

19-444465

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

**For conventional framing roof / ceilings
framing shall conform to OBC.9.23**

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

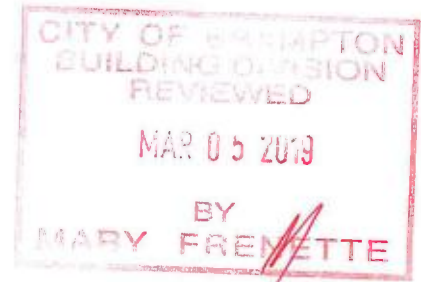


THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



For conventional framing roof / ceilings framing shall conform to OBC.9.23

All work shall conform to the Ontario Building Code O. Reg. 632/12 as amended



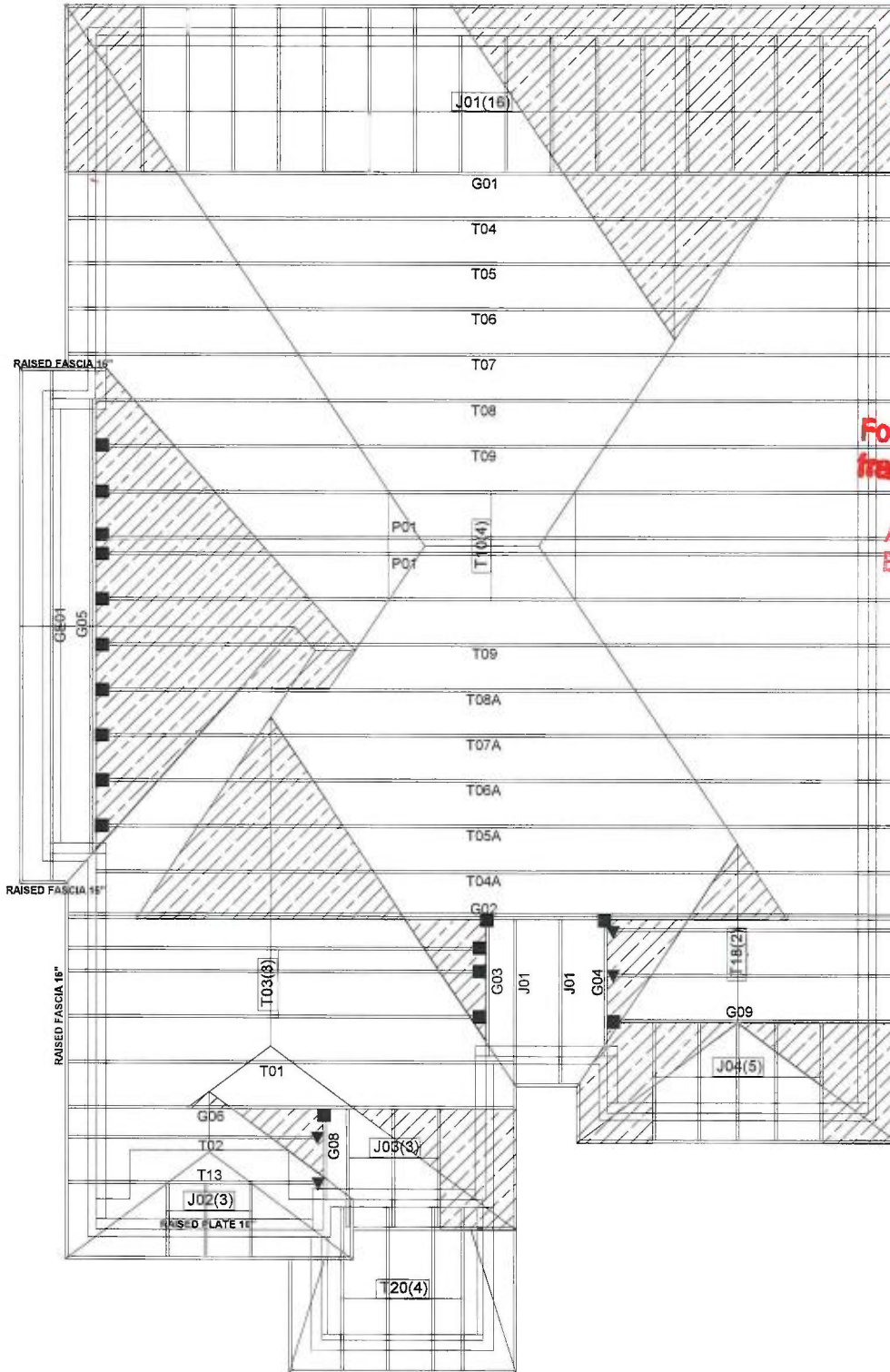
The components shown on this layout conform to our recommendations and are adequate to support the design loads required by the Ontario Building Code O.Reg 332/12 as amended and as shown on the engineering calculation page of each component.

The framing direction and girder locations shown on this layout conform to that shown on the architectural drawing.

Deflection: LL = L/360
1.33LL+DL=L/360

Architectural Drawing Info:
Date: MAY 22, 2018
Project number: 17-55
Model: CELESTIAL 2

19-444465-000-001.R.



CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24" O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

▼ LUS24 ■ LJS26DS
● HGUS26 ✕ HGUS26-2

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3 PSF
BOT CH. LL = 8 PSF
DL = 7 PSF
TOTAL LOAD = 33.3 PSF
SPACING = 24" O/C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA B54-05
- TPIC 2011

