					TOP CH. LL = 23.3 PSF				
SPF					BRG					DL = 3.0 PSF				
A - D 2x4 DRY No.2					IN-SX					BOT CH. LL = 0.0 PSF				
D - E 2x4 DRY No.2					IN-SX					DL = 7.3 PSF				
E - H 2x4 DRY No.2										TOTAL LOAD = 33.6 PSF				
N - A 2x4 DRY No.2														
I - G 2x4 DRY No.2														
N - M 2x4 DRY No.2														
M - L 2x4 DRY No.2														
L - K 2x4 DRY No.2														
K - I 2x4 DRY No.2														
ALL WEBS 2x3 DRY No.2														
EXCEPT														
ALL GABLE WEBS 2x3 DRY No.2														
DRY: SEASONED LUMBER.														
GABLE STUDS SPACED AT 2'-0-0 OC.														
PLATES (table is in inches)														
JT TYPE PLATES W LEN Y X														
A TMVW-t MT20 4.0 5.0 1.75 Edge														
B TMWW-t MT20 3.0 5.0 1.50 2.00														
C TMW+w MT20 2.0 4.0														
D TTWW-h MT20 4.0 8.0 2.00 5.00														
E TS-t MT20 3.0 6.0														
F TMVW-t MT20 3.0 4.0 1.50 5.00														
G TMVW-t MT20 4.0 5.0 1.75 2.00														
I BMV1+p MT20 2.0 4.0														
J BMVW-t MT20 4.0 4.0 2.00 1.50														
K BBWW-m MT20 5.0 6.0 2.50 2.25														
L BBWWV-p MT20 6.0 8.0														
M BBWW-m MT20 5.0 6.0 2.25 1.50														
N BMV1+p MT20 2.0 4.0														
O, O, P, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AC, AD														
O NP+w MT20 2.0 4.0														
AB NP+w MT20 2.0 4.0 1.50 1.00														
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.														
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.														
N - M 0 / 0 -18.2 -18.2 0.04 (4) 10.00														
M - L 0 / 947 -18.2 -18.2 0.23 (1) 10.00														
L - K 0 / 790 -18.2 -18.2 0.20 (1) 10.00														
K - J 0 / 956 -18.2 -18.2 0.23 (1) 10.00														
J - I 0 / 0 -18.2 -18.2 0.15 (4) 10.00														
CHORDS					WEBS					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010				
MAX. FACTORED FORCE (LBS)					MAX. FACTORED FORCE (LBS)					THIS DESIGN COMPLIES WITH:				
MEMB. VERT. LOAD (PLF)					MEMB. VERT. LOAD (PLF)					- PART 9 OF OBC 2012, BCBC 2012, ABC 2014				
FR-TO					FR-TO					- CSA 086-09				
A - B -1027 / 0 -77.4 -77.4 0.14 (1) 6.04					M - B -811 / 0 0.17 (1)					- TPIC 2011				
B - C -1795 / 0 -77.4 -77.4 0.21 (1) 4.83					B - L 0 / 621 0.14 (1)					(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD				
C - D -1807 / 0 -77.4 -77.4 0.19 (1) 4.82					L - C -378 / 0 0.11 (1)					ALLOWABLE DEFL.(LL)= L/360 (0.77")				
D - E -880 / 0 -77.4 -77.4 0.38 (1) 6.04					L - D 0 / 1508 0.34 (1)					CALCULATED VERT. DEFL.(TL)= L/999 (0.14")				
E - F -880 / 0 -77.4 -77.4 0.38 (1) 6.04					D - K -40 / 51 0.03 (1)					CSI: TC=0.38/1.00 (D-F:1), BC=0.23/1.00 (J-K:1), WB=0.44/1.00 (F-K:1), SSI=0.18/1.00 (D-F:1)				
F - G -1160 / 0 -77.4 -77.4 0.37 (1) 5.47					K - F -391 / 0 0.44 (1)					DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10				
G - H 0 / 30 -77.4 -77.4 0.09 (1) 10.00					J - F -79 / 77 0.04 (1)					COMP=1.10 SHEAR=1.10 TENS= 1.10				
N - A -1084 / 0 0.0 0.0 0.11 (1) 7.58					A - M 0 / 934 0.21 (1)					COMPANION LIVE LOAD FACTOR = 0.50				
I - G -1168 / 0 0.0 0.0 0.12 (1) 7.38					J - G 0 / 976 0.22 (1)					TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .				
N - M 0 / 0 -18.2 -18.2 0.04 (4) 10.00										NAIL VALUES				
M - L 0 / 947 -18.2 -18.2 0.23 (1) 10.00										PLATE GRIP(DRY) SHEAR SECTION				
L - K 0 / 790 -18.2 -18.2 0.20 (1) 10.00										(PSI) (PLI) (PLI)				
K - J 0 / 956 -18.2 -18.2 0.23 (1) 10.00										MAX MIN MAX MIN MAX MIN				
J - I 0 / 0 -18.2 -18.2 0.15 (4) 10.00										MT20 618 354 1667 822 2284 1656				
										PLATE PLACEMENT TOL. = 0.250 inches				
										PLATE ROTATION TOL. = 5.0 Deg.				
										JSI GRIP= 0.88 (M) (INPUT = 0.90)				
										JSI METAL= 0.40 (G) (INPUT = 1.00)				

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	DRWG NO.
NE0618-035	GE04	1	1	TRUSS DESC.		PAGE 19 OF 47

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 17:07:03 2018 Page 2
ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-msRIm2Zgxtkvf28X?RYtrCG7L2PMWDy2RumDiOz9HW6

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
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SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 86 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
Y - B	2x4	DRY	No.2	SPP
A - G	2x4	DRY	No.2	SPP
G - M	2x4	DRY	No.2	SPP
N - L	2x4	DRY	No.2	SPP
Y - R	2x4	DRY	No.2	SPP
R - N	2x4	DRY	No.2	SPP
ALL WEBS	2x3	DRY	No.2	SPP
ALL GABLE WEBS	2x3	DRY	No.2	SPP
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, F, H, I, J, K					
G	TMW+w	MT20	2.0	4.0	
C	TTW+p	MT20	3.0	4.0	2.25 1.50
L	TMV+p	MT20	2.0	4.0	
N	BMV1+p	MT20	2.0	4.0	
O, P, Q, S, T, U, V, W, X					
O	BMW1+w	MT20	2.0	4.0	
R	BS-t	MT20	3.0	4.0	
Y	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VER. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
Y-B	-172 / 0	0.0	0.0	0.02 (1)	7.81	T-G	-185 / 0	0.25 (1)
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	U-F	-160 / 0	0.13 (1)
B-C	-14 / 0	-77.4	-77.4	0.07 (1)	6.25	V-E	-149 / 0	0.06 (1)
C-D	0 / 15	-77.4	-77.4	0.04 (1)	10.00	W-D	-162 / 0	0.04 (1)
D-E	0 / 14	-77.4	-77.4	0.04 (1)	10.00	X-C	-75 / 0	0.01 (1)
E-F	0 / 20	-77.4	-77.4	0.04 (1)	10.00	S-H	-160 / 0	0.13 (1)
F-G	0 / 21	-77.4	-77.4	0.04 (1)	10.00	Q-I	-149 / 0	0.06 (1)
G-H	0 / 21	-77.4	-77.4	0.04 (1)	10.00	P-J	-162 / 0	0.04 (1)
H-I	0 / 20	-77.4	-77.4	0.04 (1)	10.00	O-K	-75 / 0	0.01 (1)
I-J	0 / 14	-77.4	-77.4	0.04 (1)	10.00			
J-K	0 / 15	-77.4	-77.4	0.04 (1)	10.00			
K-L	-14 / 0	-77.4	-77.4	0.07 (1)	6.25			
L-M	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
N-L	-172 / 0	0.0	0.0	0.02 (1)	7.81			
Y-X	-5 / 0	-18.2	-18.2	0.01 (1)	10.00			
X-W	-8 / 0	-18.2	-18.2	0.02 (4)	10.00			
W-V	-12 / 0	-18.2	-18.2	0.02 (4)	6.25			
V-U	-15 / 0	-18.2	-18.2	0.01 (4)	6.25			
U-T	-17 / 0	-18.2	-18.2	0.01 (4)	6.25			
T-S	-17 / 0	-18.2	-18.2	0.01 (4)	6.25			
S-R	-15 / 0	-18.2	-18.2	0.01 (4)	6.25			
R-Q	-15 / 0	-18.2	-18.2	0.01 (4)	6.25			
Q-P	-12 / 0	-18.2	-18.2	0.02 (4)	6.25			
P-O	-8 / 0	-18.2	-18.2	0.02 (4)	10.00			
O-N	-5 / 0	-18.2	-18.2	0.01 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (P-Q:4), WB=0.25/1.00 (G-T:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

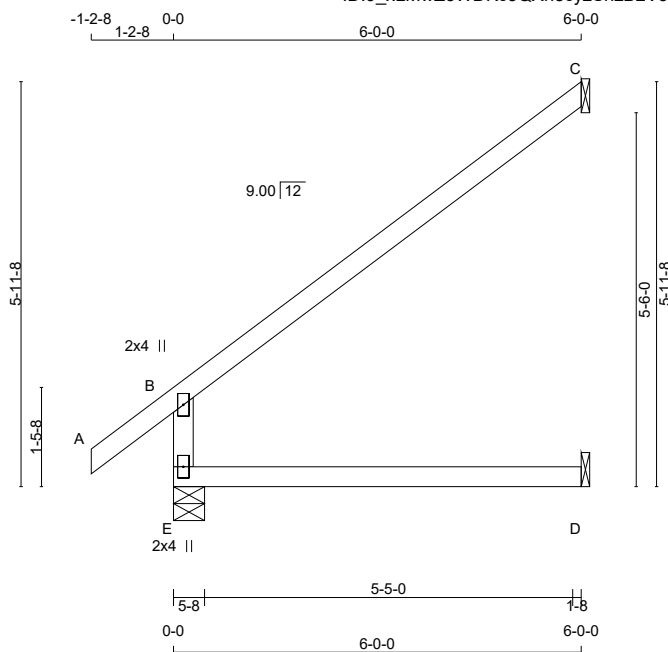
JSI GRIP= 0.51 (G) (INPUT = 0.90)
JSI METAL= 0.06 (G) (INPUT = 1.00)

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ID:9 xLMwZ3WPKc3QXh8cvzSnpB2V3-E2?qzOZIhAsmHCijZ936OPpIKSm3FI6CfYVmErz9HW5

Scale = 1:33.9



TOTAL WEIGHT = $9 \times 19 = 167 \text{ lb}$

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

BUILDING DESIGNER
BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
----------	------------------	-------	------

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	454	0	454	0	0	5-8	5-8
C	174	0	174	0	0	1-8	1-8
D	46	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
1	2	3	4
5	6	7	8
9	10	11	12
13	14	15	16
17	18	19	20
21	22	23	24
25	26	27	28
29	30	31	32
33	34	35	36
37	38	39	40
41	42	43	44
45	46	47	48
49	50	51	52
53	54	55	56
57	58	59	60
61	62	63	64
65	66	67	68
69	70	71	72
73	74	75	76
77	78	79	80
81	82	83	84
85	86	87	88
89	90	91	92
93	94	95	96
97	98	99	100

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	316	235 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LOAD CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LOAD CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-390 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
B-C	-35 / 0	-77.4	-77.4 0.37 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

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TOP CH. LL = 23.3 PSF

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	33.6 PSF

<u>STATION</u>	<u>TIME</u>	<u>WIND</u>
1	10:00	10:00
2	10:00	10:00
3	10:00	10:00
4	10:00	10:00
5	10:00	10:00
6	10:00	10:00
7	10:00	10:00
8	10:00	10:00
9	10:00	10:00
10	10:00	10:00
11	10:00	10:00
12	10:00	10:00
13	10:00	10:00
14	10:00	10:00
15	10:00	10:00
16	10:00	10:00
17	10:00	10:00
18	10:00	10:00
19	10:00	10:00
20	10:00	10:00
21	10:00	10:00
22	10:00	10:00
23	10:00	10:00
24	10:00	10:00
25	10:00	10:00
26	10:00	10:00
27	10:00	10:00
28	10:00	10:00
29	10:00	10:00
30	10:00	10:00
31	10:00	10:00
32	10:00	10:00
33	10:00	10:00
34	10:00	10:00
35	10:00	10:00
36	10:00	10:00
37	10:00	10:00
38	10:00	10:00
39	10:00	10:00
40	10:00	10:00
41	10:00	10:00
42	10:00	10:00
43	10:00	10:00
44	10:00	10:00
45	10:00	10:00
46	10:00	10:00
47	10:00	10:00
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91	10:00	10:00
92	10:00	10:00
93	10:00	10:00
94	10:00	10:00
95	10:00	10:00
96	10:00	10:00
97	10:00	10:00
98	10:00	10:00
99	10:00	10:00
100	10:00	10:00

