

M-2057 LOT 4

NE0518-224**GREEN YORK HOMES-AMELIA 2 EL 2****RESPONSIBILITIES**

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

*For conventional framing roof / ceilings
framing shall conform to OBC.9.23*

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

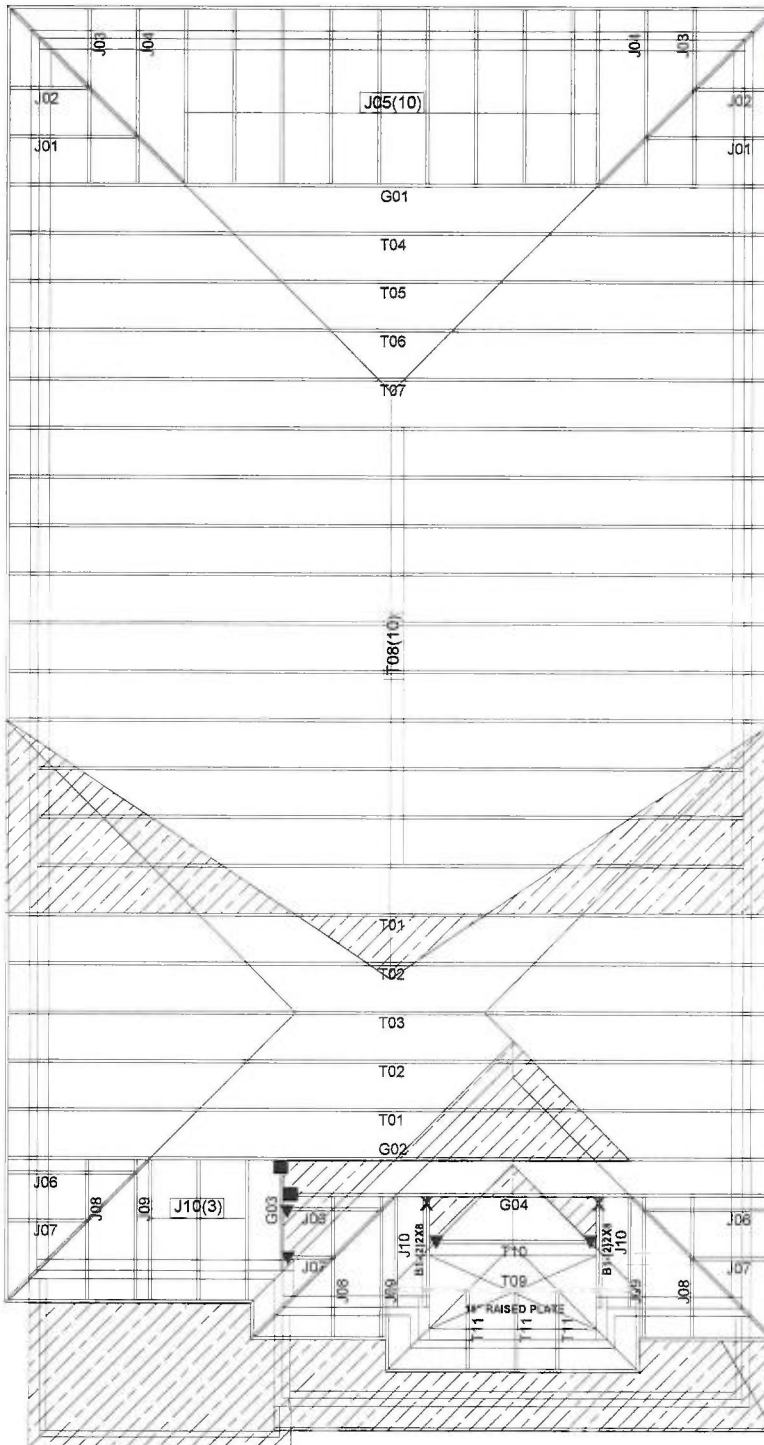
- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

For conventional framing roof / ceilings framing shall conform to OBC.9.23

The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O. Reg. 332/12 as amended and as shown on the engineering calculation page of each component.

The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.

Deflection: $LL = L/360$
 $1.33LL + DL = L/360$

Architectural Drawing Info:
 Date: MAY 22, 2018
 Project number: 17-55
 Model: AMELIA 2



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

- ▼ LUS24
- HGUS26
- LJS26DS
- ✕ HGUS26-2

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

DESIGN CRITERIA
 SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3 PSF
 BOT CH. LL = 9 PSF
 DL = 7 PSF
 TOTAL LOAD = 33.3 PSF
 SPACING = 24" O/C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 986-09
 - TPIC 2011

Model: **AMELIA 2 EL 2**

Customer: **GREEN YORK HOMES**

Project: **GRANELLI HOMES**

Location: **BRAMPTON**

Date: **5/30/2018** Drawn by: **BB**

19-444457-000-00-RR.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-AMELIA 2 EL 2	DRWG NO.
NE0518-224	G01	1	1	TRUSS DESC.		PAGE 4 OF 34

Version 8.210 S Mar 12 2018 MITek Industries, Inc. Thu May 31 16:40:29 2018 Page 2
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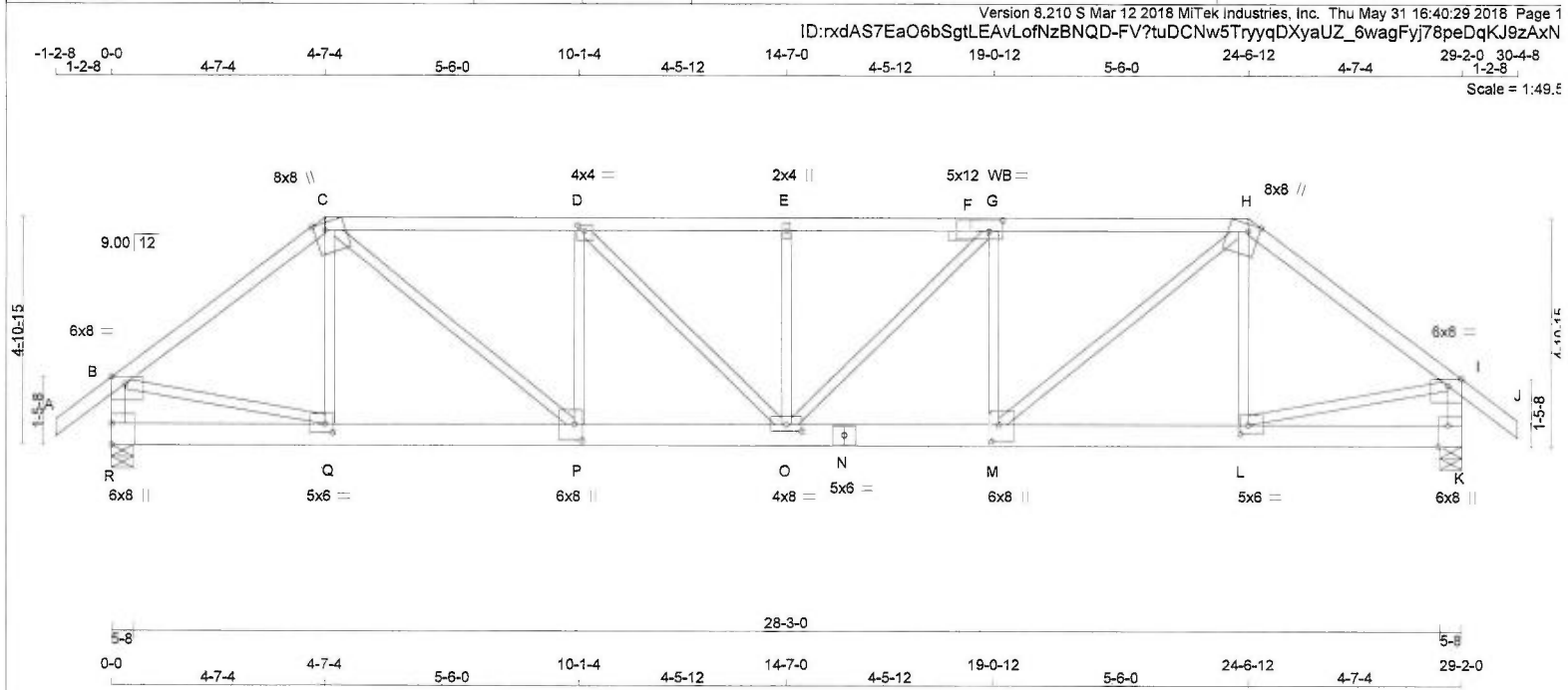
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 23-2-0, AND 348.1 lbs FACTORED DOWN AT 6-0-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.94 (Q) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Version 8.210 S Mar 12 2018 MITek Industries, Inc. Thu May 31 16:40:29 2018 Page 1
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Scale = 1:49.5

L. L. G. A. RULES					BUILDING DESIGNER										DESIGN CRITERIA				
CHORDS		SIZE	LUMBER		DESCR.	BEARINGS										*** SPECIAL LOADS ANALYSIS ***			
A - C	2x4	DRY	2100F	1.8E	SPF	FACTORED				MAXIMUM FACTORED		INPUT		REQRD		GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
C - F	2x4	DRY	2100F	1.8E	SPF	GROSS REACTION				GROSS REACTION		BRG		BRG		LOADS WERE DERIVED FROM USER INPUT			
F - H	2x4	DRY	2100F	1.3E	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	NO FURTHER MODIFICATIONS WERE MADE					
H - J	2x4	DRY	2100F	1.8E	SPF	R	3150	0	3150	0	0	5-8	5-8						
R - B	2x4	DRY	No.2		SPF	K	2256	0	2256	0	0	5-8	5-8						
K - I	2x4	DRY	No.2		SPF											SPECIFIED LOADS:			
R - N	2x6	DRY	No.2		SPF	UNFACTORED REACTIONS										TOP CH. LL = 23.3 PSF			
N - K	2x6	DRY	No.2		SPF	1ST LCASE MAX./MIN. COMPONENT REACTIONS										DL = 3.0 PSF			
ALL WEBS	2x3	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.	LIVE	WIND	DEAD	SOIL	BOT CH. LL = 0.0 PSF					
EXCEPT					R	2209	1555 / 0	0 / 0	0 / 0	0 / 0	0 / 0	654 / 0	0 / 0	DL = 7.0 PSF					
					K	1580	1120 / 0	0 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0	TOTAL LOAD = 33.3 PSF					
DRY: SEASONED LUMBER.																			

PLATES (table is in inches)							BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K							SPACING = 24.0			IN. C/C	
JT	TYPE	PLATES	W	LEN	Y	X	BRACING							LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM				
B	TMVW-p	MT20	6.0	8.0	Edge	3.50	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.							GIRDER TYPE: CPrimeHip				
C	TTWW+m	MT20	8.0	8.0	Edge	3.00	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.							SIDE SETBACK = 4-7-4				
D	TMVW-t	MT20	4.0	4.0	1.50	1.75	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.							END SETBACK = 4-7-4				
E	TMVW+w	MT20	2.0	4.0			LOADING							END WALL WIDTH = 5-8				
F	TSWW-l	MT20	5.0	12.0	3.00	3.50	TOTAL LOAD CASES: (4)							CORNER FRAMING TYPE: CONVENTIONAL				
G							CHORDS							END JACK TYPE: CONVENTIONAL				
H	TTWW+m	MT20	8.0	8.0	Edge	3.00	MAX. FACTORED							APPLIED TO FRONT SIDE				
I	TMVW-p	MT20	6.0	8.0	Edge	3.50	MEMB. FORCE							- ADD'TL LOADS BASED ON 55 % OF GSL.				
K	BMV1+t	MT20	6.0	8.0	Edge	2.50	VERT. LOAD							LOADS APPLIED TO FIRST 10-1-4 OF SPAN				
L	BMVW-t	MT20	5.0	6.0	2.25	2.00	LC1							MEASURED FROM THE LEFT.				
M	BMVW+t	MT20	6.0	8.0	4.25	2.00	MAX. FACTORED							*** NON STANDARD GIRDER ***				
N	BS-t	MT20	5.0	6.0			MEMB. FORCE							ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.				
O	BMVWV-t	MT20	4.0	8.0	1.75	4.00	CS1 (LC)							THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010				
P	BMVW+t	MT20	6.0	8.0	4.25	2.00	UNBRAC							THIS DESIGN COMPLIES WITH:				
Q	BMVW-t	MT20	5.0	6.0	2.25	2.00	MEMB. FORCE							- PART 9 OF OBC 2012, BCBC 2012, ABC 2014				
R	BMV1+t	MT20	6.0	8.0	5.50		MAX. FACTORED							- CSA 086-09				
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.							CS1 (LC)							- TPIC 2011				
WB - INDICATES BLOCKING REQUIRED							MAX. FACTORED							(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED				
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READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 184.3 lbs FACTORED DOWN AT 4-7-4 ON TOP CHORD, AND 1930.7 lbs FACTORED DOWN AT 10-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

