



ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0518-224
GREEN YORK HOMES-AMELIA 2 EL 2

For conventional framing roof / ceilings
 framing shall conform to OBC.9.23

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

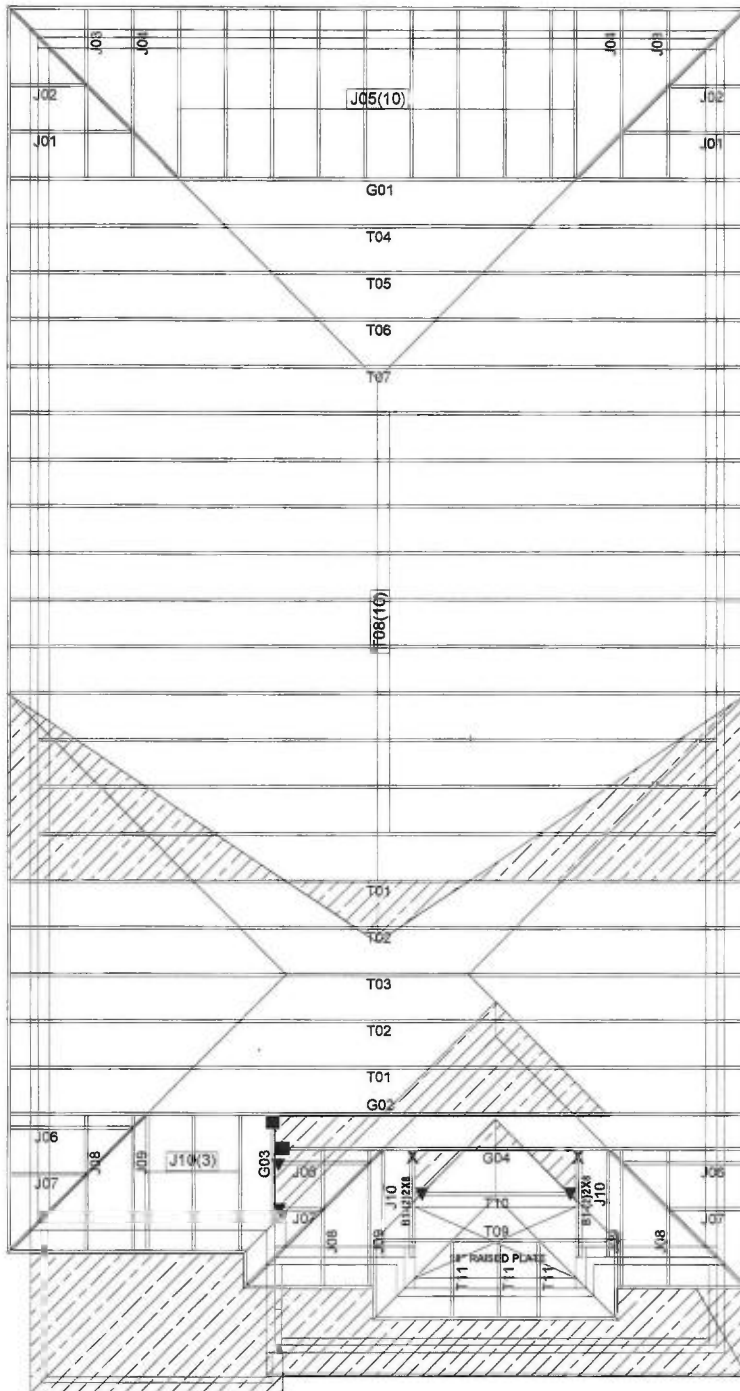
DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



For conventional framing roof / ceilings
framing shall conform to OBC.9.23

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



May 31, 2018
NE0518-224



The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O.Reg 332/12 as amended and as shown on the engineering calculation page of each component.

The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.

Deflection: $LL = L/360$
 $1.33LL + DL = L/360$

Architectural Drawing Info:
Date: MAY 22, 2018
Project number: 17-55
Model: AMELIA 2



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSSES UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

- ▼ LUS24
- HGUS26
- LJS26DS
- ✕ HGUS26-2

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

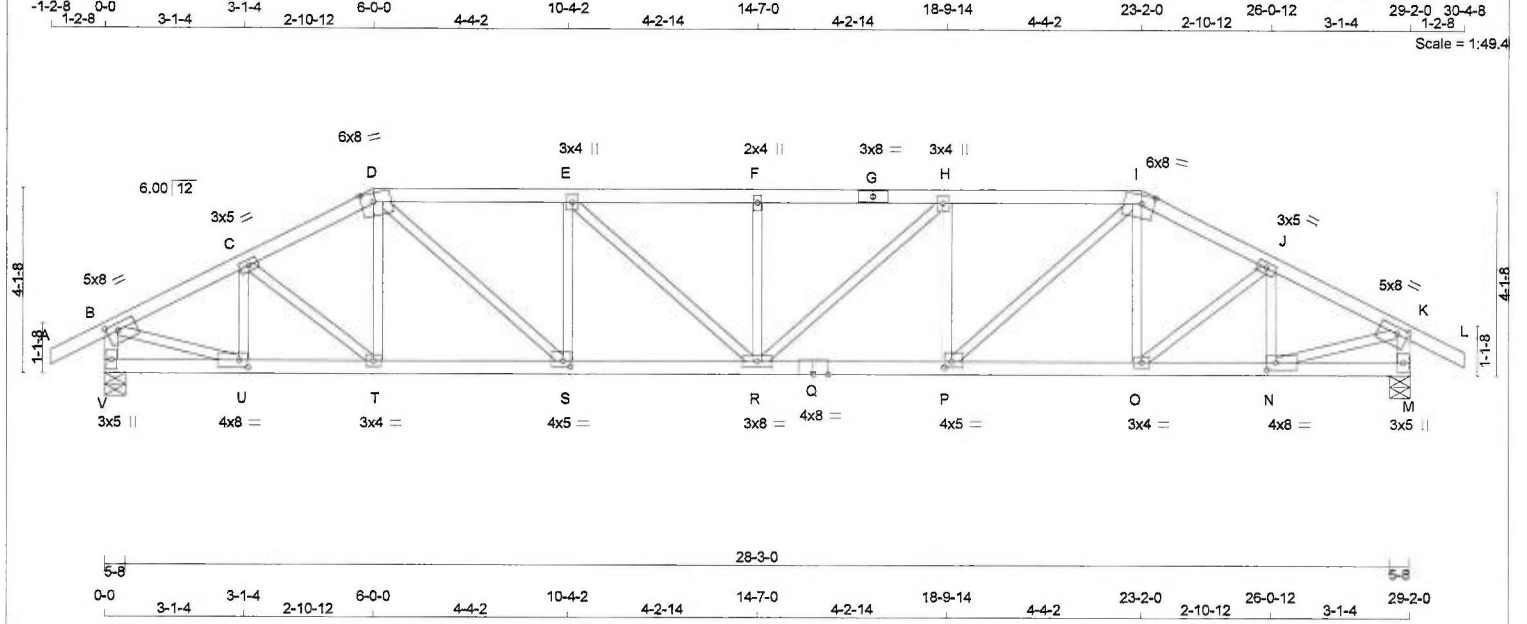
DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 33.3 PSF
DL = 3 PSF
BOT CH. LL = 0 PSF
DL = 7 PSF
TOTAL LOAD = 33.3 PSF
SPACING = 24" O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, BCBC 2012, ABC 2014
-CSA 086-09
-TRC 2011

Model: **AMELIA 2 EL 2**
Customer: **GREEN YORK HOMES**
Project: **GRANELLI HOMES**
Location: **BRAMPTON**
Date: **5/30/2018** Drawn by: **BB**

19-444463.000.00.RR



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
V - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
V - Q	2x4	DRY	No.2	SPF	
Q - M	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.75	3.00	
C	TMVW-t	MT20	3.0	5.0			
D	TMVW-m	MT20	6.0	8.0	2.25	3.25	
E	TMVW-t	MT20	3.0	4.0			
F	TMVW-w	MT20	2.0	4.0			
G	TS-t	MT20	3.0	8.0			
H	TMVW-t	MT20	3.0	4.0			
I	TTWW-m	MT20	6.0	8.0	2.25	3.25	
J	TMVW-t	MT20	3.0	5.0			
K	TMVW-t	MT20	5.0	8.0	1.75	3.00	
M	BMV1+p	MT20	3.0	5.0			
N	BMVW-t	MT20	4.0	8.0	2.00	2.25	
O	BMVW-t	MT20	3.0	4.0			
P	BMVW-t	MT20	4.0	5.0	1.50	2.00	
Q	BS-t	MT20	4.0	8.0			
R	BMVW-t	MT20	3.0	8.0			
S	BMVW-t	MT20	4.0	5.0	1.50	2.00	
T	BMVW-t	MT20	3.0	4.0			
U	BMVW-t	MT20	4.0	8.0	2.00	2.25	
V	BMV1+p	MT20	3.0	5.0			

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
V 2644	0	2644	0	0
M 2644	0	2644	0	0

UNFACTORED REACTIONS	1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
V 1855	1302 / 0	0 / 0
M 1855	1302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(PLF)	MAX	(LBS)	MAX
FR-TO	FROM	TO	FR-TO	FROM
A-B	0 / 22	-77.4	U-C	-748 / 0
B-C	-3483 / 0	-77.4	C-T	0 / 473
C-D	-3915 / 0	-77.4	T-D	-147 / 76
D-E	-4840 / 0	-145.9	D-S	0 / 1802
E-F	-5231 / 0	-145.9	S-E	-1054 / 0
F-G	-5231 / 0	-145.9	E-R	0 / 527
G-H	-5231 / 0	-145.9	R-F	-570 / 0
H-I	-4840 / 0	-145.9	R-H	0 / 527
I-J	-3915 / 0	-77.4	P-H	-1054 / 0
J-K	-3483 / 0	-77.4	P-I	0 / 1802
K-L	0 / 22	-77.4	O-I	-147 / 76
V-B	-2586 / 0	0.0	O-J	0 / 473
M-K	-2586 / 0	0.0	N-J	-748 / 0
V-U	0 / 0	-33.0	B-U	0 / 3226
U-T	0 / 3120	-33.0	N-K	0 / 3226
T-S	0 / 3488	-33.0		
S-R	0 / 4840	-33.0		
R-Q	0 / 4840	-33.0		
Q-P	0 / 4840	-33.0		
P-O	0 / 3488	-33.0		
O-N	0 / 3120	-33.0		
N-M	0 / 0	-33.0		

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6-0-0	-348	-348	---	FRONT	VERT	TOTAL
I	23-2-0	-348	-348	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-0-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.27")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/739 (0.47")

CSI: TC=0.89/1.00 (F+H:1), BC=0.93/1.00 (R-S:1), WB=0.80/1.00 (B-U:1), SSI=0.32/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

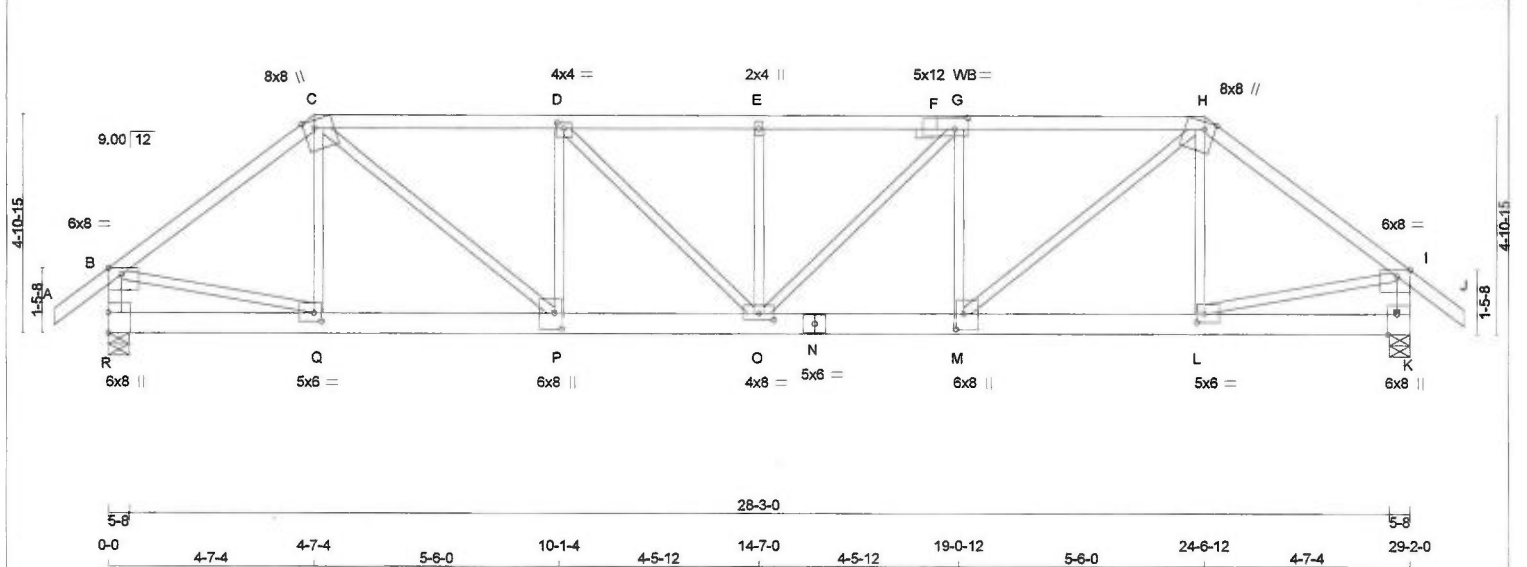
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READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 23-2-0, AND 348.1 lbs FACTORED DOWN AT 6-0-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER. PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (I) (INPUT = 0.90) JSI METAL= 0.94 (Q) (INPUT = 1.00)	
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. 	



TOTAL WEIGHT = 139 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS		
A - C	2x4	DRY	2100F 1.8E	SPF	FACTORED	MAXIMUM FACTORED	INPUT
C - F	2x4	DRY	2100F 1.8E	SPF	GROSS REACTION	GROSS REACTION	BRG
F - H	2x4	DRY	2100F 1.8E	SPF	JT VERT	DOWN	UPLIFT
H - J	2x4	DRY	2100F 1.8E	SPF	R	3150	0
J - B	2x4	DRY	No.2	SPF	K	2256	0
B - I	2x4	DRY	No.2	SPF			
I - N	2x4	DRY	No.2	SPF			
N - K	2x6	DRY	No.2	SPF			
K - L	2x6	DRY	No.2	SPF			
L - M	2x6	DRY	No.2	SPF			
M - O	2x6	DRY	No.2	SPF			
O - P	2x6	DRY	No.2	SPF			
P - Q	2x6	DRY	No.2	SPF			
Q - R	2x6	DRY	No.2	SPF			
R - K	2x6	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							

DRY: SEASONED LUMBER.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 4'-7-4

END SETBACK = 4'-7-4

END WALL WIDTH = 5'-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL. LOADS BASED ON 55 % OF G.S.L.