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIQ 2011

-OVERHANG NOT TO BE ALTERED OR CUT

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
1	2	3	4

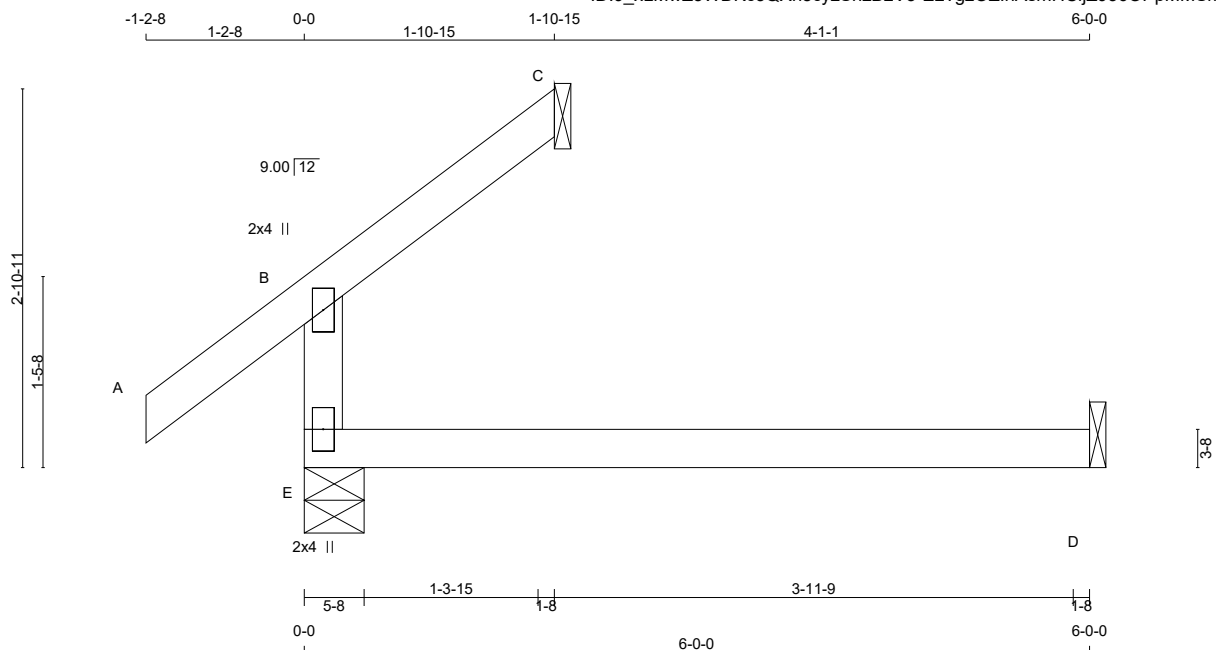
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)





Scale = 1:17.6

TOTAL WEIGHT = 13 lb
[M][F]

LUMBER

CONSIDER N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	256	0	256	0	0	5-8	5-8
C	56	0	56	0	0	1-8	1-8
D	46	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	181	116 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LOAD CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LOAD CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-192 / 0	0.0	0.0 0.11 (4)	7.81			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
B-C	-11 / 0	-77.4	-77.4 0.04 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (B-E:4) , BC=0.14/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

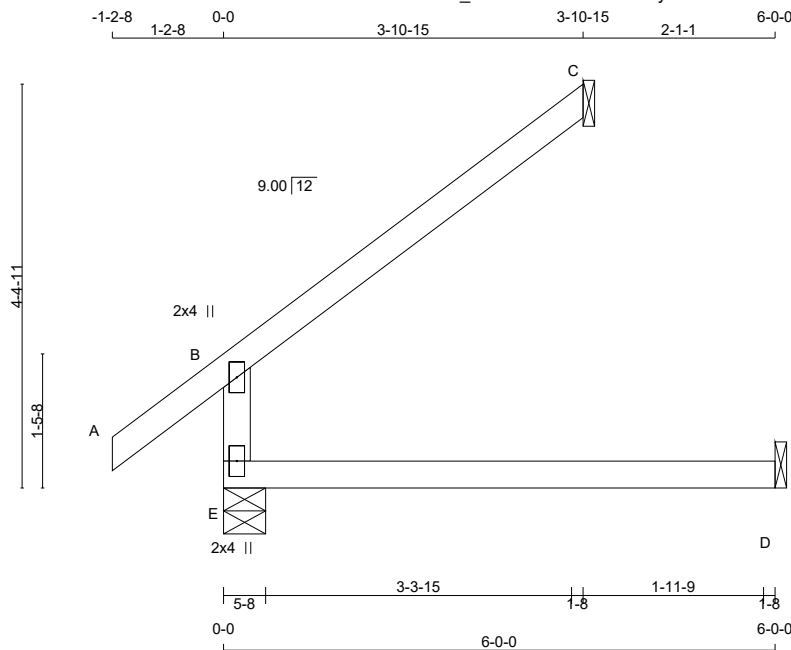


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	DRWG NO.	PAGE 23 OF 47
NE0618-035	J03	1	1	TRUSS DESC.			

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ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-E2?gzOZlhAsmHCijZ936OPpLdSm3FI6CfyVmErz9HW5

Scale = 1:25.1



TOTAL WEIGHT = 16 lb [M][F]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	353	0	353	0	0	5-8	5-8	
C	114	0	114	0	0	1-8	1-8	
D	46	0	52	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	247	174 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MAX. MEMB. UNBRAC LENGTH	FR-TO	MAX. FORCE (LBS)	CS1 (LC)
FR-TO		FROM	TO					
E-B	-289 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-23 / 0	-77.4	-77.4	0.15 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.3	PSF	
TOTAL LOAD =	33.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 822 2284 1656

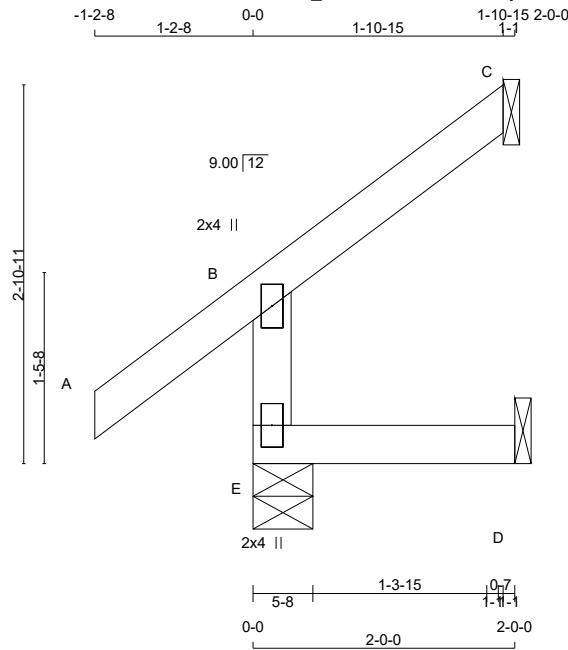
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 8 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	212	0	212	0	5-8	5-8		
C	56	0	56	0	1-8	1-8		
D	17	0	19	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	147	116 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MEMB. UNBRAC LENGTH	FR-TO	MAX. FORCE (LBS)	CS1 (LC)
FR-TO		FROM	TO					
E-B	-192 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

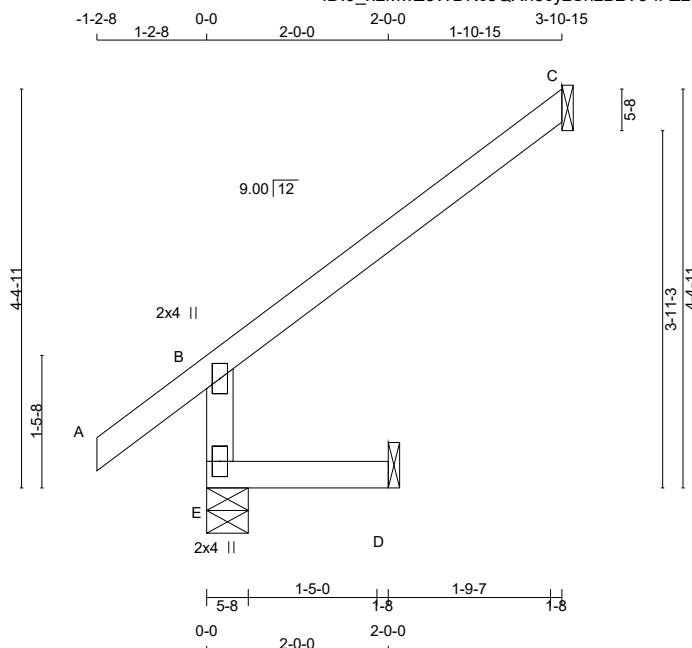
JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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ID:9_xLMwZ3WDKc3QXh8cyzSznB2V3-iFZ2BkawSU_dvMHv6saLwdLVes88_CLLuCFKnHz9HW4

Scale = 1:25.4



TOTAL WEIGHT = 11 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
E	309	0	0	309	0	0	5-8	5-8	
C	114	0	0	114	0	0	1-8	1-8	
D	17	0	0	19	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
E	213	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0		
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0		
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-289 / 0	0.0	0.0 0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
B-C	-23 / 0	-77.4	-77.4 0.20 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.3	PSF	
TOTAL LOAD =	33.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

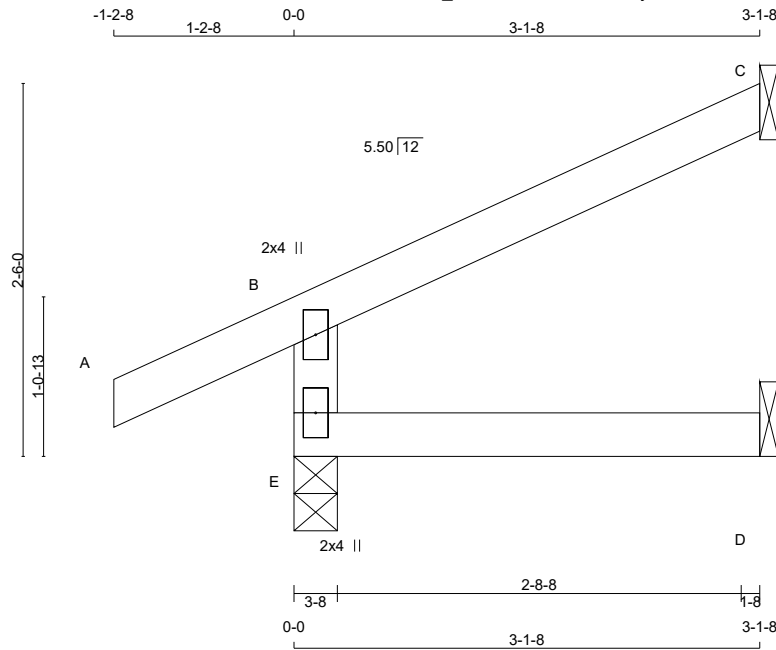
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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ID:9 xLMwZ3WDKc3QXh8cyczSnzB2V3-iFZ2BkawSU dvMHv6saLwdLXns8p CLLuCFKnHz9HW4

Scale = 1:15.5



TOTAL WEIGHT = 4 X 10 = 39 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS	REACTION	GROSS	REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
E	282	0	282	0	0	3-8
C	91	0	91	0	0	1-8
D	24	0	27	0	0	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	195	150 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	62	55 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-249 / 0	0.0	0.0 0.03 (4)	7.81			
A-B	0 / 20	-77.4	-77.4 0.09 (1)	10.00			
B-C	-13 / 0	-77.4	-77.4 0.13 (1)	6.25			
E-D	0 / 0	-18.2	-18.2 0.04 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.04/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

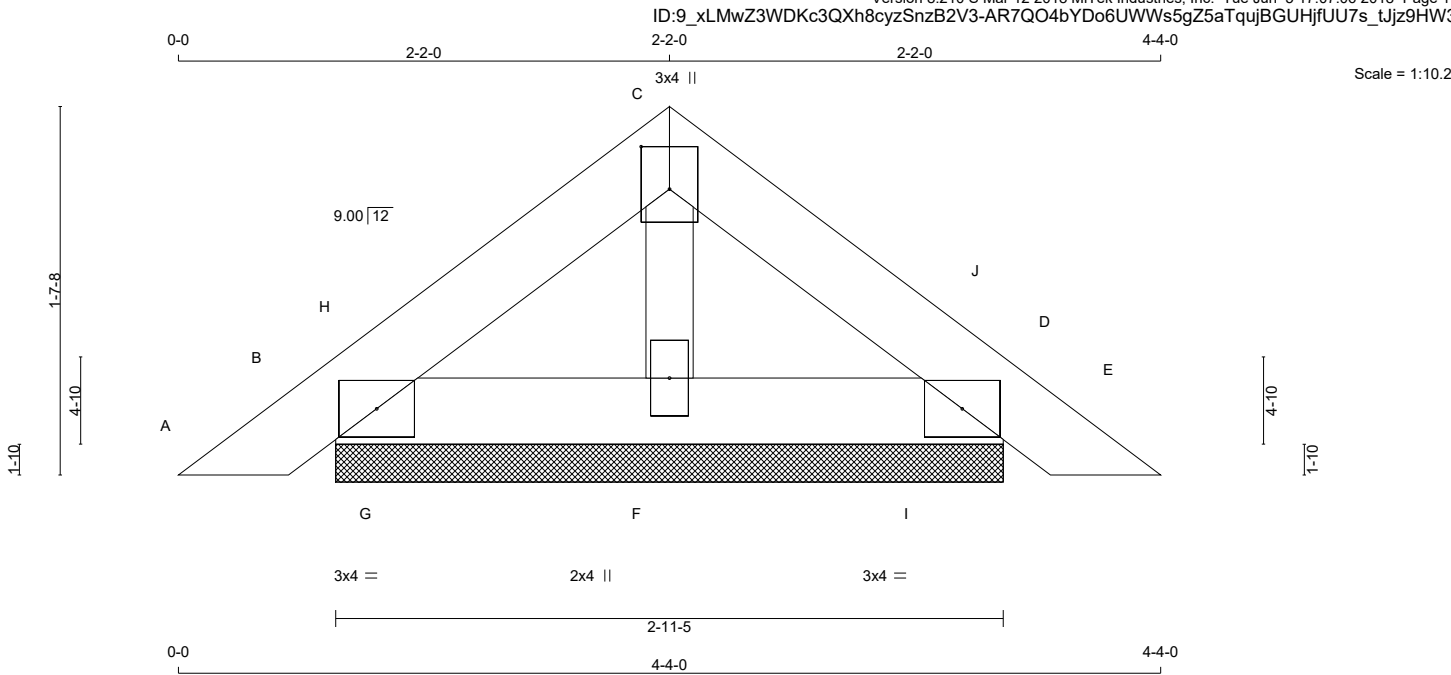
NAIL VALUES PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)





TOTAL WEIGHT = 5 X 10 = 51 lb [M]