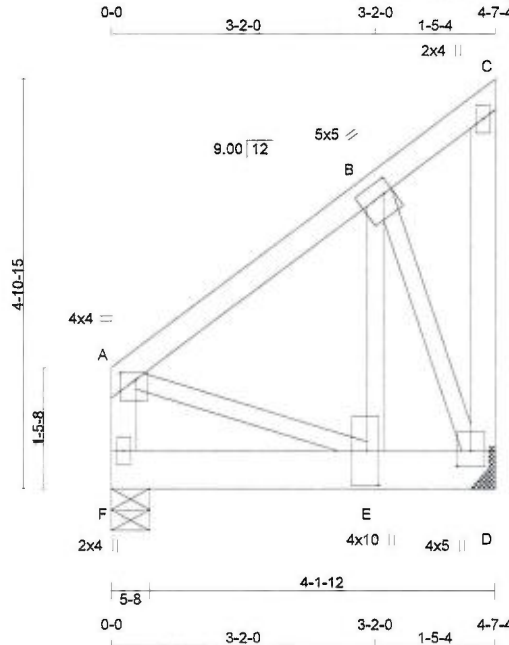
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.72 (Q) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
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 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





Scale = 1:27.6

TOTAL WEIGHT = 28 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-p	MT20	4.0	4.0	1.00	2.25	
B	TMVV-t	MT20	5.0	5.0	2.00	1.25	
C	TMV+p	MT20	2.0	4.0			
D	BMVV+1+p	MT20	4.0	5.0	2.25	2.00	
E	BMVV+t	MT20	4.0	10.0	5.00	1.75	
F	BMV+1+p	MT20	2.0	4.0			

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2438.0 lbs FACTORED DOWN AT 3-2-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1047	0	1047	0
D	1931	0	1931	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	735	514 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0
D	1355	948 / 0	0 / 0	0 / 0	0 / 0	407 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
F-A	-876 / 0	0.0	0.0	0.10 (1)	7.81	A-E	0 / 748	0.19 (1)	
A-B	-879 / 0	-77.4	-77.4	0.13 (1)	6.25	E-B	0 / 2000	0.50 (1)	
B-C	-18 / 0	-77.4	-77.4	0.06 (1)	6.25	B-D	-1832 / 0	0.46 (1)	
D-C	-26 / 0	0.0	0.0	0.01 (1)	7.81				
F-E	0 / 0	-50.1	-50.1	0.21 (1)	10.00				
E-D	0 / 710	-17.5	-17.5	0.31 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
	E	3-2-0	-2438	-2438	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 3-4-8
 END DISTANCE = 3-2-0
 END SPAN CARRIED = 3-4-8
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADD'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.31/1.00 (D-E:1), WB=0.50/1.00 (B-E:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-AMELIA 2 EL	DRWG NO.
NE0518-224	G03	1	1	TRUSS DESC.		

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 16:40:30 2018 Page 2
ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-jhZF5ZC?hPbia5PQ5f5j6CfCR4jbSfgyttZurczAxN

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)

JSI METAL= 0.41 (E) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	PRWG NO.
NE0518-224	G04	1	1	GREEN YORK HOMES-AMELIA 2 EL 2	
				TRUSS DESC.	PAGE 10 OF 34

Version 8.210 S Mar 12 2018 Mittek Industries, Inc. Thu May 31 16:40:31 2018 Page 2
ID:rxdAS7EaO6bSgtLEAvLofNzBNQD-Bt7dJvDdSjjZCF_cfNcyfPCHnUvWB4P56XJRN2zAxN

ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.90/1.00 (H-J:1) ,
WB=0.59/1.00 (B-K:1) , SSI=0.63/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(Psi) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

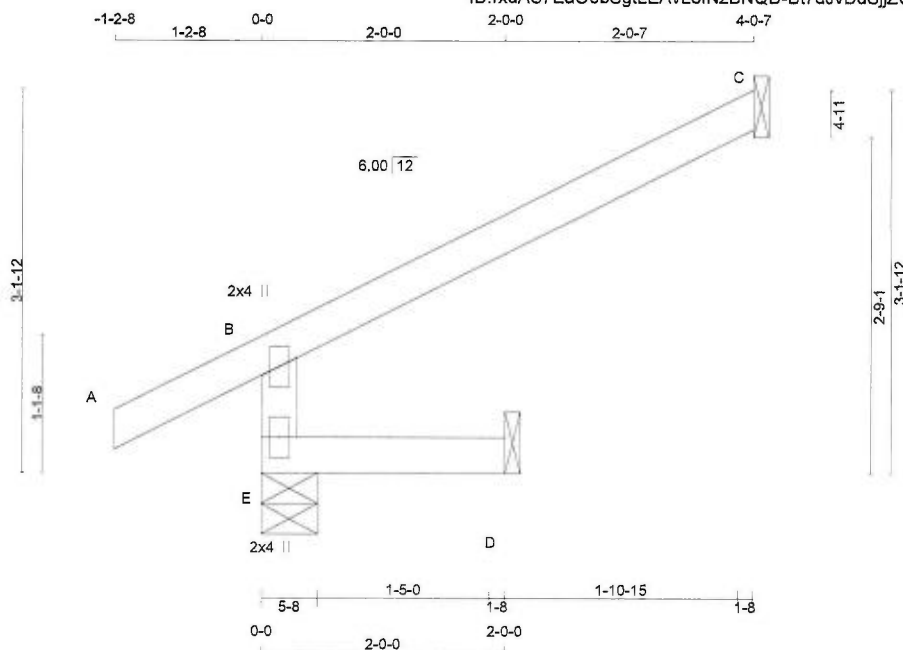
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.80 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1/16" = 1'-0"

TOTAL WEIGHT = 2 X 10 = 20 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	No.2	SPF
A - C	2x4	No.2	SPF
E - D	2x4	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	313	0	313	0	5-8	5-8
C	117	0	117	0	1-8	1-8
D	16	0	17	0	1-8	1-8

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.21/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		SNOW	LIVE	PERM.LIVE	IN-SX			
E	215	177 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

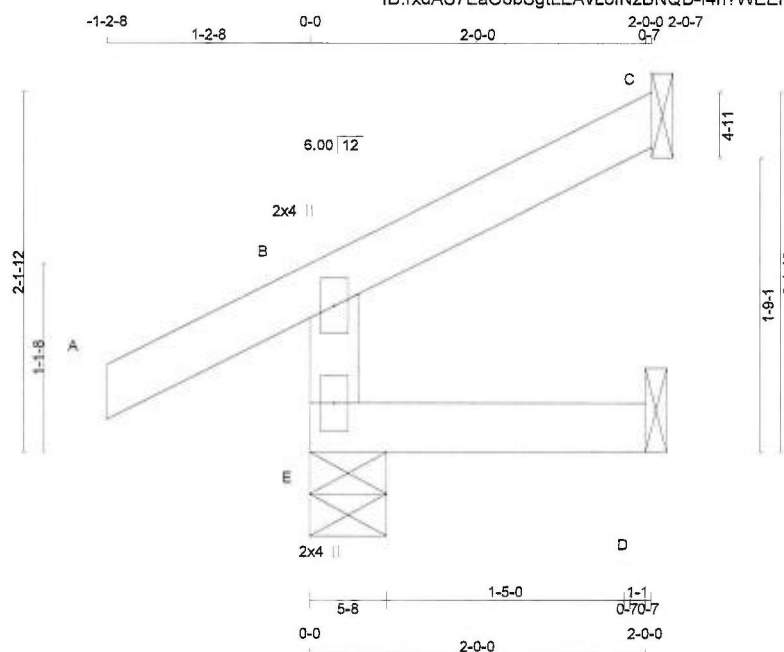
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (CSI (LC))	UNBRAC LENGTH (FR-TO)	MEMB. FORCE (LBS)	MAX. FACTORED (CSI (LC))
FR-TO						
E-B	-294 / 0	0.0	0.0 0.01 (4)	7.81		
A-B	0 / 22	-77.4	-77.4 0.09 (1)	10.00		
B-C	-17 / 0	-77.4	-77.4 0.21 (1)	6.25		
E-D	0 / 0	-17.5	-17.5 0.02 (4)	10.00		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:13.7

TOTAL WEIGHT = $2 \times 7 = 15$ lb

LUMBER

N. L. G. A. RULES		LUMBER	
CHORDS	SIZE		
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BOILING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
E	216	0	216	0	0	5-8
C	59	0	59	0	0	1-8
D	16	0	17	0	0	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	149	118 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
E-B	-197 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00				
B-C	-9 / 0	-77.4	-77.4	0.05 (1)	10.00				
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

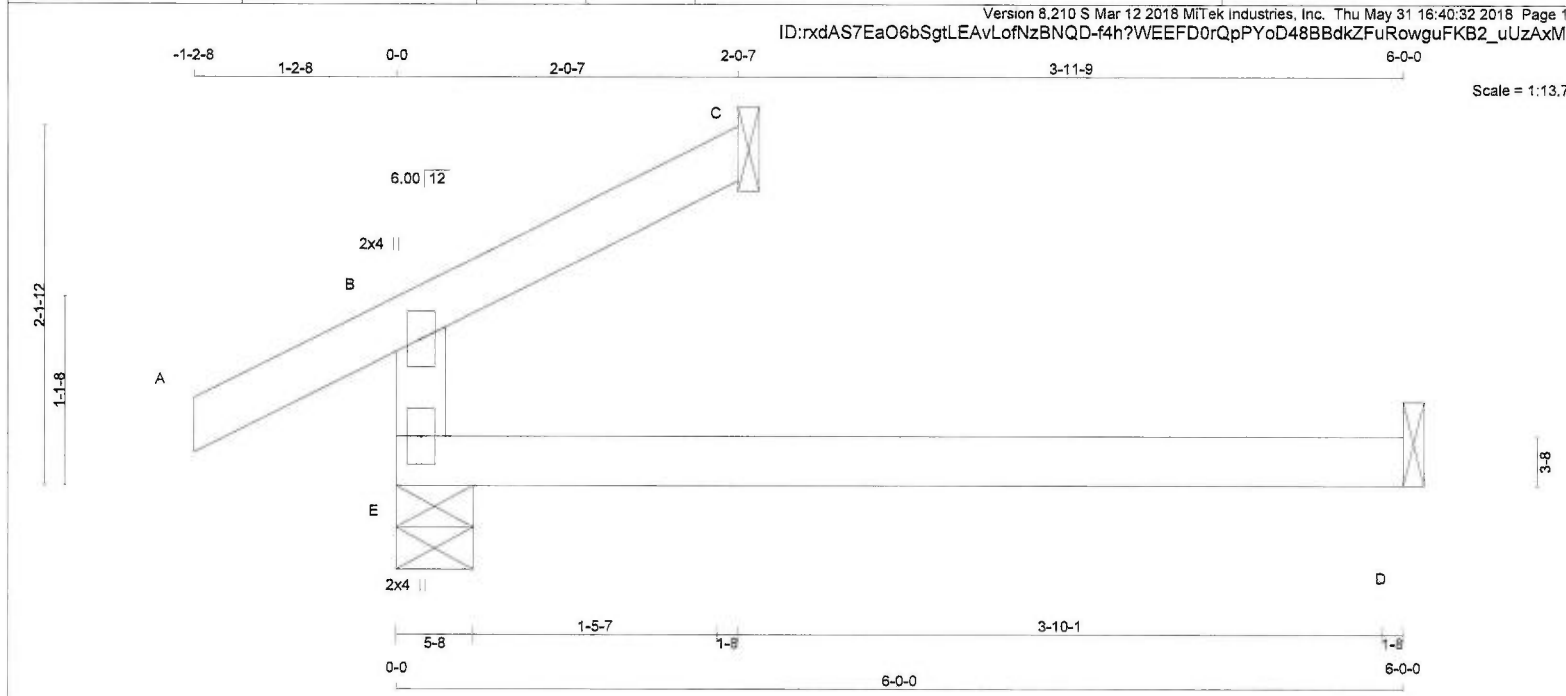
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





LUMBER: N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. E - B 2x4 DRY No.2 E - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.