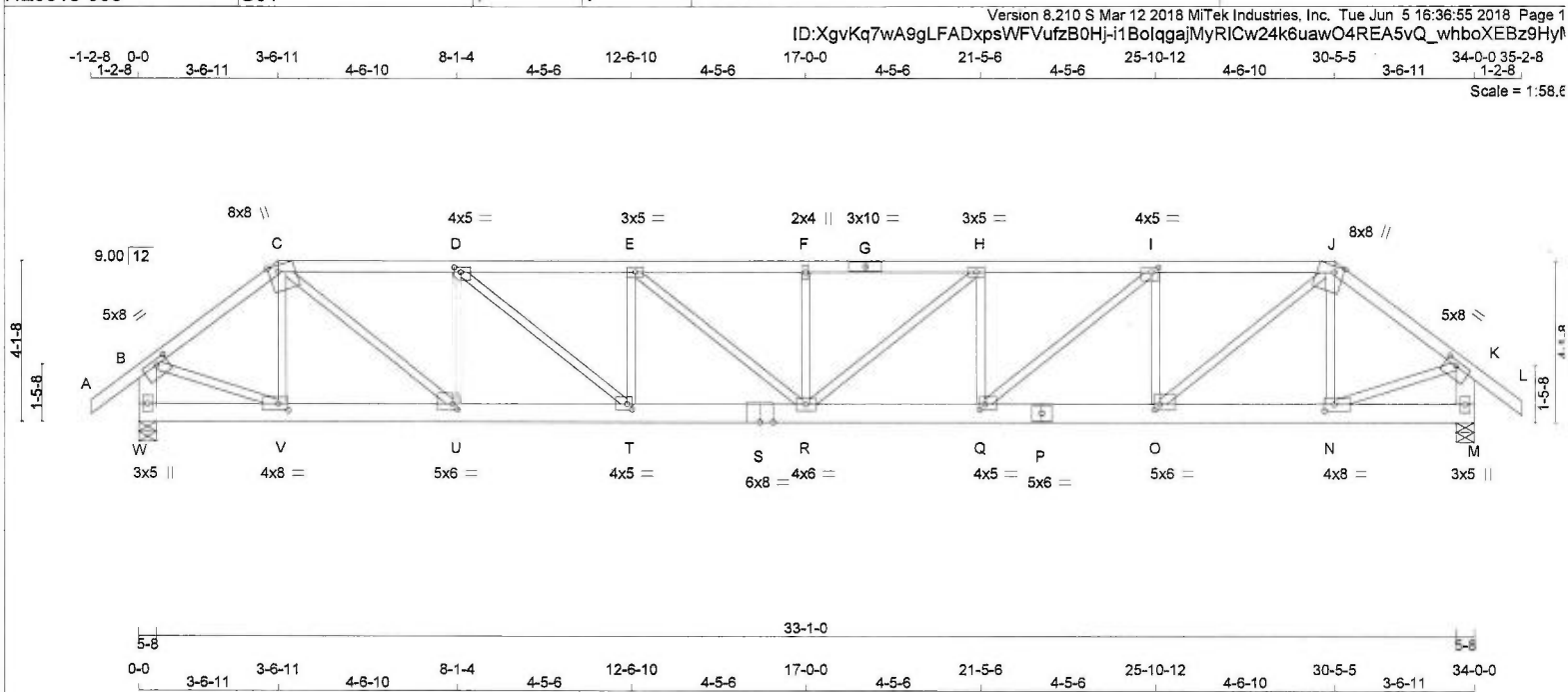
Model: CELESTIAL 2 EL 2 4&5 BED

Customer: GREEN YORK HOMES

Project: GRANELLI HOMES

Location: BRAMPTON

Date: 5/31/2018 Drawn by: BB



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	2100F 1.8E	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.50	3.50
C	TTWW+m	MT20	8.0	8.0	Edge	3.00
D	TMVW-t	MT20	4.0	5.0	1.50	2.00
E	TMVW-t	MT20	3.0	5.0		
F	TMVW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	10.0		
H	TMVW-t	MT20	3.0	5.0		
I	TMVW-t	MT20	4.0	5.0	1.50	2.00
J	TTWW+m	MT20	8.0	8.0	Edge	3.00
K	TMVW-t	MT20	5.0	8.0	1.50	3.50
M	BMV1+p	MT20	3.0	5.0		
N	BMVW-t	MT20	4.0	8.0	2.00	3.00
O	BMVW-t	MT20	5.0	6.0	1.75	1.50
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	4.0	5.0	1.75	1.50
R	BMVW-w	MT20	4.0	6.0		
S	BS-t	MT20	6.0	8.0		
T	BMVW-t	MT20	4.0	5.0	1.75	1.50
U	BMVW-t	MT20	5.0	6.0	1.75	1.50
V	BMVW-t	MT20	4.0	8.0	2.00	3.00
W	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
JT	3104	0	3104	0
W	3104	0	3104	0
M	3104	0	3104	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	2177	1531 / 0	0 / 0	0 / 0	0 / 0	646 / 0	0 / 0
M	2177	1531 / 0	0 / 0	0 / 0	0 / 0	646 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED MAX (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 30	-77.4	-77.4	0.10 (1)	10.00	V-C	-602 / 0
B-C	-3398 / 0	-77.4	-77.4	0.38 (1)	3.46	C-U	0 / 3178
C-D	-5152 / 0	-145.9	-145.9	0.48 (1)	3.47	U-D	-1899 / 0
D-E	-6542 / 0	-145.9	-145.9	0.58 (1)	3.00	D-T	0 / 1818
E-F	-7008 / 0	-145.9	-145.9	0.54 (1)	2.97	T-E	-1018 / 0
F-G	-7008 / 0	-145.9	-145.9	0.54 (1)	2.97	E-R	0 / 609
G-H	-7008 / 0	-145.9	-145.9	0.54 (1)	2.97	R-F	-656 / 0
H-I	-6542 / 0	-145.9	-145.9	0.58 (1)	3.00	R-H	0 / 609
I-J	-5152 / 0	-145.9	-145.9	0.48 (1)	3.47	Q-H	-1018 / 0
J-K	-3398 / 0	-77.4	-77.4	0.38 (1)	3.46	Q-I	0 / 1818
K-L	0 / 30	-77.4	-77.4	0.10 (1)	10.00	O-I	-1899 / 0
W-B	-3061 / 0	0.0	0.0	0.22 (1)	5.98	O-J	0 / 3178
M-K	-3061 / 0	0.0	0.0	0.22 (1)	5.98	N-J	-602 / 0
W-V	0 / 0	-33.0	-33.0	0.07 (4)	10.00	B-V	0 / 2821
V-U	0 / 2695	-33.0	-33.0	0.40 (1)	10.00	N-K	0 / 2821
U-T	0 / 5153	-33.0	-33.0	0.75 (1)	10.00		
T-S	0 / 6542	-33.0	-33.0	0.95 (1)	10.00		
S-R	0 / 6542	-33.0	-33.0	0.95 (1)	10.00		
R-Q	0 / 6542	-33.0	-33.0	0.95 (1)	10.00		
Q-P	0 / 5153	-33.0	-33.0	0.75 (1)	10.00		
P-O	0 / 5153	-33.0	-33.0	0.75 (1)	10.00		
O-N	0 / 2695	-33.0	-33.0	0.40 (1)	10.00		
N-M	0 / 0	-33.0	-33.0	0.07 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-6-11	-206	-206	—	FRONT	VERT	TOTAL
J	30-5-5	-206	-206	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 3-6-11
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.37")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/638 (0.64")

CSI: TC=0.58/1.00 (D-E:1), BC=0.95/1.00 (R-T:1), WB=0.79/1.00 (J-O:1), SS=0.34/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
NE0618-033	G01	1	1	TR-GREEN YORK HOMES-CELESTIAL EL 2		PAGE 4 OF 46

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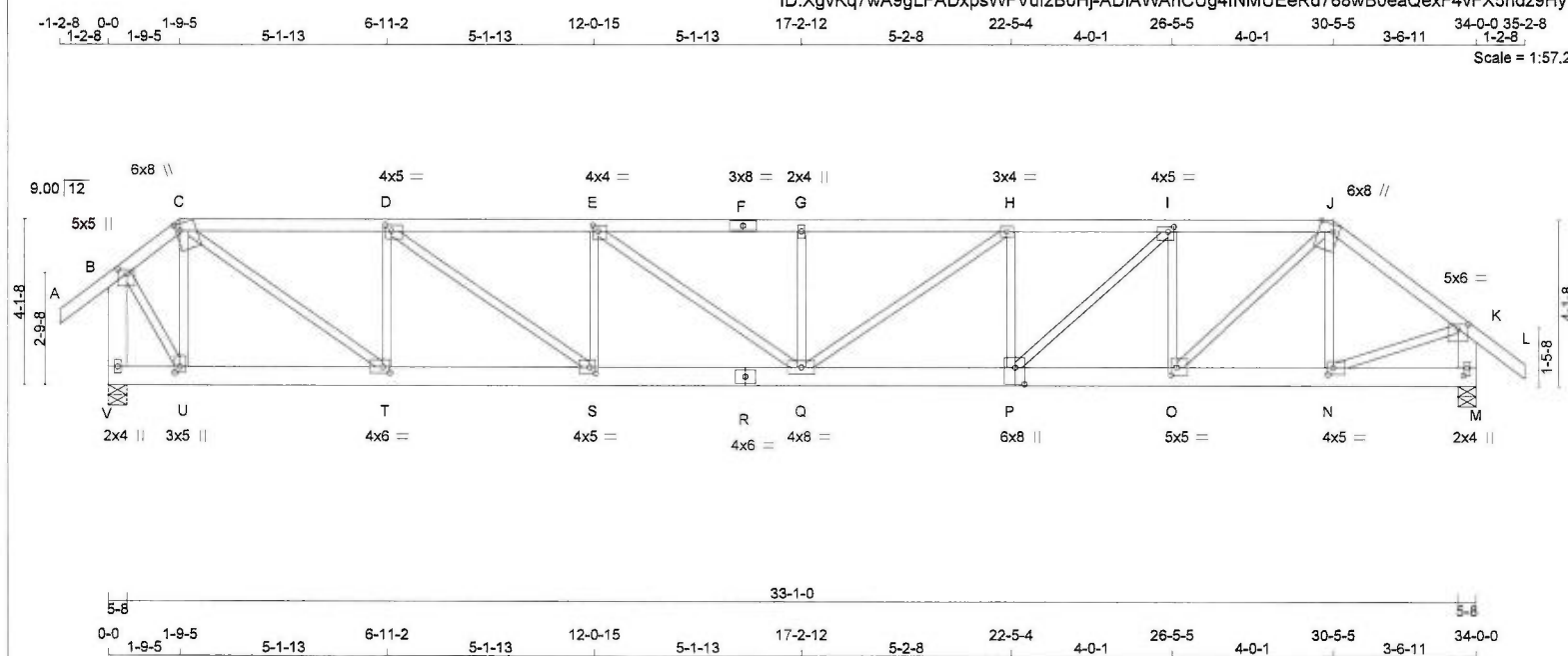
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HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 206.4 lbs FACTORED DOWN AT 30-5-5,
AND 206.4 lbs FACTORED DOWN AT 3-6-11 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.88 (S) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