LOADS APPLIED TO FIRST 10'-1-4 OF SPAN MEASURED FROM THE LEFT.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.07 (1)	Q-C	-445 / 0	0.15 (1)	
B-C	-3631 / 0	-77.4 -77.4	0.32 (1)	C-P	0 / 3109	0.77 (1)	
C-D	-5291 / 0	-118.9 -118.9	0.58 (1)	P-D	0 / 108	0.04 (4)	
D-E	-4711 / 0	-77.4 -77.4	0.43 (1)	D-O	-826 / 0	0.59 (1)	
E-F	-4711 / 0	-77.4 -77.4	0.36 (1)	O-E	-259 / 0	0.09 (1)	
F-G	-4711 / 0	-77.4 -77.4	0.36 (1)	G-O	0 / 1331	0.33 (1)	
G-H	-3776 / 0	-77.4 -77.4	0.34 (1)	M-G	-1396 / 0	0.48 (1)	
H-I	-2499 / 0	-77.4 -77.4	0.27 (1)	M-H	0 / 2314	0.57 (1)	
I-J	0 / 30	-77.4 -77.4	0.07 (1)	L-H	-330 / 0	0.11 (1)	
R-B	-3109 / 0	0.0 0.0	0.35 (1)	B-Q	0 / 2970	0.74 (1)	
K-I	-2226 / 0	0.0 0.0	0.25 (1)	L-I	0 / 2044	0.51 (1)	
R-Q	0 / 0	-26.9 -26.9	0.10 (4)				
Q-P	0 / 2889	-26.9 -26.9	0.48 (1)				
P-O	0 / 5291	-17.5 -17.5	0.78 (1)				
O-N	0 / 3776	-17.5 -17.5	0.54 (1)				
N-M	0 / 3776	-17.5 -17.5	0.54 (1)				
M-L	0 / 1988	-17.5 -17.5	0.30 (1)				
L-K	0 / 0	-17.5 -17.5	0.06 (4)				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-7-4	-184	-184	—	FRONT	VERT	TOTAL
P	10-1-4	-1931	-1931	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")

CALCULATED VERT. DEFL.(LL) = L/999 (0.19")

ALLOWABLE DEFL.(TL) = L/360 (0.97")

CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.58/1.00 (C-D:1), BC=0.78/1.00 (O-P:1), WB=0.77/1.00 (C-P:1), SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN YORK HOMES-AMELIA 2 EL 2	DRWG NO.
NE0518-224	G02	1	1	TRUSS DESC.		PAGE 6 OF 34

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 16:40:30 2018 Page 2
ID: rxdAS7EaO6bSgtLEAvLofNzBNQD-jhZF5ZC7hPbia5PQ5f5j6Cf5J4bBSaOyttZurczAxN

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 184.3 lbs FACTORED DOWN AT 4-7-4 ON TOP CHORD, AND 1930.7 lbs FACTORED DOWN AT 10-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

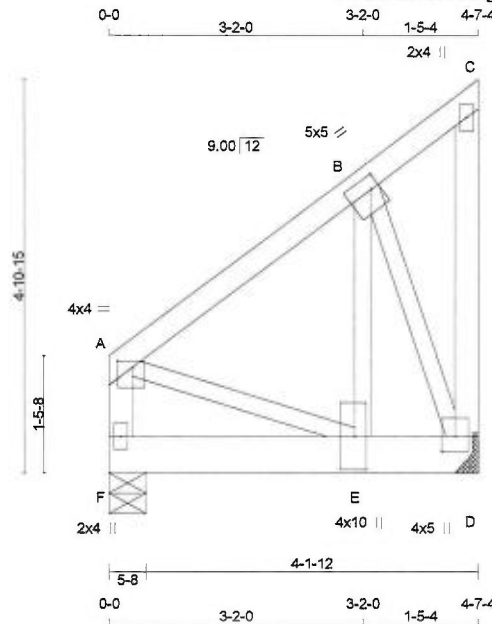
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)

JSI METAL= 0.72 (Q) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:27

TOTAL WEIGHT = 28 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.00	2.25
B	TMVW-t	MT20	5.0	5.0	2.00	1.25
C	TMV+p	MT20	2.0	4.0		
D	BMVW1+p	MT20	4.0	5.0	2.25	2.00
E	BMVW+t	MT20	4.0	10.0	5.00	1.75
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2438.0 lbs FACTORED DOWN AT 3-2-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1047	0	1047	0
D	1931	0	1931	0

5-8 MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	735	514 / 0	0 / 0	0 / 0	0 / 0	221 / 0	0 / 0	0 / 0
D	1355	948 / 0	0 / 0	0 / 0	0 / 0	407 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH
FR-TO		FROM	TO		FR-TO		FROM	TO
F-A	-876 / 0	0.0	0.0	0.10 (1)	7.81	A-E	0 / 748	0.19 (1)
A-B	-879 / 0	-77.4	-77.4	0.13 (1)	6.25	E-B	0 / 2000	0.50 (1)
B-C	-18 / 0	-77.4	-77.4	0.06 (1)	6.25	B-D	-1832 / 0	0.46 (1)
D-C	-26 / 0	0.0	0.0	0.01 (1)	7.81			
F-E	0 / 0	-50.1	-50.1	0.21 (1)	10.00			
E-D	0 / 710	-17.5	-17.5	0.31 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	3-2-0	-2438	-2438	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-4-8
END DISTANCE = 3-2-0
END SPAN CARRIED = 3-4-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.31/1.00 (D-E:1), WB=0.50/1.00 (B-E:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

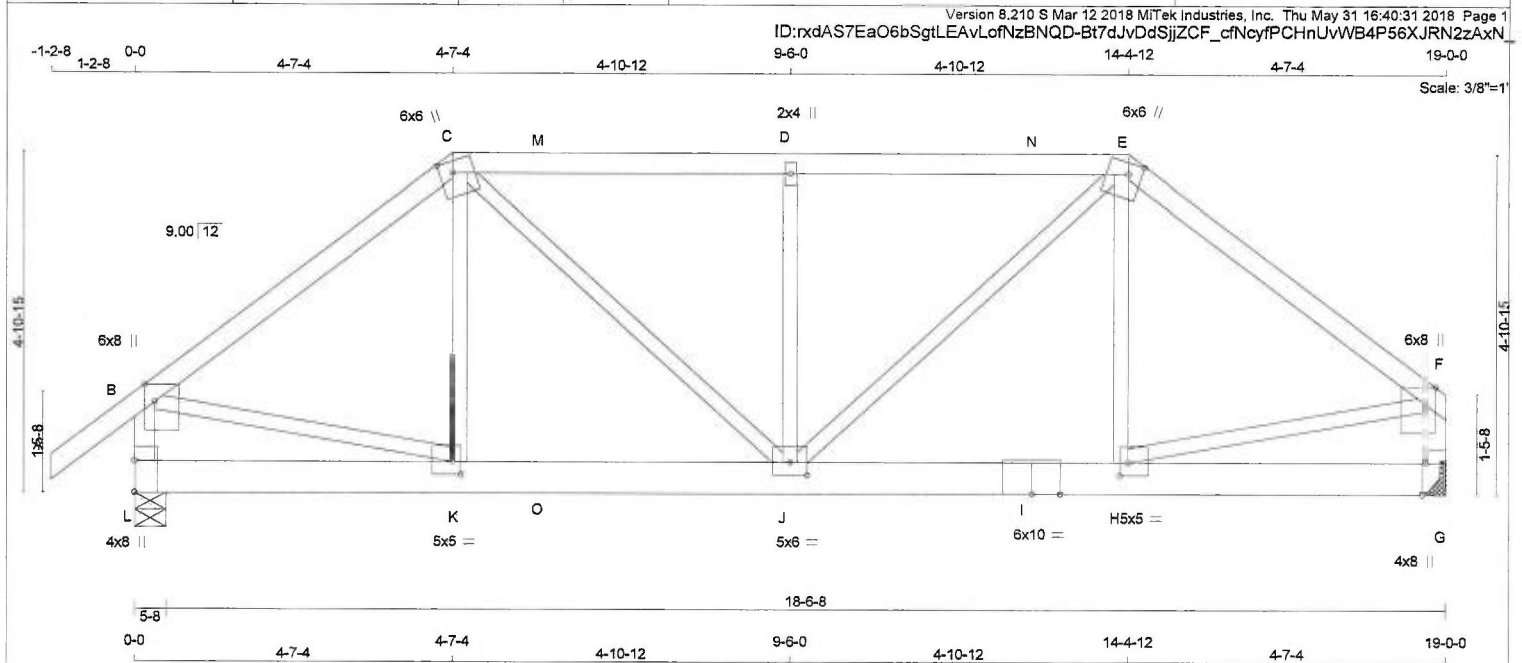
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.41 (E) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
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Version 8.210 S Mar 12 2018 Mitek Industries, Inc. Thu May 31 16:40:31 2018 Page 1
ID: rxdAS7EaO6bSgtLEAvLoNzBNQD-Bt7dJvDdSjjZCF_cfNcyfPCHnUvWB4P56XJRN2zAxN
Scale: 3/8"=1'

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - C	2x4	DRY	No.2	SPF		GROSS REACTION	GROSS REACTION
C - E	2x4	DRY	No.2	SPF	JT	VERT	HORZ
E - F	2x4	DRY	No.2	SPF	L	2548	0
L - B	2x4	DRY	No.2	SPF	G	2437	0
G - F	2x4	DRY	No.2	SPF			
L - I	2x6	DRY	No.2	SPF			
I - G	2x6	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER. TOTAL WEIGHT = 90 lb

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS		*** SPECIAL LOADS ANALYSIS ***			
JT	COMBINED	SNOW	LIVE	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
L	1786	1261 / 0	0 / 0	LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE			
G	1711	1196 / 0	0 / 0				

PLATES (table is in inches)				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L			
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	6.0	8.0	3.00	1.75	
C	TTWW+m	MT20	6.0	6.0	2.00	2.25	
D	TMVW+w	MT20	2.0	4.0			
E	TTWW+m	MT20	6.0	6.0	2.00	2.25	
F	TMVW+p	MT20	6.0	8.0	3.00	1.75	
G	BMV1+t	MT20	4.0	8.0	Edge	0.50	
H	BMVW+t	MT20	5.0	5.0	2.25	3.00	
I	BS-1	MT20	6.0	10.0			
J	BMVW+t	MT20	5.0	6.0	2.25	3.00	
K	BMVW+t	MT20	5.0	5.0	2.25	1.50	
L	BMV1+t	MT20	4.0	8.0	5.50		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES				LOADING			
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 184.3 lbs FACTORED DOWN AT 4-7-4, AND 184.3 lbs FACTORED DOWN AT 14-4-12 ON TOP CHORD, AND 1246.8 lbs FACTORED DOWN AT 5-11-0, AND 1246.8 lbs FACTORED DOWN AT 13-1-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.10 (1)	K-C	0 / 571	0.14 (1)	
B-C	-2927 / 0	-77.4 -77.4	0.54 (1)	C-J	0 / 924	0.23 (1)	
C-M	-3030 / 0	-118.9 -118.9	0.53 (1)	J-D	-461 / 0	0.16 (1)	
M-D	-3030 / 0	-77.4 -77.4	0.53 (1)	J-E	0 / 934	0.23 (1)	
D-N	-3030 / 0	-77.4 -77.4	0.53 (1)	H-E	0 / 558	0.14 (1)	
N-E	-3030 / 0	-118.9 -118.9	0.53 (1)	B-K	0 / 2394	0.59 (1)	
E-F	-2918 / 0	-77.4 -77.4	0.54 (1)	H-F	0 / 2387	0.59 (1)	
L-B	-2560 / 0	0.0 0.0	0.29 (1)				
G-F	-2453 / 0	0.0 0.0	0.27 (1)				
L-K	0 / 0	-26.9 -26.9	0.26 (1)				
K-O	0 / 2350	-26.9 -26.9	0.88 (1)				
O-J	0 / 2350	-17.5 -17.5	0.88 (1)				
J-I	0 / 2343	-17.5 -17.5	0.90 (1)				
I-H	0 / 2343	-26.9 -26.9	0.90 (1)				
H-G	0 / 0	-26.9 -26.9	0.27 (1)				

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.09")

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ALLOWABLE DEFL(TL)= L/360 (0.83")
CALCULATED VERT. DEFL(TL) = L/ 999 (0.16")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.90/1.00 (H-J:1) ,
WB=0.59/1.00 (B-K:1) , SSI=0.63/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

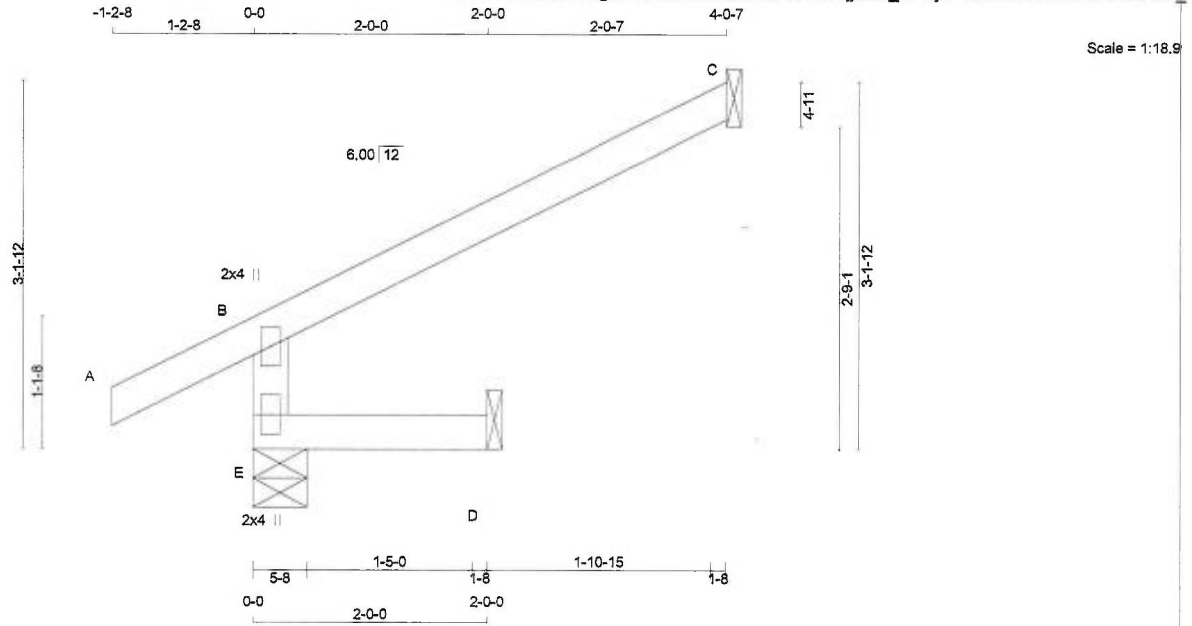
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.80 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	313	0	313	0	5-8	5-8
C	117	0	117	0	1-8	1-8
D	15	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	215	177 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-294 / 0	0.0	0.0	0.01 (4)	7.81	
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	
B-C	-17 / 0	-77.4	-77.4	0.21 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.25 inches