N. L. G. A. RULES										BUILDING DESIGNER										DESIGN CRITERIA									
CHORDS		SIZE		LUMBER		DESCR.		BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD		SPECIFIED LOADS:											
E -	B	2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		DOWN		HORZ		UPLIFT		BRG		TOP CH.		LL = 23.3 PSF									
A -	C	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					BOT CH.		LL = 0.0 PSF										
E -	D	2x4	DRY	No.2	SPF												DL = 3.0 PSF												
DRY: SEASONED LUMBER.																													
						258	0	258	0	0	5-8	5-8					DL = 7.0 PSF												
						59	0	59	0	0	1-8	1-8					TOTAL LOAD = 33.3 PSF												
						43	0	48	0	0	1-8	1-8																	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS				DESIGN ASSUMPTIONS			
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS		THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
	COMBINED	SNOW	LIVE	THIS DESIGN COMPLIES WITH:			
E	183	118 / 0	0 / 0	- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
C	40	36 / 0	0 / 0	- CSA 086-09			
D	35	0 / 0	0 / 0	- TPIC 2011			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1 MAX)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC1 MAX)	
FR-TO				FR-TO			
E-B	-197 / 0	0.0	0.0 0.11 (4)	A-B	0 / 22	-77.4	0.09 (1)
A-B	0 / 22	-77.4	-77.4 0.09 (1)	B-C	-9 / 0	-77.4	0.04 (1)
B-C	-9 / 0	-77.4	-77.4 0.04 (1)	E-D	0 / 0	-17.5	-17.5 0.13 (4)
E-D	0 / 0	-17.5	-17.5 0.13 (4)				

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.11/1.00 (B-E-4), BC=0.13/1.00 (D-E-4),
WB=0.00/1.00 (n/a-0), SSI=0.08/1.00 (D-E-4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

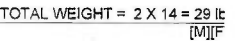
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



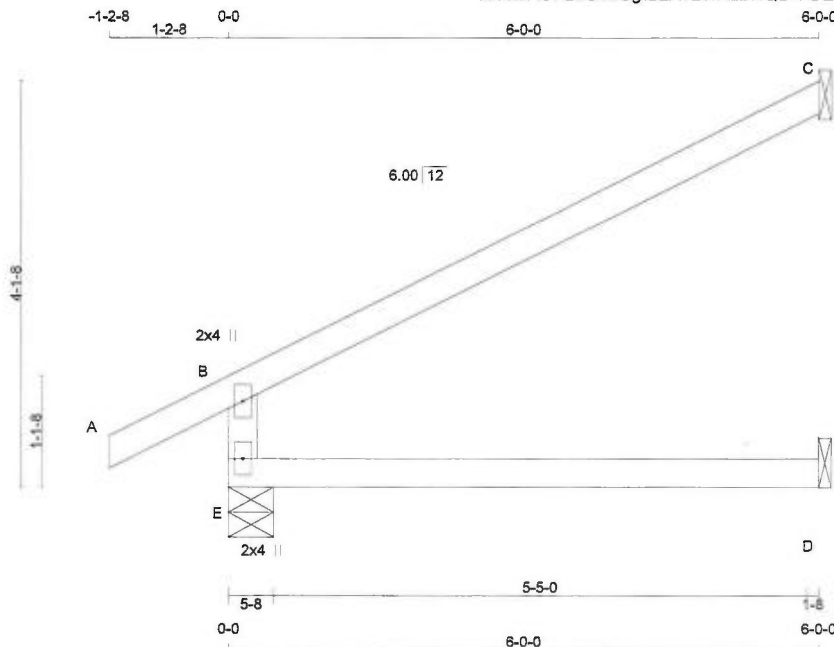


SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)





Scale = 1:23.4

TOTAL WEIGHT = 10 X 17 = 169 lb [M]F

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP
E	450	0	450	0
C	174	0	174	0
D	43	0	43	0

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	314	234 / 0	0 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO											
E-B		-389 / 0	0.0	0.0	0.11 (4)	7.81					
A-B		0 / 22	-77.4	-77.4	0.09 (1)	10.00					
B-C		-26 / 0	-77.4	-77.4	0.36 (1)	6.25					
E-D		0 / 0	-17.5	-17.5	0.13 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

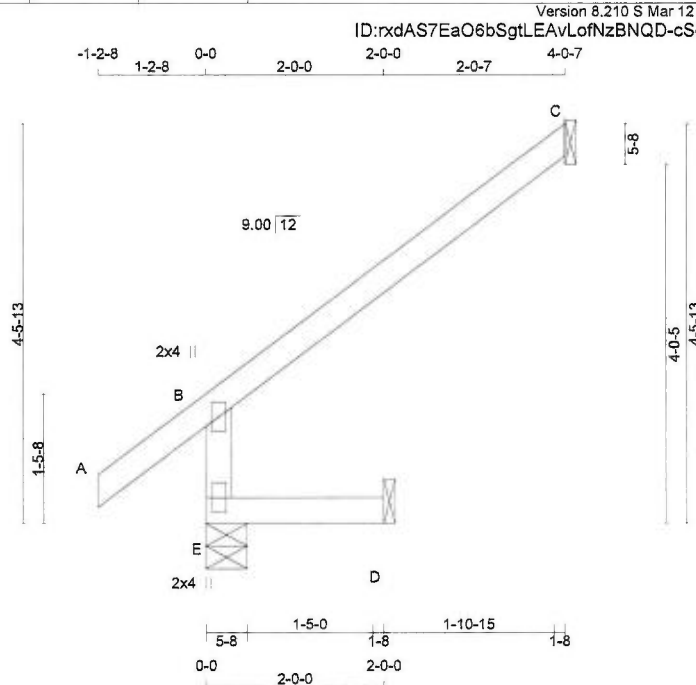
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:25.5

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	314	0	314	0	5-8	5-8
C	117	0	117	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	216	178 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH		MEMB.	WEBS MAX. FACTORED FORCE (LBS)
			FROM	TO		
FR-TO						
E-B	-295 / 0	0.0	0.0	0.01 (4)		7.81
A-B	0 / 30	-77.4	-77.4	0.09 (1)		10.00
B-C	-23 / 0	-77.4	-77.4	0.21 (1)		6.25
E-D	0 / 0	-17.5	-17.5	0.02 (4)		10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.21/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

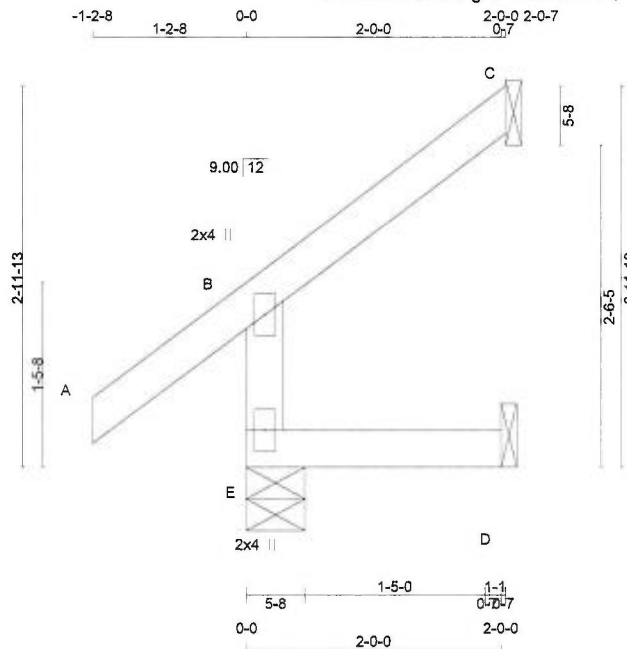
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:18.1

TOTAL WEIGHT = $3 \times 8 = 25 \text{ lb}$
[M]

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	217	0	217	0	0	5-8	5-8
C	60	0	60	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	150	119 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEB S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-198 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00		
B-C	-12 / 0	-77.4	-77.4	0.05 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

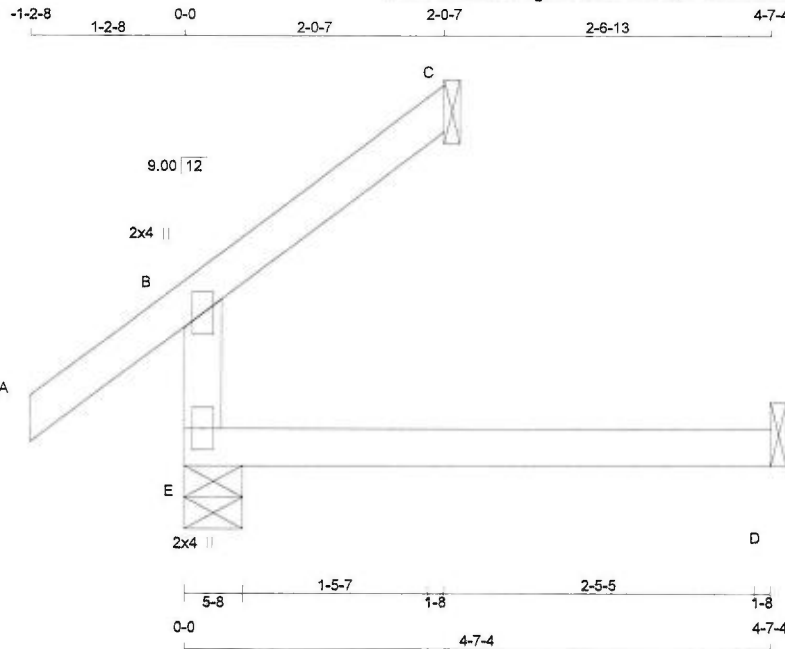
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





Scale = 1:18.1

TOTAL WEIGHT = 3 X 11 = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV+1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	244	0	244	0	5-8	5-8
C	60	0	60	0	1-8	1-8
D	34	0	39	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	172	119 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO			FR-TO			
E-B	-198 / 0	0.0	0.0	0.06 (4)	7.81				
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00				
B-C	-12 / 0	-77.4	-77.4	0.05 (1)	6.25				
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.09/1.00 (A-B:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

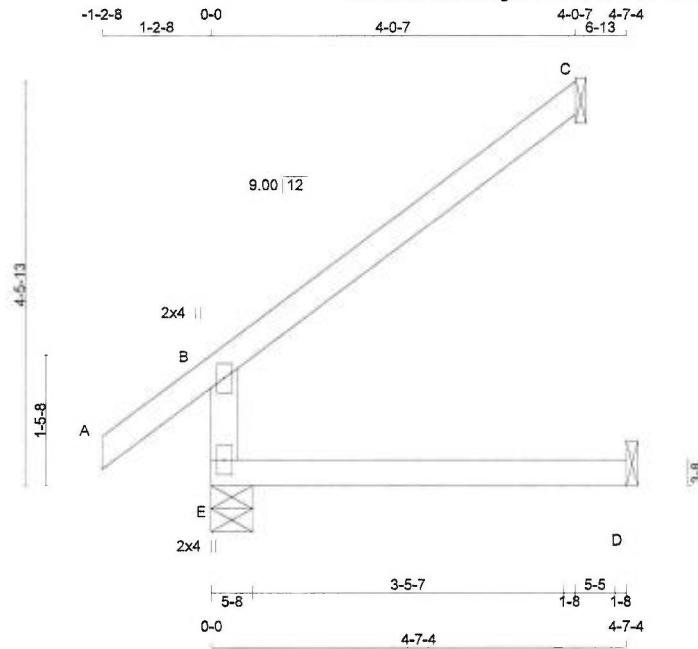
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:25.5

TOTAL WEIGHT = 3 X 14 = 42 lb [M]

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF
CHORDS				
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	341	0	341	0	5-8	5-8	5-8	5-8
C	117	0	117	0	1-8	1-8	1-8	1-8
D	34	0	39	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS						
		1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	238	178 / 0	0 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	MAX. (LC)
FR-TO								
E-B	-295 / 0	0.0	0.0	0.06 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-23 / 0	-77.4	-77.4	0.21 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

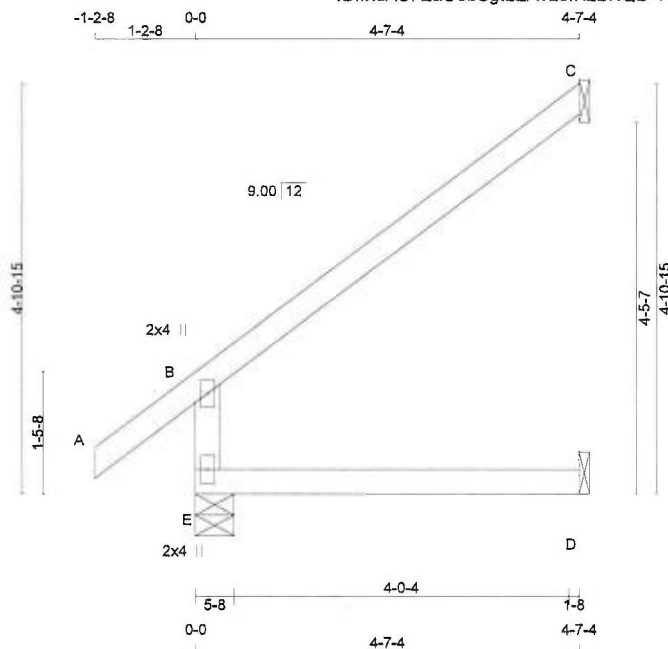
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1/27.6

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	369	0	369	0	0	5-8	5-8	
C	134	0	134	0	0	1-8	1-8	
D	34	0	39	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	91	80 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-323 / 0	0.0	0.0	0.06 (4)	7.81		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00		
B-C	-27 / 0	-77.4	-77.4	0.28 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00		

TOTAL WEIGHT = 5 X 15 = 75 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.28/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

