

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-008 GREEN YORK HOMES-BELLE 1 EL 2

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

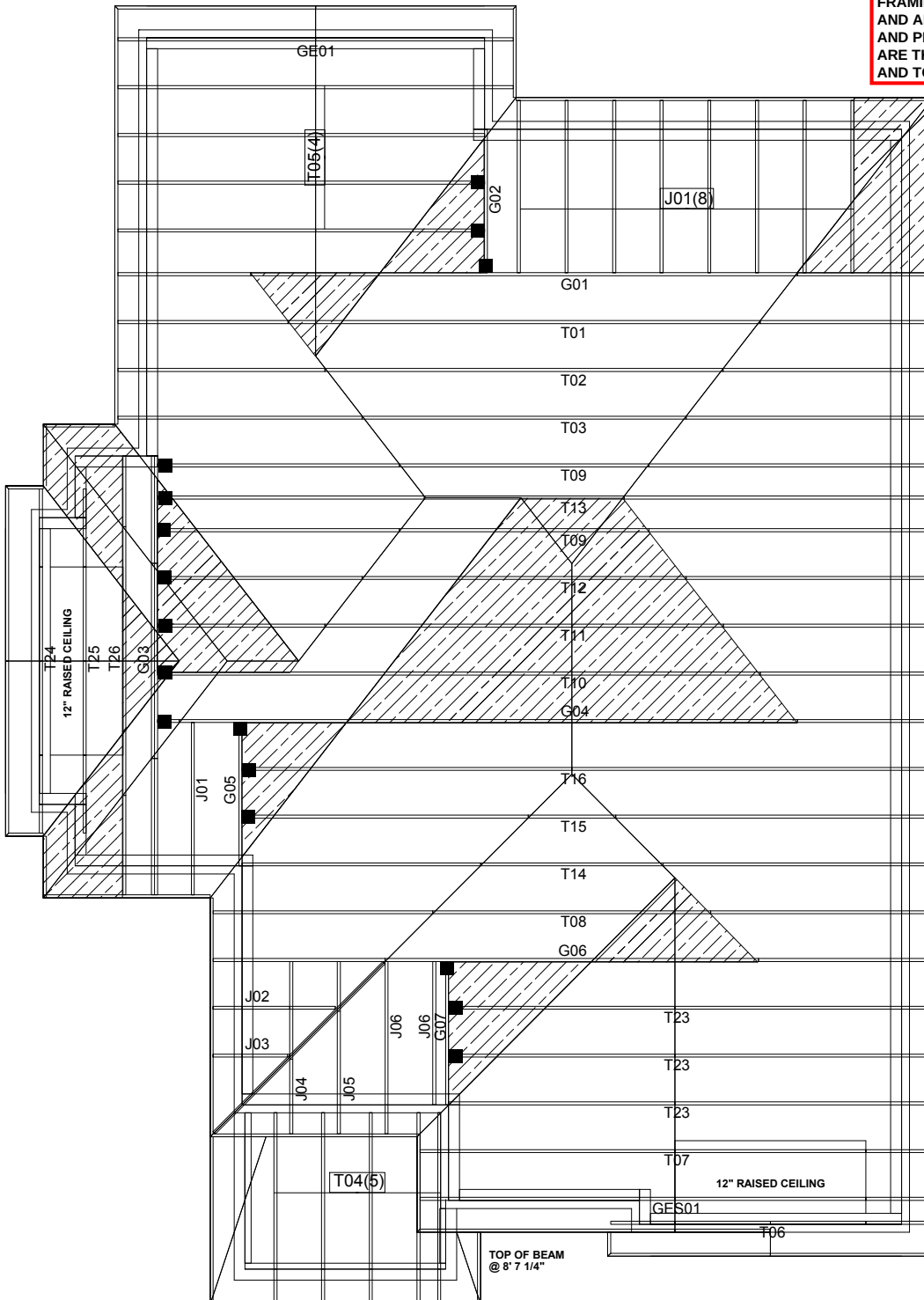
- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-008

The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O.Reg 33/212 as amended and as shown on the engineering calculation page of each component.

The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.

Deflection: LL = L/260
1.33LL+DL=L/360

Architectural Drawing Info:
Date: MAY 22, 2018
Project number: 17-55
Model: BELLE 1



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C.
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

▼ LUS24
● HGUS26
■ LJS26DS
✕ HGUS26-2

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3 PSF
BOT CH. LL = 0 PSF
DL = 7 PSF
TOTAL LOAD = 33.3 PSF
SPACING = 24" O/C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA B86-09
- TPIC 2011

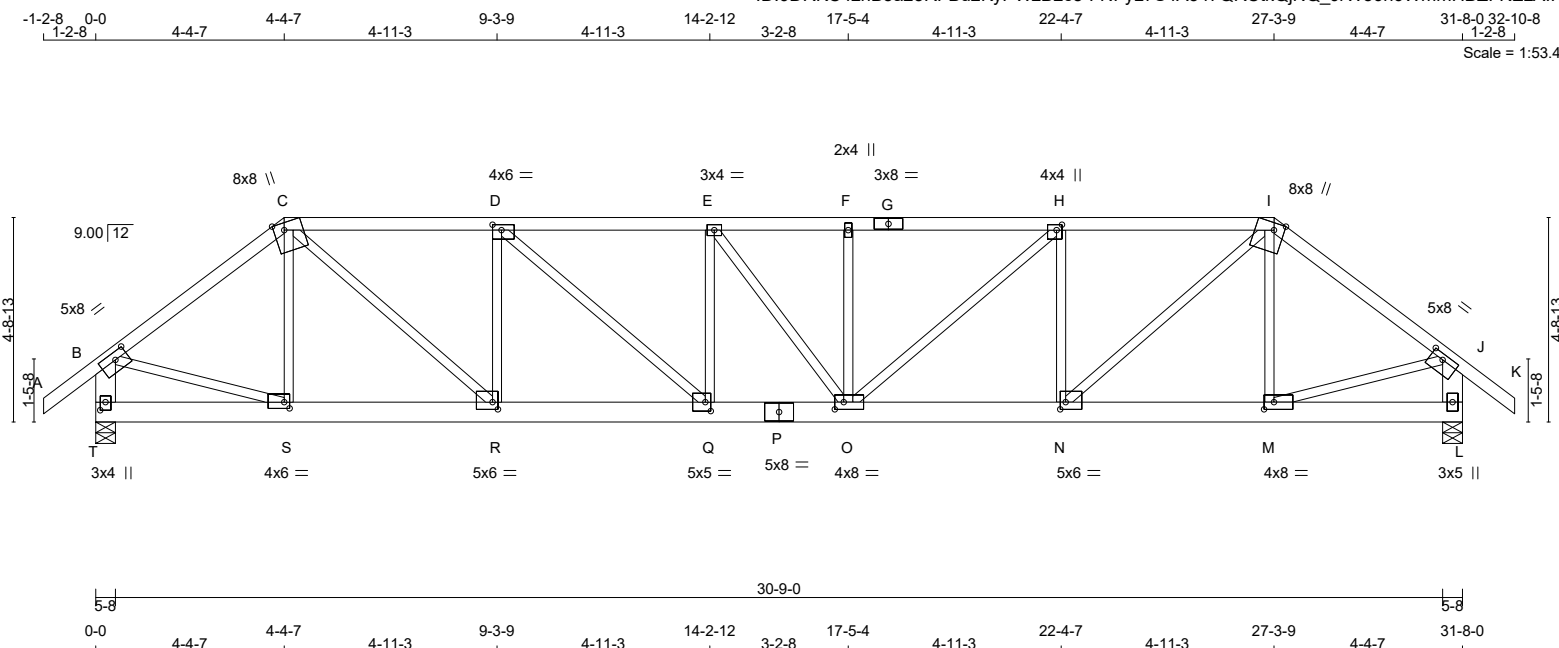
Model: **BELLE 1 EL 2**

Customer: **GREEN YORK HOMES**

Project: **GRANELLI HOMES**

Location: **BRAMPTON**

Date: **5/31/2018** Drawn by: **BB**



TOTAL WEIGHT = 153 lb

LUMBER

CONSIDER N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	No.2	SPF
C - G	2x4	2100F 1.8E	SPF
G - I	2x4	2100F 1.8E	SPF
I - K	2x4	No.2	SPF
T - B	2x6	No.2	SPF
L - J	2x6	No.2	SPF
T - P	2x6	No.2	SPF
P - L	2x6	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

B	T	TYPE	PLATES	W	LEN	Y	X
A	B	TMW-t	MT20	5.0	8.0	2.00	3.50
C	D	TTWW+m	MT20	8.0	8.0	Edge	3.00
D	E	DMW-t	MT20	4.0	6.0	1.50	2.50
E	F	MMW-t	MT20	3.0	4.0		
F	G	TMW+w	MT20	2.0	4.0		
G	H	TS-t	MT20	3.0	8.0		
H	I	MMW+t	MT20	4.0	4.0	1.50	1.50
I	J	TTWW+m	MT20	8.0	8.0	Edge	3.00
J	K	TMW-t	MT20	5.0	8.0	1.50	3.50
K	L	BMV1+p	MT20	3.0	5.0		
L	M	BMWV-t	MT20	4.0	8.0	2.00	2.75
M	N	BMWV-t	MT20	5.0	6.0	2.00	1.50
N	O	BMWVW-t	MT20	4.0	8.0	2.00	2.50
O	P	BS-t	MT20	5.0	8.0		
P	Q	BMWV-t	MT20	5.0	5.0	2.50	1.50
Q	R	BMWV-t	MT20	5.0	6.0	2.00	1.50
R	S	BMWV-t	MT20	4.0	6.0	1.75	1.50
S	T	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2635	0	2635	0	0	5-8	5-8
L	3106	0	3106	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1847	1306 / 0	0 / 0	0 / 0	0 / 0	540 / 0	0 / 0
L	2179	1529 / 0	0 / 0	0 / 0	0 / 0	649 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T. L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 30	-77.4	-77.4	0.10 (1)	10.00	S-C	-456 / 0	0.15 (1)	
B-C	-2959 / 0	-77.4	-77.4	0.49 (1)	3.60	C-R	0 / 2874	0.71 (1)	
C-D	-4509 / 0	-77.4	-77.4	0.34 (1)	3.85	R-D	-1798 / 0	0.58 (1)	
D-E	-6064 / 0	-77.4	-77.4	0.45 (1)	3.29	D-Q	0 / 2076	0.51 (1)	
E-F	-5938 / 0	-145.9	-145.9	0.36 (1)	3.34	Q-E	-204 / 11	0.07 (1)	
F-G	-5938 / 0	-145.9	-145.9	0.65 (1)	3.06	E-O	-213 / 0	0.10 (1)	
G-H	-5938 / 0	-145.9	-145.9	0.65 (1)	3.06	O-F	-583 / 0	0.19 (1)	
H-I	-4923 / 0	-145.9	-145.9	0.57 (1)	3.42	O-H	0 / 1355	0.34 (1)	
I-J	-3519 / 0	-77.4	-77.4	0.57 (1)	3.25	N-H	-1711 / 0	0.55 (1)	
J-K	0 / 30	-77.4	-77.4	0.10 (1)	10.00	N-I	0 / 2831	0.70 (1)	
T-B	-2604 / 0	0.0	0.0	0.19 (1)	6.39	M-I	-479 / 0	0.15 (1)	
L-J	-3046 / 0	0.0	0.0	0.22 (1)	6.00	B-S	0 / 2427	0.60 (1)	
						M-J	0 / 2885	0.71 (1)	
T-S	0 / 0	-17.5	-17.5	0.05 (4)	10.00				
S-R	0 / 2352	-17.5	-17.5	0.33 (1)	10.00				
R-Q	0 / 4509	-17.5	-17.5	0.67 (1)	10.00				
Q-P	0 / 6064	-33.0	-33.0	0.88 (1)	10.00				
P-O	0 / 6064	-33.0	-33.0	0.88 (1)	10.00				
O-N	0 / 4923	-33.0	-33.0	0.73 (1)	10.00				
N-M	0 / 2798	-33.0	-33.0	0.41 (1)	10.00				
M-L	0 / 0	-33.0	-33.0	0.08 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	27-3-9	-254	-254	---	FRONT	VERT	TOTAL
Q	14-2-12	-1117	-1117	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-4-7
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 17.5-4 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 833 (0.46")

CSI: TC=0.65/1.00 (F-H:1) , BC=0.88/1.00 (O-Q:1) ,
WB=0.71/1.00 (J-M:1) , SSI=0.37/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-BELLE 1 EL 2	DRWG NO.
NE0618-008	G01	1	1	TRUSS DESC.		PAGE 4 OF 46

Version 8.210 S Mar 12 2018 MITek Industries, Inc. Sat Jun 2 15:24:46 2018 Page 2
ID:9DRKC4znB5a23RFBu2KyPWzB2c8-FNFyL7S4A347QRCtkQjNQ JrW56h6WmmHBZFKEzAll?

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 253.5 lbs FACTORED DOWN AT 27-3-9
ON TOP CHORD, AND 1117.1 lbs FACTORED
DOWN AT 14-2-12 ON BOTTOM CHORD.
DESIGN FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

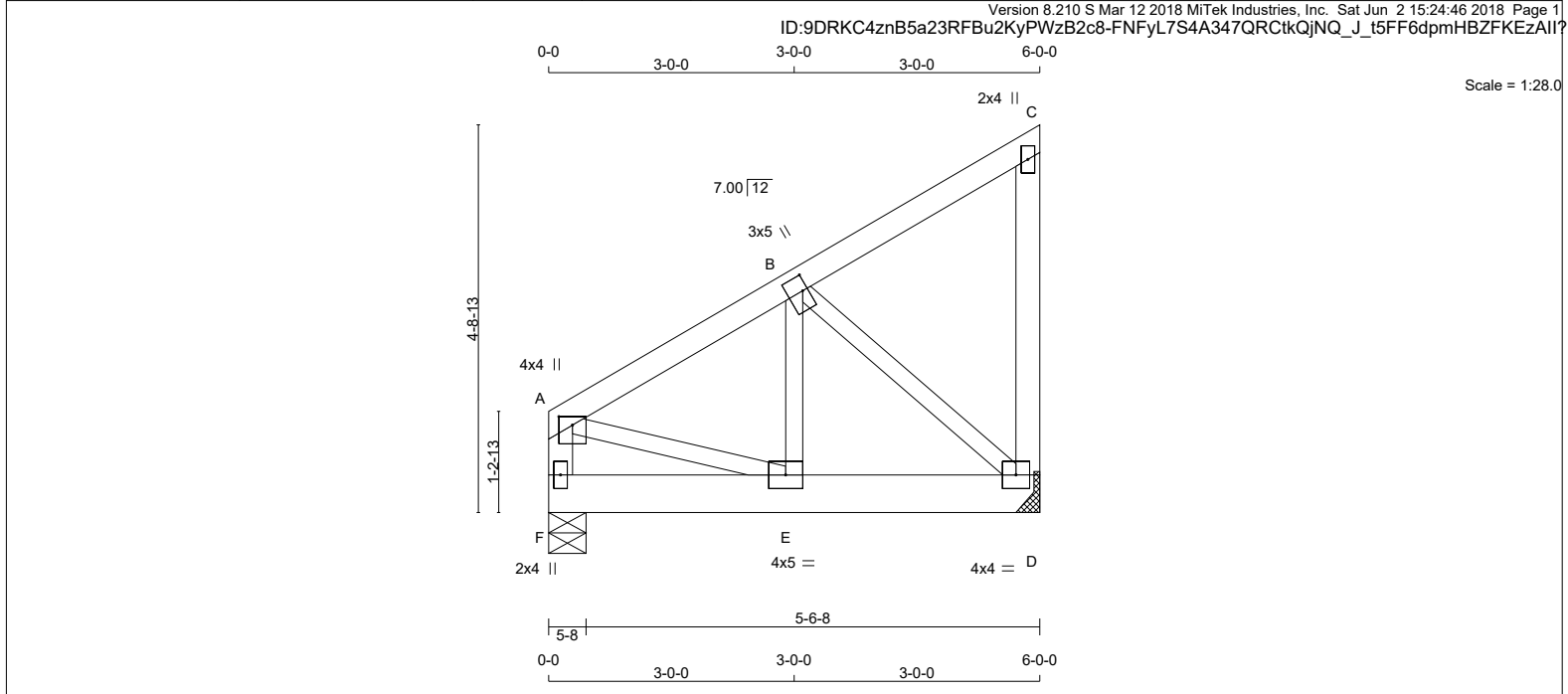
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 1.00 (P) (INPUT = 1.00)

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TOTAL WEIGHT = 31 lb										[M]
LUMBER N. L. G. A. RULES CHORDS SIZE										

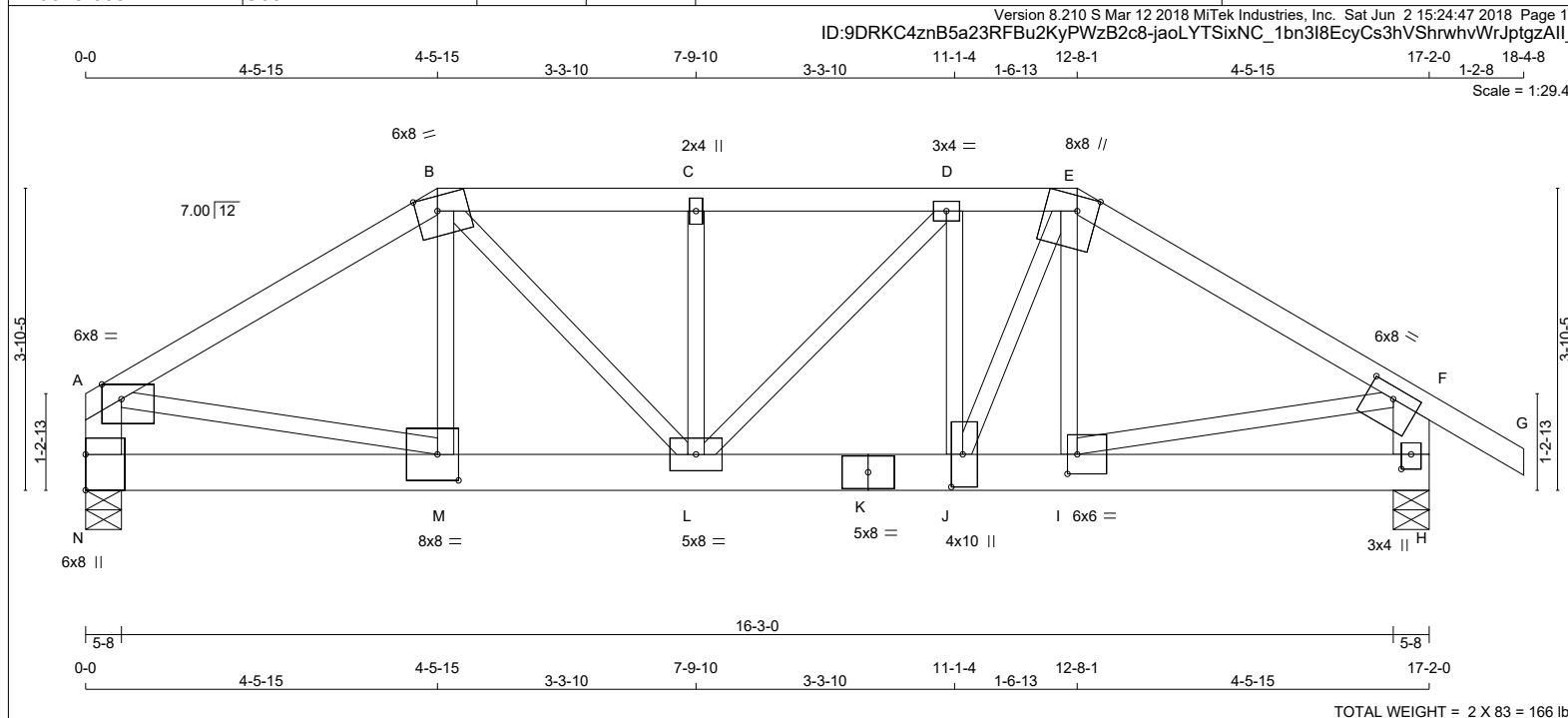
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-BELLE 1 EL 2	DRWG NO.
NE0618-008	G02	1	1	TRUSS DESC.		PAGE 6 OF 46

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:24:46 2018 Page 2
ID:9DRKC4znB5a23RFBu2KyPWzB2c8-FNFyL7S4A347QRCtkQjNQ_J_t5FF6dpmHBZFKEzAlI?

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

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LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										[M]
N. L. G. A. RULES		LUMBER		DESCR.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	DESIGN CRITERIA				
CHORDS	SIZE				VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX				IN-SX	
A - B	2x4	DRY	No.2	SPF	JT							*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE			
B - E	2x4	DRY	No.2	SPF	N	7209 0	7209 0 0	5-8	5-8						
E - G	2x4	DRY	No.2	SPF	H	5692 0	5692 0 0	5-8	5-8						
N - A	2x6	DRY	No.2	SPF											
H - F	2x6	DRY	No.2	SPF											
N - K	2x6	DRY	No.2	SPF							SPECIFIED LOADS:				
K - H	2x6	DRY	No.2	SPF											

[illegible]

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H	<u>SPACING</u> = <u>24.0</u> <u>IN. C/C</u>
	BRACING	

CHORDS #ROWS			SURFACE SPACING (IN)		LOAD(PLF)		TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
TOP CHORDS : (0.122"x3") SPIRAL NAILS							ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										GIRDER TYPE: CStdGirder									
A- B	1	12			TOP											START DISTANCE = 0-0										
B- E	1	12			TOP											START SPAN CARRIED = 31-2-8										
E- G	1	12			TOP											END DISTANCE = 11-1-4										
N- A	2	12			TOP											END SPAN CARRIED = 31-2-8										
H- F	2	12			TOP											END WALL WIDTH = 5-8										
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS																	APPLIED TO FRONT SIDE OF BOTTOM CHORD.									
N- K	2	8			SIDE(340.9)	C H O R D S										W E B S										
K- H	2	8			SIDE(340.9)	MAX. FACTORED MEMB. FORCE (LBS)										FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)										
WEBS : (0.122"x3") SPIRAL NAILS																	MAX. MEMB. FORCE (LBS) CSI (LC)									
2x3	1	6				FR.TO FROM_TO LENGTH FR.TO																				
																	*** NON STANDARD GIRDER ***									
																	ADDTL USER-DEFINED LOADS APPLIED TO									

240	1	0	A-B	-8544 / 0	-77.4	-77.4	0.50 (1)	3.05	M-B	0 / 1846	0.23 (1)	ADDITIONAL DEFLECTIONS APPLIED TO ALL LOAD CASES.
			B-C	-9473 / 0	-77.4	-77.4	0.37 (1)	2.94	B-L	0 / 2967	0.37 (1)	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS	C-D	-9473 / 0	-77.4	-77.4	0.40 (1)	2.91	L-C	-259 / 0	0.03 (1)	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010
	D-E	-9209 / 0	-77.4	-77.4	0.31 (1)	3.02	L-D	0 / 385	0.05 (1)	
	E-F	-8119 / 0	-77.4	-77.4	0.47 (1)	3.16	L-D	-335 / 0	0.04 (1)	

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR										THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014	
F-G	0/25	-77.4	-77.4	0.05 (1)	10.00	J-E	0/5441	0.67 (1)			
N-A	-5834/0	0.0	0.0	0.21 (1)	6.10	I-E	-1196/0	0.14 (1)			
H-F	-5651/0	0.0	0.0	0.20 (1)	6.19	A-M	0/7493	0.93 (1)			

THE LOAD TO BE TRANSFERRED TO EACH PLY.	N-M	0/0	-699.3	-699.3	0.50 (1)	10.00	I-F	0/7120	0.88 (1)	- CSA 086-09 - TPIC 2011
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED	M-L	0/7427	-699.3	-699.3	0.88 (1)	10.00				
TO ONE SIDE THAT THE CORRESPONDING NAILING	L-K	0/9209	-699.3	-699.3	0.90 (1)	10.00				
PATTERN SHALL BE CAPABLE OF TRANSFERRING.	K-J	0/9209	-699.3	-699.3	0.90 (1)	10.00				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE	J-I	0/6969	-17.5	-17.5	0.47 (1)	10.00				RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED
SIDE OR ON THE TOP	L-H	0/0	-17.5	-17.5	0.02 (4)	10.00				ROOF LIVE LOAD


 FACTORED CONCENTRATED LOADS (LBS)
 JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE
 TOTAL DEFLECTION (IN)

	J	11-14	-3602	-3602	---	FRONT	VERT	TOTAL	CALCULATED VERT. DEFL.(1L) = 0.950 (0.22") CSI: TC=0.50/1.00 (A-B:1), BC=0.90/1.00 (J-L:1), WB=0.93/1.00 (A-M:1), SSI=0.61/1.00 (M-N:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

PROVINCE OF ONTARIO
Jun 02, 2018

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SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	8.0	2.25	3.00
B	TTWW-m	MT20	6.0	8.0	Edge	3.25
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	3.0	4.0		
E	TTWW+m	MT20	8.0	8.0	Edge	3.00
F	TMVW-t	MT20	6.0	8.0	1.75	4.00
H	BMV1+p	MT20	3.0	4.0	2.25	1.50
I	BMWW-t	MT20	6.0	6.0	3.00	1.50
J	BMWW+t	MT20	4.0	10.0	5.00	1.75
K	BS-t	MT20	5.0	8.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	8.0	8.0	4.00	3.25
N	BMV1+t	MT20	6.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3602.1 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