CHORD	ROWS	SPACING (IN)	LOAD (PLF)
A - C	1	12	TOP
C - F	1	12	TOP
F - J	1	12	SIDE(58.1)
J - L	1	12	TOP
V - B	2	12	TOP
M - K	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
V - R	2	12	TOP
R - P	2	12	SIDE(14.7)
P - M	2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 3049	DOWN 3049	0	5-8
V	0	0	0	5-8
M	3534	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX/MIN. SNOW	COMPONENT LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	2138	1509 / 0	0 / 0	0 / 0	0 / 0	629 / 0	0 / 0
V	2478	1746 / 0	0 / 0	0 / 0	0 / 0	732 / 0	0 / 0
M							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.55 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC1)	MAX. FACTORED UNBRACED LENGTH (FR-TO)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC1)
FR-TO						
A - B	0 / 30	-77.4	-77.4 0.05 (1)	10.00	U - C	-1630 / 0 0.21 (1)
B - C	-1827 / 0	-77.4	-77.4 0.06 (1)	6.25	C - T	0 / 4282 0.53 (1)
C - D	-4858 / 0	-77.4	-77.4 0.29 (1)	4.16	T - D	-2476 / 0 0.32 (1)
D - E	-7651 / 0	-77.4	-77.4 0.48 (1)	3.27	D - S	0 / 3455 0.43 (1)
E - F	-6800 / 0	-77.4	-77.4 0.75 (1)	2.66	S - E	-1904 / 0 0.24 (1)
F - G	-9800 / 0	-77.4	-77.4 0.75 (1)	2.66	E - Q	0 / 2657 0.33 (1)
G - H	-9800 / 0	-193.6	-193.6 0.85 (1)	2.55	Q - G	-738 / 0 0.09 (1)
H - I	-8953 / 0	-77.4	-77.4 0.59 (1)	2.85	Q - H	0 / 1044 0.13 (1)
I - J	-6195 / 0	-77.4	-77.4 0.27 (1)	3.75	P - H	-1331 / 0 0.17 (1)
J - K	-3946 / 0	-77.4	-77.4 0.16 (1)	4.65	P - I	0 / 3778 0.47 (1)
K - L	0 / 30	-77.4	-77.4 0.05 (1)	10.00	O - I	-2815 / 0 0.36 (1)
V - B	-3114 / 0	0.0	0.0 0.15 (1)	7.81	O - J	0 / 4194 0.52 (1)
M - K	-3517 / 0	0.0	0.0 0.13 (1)	7.46	N - J	-791 / 0 0.10 (1)
					B - U	0 / 2316 0.29 (1)
					N - K	0 / 3275 0.41 (1)
V - U	0 / 0	-17.5	-17.5 0.06 (1)	10.00		
U - T	0 / 1391	-17.5	-17.5 0.15 (1)	10.00		
T - S	0 / 4859	-17.5	-17.5 0.36 (1)	10.00		
S - R	0 / 7651	-17.5	-17.5 0.57 (1)	10.00		
R - Q	0 / 7651	-17.5	-17.5 0.57 (1)	10.00		
Q - P	0 / 8953	-43.8	-43.8 0.69 (1)	10.00		
P - O	0 / 6195	-17.5	-17.5 0.44 (1)	10.00		
O - N	0 / 3127	-17.5	-17.5 0.23 (1)	10.00		
N - M	0 / 0	-17.5	-17.5 0.02 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	22-5-4	-1084	-1084		FRONT	VERT	TOTAL
Q	17-2-12	-1331	-1331		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
 ALLOWABLE DEFL.(TL) = L/360 (1.13")
 CALCULATED VERT. DEFL.(TL) = L/807 (0.51")

CSI: TC=0.85/1.00 (G-H:1), BC=0.69/1.00 (P-Q:1),
 WB=0.53/1.00 (C-T:1), SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	5.0	1.25	2.75
C	TTWW+m	MT20	6.0	8.0	2.00	1.25
D	TMWW-t	MT20	4.0	5.0	1.75	1.75
E	TMWW-t	MT20	4.0	4.0	1.75	1.50
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	3.0	4.0		
I	TMWW-t	MT20	4.0	5.0	1.50	1.75
J	TTWW+m	MT20	6.0	8.0	Edge	4.25
K	TMVW-p	MT20	5.0	6.0	1.25	3.00
M	BMV1+p	MT20	2.0	4.0	2.25	1.00
N	BMWW-t	MT20	4.0	5.0	2.00	1.50
O	BMWW-t	MT20	5.0	5.0	2.25	1.75
P	BSWW-t	MT20	6.0	8.0	5.00	2.75
Q	BMWWW-t	MT20	4.0	8.0		
R	BS-t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	5.0	1.75	1.75
T	BMWW-t	MT20	4.0	6.0	1.75	2.00
U	BMWW-t	MT20	3.0	5.0	1.75	1.50
V	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1084.3 lbs FACTORED DOWN AT 22-5-4, AND 1330.9 lbs FACTORED DOWN AT 17-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

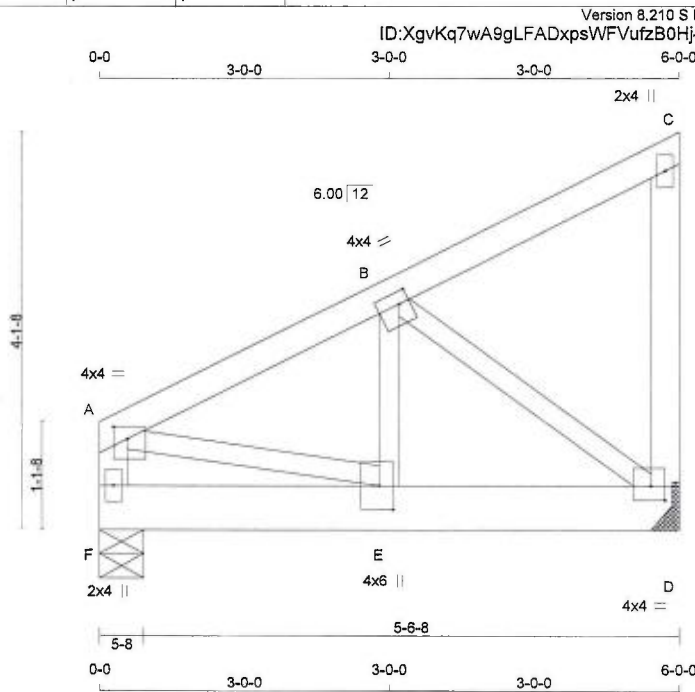
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)

JSI METAL= 0.84 (R) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale: 1/2"=1'

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	1331	0	1331	0	5-8	5-8
D	1331	0	1331	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CSIdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 17-2-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 17-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.11/1.00 (A-B:1), BC=0.35/1.00 (D-E:1), WB=0.31/1.00 (B-D:1), SSI=0.42/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.50	1.75
B	TMVW-t	MT20	4.0	4.0	1.50	1.25
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0	1.75	1.75
E	BMVW+t	MT20	4.0	6.0	3.00	1.75
F	BMV1+p	MT20	2.0	4.0		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	934	653 / 0	0 / 0	0 / 0	0 / 0	281 / 0	0 / 0
D	934	653 / 0	0 / 0	0 / 0	0 / 0	281 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS				
MAX. FACTORED		FACTORED					MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CSI (LC)		
FR-TO		FROM	TO							
F-A	-870 / 0	0.0	0.0	0.10 (1)	7.81	A-E	0 / 1071	0.27 (1)		
A-B	-1154 / 0	-77.4	-77.4	0.11 (1)	5.78	E-B	0 / 1021	0.25 (1)		
B-C	-9 / 0	-77.4	-77.4	0.09 (1)	10.00	B-D	-1297 / 0	0.31 (1)		
D-C	-95 / 0	0.0	0.0	0.02 (1)	7.81					
F-E	0 / 0	-366.3	-366.3	0.21 (1)	10.00					
E-D	0 / 1041	-366.3	-366.3	0.35 (1)	10.00					