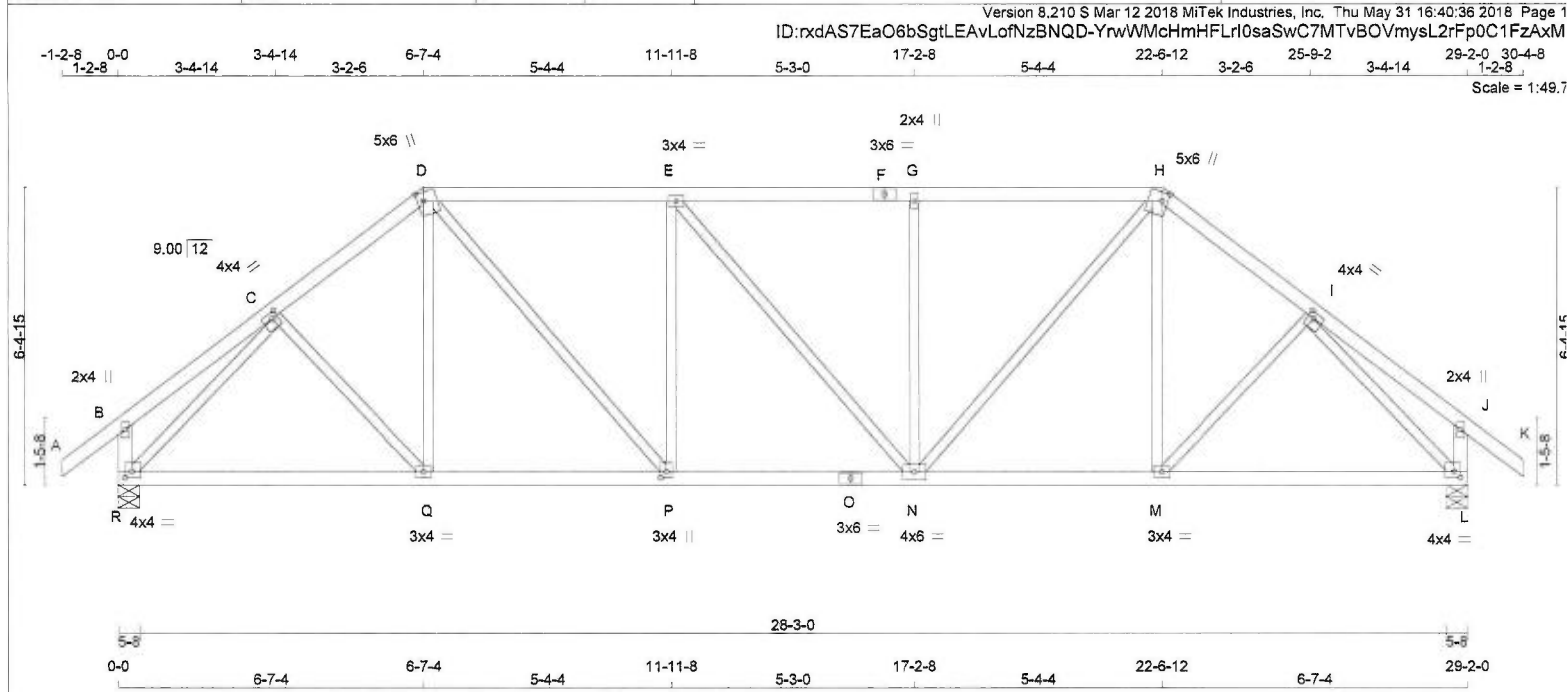
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW+t	MT20	4.0	4.0	1.50	1.50
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+t	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW+w	MT20	2.0	4.0		
H	TTVW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW+t	MT20	4.0	4.0	1.50	1.50
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	1.75
M	BMVW-t	MT20	3.0	4.0		
N	BMVWV-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW+t	MT20	3.0	4.0	1.50	1.50
Q	BMVW-t	MT20	3.0	4.0		
R	BMVW1-t	MT20	4.0	4.0	1.50	1.75



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
R	1484	0	1484	0	5-8	5-8
L	1484	0	1484	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
R	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
L	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	C-Q	0 / 60	0.02 (4)	10.00
B-C	0 / 16	-77.4	-77.4 0.12 (1)	Q-D	0 / 94	0.03 (4)	10.00
C-D	-1481 / 0	-77.4	-77.4 0.15 (1)	D-P	0 / 703	0.16 (1)	5.24
D-E	-1634 / 0	-77.4	-77.4 0.36 (1)	P-E	-443 / 0	0.29 (1)	4.79
E-F	-1633 / 0	-77.4	-77.4 0.35 (1)	E-N	-2 / 0	0.00 (1)	4.79
F-G	-1633 / 0	-77.4	-77.4 0.35 (1)	N-G	-443 / 0	0.29 (1)	4.79
G-H	-1633 / 0	-77.4	-77.4 0.35 (1)	H-N	0 / 701	0.16 (1)	4.80
H-I	-1481 / 0	-77.4	-77.4 0.15 (1)	M-H	0 / 94	0.03 (4)	5.24
I-J	0 / 16	-77.4	-77.4 0.12 (1)	M-I	0 / 60	0.02 (4)	10.00
J-K	0 / 30	-77.4	-77.4 0.09 (1)	R-C	-1672 / 0	0.63 (1)	10.00
R-B	-201 / 0	0.0	0.0 0.02 (1)	I-L	-1672 / 0	0.63 (1)	7.81
L-J	-201 / 0	0.0	0.0 0.02 (1)				7.81
R-Q	0 / 1128	-17.5	-17.5 0.27 (1)				10.00
Q-P	0 / 1170	-17.5	-17.5 0.29 (1)				10.00
P-O	0 / 1634	-17.5	-17.5 0.31 (1)				10.00
O-N	0 / 1634	-17.5	-17.5 0.31 (1)				10.00
N-M	0 / 1171	-17.5	-17.5 0.28 (1)				10.00
M-L	0 / 1128	-17.5	-17.5 0.28 (1)				10.00

TOTAL WEIGHT = 2 X 129 = 258 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.36/1.00 (D-E-1), BC=0.31/1.00 (N-P-1), WB=0.63/1.00 (L-L-1), SS=0.19/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

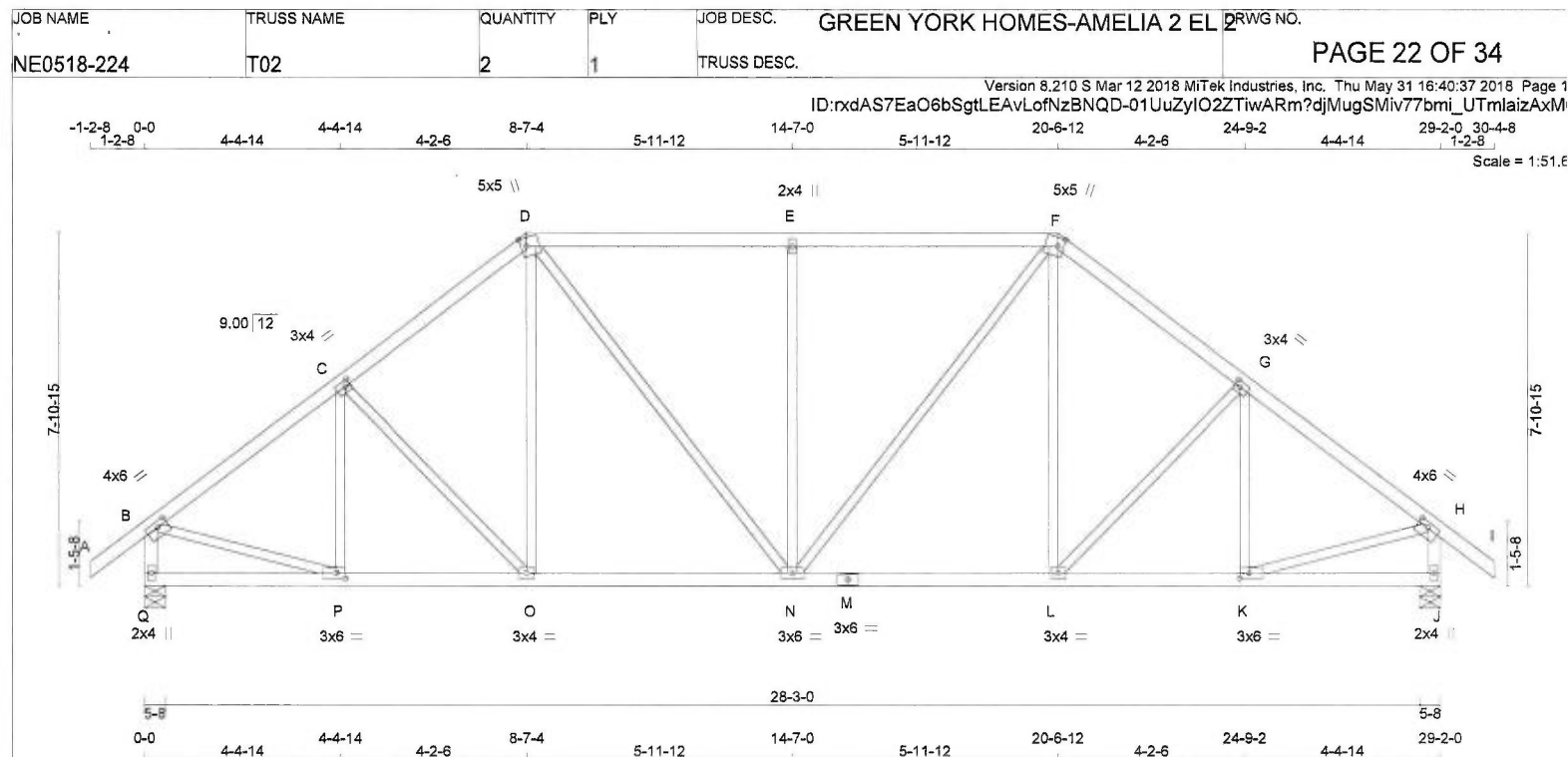
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS EXCEPT 2x3 DRY
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TMVW-t	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	2.25	1.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	3.0	6.0	1.50	2.50
L	BMVW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVWVW-t	MT20	3.0	6.0		
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	3.0	6.0	1.50	2.50
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
Q	1484	0	1484	0	5-8	5-8
J	1484	0	1484	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
Q	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	P-C	-235 / 0	10.00	0.08 (1)
B-C	-1488 / 0	-77.4	-77.4 0.21 (1)	C-O	-146 / 0	5.18	0.10 (1)
C-D	-1411 / 0	-77.4	-77.4 0.20 (1)	O-D	0 / 197	5.29	0.04 (1)
D-E	-1374 / 0	-77.4	-77.4 0.38 (1)	D-N	0 / 427	5.10	0.10 (1)
E-F	-1374 / 0	-77.4	-77.4 0.38 (1)	N-E	-566 / 0	5.10	0.67 (1)
F-G	-1411 / 0	-77.4	-77.4 0.20 (1)	N-F	0 / 427	5.29	0.10 (1)
G-H	-1488 / 0	-77.4	-77.4 0.21 (1)	L-F	0 / 197	5.18	0.04 (1)
H-I	0 / 30	-77.4	-77.4 0.09 (1)	L-G	-146 / 0	10.00	0.10 (1)
Q-B	-1449 / 0	0.0	0.0 0.15 (1)	K-G	-235 / 0	6.79	0.08 (1)
J-H	-1449 / 0	0.0	0.0 0.15 (1)	B-P	0 / 1248	6.79	0.28 (1)
Q-P	0 / 0	-17.5	-17.5 0.07 (4)	K-H	0 / 1248	6.79	0.28 (1)
P-O	0 / 1209	-17.5	-17.5 0.25 (1)				
O-N	0 / 1110	-17.5	-17.5 0.24 (1)				
N-M	0 / 1110	-17.5	-17.5 0.24 (1)				
M-L	0 / 1110	-17.5	-17.5 0.24 (1)				
L-K	0 / 1209	-17.5	-17.5 0.25 (1)				
K-J	0 / 0	-17.5	-17.5 0.07 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.38/1.00 (D-E:1), BC=0.25/1.00 (O-P:1)
WB=0.67/1.00 (E-N:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