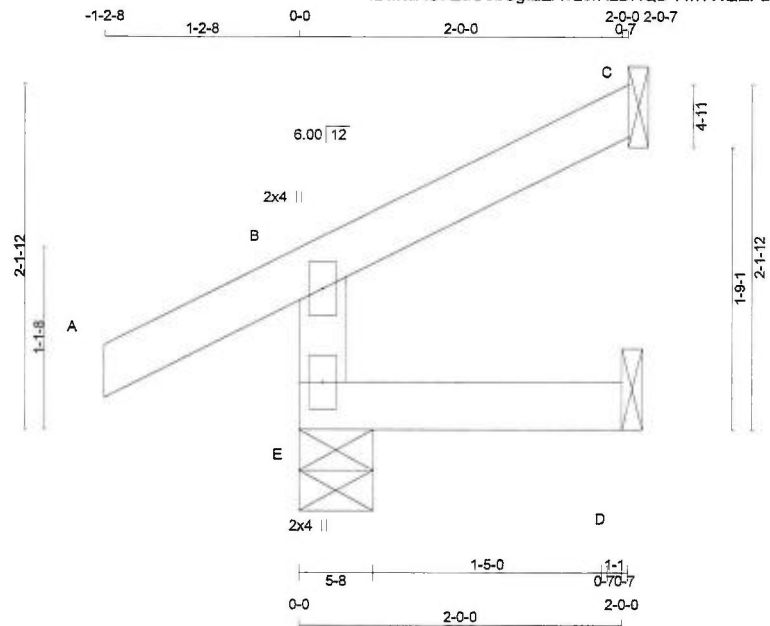
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:13.7

TOTAL WEIGHT = $2 \times 7 = 15 \text{ lb}$

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	216	0	216	0	0	5-8	5-8
C	59	0	59	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	149	118 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-197 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00		
B-C	-9 / 0	-77.4	-77.4	0.05 (1)	10.00		
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.19")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.19")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

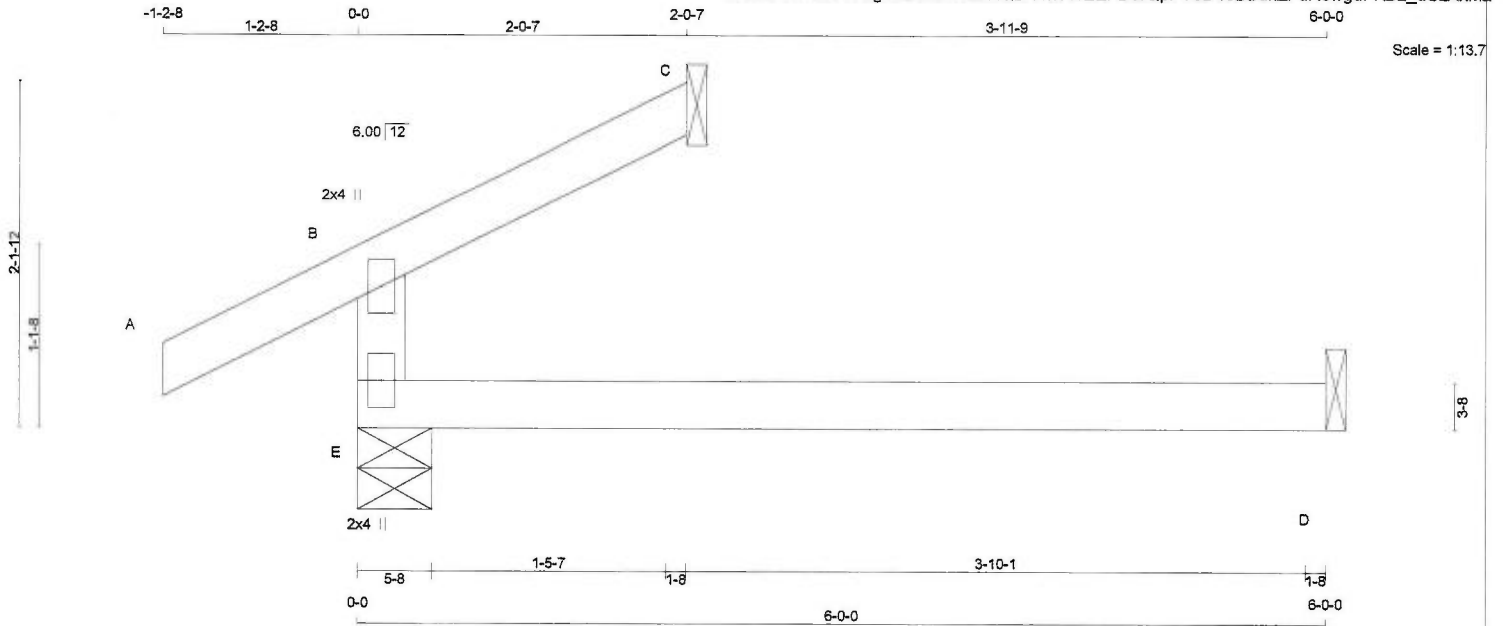
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
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THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 2 X 12 = 24 lb [M][F]

LUMBER		
N. L. G. A. RULES	SIZE	LUMBER
CHORDS	2x4	DRY
E - B	2x4	DRY
A - C	2x4	DRY
E - D	2x4	DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	258	0	258	0	5-8	5-8
C	59	0	59	0	1-8	1-8
D	43	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	183	118 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0
C	40	38 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. MEMB. FORCE (LBS)	MAX. MEMB. FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
E-B	-197 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
B-C	-9 / 0	-77.4	-77.4	0.04 (1)	10.00			
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.11/1.00 (B-E:4), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

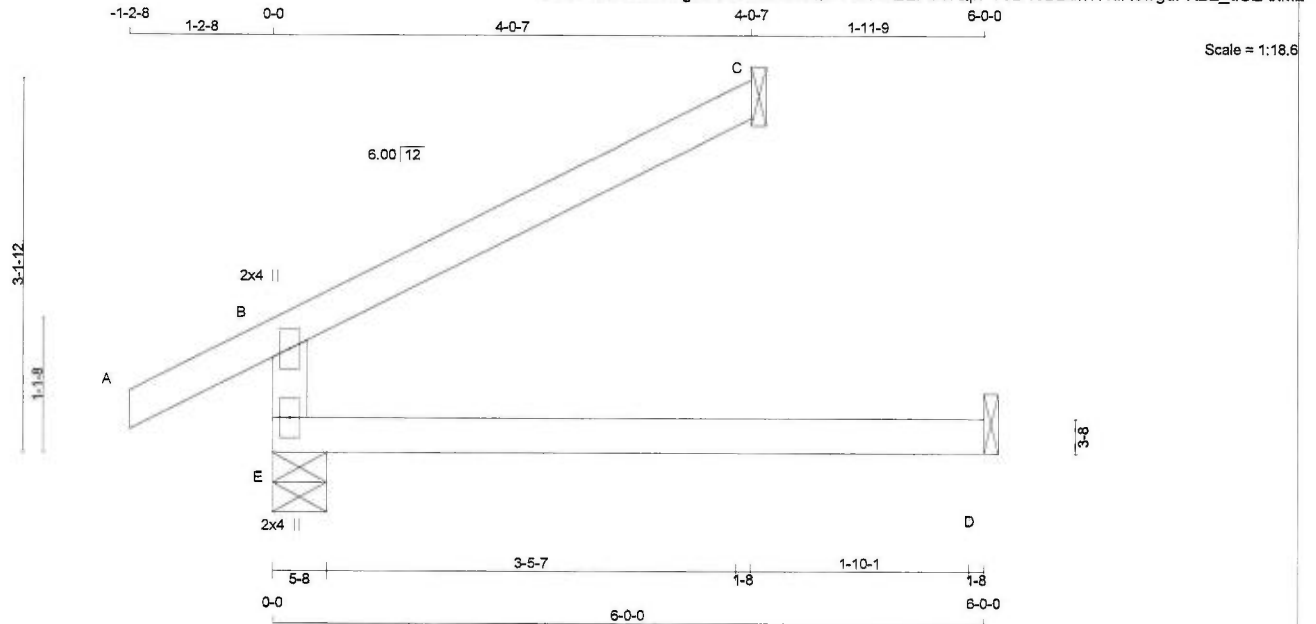
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 14 = 28 lb [M/F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	355	0	355	0	0	5-8	5-8	
C	117	0	117	0	0	1-8	1-8	
D	43	0	48	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM.LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
E	249	177 / 0	0 / 0		0 / 0	0 / 0	72 / 0	0 / 0
C	80	71 / 0	0 / 0		0 / 0	0 / 0	9 / 0	0 / 0
D	35	0 / 0	0 / 0		0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX LC1	MAX LC2	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FORCE (LBS)	MAX LC1
E-B	-294 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
B-C	-17 / 0	-77.4	-77.4	0.16 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.16/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

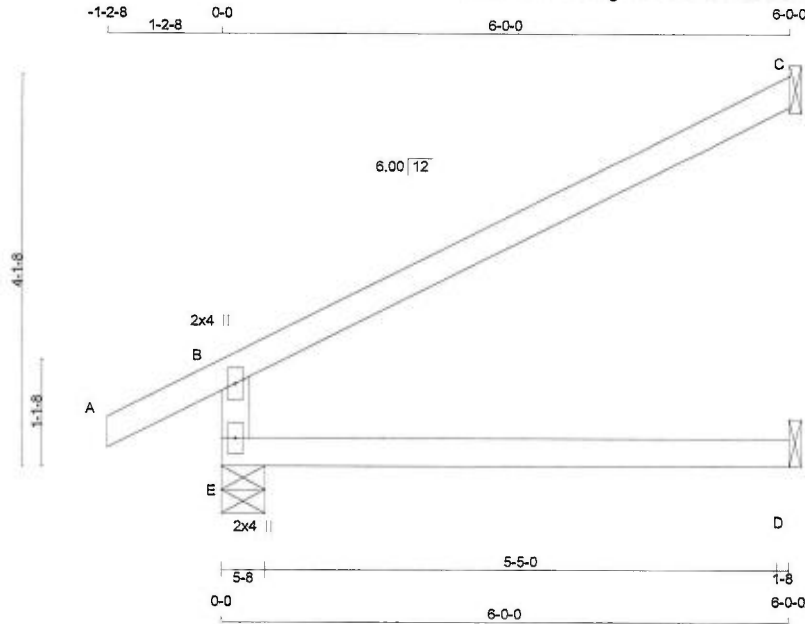
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.4

TOTAL WEIGHT = 10 X 17 = 169 lb [M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPFF SPFF SPFF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	450	0	450	0	0	5-8	5-8	
C	174	0	174	0	0	1-8	1-8	
D	43	0	48	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
	COMBINED	SNOW	LIVE									
E	314	234 / 0	0 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	MAX. LC (LC)
FR-TO		FROM	TO					
E-B	-389 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
B-C	-26 / 0	-77.4	-77.4	0.36 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

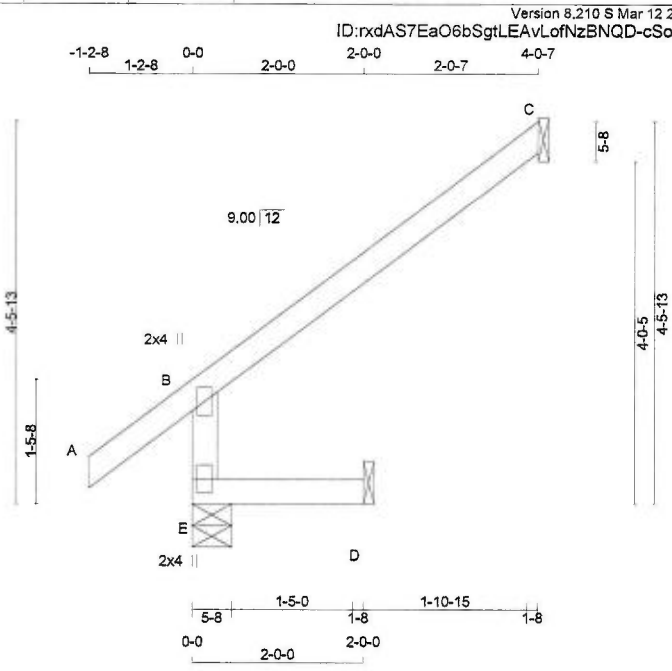
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:25.9

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	314	314	5-8	5-8
C	117	117	1-8	1-8
D	16	16	1-8	1-8

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	7.0	PSF

TOTAL LOAD = 33.3 PSF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	216	178 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-295 / 0	0.0	0.0 0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00			
B-C	-23 / 0	-77.4	-77.4 0.21 (1)	6.25			
E-D	0 / 0	-17.5	-17.5 0.02 (4)	10.00			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.21/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1867
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

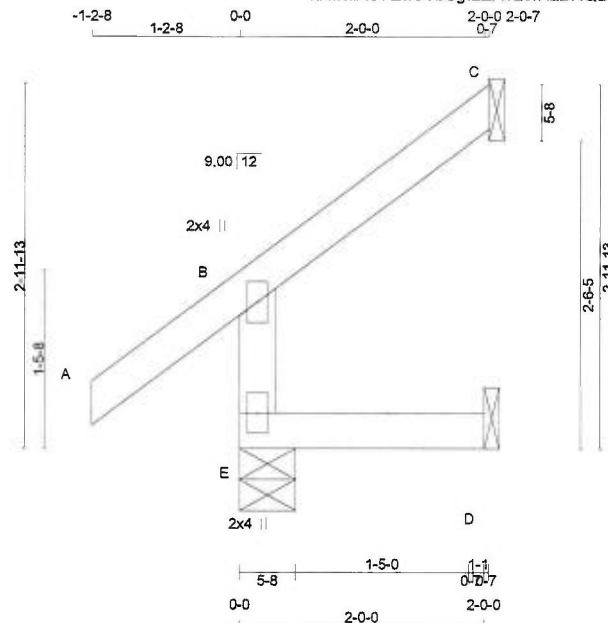
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:18.1

TOTAL WEIGHT = 3 X 8 = 25 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	217	0	217	0	0	5-8	5-8	
C	60	0	60	0	0	1-8	1-8	
D	16	0	18	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
E	150	119 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX (LC)	MAX (LC)	
FR-TO		FROM	TO		FR-TO			
E-B	-198 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-12 / 0	-77.4	-77.4	0.05 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