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TR-GREEN YORK HOMES-CELESTIAL 2	DRWG NO.
NE0618-033	G03	1	1	TRUSS DESC.	EL 2	PAGE 8 OF 46

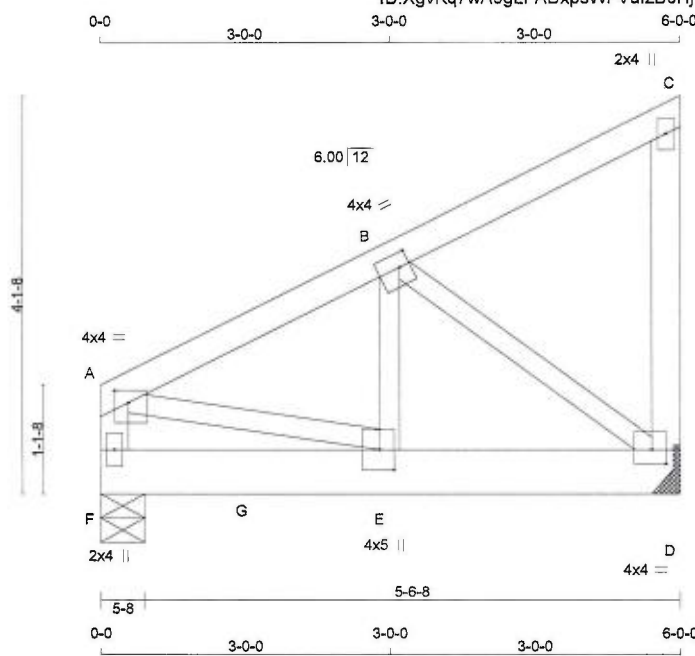
Version 8.210 S Mar 12 2018 MITek Industries, Inc. Tue Jun 5 16:36:57 2018 Page 2

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JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
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THE DESIGN OF THIS COMPONENT.





Scale: 1/2"=1'

TOTAL WEIGHT = 29 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	1205	0	1205	0	5-8	5-8
D	1085	0	1085	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder

START DISTANCE = 1'-6-12

START SPAN CARRIED = 11'-6-0

END DISTANCE = 6'-0-0

END SPAN CARRIED = 11'-6-0

END WALL WIDTH = 5'-8

APPLIED TO FRONT SIDE OF BOTTOM CHORD.

- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.11/1.00 (A-B:1), BC=0.41/1.00 (E-F:1), WB=0.30/1.00 (B-D:1), SSI=0.50/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.50	1.75
B	TMWW-t	MT20	4.0	4.0	1.75	1.25
C	TMVW-p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0	1.75	1.75
E	BMWW-t	MT20	4.0	5.0	2.50	1.75
F	BMV1-p	MT20	2.0	4.0		

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	846	592 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0
D	761	533 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)		MAX. MEMB. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
			FROM	TO			FROM	TO
F-A	-831 / 0	0.0	0.0	0.09 (1)	7.81	A-E	0 / 1018	0.25 (1)
A-B	-1097 / 0	-77.4	-77.4	0.11 (1)	5.89	E-B	0 / 956	0.24 (1)
B-C	-10 / 0	-77.4	-77.4	0.10 (1)	10.00	B-D	-1233 / 0	0.30 (1)
D-C	-95 / 0	0.0	0.0	0.02 (1)	7.81			
F-G	0 / 0	-17.5	-17.5	0.41 (1)	10.00			
G-E	0 / 0	-231.9	-231.9	0.41 (1)	10.00			
E-D	0 / 999	-231.9	-231.9	0.34 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-6-12	-769	-769	---	FRONT	VERT	TOTAL

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2



NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

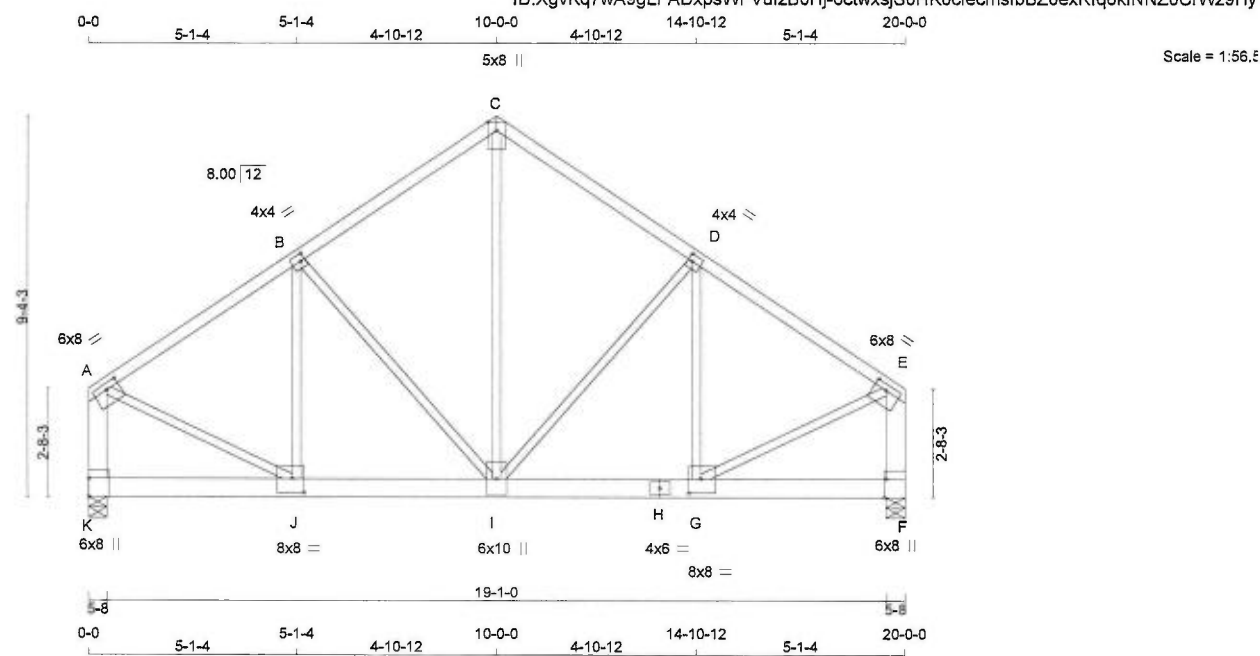
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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TOTAL WEIGHT = 2 X 107 = 214 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x6	DRY	No.2
F - E	2x6	DRY	No.2
K - H	2x6	DRY	2100F 1.8E
H - F	2x6	DRY	2100F 1.8E

LUMBER			
ALL WEBS EXCEPT	SIZE	DRY	No.2
2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
K - A	12	TOP
F - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - H	8	SIDE(374.0)
H - F	8	SIDE(374.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	6.0	8.0 1.75 3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	
JT	VERT	HORZ	UPLIFT	IN-SX	IN-SX
K	8429	0	8429	0	5-8
F	8429	0	8429	0	5-8