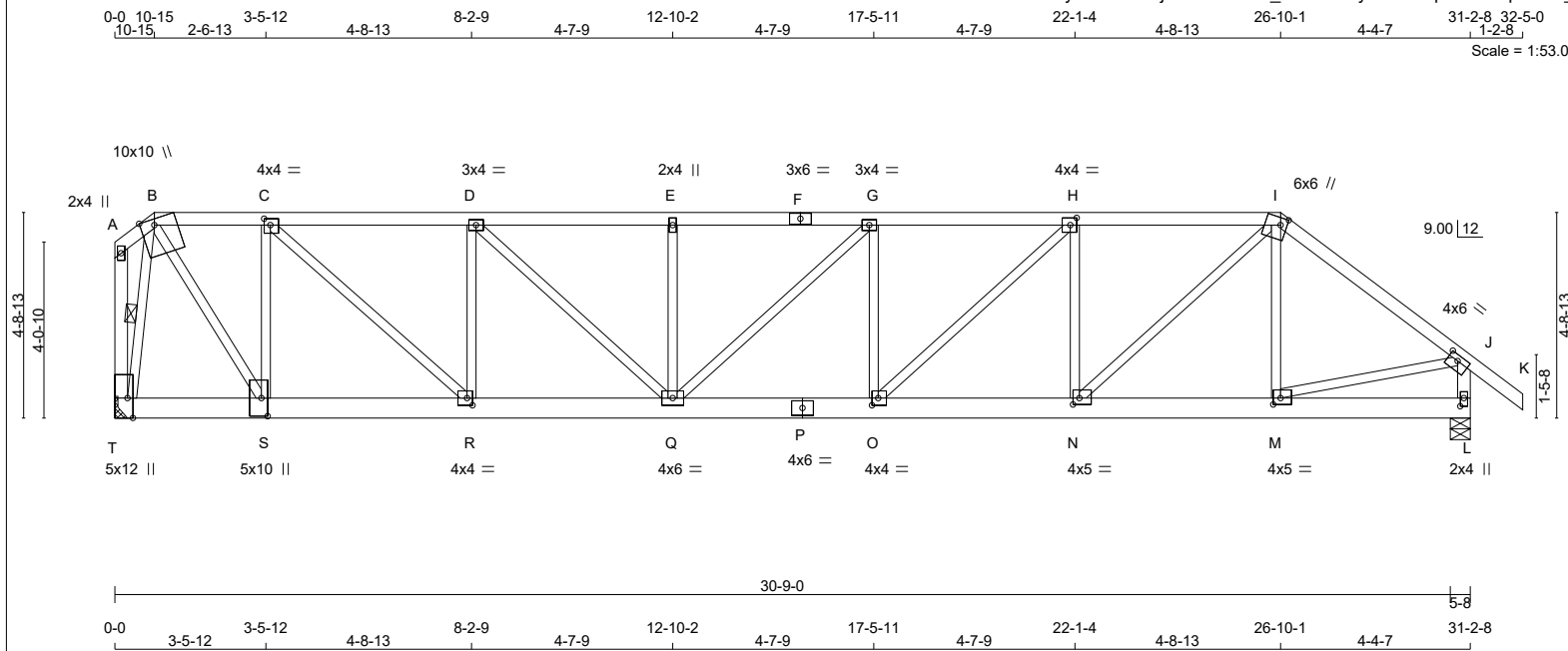
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 1.00 (K) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 154 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
T - A	2x4	DRY No.2	SPF
B - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
I - K	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
A - B	2x4	DRY No.2	SPF
T - P	2x6	DRY No.2	SPF
P - L	2x6	DRY No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TTWW+m	MT20	10.0	10.0	Edge	4.00
C	TMWW-t	MT20	4.0	4.0	1.75	1.75
D	TMWW-t	MT20	3.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	3.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	6.0	6.0	2.00	1.75
J	TMVW-t	MT20	4.0	6.0	1.50	2.75
L	BMV1+p	MT20	2.0	4.0	2.25	1.00
M	BMWW-t	MT20	4.0	5.0	1.75	2.00
N	BMWW-t	MT20	4.0	5.0	1.75	1.75
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.00	1.50
S	BMWW+t	MT20	5.0	10.0	5.00	1.75
T	BMVW1+t	MT20	5.0	12.0	Edge	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
T	3602	3602	0	MECHANICAL
L	1828	1828	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT T TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
T	2528 1769 / 0 0 / 0 0 / 0 0 759 / 0 0 / 0
L	1281 909 / 0 0 / 0 0 / 0 0 372 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF B-T. DBS = 8-0-0. CBF = 176 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	(LC)			(LBS)	(LC)		
FR-TO		FROM	TO		FR-TO				
T-A	-66 / 0	0.0	0.0	0.02 (1)	7.81	B-S	0 / 3736	0.92 (1)	
B-C	-2626 / 0	-145.9	-145.9	0.25 (1)	4.02	S-C	-1089 / 0	0.35 (1)	
C-D	-3396 / 0	-77.4	-77.4	0.45 (1)	3.43	C-R	0 / 1047	0.26 (1)	
D-E	-3627 / 0	-77.4	-77.4	0.48 (1)	3.30	R-D	-592 / 0	0.19 (1)	
E-F	-3627 / 0	-77.4	-77.4	0.44 (1)	3.34	D-Q	0 / 316	0.08 (1)	
F-G	-3627 / 0	-77.4	-77.4	0.44 (1)	3.34	Q-E	-354 / 0	0.11 (1)	
G-H	-3416 / 0	-77.4	-77.4	0.50 (1)	3.36	Q-G	0 / 289	0.07 (1)	
H-I	-2742 / 0	-77.4	-77.4	0.42 (1)	3.79	O-G	-546 / 0	0.18 (1)	
I-J	-1936 / 0	-77.4	-77.4	0.38 (1)	4.43	O-H	0 / 925	0.23 (1)	
J-K	0 / 30	-77.4	-77.4	0.10 (1)	10.00	N-H	-1031 / 0	0.33 (1)	
L-J	-1797 / 0	0.0	0.0	0.20 (1)	6.19	N-I	0 / 1632	0.40 (1)	
A-B	0 / 0	-145.9	-145.9	0.02 (1)	10.00	M-I	-265 / 0	0.09 (1)	
						M-J	0 / 1588	0.39 (1)	
						T-B	-3526 / 0	0.68 (1)	
T-S	0 / 724	-33.0	-33.0	0.17 (1)	10.00				
S-R	0 / 2626	-17.5	-17.5	0.42 (1)	10.00				
R-Q	0 / 3396	-17.5	-17.5	0.49 (1)	10.00				
Q-P	0 / 3416	-17.5	-17.5	0.50 (1)	10.00				
P-O	0 / 3416	-17.5	-17.5	0.50 (1)	10.00				
O-N	0 / 2743	-17.5	-17.5	0.40 (1)	10.00				
N-M	0 / 1539	-17.5	-17.5	0.23 (1)	10.00				
M-L	0 / 0	-17.5	-17.5	0.04 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	3-5-12	-2077	-2077	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-5-12 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.33")

CSI: TC=0.50/1.00 (G-H:1), BC=0.50/1.00 (O-Q:1), WB=0.92/1.00 (B-S:1), SSi=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2077.5 lbs FACTORED DOWN AT 3-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

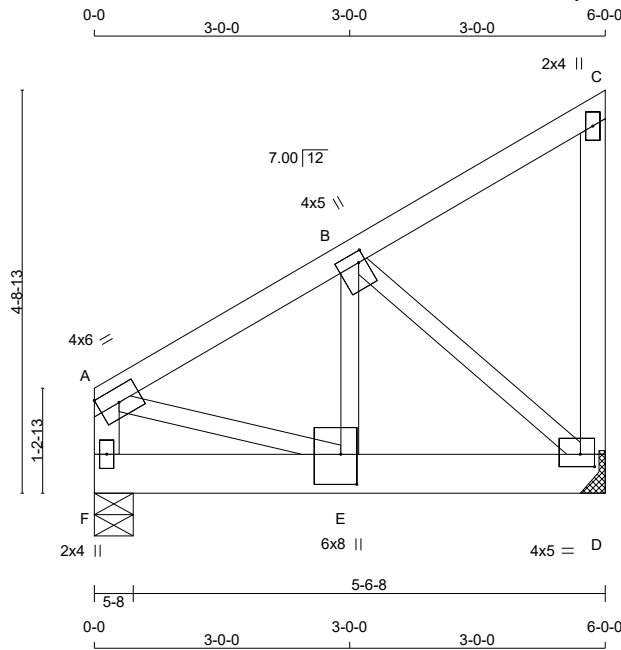
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

JSI METAL= 0.77 (B) (INPUT = 1.00)





TOTAL WEIGHT = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+t	MT20	4.0	6.0		Edge
B	TMWW+t	MT20	4.0	5.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	5.0	1.75	2.00
E	BMWW+t	MT20	6.0	8.0	4.25	2.25
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	2078	0	2078	0	5-8	5-8
D	2078	0	2078	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1458	1020 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0
D	1458	1020 / 0	0 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)		MAX. MEMB. UNBRAC. LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)	
			FROM	TO			FROM	TO
F-A	-1315 / 0	0.0	0.0	0.15 (1)	7.02	A-E	0 / 1467	0.36 (1)
A-B	-1627 / 0	-77.4	-77.4	0.13 (1)	5.05	E-B	0 / 1772	0.44 (1)
B-C	-10 / 0	-77.4	-77.4	0.09 (1)	10.00	B-D	-1866 / 0	0.49 (1)
D-C	-97 / 0	0.0	0.0	0.03 (1)	7.81			
F-E	0 / 0	-615.3	-615.3	0.35 (1)	10.00			
E-D	0 / 1413	-615.3	-615.3	0.54 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 27-8-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 27-8-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.15/1.00 (A-F:1), BC=0.54/1.00 (D-E:1), WB=0.49/1.00 (B-D:1), SSI=0.72/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



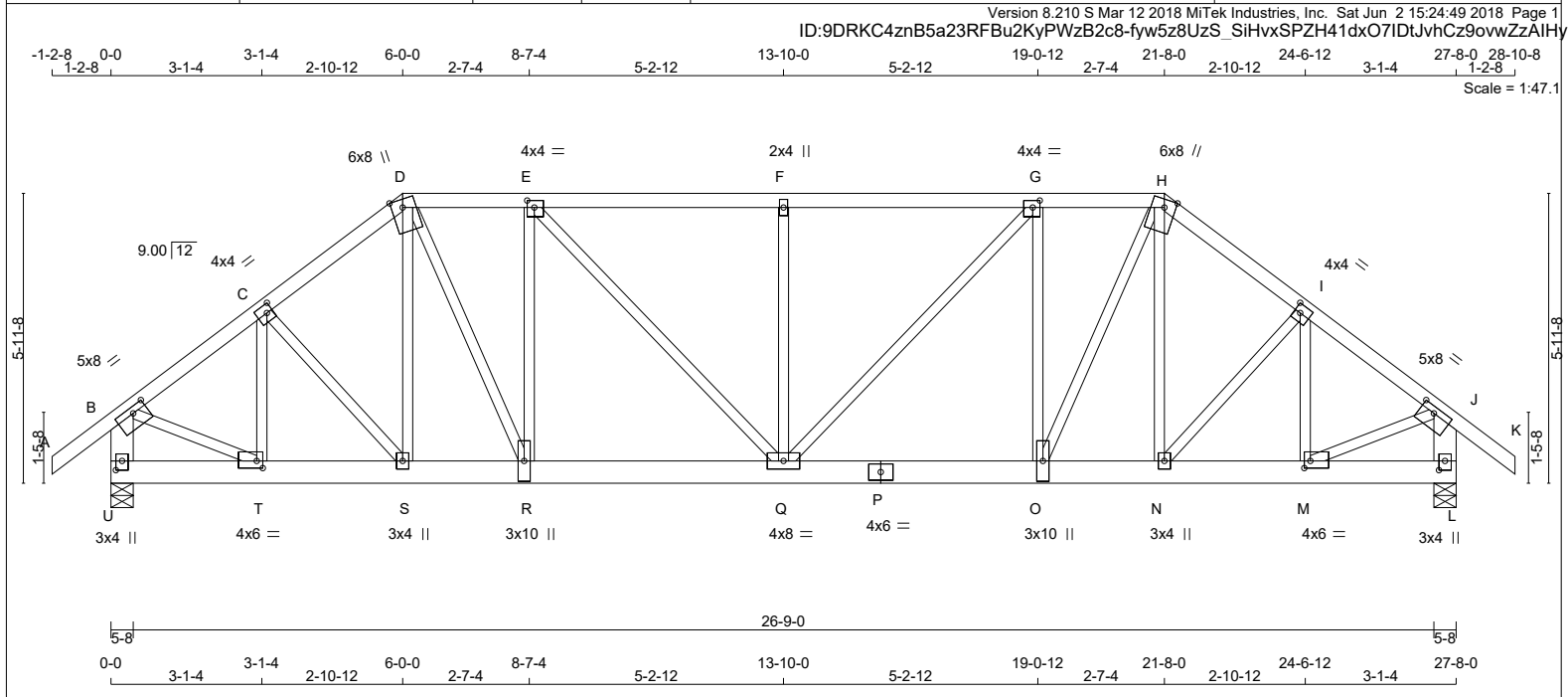
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-BELLE 1 EL 2	DRWG NO.
NE0618-008	G05	1	1	TRUSS DESC.		PAGE 12 OF 46

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:24:48 2018 Page 2
ID:9DRKC4znB5a23RFBu2KyPWzB2c8-BmMjnpTKihKrfIMGsrnrVPOJyvtWaUo3kV2MO6zAIHz

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





										TOTAL WEIGHT = 150 lb	
										[M]	
LUMBER N. L. G. A. RULES				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						DESIGN CRITERIA	
CHORDS SIZE LUMBER DESCR.				BEARINGS							
				FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
				GROSS REACTION		GROSS REACTION		BRG		BRG	
				JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A - D 2x4 DRY No.2 SPF				U	2936	0	2936	0	0	5-8	5-8
D - H 2x4 DRY No.2 SPF				L	2010	0	2010	0	0	5-8	5-8
H - K 2x4 DRY No.2 SPF											
U - B 2x6 DRY No.2 SPF											
L - J 2x6 DRY No.2 SPF											
U - P 2x6 DRY No.2 SPF											
P - L 2x6 DRY No.2 SPF											
				UNFACTORED REACTIONS							
				1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
				JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
ALL WEBS 2x3 DRY No.2 SPF				U	2061	1442 / 0	0 / 0	0 / 0	0 / 0	618 / 0	0 / 0
EXCEPT				L	1408	1002 / 0	0 / 0	0 / 0	0 / 0	406 / 0	0 / 0
DRY: SEASONED LUMBER.											
				SPECIFIED LOADS:							
				TOP CH. LL = 23.3 PSF							
				DL = 3.0 PSF							
				BOT CH. LL = 0.0 PSF							
				DL = 7.0 PSF							
				TOTAL LOAD = 33.3 PSF							

TOTAL WEIGHT = 150 lb

PLATES (table is in inches)							BRACING							LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM																																																
JT	TYPE	PLATES	W	LEN	Y	X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.																																																							
B	TMVW-t	MT20	5.0	8.0	1.50	3.50	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.							GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 8-7-4 OF SPAN MEASURED FROM THE LEFT.																																																
C	TMVW-t	MT20	4.0	4.0	2.00	1.50																																																								
D	TTVW+m	MT20	6.0	8.0	Edge	2.75	LOADING TOTAL LOAD CASES: (4)							*** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.																																																
E	TMVW-t	MT20	4.0	4.0	1.75	1.75																																																								
F	TMW+w	MT20	2.0	4.0			CHORDS							WEBS																																																
G	TMVW-t	MT20	4.0	4.0	1.75	1.75																																																								
H	TTVW+m	MT20	6.0	8.0	Edge	2.75	MEMB. FORCE (LBS)							FACTORED VERT. LOAD (PLF)			MAX. UNBRACED LENGTH			MEMB. FORCE (LBS)			FACTORED MAX. CSI (LC)																																							
I	TMVW-t	MT20	4.0	4.0	2.00	1.50																																																								
J	TMVW-t	MT20	5.0	8.0	1.50	3.50	FR-TO							FROM TO							FR-TO																																									
L	BMV1+p	MT20	3.0	4.0	2.25	1.50																																																								
M	BMVW-t	MT20	4.0	6.0	1.75	1.50	A-B							0 / 30							-77.4 -77.4 0.10 (1)							10.00							T-C							-805 / 0							0.18 (1)													
N	BMVW-t	MT20	3.0	4.0																																																										
O	BMVW-t	MT20	3.0	10.0			B-C							-3047 / 0							-77.4 -77.4 0.24 (1)							3.79							C-S							0 / 413							0.10 (1)													
P	BS-t	MT20	4.0	6.0																																																										
Q	BMVW-t	MT20	4.0	8.0			C-D							-3425 / 0							-77.4 -77.4 0.26 (1)							3.56							S-D							-89 / 62							0.05 (1)													
R	BMVW-t	MT20	3.0	10.0																																																										
S	BMVW-t	MT20	3.0	4.0			D-E							-3441 / 0							-145.9 -145.9 0.35 (1)							3.42							D-R							0 / 1704							0.42 (1)													
T	BMVW-t	MT20	4.0	6.0	1.75	1.50																																																								
U	BMV1+p	MT20	3.0	4.0	2.25	1.50	E-F							-3141 / 0							-77.4 -77.4 0.50 (1)							3.48							R-E							-85 / 51							0.04 (1)													
							F-G							-3141 / 0							-77.4 -77.4 0.50 (1)							3.48							E-Q							-440 / 0							0.51 (1)													
							G-H							-2337 / 0							-77.4 -77.4 0.25 (1)							4.21							Q-F							-428 / 0							0.23 (1)													
							H-I							-2227 / 0							-77.4 -77.4 0.18 (1)							4.39							Q-G							0 / 1176							0.29 (1)													
							I-J							-2018 / 0							-77.4 -77.4 0.17 (1)							4.58							O-G							-1177 / 0							0.62 (1)													
							J-K							0 / 30							-77.4 -77.4 0.10 (1)							10.00							O-H							0 / 1352							0.33 (1)													
							U-B							-2861 / 0							0.0							0.0 0.21 (1)							6.16							N-H							-49 / 14							0.03 (1)						
							L-J							-1965 / 0							0.0							0.0 0.14 (1)							7.16							N-I							0 / 210							0.05 (1)						
							M-I							-551 / 0							0.13 (1)																																									
							U-T							0 / 0							-33.0 -33.0 0.06 (1)							10.00							B-T							0 / 2582							0.64 (1)													
							T-S							0 / 2447							-33.0 -33.0 0.39 (1)							10.00							M-I							0 / 1716							0.42 (1)													

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD/EQUAL S.23.3 P.S.F. SPECIFIED

	S-R	0 / 2720	-33.0	-33.0	0.42 (1)	10.00	M-F	0 / 1716	-0.42 (1)	TOTAL LOAD CASES: (4) CHORDS FACTORED WEBS ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.92") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12") ALLOWABLE DEFL.(TL)= L/360 (0.92") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")		
	R-Q	0 / 3442	-17.5	-17.5	0.53 (1)	10.00						
	Q-P	0 / 2337	-17.5	-17.5	0.34 (1)	10.00						
	P-O	0 / 2337	-17.5	-17.5	0.34 (1)	10.00						
	O-N	0 / 1764	-17.5	-17.5	0.27 (1)	10.00						
	N-M	0 / 1626	-17.5	-17.5	0.25 (1)	10.00						
	M-L	0 / 0	-17.5	-17.5	0.04 (1)	10.00						

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6'-0"	-348	-348	---	FRONT	VERT	TOTAL
R	8'-7.4"	-1462	-1462	---	FRONT	VERT	TOTAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-BELLE 1 EL 2	DRWG NO.
NE0618-008	G06	1	1	TRUSS DESC.		PAGE 14 OF 46

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:24:49 2018 Page 2
ID:9DRKC4znB5a23RFBu2KyPWzB2c8-fyw5z8UzS SiHvxSPZH41dxO7IDtJvhCz9ovwZzAIHy

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 6-0-0 ON TOP CHORD, AND 1461.9 lbs FACTORED DOWN AT 8-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

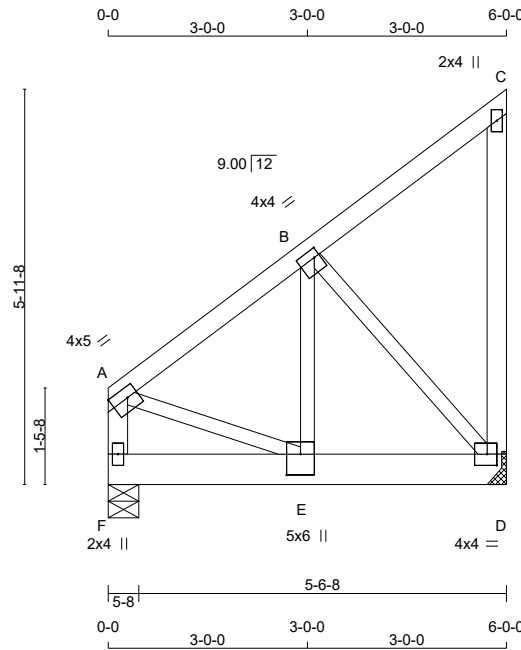
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.75 (P) (INPUT = 1.00)





Scale = 1:34.7

TOTAL WEIGHT = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+t	MT20	4.0	5.0	1.75	Edge
B	TMWW+t	MT20	4.0	4.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	BMVW1+t	MT20	4.0	4.0	2.00	1.75
E	BMWW+t	MT20	5.0	6.0	3.75	2.50
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	1461	0	1461	0	5-8	5-8
D	1461	0	1461	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1026	718 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
D	1026	718 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1	
FR-TO		FROM	TO	LENGTH	FR-TO				
F-A	-956 / 0	0.0	0.0	0.11 (1)	7.81	A-E	0 / 833	0.21 (1)	
A-B	-969 / 0	-77.4	-77.4	0.13 (1)	6.15	E-B	0 / 1173	0.29 (1)	
B-C	-15 / 0	-77.4	-77.4	0.11 (1)	6.25	B-D	-1169 / 0	0.37 (1)	
D-C	-92 / 0	0.0	0.0	0.06 (1)	7.81				
F-E	0 / 0	-409.8	-409.8	0.24 (1)	10.00				
E-D	0 / 787	-409.8	-409.8	0.34 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 19-0-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 19-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.13/1.00 (A-B:1), BC=0.34/1.00 (D-E:1), WB=0.37/1.00 (B-D:1), SSI=0.48/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



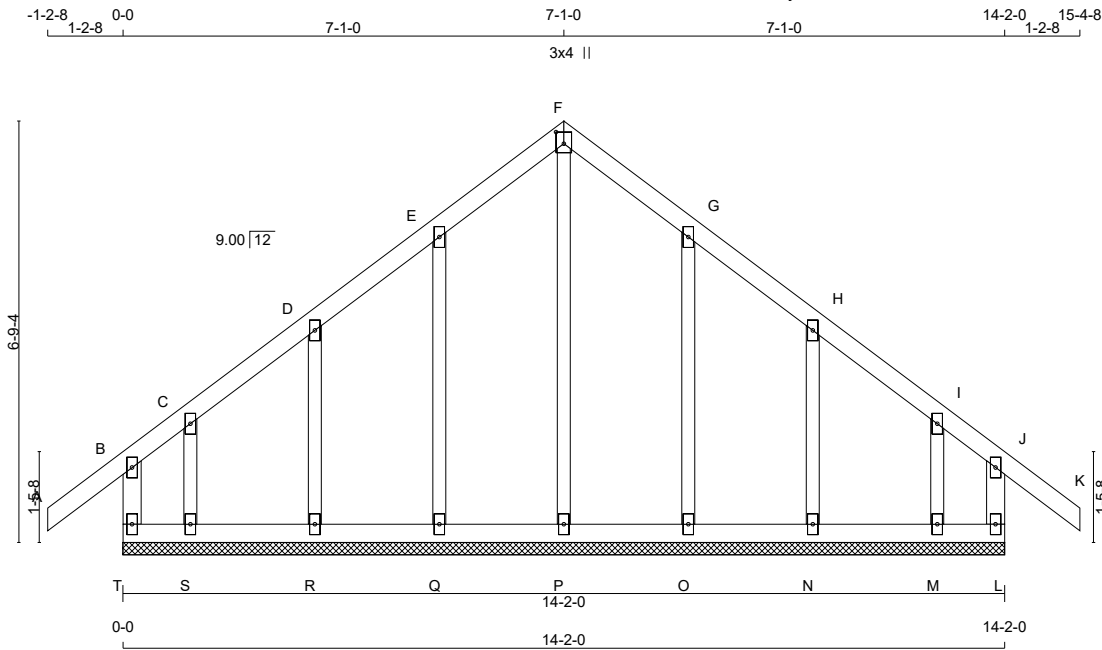
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-BELLE 1 EL 2	DRWG NO.
NE0618-008	G07	1	1	TRUSS DESC.		PAGE 16 OF 46

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 15:24:49 2018 Page 2
ID:9DRKC4znB5a23RFBu2KyPWzB2c8-fyw5z8UzS_SiHvxSPZH41dxU0IGnJzvCz9ovwZzAIHy

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.31 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:37.0