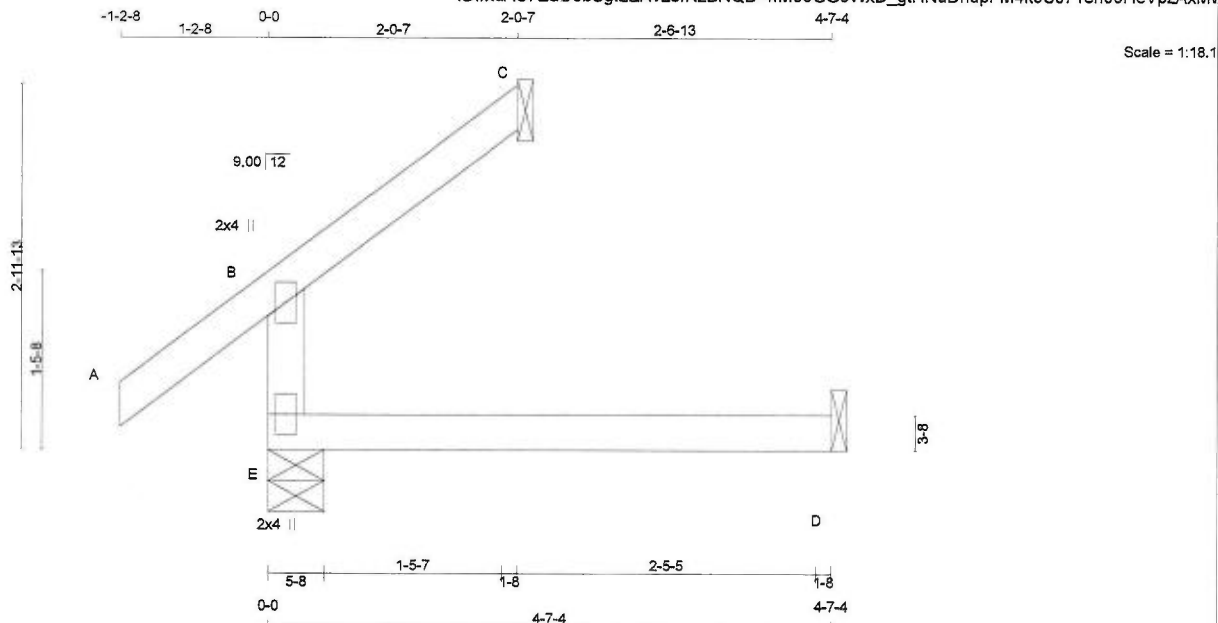
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 11 = 34 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	244	0	244	0	0	5-8	5-8	
C	60	0	60	0	0	1-8	1-8	
D	34	0	39	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	172	119 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	40	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
E-B	-198 / 0	0.0	0.0	0.06 (4)	7.81		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00		
B-C	-12 / 0	-77.4	-77.4	0.05 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.09/1.00 (A-B:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

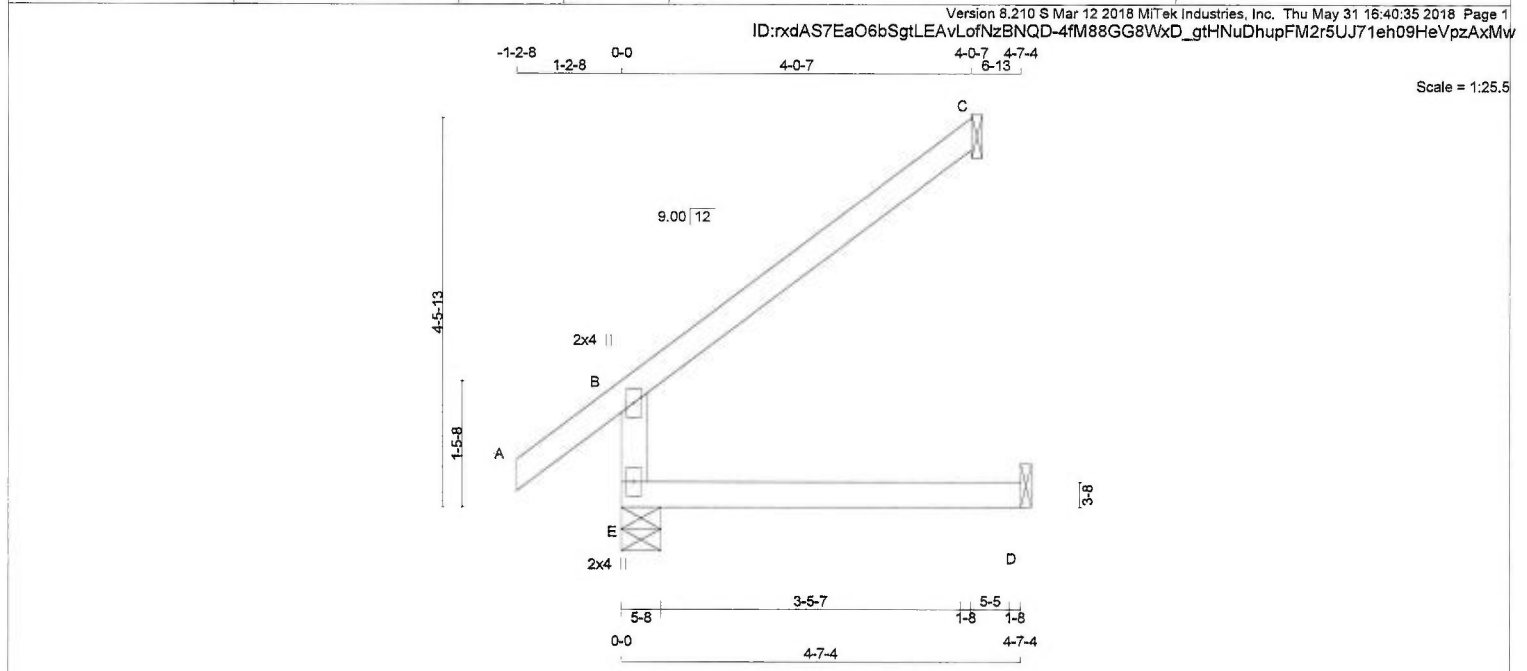
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	341	0	341	0	5-8	5-8
C	117	0	117	0	1-8	1-8
D	34	0	39	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	238	178 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	80	71 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-295 / 0	0.0	0.0	0.06 (4)	7.81		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00		
B-C	-23 / 0	-77.4	-77.4	0.21 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00		

TOTAL WEIGHT = 3 X 14 = 42 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

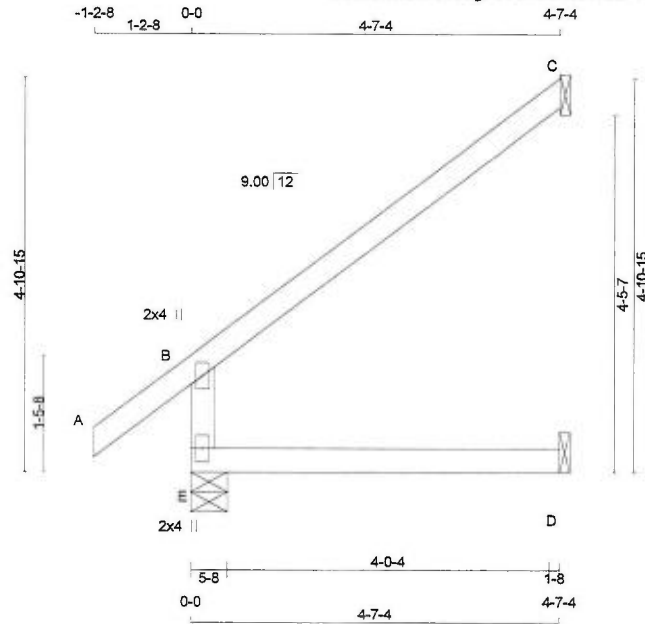
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:27.6

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	369	0	369	0	0	5-8	5-8	
C	134	0	134	0	0	1-8	1-8	
D	34	0	39	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
E	256	194 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	91	80 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX.	FORCE	VERT.	LOAD LC1	MAX.	MEMB.	MAX.	FORCE
		(LBS)		(PLF)				(LBS)
FR-TO			FROM	TO		LENGTH	FR-TO	
E-B	-323	0	0.0	0.0	0.06 (4)	7.61		
A-B	0	30	-77.4	-77.4	0.09 (1)	10.00		
B-C	-27	0	-77.4	-77.4	0.28 (1)	6.25		
E-D	0	0	-17.5	-17.5	0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.28/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

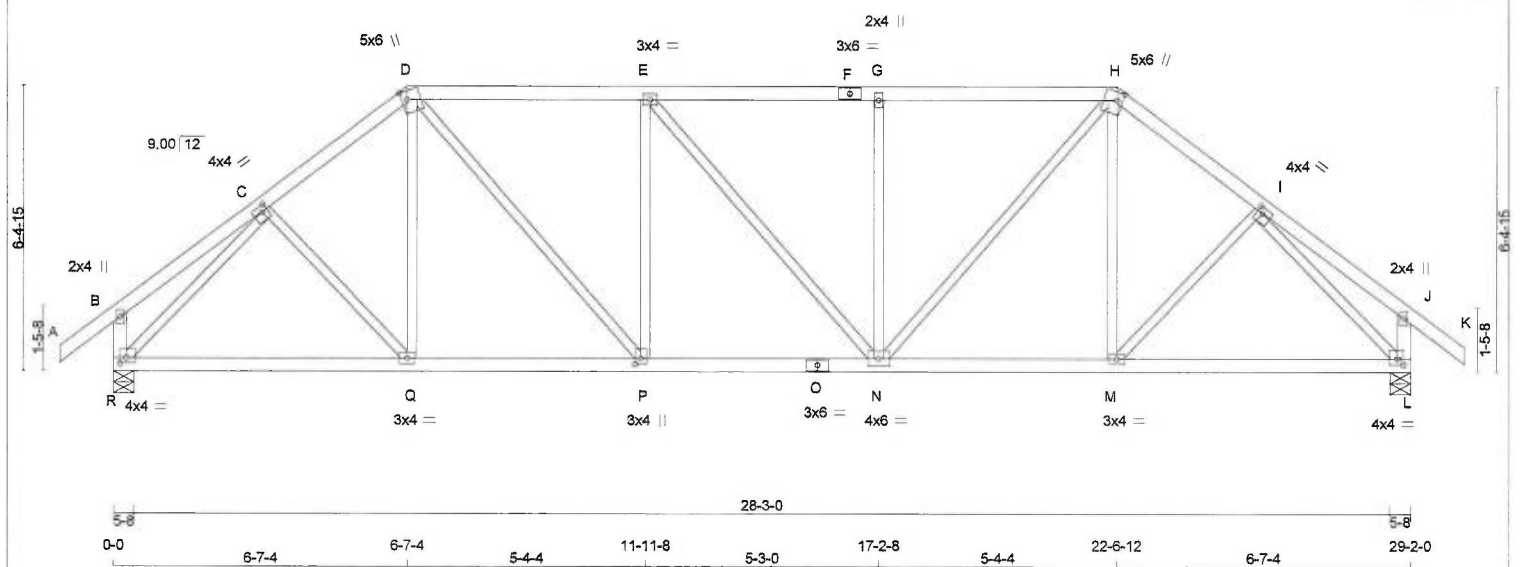
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 129 = 258 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF	JT	VERT	GROSS REACTION
D - F	2x4	DRY	No.2	SPF	R	HORZ	GROSS REACTION
F - H	2x4	DRY	No.2	SPF	L	DOWN	GROSS REACTION
H - K	2x4	DRY	No.2	SPF		UPLIFT	BRG
R - B	2x4	DRY	No.2	SPF		IN-SX	BRG
L - J	2x4	DRY	No.2	SPF		IN-SX	BRG
R - O	2x4	DRY	No.2	SPF			
O - L	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS		SPECIFIED LOADS:			
JT	COMBINED	SNOW	LIVE	TOP CH. LL =	23.3	PSF	
R	1039	740 / 0	0 / 0	DL =	3.0	PSF	
L	1039	740 / 0	0 / 0	BOT CH. LL =	0.0	PSF	
				DL =	7.0	PSF	
				TOTAL LOAD =	33.3	PSF	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	FR-TO	FORCE (LBS)	MAX. CSI (LC)	
A-B	0 / 30	-77.4	-77.4 0.09 (1)	C-Q	0 / 60	0.02 (4)	
B-C	0 / 16	-77.4	-77.4 0.12 (1)	Q-D	0 / 94	0.03 (4)	
C-D	-1481 / 0	-77.4	-77.4 0.15 (1)	D-P	0 / 703	0.16 (1)	
D-E	-1634 / 0	-77.4	-77.4 0.36 (1)	P-E	-443 / 0	0.29 (1)	
E-F	-1633 / 0	-77.4	-77.4 0.35 (1)	E-N	-2 / 0	0.00 (1)	
F-G	-1633 / 0	-77.4	-77.4 0.35 (1)	N-G	-443 / 0	0.29 (1)	
G-H	-1633 / 0	-77.4	-77.4 0.35 (1)	H-N	0 / 701	0.16 (1)	
H-I	-1481 / 0	-77.4	-77.4 0.15 (1)	M-H	0 / 94	0.03 (4)	
I-J	0 / 16	-77.4	-77.4 0.12 (1)	M-I	0 / 60	0.02 (4)	
J-K	0 / 30	-77.4	-77.4 0.09 (1)	R-C	-1672 / 0	0.83 (1)	
R-B	-201 / 0	0.0	0.0 0.02 (1)	I-L	-1672 / 0	0.83 (1)	
L-J	-201 / 0	0.0	0.0 0.02 (1)				
R-Q	0 / 1128	-17.5	-17.5 0.27 (1)				
Q-P	0 / 1170	-17.5	-17.5 0.28 (1)				
P-O	0 / 1634	-17.5	-17.5 0.31 (1)				
O-N	0 / 1634	-17.5	-17.5 0.31 (1)				
N-M	0 / 1171	-17.5	-17.5 0.28 (1)				
M-L	0 / 1128	-17.5	-17.5 0.28 (1)				

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.31/1.00 (N-P:1) ,
WB=0.63/1.00 (I-L:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

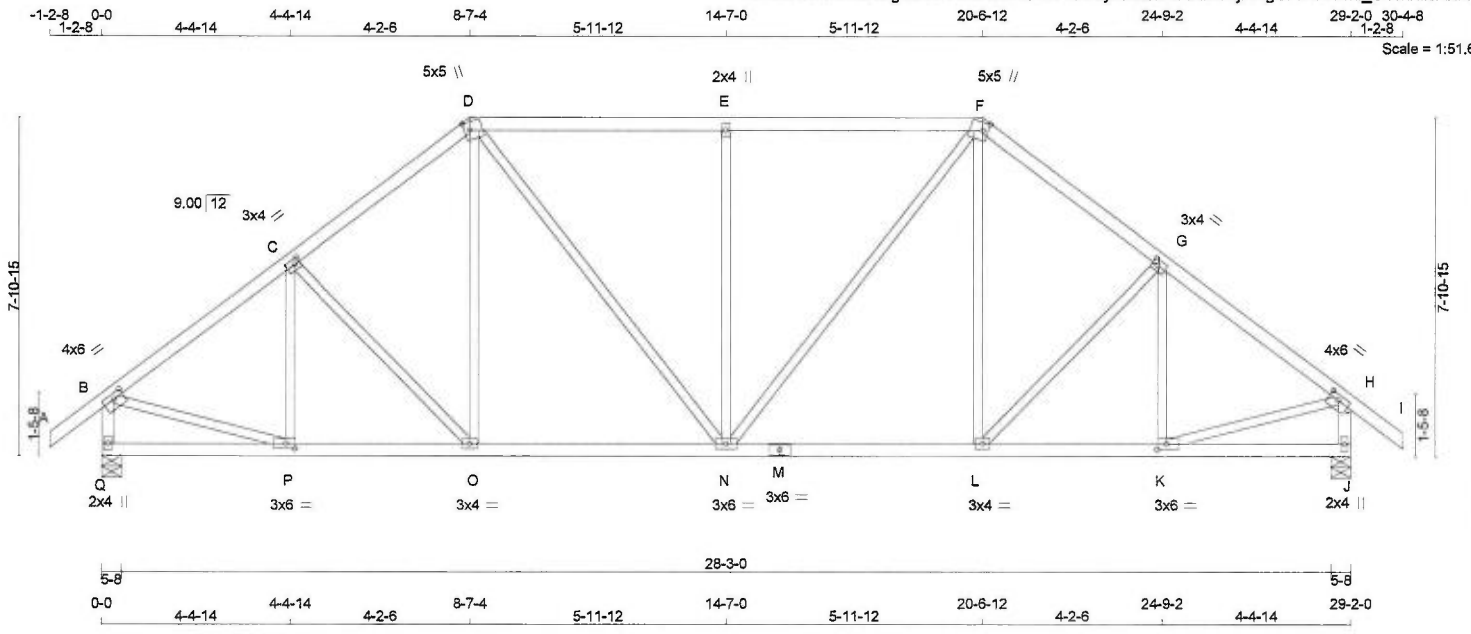
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