UNFACTORED REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	5915	4138 / 0	0 / 0	0 / 0	0 / 0	1777 / 0
F	5915	4138 / 0	0 / 0	0 / 0	0 / 0	1777 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH			
FR-TO		FROM TO	CSI (LC)	FR-TO		CSI (LC)			
A - B	-7161 / 0	-77.4 -77.4	0.44 (1)	3.43	I - C	0 / 6171	0.76 (1)		
B - C	-5861 / 0	-77.4 -77.4	0.33 (1)	3.83	I - D	-1705 / 0	0.94 (1)		
C - D	-5861 / 0	-77.4 -77.4	0.33 (1)	3.83	G - D	0 / 1579	0.20 (1)		
D - E	-7161 / 0	-77.4 -77.4	0.44 (1)	3.43	B - I	-1705 / 0	0.94 (1)		
K - A	-6812 / 0	0.0 0.0	0.32 (1)	5.71	J - B	0 / 1579	0.20 (1)		
F - E	-6812 / 0	0.0 0.0	0.32 (1)	5.71	A - J	0 / 6541	0.81 (1)		
					G - E	0 / 6541	0.81 (1)		
K - J	0 / 0	-765.5 -765.5	0.34 (1)	10.00					
J - I	0 / 5974	-765.5 -765.5	0.50 (1)	10.00					
I - H	0 / 5974	-765.5 -765.5	0.50 (1)	10.00					
H - G	0 / 5974	-765.5 -765.5	0.50 (1)	10.00					
G - F	0 / 0	-765.5 -765.5	0.34 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 34-0-0
END DISTANCE = 20-0-0
END SPAN CARRIED = 34-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.44/1.00 (D-E:1), BC=0.50/1.00 (G-I:1), WB=0.94/1.00 (D-I:1), SSI=0.76/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



PLATES (table is in inches)

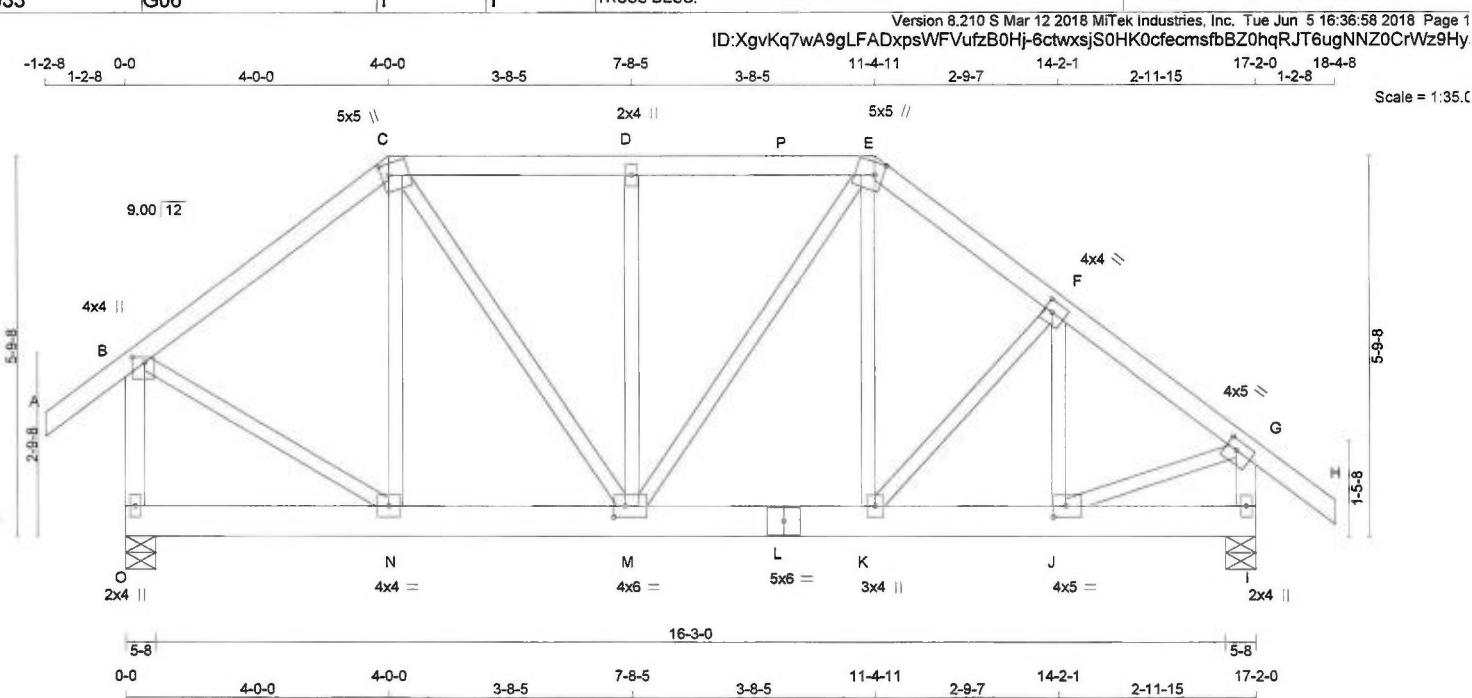
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW-t	MT20	6.0	8.0	1.75	3.75
F	BMV1-t	MT20	6.0	8.0	Edge	0.50
G	BMWW-t	MT20	8.0	8.0	4.25	3.50
H	BS-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	6.0	10.0		
J	BMWW-t	MT20	8.0	8.0	4.25	3.50
K	BMV1-t	MT20	6.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.74 (H) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
O	1230	0	1230	0	0	5-8
I	1420	0	1420	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	861	619 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
I	995	706 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.0	PSF	
TOTAL LOAD = 33.3	PSF	

SPACING = 24.0 IN./C/C

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWW+m	MT20	5.0	5.0	2.00	1.25
D	TMVW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.25	1.50
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-t	MT20	4.0	5.0	1.75	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	5.0	2.00	2.25
K	BMVW-t	MT20	3.0	4.0		
L	BS-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	6.0	2.00	2.00
N	BMVW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4 0.10 (1)	10.00	N-C	-362 / 0	0.18 (1)	
B-C	-885 / 0	-77.4 -77.4 0.25 (1)	6.17	C-M	0 / 757	0.19 (1)	
C-D	-1127 / 0	-77.4 -77.4 0.21 (1)	5.70	M-D	-348 / 0	0.17 (1)	
D-P	-1127 / 0	-77.4 -77.4 0.21 (1)	5.70	M-E	-18 / 0	0.01 (1)	
P-E	-1127 / 0	-107.2 -107.2 0.21 (1)	5.70	K-E	0 / 451	0.11 (1)	
E-F	-1430 / 0	-77.4 -77.4 0.13 (1)	5.30	K-F	0 / 100	0.02 (1)	
F-G	-1315 / 0	-77.4 -77.4 0.13 (1)	5.49	J-F	-401 / 0	0.09 (1)	
G-H	0 / 30	-77.4 -77.4 0.10 (1)	10.00	B-N	0 / 817	0.20 (1)	
O-H	-1198 / 0	0.0 0.0 0.18 (1)	7.27	J-G	0 / 1126	0.28 (1)	
I-G	-1363 / 0	0.0 0.0 0.15 (1)	6.91				
O-N	0 / 0	-17.5 -17.5 0.03 (4)	10.00				
N-M	0 / 700	-17.5 -17.5 0.16 (1)	10.00				
M-L	0 / 1138	-17.5 -17.5 0.40 (1)	10.00				
L-K	0 / 1138	-24.2 -24.2 0.40 (1)	10.00				
K-J	0 / 1063	-24.2 -24.2 0.26 (1)	10.00				
J-I	0 / 0	-24.2 -24.2 0.05 (1)	10.00				

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

LEFT SETBACK = 4-0-0

RIGHT SETBACK = 5-9-5

END SETBACK = 4-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'L LOADS BASED ON 55 % OF GSL

LOADS APPLIED TO FIRST 7-1-4 OF SPAN

MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	11-4-11	-186	-186	—	FRONT	VERT	TOTAL
L	10-0-12	-547	-547	—	FRONT	VERT	TOTAL

ALLOWABLE DEFL.(LL)= L/360 (0.57")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")

ALLOWABLE DEFL.(TL)= L/360 (0.57")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.25/1.00 (B-C:1), BC=0.40/1.00 (K-M:1), WB=0.28/1.00 (G-J:1), SSI=0.27/1.00 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354 1667	822 2284 1656