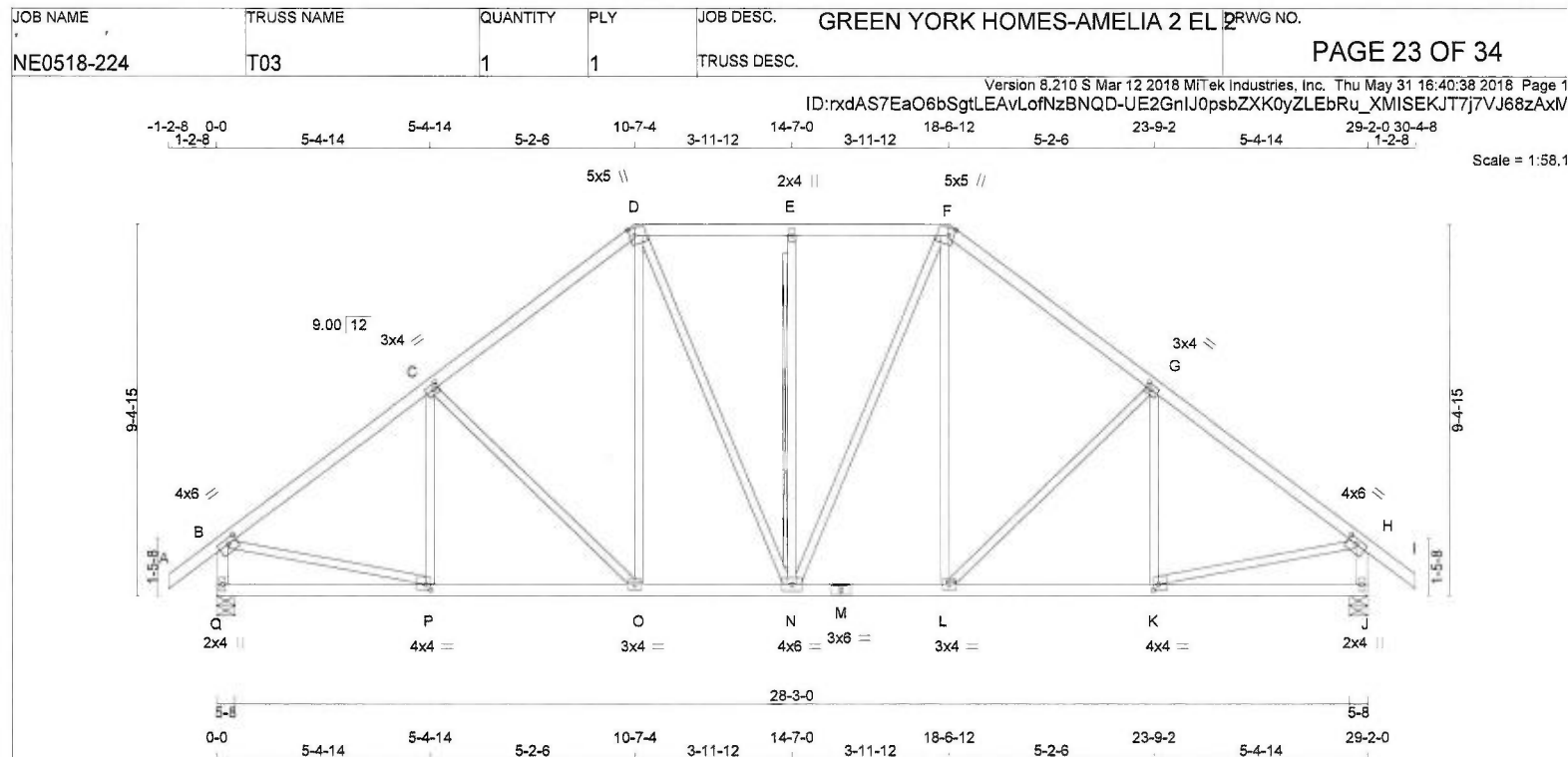
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



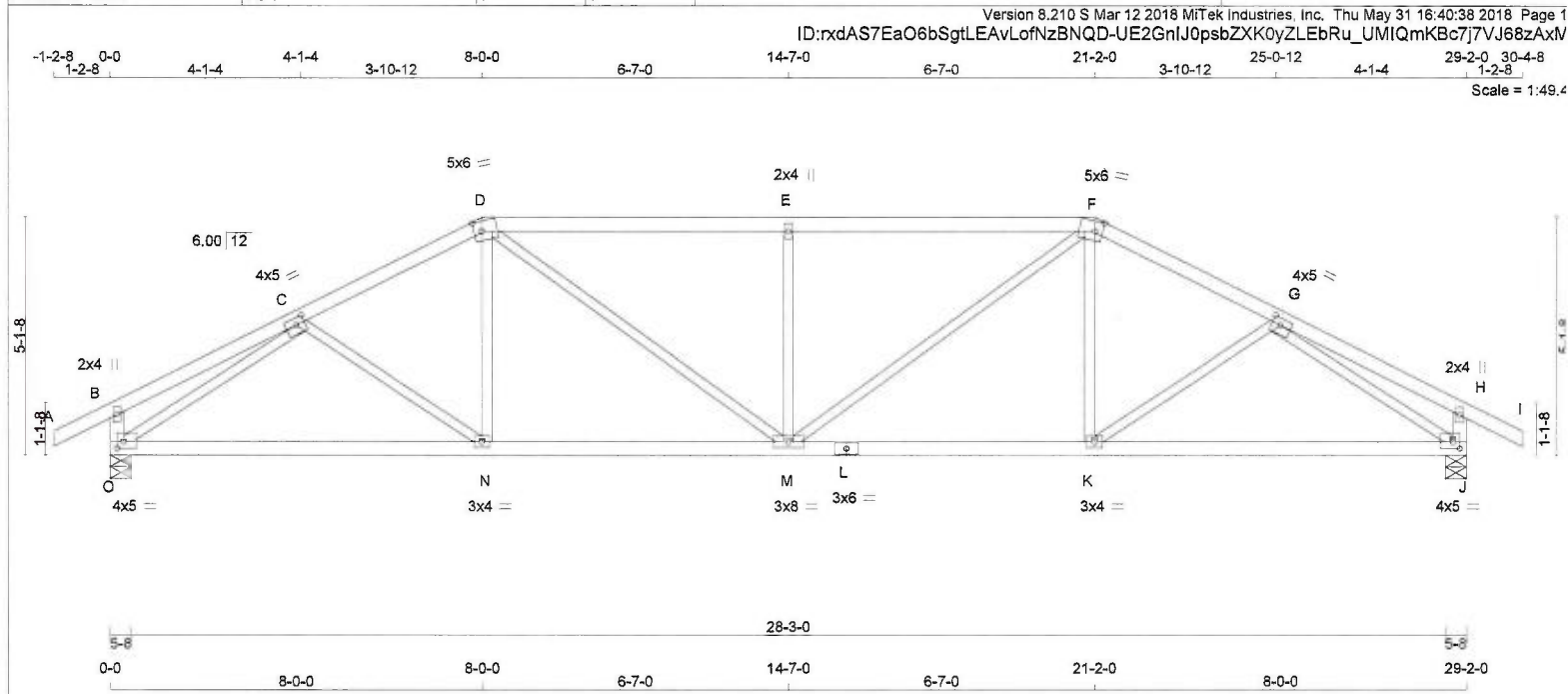


LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 144 [M]			
N. L. G. A. RULES								DESIGN CRITERIA			
CHORDS SIZE LUMBER DESCR.				BEARINGS				SPECIFIED LOADS:			
A - D 2x4 DRY No.2 SPF				FACTORED MAXIMUM FACTORED INPUT REQD				TOP CH. LL = 23.3 PSF			
D - F 2x4 DRY No.2 SPF				GROSS REACTION GROSS REACTION BRG BRG				DL = 3.0 PSF			
F - I 2x4 DRY No.2 SPF				JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX				BOT CH. LL = 0.0 PSF			
Q - B 2x4 DRY No.2 SPF				Q 1484 0 1484 0 0 5-8 5-8				DL = 7.0 PSF			
J - H 2x4 DRY No.2 SPF				J 1484 0 1484 0 0 5-8 5-8				TOTAL LOAD = 33.3 PSF			
Q - M 2x4 DRY No.2 SPF								SPACING = 24.0 IN. C/C			
M - J 2x4 DRY No.2 SPF								LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
ALL WEBS 2x3 DRY No.2 SPF				UNFACTORED REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
EXCEPT				1ST LCASE MAX./MIN. COMPONENT REACTIONS				THIS DESIGN COMPLIES WITH:			
DRY: SEASONED LUMBER.				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL				- PART 9 OF OBC 2012 . BCBC 2012 . ABC 2014			
				Q 1039 740 / 0 0 / 0 0 / 0 0 / 0 299 / 0 0 / 0				- CSA 086-09			
				J 1039 740 / 0 0 / 0 0 / 0 0 / 0 299 / 0 0 / 0				- TPIC 2011			
				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J				(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
				BRACING				ALLOWABLE DEFL.(LL)= L/360 (0.97")			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT.				CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				ALLOWABLE DEFL.(TL)= L/360 (0.97")			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.				CALCULATED VERT. DEFL.(TL)= L/ 999 (0.09")			
				2x4 DRY SPF No.2 T-BRACE AT E-N				CSI: TC=0.32/1.00 (G-H:1) , BC=0.26/1.00 (K-L:1) ,			
				FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.				WB=0.31/1.00 (G-L:1) , SSI=0.16/1.00 (G-H:1)			
				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				LOADING				COMPANION LIVE LOAD FACTOR = 0.50			
				TOTAL LOAD CASES: (4)				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
				CHORDS				WEBS			
				MAX. FACTORED FACTORED				MAX. FACTORED			
				MEMB. FORCE (LBS) VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC LENGTH FR-TO				MEMB. FORCE (LBS) MAX. CSI (LC)			
				FR-TO				FR-TO			
				A-B 0 / 30 -77.4 -77.4 0.09 (1) 10.00 P-C -154 / 45 0.07 (1)							
				B-C -1513 / 0 -77.4 -77.4 0.32 (1) 5.01 C-O -290 / 0 0.31 (1)							
				C-D -1315 / 0 -77.4 -77.4 0.31 (1) 5.30 O-D 0 / 284 0.06 (1)							
				D-E -1123 / 0 -77.4 -77.4 0.16 (1) 5.82 D-N 0 / 234 0.05 (1)							
				E-F -1123 / 0 -77.4 -77.4 0.16 (1) 5.82 N-E -368 / 0 0.24 (1)							
				F-G -1315 / 0 -77.4 -77.4 0.31 (1) 5.30 N-F 0 / 234 0.05 (1)							
				G-H -1513 / 0 -77.4 -77.4 0.32 (1) 5.01 L-F 0 / 284 0.06 (1)							
				H-I 0 / 30 -77.4 -77.4 0.09 (1) 10.00 L-G -290 / 0 0.31 (1)							
				Q-B -1444 / 0 0.0 0.0 0.15 (1) 6.80 K-G -154 / 45 0.07 (1)							
				J-H -1444 / 0 0.0 0.0 0.15 (1) 6.80 B-P 0 / 1260 0.28 (1)							
								K-H 0 / 1260 0.28 (1)			
				Q-P 0 / 0 -17.5 -17.5 0.12 (4) 10.00							
				P-O 0 / 1234 -17.5 -17.5 0.26 (1) 10.00							
				O-N 0 / 1030 -17.5 -17.5 0.21 (1) 10.00							
				N-M 0 / 1030 -17.5 -17.5 0.21 (1) 10.00							
				M-L 0 / 1030 -17.5 -17.5 0.21 (1) 10.00							
				L-K 0 / 1234 -17.5 -17.5 0.26 (1) 10.00							
				K-J 0 / 0 -17.5 -17.5 0.12 (4) 10.00							



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	5.0	1.75	2.25
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMWW-t	MT20	4.0	5.0	1.75	2.25
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	5.0	1.75	1.75
K	BMWW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	3.0	8.0		
N	BMWW-t	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	5.0	1.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
O	1482	0	1482	0
J	1482	0	1482	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	WIND	DEAD	SOIL
O	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX CSI (LC)	UNBRAC LENGTH
FR-TO				FR-TO			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	C-N	-29 / 57	0.02 (4)	10.00
B-C	0 / 14	-77.4	-77.4 0.17 (1)	N-D	0 / 169	0.05 (4)	10.00
C-D	-1898 / 0	-77.4	-77.4 0.18 (1)	D-M	0 / 605	0.14 (1)	4.74
D-E	-2175 / 0	-77.4	-77.4 0.52 (1)	M-E	-626 / 0	0.24 (1)	4.09
E-F	-2175 / 0	-77.4	-77.4 0.52 (1)	M-F	0 / 605	0.14 (1)	4.09
F-G	-1898 / 0	-77.4	-77.4 0.18 (1)	K-F	0 / 169	0.05 (4)	4.74
G-H	0 / 14	-77.4	-77.4 0.17 (1)	K-G	-29 / 57	0.02 (4)	10.00
H-I	0 / 22	-77.4	-77.4 0.09 (1)	O-C	-2088 / 0	0.82 (1)	10.00
O-B	-223 / 0	0.0	0.0 0.02 (1)	G-J	-2088 / 0	0.82 (1)	7.81
J-H	-223 / 0	0.0	0.0 0.02 (1)				7.81
O-N	0 / 1708	-17.5	-17.5 0.41 (1)				10.00
N-M	0 / 1688	-17.5	-17.5 0.41 (1)				10.00
M-L	0 / 1688	-17.5	-17.5 0.41 (1)				10.00
L-K	0 / 1688	-17.5	-17.5 0.41 (1)				10.00
K-J	0 / 1708	-17.5	-17.5 0.41 (1)				10.00

TOTAL WEIGHT = 112 lb [M]F

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.52/1.00 (D-E:1), BC=0.41/1.00 (N-C:1), WB=0.82/1.00 (C-O:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