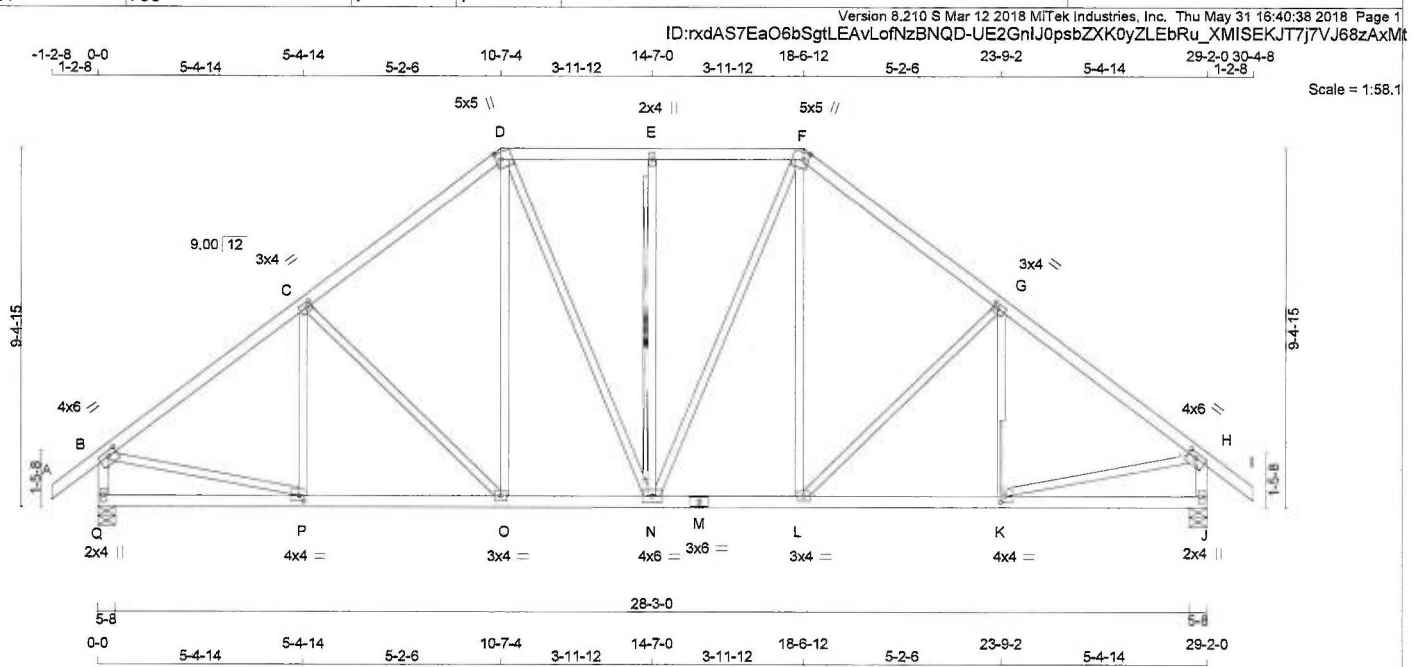
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - I 2x4 DRY No.2 SPF Q - B 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF Q - M 2x4 DRY No.2 SPF M - J 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.							DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td></td><td>FACTORED GROSS REACTION</td><td>MAXIMUM FACTORED GROSS REACTION</td><td>INPUT BRG</td><td>REQRD BRG</td></tr><tr><td>JT</td><td>VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td></tr><tr><td>Q</td><td>1484</td><td>0</td><td>1484</td><td>0</td></tr><tr><td>J</td><td>1484</td><td>0</td><td>1484</td><td>0</td></tr></table> UNFACTORED REACTIONS <table><tr><td></td><td>1ST LCASE</td><td colspan="6">MAX./MIN. COMPONENT REACTIONS</td></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>Q</td><td>1039</td><td>740 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>299 / 0</td><td>0 / 0</td></tr><tr><td>J</td><td>1039</td><td>740 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>299 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><td colspan="4">CHORDS</td><td colspan="4">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>FACTORED VERT. LOAD (PLF)</td><td>MAX LC1 CSI (LC)</td><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>MAX UNBRACED LENGTH</td><td>MAX FACTORED CSI (LC)</td></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 30</td><td>-77.4</td><td>-77.4 0.09 (1)</td><td>10.00</td><td>P-C</td><td>-235 / 0</td><td>0.08 (1)</td></tr><tr><td>B-C</td><td>-1488 / 0</td><td>-77.4</td><td>-77.4 0.21 (1)</td><td>5.18</td><td>C-O</td><td>-146 / 0</td><td>0.10 (1)</td></tr><tr><td>C-D</td><td>-1411 / 0</td><td>-77.4</td><td>-77.4 0.20 (1)</td><td>5.29</td><td>O-D</td><td>0 / 197</td><td>0.04 (1)</td></tr><tr><td>D-E</td><td>-1374 / 0</td><td>-77.4</td><td>-77.4 0.38 (1)</td><td>5.10</td><td>D-N</td><td>0 / 427</td><td>0.10 (1)</td></tr><tr><td>E-F</td><td>-1374 / 0</td><td>-77.4</td><td>-77.4 0.38 (1)</td><td>5.10</td><td>N-E</td><td>-566 / 0</td><td>0.67 (1)</td></tr><tr><td>F-G</td><td>-1411 / 0</td><td>-77.4</td><td>-77.4 0.20 (1)</td><td>5.29</td><td>N-F</td><td>0 / 427</td><td>0.10 (1)</td></tr><tr><td>G-H</td><td>-1488 / 0</td><td>-77.4</td><td>-77.4 0.21 (1)</td><td>5.18</td><td>L-F</td><td>0 / 197</td><td>0.04 (1)</td></tr><tr><td>H-I</td><td>0 / 30</td><td>-77.4</td><td>-77.4 0.09 (1)</td><td>10.00</td><td>L-G</td><td>-146 / 0</td><td>0.10 (1)</td></tr><tr><td>Q-B</td><td>-1449 / 0</td><td>0.0</td><td>0.0 0.15 (1)</td><td>6.79</td><td>K-G</td><td>-235 / 0</td><td>0.08 (1)</td></tr><tr><td>J-H</td><td>-1449 / 0</td><td>0.0</td><td>0.0 0.15 (1)</td><td>6.79</td><td>B-P</td><td>0 / 1248</td><td>0.28 (1)</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>K-H</td><td>0 / 1248</td><td>0.28 (1)</td></tr><tr><td>Q-P</td><td>0 / 0</td><td>-17.5</td><td>-17.5 0.07 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>P-O</td><td>0 / 1209</td><td>-17.5</td><td>-17.5 0.25 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 1110</td><td>-17.5</td><td>-17.5 0.24 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 1110</td><td>-17.5</td><td>-17.5 0.24 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>M-L</td><td>0 / 1110</td><td>-17.5</td><td>-17.5 0.24 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>L-K</td><td>0 / 1209</td><td>-17.5</td><td>-17.5 0.25 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>K-J</td><td>0 / 0</td><td>-17.5</td><td>-17.5 0.07 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></table>								FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	HORZ	DOWN	HORZ	Q	1484	0	1484	0	J	1484	0	1484	0		1ST LCASE	MAX./MIN. COMPONENT REACTIONS						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	Q	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0	J	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	MAX FACTORED CSI (LC)	FR-TO		FROM TO		FR-TO				A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	P-C	-235 / 0	0.08 (1)	B-C	-1488 / 0	-77.4	-77.4 0.21 (1)	5.18	C-O	-146 / 0	0.10 (1)	C-D	-1411 / 0	-77.4	-77.4 0.20 (1)	5.29	O-D	0 / 197	0.04 (1)	D-E	-1374 / 0	-77.4	-77.4 0.38 (1)	5.10	D-N	0 / 427	0.10 (1)	E-F	-1374 / 0	-77.4	-77.4 0.38 (1)	5.10	N-E	-566 / 0	0.67 (1)	F-G	-1411 / 0	-77.4	-77.4 0.20 (1)	5.29	N-F	0 / 427	0.10 (1)	G-H	-1488 / 0	-77.4	-77.4 0.21 (1)	5.18	L-F	0 / 197	0.04 (1)	H-I	0 / 30	-77.4	-77.4 0.09 (1)	10.00	L-G	-146 / 0	0.10 (1)	Q-B	-1449 / 0	0.0	0.0 0.15 (1)	6.79	K-G	-235 / 0	0.08 (1)	J-H	-1449 / 0	0.0	0.0 0.15 (1)	6.79	B-P	0 / 1248	0.28 (1)						K-H	0 / 1248	0.28 (1)	Q-P	0 / 0	-17.5	-17.5 0.07 (4)	10.00				P-O	0 / 1209	-17.5	-17.5 0.25 (1)	10.00				O-N	0 / 1110	-17.5	-17.5 0.24 (1)	10.00				N-M	0 / 1110	-17.5	-17.5 0.24 (1)	10.00				M-L	0 / 1110	-17.5	-17.5 0.24 (1)	10.00				L-K	0 / 1209	-17.5	-17.5 0.25 (1)	10.00				K-J	0 / 0	-17.5	-17.5 0.07 (4)	10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.0 PSF TOTAL LOAD = 33.3 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.97") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.97") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10") CSI: TC=0.38/1.00 (D-E:1) , BC=0.25/1.00 (O-P:1) , WB=0.67/1.00 (E-N:1) , SSI=0.22/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.87 (N) (INPUT = 0.90) JSI METAL= 0.41 (B) (INPUT = 1.00)						
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG																																																																																																																																																																																																																																												
JT	VERT	HORZ	DOWN	HORZ																																																																																																																																																																																																																																												
Q	1484	0	1484	0																																																																																																																																																																																																																																												
J	1484	0	1484	0																																																																																																																																																																																																																																												
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS																																																																																																																																																																																																																																														
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Q	1039	740 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0																																																																																																																																																																																																																																									
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CHORDS				WEBS																																																																																																																																																																																																																																												
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	MAX FACTORED CSI (LC)																																																																																																																																																																																																																																									
FR-TO		FROM TO		FR-TO																																																																																																																																																																																																																																												
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F-G	-1411 / 0	-77.4	-77.4 0.20 (1)	5.29	N-F	0 / 427	0.10 (1)																																																																																																																																																																																																																																									
G-H	-1488 / 0	-77.4	-77.4 0.21 (1)	5.18	L-F	0 / 197	0.04 (1)																																																																																																																																																																																																																																									
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Q-B	-1449 / 0	0.0	0.0 0.15 (1)	6.79	K-G	-235 / 0	0.08 (1)																																																																																																																																																																																																																																									
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Q-P	0 / 0	-17.5	-17.5 0.07 (4)	10.00																																																																																																																																																																																																																																												
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N-M	0 / 1110	-17.5	-17.5 0.24 (1)	10.00																																																																																																																																																																																																																																												
M-L	0 / 1110	-17.5	-17.5 0.24 (1)	10.00																																																																																																																																																																																																																																												
L-K	0 / 1209	-17.5	-17.5 0.25 (1)	10.00																																																																																																																																																																																																																																												
K-J	0 / 0	-17.5	-17.5 0.07 (4)	10.00																																																																																																																																																																																																																																												





READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
Q - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75	
C	TMVW-t	MT20	3.0	4.0	1.50	1.50	
D	TTWW+m	MT20	5.0	5.0	2.25	1.50	
E	TMVW-t	MT20	2.0	4.0			
F	TTWW+m	MT20	5.0	5.0	2.25	1.50	
G	TMVW-t	MT20	3.0	4.0	1.50	1.50	
H	TMVW-t	MT20	4.0	6.0	1.50	2.75	
J	BMV1+p	MT20	2.0	4.0			
K	BMVW-t	MT20	4.0	4.0	1.75	1.50	
L	BMVW-t	MT20	3.0	4.0			
M	BS-t	MT20	3.0	6.0			
N	BMVWWW-t	MT20	4.0	6.0			
O	BMVW-t	MT20	3.0	4.0			
P	BMVW-t	MT20	4.0	4.0	1.75	1.50	
Q	BMV1+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	1484	0	1484	0
J	1484	0	1484	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
Q	1039	740 / 0	0 / 0	0 / 0	299 / 0
J	1039	740 / 0	0 / 0	0 / 0	299 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF NO.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	P-C	-154 / 45	0.07 (1)
B-C	-1513 / 0	-77.4	-77.4	0.32 (1)	5.01	C-O	-290 / 0	0.31 (1)
C-D	-1315 / 0	-77.4	-77.4	0.31 (1)	5.30	O-D	0 / 284	0.06 (1)
D-E	-1123 / 0	-77.4	-77.4	0.16 (1)	5.82	D-N	0 / 234	0.05 (1)
E-F	-1123 / 0	-77.4	-77.4	0.16 (1)	5.82	N-E	-368 / 0	0.24 (1)
F-G	-1315 / 0	-77.4	-77.4	0.31 (1)	5.30	N-F	0 / 234	0.05 (1)
G-H	-1513 / 0	-77.4	-77.4	0.32 (1)	5.01	L-F	0 / 284	0.06 (1)
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-G	-290 / 0	0.31 (1)
Q-B	-1444 / 0	0.0	0.0	0.15 (1)	6.80	K-G	-154 / 45	0.07 (1)
J-H	-1444 / 0	0.0	0.0	0.15 (1)	6.80	B-P	0 / 1260	0.28 (1)
Q-P	0 / 0	-17.5	-17.5	0.12 (4)	10.00	K-H	0 / 1260	0.28 (1)
P-O	0 / 1234	-17.5	-17.5	0.26 (1)	10.00			
O-N	0 / 1030	-17.5	-17.5	0.21 (1)	10.00			
N-M	0 / 1030	-17.5	-17.5	0.21 (1)	10.00			
M-L	0 / 1030	-17.5	-17.5	0.21 (1)	10.00			
L-K	0 / 1234	-17.5	-17.5	0.26 (1)	10.00			
K-J	0 / 0	-17.5	-17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.32/1.00 (G-H:1), BC=0.26/1.00 (K-L:1), WB=0.31/1.00 (G-L:1), SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