TOTAL WEIGHT = 63 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0-0 OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, G, H, I					
C	TMW+w	MT20	2.0	4.0	
F	TTW+p	MT20	3.0	4.0	2.25 1.50
J	TMV+p	MT20	2.0	4.0	
L	BMV1+p	MT20	2.0	4.0	
M, N, O, P, Q, R, S					
M	BMW1+w	MT20	2.0	4.0	
T	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
T-B	-165 / 0	0.0 0.0 0.03 (1)	7.81	P-F	-189 / 0	0.14 (1)	
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00	Q-E	-157 / 0	0.06 (1)	
B-C	-8 / 0	-77.4 -77.4 0.06 (1)	10.00	R-D	-159 / 0	0.04 (1)	
C-D	0 / 20	-77.4 -77.4 0.04 (1)	10.00	S-C	-73 / 0	0.01 (1)	
D-E	0 / 21	-77.4 -77.4 0.04 (1)	10.00	O-G	-157 / 0	0.06 (1)	
E-F	0 / 24	-77.4 -77.4 0.04 (1)	10.00	N-H	-159 / 0	0.04 (1)	
F-G	0 / 24	-77.4 -77.4 0.04 (1)	10.00	M-I	-73 / 0	0.01 (1)	
G-H	0 / 21	-77.4 -77.4 0.04 (1)	10.00				
H-I	0 / 20	-77.4 -77.4 0.04 (1)	10.00				
I-J	-8 / 0	-77.4 -77.4 0.06 (1)	10.00				
J-K	0 / 30	-77.4 -77.4 0.09 (1)	10.00				
L-J	-165 / 0	0.0 0.0 0.03 (1)	7.81				
T-S	-9 / 0	-17.5 -17.5 0.01 (1)	10.00				
S-R	-13 / 0	-17.5 -17.5 0.01 (4)	6.25				
R-Q	-17 / 0	-17.5 -17.5 0.01 (4)	6.25				
Q-P	-20 / 0	-17.5 -17.5 0.01 (4)	6.25				
P-O	-20 / 0	-17.5 -17.5 0.01 (4)	6.25				
O-N	-17 / 0	-17.5 -17.5 0.01 (4)	6.25				
N-M	-13 / 0	-17.5 -17.5 0.01 (4)	6.25				
M-L	-9 / 0	-17.5 -17.5 0.01 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (J-K:1), BC=0.01/1.00 (Q-R:4), WB=0.14/1.00 (F-P:1), SSI=0.06/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

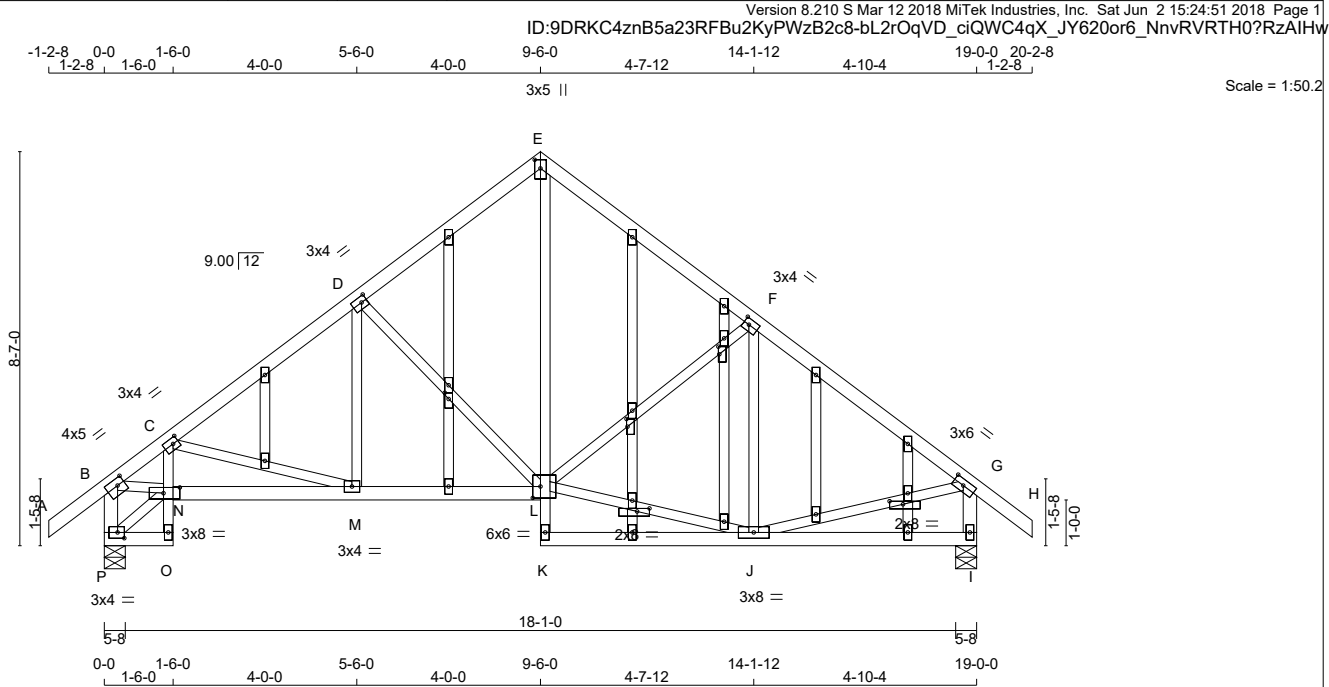
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (F) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 111 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - C	2x3	DRY	No.2
N - L	2x4	DRY	No.2
K - E	2x3	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	5.0 1.75 2.00
C	TMVW-t	MT20	3.0	4.0 1.50 1.50
D	TMVW-t	MT20	3.0	4.0 1.50 1.50
E	TTV+p	MT20	3.0	5.0 2.25 1.50
F	TMVW-t	MT20	3.0	4.0 1.50 1.50
G	TMVW-t	MT20	3.0	6.0 1.50 2.75
I	BMV1+p	MT20	2.0	4.0
J	BMVWW-t	MT20	3.0	8.0
J, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AC, AD, AE, AF	J NP+w	MT20	2.0	4.0
K	BMV+p	MT20	2.0	4.0
L	BVMWW-t	MT20	6.0	6.0 3.00 2.00
M	BMVW-t	MT20	3.0	4.0
N	BVMWW-t	MT20	3.0	8.0 1.50 4.25
O	BMV+p	MT20	2.0	4.0
P	BMVW-t	MT20	3.0	4.0 1.50 1.75
Q	NP+w	MT20	2.0	4.0 1.50 1.00
W	NP+w	MT20	2.0	4.0 2.00 0.25
Y	NP-p	MT20	2.0	8.0 0.75 3.25
AA	NP+w	MT20	2.0	4.0 2.00 0.25
AF	NP-p	MT20	2.0	8.0 0.75 3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
I	998 0	998 0	0 0	5-8	5-8
P	1005 0	1005 0	0 0	5-8	5-8

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	698	501 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
P	703	505 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	C-M	-209 / 0	0.06 (1)
B-C	-1312 / 0	-77.4 -77.4	0.09 (1)	5.57	M-D	0 / 118	0.04 (4)
C-D	-1055 / 0	-77.4 -77.4	0.16 (1)	5.95	D-L	-397 / 0	0.24 (1)
D-E	-747 / 0	-77.4 -77.4	0.15 (1)	6.25	L-J	0 / 743	0.17 (1)
E-F	-763 / 0	-77.4 -77.4	0.23 (1)	6.25	L-F	-185 / 0	0.11 (1)
F-G	-888 / 0	-77.4 -77.4	0.23 (1)	6.24	J-F	-231 / 16	0.09 (1)
G-H	0 / 30	-77.4 -77.4	0.09 (1)	10.00	B-N	0 / 1040	0.23 (1)
P-B	-979 / 0	0.0 0.0	0.10 (1)	7.81	J-G	0 / 751	0.17 (1)
I-G	-964 / 0	0.0 0.0	0.10 (1)	7.81	P-N	-22 / 0	0.00 (1)
P-O	0 / 17	-17.5 -17.5	0.01 (4)	10.00			
O-N	0 / 14	0.0 0.0	0.05 (1)	10.00			
N-C	0 / 41	0.0 0.0	0.06 (1)	10.00			
N-M	0 / 1057	-17.5 -17.5	0.21 (1)	10.00			
M-L	0 / 856	-17.5 -17.5	0.18 (1)	10.00			
K-L	0 / 33	0.0 0.0	0.02 (1)	10.00			
L-E	0 / 623	0.0 0.0	0.16 (1)	10.00			
K-J	0 / 6	-17.5 -17.5	0.11 (4)	10.00			
J-I	0 / 0	-17.5 -17.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.23/1.00 (F-G:1), BC=0.21/1.00 (M-N:1),
WB=0.24/1.00 (D-L:1), SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

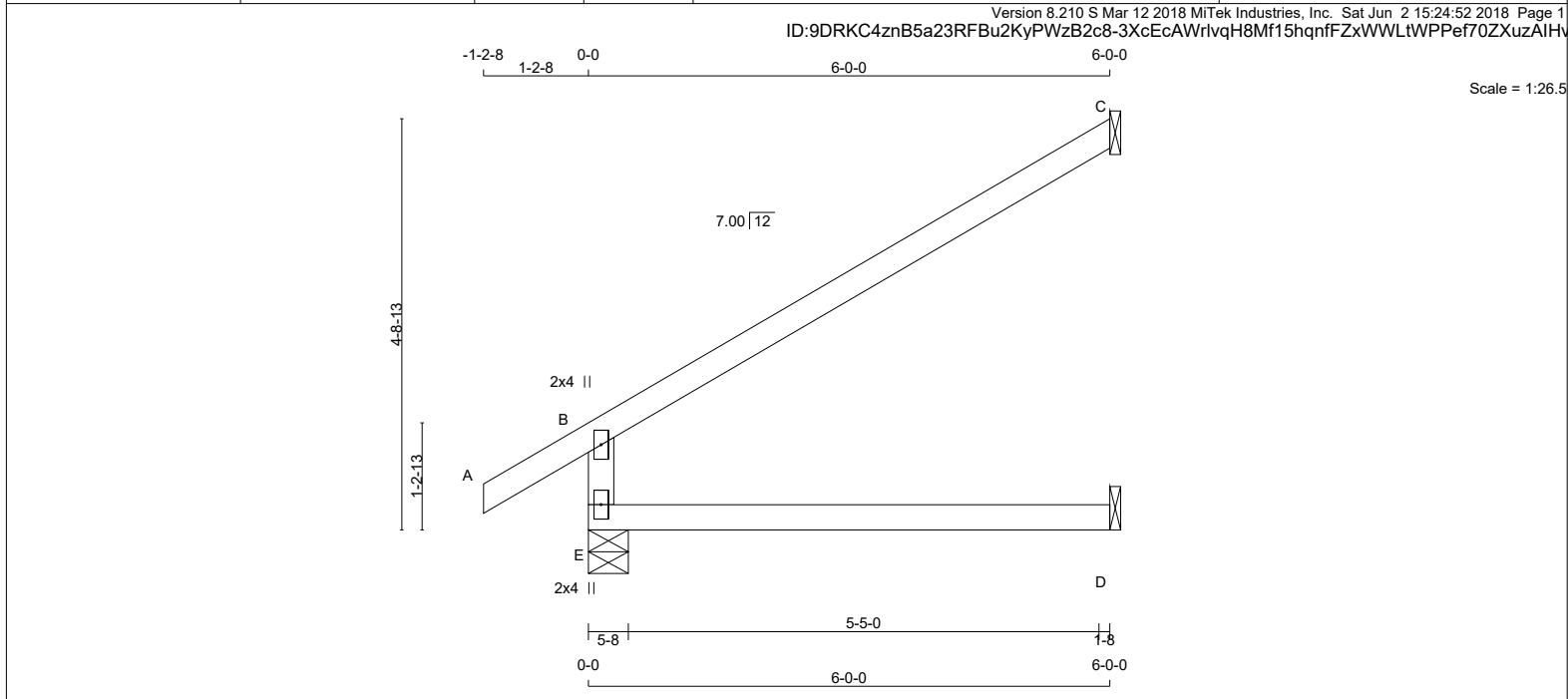
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 9 X 17 = 157 lb [M][F]

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER		SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT
E	451	0	451	0	0
C	174	0	174	0	0
D	44	0	49	0	0

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS					
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	314	234 / 0	0 / 0	0 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING					
TOTAL LOAD CASES: (4)					
MEMB.	CHORDS			WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. MEMB. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS) MAX CSI (LC)
FR-TO		FROM	TO		
E-B	-389 / 0	0.0	0.0 0.11 (4)	7.81	
A-B	0 / 25	-77.4	-77.4 0.09 (1)	10.00	
B-C	-29 / 0	-77.4	-77.4 0.36 (1)	6.25	
E-D	0 / 0	-17.5	-17.5 0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

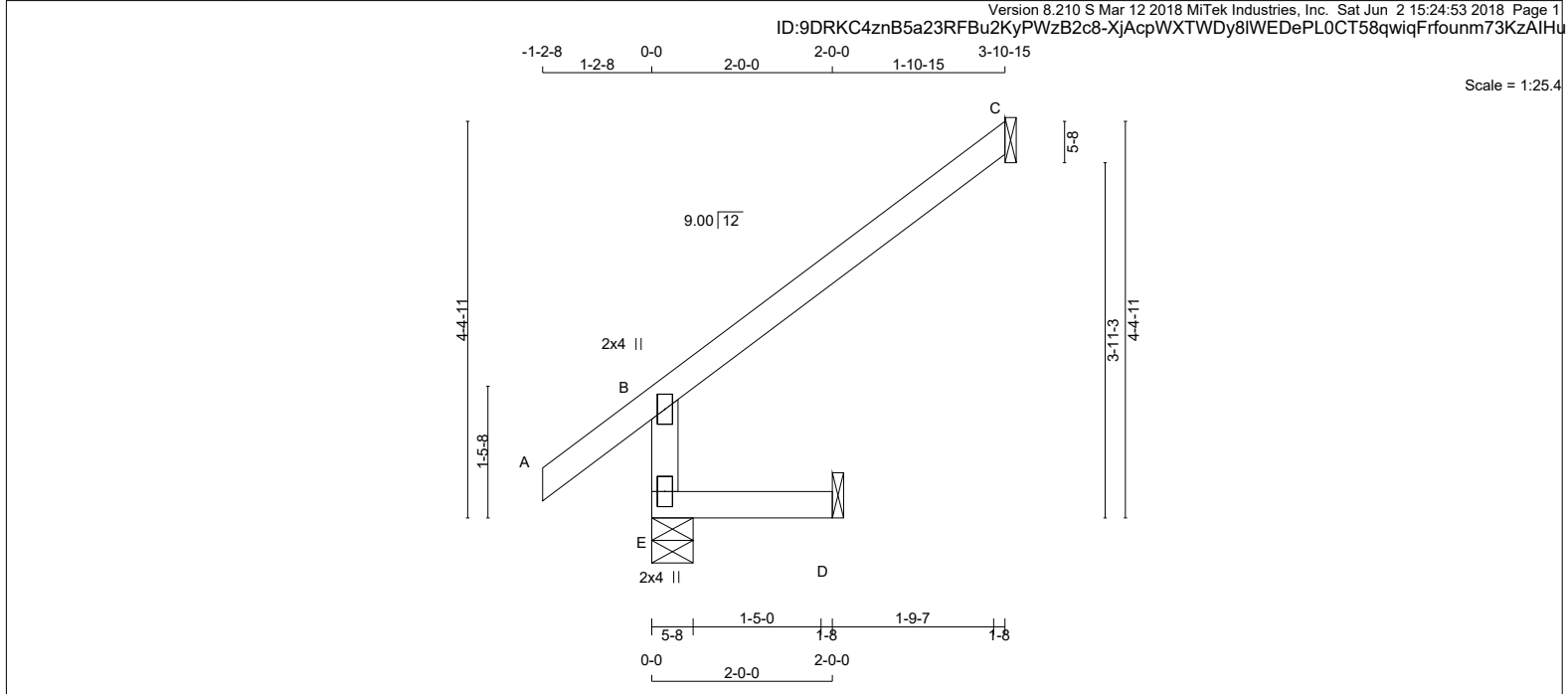
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





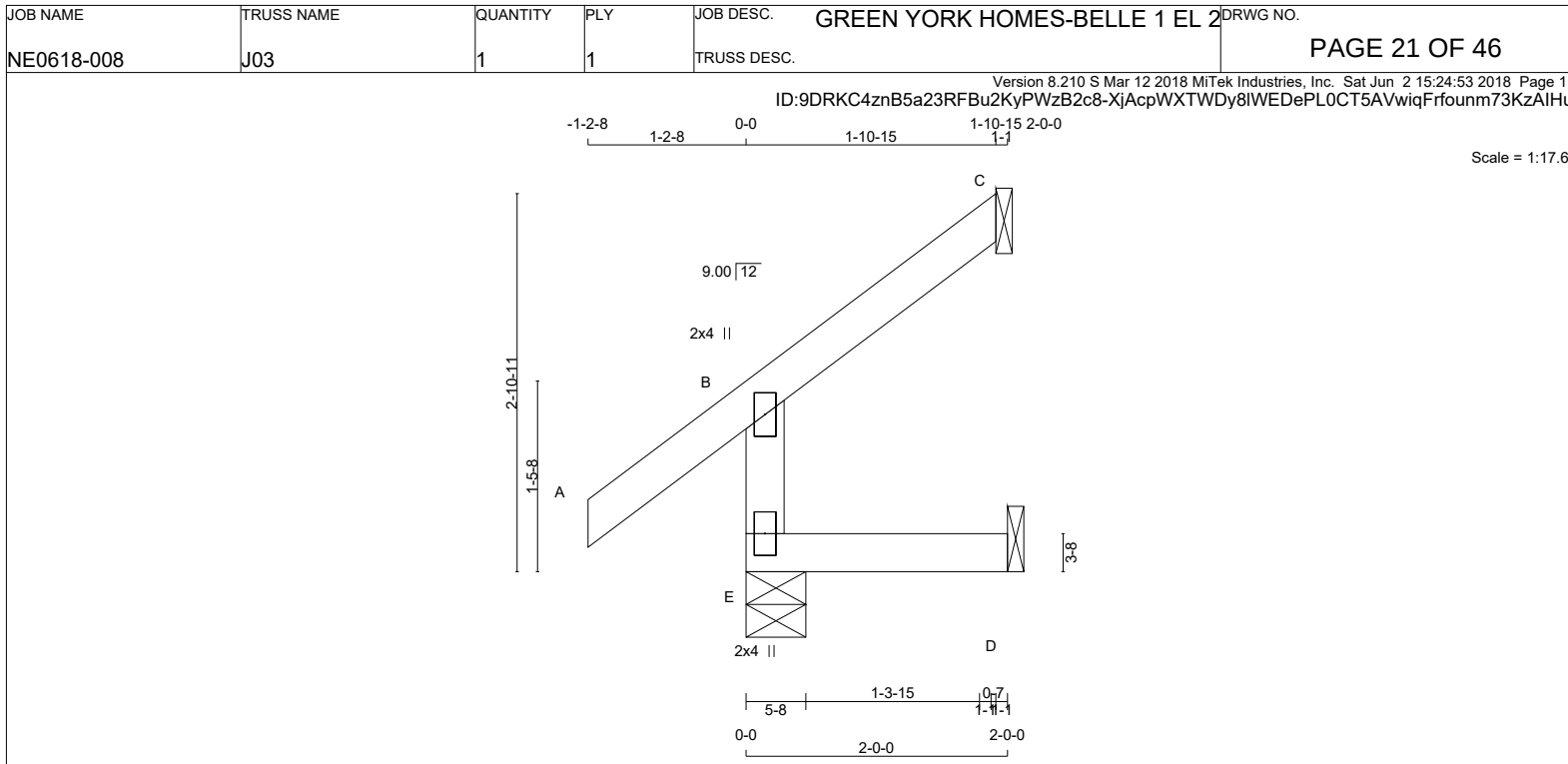
LUMBER N. L. G. A. RULES <table><tr><td>CHORDS</td><td>SIZE</td><td>DRY</td><td>LUMBER</td><td>DESCR.</td></tr><tr><td>E - B</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>A - C</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr><tr><td>E - D</td><td>2x4</td><td>DRY</td><td>No.2</td><td>SPF</td></tr></table> DRY: SEASONED LUMBER.					CHORDS	SIZE	DRY	LUMBER	DESCR.	E - B	2x4	DRY	No.2	SPF	A - C	2x4	DRY	No.2	SPF	E - D	2x4	DRY	No.2	SPF	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td>JT</td><td>FACTORED GROSS REACTION</td><td>MAXIMUM FACTORED GROSS REACTION</td><td>INPUT BRG</td><td>REQRD BRG</td></tr><tr><td>E</td><td>308</td><td>308</td><td>5-8</td><td>5-8</td></tr><tr><td>C</td><td>114</td><td>114</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>16</td><td>18</td><td>1-8</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><td>JT</td><td>1ST LCASE COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>E</td><td>212</td><td>174 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>38 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>77</td><td>68 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>9 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>13</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>13 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><td colspan="4">CHORDS</td><td colspan="4">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>FACTORED VERT. LOAD (PLF)</td><td>LC1 MAX. CSI (LC)</td><td>MAX. UNBRACED LENGTH</td><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>MAX. CSI (LC)</td></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>E- B</td><td>-289 / 0</td><td>0.0</td><td>0.0</td><td>0.01 (4)</td><td>7.81</td><td></td><td></td></tr><tr><td>A- B</td><td>0 / 30</td><td>-77.4</td><td>-77.4</td><td>0.09 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>B- C</td><td>-23 / 0</td><td>-77.4</td><td>-77.4</td><td>0.20 (1)</td><td>6.25</td><td></td><td></td></tr><tr><td>E- D</td><td>0 / 0</td><td>-17.5</td><td>-17.5</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td></tr></table>					JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	E	308	308	5-8	5-8	C	114	114	1-8	1-8	D	16	18	1-8	1-8	JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	212	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0	C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	FR-TO		FROM TO			FR-TO			E- B	-289 / 0	0.0	0.0	0.01 (4)	7.81			A- B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			B- C	-23 / 0	-77.4	-77.4	0.20 (1)	6.25			E- D	0 / 0	-17.5	-17.5	0.02 (4)	10.00			DESIGN CRITERIA SPECIFIED LOADS: <table><tr><td>TOP</td><td>CH.</td><td>LL</td><td>PSF</td></tr><tr><td></td><td></td><td>DL</td><td>= 23.3</td></tr><tr><td></td><td></td><td>DL</td><td>= 3.0</td></tr><tr><td>BOT</td><td>CH.</td><td>LL</td><td>PSF</td></tr><tr><td></td><td></td><td>DL</td><td>= 0.0</td></tr><tr><td></td><td></td><td>DL</td><td>= 7.0</td></tr><tr><td colspan="2">TOTAL LOAD</td><td>=</td><td>33.3</td></tr></table> SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00") CSI: TC=0.20/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES <table><tr><td>PLATE</td><td>GRIP(DRY)</td><td>SHEAR</td><td>SECTION</td></tr><tr><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td><td>(PLI)</td></tr><tr><td>MAX</td><td>MIN</td><td>MAX</td><td>MIN</td></tr><tr><td>MT20</td><td>618</td><td>354</td><td>1667</td></tr><tr><td></td><td>822</td><td>2284</td><td>1656</td></tr></table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.20 (B) (INPUT = 0.90) JSI METAL= 0.07 (B) (INPUT = 1.00)					TOP	CH.	LL	PSF			DL	= 23.3			DL	= 3.0	BOT	CH.	LL	PSF			DL	= 0.0			DL	= 7.0	TOTAL LOAD		=	33.3	PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)	(PLI)	(PLI)	MAX	MIN	MAX	MIN	MT20	618	354	1667		822	2284	1656
CHORDS	SIZE	DRY	LUMBER	DESCR.																																																																																																																																																																																										
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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
E	211	211	5-8	5-8
C	56	56	1-8	1-8
D	16	18	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	146	116 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED				WEBS		FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO				
E-B	-192 / 0	0.0	0.0	0.01 (4)	7.81					
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00					
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25					
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00					

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
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- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

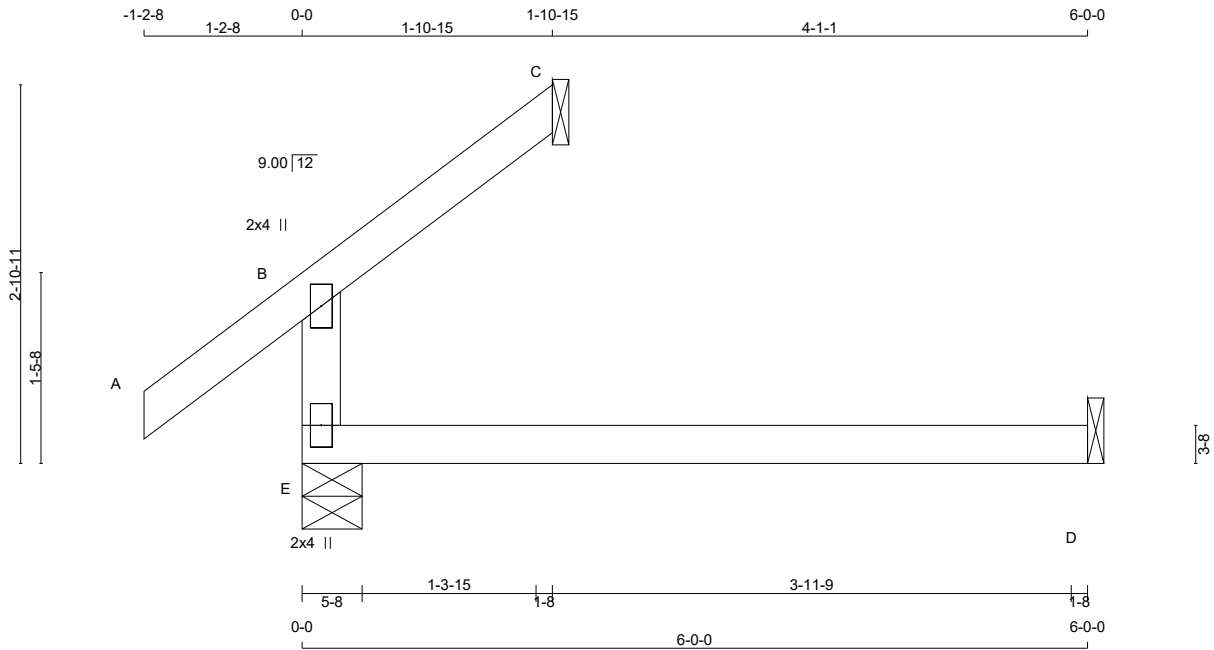
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