LUMBER				DESCR.	SPF
N.L.G.A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	127	0	127	0	0
D	127	0	127	0	0
F	121	0	121	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	88	70/0	0/0	0/0	0/0	18/0	0/0
D	88	70/0	0/0	0/0	0/0	18/0	0/0
F	86	55/0	0/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/13	-77.4	-77.4 0.02 (1)	10.00	F-C	-54/0	0.01 (1)
B-H	-27/0	-77.4	-77.4 0.00 (1)	6.25	G-H	-62/0	0.00 (1)
H-C	-31/0	-77.4	-77.4 0.02 (1)	6.25	I-J	-62/0	0.00 (1)
C-J	-31/0	-77.4	-77.4 0.02 (1)	6.25			
J-D	-27/0	-77.4	-77.4 0.00 (1)	6.25			
D-E	0/13	-77.4	-77.4 0.02 (1)	10.00			
B-G	0/24	-18.2	-18.2 0.02 (1)	10.00			
G-F	0/24	-18.2	-18.2 0.02 (1)	10.00			
F-I	0/24	-18.2	-18.2 0.02 (1)	10.00			
I-D	0/24	-18.2	-18.2 0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.02/1.00 (D-E:1), BC=0.02/1.00 (B-G:1), WB=0.01/1.00 (C-F:1), SSI=0.04/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

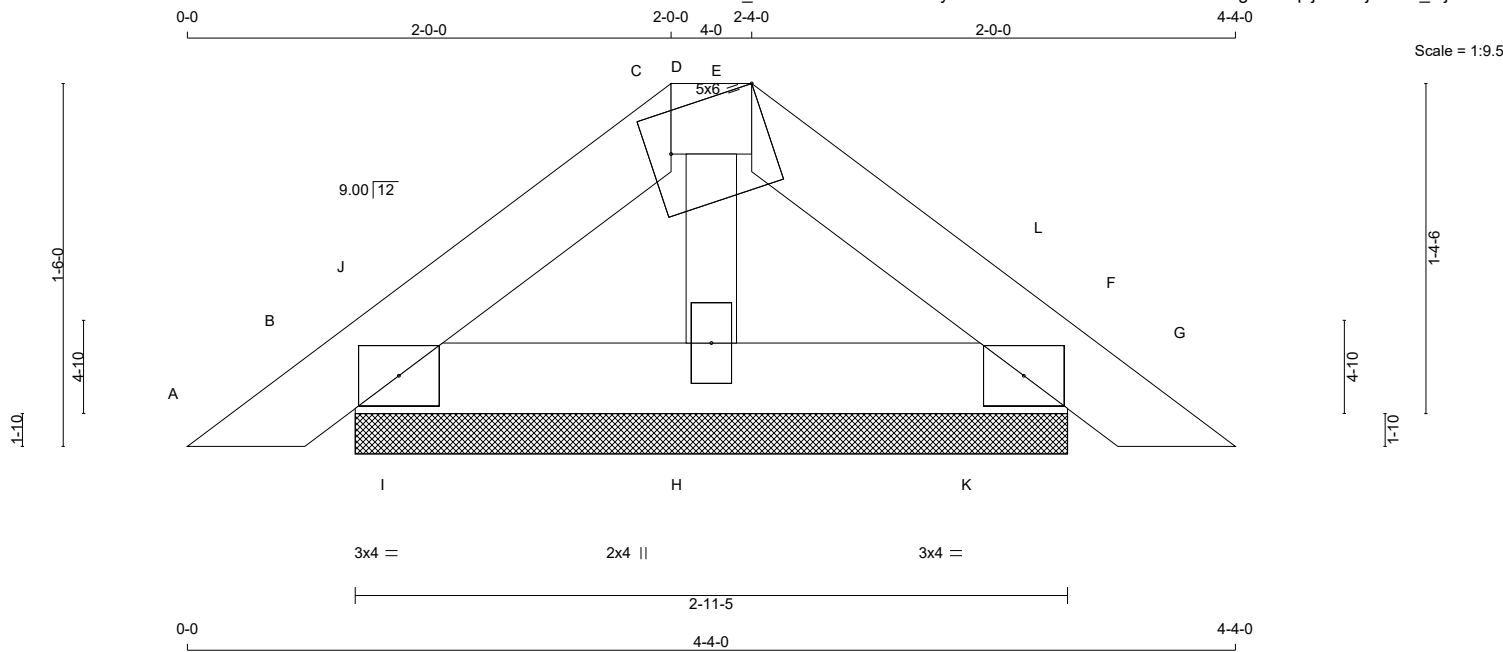
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER					DESCR.
CHORDS	SIZE	DRY	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E						
C	TTTW*-m	MT20	5.0	6.0	Edge 5.00	
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	125	0	125	0	2-11-5	2-11-5
F	125	0	125	0	2-11-5	2-11-5
H	126	0	126	0	2-11-5	2-11-5

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
B	86	68 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
F	86	68 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
H	89	57 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 13	-77.4	-77.4 0.02 (1)	H-D	-67 / 0	0.01 (1)	
B-J	-34 / 0	-77.4	-77.4 0.00 (4)	I-J	-47 / 2	0.00 (1)	
J-C	-32 / 0	-77.4	-77.4 0.01 (1)	K-L	-47 / 2	0.00 (1)	
C-D	-22 / 0	-77.4	-77.4 0.01 (1)				
D-E	-22 / 0	-77.4	-77.4 0.01 (1)				
E-L	-32 / 0	-77.4	-77.4 0.01 (1)				
L-F	-34 / 0	-77.4	-77.4 0.00 (4)				
F-G	0 / 13	-77.4	-77.4 0.02 (1)				
B-I	0 / 25	-18.2	-18.2 0.02 (1)				
I-H	0 / 25	-18.2	-18.2 0.02 (1)				
H-K	0 / 25	-18.2	-18.2 0.02 (1)				
K-F	0 / 25	-18.2	-18.2 0.02 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.02/1.00 (F-G:1), BC=0.02/1.00 (H-K:1), WB=0.01/1.00 (D-H:1), SSI=0.03/1.00 (F-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	822 2284 1656

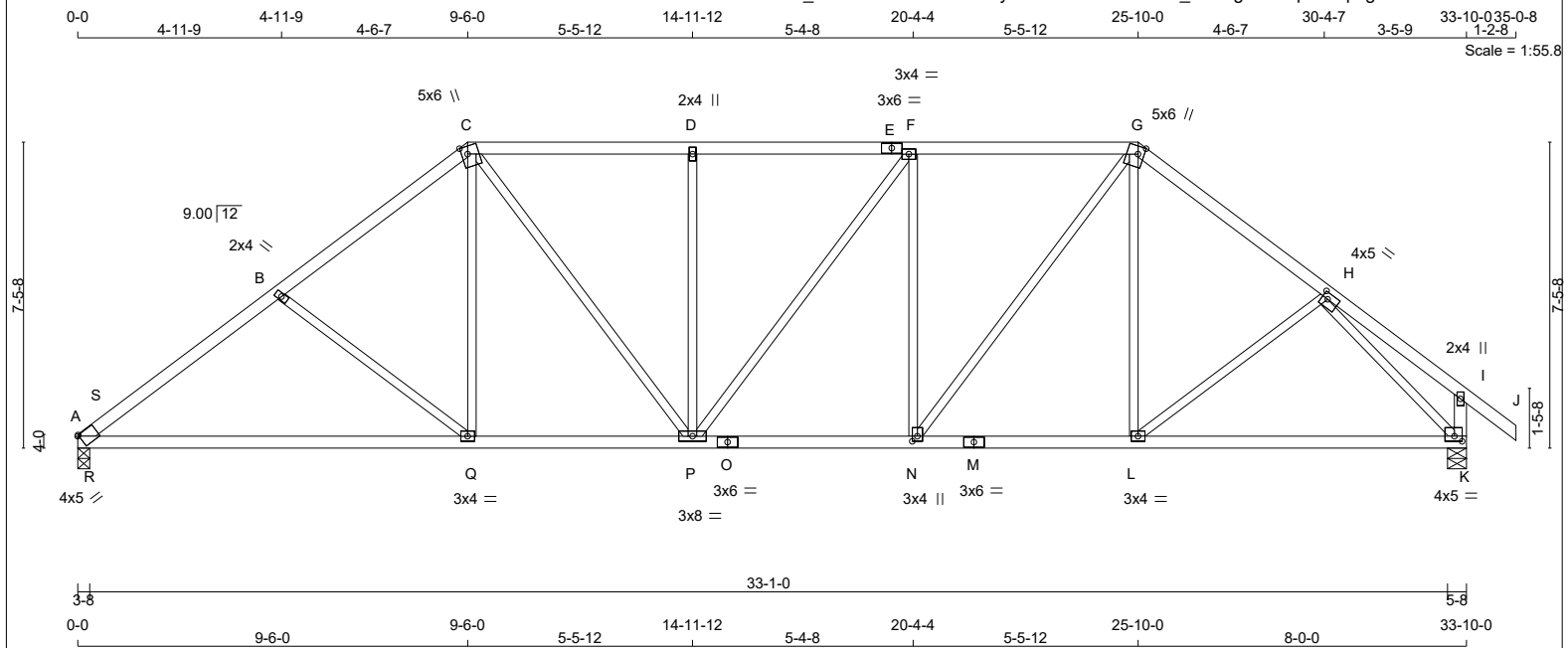
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (F) (INPUT = 0.90)
JSI METAL= 0.02 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 142 lb
[M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
K - I	2x4	DRY	No.2
A - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	4.0	5.0	0.25	0.25
B	TMW+w	MT20	2.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	3.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.75
H	TMWW-t	MT20	4.0	5.0	1.75	1.75
I	TMV+p	MT20	2.0	4.0		
K	BMVW1-t	MT20	4.0	5.0	1.50	2.25
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW+t	MT20	3.0	4.0	1.50	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	UPLIFT	IN-SX	IN-SX
A	1617	0	1617	0	0	3-8	3-8		
K	1718	0	1718	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1136	788 / 0	0 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0
K	1204	848 / 0	0 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-S	-2481 / 0	-77.4 -77.4	0.19 (4)	4.28	B-Q	-387 / 0	0.24 (1)
S-B	-2295 / 0	-77.4 -77.4	0.31 (1)	4.26	Q-C	0 / 405	0.09 (1)
B-C	-1980 / 0	-77.4 -77.4	0.28 (1)	4.55	C-P	0 / 577	0.13 (1)
C-D	-1916 / 0	-77.4 -77.4	0.31 (1)	4.57	P-D	-452 / 0	0.45 (1)
D-E	-1916 / 0	-77.4 -77.4	0.29 (1)	4.60	P-F	0 / 86	0.02 (1)
E-F	-1916 / 0	-77.4 -77.4	0.29 (1)	4.60	N-F	-523 / 0	0.52 (1)
F-G	-1865 / 0	-77.4 -77.4	0.31 (1)	4.62	N-G	0 / 769	0.17 (1)
G-H	-1767 / 0	-77.4 -77.4	0.24 (1)	4.81	L-G	0 / 126	0.04 (4)
H-I	0 / 26	-77.4 -77.4	0.20 (1)	10.00	L-H	0 / 71	0.02 (4)
I-J	0 / 30	-77.4 -77.4	0.09 (1)	10.00	H-K	-1997 / 0	0.78 (1)
K-I	-186 / 0	0.0	0.02 (1)	7.81	R-S	0 / 343	0.00 (1)
A-R	0 / 1865	-18.2 -18.2	0.32 (1)	10.00			
R-Q	0 / 1865	-18.2 -18.2	0.48 (1)	10.00			
Q-P	0 / 1566	-18.2 -18.2	0.43 (1)	10.00			
P-O	0 / 1865	-18.2 -18.2	0.36 (1)	10.00			
O-N	0 / 1865	-18.2 -18.2	0.36 (1)	10.00			
N-M	0 / 1398	-18.2 -18.2	0.37 (1)	10.00			
M-L	0 / 1398	-18.2 -18.2	0.37 (1)	10.00			
L-K	0 / 1352	-18.2 -18.2	0.36 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.31/1.00 (C-D:1), BC=0.48/1.00 (Q-R:1), WB=0.78/1.00 (H-K:1), SSI=0.31/1.00 (A-S:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