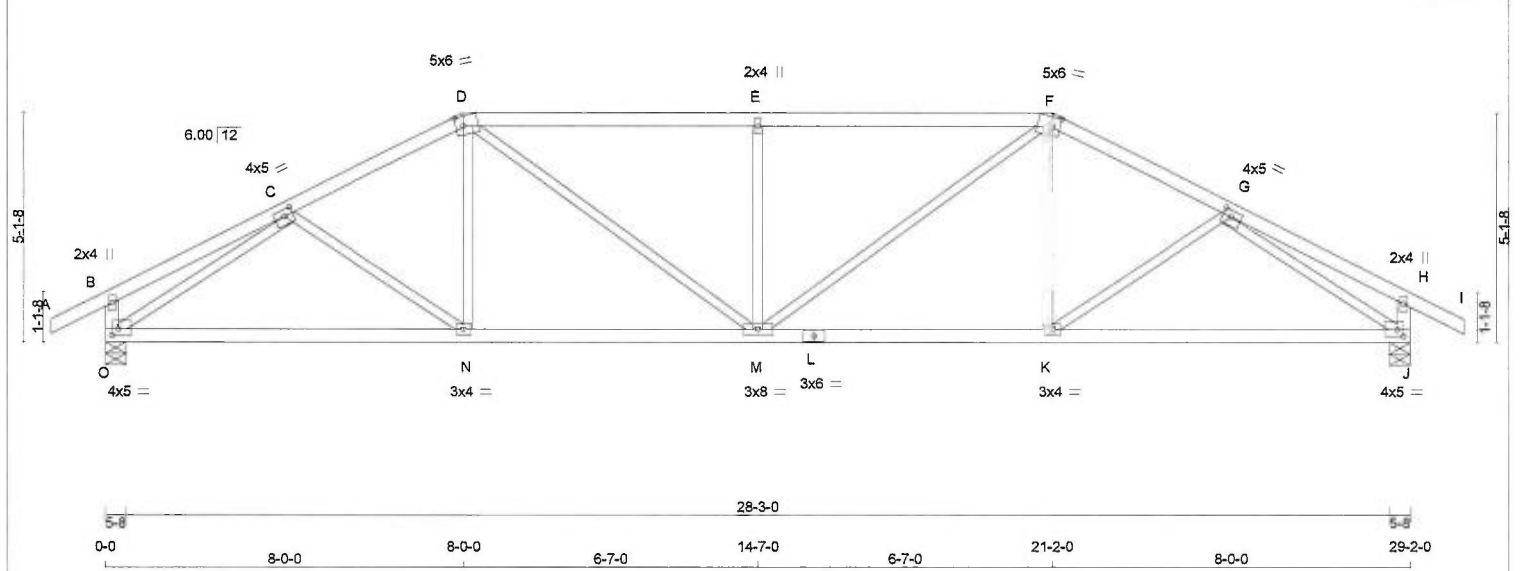
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.46 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 112 lb [M/F]

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF	SPF	GROSS REACTION	INPUT
D - F	2x4	DRY	No.2	SPF	SPF	VERT	BRG
F - I	2x4	DRY	No.2	SPF	SPF	HORZ	BRG
O - B	2x4	DRY	No.2	SPF	SPF	DOWN	IN-SX
J - H	2x4	DRY	No.2	SPF	SPF	UPLIFT	IN-SX
O - L	2x4	DRY	No.2	SPF	SPF		
L - J	2x4	DRY	No.2	SPF	SPF		

UNFACTORED REACTIONS				DESIGN CRITERIA			
1ST LCASE	MAX./MIN.	COMPONENT REACTIONS		SPECIFIED LOADS:			
JT	COMBINED	SNOW	LIVE	TOP CH.	LL	=	23.3 PSF
O	1038	739 / 0	0 / 0	DL	=	3.0 PSF	
J	1038	739 / 0	0 / 0	BOT CH.	LL	=	0.0 PSF
				DL	=	7.0 PSF	
				TOTAL LOAD	=	33.3 PSF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	5.0	1.75	2.25
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMVW-t	MT20	4.0	5.0	1.75	2.25
H	TMV+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	5.0	1.75	1.75
K	BMVW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	4.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	4.0	5.0	1.75	1.75

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD	LC1 MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	C-N	-29 / 57	0.02 (4)	
B-C	0 / 14	-77.4	-77.4 0.17 (1)	N-D	0 / 169	0.05 (4)	
C-D	-1898 / 0	-77.4	-77.4 0.18 (1)	D-M	0 / 605	0.14 (1)	
D-E	-2175 / 0	-77.4	-77.4 0.52 (1)	M-E	-626 / 0	0.24 (1)	
E-F	-2175 / 0	-77.4	-77.4 0.52 (1)	M-F	0 / 605	0.14 (1)	
F-G	-1898 / 0	-77.4	-77.4 0.18 (1)	K-F	0 / 169	0.05 (4)	
G-H	0 / 14	-77.4	-77.4 0.17 (1)	K-G	-29 / 57	0.02 (4)	
H-I	0 / 22	-77.4	-77.4 0.09 (1)	O-C	-2088 / 0	0.82 (1)	
O-B	-223 / 0	0.0	0.0 0.02 (1)	G-J	-2088 / 0	0.82 (1)	
J-H	-223 / 0	0.0	0.0 0.02 (1)				
O-N	0 / 1708	-17.5	-17.5 0.41 (1)				
N-M	0 / 1688	-17.5	-17.5 0.41 (1)				
M-L	0 / 1688	-17.5	-17.5 0.41 (1)				
L-K	0 / 1688	-17.5	-17.5 0.41 (1)				
K-J	0 / 1708	-17.5	-17.5 0.41 (1)				

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.19")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.41/1.00 (N-O:1) ,
WB=0.82/1.00 (C-O:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

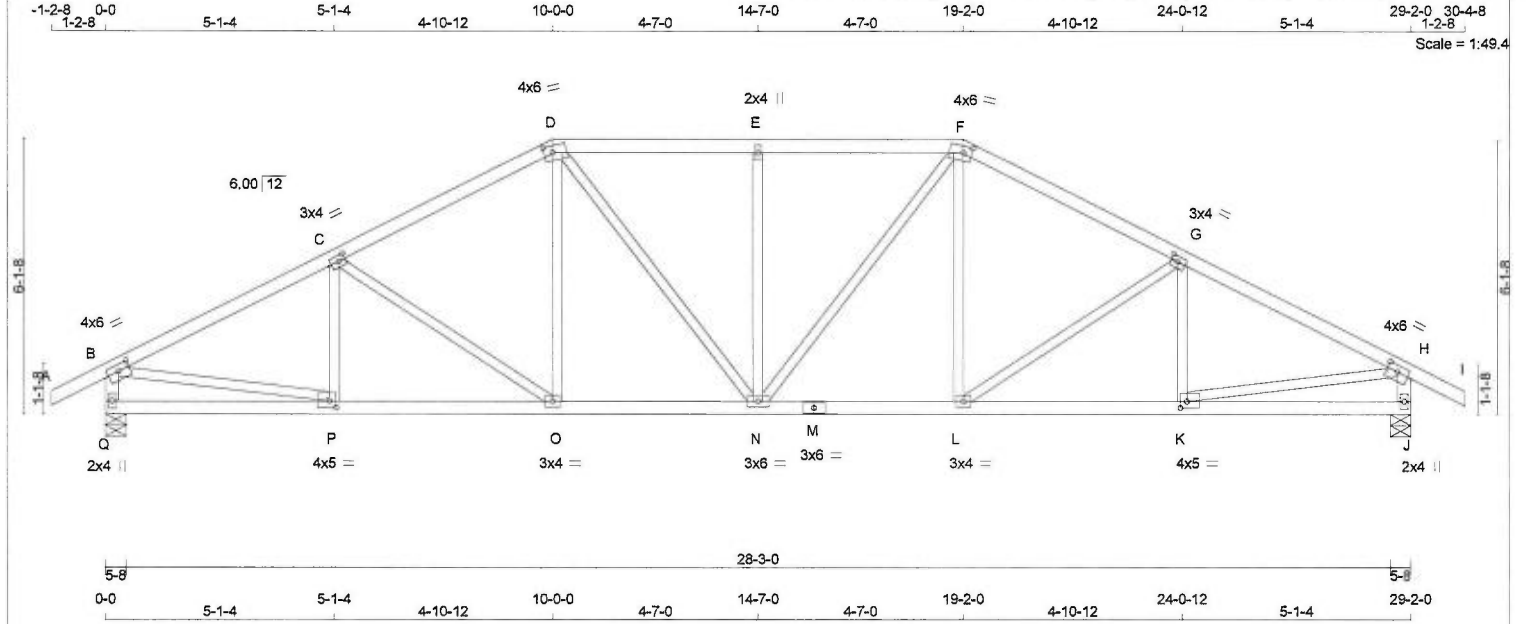
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(Psi) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.61 (C) (INPUT = 1.00)



TOTAL WEIGHT = 121 lb [M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	1482	0	1482	0	0
J	1482	0	1482	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
Q	1038	739 / 0	0 / 0
J	1038	739 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1 MAX	MAX. UNBRAC	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CS1 (LC)	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH	FR-TO
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	P-C	-197 / 26	0.04 (1)
B-C	-1995 / 0	-77.4	-77.4	0.30 (1)	4.52	C-O	-288 / 0	0.18 (1)
C-D	-1769 / 0	-77.4	-77.4	0.28 (1)	4.76	O-D	0 / 249	0.06 (1)
D-E	-1765 / 0	-77.4	-77.4	0.24 (1)	4.80	D-N	0 / 319	0.07 (1)
E-F	-1765 / 0	-77.4	-77.4	0.24 (1)	4.80	N-E	-430 / 0	0.25 (1)
F-G	-1769 / 0	-77.4	-77.4	0.28 (1)	4.76	E-F	0 / 319	0.07 (1)
G-H	-1995 / 0	-77.4	-77.4	0.30 (1)	4.52	L-F	0 / 249	0.06 (1)
H-I	0 / 22	-77.4	-77.4	0.09 (1)	10.00	L-G	-288 / 0	0.18 (1)
Q-B	-1442 / 0	0.0	0.0	0.15 (1)	6.81	K-G	-197 / 26	0.04 (1)
J-H	-1442 / 0	0.0	0.0	0.15 (1)	6.81	B-P	0 / 1825	0.41 (1)
						K-H	0 / 1825	0.41 (1)
Q-P	0 / 0	-17.5	-17.5	0.10 (4)	10.00			
P-O	0 / 1802	-17.5	-17.5	0.34 (1)	10.00			
O-N	0 / 1567	-17.5	-17.5	0.30 (1)	10.00			
N-M	0 / 1567	-17.5	-17.5	0.30 (1)	10.00			
M-L	0 / 1567	-17.5	-17.5	0.30 (1)	10.00			
L-K	0 / 1802	-17.5	-17.5	0.34 (1)	10.00			
K-J	0 / 0	-17.5	-17.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.30/1.00 (B-C:1), BC=0.34/1.00 (O-P:1), WB=0.41/1.00 (B-P:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

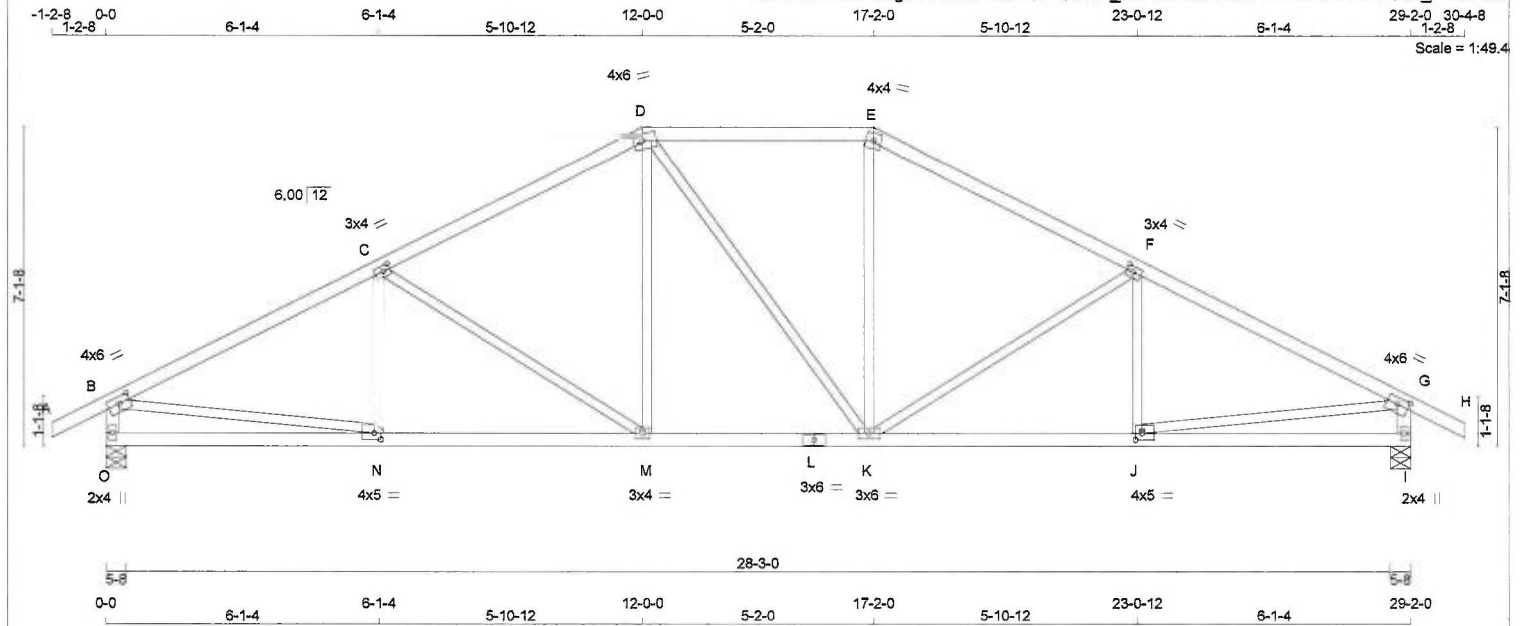
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.54 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTWW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	3.0	4.0	1.50	1.75
G	TMWW-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	5.0	1.75	1.75
K	BMWW-t	MT20	3.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	3.0	4.0		
N	BMWW-t	MT20	4.0	5.0	1.75	1.75
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1482	0	1482	0
I	1482	0	1482	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
I	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. FACTORED (LC1)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 22	-77.4	-77.4 0.09 (1)	N-C	-128 / 64	0.03 (1)	10.00
B-C	-2015 / 0	-77.4	-77.4 0.44 (1)	C-M	-465 / 0	0.46 (1)	4.34
C-D	-1628 / 0	-77.4	-77.4 0.40 (1)	M-D	0 / 346	0.08 (1)	4.77
D-E	-1439 / 0	-77.4	-77.4 0.29 (1)	D-K	0 / 0	0.00 (1)	5.13
E-F	-1628 / 0	-77.4	-77.4 0.40 (1)	K-E	0 / 347	0.08 (1)	4.77
F-G	-2015 / 0	-77.4	-77.4 0.44 (1)	K-F	-464 / 0	0.46 (1)	4.34
G-H	0 / 22	-77.4	-77.4 0.09 (1)	J-F	-129 / 63	0.03 (1)	10.00
O-B	-1437 / 0	0.0	0.0 0.14 (1)	B-N	0 / 1840	0.41 (1)	6.81
I-G	-1436 / 0	0.0	0.0 0.14 (1)	J-G	0 / 1840	0.41 (1)	6.81
O-N	0 / 0	-17.5	-17.5 0.15 (4)				10.00
N-M	0 / 1824	-17.5	-17.5 0.36 (1)				10.00
M-L	0 / 1438	-17.5	-17.5 0.29 (1)				10.00
L-K	0 / 1438	-17.5	-17.5 0.29 (1)				10.00
K-J	0 / 1823	-17.5	-17.5 0.36 (1)				10.00
J-I	0 / 0	-17.5	-17.5 0.15 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. / C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CSI: TC=0.44/1.00 (B-C:1), BC=0.36/1.00 (M-N:1),
WB=0.46/1.00 (C-M:1), SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