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TOTAL WEIGHT = 13 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	253	0	253	0	0	5-8	5-8	
C	56	0	56	0	0	1-8	1-8	
D	44	0	49	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND			
E	179	116 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0	
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO				
E-B	-192 / 0	0.0 0.0 0.10 (4)	7.81			
A-B	0 / 30	-77.4 -77.4 0.09 (1)	10.00			
B-C	-11 / 0	-77.4 -77.4 0.04 (1)	6.25			
E-D	0 / 0	-17.5 -17.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.03")

CSI: TC=0.10/1.00 (B-E:4) , BC=0.13/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

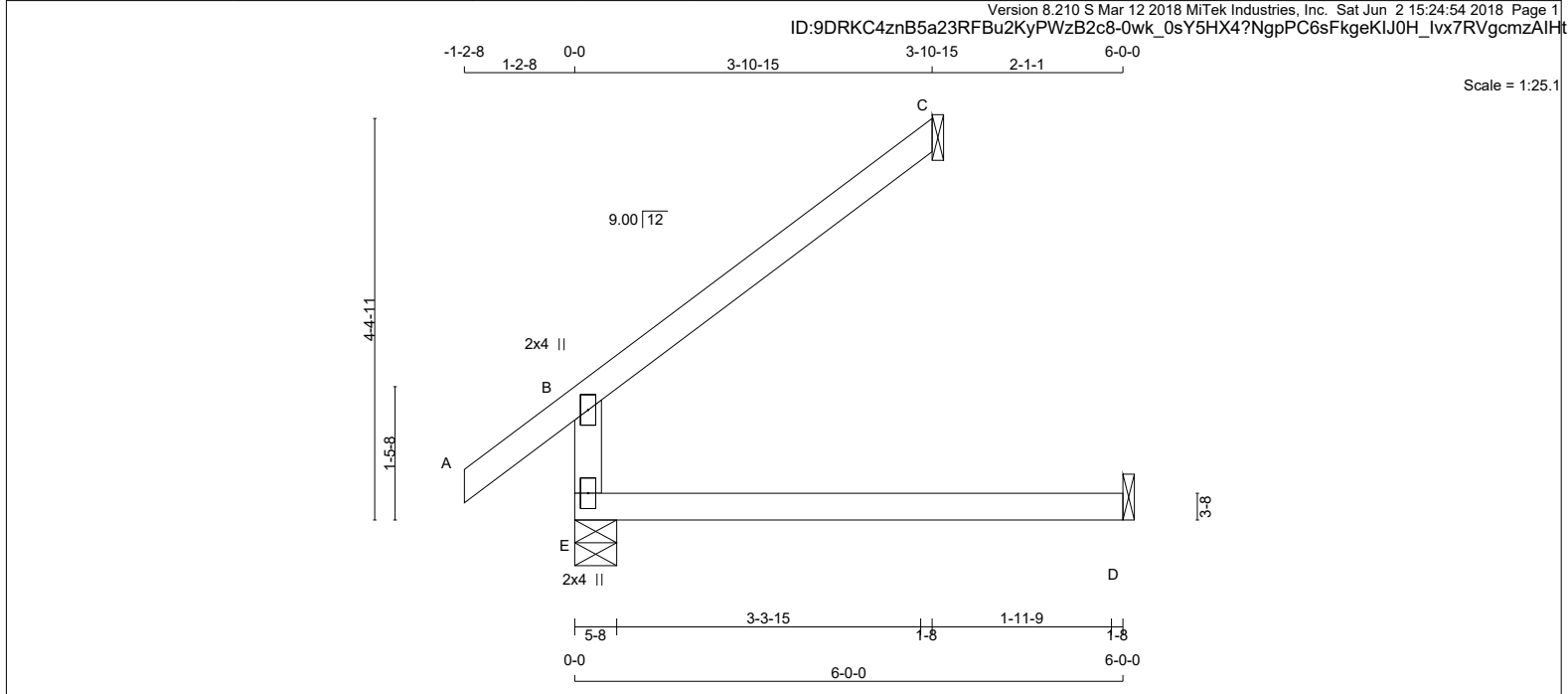
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 16 lb [M][F]

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	SPF	SPF	SPF
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION	FACTORED GROSS REACTION
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	350	0	350	0	0	5-8	5-8	5-8	5-8
C	114	0	114	0	0	1-8	1-8	1-8	1-8
D	44	0	49	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	245	174 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	FACTORED MAX. MOMENT (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. MOMENT (LC)	FACTORED MAX. MOMENT (LC)	FACTORED MAX. MOMENT (LC)
FR-TO		FROM	TO		FR-TO		FROM	TO	
E-B	-289 / 0	0.0	0.0	0.10 (4)	7.81				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00				
B-C	-23 / 0	-77.4	-77.4	0.15 (1)	6.25				
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

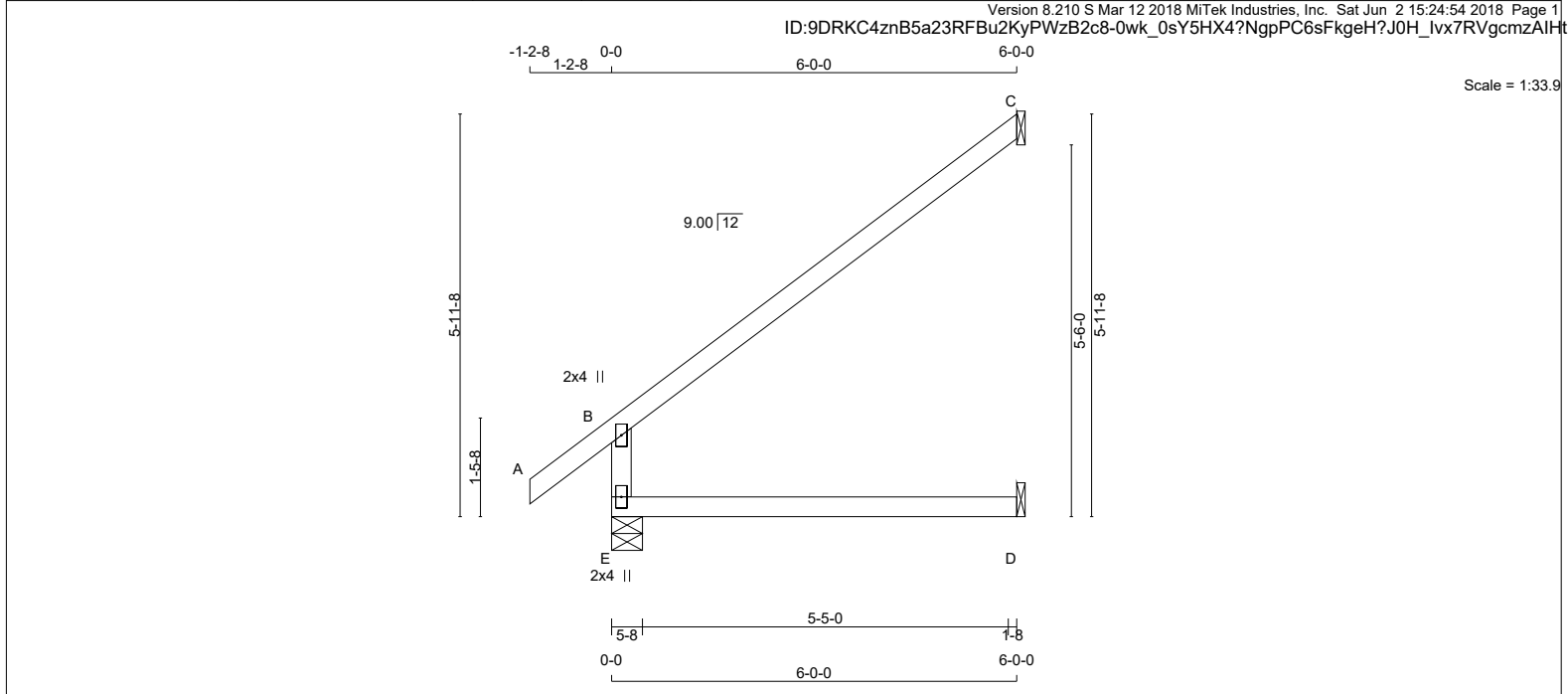
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 2 X 19 = 37 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	451	0	451	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	44	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND			
E	314	235 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0	
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0	
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
E-B	-390 / 0	0.0	0.0	0.10 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-35 / 0	-77.4	-77.4	0.37 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.37/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

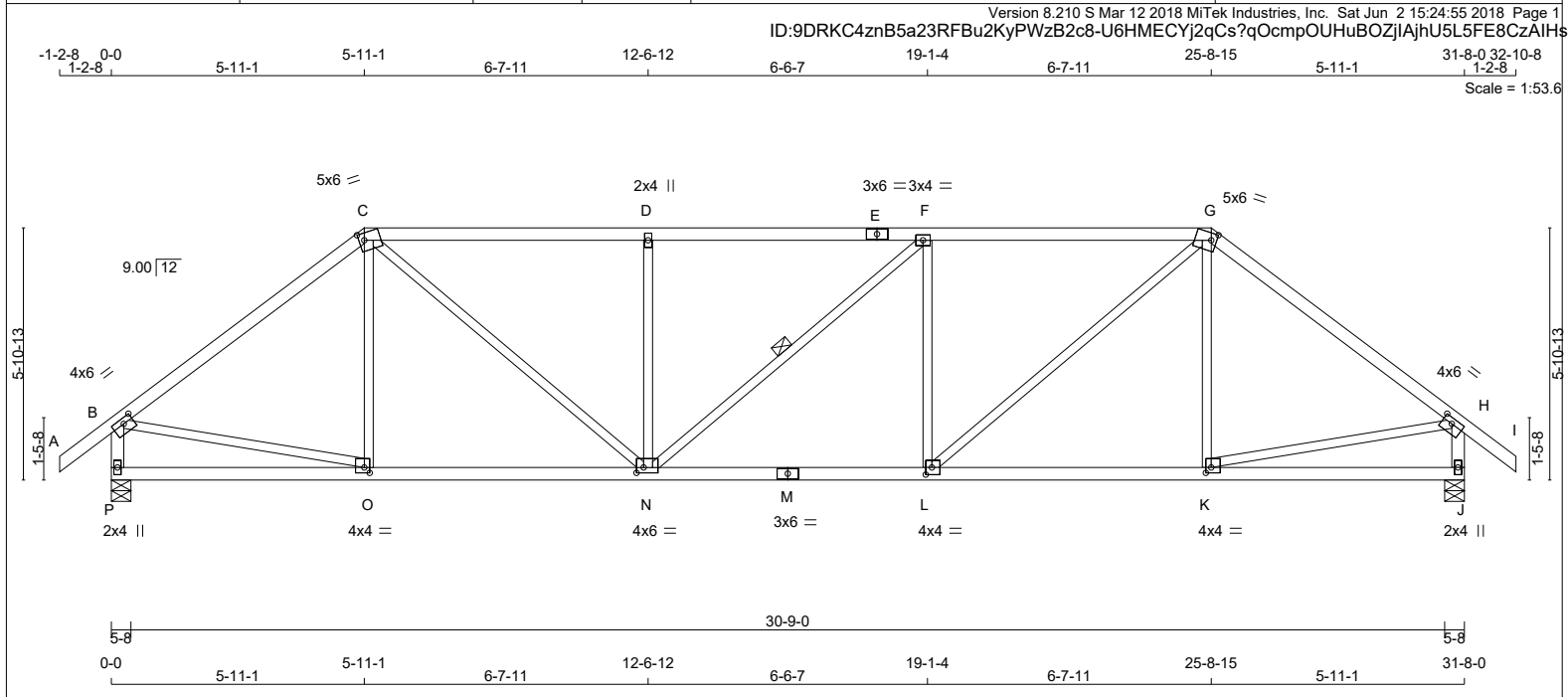
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 129 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0	1.50	2.00
O	BMWW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
P	1602	0	1602	0	5-8	5-8
J	1602	0	1602	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
J	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N. DBS = 20-0-0 . CBF = 0 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	O-C	-121 / 64	0.07 (1)
B-C	-1639 / 0	-77.4 -77.4	0.62 (1)	4.47	C-N	0 / 1031	0.23 (1)
C-D	-2096 / 0	-77.4 -77.4	0.63 (1)	4.04	N-D	-552 / 0	0.30 (1)
D-E	-2096 / 0	-77.4 -77.4	0.63 (1)	4.03	N-F	-1 / 0	0.00 (1)
E-F	-2096 / 0	-77.4 -77.4	0.63 (1)	4.03	L-F	-553 / 0	0.30 (1)
F-G	-2097 / 0	-77.4 -77.4	0.63 (1)	4.03	L-G	0 / 1033	0.23 (1)
G-H	-1639 / 0	-77.4 -77.4	0.62 (1)	4.47	K-G	-122 / 64	0.07 (1)
H-I	0 / 30	-77.4 -77.4	0.09 (1)	10.00	B-O	0 / 1332	0.30 (1)
P-B	-1561 / 0	0.0 0.0	0.16 (1)	6.59	K-H	0 / 1332	0.30 (1)
J-H	-1561 / 0	0.0 0.0	0.16 (1)	6.59			
P-O	0 / 0	-17.5 -17.5	0.16 (4)	10.00			
O-N	0 / 1307	-17.5 -17.5	0.30 (1)	10.00			
N-M	0 / 2097	-17.5 -17.5	0.41 (1)	10.00			
M-L	0 / 2097	-17.5 -17.5	0.41 (1)	10.00			
L-K	0 / 1307	-17.5 -17.5	0.30 (1)	10.00			
K-J	0 / 0	-17.5 -17.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.63/1.00 (F-G:1), BC=0.41/1.00 (L-N:1), WB=0.30/1.00 (B-O:1), SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

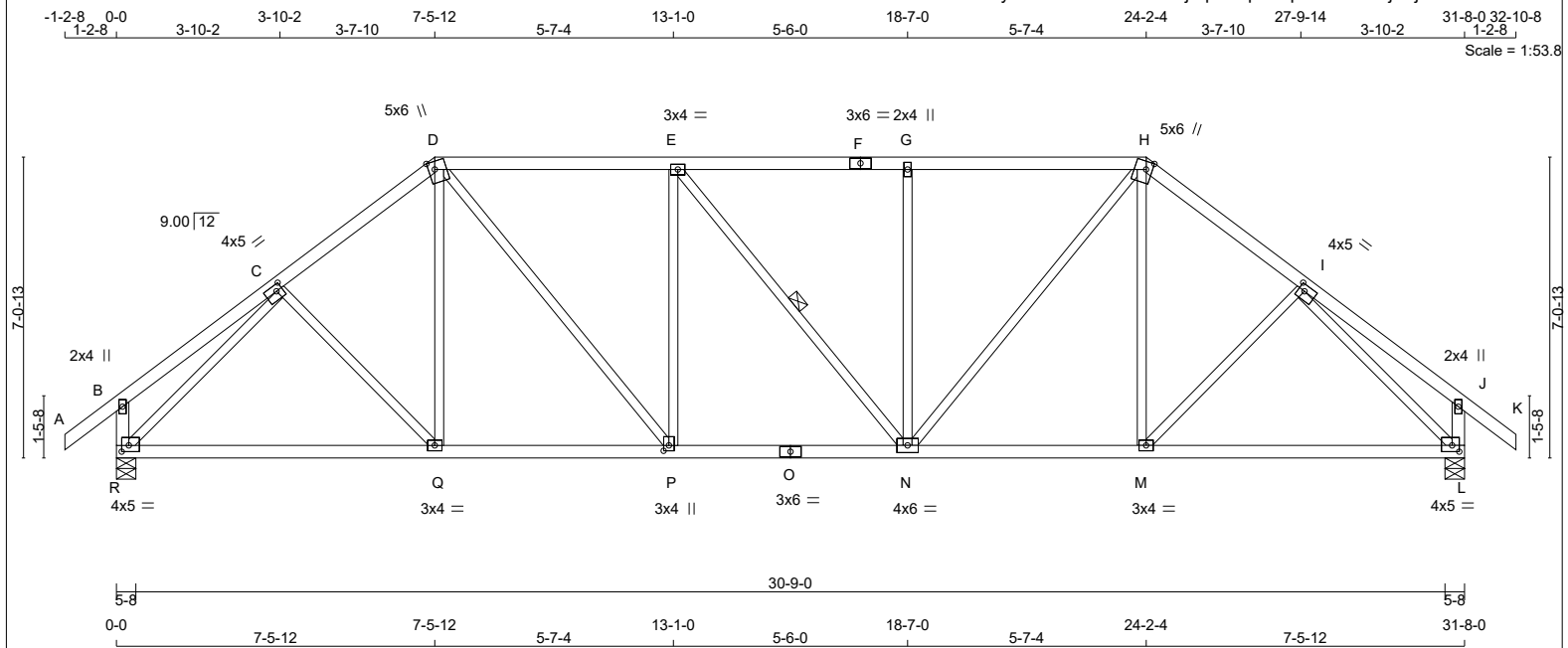
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.62 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	5.0	1.75	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMWW-t	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.75
I	TMWW-t	MT20	4.0	5.0	1.75	1.75
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	5.0	1.75	2.00
M	BMWW-t	MT20	3.0	4.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	3.0	4.0	1.50	1.50
Q	BMWW-t	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	5.0	1.75	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
R	1602	0	1602	0	5-8	5-8
L	1602	0	1602	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	
R	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
L	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N. DBS = 20-0-0. CBF = 0 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB. MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		
A-B	0 / 30	-77.4 -77.4	0.09 (1)	C-Q	0 / 55	0.02 (4)
B-C	0 / 19	-77.4 -77.4	0.16 (1)	Q-D	0 / 120	0.04 (4)
C-D	-1624 / 0	-77.4 -77.4	0.20 (1)	D-P	0 / 716	0.16 (1)
D-E	-1741 / 0	-77.4 -77.4	0.40 (1)	P-E	-465 / 0	0.40 (1)
E-F	-1739 / 0	-77.4 -77.4	0.40 (1)	E-N	-2 / 0	0.00 (1)
F-G	-1739 / 0	-77.4 -77.4	0.40 (1)	N-G	-464 / 0	0.40 (1)
G-H	-1739 / 0	-77.4 -77.4	0.40 (1)	H-N	0 / 714	0.16 (1)
H-I	-1624 / 0	-77.4 -77.4	0.20 (1)	M-H	0 / 121	0.04 (4)
I-J	0 / 19	-77.4 -77.4	0.16 (1)	M-I	0 / 55	0.02 (4)
J-K	0 / 30	-77.4 -77.4	0.09 (1)	R-C	-1839 / 0	0.87 (1)
R-B	-213 / 0	0.0 0.0	0.02 (1)	I-L	-1839 / 0	0.87 (1)
L-J	-213 / 0	0.0 0.0	0.02 (1)			
R-Q	0 / 1266	-17.5 -17.5	0.32 (1)			
Q-P	0 / 1283	-17.5 -17.5	0.32 (1)			
P-O	0 / 1741	-17.5 -17.5	0.33 (1)			
O-N	0 / 1741	-17.5 -17.5	0.33 (1)			
N-M	0 / 1284	-17.5 -17.5	0.33 (1)			
M-L	0 / 1266	-17.5 -17.5	0.32 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.40/1.00 (D-E:1), BC=0.33/1.00 (N-P:1), WB=0.87/1.00 (L-L:1), SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

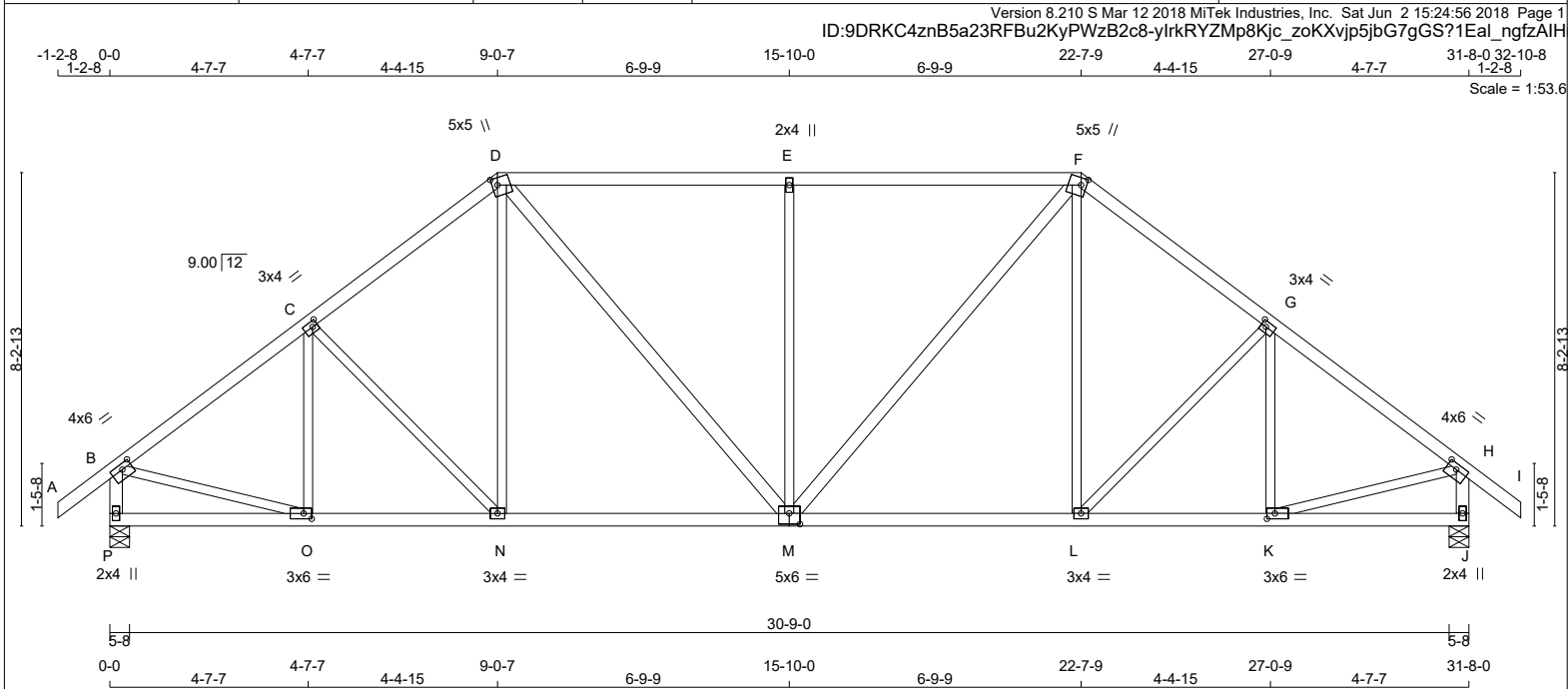
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.54 (I) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 149 lb

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.50 2.75
C	CTMWW-t	MT20	3.0	4.0	1.50 1.50
D	TTVW+m	MT20	5.0	5.0	2.00 1.50
E	TMW+m	MT20	2.0	4.0	
F	TTVW+m	MT20	5.0	5.0	2.00 1.50
G	CTMWW-t	MT20	3.0	4.0	1.50 1.50
H	TMVW-t	MT20	4.0	6.0	1.50 2.75
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	3.0	6.0	1.50 2.25
L	BMWW-t	MT20	3.0	4.0	
M	BSWWW-I	MT20	5.0	6.0	3.00 3.00
N	BMWW-t	MT20	3.0	4.0	
O	BMWW-t	MT20	3.0	6.0	1.50 2.25
P	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1602	0	1602	0	0	5-8	5-8
J	1602	0	1602	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
J	1122	798 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0/30	-77.4	-77.4	0.09 (1)	10.00	Q-C	-249 / 0	0.09 (1)
B-C	-1644 / 0	-77.4	-77.4	0.24 (1)	4.95	C-N	-152 / 0	0.11 (1)
C-D	-1563 / 0	-77.4	-77.4	0.23 (1)	5.05	N-D	0 / 210	0.05 (1)
D-E	-1560 / 0	-77.4	-77.4	0.51 (1)	4.67	D-M	0 / 506	0.08 (1)
E-F	-1560 / 0	-77.4	-77.4	0.51 (1)	4.67	M-E	-646 / 0	0.86 (1)
F-G	-1563 / 0	-77.4	-77.4	0.23 (1)	5.05	M-F	0 / 506	0.08 (1)
G-H	-1644 / 0	-77.4	-77.4	0.24 (1)	4.95	L-F	0 / 210	0.05 (1)
H-I	0/30	-77.4	-77.4	0.09 (1)	10.00	L-G	-152 / 0	0.11 (1)
P-B	-1566 / 0	0.0	0.0	0.16 (1)	6.59	K-G	-249 / 0	0.09 (1)
J-H	-1566 / 0	0.0	0.0	0.16 (1)	6.59	B-O	0 / 1374	0.31 (1)
						K- H	0 / 1374	0.31 (1)
P-O	0/0	-17.5	-17.5	0.08 (4)	10.00			
O-N	0/1334	-17.5	-17.5	0.28 (1)	10.00			
N-M	0/1231	-17.5	-17.5	0.29 (1)	10.00			
M-L	0/1231	-17.5	-17.5	0.29 (1)	10.00			
L-K	0/1334	-17.5	-17.5	0.28 (1)	10.00			
K-J	0/0	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
TOTAL LOAD			=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.51/1.00 (D-E:1) , BC=0.29/1.00 (L-M:1) ,
WB=0.86/1.00 (E-M:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