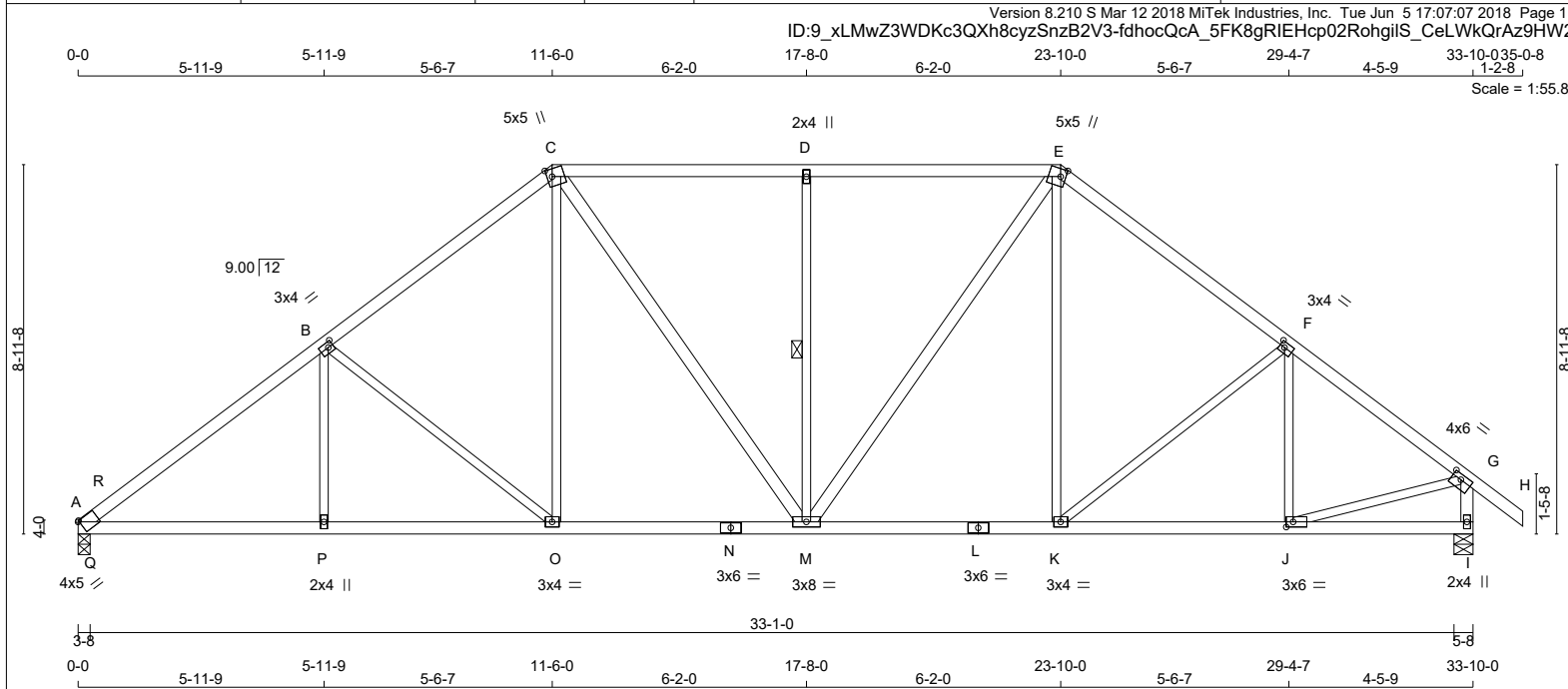
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.64 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 152 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - H	2x4	DRY	No.2 SPF
I - G	2x4	DRY	No.2 SPF
A - N	2x4	DRY	No.2 SPF
N - L	2x4	DRY	No.2 SPF
L - I	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
C - M	2x4	DRY	No.2 SPF
M - E	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	4.0	5.0	0.25	0.25
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TTWW+m	MT20	5.0	5.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	3.0	6.0	1.50	2.00
K	BMWW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	3.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	3.0	4.0		
P	BMW+w	MT20	2.0	4.0		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	UP	IN-SX
A 1617	0	0	3-8
I 1718	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT COMBINED	SNOW	PERM. LIVE	WIND
A 1136	788 / 0	0 / 0	0 / 0
I 1204	848 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M. DBS = 20-0-0. CBF = 73 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-R	-2245 / 0	-77.4	-77.4 0.10 (1)	P-B	0 / 139	0.04 (4)	
R-B	-2290 / 0	-77.4	-77.4 0.41 (1)	B-O	-551 / 0	0.55 (1)	
B-C	-1808 / 0	-77.4	-77.4 0.37 (1)	O-C	0 / 443	0.10 (1)	
C-D	-1623 / 0	-77.4	-77.4 0.42 (1)	C-M	0 / 341	0.05 (1)	
D-E	-1623 / 0	-77.4	-77.4 0.42 (1)	M-D	-583 / 0	0.30 (1)	
E-F	-1680 / 0	-77.4	-77.4 0.34 (1)	M-E	0 / 515	0.08 (1)	
F-G	-1792 / 0	-77.4	-77.4 0.30 (1)	K-E	0 / 220	0.05 (4)	
G-H	0 / 30	-77.4	-77.4 0.09 (1)	K-F	-176 / 0	0.18 (1)	
I-G	-1684 / 0	0.0	0.0 0.17 (1)	J-F	-274 / 0	0.09 (1)	
				J-G	0 / 1505	0.34 (1)	
				Q-R	-305 / 58	0.00 (1)	
A-Q	0 / 1852	-18.2	-18.2 0.44 (1)				
Q-P	0 / 1852	-18.2	-18.2 0.46 (1)				
P-O	0 / 1852	-18.2	-18.2 0.39 (1)				
O-N	0 / 1425	-18.2	-18.2 0.30 (1)				
N-M	0 / 1425	-18.2	-18.2 0.30 (1)				
M-L	0 / 1324	-18.2	-18.2 0.29 (1)				
L-K	0 / 1324	-18.2	-18.2 0.29 (1)				
K-J	0 / 1459	-18.2	-18.2 0.31 (1)				
J-I	0 / 0	-18.2	-18.2 0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	DL	PSF
	23.3	3.0	PSF
BOT CH.	LL	DL	PSF
	0.0	7.3	PSF
TOTAL LOAD	33.6		PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.42/1.00 (C-D:1), BC=0.46/1.00 (P-Q:1), WB=0.55/1.00 (B-O:1), SSI=0.26/1.00 (A-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

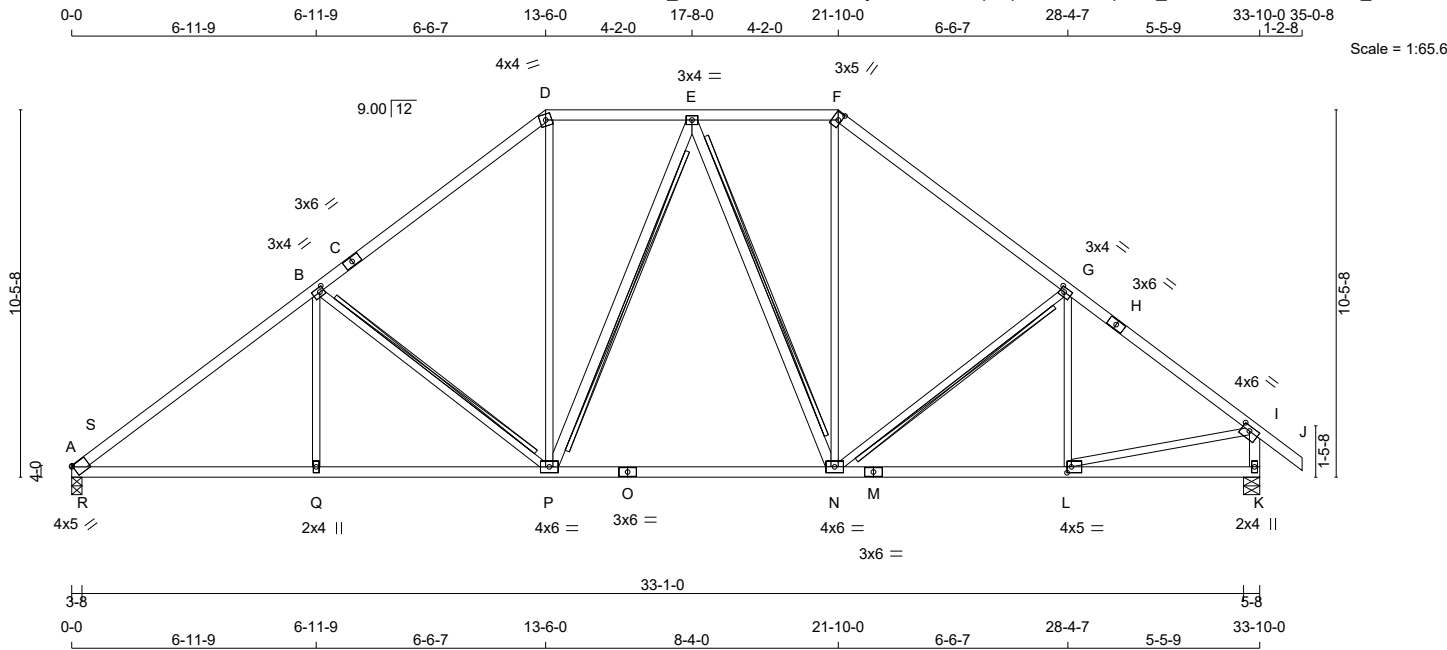
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.64 (A) (INPUT = 1.00)





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
A - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
P - E	2x4	DRY No.2	SPF
E - N	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	4.0	5.0	0.25	0.25
B TMWW-t	MT20	3.0	4.0	1.50	1.50
C TS-t	MT20	3.0	6.0		
D TTW-m	MT20	4.0	4.0		
E TMWW-t	MT20	3.0	4.0		
F TTW+h	MT20	3.0	5.0	2.50	1.00
G TMWW-t	MT20	3.0	4.0	1.50	1.50
H TS-t	MT20	3.0	6.0		
I TMVW-t	MT20	4.0	6.0	1.50	2.75
K BMV1+p	MT20	2.0	4.0		
L BMWW-t	MT20	4.0	5.0	2.00	1.50
M BS-t	MT20	3.0	6.0		
N BMWWW-t	MT20	4.0	6.0		
O BS-t	MT20	3.0	6.0		
P BMWWW-t	MT20	4.0	6.0		
Q BMW+w	MT20	2.0	4.0		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	1617	0	1617	0	3-8	3-8
K	1718	0	1718	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1136	788 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0
K	1204	848 / 0	0 / 0	0 / 0	0 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-P, E-P, E-N, G-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-S	-2144 / 0	-77.4 -77.4	0.12 (1)	4.56	Q-B	0 / 162	0.05 (4)
S-B	-2228 / 0	-77.4 -77.4	0.59 (1)	4.05	B-P	-649 / 0	0.36 (1)
B-C	-1658 / 0	-77.4 -77.4	0.52 (1)	4.60	P-D	0 / 652	0.15 (1)
C-D	-1658 / 0	-77.4 -77.4	0.52 (1)	4.60	P-E	-132 / 2	0.11 (1)
D-E	-1302 / 0	-77.4 -77.4	0.18 (1)	5.48	E-N	-282 / 0	0.25 (1)
E-F	-1247 / 0	-77.4 -77.4	0.18 (1)	5.57	N-F	0 / 603	0.14 (1)
F-G	-1584 / 0	-77.4 -77.4	0.48 (1)	4.74	N-G	-320 / 0	0.18 (1)
G-H	-1827 / 0	-77.4 -77.4	0.43 (1)	4.52	L-G	-197 / 37	0.09 (1)
H-I	-1827 / 0	-77.4 -77.4	0.43 (1)	4.52	L-I	0 / 1523	0.34 (1)
I-J	0 / 30	-77.4 -77.4	0.09 (1)	10.00	R-S	-406 / 80	0.00 (1)
K-I	-1676 / 0	0.0 0.0	0.17 (1)	6.41			
A-R	0 / 1806	-18.2 -18.2	0.48 (1)	10.00			
R-Q	0 / 1806	-18.2 -18.2	0.50 (1)	10.00			
Q-P	0 / 1806	-18.2 -18.2	0.43 (1)	10.00			
P-O	0 / 1351	-18.2 -18.2	0.34 (1)	10.00			
O-N	0 / 1351	-18.2 -18.2	0.34 (1)	10.00			
N-M	0 / 1492	-18.2 -18.2	0.37 (1)	10.00			
M-L	0 / 1492	-18.2 -18.2	0.37 (1)	10.00			
L-K	0 / 0	-18.2 -18.2	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	23.3	PSF
	3.0	PSF
BOT CH.	LL	0.0 PSF
	DL	7.3 PSF
TOTAL LOAD	=	33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.59/1.00 (B-S:1), BC=0.50/1.00 (Q-R:1), WB=0.36/1.00 (B-P:1), SSI=0.34/1.00 (A-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

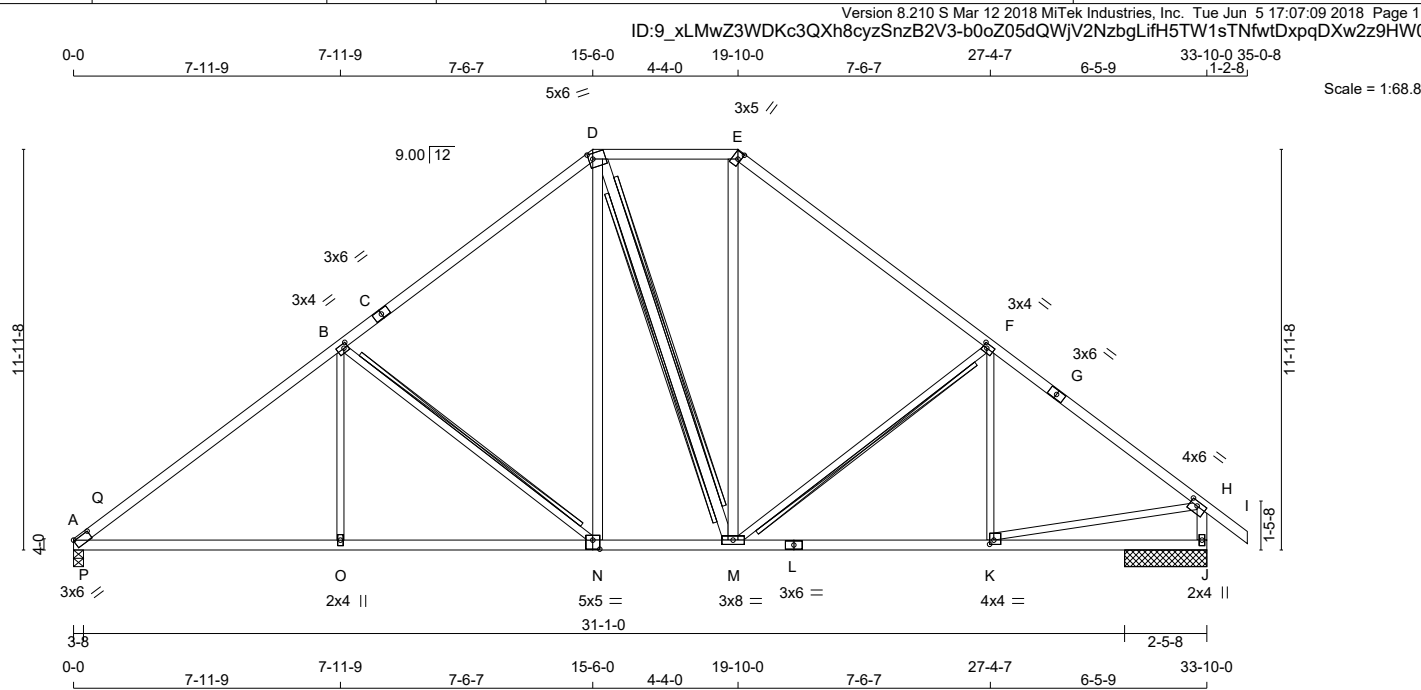
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.63 (A) (INPUT = 1.00)