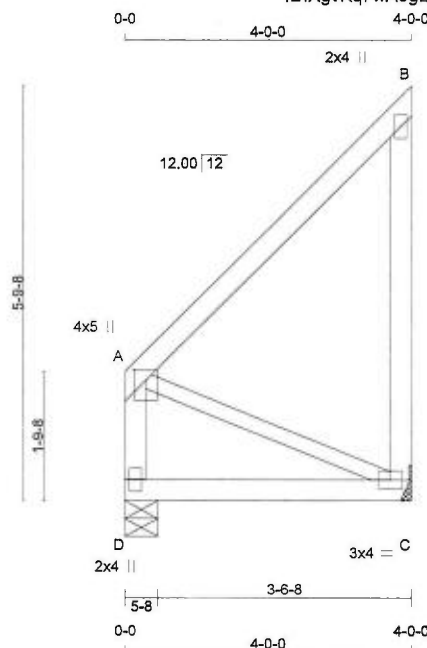
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.45 (L) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale: $3/8"=1'$

TOTAL WEIGHT = 22 lb

LUMBER				
N. L. G. A.	RULES			
CHORDS	SIZE		LUMBER	DESCR
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
<u>BEARINGS</u>							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	547	0	547	0	0	5-8	5-8
C	547	0	547	0	0	MECHANICAL	

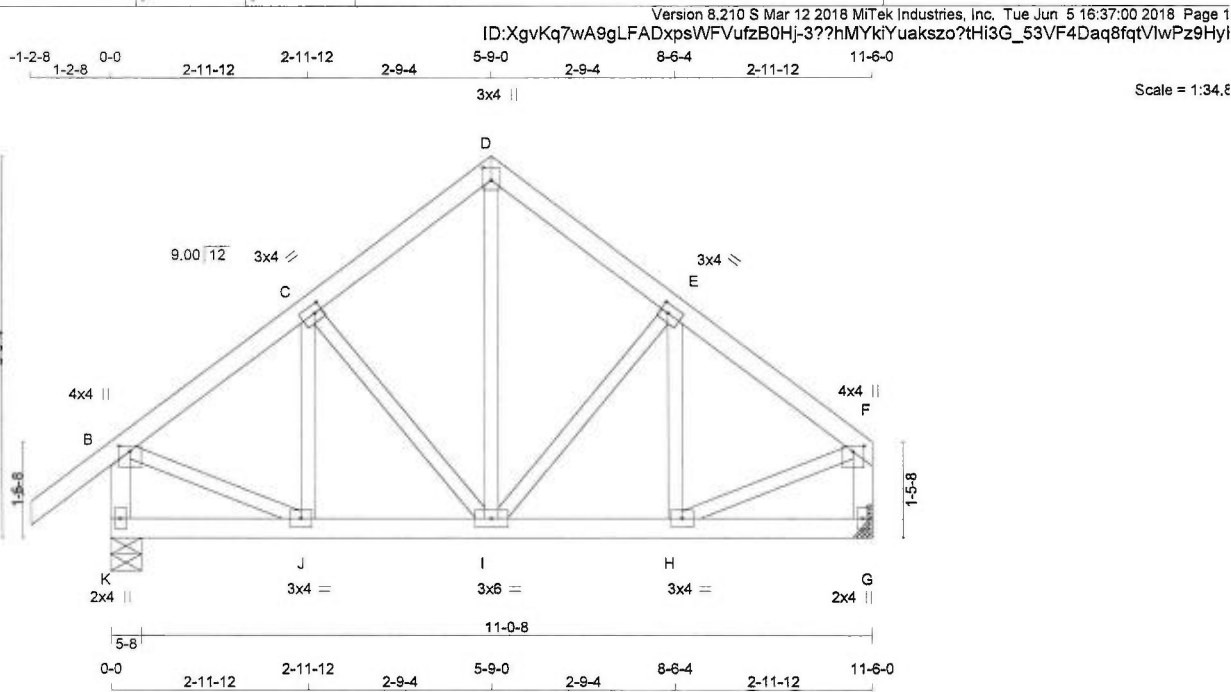


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TR- GREEN YORK HOMES-CELESTIAL	DRWG NO.
NE0618-033	G08	1	1	TRUSS DESC.	EL 2	

JSI GRIP= 0.15 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
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TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - B	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
G - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
K	870	0	870	0	5-8	5-8
G	770	0	770	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	608	440 / 0	0 / 0	0 / 0	0 / 0	168 / 0
G	540	379 / 0	0 / 0	0 / 0	0 / 0	160 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 30	-77.4	-77.4	0.10 (1)	10.00	I-D	0 / 225		
B-C	-701 / 0	-77.4	-77.4	0.11 (1)	6.25	I-E	-104 / 0		
C-D	-648 / 0	-77.4	-77.4	0.11 (1)	6.25	H-E	-142 / 27		
D-E	-648 / 0	-77.4	-77.4	0.11 (1)	6.25	C-I	-104 / 0		
E-F	-701 / 0	-77.4	-77.4	0.11 (1)	6.25	J-C	-142 / 27		
K-B	-838 / 0	0.0	0.0	0.10 (1)	7.81	B-J	0 / 612		
G-F	-738 / 0	0.0	0.0	0.08 (1)	7.81	H-F	0 / 612		
K-J	0 / 0	-24.2	-24.2	0.05 (4)	10.00				
J-I	0 / 572	-24.2	-24.2	0.12 (1)	10.00				
I-H	0 / 572	-24.2	-24.2	0.12 (1)	10.00				
H-G	0 / 0	-24.2	-24.2	0.05 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-9-0	-371	-371		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN./C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-9-0
END SETBACK = 4-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.11/1.00 (B-C:1), BC=0.12/1.00 (H-I:1), WB=0.15/1.00 (F-H:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

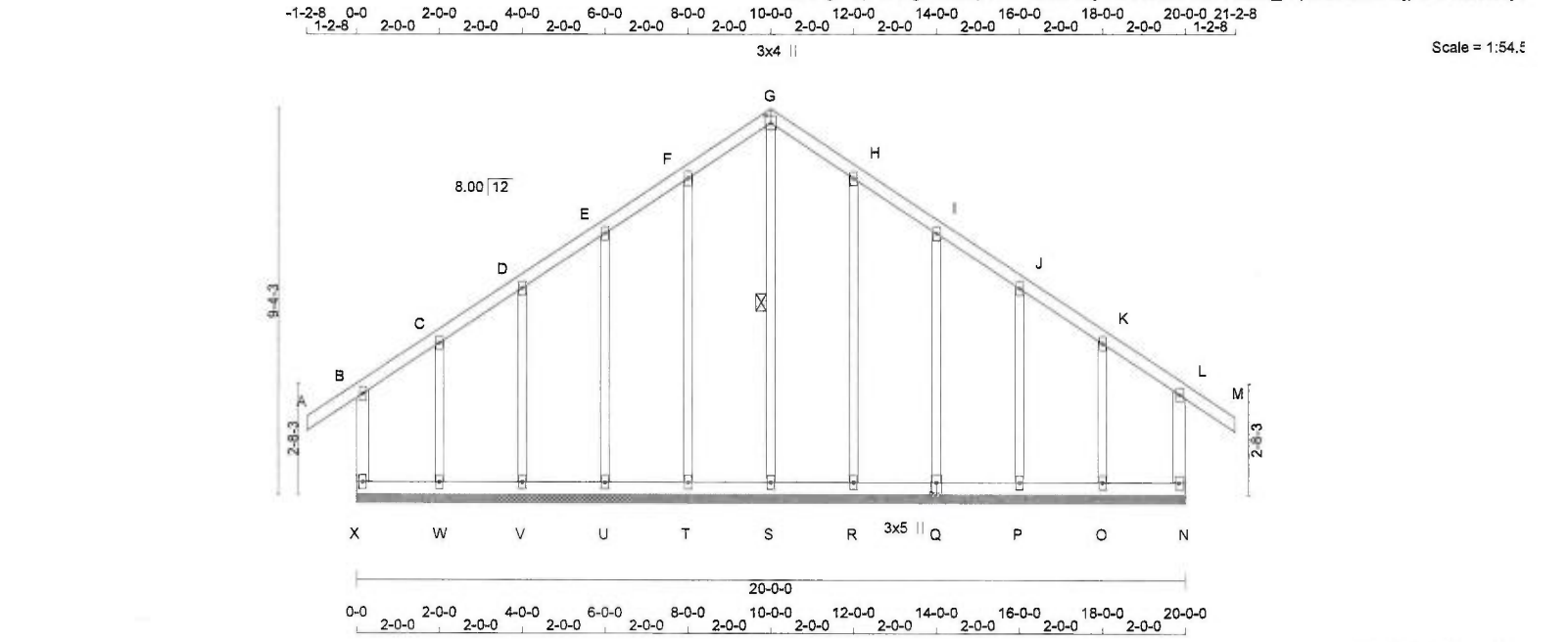


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS NO.
NE0618-033	G09	1	1	TR- GREEN YORK HOMES-CELESTIAL EL 2	
				TRUSS DESC.	