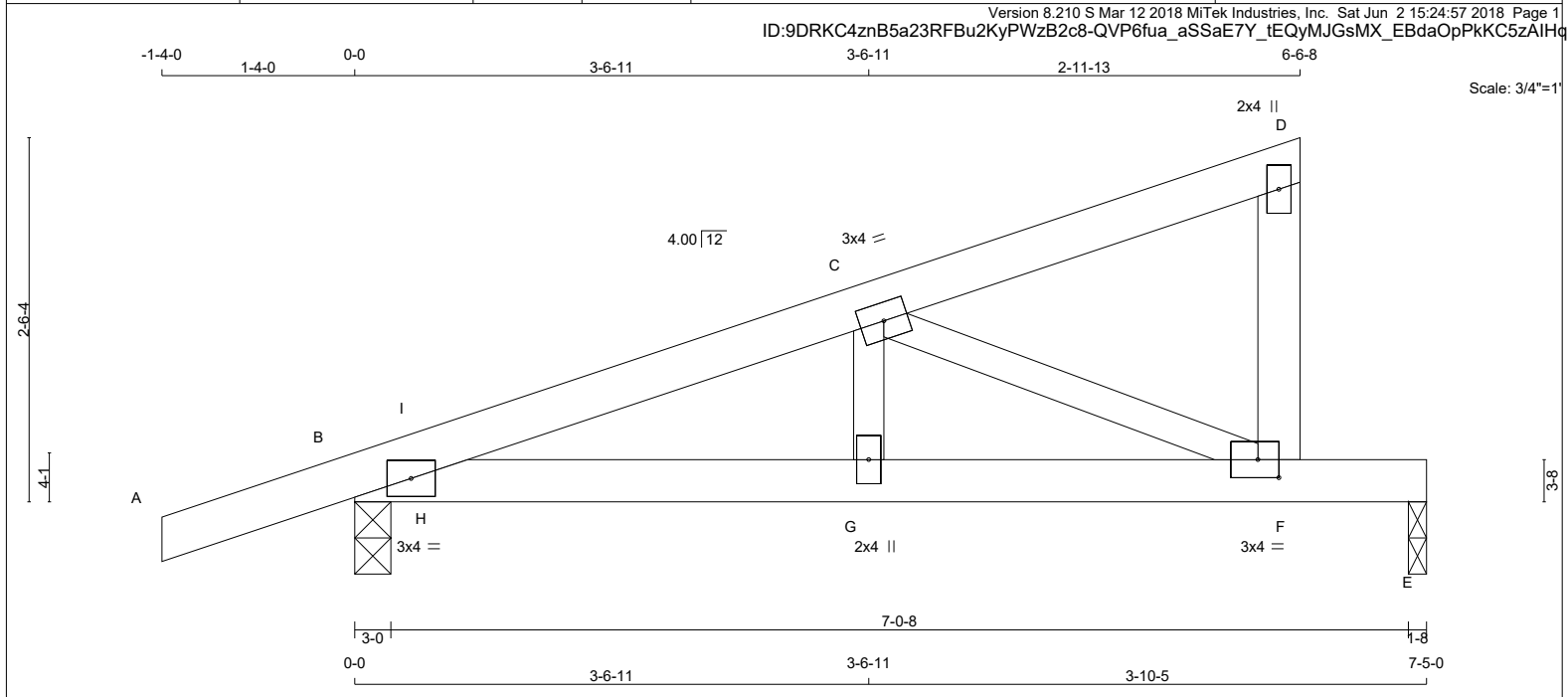
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 5 X 23 = 116 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMWW-t	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
F	BMVW-t	MT20	3.0	4.0	1.50	1.75
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	454	0	454	0	3-0	3-0
E	288	0	288	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	317	235 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
E	204	134 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	FACTORED CSI (LC)	
FR-TO		FROM TO	TO	LENGTH	FR-TO				
A-B	0 / 16	-77.4	-77.4	0.10 (1)	10.00	G-C	0 / 197	0.04 (1)	
B-I	-667 / 0	-77.4	-77.4	0.01 (1)	6.25	C-F	-676 / 0	0.13 (1)	
I-C	-654 / 0	-77.4	-77.4	0.07 (1)	6.25	H-I	-95 / 8	0.00 (1)	
C-D	-6 / 0	-77.4	-77.4	0.07 (1)	10.00				
F-D	-95 / 0	0.0	0.0	0.01 (1)	7.81				
B-H	0 / 625	-17.5	-17.5	0.17 (1)	10.00				
H-G	0 / 625	-17.5	-17.5	0.19 (1)	10.00				
G-F	0 / 625	-17.5	-17.5	0.43 (1)	10.00				
F-E	0 / 0	-17.5	-17.5	0.33 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.10/1.00 (A-B:1), BC=0.43/1.00 (F-G:1), WB=0.13/1.00 (C-F:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

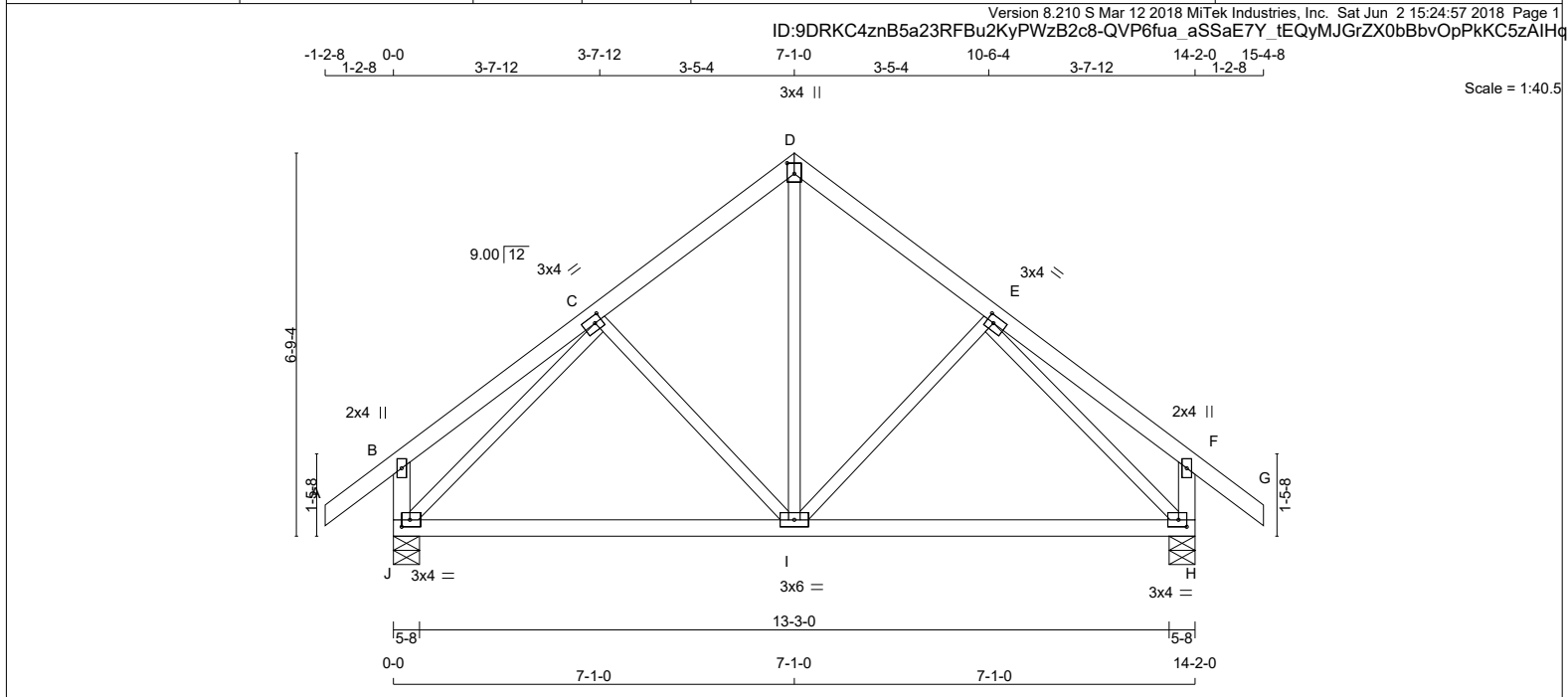
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 4 X 63 = 253 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	4.0	1.50	1.50
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0	1.50	1.75
I	BMWW1-t	MT20	3.0	6.0		
J	BMVW1-t	MT20	3.0	4.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
JT	772	0	772	0
H	772	0	772	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	540	390 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
H	540	390 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	I-D	0 / 365	0.08 (1)	
B-C	0 / 19	-77.4	-77.4 0.15 (1)	I-E	-145 / 4	0.06 (1)	
C-D	-500 / 0	-77.4	-77.4 0.12 (1)	C-I	-145 / 4	0.06 (1)	
D-E	-500 / 0	-77.4	-77.4 0.12 (1)	J-C	-711 / 0	0.30 (1)	
E-F	0 / 19	-77.4	-77.4 0.15 (1)	E-H	-711 / 0	0.30 (1)	
F-G	0 / 30	-77.4	-77.4 0.09 (1)				
J-B	-205 / 0	0.0	0.0 0.02 (1)				
H-F	-205 / 0	0.0	0.0 0.02 (1)				
J-I	0 / 485	-17.5	-17.5 0.28 (4)				
I-H	0 / 485	-17.5	-17.5 0.28 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.15/1.00 (E-F:1), BC=0.28/1.00 (H-I:4), WB=0.30/1.00 (E-H:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

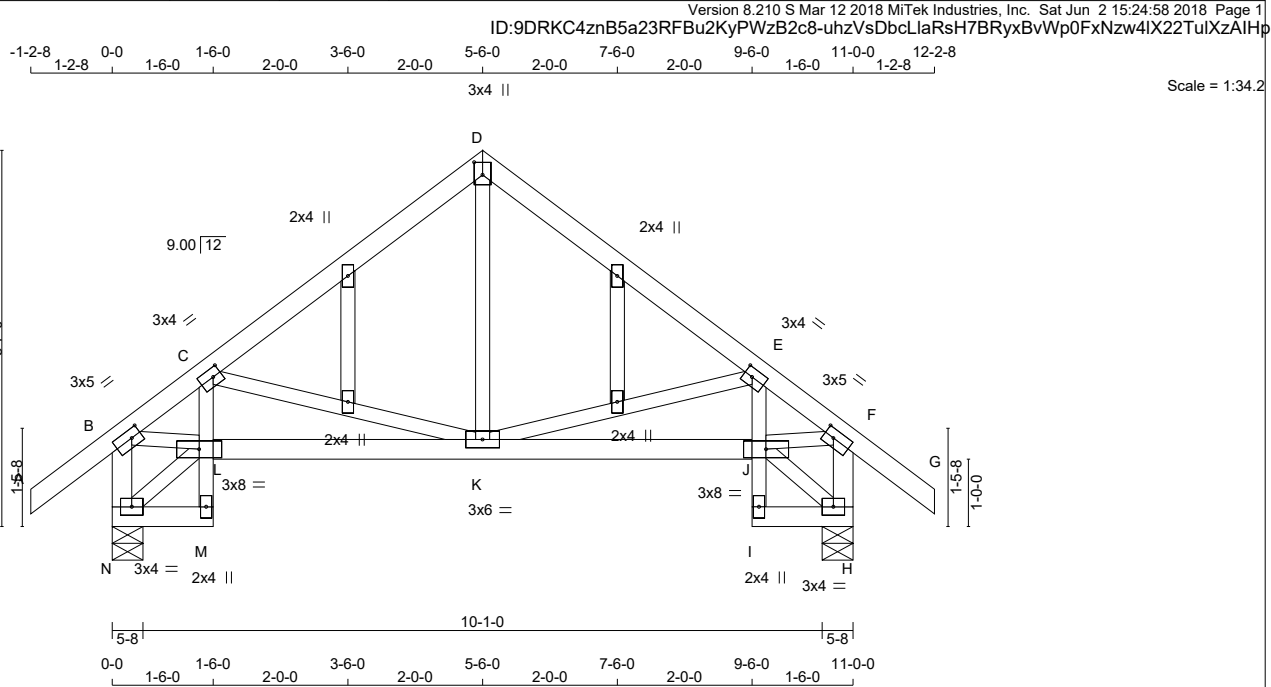
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 54 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0.0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	3.0	5.0	1.50	1.75
H	BMVW1-t	MT20	3.0	4.0		
I	BMV+p	MT20	2.0	4.0		
J	BVMWW-I	MT20	3.0	8.0		
K	BMVWW-t	MT20	3.0	6.0		
L	BVMWW-I	MT20	3.0	8.0		
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		
O, P, Q, R						
O	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	622	0	622	0	5-8	5-8
H	622	0	622	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	434	317 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	434	317 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-D	0 / 225	0.05 (1)	
B-C	-747 / 0	-77.4	-77.4	0.11 (1)	6.25	K-E	-301 / 0	0.09 (1)	
C-D	-427 / 0	-77.4	-77.4	0.16 (1)	6.25	C-K	-301 / 0	0.09 (1)	
D-E	-427 / 0	-77.4	-77.4	0.16 (1)	6.25	B-L	0 / 609	0.14 (1)	
E-F	-747 / 0	-77.4	-77.4	0.11 (1)	6.25	J-F	0 / 609	0.14 (1)	
F-G	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-L	-13 / 0	0.00 (1)	
N-B	-602 / 0	0.0	0.0	0.06 (1)	7.81	J-H	-13 / 0	0.00 (1)	
H-F	-602 / 0	0.0	0.0	0.06 (1)	7.81				
N-M	0 / 10	-17.5	-17.5	0.01 (4)	10.00				
M-L	0 / 14	0.0	0.0	0.03 (1)	10.00				
L-C	0 / 42	0.0	0.0	0.04 (1)	10.00				
L-K	0 / 619	-17.5	-17.5	0.15 (1)	10.00				
K-J	0 / 619	-17.5	-17.5	0.15 (1)	10.00				
I-J	0 / 14	0.0	0.0	0.03 (1)	10.00				
J-E	0 / 42	0.0	0.0	0.04 (1)	10.00				
I-H	0 / 10	-17.5	-17.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.16/1.00 (C-D:1), BC=0.15/1.00 (J-K:1), WB=0.14/1.00 (F-J:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

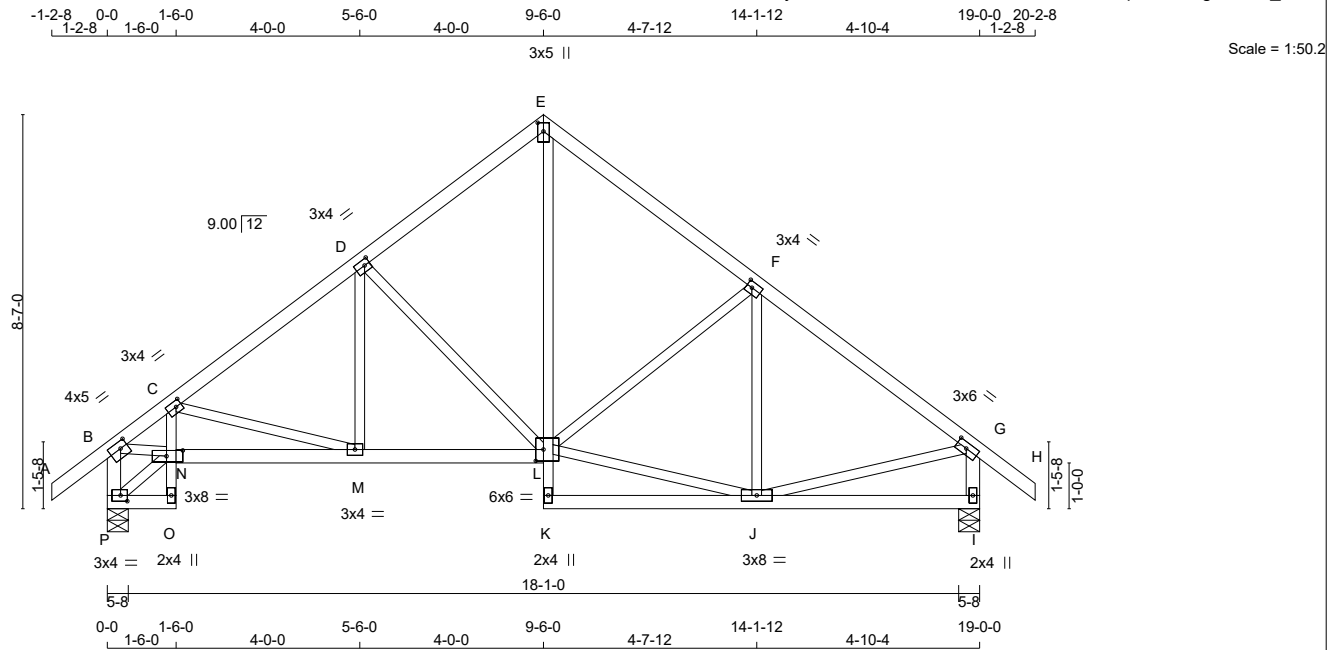
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 92 lb [M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - E	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	5.0	1.75 2.00
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TMVW-t	MT20	3.0	4.0	1.50 1.50
E	TTV+p	MT20	3.0	5.0	2.25 1.50
F	TMVW-t	MT20	3.0	4.0	1.50 1.50
G	TMVW-t	MT20	3.0	6.0	1.50 2.75
I	BMV1+p	MT20	2.0	4.0	
J	BMVWW-t	MT20	3.0	8.0	
K	BMV+p	MT20	2.0	4.0	
L	BVMVWW-I	MT20	6.0	6.0	3.00 2.00
M	BMVW-t	MT20	3.0	4.0	
N	BVMVWW-I	MT20	3.0	8.0	1.50 4.25
O	BMV+p	MT20	2.0	4.0	
P	BMVW1-t	MT20	3.0	4.0	1.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
I	998	0	998	0	5-8	5-8	5-8
P	1005	0	1005	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	698	501 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
P	703	505 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	C-M	-209 / 0	0.06 (1)
B-C	-1312 / 0	-77.4	-77.4 0.09 (1)	5.57	M-D	0 / 118	0.04 (4)
C-D	-1055 / 0	-77.4	-77.4 0.16 (1)	5.95	D-L	-397 / 0	0.24 (1)
D-E	-747 / 0	-77.4	-77.4 0.15 (1)	6.25	L-J	0 / 743	0.17 (1)
E-F	-763 / 0	-77.4	-77.4 0.23 (1)	6.25	L-F	-185 / 0	0.11 (1)
F-G	-888 / 0	-77.4	-77.4 0.23 (1)	6.24	J-F	-231 / 16	0.09 (1)
G-H	0 / 30	-77.4	-77.4 0.09 (1)	10.00	B-N	0 / 1040	0.23 (1)
P-B	-979 / 0	0.0	0.0 0.10 (1)	7.81	J-G	0 / 751	0.17 (1)
I-G	-964 / 0	0.0	0.0 0.10 (1)	7.81	P-N	-22 / 0	0.00 (1)
P-O	0 / 17	-17.5	-17.5 0.01 (4)	10.00			
O-N	0 / 14	0.0	0.0 0.05 (1)	10.00			
N-C	0 / 41	0.0	0.0 0.06 (1)	10.00			
N-M	0 / 1057	-17.5	-17.5 0.21 (1)	10.00			
M-L	0 / 856	-17.5	-17.5 0.18 (1)	10.00			
K-L	0 / 33	0.0	0.0 0.02 (1)	10.00			
L-E	0 / 623	0.0	0.0 0.16 (1)	10.00			
K-J	0 / 6	-17.5	-17.5 0.11 (4)	10.00			
J-I	0 / 0	-17.5	-17.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.06")

CSI: TC=0.23/1.00 (F-G:1), BC=0.21/1.00 (M-N:1), WB=0.24/1.00 (D-L:1), SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

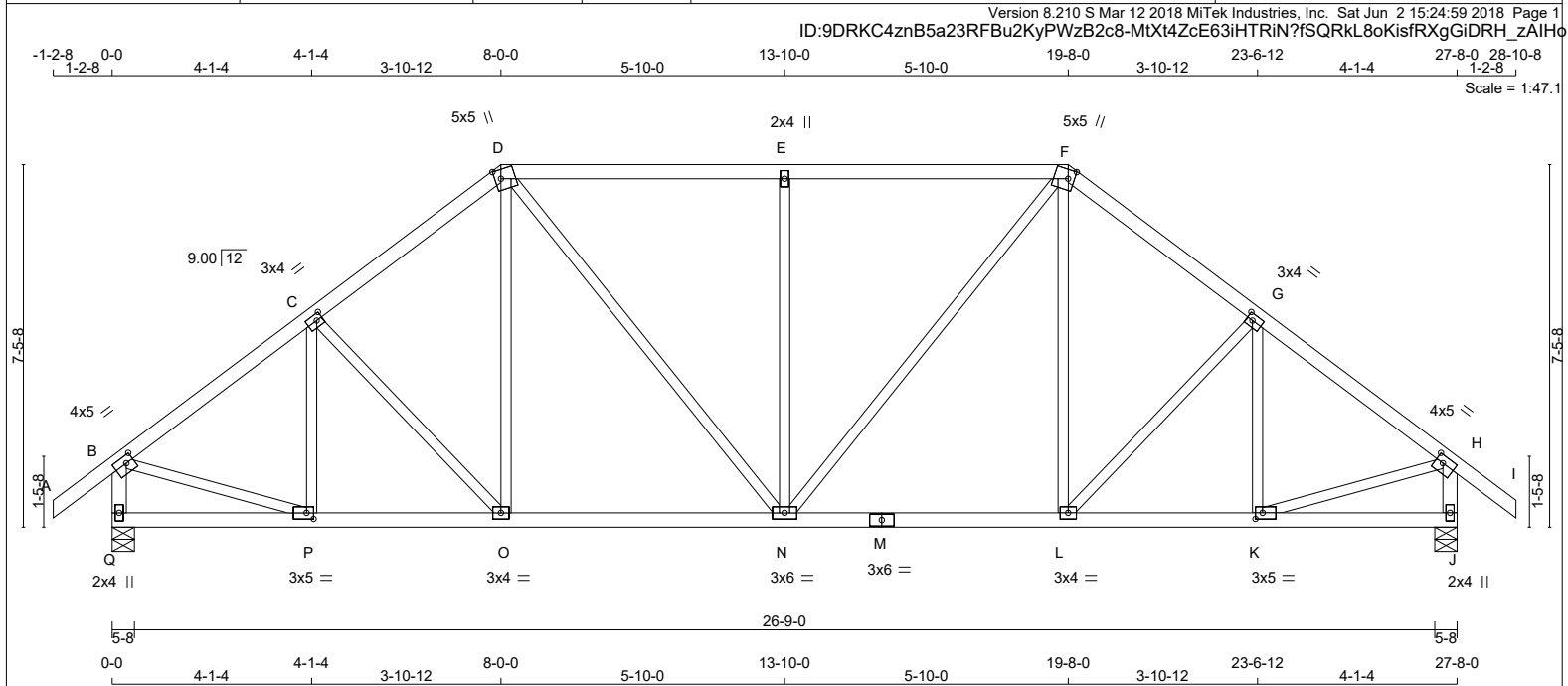
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





										TOTAL WEIGHT = 127 lb							
LUMBER N. L. G. A. RULES															[M][F]		
CHORDS		SIZE		LUMBER	DESCR.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA	
						BEARINGS											
A - D		2x4	DRY	No.2	SPF	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		SPECIFIED LOADS:			
D - F		2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	TOP CH. LL = 23.3 PSF			
F - I		2x4	DRY	No.2	SPF	Q	1412	0	1412	0	0	5-8	5-8	DL = 3.0 PSF			
Q - B		2x4	DRY	No.2	SPF	J	1412	0	1412	0	0	5-8	5-8	BOT CH. LL = 0.0 PSF			
J - H		2x4	DRY	No.2	SPF											DL = 7.0 PSF	
Q - M		2x4	DRY	No.2	SPF											TOTAL LOAD = 33.3 PSF	
M - J		2x4	DRY	No.2	SPF												
						UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C	
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF	1ST LCASE		MAX./MIN. COMPONENT REACTIONS									
						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
						Q	989	705 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0				
						J	989	705 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0				
DRY: SEASONED LUMBER.														LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			