TOTAL WEIGHT = 158 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
A - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	6.0	0.25	5.75
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TS-t	MT20	3.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	1.50
E	TTW+h	MT20	3.0	5.0	2.50	1.00
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	3.0	8.0		
N	BSWW-t	MT20	5.0	5.0	3.25	2.50
O	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	1617	0	1617	0	3-8	3-8
J	1718	0	1718	0	2-5-8	2-5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
A	1136	788 / 0	0 / 0	0 / 0	348 / 0	0 / 0			
J	1204	848 / 0	0 / 0	0 / 0	356 / 0	0 / 0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-N, F-M
2x4 DRY SPF No.2 I-BRACE AT D-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-Q	-2028 / 0	-77.4	-77.4	0.15 (1)	4.63	O-B	0 / 202	0.06 (4)	
Q-B	-2169 / 0	-77.4	-77.4	0.83 (1)	3.82	B-N	-760 / 0	0.58 (1)	
B-C	-1496 / 0	-77.4	-77.4	0.70 (1)	4.50	N-D	0 / 548	0.09 (1)	
C-D	-1496 / 0	-77.4	-77.4	0.70 (1)	4.50	D-M	-67 / 0	0.06 (1)	
D-E	-1147 / 0	-77.4	-77.4	0.20 (1)	5.72	M-E	0 / 462	0.07 (1)	
E-F	-1465 / 0	-77.4	-77.4	0.66 (1)	4.61	M-F	-465 / 0	0.36 (1)	
F-G	-1841 / 0	-77.4	-77.4	0.60 (1)	4.28	K-F	-119 / 87	0.08 (1)	
G-H	-1841 / 0	-77.4	-77.4	0.60 (1)	4.28	K-H	0 / 1531	0.34 (1)	
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	P-Q	-539 / 87	0.00 (1)	
J-H	-1672 / 0	0.0	0.0	0.17 (1)	6.42				
A-P	0 / 1762	-18.2	-18.2	0.53 (1)	10.00				
P-O	0 / 1762	-18.2	-18.2	0.56 (1)	10.00				
O-N	0 / 1762	-18.2	-18.2	0.50 (1)	10.00				
N-M	0 / 1169	-18.2	-18.2	0.28 (1)	10.00				
M-L	0 / 1508	-18.2	-18.2	0.36 (1)	10.00				
L-K	0 / 1508	-18.2	-18.2	0.36 (1)	10.00				
K-J	0 / 0	-18.2	-18.2	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.83/1.00 (B-Q:1), BC=0.56/1.00 (O-P:1), WB=0.58/1.00 (B-N:1), SSI=0.44/1.00 (A-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

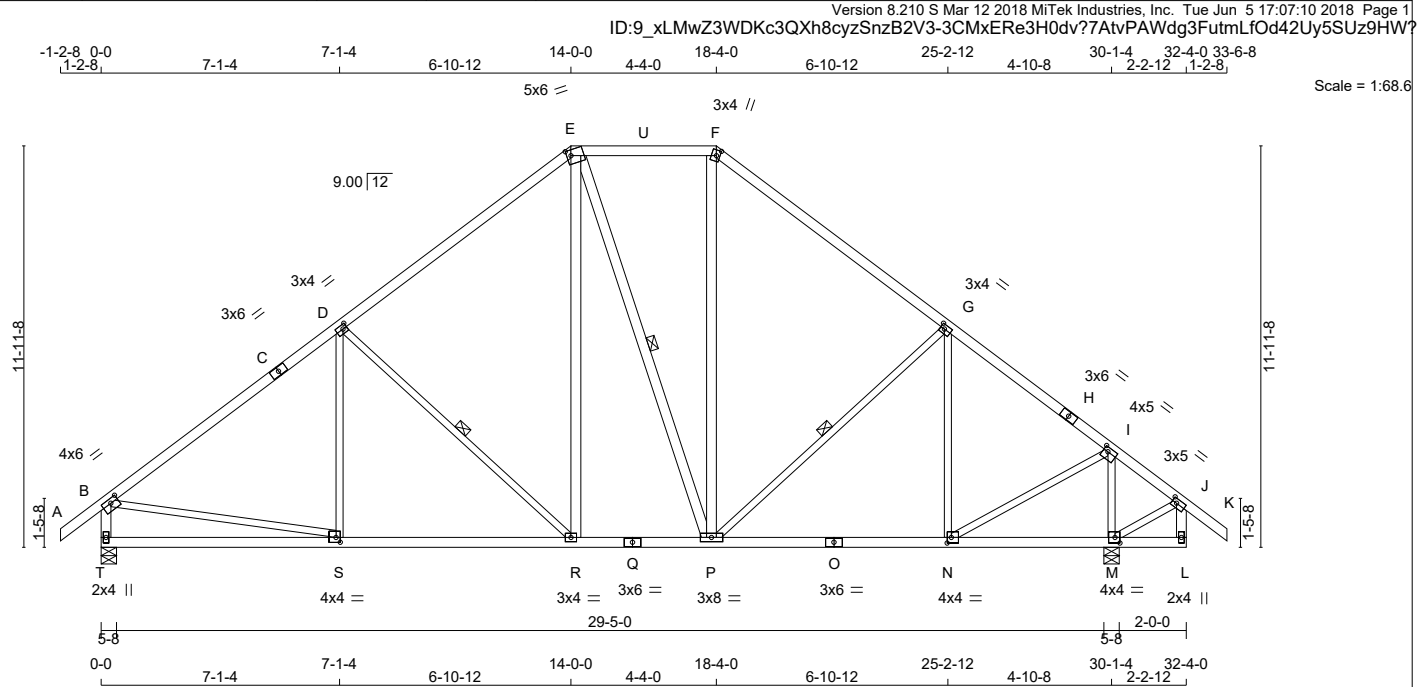
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.77 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 4 X 166 = 665 lbs [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW-m	MT20	5.0	6.0	2.00	1.50
F	TTWW-m	MT20	3.0	4.0	2.00	1.25
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	5.0	1.50	1.75
J	TMVW-t	MT20	3.0	5.0	1.50	1.75
L	BMV+p	MT20	2.0	4.0		
M	BMWW1-t	MT20	4.0	4.0	2.00	1.75
N	BMWW-t	MT20	4.0	4.0	2.00	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	8.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	3.0	4.0		
S	BMWW-t	MT20	4.0	4.0	1.75	1.50
T	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
T	1539	0	1539	0	5-8	5-8
M	1785	0	1785	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1081	753 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0
M	1254	874 / 0	0 / 0	0 / 0	0 / 0	380 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

FOR SECTION E-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-R, E-P, G-P. DBS = 20-0-0. CBF = 63 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	S-D	-57 / 105 0.04 (1)
B-C	-1580 / 0	-77.4	-77.4	0.62 (1)	4.54	D-R	-505 / 0 0.32 (1)
C-D	-1580 / 0	-77.4	-77.4	0.62 (1)	4.54	R-E	0 / 430 0.07 (1)
D-E	-1196 / 0	-77.4	-77.4	0.56 (1)	5.12	E-P	-126 / 0 0.11 (1)
E-U	-886 / 0	-84.9	-84.9	0.28 (1)	2.00	P-F	0 / 264 0.04 (1)
U-F	-886 / 0	-84.9	-84.9	0.28 (1)	2.00	P-G	-58 / 0 0.04 (1)
F-G	-1135 / 0	-77.4	-77.4	0.49 (1)	5.35	N-G	-495 / 0 0.38 (1)
G-H	-1122 / 0	-77.4	-77.4	0.38 (1)	5.51	N-I	0 / 1213 0.27 (1)
H-I	-1122 / 0	-77.4	-77.4	0.38 (1)	5.51	M-I	-1638 / 0 0.30 (1)
I-J	0 / 175	-77.4	-77.4	0.11 (1)	10.00	B-S	0 / 1312 0.30 (1)
J-K	0 / 30	-77.4	-77.4	0.09 (1)	10.00	M-J	-141 / 0 0.02 (1)
T-B	-1487 / 0	0.0	0.0	0.15 (1)	6.72		
L-J	0 / 11	0.0	0.0	0.00 (4)	10.00		
T-S	0 / 0	-18.2	-18.2	0.23 (4)	10.00		
S-R	0 / 1295	-18.2	-18.2	0.33 (4)	10.00		
R-Q	0 / 929	-18.2	-18.2	0.23 (1)	10.00		
Q-P	0 / 929	-18.2	-18.2	0.23 (1)	10.00		
P-O	0 / 925	-18.2	-18.2	0.23 (1)	10.00		
O-N	0 / 925	-18.2	-18.2	0.23 (1)	10.00		
N-M	-126 / 0	-18.2	-18.2	0.12 (4)	6.25		
M-L	0 / 0	-18.2	-18.2	0.06 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
LL	3.0	PSF
BOT CH.	LL	0.0 PSF
DL	7.3	PSF
TOTAL LOAD	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.62/1.00 (B-D:1), BC=0.33/1.00 (R-S:4), WB=0.38/1.00 (G-N:1), SSI=0.21/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



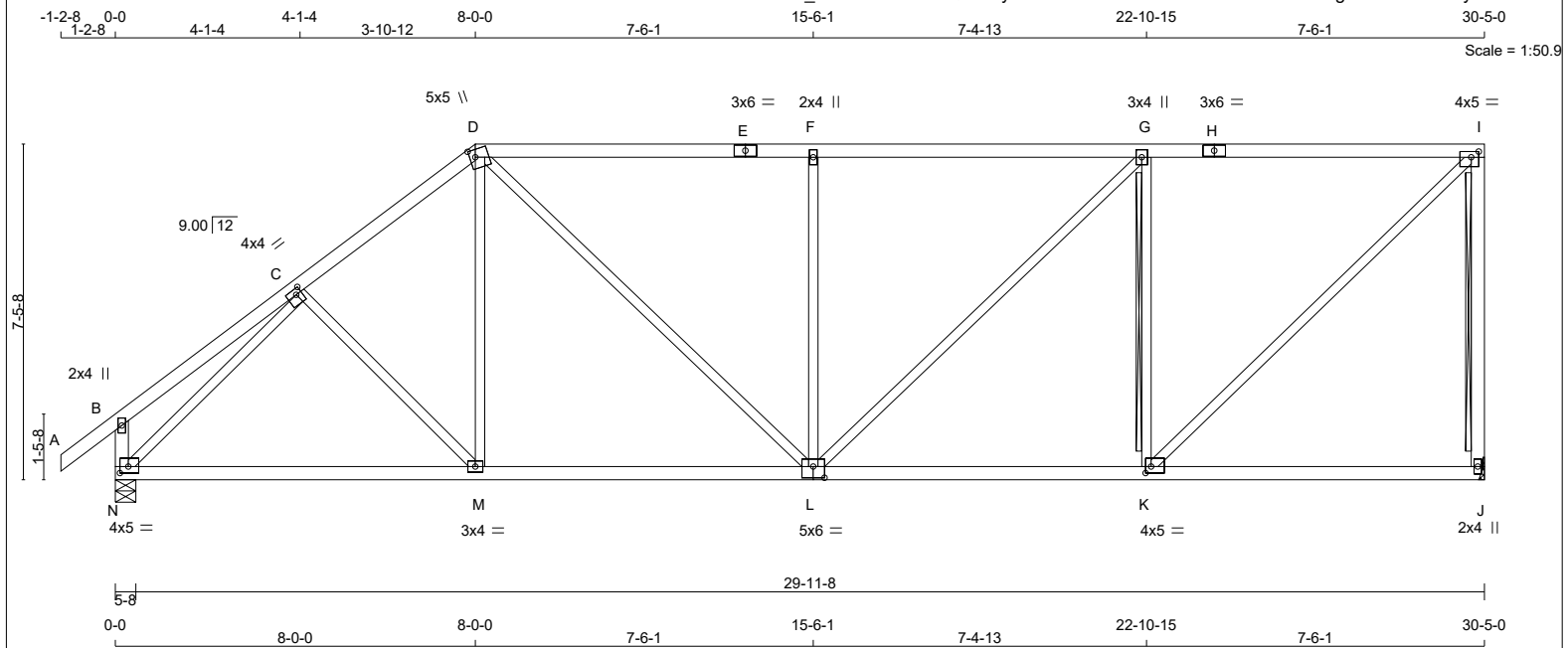
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	DRWG NO.
NE0618-035	T05	4	1	TRUSS DESC.		PAGE 34 OF 47

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ID:9_xLMwZ3WDKc3QXh8cyzSnzB2V3-3CMxERe3H0dv?7AtvPAWdg3FutmLfOd42Uy5SUz9HW?