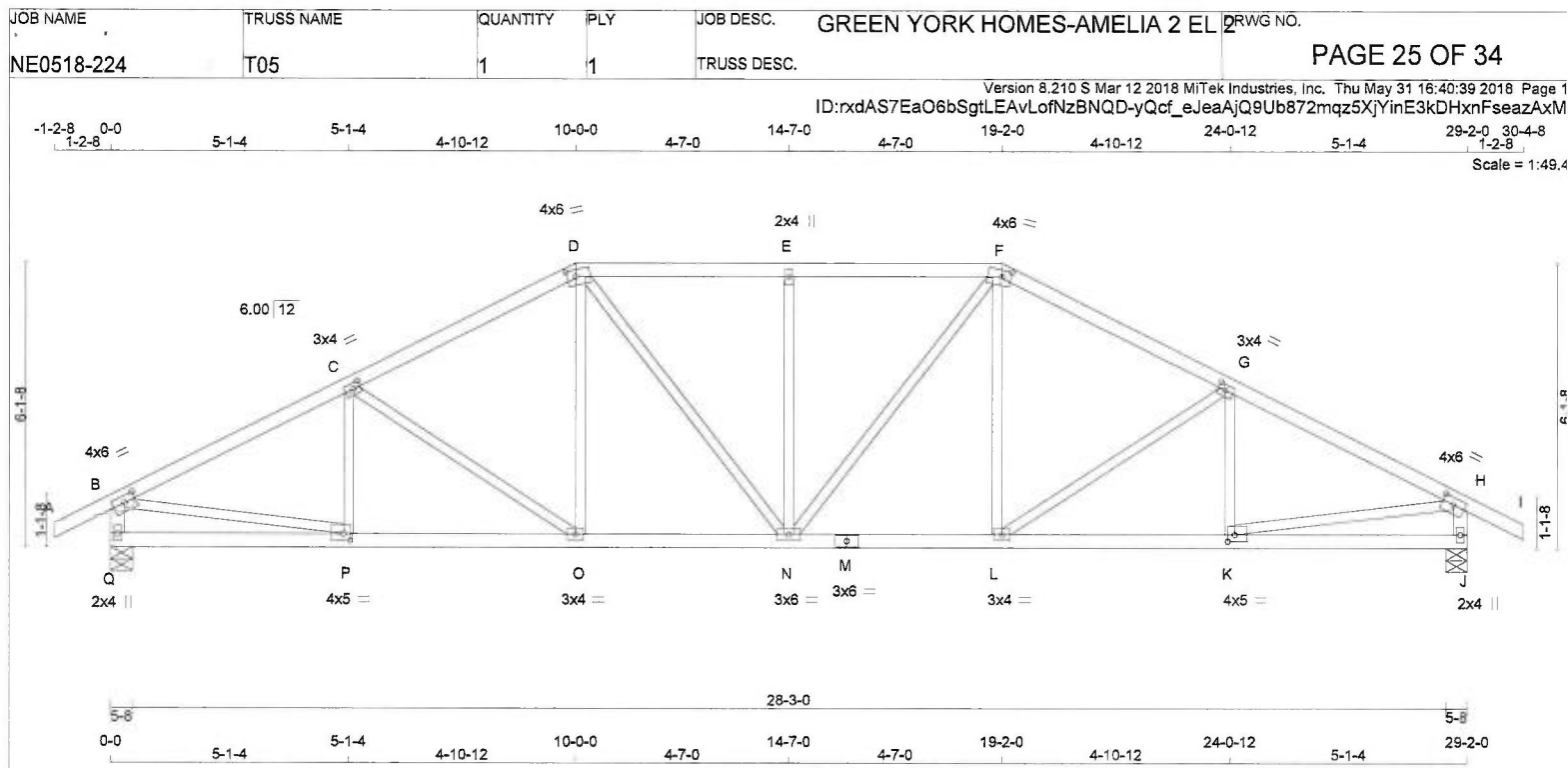
JSI GRIP= 0.90 (F) (INPUT = 0.90)

JSI METAL= 0.61 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMVW-t	MT20	3.0	4.0	1.50	1.75
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	5.0	1.75	1.75
L	BMVW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	3.0	6.0		
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	4.0	5.0	1.75	1.75
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
Q	1482	0	1482	0	5-8	5-8
J	1482	0	1482	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
Q	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH	MAX. FACTORED MAX CSI (LC)
FR-TO				FR-TO			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	P-C	-197 / 26	10.00	0.04 (1)
B-C	-1995 / 0	-77.4	-77.4 0.30 (1)	C-O	-288 / 0	10.00	0.18 (1)
C-D	-1769 / 0	-77.4	-77.4 0.28 (1)	O-D	0 / 249	10.00	0.06 (1)
D-E	-1765 / 0	-77.4	-77.4 0.24 (1)	D-N	0 / 319	10.00	0.07 (1)
E-F	-1765 / 0	-77.4	-77.4 0.24 (1)	N-E	-430 / 0	10.00	0.25 (1)
F-G	-1769 / 0	-77.4	-77.4 0.28 (1)	N-F	0 / 319	10.00	0.07 (1)
G-H	-1995 / 0	-77.4	-77.4 0.30 (1)	L-F	0 / 249	10.00	0.06 (1)
H-I	0 / 22	-77.4	-77.4 0.09 (1)	L-G	-288 / 0	10.00	0.18 (1)
Q-B	-1442 / 0	0.0	0.0 0.15 (1)	K-G	-197 / 26	10.00	0.04 (1)
J-H	-1442 / 0	0.0	0.0 0.15 (1)	B-P	0 / 1825	10.00	0.41 (1)
				K-H	0 / 1825	10.00	0.41 (1)
Q-P	0 / 0	-17.5	-17.5 0.10 (4)				
P-O	0 / 1802	-17.5	-17.5 0.34 (1)				
O-N	0 / 1567	-17.5	-17.5 0.30 (1)				
N-M	0 / 1567	-17.5	-17.5 0.30 (1)				
M-L	0 / 1567	-17.5	-17.5 0.30 (1)				
L-K	0 / 1802	-17.5	-17.5 0.34 (1)				
K-J	0 / 0	-17.5	-17.5 0.10 (4)				

TOTAL WEIGHT = 121 lb [M]F

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.30/1.00 (B-C:1), BC=0.34/1.00 (O-P:1), WB=0.41/1.00 (B-P:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

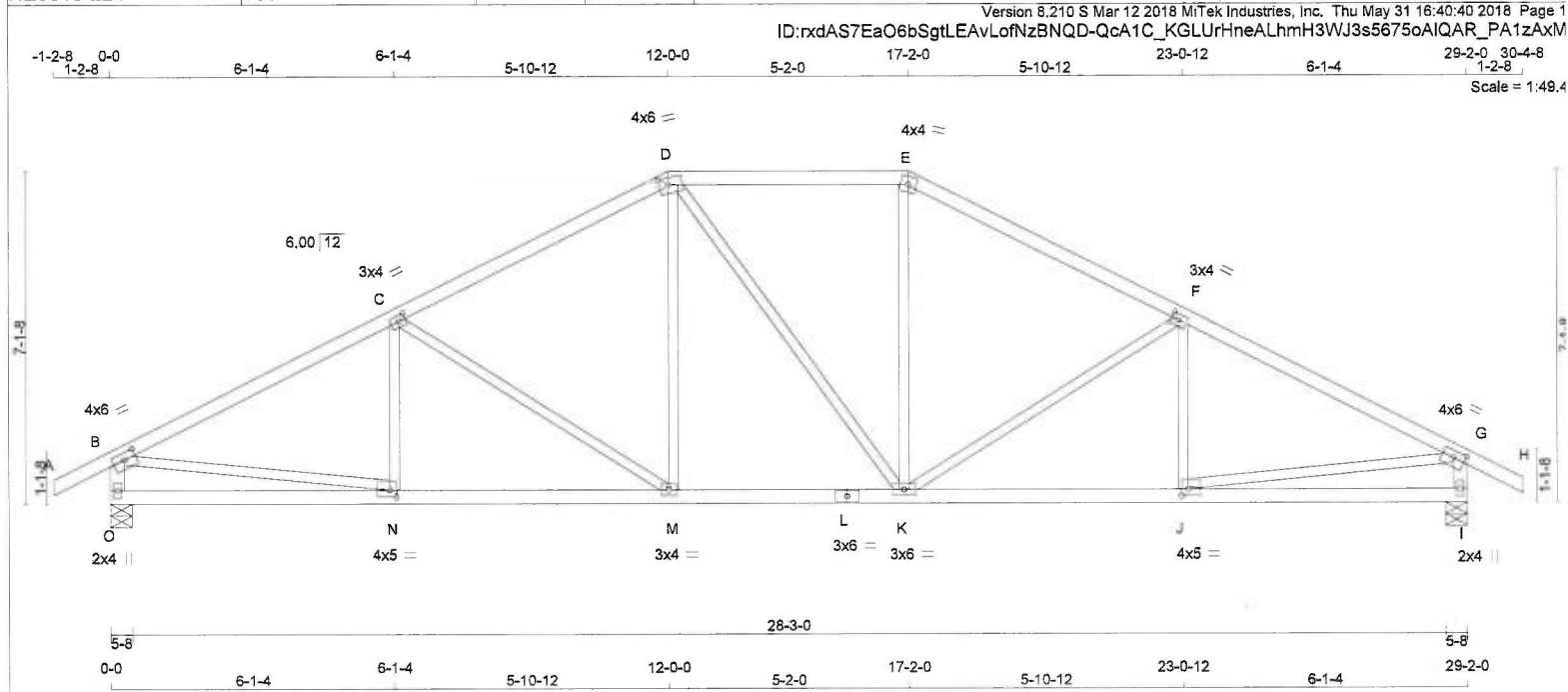
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.54 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 16:40:40 2018 Page 1
ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-QcA1C_KGLUrHneALhmH3WJ3s5675oAIQAR_PA1zAxM
Scale = 1:48.4

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF		GROSS REACTION	GROSS REACTION
D - E	2x4	DRY	No.2	SPF		DOWN	UP
E - H	2x4	DRY	No.2	SPF		VERT	HORZ
O - B	2x4	DRY	No.2	SPF		1482	0
I - G	2x4	DRY	No.2	SPF		1482	0
O - L	2x4	DRY	No.2	SPF		1482	0
L - I	2x4	DRY	No.2	SPF		1482	0

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX./MIN.	COMPONENT	REACTIONS	SPECIFIED LOADS:			
JT	COMBINED	SNOW	LIVE	TOP CH.	LL	=	23.3 PSF
O	1038	739 / 0	0 / 0	DL	=	3.0 PSF	
I	1038	739 / 0	0 / 0	BOT CH.	LL	=	0.0 PSF
				DL	=	7.0 PSF	
				TOTAL LOAD	=	33.3 PSF	

ALL WEBS EXCEPT: DRY: SEASONED LUMBER. TOTAL WEIGHT = 118 lb [M]F

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMVW-t MT20 4.0 6.0 1.75 3.00
C TMVW-t MT20 3.0 4.0 1.50 1.75
D TTW-m MT20 4.0 6.0 1.75 2.25
E TTW-m MT20 4.0 4.0
F TMVW-t MT20 3.0 4.0 1.50 1.75
G TMVW-t MT20 4.0 6.0 1.75 3.00
I BMV1+p MT20 2.0 4.0
J BMVW-t MT20 4.0 5.0 1.75 1.75
K BMVW-t MT20 3.0 6.0
L BS-t MT20 3.0 6.0
M BMVW-t MT20 3.0 4.0
N BMVW-t MT20 4.0 5.0 1.75 1.75
O BMV1+p MT20 2.0 4.0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)
FR-TO	A-B	0 / 22	-77.4	FR-TO	N-C	-128 / 64	0.03 (1)
	B-C	-2015 / 0	-77.4		C-M	-465 / 0	0.46 (1)
	C-D	-1628 / 0	-77.4		M-D	0 / 346	0.08 (1)
	D-E	-1439 / 0	-77.4		D-K	0 / 0	0.00 (1)
	E-F	-1628 / 0	-77.4		K-E	0 / 347	0.08 (1)
	F-G	-2015 / 0	-77.4		K-F	-464 / 0	0.46 (1)
	G-H	0 / 22	-77.4		J-F	-129 / 63	0.03 (1)
	O-B	-1437 / 0	0.0		B-N	0 / 1840	0.41 (1)
	I-G	-1436 / 0	0.0		J-G	0 / 1840	0.41 (1)

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")
CSI: TC=0.44/1.00 (B-C:1) , BC=0.36/1.00 (M-N:1) , WB=0.46/1.00 (C-M:1) , SSI=0.20/1.00 (F-G:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10
COMPANION LIVE LOAD FACTOR = 0.50