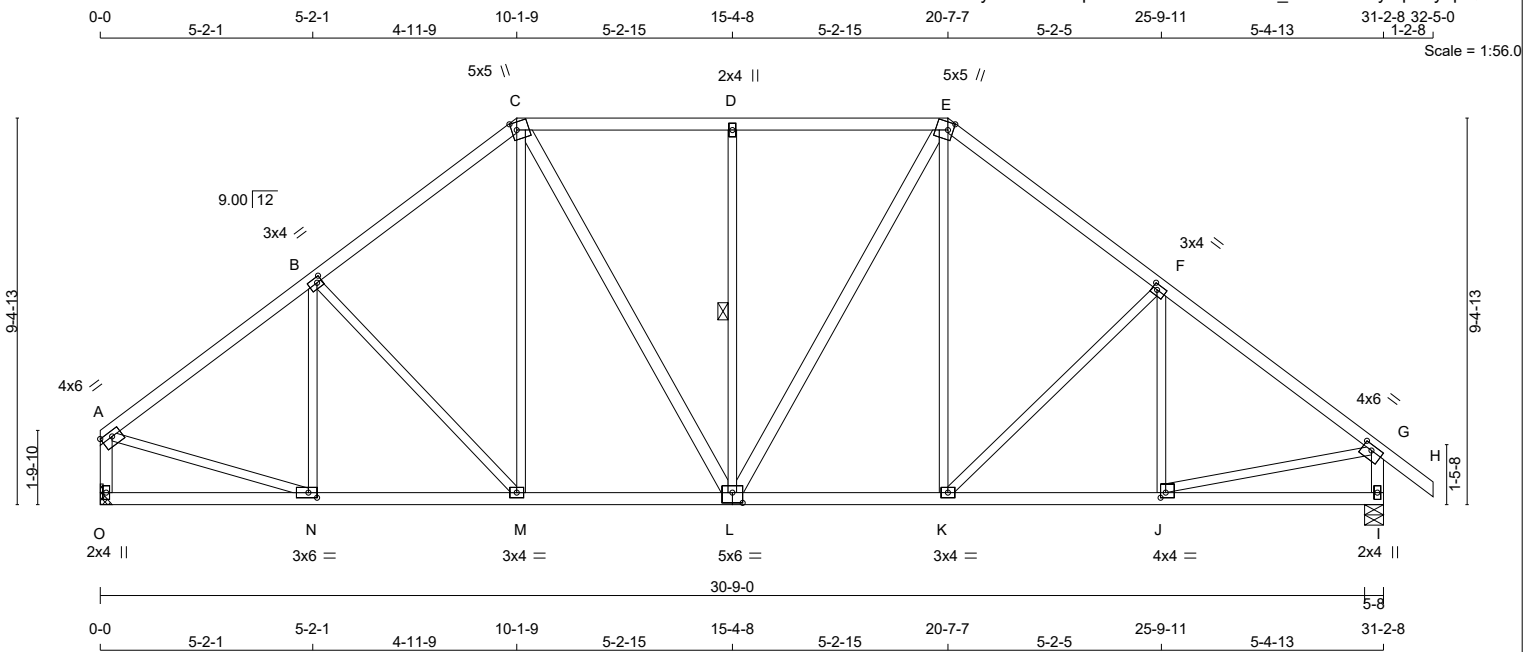
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.75	2.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.25	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	5.0	1.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	3.0	5.0	1.50	1.75
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	3.0	6.0		
O	BMWW-t	MT20	3.0	4.0		
P	BMWW-t	MT20	3.0	5.0	1.50	1.75
Q	BMV1+p	MT20	2.0	4.0		

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	P-C	-242 / 0	0.07 (1)
B-C	-1388 / 0	-77.4	-77.4 0.18 (1)	5.35	C-O	-116 / 0	0.07 (1)
C-D	-1334 / 0	-77.4	-77.4 0.17 (1)	5.44	O-D	0 / 174	0.04 (4)
D-E	-1317 / 0	-77.4	-77.4 0.36 (1)	5.21	D-N	0 / 423	0.10 (1)
E-F	-1317 / 0	-77.4	-77.4 0.36 (1)	5.21	N-E	-552 / 0	0.55 (1)
F-G	-1334 / 0	-77.4	-77.4 0.17 (1)	5.44	N-F	0 / 423	0.10 (1)
G-H	-1388 / 0	-77.4	-77.4 0.18 (1)	5.35	L-F	0 / 174	0.04 (4)
H-I	0 / 30	-77.4	-77.4 0.09 (1)	10.00	L-G	-116 / 0	0.07 (1)
Q-B	-1380 / 0	0.0	0.0 0.14 (1)	6.92	K-G	-242 / 0	0.07 (1)
J-H	-1380 / 0	0.0	0.0 0.14 (1)	6.92	B-P	0 / 1169	0.26 (1)
					K-H	0 / 1169	0.26 (1)
Q-P	0 / 0	-17.5	-17.5 0.06 (4)	10.00			
P-O	0 / 1127	-17.5	-17.5 0.23 (1)	10.00			
O-N	0 / 1050	-17.5	-17.5 0.23 (1)	10.00			
N-M	0 / 1050	-17.5	-17.5 0.23 (1)	10.00			
M-L	0 / 1050	-17.5	-17.5 0.23 (1)	10.00			
L-K	0 / 1127	-17.5	-17.5 0.23 (1)	10.00			
K-J	0 / 0	-17.5	-17.5 0.06 (4)	10.00			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 2 X 154 = 309 lb
[M][F]

LUMBER					DESCR.	
N. L. G. A. RULES						
CHORDS	SIZE		LUMBER		SPF	
A - C	2x4	DRY	No.2		SPF	
C - E	2x4	DRY	No.2		SPF	
E - H	2x4	DRY	No.2		SPF	
O - A	2x4	DRY	No.2		SPF	
I - G	2x4	DRY	No.2		SPF	
O - L	2x4	DRY	No.2		SPF	
L - I	2x4	DRY	No.2		SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2		SPF	
C - L	2x4	DRY	No.2		SPF	
L - E	2x4	DRY	No.2		SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	Edge
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TTWW+m	MT20	5.0	5.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0	1.50	1.50
K	BMWW-t	MT20	3.0	4.0		
L	BSWWW-I	MT20	5.0	6.0	3.00	3.00
M	BMWW-t	MT20	3.0	4.0		
N	BMWW-t	MT20	3.0	6.0	1.50	2.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1480	0	1480	0	0	MECHANICAL	
I	1580	0	1580	0	0	5-8	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT O TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1039	727 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
I	1107	787 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-L. DBS = 20-0-0 . CBF = 62 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1528 / 0	-77.4 -77.4	0.30 (1)	5.03	N-B	-258 / 8	0.13 (1)
B-C	-1426 / 0	-77.4 -77.4	0.29 (1)	5.17	B-M	-187 / 0	0.20 (1)
C-D	-1299 / 0	-77.4 -77.4	0.29 (1)	5.34	M-C	0 / 224	0.05 (1)
D-E	-1299 / 0	-77.4 -77.4	0.29 (1)	5.34	C-L	0 / 362	0.06 (1)
E-F	-1458 / 0	-77.4 -77.4	0.31 (1)	5.10	L-D	-494 / 0	0.29 (1)
F-G	-1639 / 0	-77.4 -77.4	0.33 (1)	4.85	L-E	0 / 312	0.05 (1)
G-H	0 / 30	-77.4 -77.4	0.09 (1)	10.00	K-E	0 / 280	0.06 (1)
O-A	-1442 / 0	0.0 0.0	0.16 (1)	6.81	K-F	-271 / 0	0.29 (1)
I-G	-1541 / 0	0.0 0.0	0.16 (1)	6.63	J-F	-177 / 36	0.08 (1)
O-N	0 / 0	-17.5 -17.5	0.11 (4)	10.00	A-N	0 / 1294	0.29 (1)
N-M	0 / 1244	-17.5 -17.5	0.25 (1)	10.00	J-G	0 / 1363	0.31 (1)
M-L	0 / 1118	-17.5 -17.5	0.23 (1)	10.00			
L-K	0 / 1144	-17.5 -17.5	0.23 (1)	10.00			
K-J	0 / 1334	-17.5 -17.5	0.27 (1)	10.00			
J-I	0 / 0	-17.5 -17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.33/1.00 (F-G:1), BC=0.27/1.00 (J-K:1), WB=0.31/1.00 (G-J:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

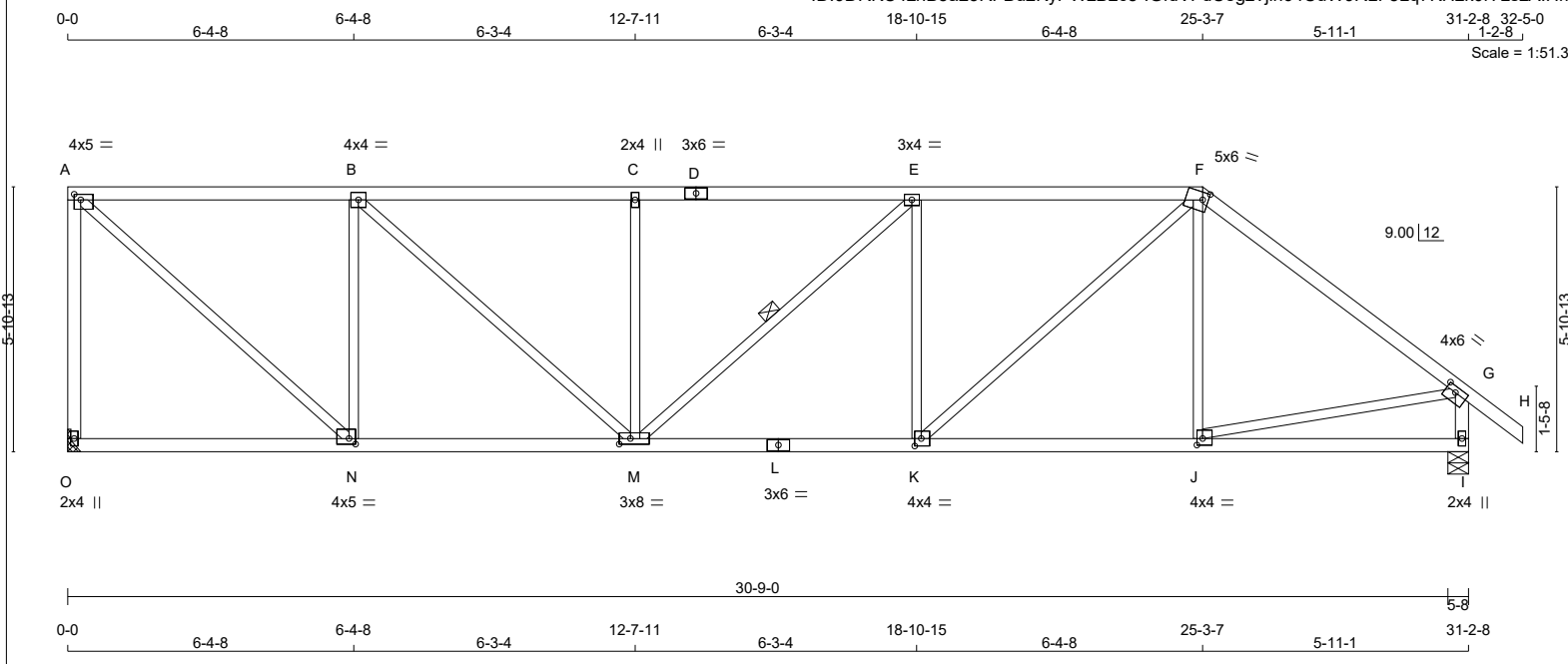
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.50 (J) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 130 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.50	1.75
B	TMVW-t	MT20	4.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	3.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BMVW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	8.0	1.50	3.00
N	BMVW-t	MT20	4.0	5.0	1.50	1.75
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
O	1480	0	1480	0
I	1580	0	1580	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT O TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1039	727 / 0	0 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
I	1107	787 / 0	0 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 20-0-0 . CBF = 1 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
O-A	-1435 / 0	0.0 0.0	0.94 (1)	6.82	A-N	0 / 1876	0.42 (1)
A-B	-1408 / 0	-77.4 -77.4	0.54 (1)	4.82	N-B	-1114 / 0	0.60 (1)
B-C	-2029 / 0	-77.4 -77.4	0.60 (1)	4.10	B-M	0 / 833	0.19 (1)
C-D	-2029 / 0	-77.4 -77.4	0.60 (1)	4.10	M-C	-447 / 0	0.24 (1)
D-E	-2029 / 0	-77.4 -77.4	0.60 (1)	4.10	M-E	-5 / 4	0.00 (1)
E-F	-2032 / 0	-77.4 -77.4	0.61 (1)	4.10	K-E	-549 / 0	0.30 (1)
F-G	-1609 / 0	-77.4 -77.4	0.62 (1)	4.50	K-F	0 / 997	0.22 (1)
G-H	0 / 30	-77.4 -77.4	0.09 (1)	10.00	J-F	-120 / 63	0.06 (1)
I-G	-1539 / 0	0.0 0.0	0.16 (1)	6.64	J-G	0 / 1308	0.29 (1)
O-N	0 / 0	-17.5 -17.5	0.17 (4)	10.00			
N-M	0 / 1408	-17.5 -17.5	0.32 (1)	10.00			
M-L	0 / 2032	-17.5 -17.5	0.39 (1)	10.00			
L-K	0 / 2032	-17.5 -17.5	0.39 (1)	10.00			
K-J	0 / 1283	-17.5 -17.5	0.29 (1)	10.00			
J-I	0 / 0	-17.5 -17.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.0	PSF	
TOTAL LOAD = 33.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.94/1.00 (A-O:1), BC=0.39/1.00 (K-M:1), WB=0.60/1.00 (B-N:1), SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

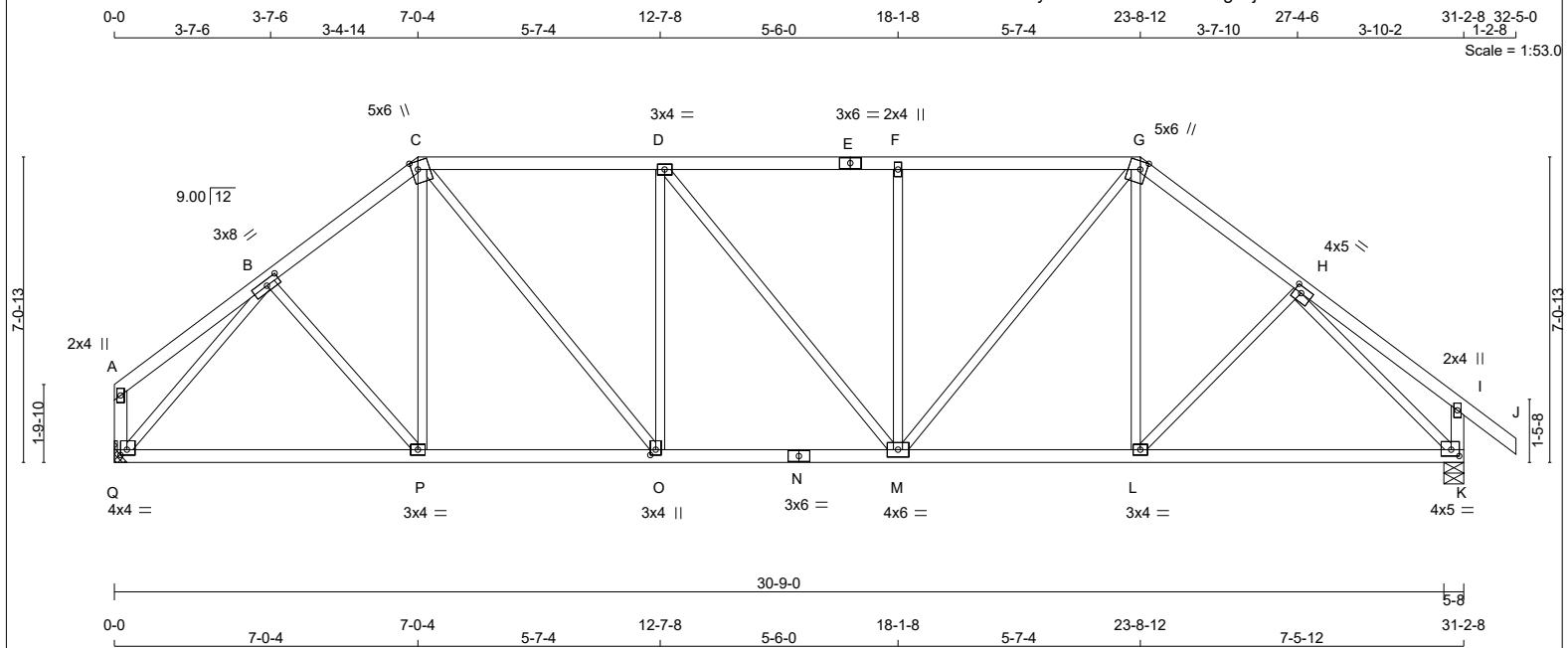
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.60 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 138 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
B	TMWW-t	MT20	3.0	8.0	1.50 3.75
C	TTVW+m	MT20	5.0	6.0	2.25 1.75
D	TMWW-t	MT20	3.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TTVW+m	MT20	5.0	6.0	2.25 1.75
H	TMWW-t	MT20	4.0	5.0	1.75 2.00
I	TMV+p	MT20	2.0	4.0	
K	BMVW1-t	MT20	4.0	5.0	1.75 2.25
L	BMWW-t	MT20	3.0	4.0	
M	BMWWW-t	MT20	4.0	6.0	
N	BS-t	MT20	3.0	6.0	
O	BMWW-t	MT20	3.0	4.0	1.50 1.50
P	BMWW-t	MT20	3.0	4.0	
Q	BMVW1-t	MT20	4.0	4.0	1.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	1480	0	1480	0	0
K	1580	0	1580	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT Q TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1039	727 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
K	1107	787 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-77.4	-77.4 0.14 (1)	B-P	0 / 111	0.03 (4)	
B-C	-1526 / 0	-77.4	-77.4 0.17 (1)	P-C	0 / 88	0.03 (4)	
C-D	-1681 / 0	-77.4	-77.4 0.40 (1)	C-O	0 / 747	0.17 (1)	
D-E	-1697 / 0	-77.4	-77.4 0.40 (1)	O-D	-486 / 0	0.42 (1)	
E-F	-1697 / 0	-77.4	-77.4 0.40 (1)	D-M	0 / 25	0.01 (1)	
F-G	-1697 / 0	-77.4	-77.4 0.39 (1)	M-F	-464 / 0	0.40 (1)	
G-H	-1594 / 0	-77.4	-77.4 0.20 (1)	M-G	0 / 686	0.15 (1)	
H-I	0 / 19	-77.4	-77.4 0.16 (1)	L-G	0 / 125	0.04 (4)	
I-J	0 / 30	-77.4	-77.4 0.09 (1)	L-H	0 / 54	0.02 (4)	
Q-A	-108 / 0	0.0	0.0 0.01 (1)	Q-B	-1739 / 0	0.81 (1)	
K-I	-213 / 0	0.0	0.0 0.02 (1)	H-K	-1809 / 0	0.86 (1)	
Q-P	0 / 1130	-17.5	-17.5 0.29 (1)				
P-O	0 / 1204	-17.5	-17.5 0.30 (1)				
O-N	0 / 1681	-17.5	-17.5 0.32 (1)				
N-M	0 / 1681	-17.5	-17.5 0.32 (1)				
M-L	0 / 1260	-17.5	-17.5 0.32 (1)				
L-K	0 / 1246	-17.5	-17.5 0.32 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.40/1.00 (C-D:1), BC=0.32/1.00 (L-M:1), WB=0.86/1.00 (H-K:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

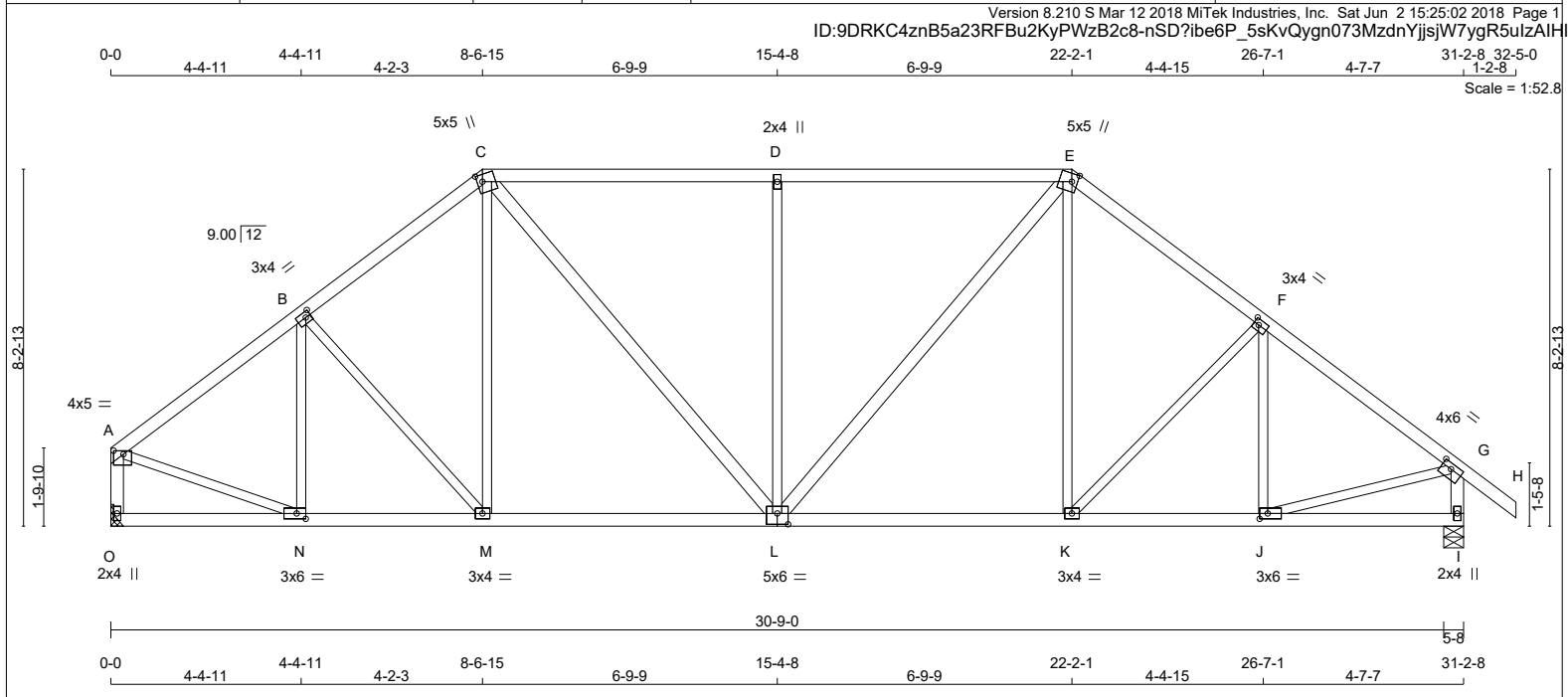
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.54 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE		LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH.	LL	PSF
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL	=	3.0
C - E	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	UP	BOT CH.	LL	PSF
E - H	2x4	DRY	No.2	SPF	O	I	0	0	DL	=	0.0
O - A	2x4	DRY	No.2	SPF	1480	0	1480	0	MECHANICAL	DL	=
I - G	2x4	DRY	No.2	SPF	1580	0	1580	0	5-8	TOTAL LOAD	=
O - L	2x4	DRY	No.2	SPF							
L - I	2x4	DRY	No.2	SPF							
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT O TO RESIST THE MAX FACTORED REACTIONS.						
C - L	2x4	DRY	No.2	SPF	UNFACTORED REACTIONS						
L - E	2x4	DRY	No.2	SPF	LOADING						

DRY: SEASONED LUMBER.