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.49 (S) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 133 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMWW-t	MT20	4.0	4.0	1.50 1.50
D	TTWW+m	MT20	5.0	5.0	2.00 1.50
E	TS-t	MT20	3.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TMWW+t	MT20	3.0	4.0	
H	TS-t	MT20	3.0	6.0	
I	TMVW-t	MT20	4.0	5.0	1.50 2.00
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	4.0	5.0	1.75 1.50
L	BSWWW-t	MT20	5.0	6.0	3.00 3.00
M	BMWW-t	MT20	3.0	4.0	
N	BMVW1-t	MT20	4.0	5.0	1.75 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1454	0	1454	0	0
N	1554	0	1554	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1022	708 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
N	1090	769 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-J, G-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	C-M	-26 / 46	0.02 (4)
B-C	0 / 20	-77.4 -77.4	0.19 (1)	10.00	M-D	0 / 182	0.05 (4)
C-D	-1541 / 0	-77.4 -77.4	0.23 (1)	5.07	D-L	0 / 541	0.12 (1)
D-E	-1609 / 0	-77.4 -77.4	0.76 (1)	4.26	L-F	-628 / 0	0.63 (1)
E-F	-1609 / 0	-77.4 -77.4	0.76 (1)	4.26	L-G	0 / 540	0.12 (1)
F-G	-1609 / 0	-77.4 -77.4	0.76 (1)	4.24	K-G	-1010 / 0	0.39 (1)
G-H	-1221 / 0	-77.4 -77.4	0.70 (1)	4.79	K-I	0 / 1688	0.38 (1)
H-I	-1221 / 0	-77.4 -77.4	0.70 (1)	4.79	N-C	-1773 / 0	0.96 (1)
J-I	-1399 / 0	0.0	0.0	4.43 (1)			
N-B	-220 / 0	0.0	0.0	0.02 (1)			
N-M	0 / 1233	-18.2	-18.2	0.37 (4)	10.00		
M-L	0 / 1217	-18.2	-18.2	0.38 (4)	10.00		
L-K	0 / 1221	-18.2	-18.2	0.34 (4)	10.00		
K-J	0 / 0	-18.2	-18.2	0.25 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.76/1.00 (F-G:1), BC=0.38/1.00 (L-M:4), WB=0.96/1.00 (C-N:1), SSI=0.27/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.65 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



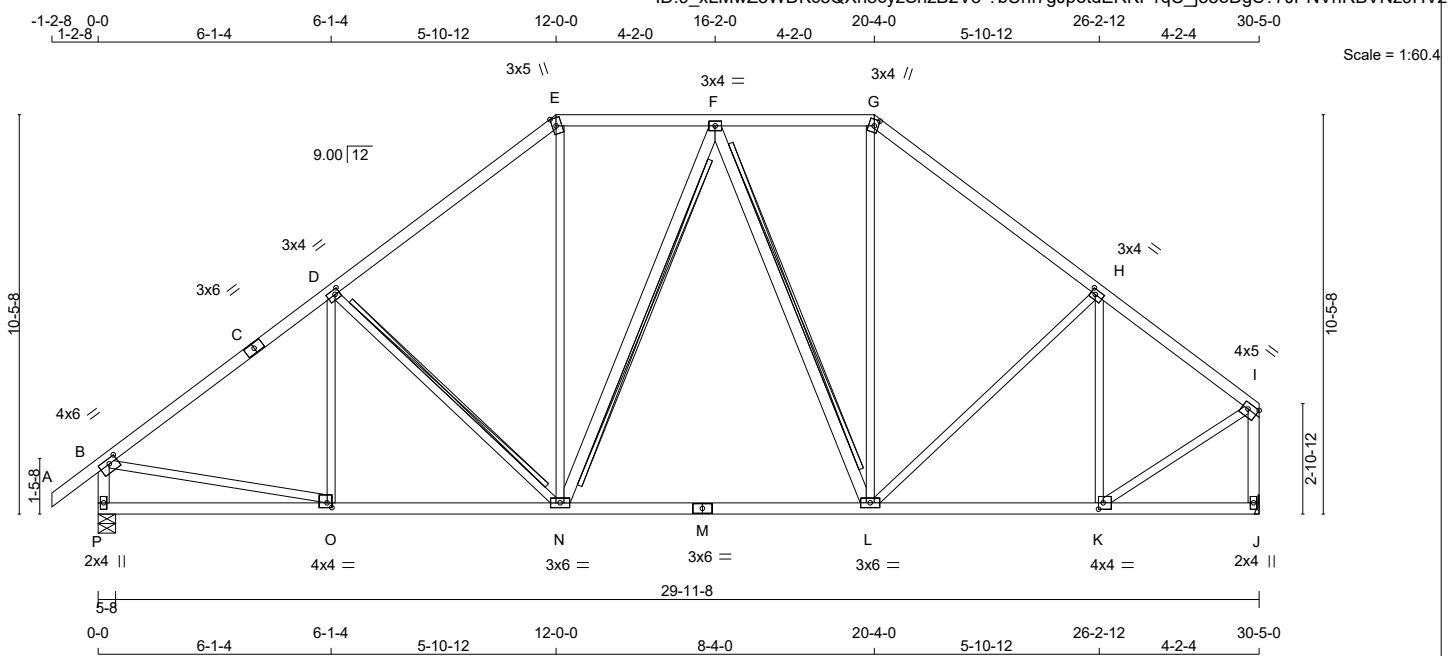
GREEN YORK HOMES - GRANELLI
HOME CORP - INDIGO 1 ELEV 2

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TOTAL WEIGHT = 152 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
F - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+m	MT20	3.0	5.0	Edge	1.25
F	TMWW-t	MT20	3.0	4.0		
G	TTW+m	MT20	3.0	4.0	2.00	1.25
H	TMWW-t	MT20	3.0	4.0	1.50	1.50
I	TMVW-t	MT20	4.0	5.0	1.75	Edge
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMWW-t	MT20	3.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
P	1554	0	1554	0	5-8	5-8
J	1454	0	1454	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1090	769 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
J	1022	708 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-N, F-N, F-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	O-D	-129 / 58 0.07 (1)
B-C	-1605 / 0	-77.4	-77.4	0.43 (1)	4.77	D-N	-366 / 0 0.19 (1)
C-D	-1605 / 0	-77.4	-77.4	0.43 (1)	4.77	N-E	0 / 488 0.11 (1)
D-E	-1344 / 0	-77.4	-77.4	0.40 (1)	5.14	N-F	-111 / 8 0.10 (1)
E-F	-1051 / 0	-77.4	-77.4	0.18 (1)	5.94	F-L	-307 / 0 0.27 (1)
F-G	-978 / 0	-77.4	-77.4	0.17 (1)	6.10	L-G	0 / 421 0.09 (1)
G-H	-1245 / 0	-77.4	-77.4	0.35 (1)	5.36	L-H	0 / 16 0.01 (4)
H-I	-1176 / 0	-77.4	-77.4	0.29 (1)	5.55	K-H	-504 / 0 0.29 (1)
P-B	-1507 / 0	0.0	0.0	0.16 (1)	6.68	B-O	0 / 1333 0.30 (1)
J-I	-1423 / 0	0.0	0.0	0.22 (1)	6.84	K-I	0 / 1138 0.26 (1)
P-O	0 / 0	-18.2	-18.2	0.14 (4)	10.00		
O-N	0 / 1311	-18.2	-18.2	0.32 (1)	10.00		
N-M	0 / 1092	-18.2	-18.2	0.29 (1)	10.00		
M-L	0 / 1092	-18.2	-18.2	0.29 (1)	10.00		
L-K	0 / 971	-18.2	-18.2	0.28 (4)	10.00		
K-J	0 / 0	-18.2	-18.2	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL	=	23.3 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.3 PSF
TOTAL LOAD			=	33.6 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.43/1.00 (B-D:1), BC=0.32/1.00 (N-O:1), WB=0.30/1.00 (B-O:1), SSI=0.18/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

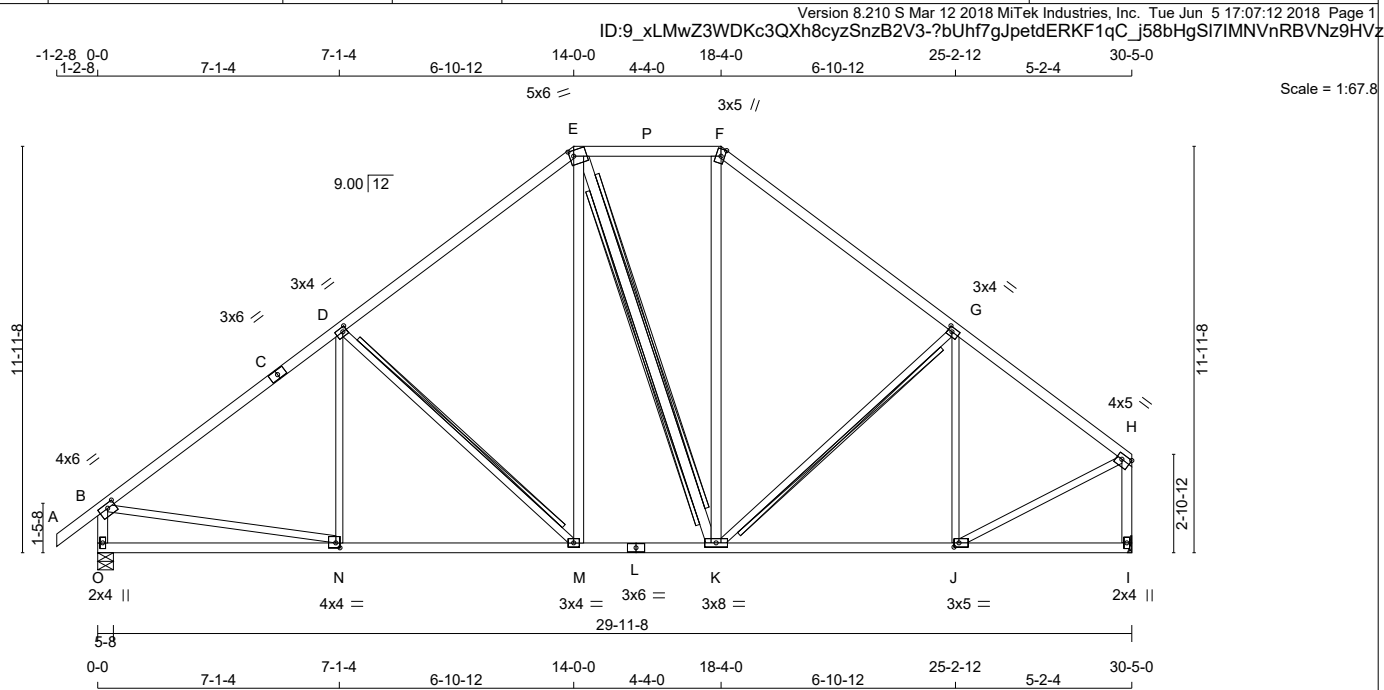
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.49 (O) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 158 = 315 lb

LUMBER				
N L G A. RULES				
CHORDS	SIZE	LUMBER	DESCR	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TS-t	MT20	3.0	6.0		
D	TMVWV-t	MT20	3.0	4.0	1.50	1.50
E	TTWV-m	MT20	5.0	6.0	2.00	1.50
F	TTW+m	MT20	3.0	5.0	Edge	1.25
G	TMVWV-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	5.0	1.75	Edge
I	BMV1+p	MT20	2.0	4.0		
J	BMWV-t	MT20	3.0	5.0	1.50	1.75
K	BMVWV-t	MT20	3.0	8.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWV-t	MT20	3.0	4.0		
N	BMWV-t	MT20	4.0	4.0	1.75	1.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1570	0	1570	0	0	5-8	5-8
I	1471	0	1471	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1102	769 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0
I	1035	708 / 0	0 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

FOR SECTION E-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-M, G-K
2x4 DRY SPF No.2 I-BRACE AT E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-D	-62 / 104	0.05 (1)
B-C	-1622 / 0	-77.4	-77.4	0.62 (1)	4.49	D-M	-500 / 0	0.37 (1)
C-D	-1622 / 0	-77.4	-77.4	0.62 (1)	4.49	M-E	0 / 428	0.07 (1)
D-E	-1242 / 0	-77.4	-77.4	0.56 (1)	5.04	E-K	-94 / 0	0.09 (1)
E-P	-934 / 0	-84.9	-84.9	0.28 (1)	2.00	K-F	0 / 302	0.05 (1)
P-F	-934 / 0	-84.9	-84.9	0.28 (1)	2.00	K-G	-163 / 0	0.12 (1)
F-G	-1196 / 0	-77.4	-77.4	0.50 (1)	5.24	J-G	-394 / 0	0.30 (1)
G-H	-1270 / 0	-77.4	-77.4	0.42 (1)	5.21	B-N	0 / 1346	0.30 (1)
O-B	-1518 / 0	0.0	0.0	0.16 (1)	6.67	J-H	0 / 1171	0.26 (1)
I-H	-1437 / 0	0.0	0.0	0.22 (1)	6.81			
Q-N	0 / 0	-18.2	-18.2	0.23 (4)	10.00			
N-M	0 / 1329	-18.2	-18.2	0.34 (1)	10.00			
M-L	0 / 965	-18.2	-18.2	0.23 (1)	10.00			
L-K	0 / 965	-18.2	-18.2	0.23 (1)	10.00			
K-J	0 / 1050	-18.2	-18.2	0.26 (1)	10.00			
J-I	0 / 0	-18.2	-18.2	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.62/1.00 (B-D:1) , BC=0.34/1.00 (M-N:1) ,
WB=0.37/1.00 (D-M:1) , SSI=0.21/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

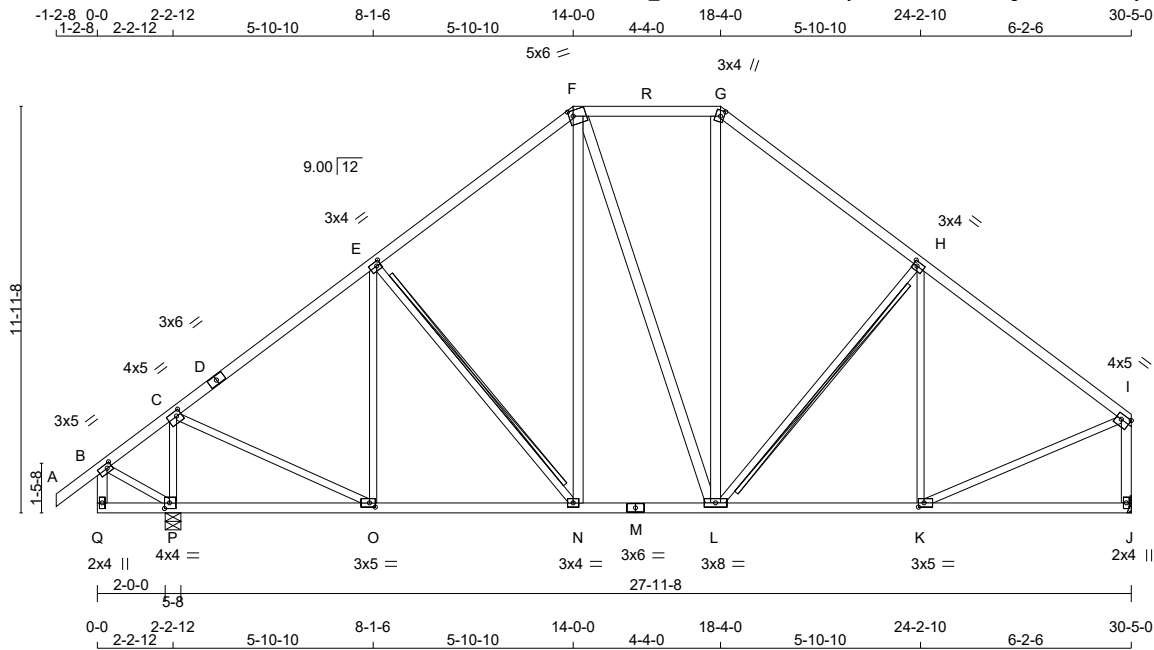
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.50 (N) (INPUT = 1.00)