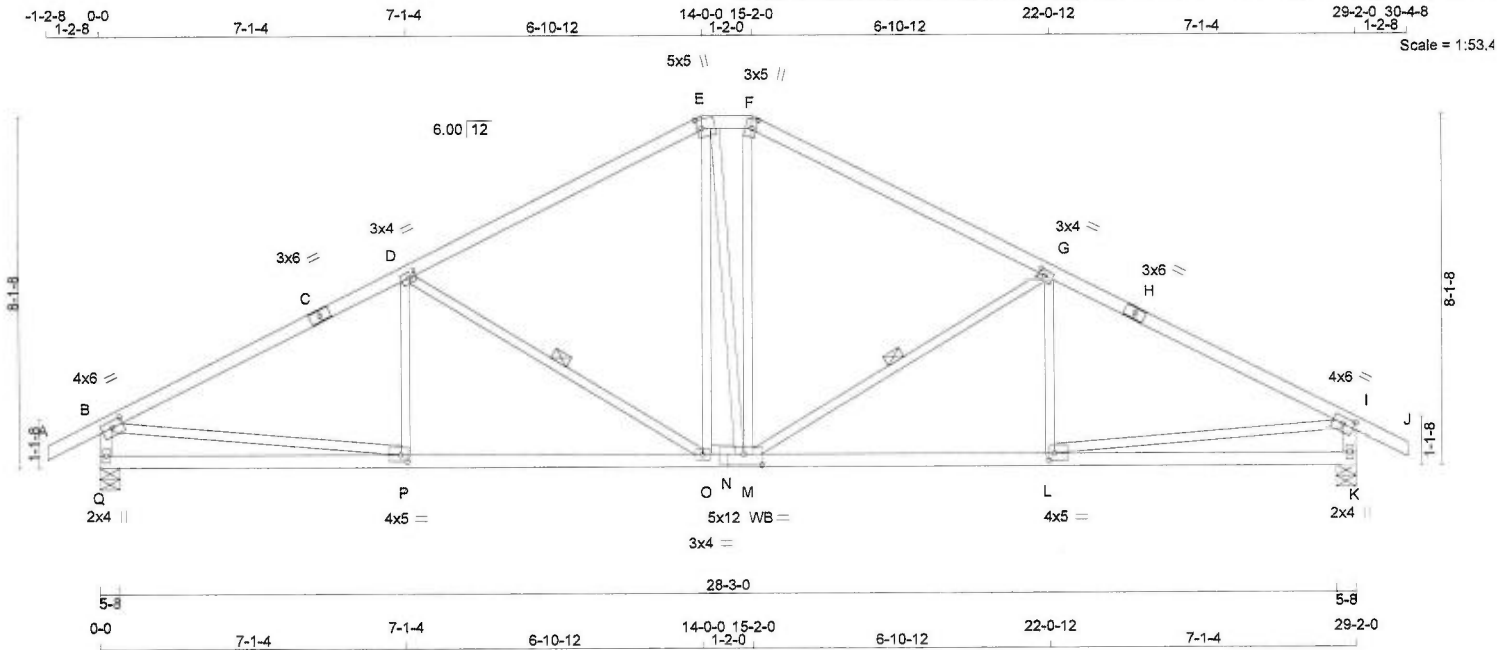
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION (PL)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 125 lb

LUMBER		LUMBER		
N. L. G. A. RULES				
CHORDS	SIZE			DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)		PLATES	W	LEN	Y	X
JT	TYPE	PLATES				
B	BMWV-t	MT20	4.0	6.0	1.75	3.00
C	TS-t	MT20	3.0	6.0		
D	BMWV-t	MT20	3.0	4.0	1.50	1.75
E	TTWV+m	MT20	5.0	5.0	2.50	1.25
F	TTW+m	MT20	3.0	5.0	2.50	1.25
G	BMWV-t	MT20	3.0	4.0	1.50	1.75
H	TS-t	MT20	3.0	6.0		
I	BMWV-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWV-t	MT20	4.0	5.0	2.00	1.50
M						
N						
O	BSWWW-I	MT20	5.0	12.0	3.00	5.00
N	BMWV-t	MT20	3.0	4.0		
P	BMWV-t	MT20	4.0	5.0	1.75	1.75
Q	BMV1+p	MT20	2.0	4.0		

WB - INDICATES BLOCKING REQUIRED



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1482	0	1482	0	0	5-8	5-8
K	1482	0	1482	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
K	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, G-M. DBS = 20-0-0 . CBF = 79 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S). 2 FOR 1x4, 2x4, 2x5. 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A - B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	P - D	-70 / 98 0.03 (4)
B - C	-2011 / 0	-77.4	-77.4	0.62 (1)	4.11	D - O	-633 / 0 0.29 (1)
C - D	-2011 / 0	-77.4	-77.4	0.62 (1)	4.11	O - E	0 / 373 0.08 (1)
D - E	-1470 / 0	-77.4	-77.4	0.54 (1)	4.74	E - M	0 / 27 0.01 (1)
E - F	-1296 / 0	-77.4	-77.4	0.03 (1)	5.66	M - F	0 / 401 0.09 (1)
F - G	-1473 / 0	-77.4	-77.4	0.55 (1)	4.74	M - G	-627 / 0 0.29 (1)
G - H	-2009 / 0	-77.4	-77.4	0.62 (1)	4.11	L - G	-75 / 96 0.03 (4)
H - I	-2009 / 0	-77.4	-77.4	0.62 (1)	4.11	B - P	0 / 1836 0.41 (1)
I - J	0 / 22	-77.4	-77.4	0.09 (1)	10.00	L - I	0 / 1835 0.41 (1)
Q - B	-1431 / 0	0.0	0.0	0.14 (1)	6.82		
K - I	-1430 / 0	0.0	0.0	0.14 (1)	6.82		
Q - P	0 / 0	-17.5	-17.5	0.22 (4)	10.00		
P - O	0 / 1824	-17.5	-17.5	0.40 (1)	10.00		
Q - N	0 / 1292	-17.5	-17.5	0.30 (1)	10.00		
N - M	0 / 1292	-17.5	-17.5	0.30 (1)	10.00		
M - L	0 / 1823	-17.5	-17.5	0.39 (1)	10.00		
L - K	0 / 0	-17.5	-17.5	0.21 (4)	10.00		

DESIGN CRITERIA

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SPECIFIED LOADS:
TOP CH.  LL = 23.3 PSF
          DL = 3.0 PSF
BOT CH.  LL = 0.0 PSF
          DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

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SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.62/1.00 (B-D:1), BC=0.40/1.00 (O-P:1),
WB=0.41/1.00 (B-P:1), SSI=0.24/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

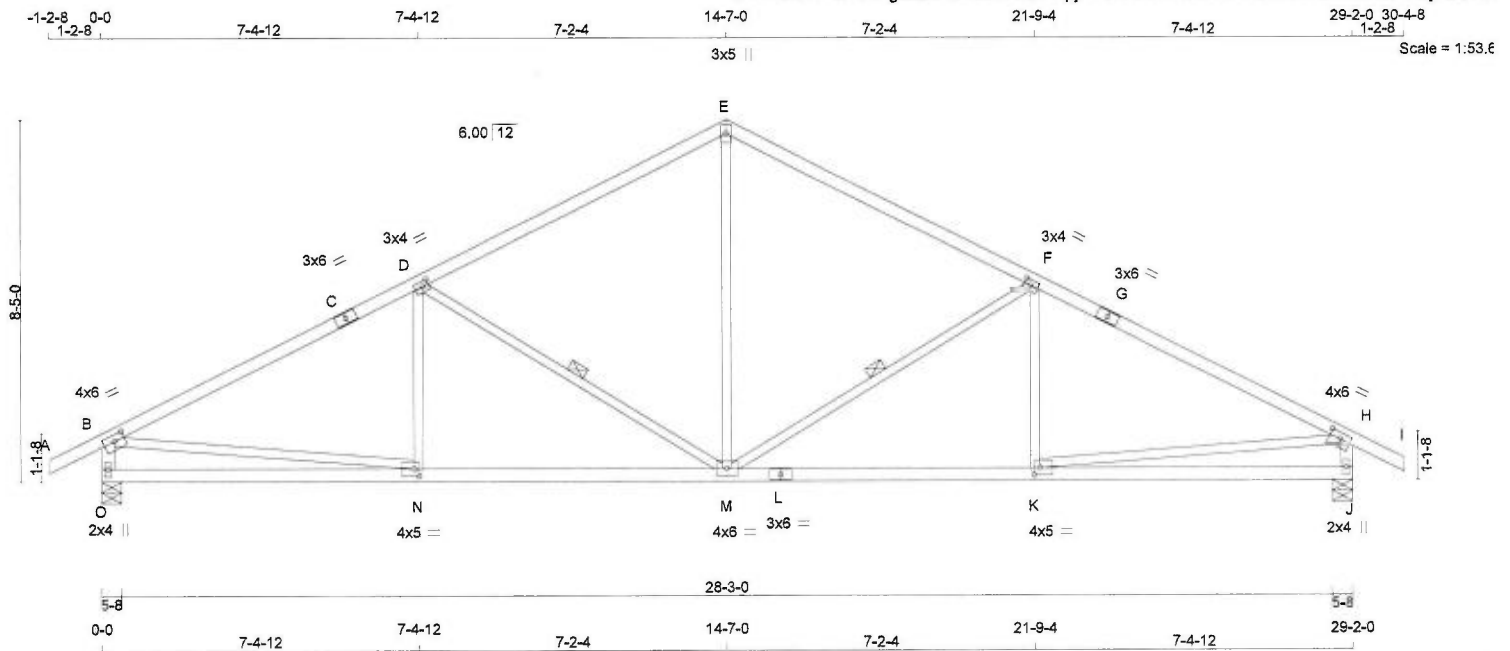
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.55 (P) (INPUT = 1.00)

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = $10 \times 113 = 1134$ lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVVW-t	MT20	4.0	6.0	1.75	3.00
C	TS-t	MT20	3.0	6.0		
D	TMVVW-t	MT20	3.0	4.0	1.50	1.75
E	TTW+p	MT20	3.0	5.0		
F	TMVVW-t	MT20	3.0	4.0	1.50	1.75
G	TS-t	MT20	3.0	6.0		
H	TMVVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVVW-t	MT20	4.0	5.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMVVWVW-t	MT20	4.0	6.0		
N	BMVVW-t	MT20	4.0	5.0	2.00	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1482	0	1482	0	0	5-8	5-8
J	1482	0	1482	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
J	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE), FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	LENGTH		(LBS)	CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	M-E	0 / 835
B-C	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	M-F	-669 / 0
C-D	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	K-F	-81 / 103
D-E	-1431 / 0	-77.4	-77.4	0.59 (1)	4.70	D-M	-669 / 0
E-F	-1431 / 0	-77.4	-77.4	0.59 (1)	4.70	N-D	-81 / 103
F-G	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	B-N	0 / 1830
G-H	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	K-H	0 / 1830
H-I	0 / 22	-77.4	-77.4	0.09 (1)	10.00		
Q-B	-1428 / 0	0.0	0.0	0.14 (1)	6.83		
J-H	-1428 / 0	0.0	0.0	0.14 (1)	6.83		
Q-N	0 / 0	-17.5	-17.5	0.22 (4)	10.00		
N-M	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
M-L	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
L-K	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
K-J	0 / 0	-17.5	-17.5	0.22 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD		=	33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.68/1.00 (B-D:1), BC=0.40/1.00 (M-N:1),
WB=0.41/1.00 (B-N:1), SSI=0.25/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

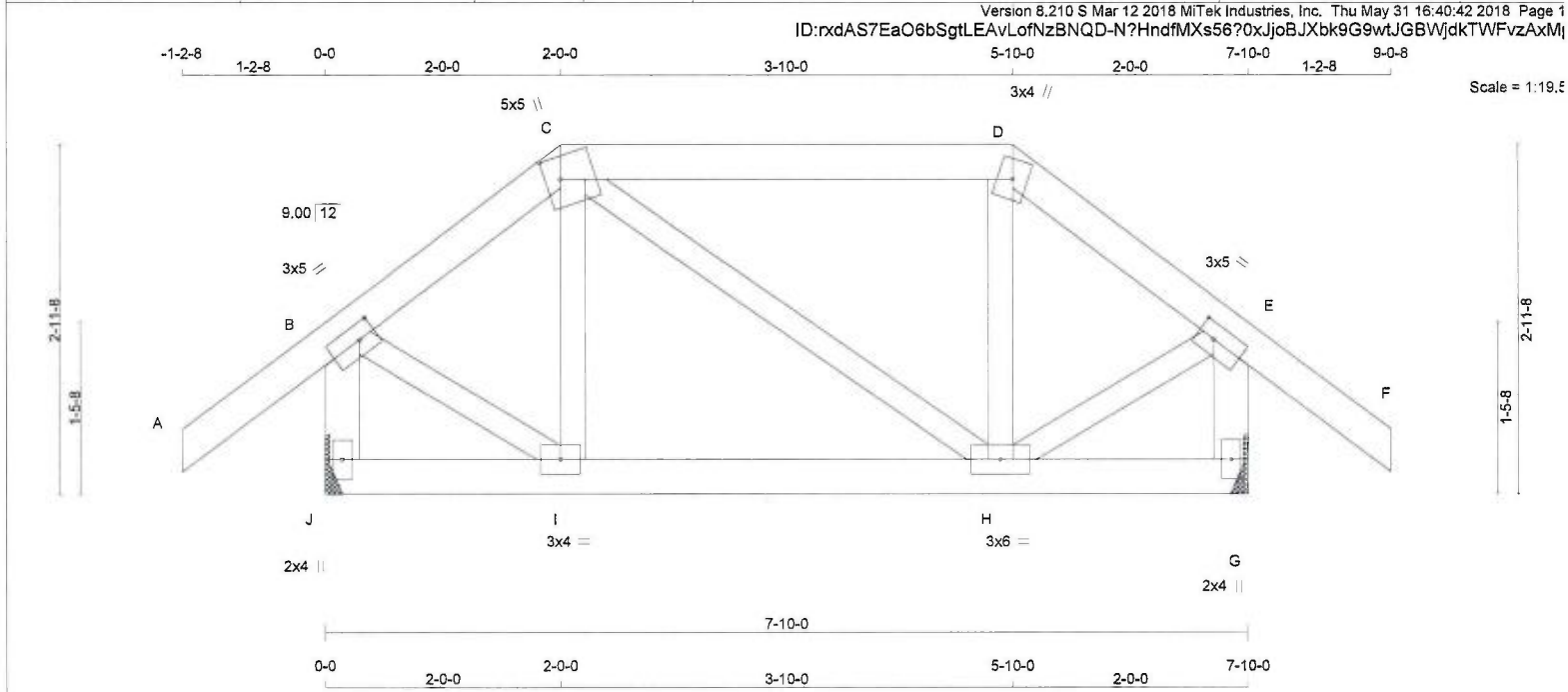
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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THE DESIGN OF THIS COMPONENT.**





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

J

472

0

472

0

0

MECHANICAL

G

472

0

472

0

0

MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J, G TO RESIST THE MAX FACTORED REACTIONS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

23.3

PSF

DL

=

3.0

PSF

BOT CH.