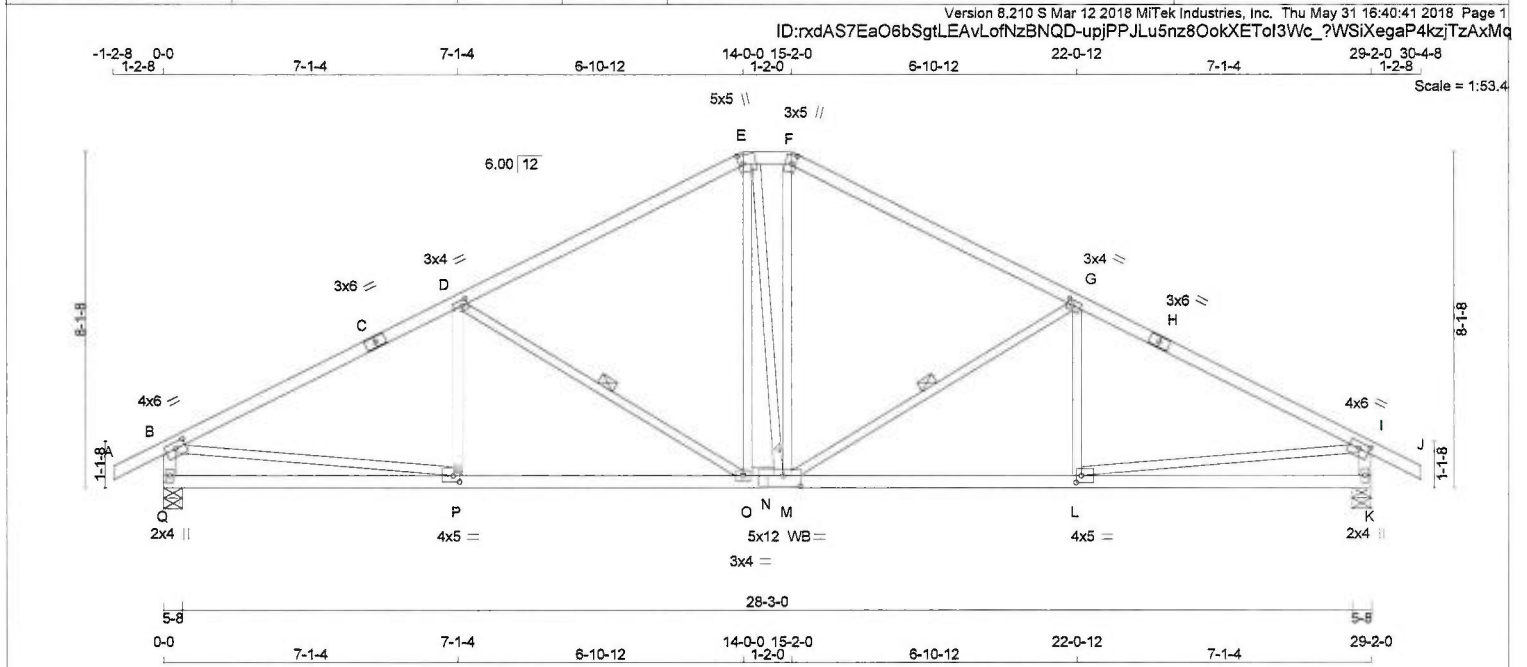
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	BRG	BRG
C - E	2x4	DRY	No.2	SPF	VERT	DOWN	IN-SX
E - F	2x4	DRY	No.2	SPF	Q	1482	0
F - H	2x4	DRY	No.2	SPF	K	1482	0
H - J	2x4	DRY	No.2	SPF			
Q - B	2x4	DRY	No.2	SPF			
K - I	2x4	DRY	No.2	SPF			
Q - N	2x4	DRY	No.2	SPF			
N - K	2x4	DRY	No.2	SPF			

ALL WEBS EXCEPT DRY: SEASONED LUMBER. TOTAL WEIGHT = 125 lb [M][F]

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	MT20	4.0	6.0	1.75
B TMVW-t	MT20	3.0	6.0	
C TS-t	MT20	3.0	4.0	1.50
D TMVW-t	MT20	5.0	5.0	2.50
E TTWW+m	MT20	3.0	4.0	1.50
F TTWW+m	MT20	3.0	4.0	1.50
G TMVW-t	MT20	4.0	6.0	1.75
H TS-t	MT20	3.0	6.0	
I TMVW-t	MT20	4.0	6.0	1.75
K BMV1+p	MT20	2.0	4.0	
L BMVW-t	MT20	4.0	5.0	2.00
M				
N BSWWW-t	MT20	5.0	12.0	3.00
O BMVW-t	MT20	3.0	4.0	
P BMVW-t	MT20	4.0	5.0	1.75
Q BMV1+p	MT20	2.0	4.0	

WB - INDICATES BLOCKING REQUIRED



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
K	1038	739 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, G-M. DBS = 20-0-0. CBF = 79 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.
FR-TO								FR-TO							
A-B	0 / 22	-77.4	-77.4	0.09	(1)	10.00	P-D	-70 / 98	0.03	(4)					
B-C	-2011 / 0	-77.4	-77.4	0.62	(1)	4.11	D-O	-633 / 0	0.29	(1)					
C-D	-2011 / 0	-77.4	-77.4	0.62	(1)	4.11	O-E	0 / 373	0.08	(1)					
D-E	-1470 / 0	-77.4	-77.4	0.54	(1)	4.74	E-M	0 / 27	0.01	(1)					
E-F	-1296 / 0	-77.4	-77.4	0.03	(1)	5.66	M-F	0 / 401	0.09	(1)					
F-G	-1473 / 0	-77.4	-77.4	0.55	(1)	4.74	G-M	-627 / 0	0.29	(1)					
G-H	-2009 / 0	-77.4	-77.4	0.62	(1)	4.11	L-G	-75 / 96	0.03	(4)					
H-I	-2009 / 0	-77.4	-77.4	0.62	(1)	4.11	B-P	0 / 1836	0.41	(1)					
I-J	0 / 22	-77.4	-77.4	0.09	(1)	10.00	L-I	0 / 1835	0.41	(1)					
Q-B	-1431 / 0	0.0	0.0	0.14	(1)	6.82									
K-I	-1430 / 0	0.0	0.0	0.14	(1)	6.82									
Q-P	0 / 0	-17.5	-17.5	0.22	(4)	10.00									
P-O	0 / 1824	-17.5	-17.5	0.40	(1)	10.00									
O-N	0 / 1292	-17.5	-17.5	0.30	(1)	10.00									
N-M	0 / 1292	-17.5	-17.5	0.30	(1)	10.00									
M-L	0 / 1823	-17.5	-17.5	0.39	(1)	10.00									
L-K	0 / 0	-17.5	-17.5	0.21	(4)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.62/1.00 (B-D:1), BC=0.40/1.00 (O-P:1), WB=0.41/1.00 (B-P:1), SSI=0.24/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

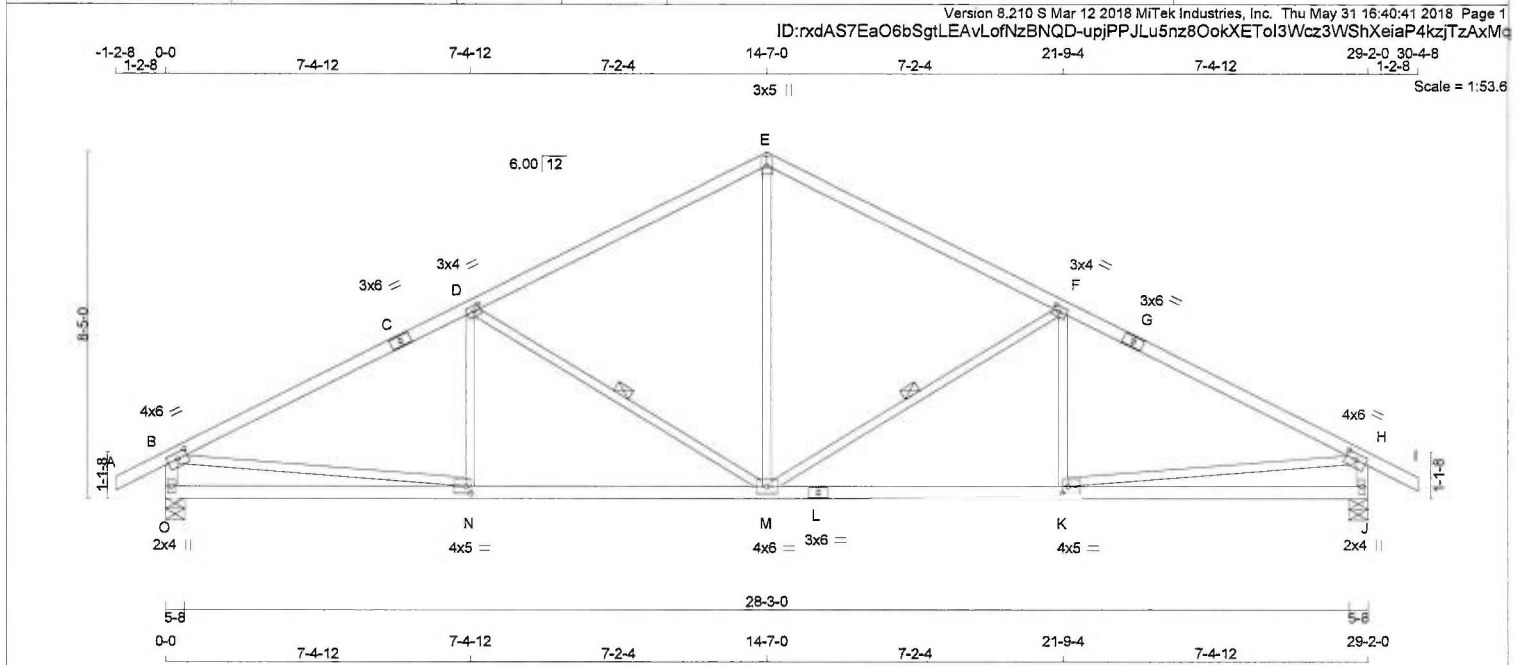
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.55 (P) (INPUT = 1.00)





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00	
C	TS-t	MT20	3.0	6.0			
D	TMVW-t	MT20	3.0	4.0	1.50	1.75	
E	TTW+p	MT20	3.0	5.0			
F	TMVW-t	MT20	3.0	4.0	1.50	1.75	
G	TS-t	MT20	3.0	6.0			
H	TMVW-t	MT20	4.0	6.0	1.75	3.00	
J	BMV1+p	MT20	2.0	4.0			
K	TMVW-t	MT20	4.0	5.0	2.00	1.50	
L	BS-t	MT20	3.0	6.0			
M	BMVWW-t	MT20	4.0	6.0			
N	BMVW-t	MT20	4.0	5.0	2.00	1.50	
O	BMV1+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
O	1482	0	1482	0
J	1482	0	1482	0

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
O	1038	739 / 0	0 / 0	0 / 0	0 / 0
J	1038	739 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0, CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. FACTORED UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00	M-E	0 / 835
B-C	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	M-F	-669 / 0
C-D	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	K-F	-61 / 103
D-E	-1431 / 0	-77.4	-77.4	0.59 (1)	4.70	D-M	-669 / 0
E-F	-1431 / 0	-77.4	-77.4	0.59 (1)	4.70	N-D	-61 / 103
F-G	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	B-N	0 / 1830
G-H	-2004 / 0	-77.4	-77.4	0.68 (1)	4.03	K-H	0 / 1830
H-I	0 / 22	-77.4	-77.4	0.09 (1)	10.00		
O-B	-1428 / 0	0.0	0.0	0.14 (1)	6.83		
J-H	-1428 / 0	0.0	0.0	0.14 (1)	6.83		
O-N	0 / 0	-17.5	-17.5	0.22 (4)	10.00		
N-M	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
M-L	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
L-K	0 / 1819	-17.5	-17.5	0.40 (1)	10.00		
K-J	0 / 0	-17.5	-17.5	0.22 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.0	PSF	
TOTAL LOAD = 33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.68/1.00 (B-D:1), BC=0.40/1.00 (M-N:1), WB=0.41/1.00 (B-N:1), SSI=0.25/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