Scale = 1:67.8

TOTAL WEIGHT = 163 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
F - L	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	3.0	5.0	1.50 1.75
C	TMWW-t	MT20	4.0	5.0	1.75 1.75
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	3.0	4.0	1.50 1.50
F	TTWW-m	MT20	5.0	6.0	2.00 1.50
G	TTW+m	MT20	3.0	4.0	2.00 1.25
H	TMWW-t	MT20	3.0	4.0	1.50 1.50
I	TMVW-t	MT20	4.0	5.0	1.75 Edge
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	3.0	5.0	1.50 2.00
L	BMWW-t	MT20	3.0	8.0	
M	BS-t	MT20	3.0	6.0	
N	BMWW-t	MT20	3.0	4.0	
O	BMWW-t	MT20	3.0	5.0	1.50 2.00
P	BMWW1-t	MT20	4.0	4.0	2.00 1.75
Q	BMV+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1694	0	1694	0	0	5-8	5-8
J	1347	0	1347	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1189	829 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
J	948	648 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

FOR SECTION F-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, H-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00	P-C	-1546 / 0	0.28 (1)	
B-C	0 / 151	-77.4 -77.4 0.24 (1)	10.00	C-O	0 / 1070	0.24 (1)	
C-D	-1086 / 0	-77.4 -77.4 0.33 (1)	5.67	O-E	-351 / 0	0.36 (1)	
D-E	-1086 / 0	-77.4 -77.4 0.33 (1)	5.67	E-N	-142 / 0	0.10 (1)	
E-F	-1016 / 0	-77.4 -77.4 0.35 (1)	5.79	N-F	0 / 201	0.03 (1)	
F-R	-801 / 0	-84.9 -84.9 0.28 (1)	2.00	F-L	0 / 32	0.01 (1)	
R-G	-801 / 0	-84.9 -84.9 0.28 (1)	2.00	L-G	0 / 249	0.04 (1)	
G-H	-1035 / 0	-77.4 -77.4 0.39 (1)	5.68	H-H	-253 / 0	0.18 (1)	
H-I	-1167 / 0	-77.4 -77.4 0.41 (1)	5.40	K-H	-269 / 21	0.28 (1)	
Q-B	-7 / 0	0.0 0.0 0.00 (1)	7.81	B-P	-97 / 0	0.02 (1)	
J-I	-1302 / 0	0.0 0.0 0.20 (1)	7.08	K-I	0 / 1039	0.23 (1)	
Q-P	0 / 0	-18.2 -18.2 0.10 (4)	10.00				
P-O	-87 / 0	-18.2 -18.2 0.10 (4)	6.25				
O-N	0 / 879	-18.2 -18.2 0.21 (1)	10.00				
N-M	0 / 790	-18.2 -18.2 0.18 (1)	10.00				
M-L	0 / 790	-18.2 -18.2 0.18 (1)	10.00				
L-K	0 / 960	-18.2 -18.2 0.25 (4)	10.00				
K-J	0 / 0	-18.2 -18.2 0.17 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.41/1.00 (H-I:1) , BC=0.25/1.00 (K-L:4) , WB=0.36/1.00 (E-O:1) , SSI=0.18/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	T10	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 40 OF 47

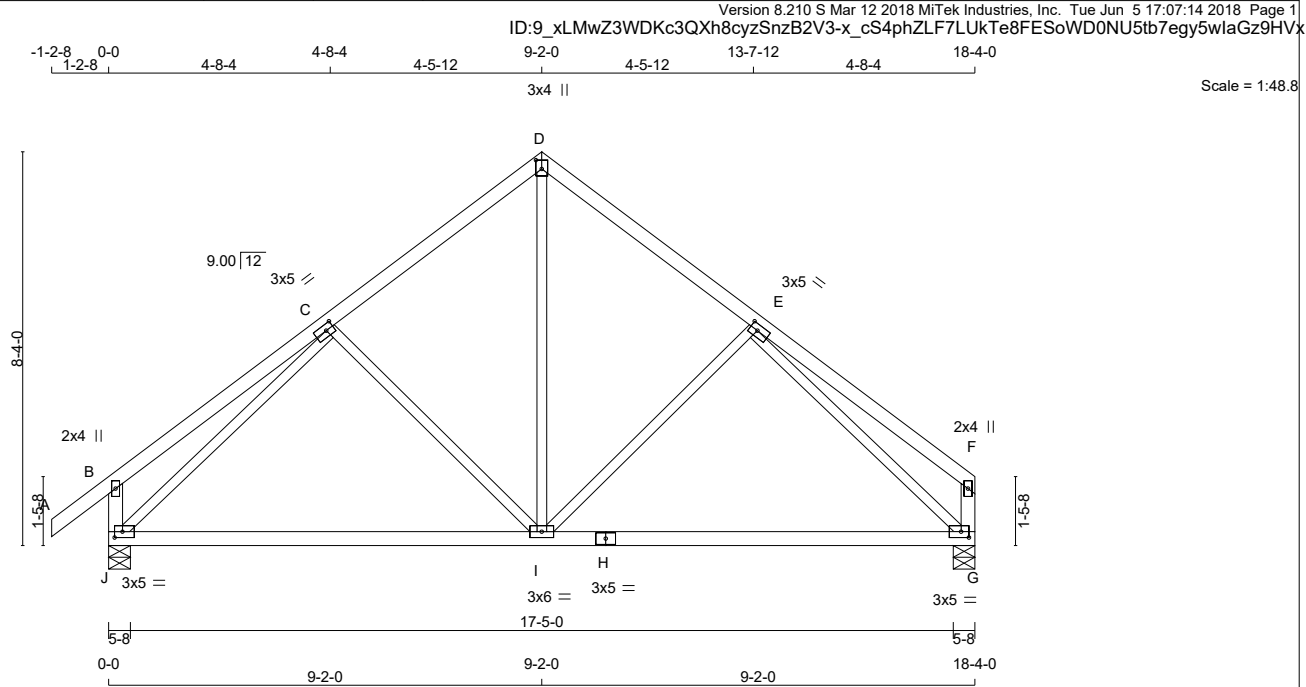
Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 17:07:14 2018 Page 2
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PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-035	T11	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - INDIGO 1 ELEV 2	PAGE 41 OF 47



TOTAL WEIGHT = 78 lb [M][F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	5.0	1.50	2.00
H	BS-t	MT20	3.0	5.0		
I	BMWWW-t	MT20	3.0	6.0		
J	BMVW1-t	MT20	3.0	5.0	1.50	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	DOWN	UP
J		977	0	977	0
G		876	0	876	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
J	COMBINED	SNOW	LIVE	PERM.LIVE
J	684	487 / 0	0 / 0	0 / 0
G	616	427 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 30	-77.4	-77.4 0.09 (1)	I-D	0 / 514	0.12 (1)	
B-C	0 / 25	-77.4	-77.4 0.26 (1)	I-E	-218 / 0	0.17 (1)	
C-D	-681 / 0	-77.4	-77.4 0.21 (1)	C-I	-218 / 0	0.17 (1)	
D-E	-681 / 0	-77.4	-77.4 0.21 (1)	J-C	-960 / 0	0.70 (1)	
E-F	0 / 25	-77.4	-77.4 0.26 (1)	E-G	-960 / 0	0.70 (1)	
J-B	-235 / 0	0.0	0.0 0.02 (1)				
G-F	-135 / 0	0.0	0.0 0.01 (1)				
J-I	0 / 680	-18.2	-18.2 0.49 (4)				
I-H	0 / 680	-18.2	-18.2 0.49 (4)				
H-G	0 / 680	-18.2	-18.2 0.49 (4)				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.26/1.00 (B-C:1), BC=0.49/1.00 (I-J:4), WB=0.70/1.00 (C-J:1), SSI=0.14/1.00 (G-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

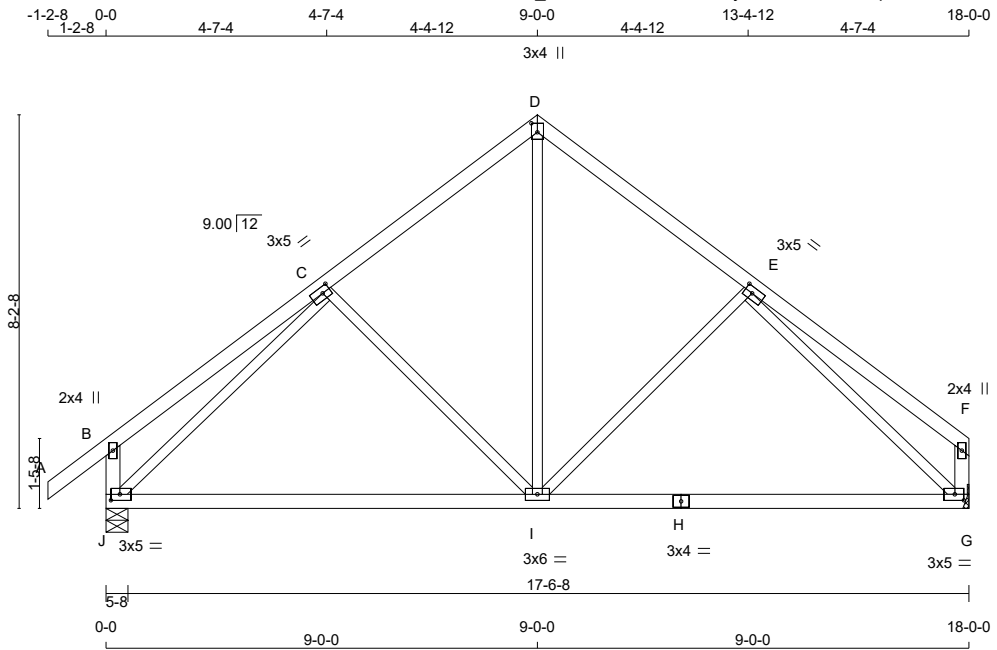
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)





Scale: 1/4"=1'

TOTAL WEIGHT = 2 X 76 = 153 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	5.0	1.50	2.25
H	BS-t	MT20	3.0	4.0		
I	BMVWW-t	MT20	3.0	6.0		
J	BMVW1-t	MT20	3.0	5.0	1.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	961	0	961	0	5-8	5-8
G	861	0	861	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	673	480 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0
G	605	419 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED		MAX CSI (LC)
		VERT. LOAD (PLF)	LC1	MAX CSI (LC)			FORCE (LBS)		
FR-TO		FROM	TO			FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	I-D	0 / 503		0.11 (1)
B-C	0 / 25	-77.4	-77.4	0.25 (1)	10.00	I-E	-212 / 0		0.15 (1)
C-D	-667 / 0	-77.4	-77.4	0.20 (1)	6.25	C-I	-212 / 0		0.15 (1)
D-E	-667 / 0	-77.4	-77.4	0.20 (1)	6.25	J-C	-941 / 0		0.66 (1)
E-F	0 / 25	-77.4	-77.4	0.25 (1)	10.00	E-G	-941 / 0		0.66 (1)
J-B	-233 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	-132 / 0	0.0	0.0	0.01 (1)	7.81				
J-I	0 / 664	-18.2	-18.2	0.47 (4)	10.00				
I-H	0 / 664	-18.2	-18.2	0.47 (4)	10.00				
H-G	0 / 664	-18.2	-18.2	0.47 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.25/1.00 (B-C:1), BC=0.47/1.00 (I-J:4), WB=0.66/1.00 (C-J:1), SSI=0.14/1.00 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

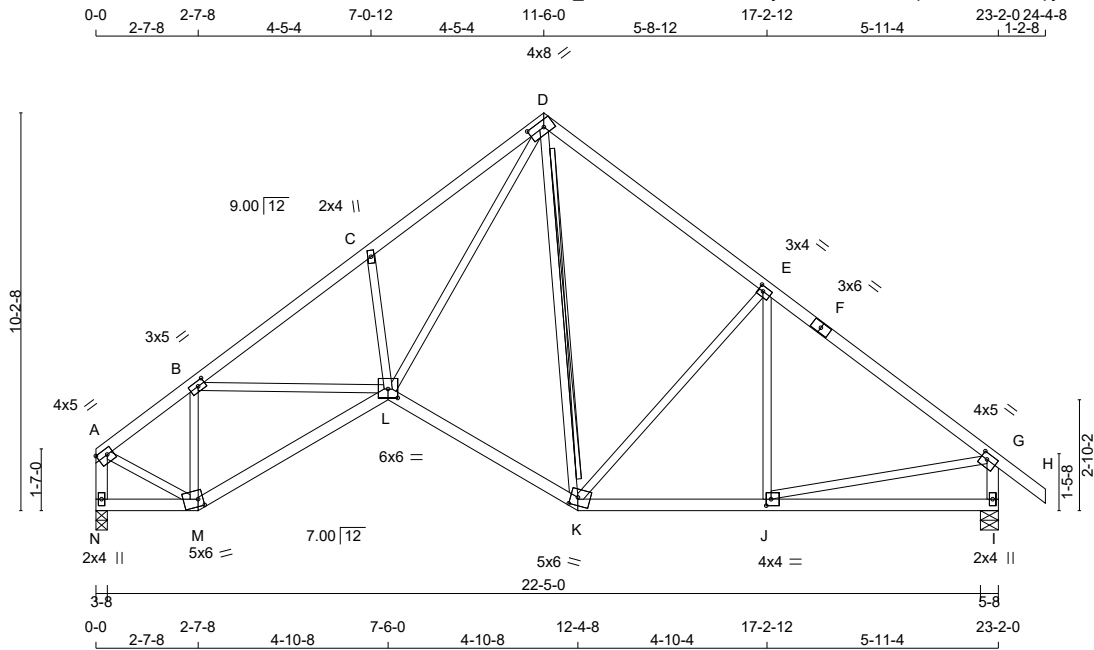
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.27 (H) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:59.1