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

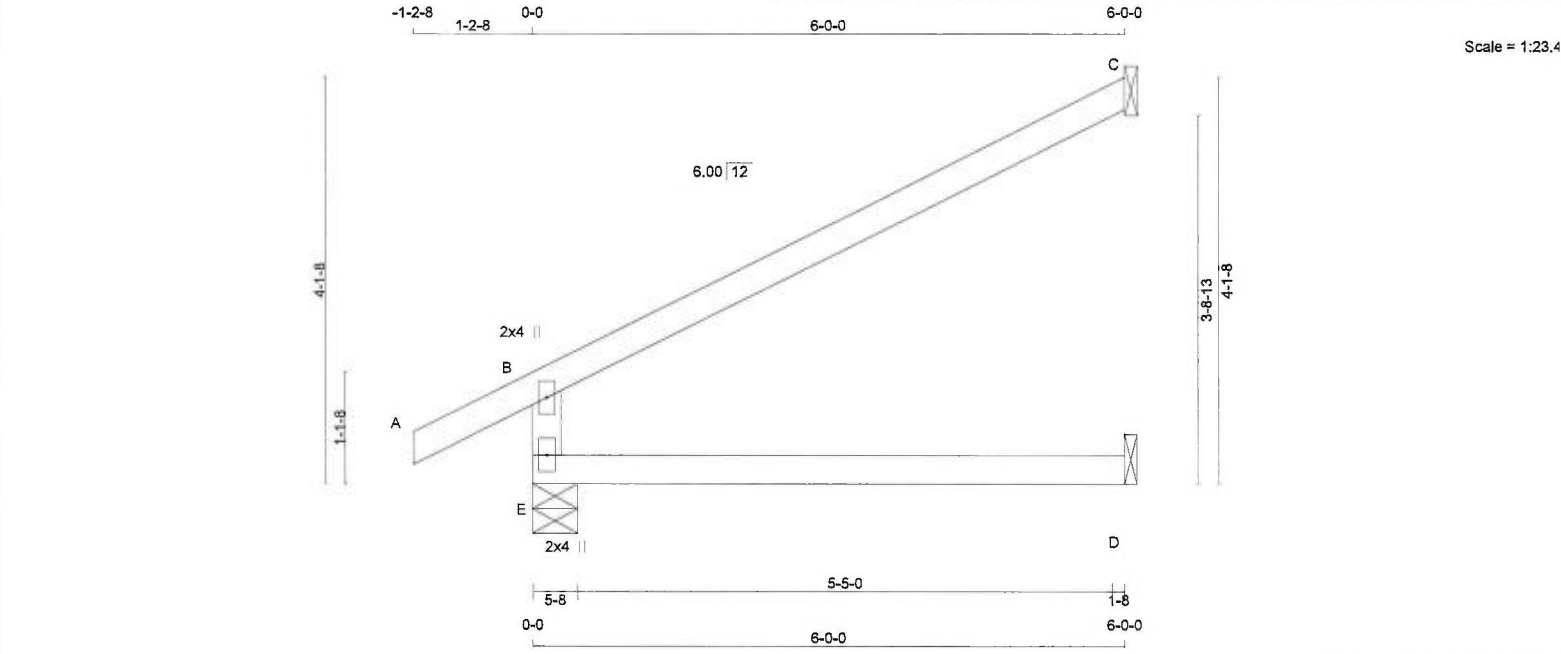
READ ALL NOTES ON THIS PAGE AND ON
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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				TOTAL WEIGHT = 102 lb			
N. L. G. A. RULES				BEARINGS				[M]			
CHORDS	SIZE	LUMBER	DESCR.	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				DESIGN CRITERIA			
X - B	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				SPECIFIED LOADS:			
A - G	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				TOP CH. LL = 23.3 PSF			
G - M	2x4	DRY	No.2	BRACING				DL = 3.0 PSF			
N - L	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.				BOT CH. LL = 0.0 PSF			
X - Q	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				DL = 7.0 PSF			
Q - N	2x4	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				TOTAL LOAD = 33.3 PSF			
ALL WEBS	2x3	DRY	No.2	1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-S. DBS = 20-0-0, CBF = 21 LBS.				SPACING = 24.0 IN. C/C			
ALL GABLE WEBS	2x3	DRY	No.2	DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
DRY: SEASONED LUMBER.				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				THIS DESIGN COMPLIES WITH:			

GABLE STUDS SPACED AT 2-0-0 OC.				LOADING			
				TOTAL LOAD CASES: (4)			



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

E - B

2x4

DRY

No.2

SPF

A - C

2x4

DRY

No.2

SPF

E - D

2x4

DRY

No.2

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

JT

GROSS REACTION

GROSS REACTION

BRG

BRG

E

450

0

450

0

0

5-8

5-8

C

174

0

174

0

0

1-8

1-8

D

43

0

48

0

0

1-8

1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

E

314

234 / 0

0 / 0

0 / 0

0 / 0

80 / 0

0 / 0

C

118

105 / 0

0 / 0

0 / 0

0 / 0

14 / 0

0 / 0

D

35

0 / 0

0 / 0

0 / 0

0 / 0

35 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

WEBS

MAX. FACTORED

MEMB.

FORCE

VERT. LOAD

LC1 MAX

MAX.

MEMB.

FORCE

MAX

FR-TO

FROM

TO

LENGTH

FR-TO

E-B

-389 / 0

0.0

0.0

0.11 (4)

7.81

A-B

0 / 22

-77.4

-77.4

0.09 (1)

10.00

B-C

-26 / 0

-77.4

-77.4

0.36 (1)

6.25

E-D

0 / 0

-17.5

-17.5

0.13 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

23.3

PSF

BOT CH.

LL

=

0.0

PSF

TOTAL LOAD

=

33.3

PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

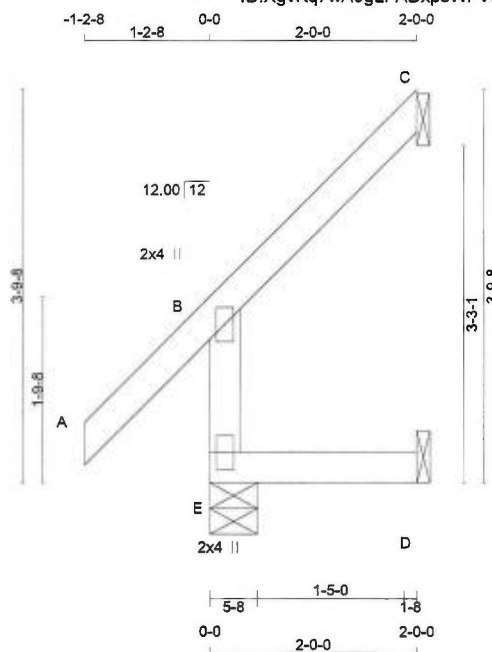
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)