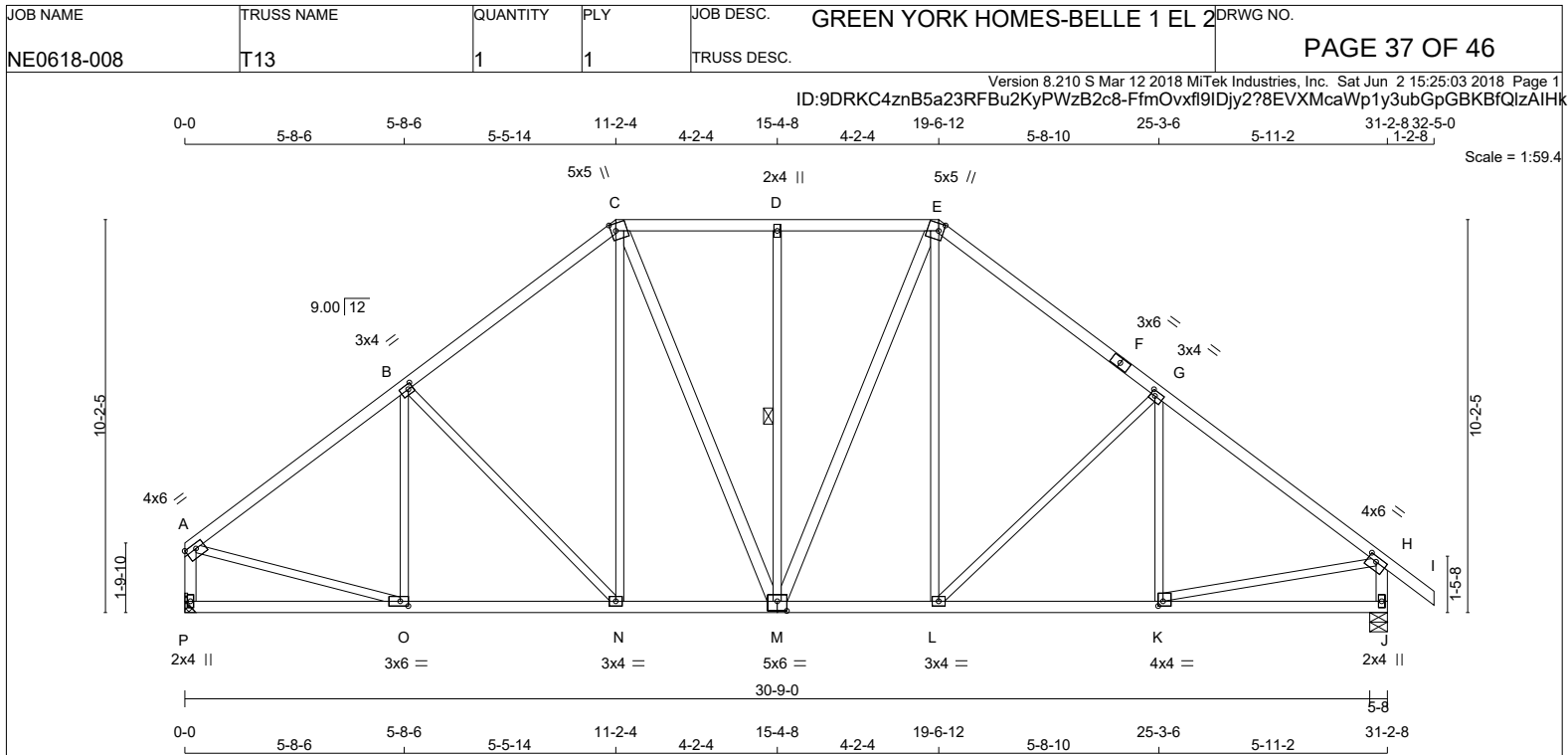
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

PLATES (table is in inches)				BRACING				LOADING			
JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX/MIN COMPONENT REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS	1ST LCASE
A	TMVW-p	MT20	4.0	5.0	1.00	2.75	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
B	TMWW-t	MT20	3.0	4.0	1.50	1.50	O	1039	727 / 0	0 / 0	0 / 0
C	TTWW+m	MT20	5.0	5.0	2.00	1.50	I	1107	787 / 0	0 / 0	0 / 0
D	TMW+w	MT20	2.0	4.0			BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I				
E	TTWW+m	MT20	5.0	5.0	2.25	1.50	BRACING				
F	TMWW-t	MT20	3.0	4.0	1.50	1.50	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.				
G	TMVW-t	MT20	4.0	6.0	1.50	2.75	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				
I	BMV1+p	MT20	2.0	4.0			ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				
J	BMWW-t	MT20	3.0	6.0	1.50	2.25	LOADING				
K	BMWW-t	MT20	3.0	4.0			TOTAL LOAD CASES: (4)				
L	BSWWW-I	MT20	5.0	6.0	3.00	3.00	CHORDS				
M	BMWW-t	MT20	3.0	4.0			MAX. FACTORED				
N	BMWW-t	MT20	3.0	6.0	1.50	2.50	MEMB. FORCE (LBS)				
O	BMV1+p	MT20	2.0	4.0			FACTORED				

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO	FROM	TO			FR-TO	FROM	TO
A-B	-1490 / 0	-77.4	-77.4 0.21 (1)	5.18	N-B	-333 / 0	0.13 (1)
B-C	-1485 / 0	-77.4	-77.4 0.21 (1)	5.18	B-M	-65 / 0	0.05 (1)
C-D	-1517 / 0	-77.4	-77.4 0.51 (1)	4.72	M-C	0 / 149	0.04 (4)
D-E	-1517 / 0	-77.4	-77.4 0.51 (1)	4.72	C-L	0 / 535	0.09 (1)
E-F	-1532 / 0	-77.4	-77.4 0.23 (1)	5.10	L-D	-646 / 0	0.86 (1)
F-G	-1617 / 0	-77.4	-77.4 0.24 (1)	4.99	L-E	0 / 478	0.08 (1)
G-H	0 / 30	-77.4	-77.4 0.09 (1)	10.00	K-E	0 / 213	0.05 (1)
O-A	-1446 / 0	0.0	0.0 0.16 (1)	6.80	K-F	-156 / 0	0.12 (1)
I-G	-1544 / 0	0.0	0.0 0.16 (1)	6.62	J-F	-243 / 0	0.09 (1)
					A-N	0 / 1277	0.29 (1)
					J-G	0 / 1351	0.30 (1)
O-N	0 / 0	-17.5	-17.5 0.07 (4)	10.00			
N-M	0 / 1210	-17.5	-17.5 0.26 (1)	10.00			
M-L	0 / 1169	-17.5	-17.5 0.28 (1)	10.00			
L-K	0 / 1206	-17.5	-17.5 0.29 (1)	10.00			
K-J	0 / 1312	-17.5	-17.5 0.28 (1)	10.00			
J-I	0 / 0	-17.5	-17.5 0.08 (4)	10.00			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - A	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

C - M 2x4 DRY No.2 SPF

M - E 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	Edge
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TTWW+m	MT20	5.0	5.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMWW-t	MT20	3.0	4.0		
M	BSWWW-l	MT20	5.0	6.0	3.00	3.00
N	BMWW-t	MT20	3.0	4.0		
O	BMWW-t	MT20	3.0	6.0	1.50	2.50
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
JT				
P	1480	0	1480	0
J	1580	0	1580	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT P TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED							
P	1039	727 / 0	0 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
J	1107	787 / 0	0 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M. DBS = 20-0-0 . CBF = 49 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1543 / 0	-77.4 -77.4	0.37 (1)	4.92	O-B	-215 / 31	0.12 (1)
B-C	-1379 / 0	-77.4 -77.4	0.35 (1)	5.16	B-N	-263 / 0	0.35 (1)
C-D	-1184 / 0	-77.4 -77.4	0.18 (1)	5.68	N-C	0 / 273	0.06 (1)
D-E	-1184 / 0	-77.4 -77.4	0.18 (1)	5.68	C-M	0 / 270	0.04 (1)
E-F	-1403 / 0	-77.4 -77.4	0.38 (1)	5.08	M-D	-389 / 0	0.28 (1)
F-G	-1403 / 0	-77.4 -77.4	0.38 (1)	5.08	M-E	0 / 222	0.04 (1)
G-H	-1645 / 0	-77.4 -77.4	0.41 (1)	4.75	L-E	0 / 324	0.07 (1)
H-I	0 / 30	-77.4 -77.4	0.09 (1)	10.00	L-G	-343 / 0	0.47 (1)
P-A	-1439 / 0	0.0 0.0	0.16 (1)	6.81	K-G	-141 / 57	0.08 (1)
J-H	-1538 / 0	0.0 0.0	0.16 (1)	6.64	A-O	0 / 1301	0.29 (1)
					K-H	0 / 1366	0.31 (1)
P-O	0 / 0	-17.5 -17.5	0.14 (4)	10.00			
O-N	0 / 1259	-17.5 -17.5	0.27 (1)	10.00			
N-M	0 / 1079	-17.5 -17.5	0.22 (1)	10.00			
M-L	0 / 1097	-17.5 -17.5	0.22 (1)	10.00			
L-K	0 / 1342	-17.5 -17.5	0.29 (1)	10.00			
K-J	0 / 0	-17.5 -17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.41/1.00 (G-H:1), BC=0.29/1.00 (K-L:1), WB=0.47/1.00 (G-L:1), SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

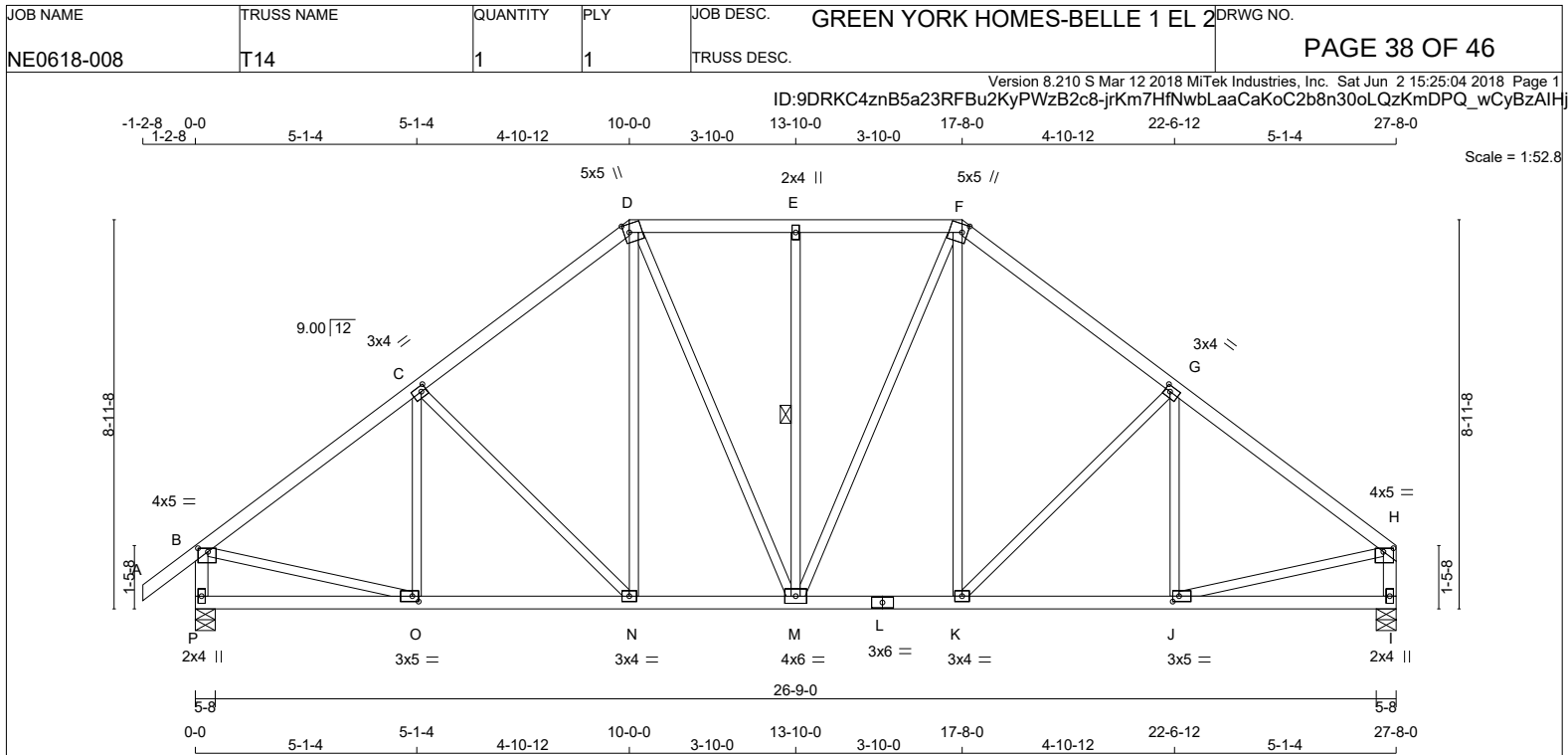
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.50 (K) (INPUT = 1.00)



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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
P - L	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	5.0	1.00	2.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.25	1.50
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-p	MT20	4.0	5.0	1.00	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	3.0	5.0	1.50	1.75
K	BMWW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	3.0	4.0		
O	BMWW-t	MT20	3.0	5.0	1.50	1.75
P	BMV1+p	MT20	2.0	4.0		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	HORZ	UPLIFT
P 1412	0	1412	0
I 1312	0	1312	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM.LIVE
P 989	705 / 0	0 / 0	0 / 0
I 921	644 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 20-0-0. CBF = 44 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	O-C	-158 / 38	0.07 (1)
B-C	-1417 / 0	-77.4 -77.4	0.28 (1)	5.19	C-N	-262 / 0	0.25 (1)
C-D	-1242 / 0	-77.4 -77.4	0.27 (1)	5.47	N-D	0 / 263	0.06 (1)
D-E	-1064 / 0	-77.4 -77.4	0.15 (1)	5.95	D-M	0 / 225	0.05 (1)
E-F	-1064 / 0	-77.4 -77.4	0.15 (1)	5.95	M-E	-354 / 0	0.18 (1)
F-G	-1242 / 0	-77.4 -77.4	0.27 (1)	5.47	M-F	0 / 225	0.05 (1)
G-H	-1417 / 0	-77.4 -77.4	0.28 (1)	5.19	K-F	0 / 263	0.06 (1)
P-B	-1375 / 0	0.0 0.0	0.14 (1)	6.93	K-G	-262 / 0	0.25 (1)
I-H	-1275 / 0	0.0 0.0	0.13 (1)	7.13	J-G	-158 / 38	0.07 (1)
					B-O	0 / 1184	0.27 (1)
					J-H	0 / 1184	0.27 (1)
P-O	0 / 0	-17.5 -17.5	0.11 (4)	10.00			
O-N	0 / 1156	-17.5 -17.5	0.24 (1)	10.00			
N-M	0 / 972	-17.5 -17.5	0.19 (1)	10.00			
M-L	0 / 972	-17.5 -17.5	0.19 (1)	10.00			
L-K	0 / 972	-17.5 -17.5	0.19 (1)	10.00			
K-J	0 / 1156	-17.5 -17.5	0.24 (1)	10.00			
J-I	0 / 0	-17.5 -17.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.28/1.00 (G-H:1), BC=0.24/1.00 (J-K:1), WB=0.27/1.00 (H-J:1), SSI=0.15/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

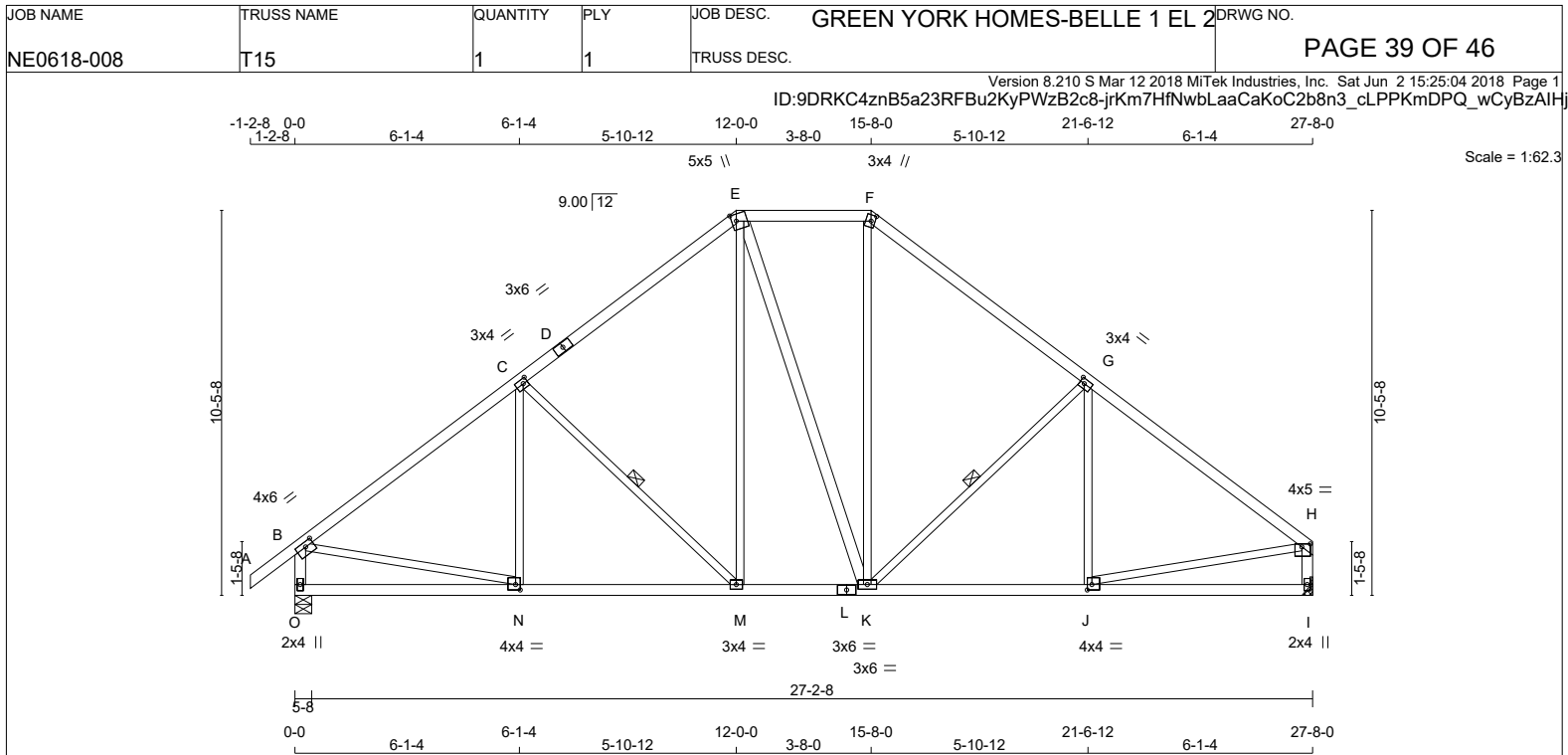
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.36 (B) (INPUT = 1.00)





LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	DRY	LUMBER	SPF	No.2
A - D	2x4	DRY	No.2	SPF	No.2
D - E	2x4	DRY	No.2	SPF	No.2
E - F	2x4	DRY	No.2	SPF	No.2
F - H	2x4	DRY	No.2	SPF	No.2
O - B	2x4	DRY	No.2	SPF	No.2
I - H	2x4	DRY	No.2	SPF	No.2
O - L	2x4	DRY	No.2	SPF	No.2
L - I	2x4	DRY	No.2	SPF	No.2
ALL WEBS	2x3	DRY	No.2	SPF	No.2
EXCEPT					
E - K	2x4	DRY	No.2	SPF	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.50 2.75
C	TMWW-t	MT20	3.0	4.0	1.50 1.50
D	TS-t	MT20	3.0	6.0	
E	TTWW+m	MT20	5.0	5.0	2.25 1.50
F	TTW+m	MT20	3.0	4.0	2.00 1.25
G	TMWW-t	MT20	3.0	4.0	1.50 1.50
H	TMVW-p	MT20	4.0	5.0	1.00 2.75
I	BMV1+p	MT20	2.0	4.0	
J	BMWW-t	MT20	4.0	4.0	1.75 1.50
K	BMWW-t	MT20	3.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW-t	MT20	3.0	4.0	
N	BMWW-t	MT20	4.0	4.0	1.75 1.50
O	BMV1+p	MT20	2.0	4.0	



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	
O	1412 0	1412 0	5-8	5-8	
I	1312 0	1312 0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS					
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS			
O	989	705 / 0	0 / 0	0 / 0	284 / 0
I	921	644 / 0	0 / 0	0 / 0	277 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, G-K. DBS = 20-0-0. CBF = 49 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-C	-94 / 75	0.05 (1)	
B-C	-1422 / 0	-77.4	-77.4	0.42 (1)	5.00	C-M	-395 / 0	0.18 (1)	
C-D	-1134 / 0	-77.4	-77.4	0.39 (1)	5.49	M-E	0 / 347	0.08 (1)	
D-E	-1134 / 0	-77.4	-77.4	0.39 (1)	5.49	E-K	0 / 3	0.00 (1)	
E-F	-883 / 0	-77.4	-77.4	0.14 (1)	6.25	K-F	0 / 349	0.08 (1)	
F-G	-1135 / 0	-77.4	-77.4	0.39 (1)	5.49	K-G	-393 / 0	0.18 (1)	
G-H	-1421 / 0	-77.4	-77.4	0.42 (1)	5.01	J-G	-96 / 74	0.05 (1)	
O-B	-1369 / 0	0.0	0.0	0.14 (1)	6.94	B-N	0 / 1184	0.27 (1)	
I-H	-1269 / 0	0.0	0.0	0.13 (1)	7.15	J-H	0 / 1183	0.27 (1)	
O-N	0 / 0	-17.5	-17.5	0.16 (4)	10.00				
N-M	0 / 1164	-17.5	-17.5	0.27 (1)	10.00				
M-L	0 / 882	-17.5	-17.5	0.19 (1)	10.00				
L-K	0 / 882	-17.5	-17.5	0.19 (1)	10.00				
K-J	0 / 1163	-17.5	-17.5	0.27 (1)	10.00				
J-I	0 / 0	-17.5	-17.5	0.16 (4)	10.00				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.0 PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.42/1.00 (B-C:1), BC=0.27/1.00 (M-N:1), WB=0.27/1.00 (B-N:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

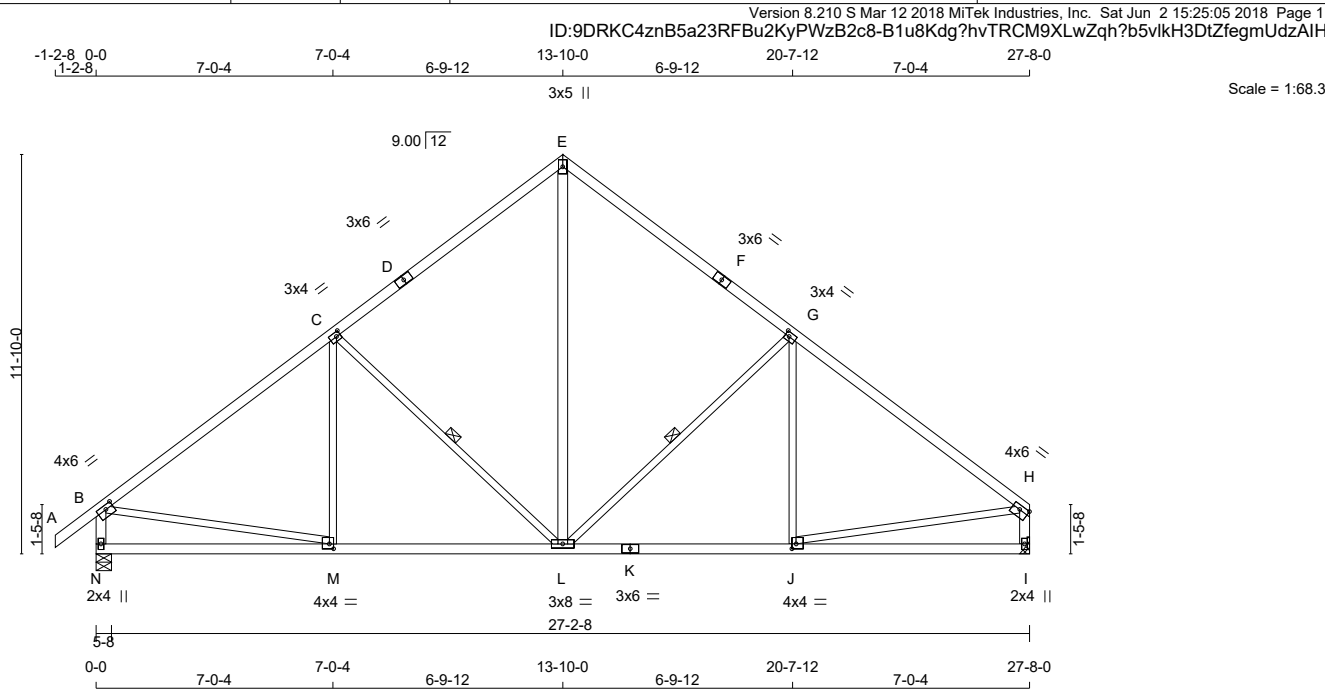
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.44 (N) (INPUT = 1.00)





TOTAL WEIGHT = 124 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT L - E	2x3	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.50 2.75
C	TMWW-t	MT20	3.0	4.0	1.50 1.50
D	TS-t	MT20	3.0	6.0	
E	TTW+p	MT20	3.0	5.0	
F	TS-t	MT20	3.0	6.0	
G	TMWW-t	MT20	3.0	4.0	1.50 1.50
H	TMVW-t	MT20	4.0	6.0	1.50 Edge
I	BMV1+p	MT20	2.0	4.0	
J	BMWW-t	MT20	4.0	4.0	1.75 1.50
K	BS-t	MT20	3.0	6.0	
L	BMWWW-t	MT20	3.0	8.0	
M	BMWW-t	MT20	4.0	4.0	1.75 1.50
N	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP	IN-SX
N	1412	0	1412	0	5-8
I	1312	0	1312	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	989	705 / 0	0 / 0	0 / 0	0 / 0	0 / 0	284 / 0	0 / 0
I	921	644 / 0	0 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-L, C-L. DBS = 20-0-0 . CBF = 62 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	L-E	0 / 797	0.13 (1)	
B-C	-1409 / 0	-77.4 -77.4	0.58 (1)	L-G	-500 / 0	0.30 (1)	
C-D	-1032 / 0	-77.4 -77.4	0.53 (1)	J-G	-51 / 100	0.04 (1)	
D-E	-1032 / 0	-77.4 -77.4	0.53 (1)	C-L	-499 / 0	0.30 (1)	
E-F	-1032 / 0	-77.4 -77.4	0.53 (1)	M-C	-51 / 100	0.04 (1)	
F-G	-1032 / 0	-77.4 -77.4	0.53 (1)	B-M	0 / 1173	0.26 (1)	
G-H	-1409 / 0	-77.4 -77.4	0.58 (1)	J-H	0 / 1173	0.26 (1)	
N-B	-1362 / 0	0.0 0.0	0.14 (1)				
I-H	-1262 / 0	0.0 0.0	0.13 (1)				
N-M	0 / 0	-17.5 -17.5	0.20 (4)				
M-L	0 / 1158	-17.5 -17.5	0.29 (1)				
L-K	0 / 1158	-17.5 -17.5	0.29 (1)				
K-J	0 / 1158	-17.5 -17.5	0.29 (1)				
J-I	0 / 0	-17.5 -17.5	0.20 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.58/1.00 (G-H:1) , BC=0.29/1.00 (J-L:1) , WB=0.30/1.00 (G-L:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES		PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)				
MT20	618	354	1667	822	2284	1656	