TOTAL WEIGHT = 106 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.75	Edge
B	TMWW-t	MT20	3.0	5.0	1.50	2.25
C	TMW-w	MT20	2.0	4.0		
D	TTWW-h	MT20	4.0	8.0	2.00	5.00
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	5.0	1.75	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BBWW-m	MT20	5.0	6.0	2.50	2.25
L	BBWWW-p	MT20	6.0	6.0	2.75	3.00
M	BBWW-m	MT20	5.0	6.0	2.25	1.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	1107	0	1107	0	3-8	3-8
I	1208	0	1208	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	778	539 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
I	846	600 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	-1022 / 0	-77.4 -77.4	0.10 (1)	6.09	M-B	-801 / 0	0.16 (1)		
B-C	-1820 / 0	-77.4 -77.4	0.20 (1)	4.80	B-L	0 / 655	0.15 (1)		
C-D	-1783 / 0	-77.4 -77.4	0.23 (1)	4.81	C-L	-356 / 0	0.09 (1)		
D-E	-884 / 0	-77.4 -77.4	0.36 (1)	6.07	L-D	0 / 1471	0.33 (1)		
E-F	-1147 / 0	-77.4 -77.4	0.38 (1)	5.49	D-K	-18 / 53	0.02 (4)		
F-G	-1147 / 0	-77.4 -77.4	0.38 (1)	5.49	K-E	-401 / 0	0.44 (1)		
G-H	0 / 30	-77.4 -77.4	0.09 (1)	10.00	J-E	-62 / 83	0.03 (1)		
N-A	-1084 / 0	0.0 0.0	0.11 (1)	7.58	A-M	0 / 924	0.21 (1)		
I-G	-1163 / 0	0.0 0.0	0.12 (1)	7.39	J-G	0 / 961	0.22 (1)		
N-M	0 / 0	-18.2 -18.2	0.04 (4)	10.00					
M-L	0 / 936	-18.2 -18.2	0.22 (1)	10.00					
L-K	0 / 790	-18.2 -18.2	0.20 (1)	10.00					
K-J	0 / 944	-18.3 -18.3	0.23 (1)	10.00					
J-I	0 / 0	-18.2 -18.2	0.15 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	PSF
	DL	=	3.0
	LL	=	0.0
	DL	=	7.3
	LL	=	33.6

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.38/1.00 (E-G:1), BC=0.23/1.00 (J-K:1), WB=0.44/1.00 (E-K:1), SSI=0.18/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

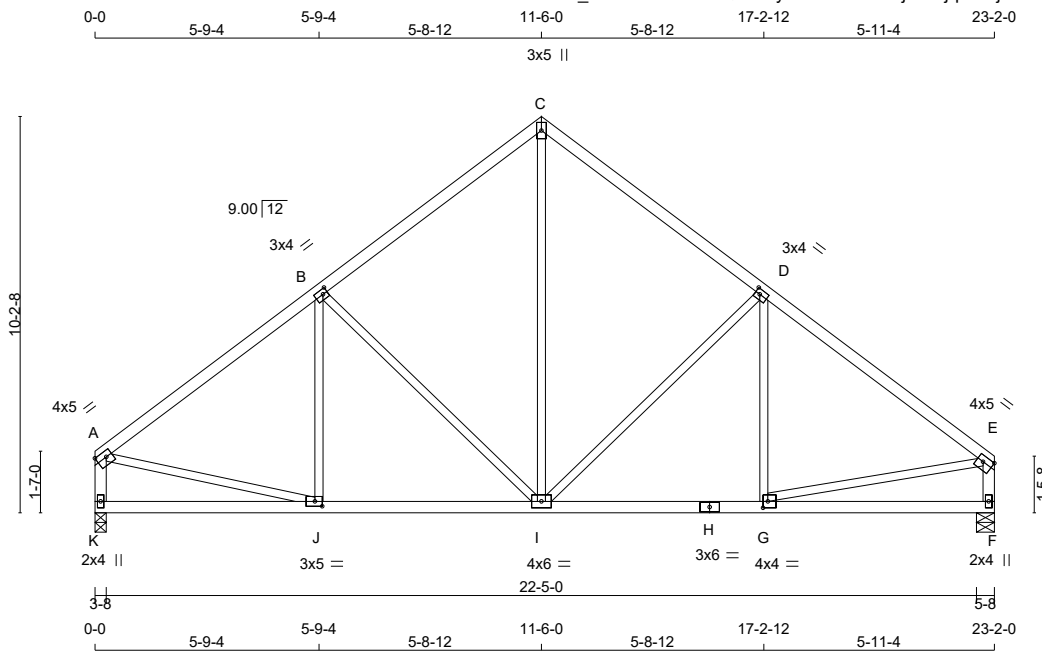
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.40 (G) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 100 = 301 lb [M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.75	Edge
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TMVW-t	MT20	4.0	5.0	1.75	Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMWW-t	MT20	4.0	4.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	3.0	5.0	1.50	2.25
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	UPLIFT	IN-SX
K		1107	0	0	3-8
F		1107	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	778	539 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0
F	778	539 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1124 / 0	-77.4 -77.4	0.36 (1)	5.55	J-B	-85 / 73	0.05 (1)
B-C	-849 / 0	-77.4 -77.4	0.34 (1)	6.18	B-I	-379 / 0	0.51 (1)
C-D	-850 / 0	-77.4 -77.4	0.35 (1)	6.16	I-C	0 / 647	0.15 (1)
D-E	-1147 / 0	-77.4 -77.4	0.38 (1)	5.49	I-D	-405 / 0	0.54 (1)
K-A	-1064 / 0	0.0 0.0	0.11 (1)	7.64	G-D	-61 / 82	0.03 (1)
F-E	-1063 / 0	0.0 0.0	0.11 (1)	7.64	A-J	0 / 946	0.21 (1)
					G-E	0 / 960	0.22 (1)
K-J	0 / 0	-18.2 -18.2	0.14 (4)	10.00			
J-I	0 / 925	-18.2 -18.2	0.22 (1)	10.00			
I-H	0 / 943	-18.2 -18.2	0.23 (1)	10.00			
H-G	0 / 943	-18.2 -18.2	0.23 (1)	10.00			
G-F	0 / 0	-18.2 -18.2	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.23/1.00 (G-I:1) , WB=0.54/1.00 (D-I:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

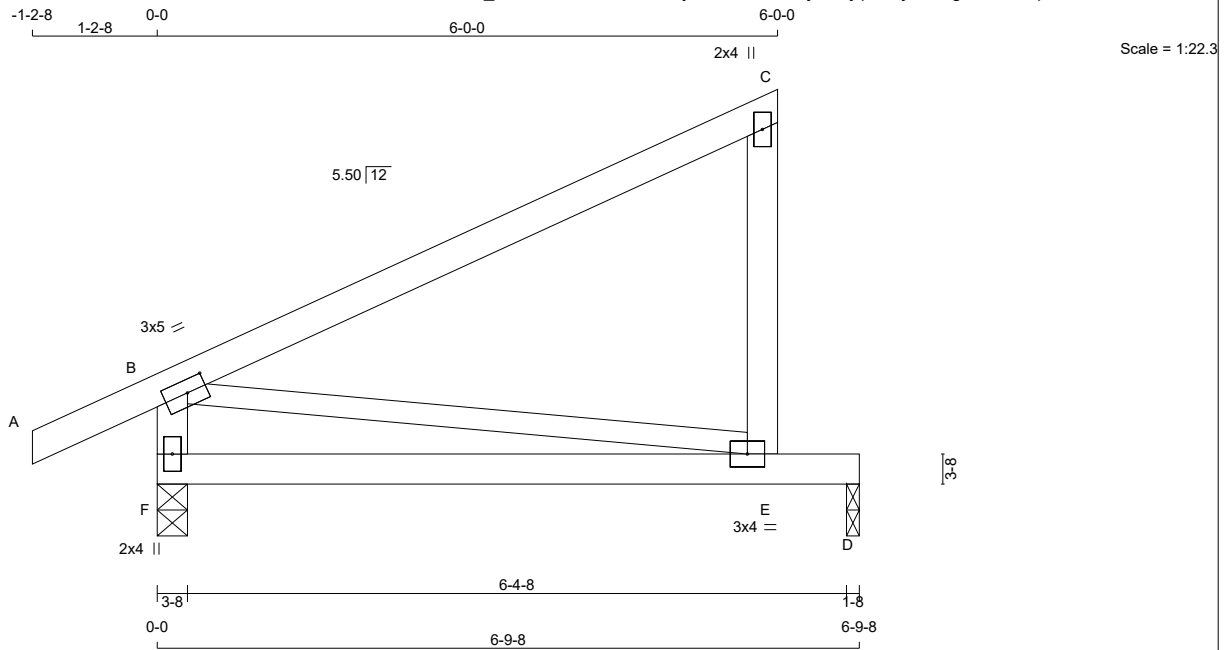
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 5 X 26 = 129 lb [M][F]

LUMBER					DESCR.
CHORDS	SIZE	DRY	LUMBER	No.2	
A - C	2x4	DRY	No.2	No.2	SPF
E - C	2x4	DRY	No.2	No.2	SPF
F - B	2x4	DRY	No.2	No.2	SPF
F - D	2x4	DRY	No.2	No.2	SPF
ALL WEBS	2x3	DRY	No.2	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	3.0	5.0	1.50 2.25
C	TMV+p	MT20	2.0	4.0	
E	BMVW-t	MT20	3.0	4.0	
F	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
F	419	0	419	0	0	3-8	3-8	3-8	3-8
D	267	0	267	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	292	215 / 0	0 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	189	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 20	-77.4	-77.4 0.09 (1)	10.00	B-E	0 / 0	0.00 (1)
B-C	0 / 0	-77.4	-77.4 0.47 (1)	10.00			
E-C	-232 / 0	0.0	0.0 0.05 (1)	7.81			
F-B	-330 / 0	0.0	0.0 0.03 (1)	7.81			
F-E	0 / 0	-18.2	-18.2 0.30 (1)	10.00			
E-D	0 / 0	-18.2	-18.2 0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/401 (0.20")

CSI: TC=0.47/1.00 (B-C:1), BC=0.30/1.00 (E-F:1), WB=0.00/1.00 (B-E:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



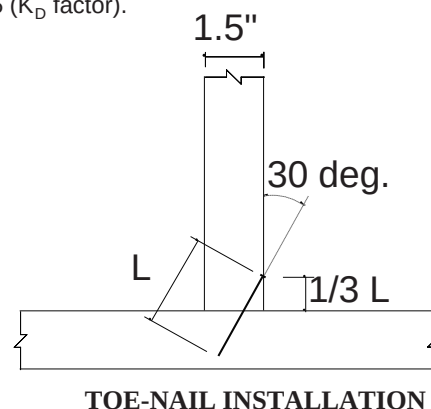
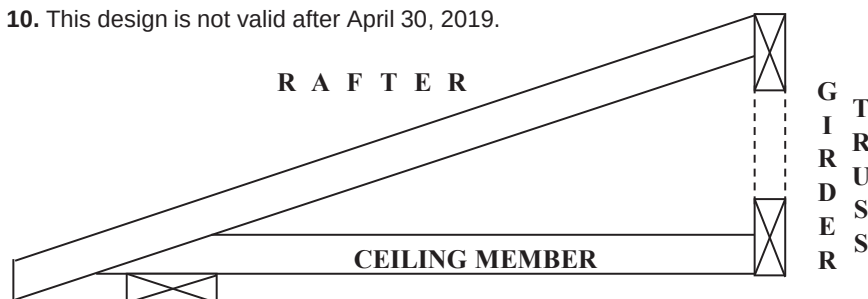
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



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Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

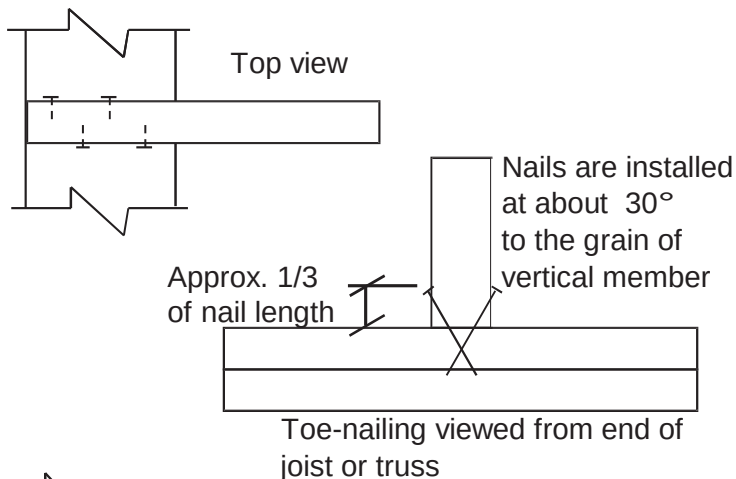
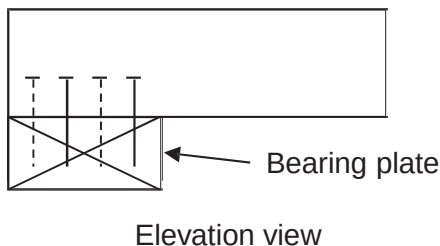
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

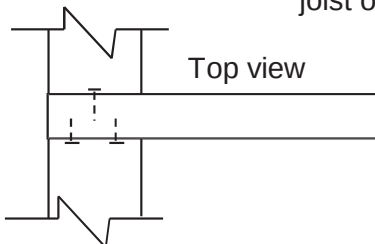
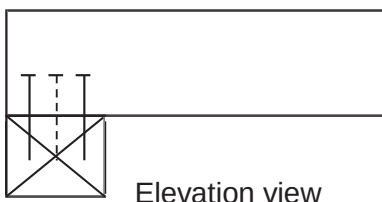
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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