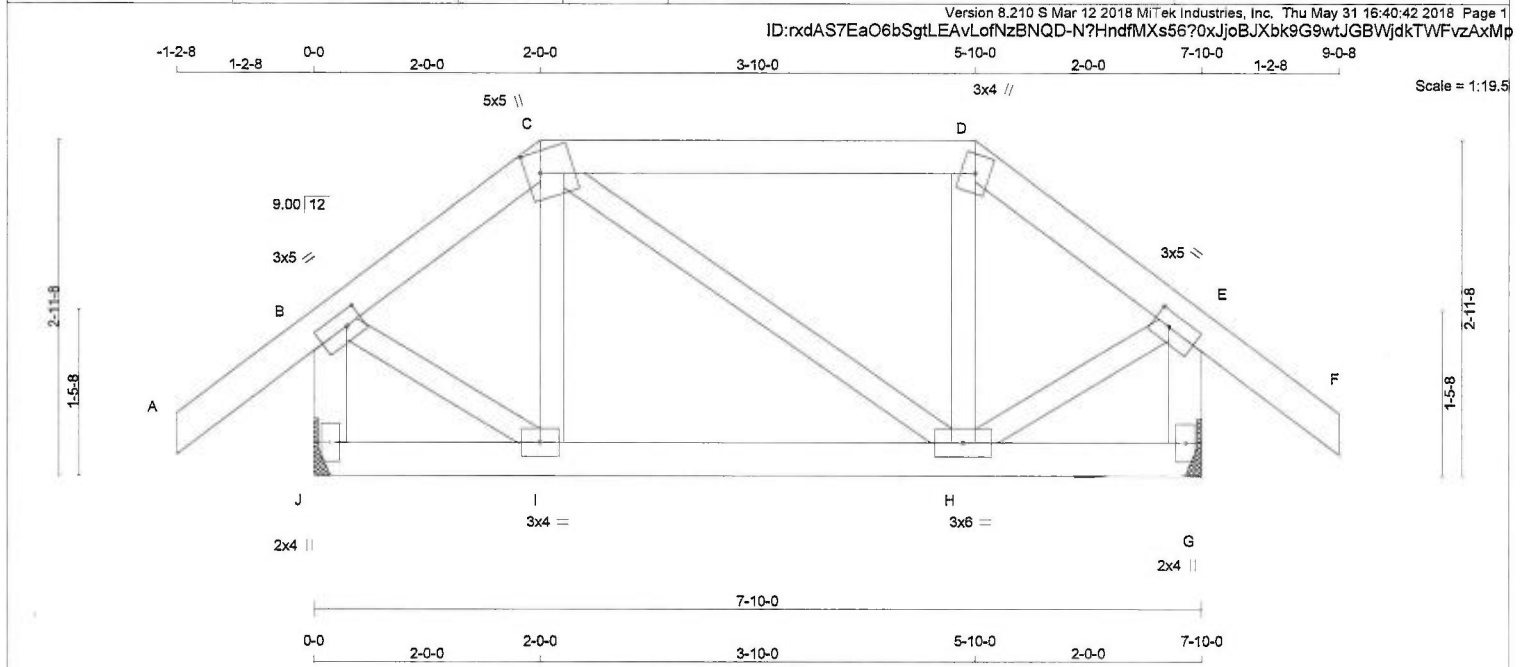
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-t	MT20	3.0	5.0	1.50
C	TTWW+m	MT20	5.0	5.0	2.25
D	TTWW+m	MT20	3.0	4.0	
E	TMVW-t	MT20	3.0	5.0	1.50
G	BMV1+p	MT20	2.0	4.0	
H	BMVWW-t	MT20	3.0	6.0	
I	BMVWW-t	MT20	3.0	4.0	
J	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	472	0	472	0	0
G	472	0	472	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J, G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	329	243 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
G	329	243 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1	LC	UNBRAC	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1	LC
FR-TO	MEMB.	FORCE (LBS)	FROM	TO	PLF	CS1 (LC)	LENGTH	FR-TO	MEMB.	FORCE (LBS)	FROM	TO	PLF	CS1 (LC)	LENGTH	FR-TO	MEMB.
A-B	0 / 30	-77.4	-77.4	0.09	(1)	10.00	I-C	-60 / 27	0.01	(1)							
B-C	-273 / 0	-77.4	-77.4	0.04	(1)	6.25	C-H	0 / 0	0.00	(1)							
C-D	-215 / 0	-77.4	-77.4	0.15	(1)	6.25	H-D	-61 / 28	0.01	(1)							
D-E	-273 / 0	-77.4	-77.4	0.04	(1)	6.25	B-I	0 / 247	0.06	(1)							
E-F	0 / 30	-77.4	-77.4	0.09	(1)	10.00	H-E	0 / 247	0.06	(1)							
J-B	-460 / 0	0.0	0.0	0.05	(1)	7.81											
G-E	-460 / 0	0.0	0.0	0.05	(1)	7.81											
J-I	0 / 0	-17.5	-17.5	0.04	(4)	10.00											
I-H	0 / 215	-17.5	-17.5	0.06	(1)	10.00											
H-G	0 / 0	-17.5	-17.5	0.04	(4)	10.00											

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (C-D:1), BC=0.06/1.00 (H-I:1), WB=0.06/1.00 (B-I:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

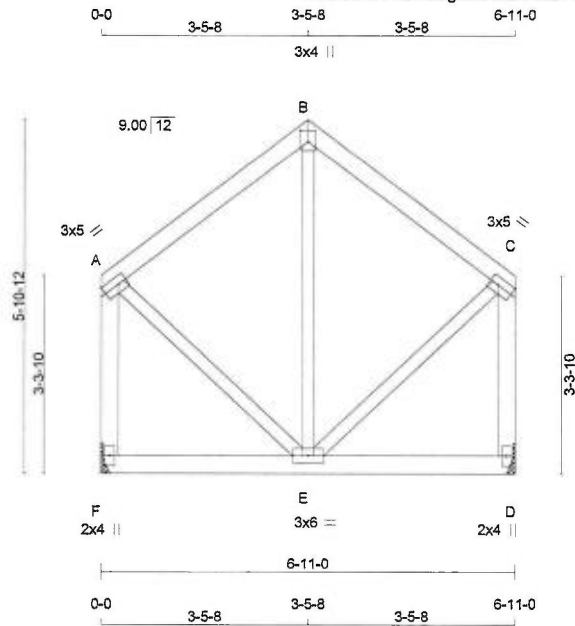
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (H) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:36.9

TOTAL WEIGHT = 36 lb
[M][F]

<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	Edge
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	Edge
D	BMV1+p	MT20	2.0	4.0		
E	BMVWWW-t	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
F	328	0	328	0	0	MECHANICAL	
D	328	0	328	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	230	161 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	230	161 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	-133 / 0	-77.4	-77.4	0.12 (1)	6.25	E-B	-108 / 24 0.06 (1)
B-C	-133 / 0	-77.4	-77.4	0.12 (1)	6.25	E-E	0 / 140 0.03 (1)
F-A	-304 / 0	0.0	0.0	0.06 (1)	7.81	E-C	0 / 140 0.03 (1)
D-C	-304 / 0	0.0	0.0	0.06 (1)	7.81		
F-E	0 / 0	-17.5	-17.5	0.06 (4)	10.00		
E-D	0 / 0	-17.5	-17.5	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) ,
WB=0.06/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

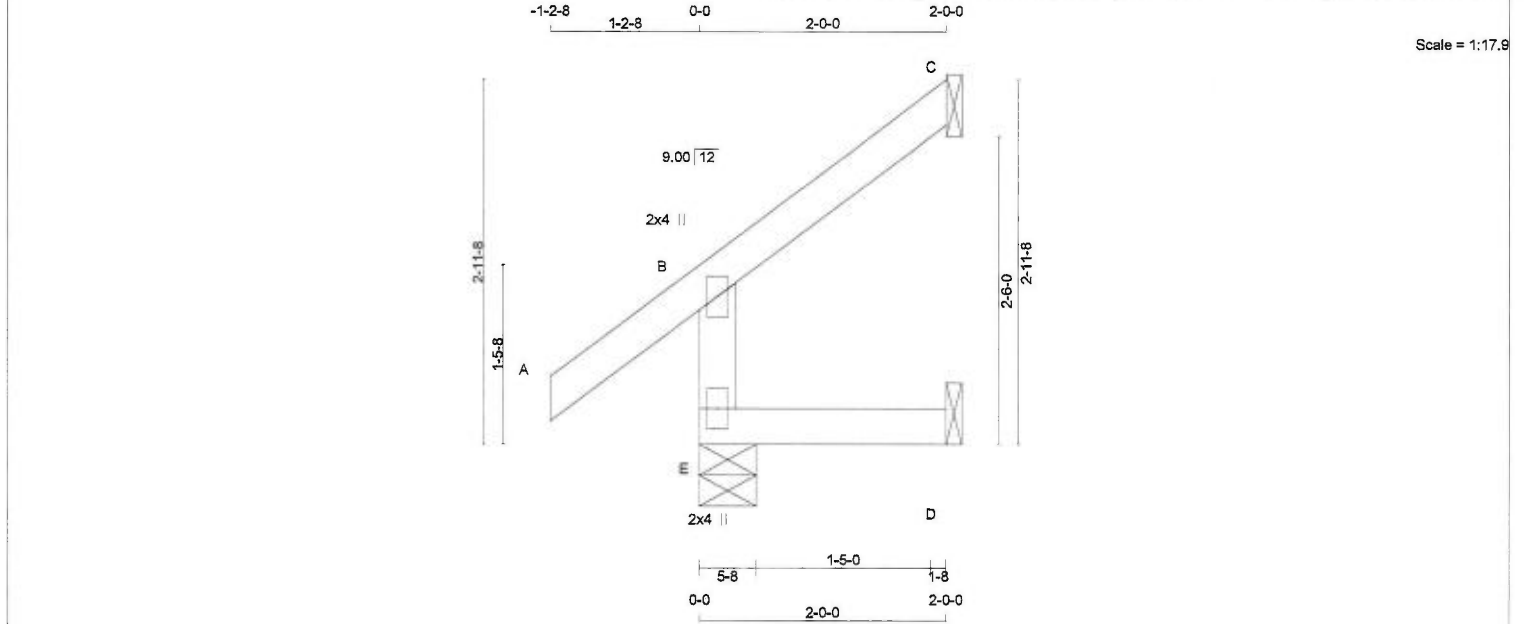
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (A) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 3 X 8 = 25 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DESCR. SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	216	0	216	0	0	5-8	5-8	
C	59	0	59	0	1-8	1-8	1-8	
D	16	0	16	0	1-8	1-8	1-8	

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
E	149	118 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	40	35 / 0	0 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	LC1 MAX (LC)	UNBRAC LENGTH
FR-TO		FROM	TO		FR-TO		FROM	TO
E-B	-196 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00			
B-C	-11 / 0	-77.4	-77.4	0.05 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





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Simpson Strong-Tie®
Component Solutions™

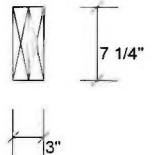
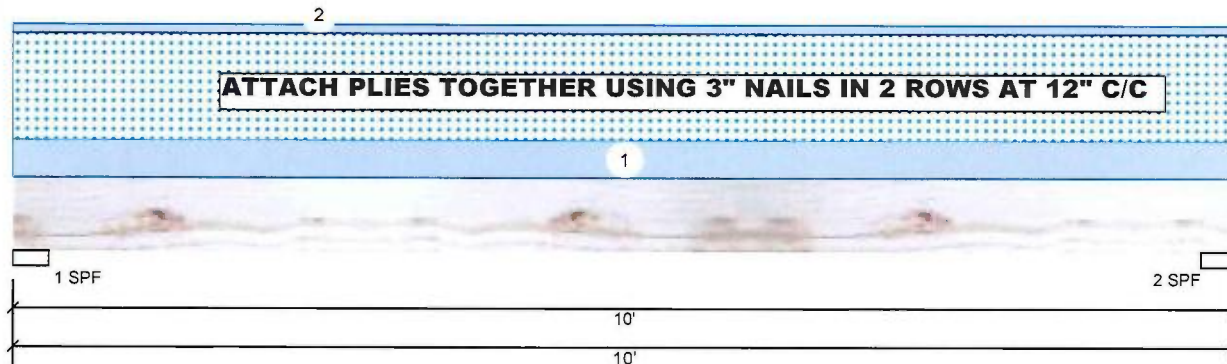
Client: **NE0518-224**
Project: **GREEN YORK HOMES-AMELIA 2 EL 2**
Address:

Date: 5/30/2018
Designer: B B
Job Name: B1
Project #:

Page 1 of 1
PAGE 32 OF 34

B1 S-P-F #2 2.000" X 8.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	480	Building Code:	NBCC 2010 / OBC 2012
Deflection TL:	240	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	265	610	0
2	0	265	610	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	3.500"	19% 331 / 915	1246 L	1.25D+1.5S
2 - SPF	3.500"	19% 331 / 915	1246 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2837 ft-lb	5'	4047 ft-lb	0.701 (70%)	1.25D+1.5S	L
Unbraced	2837 ft-lb	5'	3978 ft-lb	0.713 (71%)	1.25D+1.5S	L
Shear	1039 lb	9'2"	3406 lb	0.305 (30%)	1.25D+1.5S	L
LL Defl inch	0.173 (L/660)	5'	0.239 (L/480)	0.730 (73%)	S	L
TL Defl inch	0.249 (L/460)	5'	0.477 (L/240)	0.520 (52%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on full section width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform			Top	43 PLF	0 PLF	122 PLF	0 PLF	
2	Uniform			Top	10 PLF	0 PLF	0 PLF	0 PLF	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info



Kott Group
14 Anderson Blvd., On
L4A7X4
905-642-4400

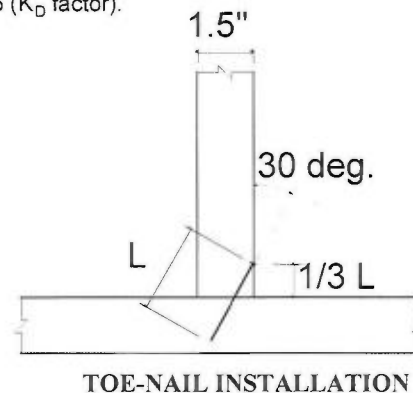
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

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Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

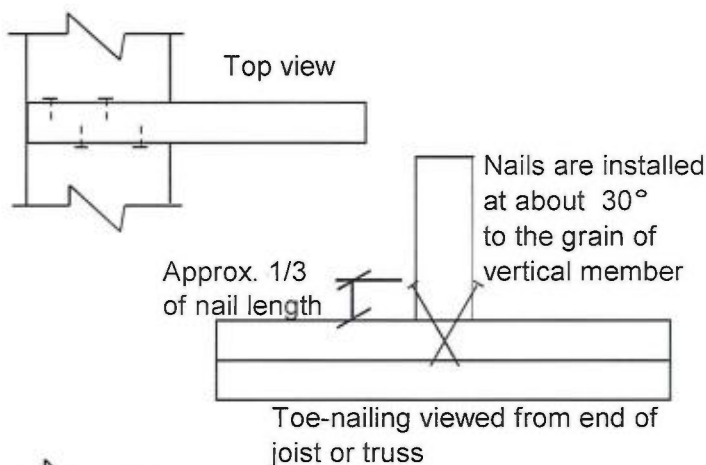
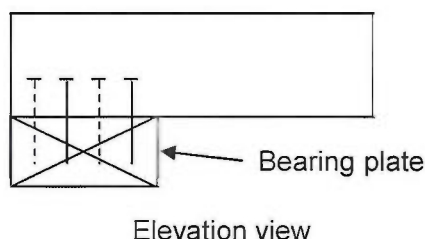
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

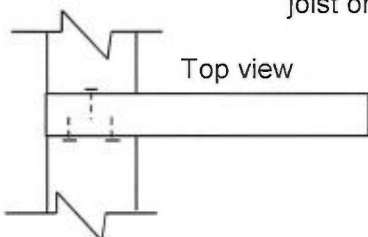
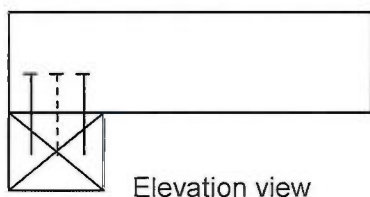
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017