LL

=

0.0

PSF

DL

=

7.0

PSF

TOTAL LOAD

=

33.3

PSF

SPACING

=

24.0

IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")

CALCULATED VERT. DEFL.(LL)= L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.28")

CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.15/1.00 (C-D:1), BC=0.06/1.00 (H-I:1), WB=0.06/1.00 (B-I:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (H) (INPUT = 0.90)

JSI METAL= 0.11 (B) (INPUT = 1.00)

LUMBER				N. L. G. A. RULES				CHORDS				SIZE				LUMBER				DESCR.			
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	J	472	0	472	0	0	MECHANICAL
C - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	G	472	0	472	0	0	MECHANICAL								
D - F	2x4	DRY	No.2	SPF																			
J - B	2x4	DRY	No.2	SPF																			
G - E	2x4	DRY	No.2	SPF																			
J - G	2x4	DRY	No.2	SPF																			
ALL WEBS	2x3	DRY	No.2	SPF																			
EXCEPT																							
DRY: SEASONED LUMBER.																							

PLATES (table is in inches)										UNFACTORED REACTIONS									
JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX	MIN	COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	TMVW-t	MT20	3.0	5.0	1.50	1.75	J	329	243	0	0	0	0	0	0	0	86	0	0
C	TTWW+m	MT20	5.0	5.0	2.25	1.50	G	329	243	0	0	0	0	0	0	0	86	0	0
D	TTWW+m	MT20	3.0	4.0															
E	TMVW-t	MT20	3.0	5.0	1.50	1.75													
G	BMV1+p	MT20	2.0	4.0															
H	BMVW-t	MT20	3.0	6.0															
I	BMVW-t	MT20	3.0	4.0															
J	BMV1+p	MT20	2.0	4.0															

BRACING										LOADING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.										TOTAL LOAD CASES: (4)									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									

CHORDS										WEBS									
MEMB.	MAX. FACTORED	FORCE	VERT.	LOAD	LC1	MAX	MAX.	UNBRAC	MEMB.	MAX. FACTORED	FORCE	VERT.	LOAD	LC1	MAX	MAX.	UNBRAC	MEMB.	MAX. FACTORED
FR-TO									FR-TO									FR-TO	
A-B	0/30		-77.4	-77.4	0.09	(1)	10.00	I-C	-60/27	0.01	(1)							I-C	0/0
B-C	-273/0		-77.4	-77.4	0.04	(1)	6.25	C-H	0/0	0.00	(1)							C-H	0/0
C-D	-215/0		-77.4	-77.4	0.15	(1)	6.25	H-D	-61/28	0.01	(1)							H-D	0/247
D-E	-273/0		-77.4	-77.4	0.04	(1)	6.25	B-I	0/247	0.06	(1)							B-I	0/247
E-F	0/30		-77.4	-77.4	0.09	(1)	10.00	H-E	0/247	0.06	(1)							H-E	0/247
J-B	-460/0		0.0	0.0	0.05	(1)	7.81												
G-E	-460/0		0.0	0.0	0.05	(1)	7.81												
J-I	0/0		-17.5	-17.5	0.04	(4)	10.00												
I-H	0/215		-17.5	-17.5	0.06	(1)	10.00												
H-G	0/0		-17.5	-17.5	0.04	(4)	10.00												

LICENSED PROFESSIONAL ENGINEER

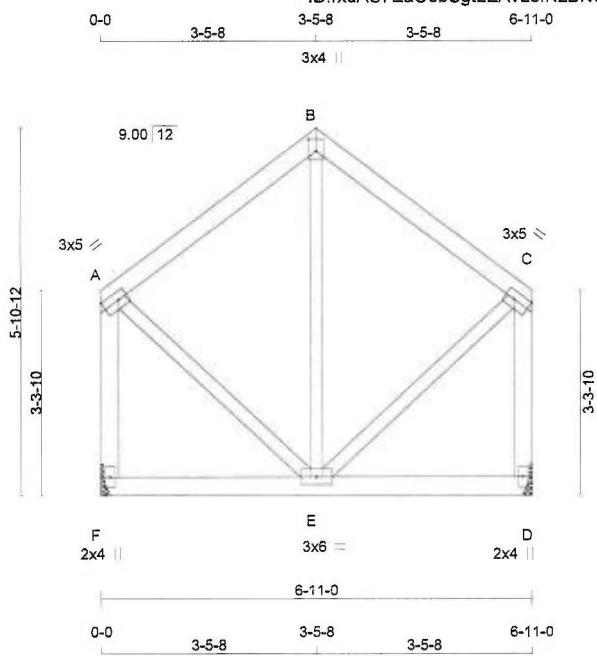
N.A. EL-MASRI

PROVINCE OF ONTARIO

May 31, 2018

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



Scale = 1:36.5

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	

ALL WEBS EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	Edge
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	Edge
D	BMV1+p	MT20	2.0	4.0		
E	BMVWW-t	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	328	0	328	0	0	0	MECHANICAL		
D	328	0	328	0	0	0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	230	161 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	230	161 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-133 / 0	-77.4	-77.4 0.12 (1)	6.25	E-B	-108 / 24	0.06 (1)
B-C	-133 / 0	-77.4	-77.4 0.12 (1)	6.25	A-E	0 / 140	0.03 (1)
F-A	-304 / 0	0.0	0.0 0.06 (1)	7.81	E-C	0 / 140	0.03 (1)
D-C	-304 / 0	0.0	0.0 0.06 (1)	7.81			
F-E	0 / 0	-17.5	-17.5 0.06 (4)	10.00			
E-D	0 / 0	-17.5	-17.5 0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (B-C:1), BC=0.06/1.00 (D-E:4), WB=0.06/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

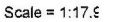
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (A) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = $3 \times 8 = 25 \text{ lb}$
[M]

DESCR.	SPF
	SPF
	SPF

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	216	0	216	0	0	5-8	5-8
C	59	0	59	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SPECIFIED LOADS:				
TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	33.3	PSF



EWP Studio
Simpson Strong-Tie®
Component Solutions™

Client:
Project:
Address:

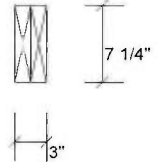
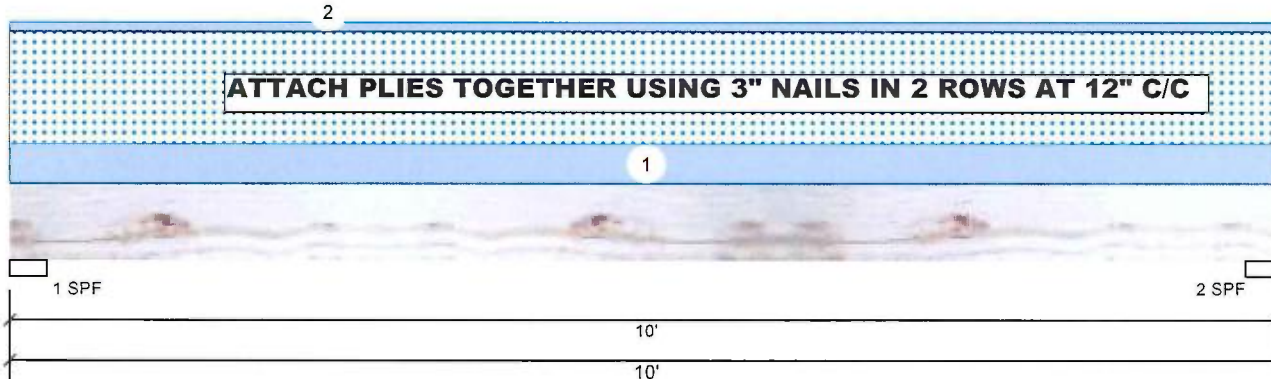
NE0518-224
GREEN YORK HOMES-
AMELIA 2 EL 2

Date: 5/30/2018
Designer: B B
Job Name: B1
Project #:

Page 1 of 1
PAGE 32 OF 34

B1 S-P-F #2 2.000" X 8.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	480	Building Code:	NBCC 2010 / OBC 2012
Deflection TL:	240	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	265	610	0
2	0	265	610	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	19%	331 / 915	1246 L	1.25D+1.5S
2 - SPF	3.500"	19%	331 / 915	1246 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2837 ft-lb	5'	4047 ft-lb	0.701 (70%)	1.25D+1.5S	L
Unbraced	2837 ft-lb	5'	3978 ft-lb	0.713 (71%)	1.25D+1.5S	L
Shear	1039 lb	9'2"	3406 lb	0.305 (30%)	1.25D+1.5S	L
LL Defl inch	0.173 (L/660)	5'	0.239 (L/480)	0.730 (73%)	S	L
TL Defl inch	0.249 (L/460)	5'	0.477 (L/240)	0.520 (52%)	D+S	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform			Top	43 PLF	0 PLF	122 PLF	0 PLF	
2	Uniform			Top	10 PLF	0 PLF	0 PLF	0 PLF	

**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**

Manufacturer Info



Kott Group
14 Anderson Blvd., On
L4A7X4
905-642-4400

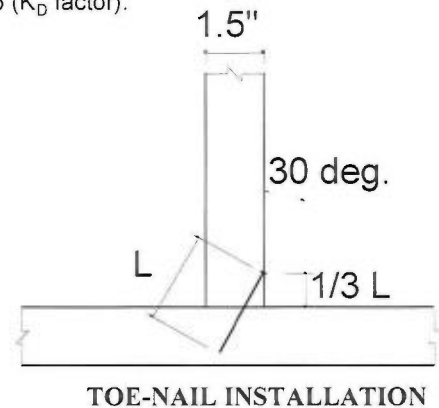
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	132	147
WIRE	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

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Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

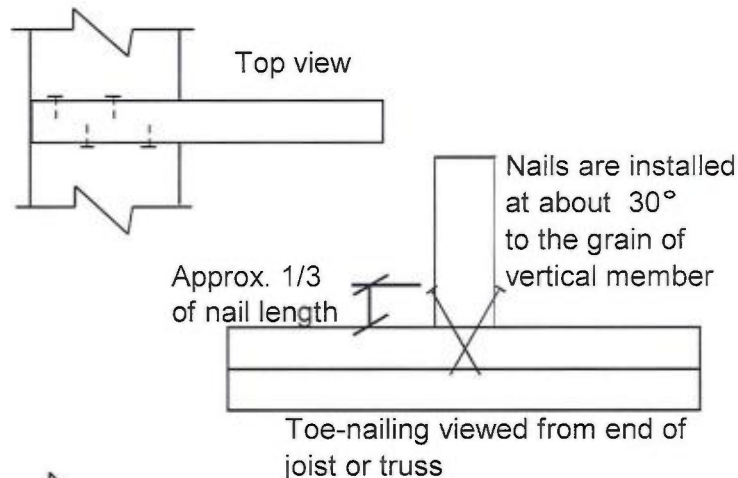
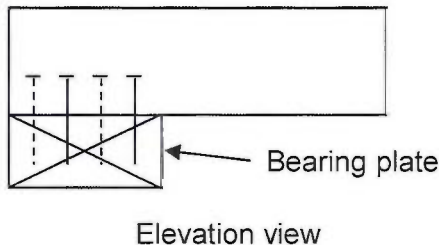
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

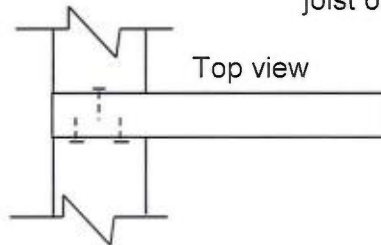
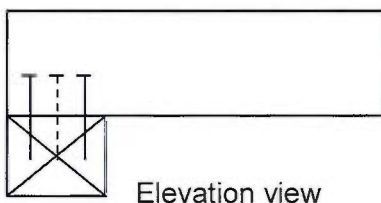
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017