PLATE PLACEMENT TOL. = 0.250 inches

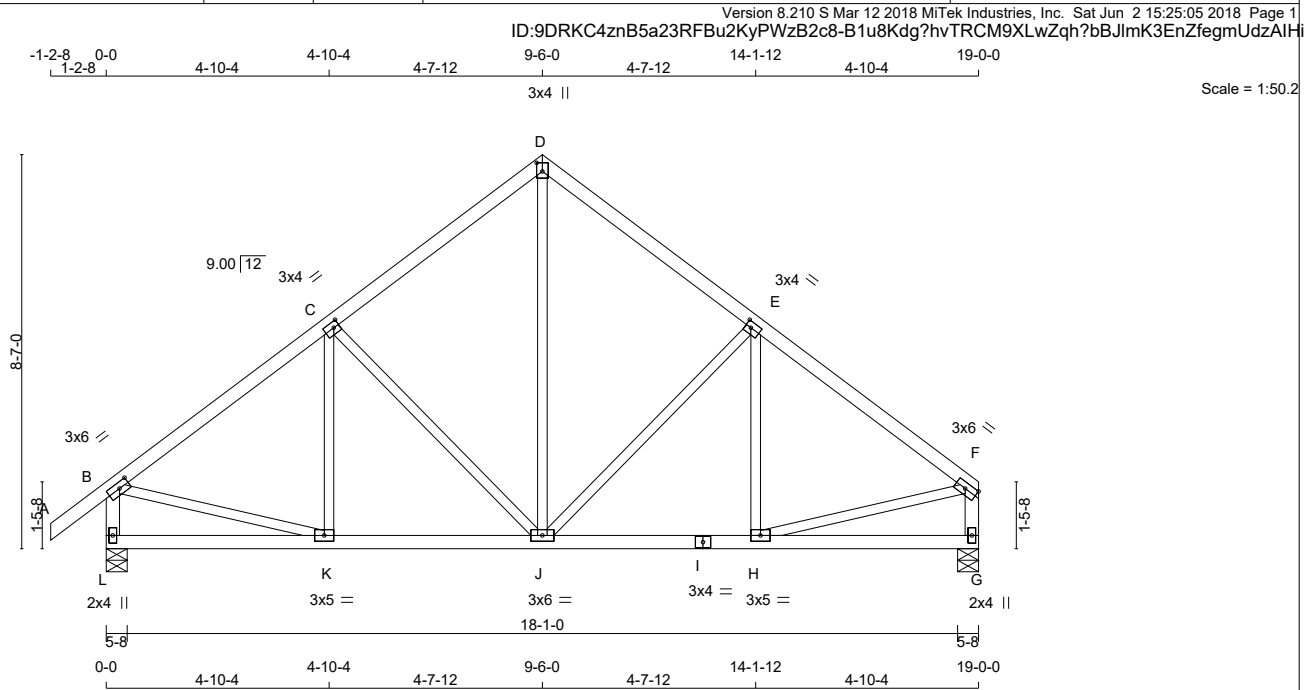
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.43 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 85 = 255 lb

LUMBER

COMB N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCRIPTION
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

BT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	3.0	6.0	1.50	2.75
B	TMWV-t	MT20	3.0	4.0	1.50	1.50
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMWV-t	MT20	3.0	4.0	1.50	1.50
E	TMWV-t	MT20	3.0	6.0	1.50	Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMWV-t	MT20	3.0	5.0		
H	BS-t	MT20	3.0	4.0		
I	BMWVWV-t	MT20	3.0	6.0		
J	BMWV-t	MT20	3.0	5.0		
K	BMV1+p	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1001	0	1001	0	0	5-8	5-8
G	901	0	901	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	701	503 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
G	632	442 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L. G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	J-D	0 / 513	0.12 (1)
B-C	-891 / 0	-77.4	-77.4 0.23 (1)	6.24	J-E	-304 / 0	0.25 (1)
C-D	-679 / 0	-77.4	-77.4 0.23 (1)	6.25	H-E	-79 / 56	0.03 (1)
D-E	-679 / 0	-77.4	-77.4 0.23 (1)	6.25	C-J	-303 / 0	0.25 (1)
E-F	-891 / 0	-77.4	-77.4 0.23 (1)	6.24	K-C	-79 / 56	0.03 (1)
L-B	-966 / 0	0.0	0.0 0.10 (1)	7.81	B-K	0 / 753	0.17 (1)
G-F	-866 / 0	0.0	0.0 0.09 (1)	7.81	H-F	0 / 753	0.17 (1)
L-K	0 / 0	-17.5	-17.5 0.09 (4)	10.00			
K-J	0 / 734	-17.5	-17.5 0.16 (1)	10.00			
J-I	0 / 734	-17.5	-17.5 0.16 (1)	10.00			
I-H	0 / 734	-17.5	-17.5 0.16 (1)	10.00			
H-G	0 / 0	-17.5	-17.5 0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9. NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.23/1.00 (E-F:1) , BC=0.16/1.00 (H-J:1) ,
WB=0.25/1.00 (E-J:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

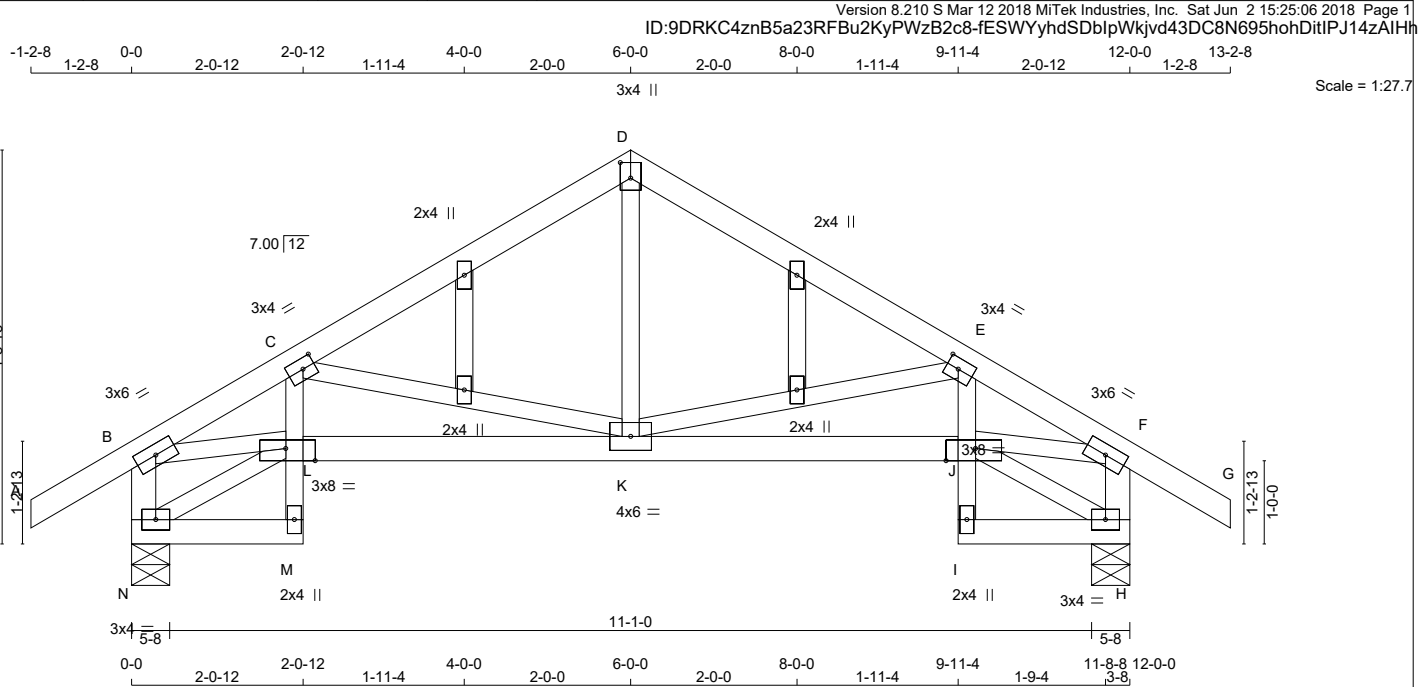
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 55 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x3	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x3	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	3.0	6.0
C	TMVW-t	MT20	3.0	4.0 1.50 1.75
D	TTW+p	MT20	3.0	4.0 2.25 1.50
E	TMVW-t	MT20	3.0	4.0 1.50 1.75
F	TMVW-t	MT20	3.0	6.0
H	BMVW1-t	MT20	3.0	4.0
I	BMV+p	MT20	2.0	4.0
J	BVMWW-I	MT20	3.0	8.0 Edge 4.25
K	BMVWW-t	MT20	4.0	6.0
L	BVMWW-I	MT20	3.0	8.0 Edge 4.25
M	BMV+p	MT20	2.0	4.0
N	BMVW1-t	MT20	3.0	4.0
O, P, Q, R				
O	NP+w	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	DOWN	UP
N		668	0	668	0
H		668	0	668	0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	467	339 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	467	339 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 25	-77.4	-77.4 0.09 (1)	10.00	K-D	0 / 318	0.07 (1)
B-C	-1183 / 0	-77.4	-77.4 0.09 (1)	5.79	K-E	-574 / 0	0.17 (1)
C-D	-599 / 0	-77.4	-77.4 0.16 (1)	6.25	C-K	-574 / 0	0.17 (1)
D-E	-599 / 0	-77.4	-77.4 0.16 (1)	6.25	B-L	0 / 1044	0.23 (1)
E-F	-1183 / 0	-77.4	-77.4 0.09 (1)	5.79	J-F	0 / 1044	0.23 (1)
F-G	0 / 25	-77.4	-77.4 0.09 (1)	10.00	N-L	-26 / 0	0.00 (1)
N-B	-638 / 0	0.0	0.0 0.06 (1)	7.81	J-H	-26 / 0	0.00 (1)
H-F	-638 / 0	0.0	0.0 0.06 (1)	7.81			
N-M	0 / 22	-17.5	-17.5 0.02 (4)	10.00			
M-L	0 / 19	0.0	0.0 0.07 (1)	10.00			
L-C	0 / 167	0.0	0.0 0.10 (1)	10.00			
L-K	0 / 1065	-17.5	-17.5 0.22 (1)	10.00			
K-J	0 / 1065	-17.5	-17.5 0.22 (1)	10.00			
I-J	0 / 19	0.0	0.0 0.07 (1)	10.00			
J-E	0 / 167	0.0	0.0 0.10 (1)	10.00			
I-H	0 / 22	-17.5	-17.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (C-D:1), BC=0.22/1.00 (K-L:1), WB=0.23/1.00 (B-L:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

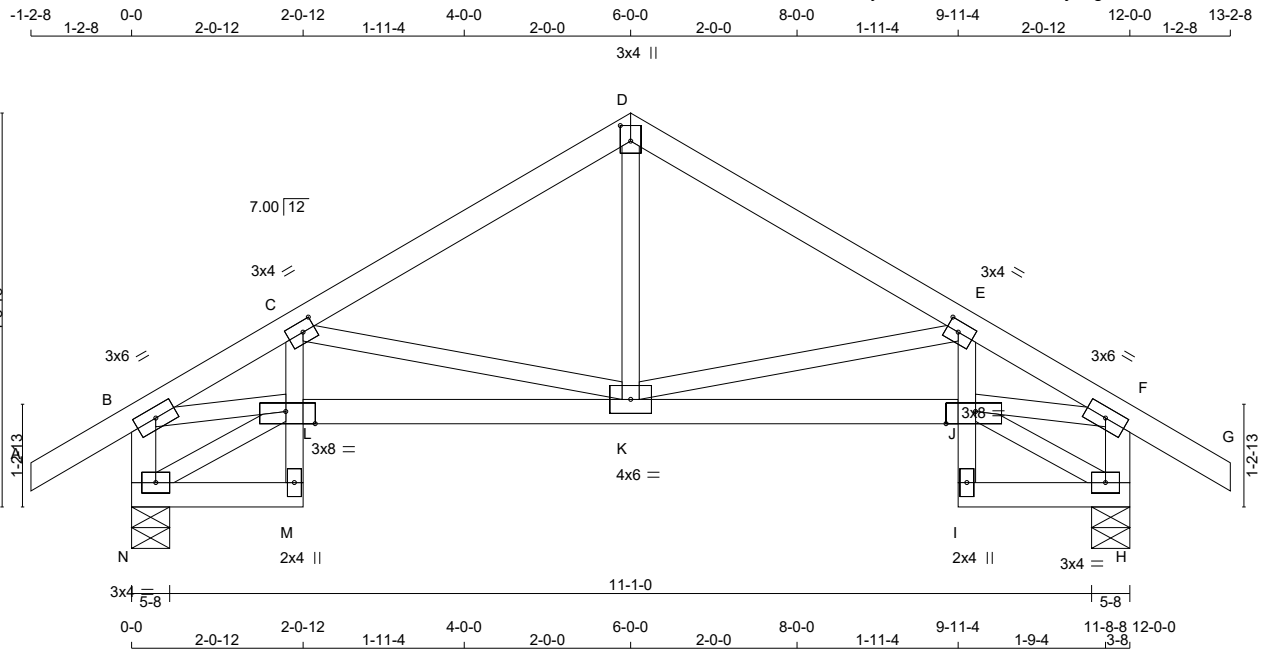
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.32 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 52 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - G	2x4	DRY	No.2 SPF
N - B	2x4	DRY	No.2 SPF
H - F	2x4	DRY	No.2 SPF
N - M	2x4	DRY	No.2 SPF
M - C	2x3	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
I - E	2x3	DRY	No.2 SPF
I - H	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	6.0		
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.75
F	TMVW-t	MT20	3.0	6.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMV+p	MT20	2.0	4.0		
J	BVMWW-I	MT20	3.0	8.0	Edge 4.25	
K	BVMWW-t	MT20	4.0	6.0		
L	BVMWW-I	MT20	3.0	8.0	Edge 4.25	
M	BMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	668	0	668	0	5-8	5-8
H	668	0	668	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	467	339 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	467	339 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 25	-77.4	-77.4	0.09 (1)	10.00	K-D	0 / 318	0.07 (1)	
B-C	-1183 / 0	-77.4	-77.4	0.09 (1)	5.79	K-E	-574 / 0	0.17 (1)	
C-D	-599 / 0	-77.4	-77.4	0.16 (1)	6.25	C-K	-574 / 0	0.17 (1)	
D-E	-599 / 0	-77.4	-77.4	0.16 (1)	6.25	B-L	0 / 1044	0.23 (1)	
E-F	-1183 / 0	-77.4	-77.4	0.09 (1)	5.79	J-F	0 / 1044	0.23 (1)	
F-G	0 / 25	-77.4	-77.4	0.09 (1)	10.00	N-L	-26 / 0	0.00 (1)	
N-B	-638 / 0	0.0	0.0	0.06 (1)	7.81	J-H	-26 / 0	0.00 (1)	
H-F	-638 / 0	0.0	0.0	0.06 (1)	7.81				
N-M	0 / 22	-17.5	-17.5	0.02 (4)	10.00				
M-L	0 / 19	0.0	0.0	0.07 (1)	10.00				
L-C	0 / 167	0.0	0.0	0.10 (1)	10.00				
L-K	0 / 1065	-17.5	-17.5	0.22 (1)	10.00				
K-J	0 / 1065	-17.5	-17.5	0.22 (1)	10.00				
I-J	0 / 19	0.0	0.0	0.07 (1)	10.00				
J-E	0 / 167	0.0	0.0	0.10 (1)	10.00				
I-H	0 / 22	-17.5	-17.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.16/1.00 (C-D:1), BC=0.22/1.00 (K-L:1), WB=0.23/1.00 (B-L:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

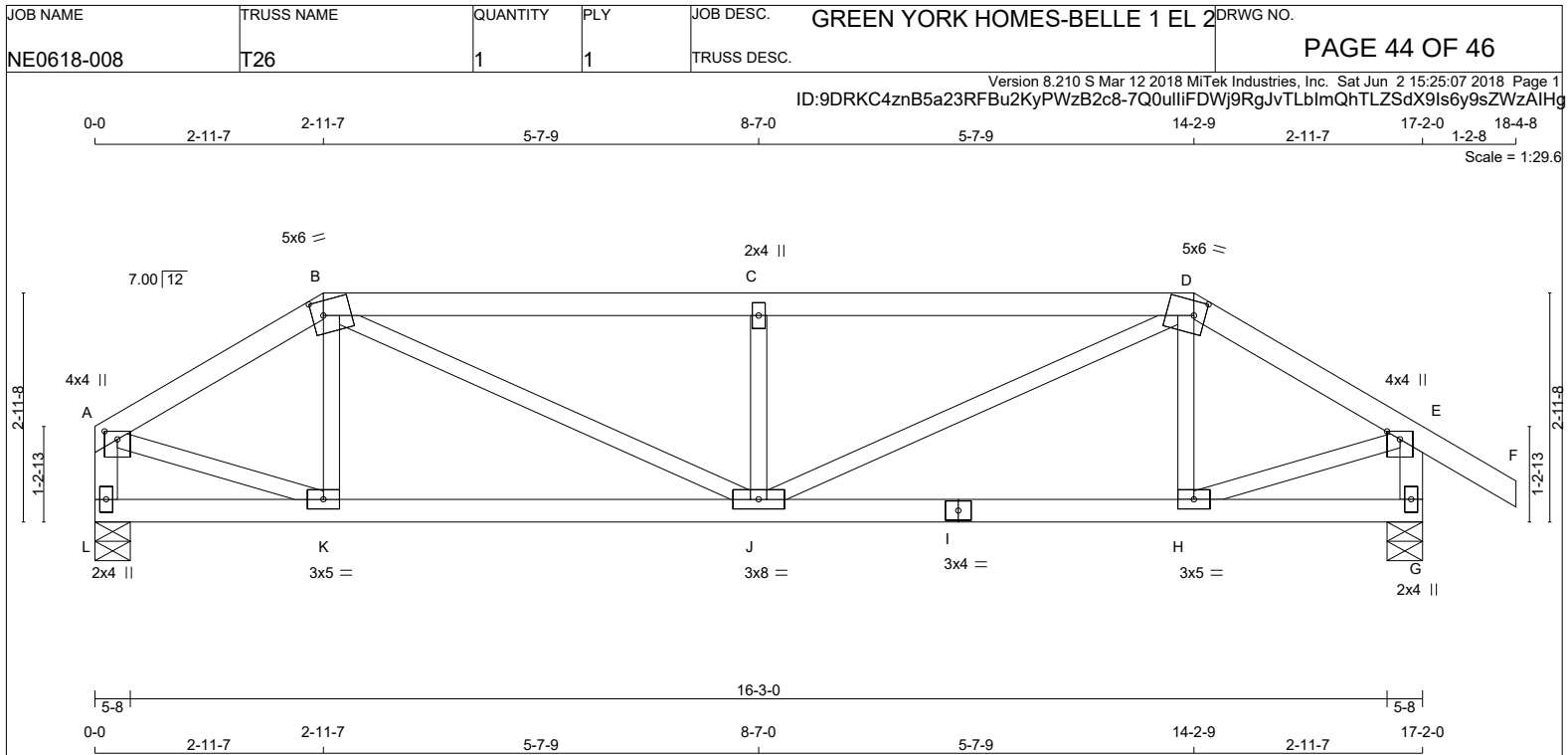
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
L - A	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	3.0	5.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW-t	MT20	3.0	8.0		
K	BMWW-t	MT20	3.0	5.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	814	0	814	0	0	5-8	5-8
G	913	0	913	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	571	400 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0
G	639	459 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX	MEMB.
		(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO			FROM	TO	LENGTH	FR-TO			
A-B	-895 / 0	-77.4	-77.4	0.13 (1)	6.25	K-B	-143 / 24	0.02 (1)	
B-C	-1426 / 0	-77.4	-77.4	0.45 (1)	4.90	B-J	0 / 730	0.16 (1)	
C-D	-1426 / 0	-77.4	-77.4	0.45 (1)	4.90	J-C	-530 / 0	0.09 (1)	
D-E	-895 / 0	-77.4	-77.4	0.13 (1)	6.25	J-D	0 / 730	0.16 (1)	
E-F	0 / 25	-77.4	-77.4	0.09 (1)	10.00	H-D	-143 / 24	0.02 (1)	
L-A	-800 / 0	0.0	0.0	0.08 (1)	7.81	A-K	0 / 806	0.18 (1)	
G-E	-899 / 0	0.0	0.0	0.09 (1)	7.81	H-E	0 / 806	0.18 (1)	
L-K	0 / 0	-17.5	-17.5	0.08 (4)	10.00				
K-J	0 / 767	-17.5	-17.5	0.17 (1)	10.00				
J-I	0 / 767	-17.5	-17.5	0.17 (1)	10.00				
I-H	0 / 767	-17.5	-17.5	0.17 (1)	10.00				
H-G	0 / 0	-17.5	-17.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.08")

CSI: TC=0.45/1.00 (B-C:1), BC=0.17/1.00 (J-K:1), WB=0.18/1.00 (A-K:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)
JSI METAL= 0.27 (I) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



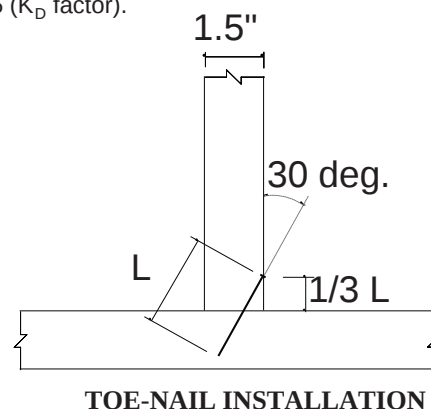
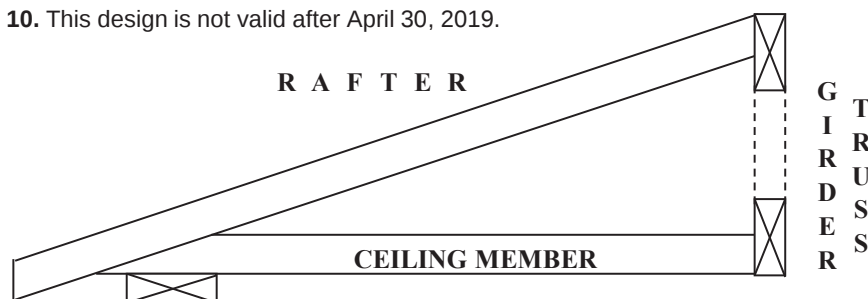
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

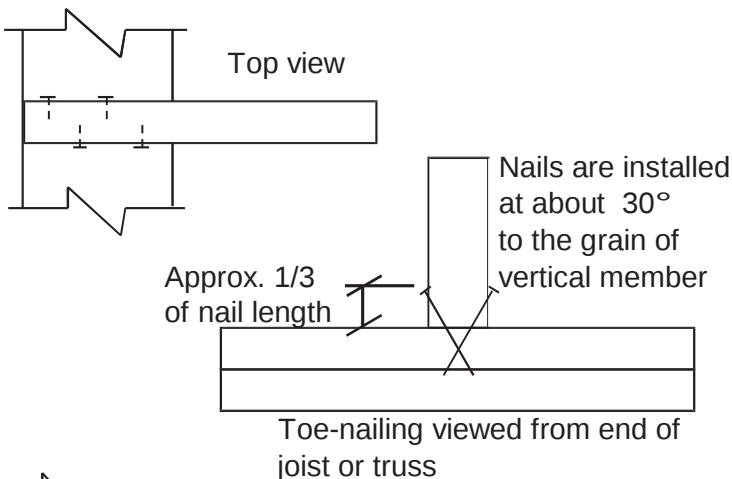
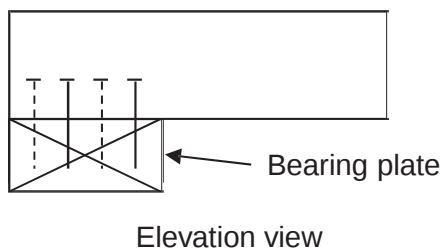
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

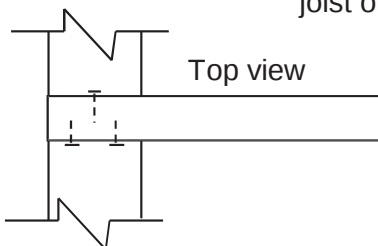
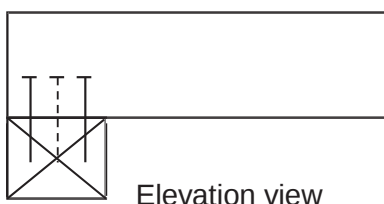
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

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Certificate No. 10889485



April 26, 2017