JSI METAL= 0.09 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:22.5

TOTAL WEIGHT = 3 X 9 = 28 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF
 E - B 2x4 DRY No.2 SPF
 A - C 2x4 DRY No.2 SPF
 E - D 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	216	0	216	0	5-8	5-8
C	59	0	59	0	1-8	1-8
D	16	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	149	119 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
C	40	35 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED REACTION		MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED REACTION (LC)
			FROM	TO				
E-B	-198 / 0	0.0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 36	-77.4	-77.4	0.10 (1)		10.00		
B-C	-13 / 0	-77.4	-77.4	0.05 (1)		6.25		
E-D	0 / 0	-17.5	-17.5	0.02 (4)		10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.10/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
 WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

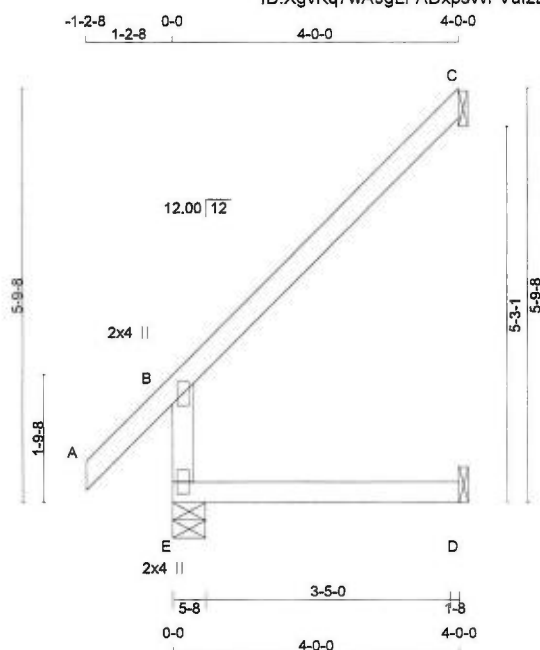
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
 JSI METAL= 0.06 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale: 3/8"=1'

TOTAL WEIGHT = 3 X 15 = 45 lb
 [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - B 2x4 DRY No.2 SPF
 A - C 2x4 DRY No.2 SPF
 E - D 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT		
E	334	0	334	0	5-8	5-8
C	116	0	116	0	1-8	1-8
D	31	0	34	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	232	177 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
C	79	70 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-295 / 0	0.0	0.0	0.04 (4)	7.81	
A-B	0 / 36	-77.4	-77.4	0.10 (1)	10.00	
B-C	-27 / 0	-77.4	-77.4	0.21 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.06/1.00 (D-E:4),
 WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

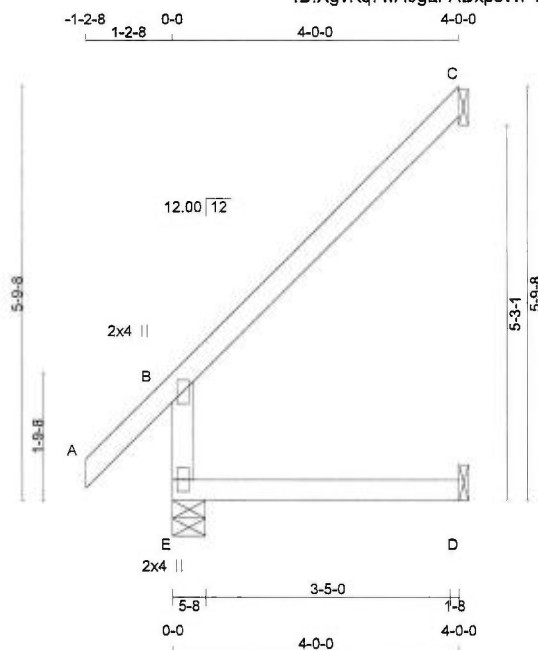
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
 JSI METAL= 0.08 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale: 3/8"=1'

TOTAL WEIGHT = 5 X 15 = 74 lb [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF
 E - B 2x4 DRY No.2 SPF
 A - C 2x4 DRY No.2 SPF
 E - D 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	334	0	334	0	5-8	5-8
C	116	0	116	0	1-8	1-8
D	31	0	34	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	232	177 / 0	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
C	79	70 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	MEMB. MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-295 / 0	0.0	0.0	0.04 (4)	7.81	
A-B	0 / 36	-77.4	-77.4	0.10 (1)	10.00	
B-C	-27 / 0	-77.4	-77.4	0.21 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.06/1.00 (D-E:4),
 VB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

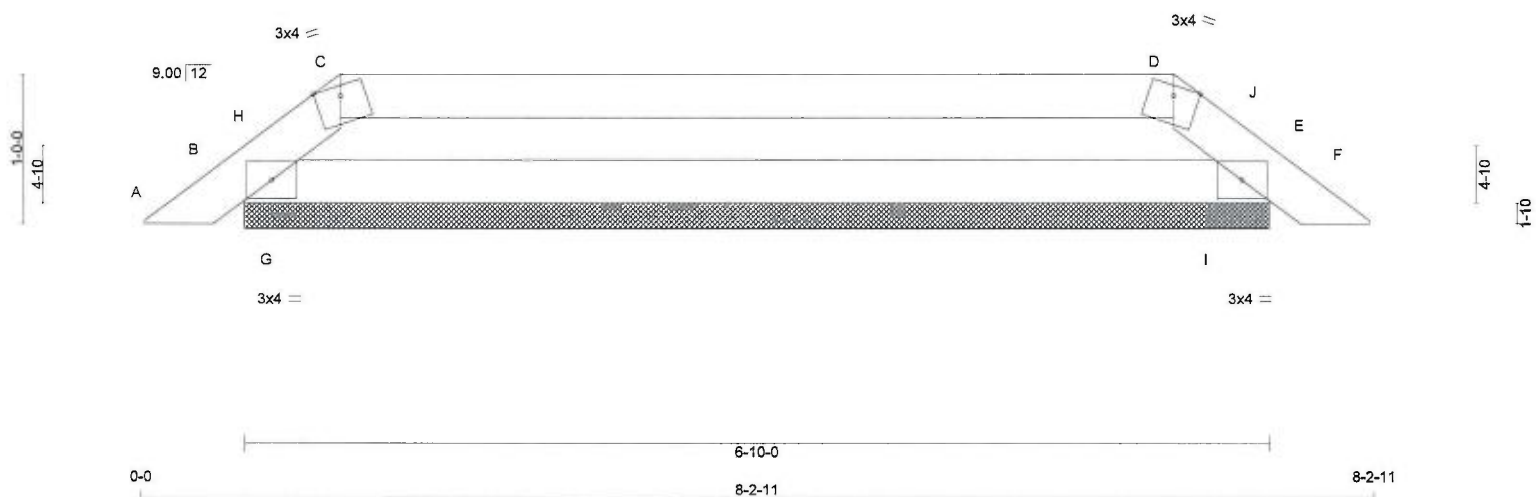
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
 JSI METAL= 0.08 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-m	MT20	3.0	4.0	Edge	
D	TT-m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
B	370	370	6-10-0	6-10-0
E	370	370	6-10-0	6-10-0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	258	187 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
E	258	187 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 12	-77.4	-77.4 0.02 (1)	10.00	G-H	-80 / 122	0.00 (1)
B-H	-247 / 0	-77.4	-77.4 0.06 (4)	6.25	I-J	-80 / 122	0.00 (1)
H-C	-656 / 0	-77.4	-77.4 0.23 (1)	6.25			
C-D	-657 / 0	-77.4	-77.4 0.24 (1)	6.25			
D-J	-656 / 0	-77.4	-77.4 0.23 (1)	6.25			
J-E	-247 / 0	-77.4	-77.4 0.06 (4)	6.25			
E-F	0 / 12	-77.4	-77.4 0.02 (1)	10.00			
B-G	0 / 176	-17.5	-17.5 0.06 (1)	10.00			
G-I	0 / 657	-17.5	-17.5 0.17 (4)	10.00			
I-E	0 / 176	-17.5	-17.5 0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.17/1.00 (G-I:4), WB=0.00/1.00 (G-H:1), SSI=0.19/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

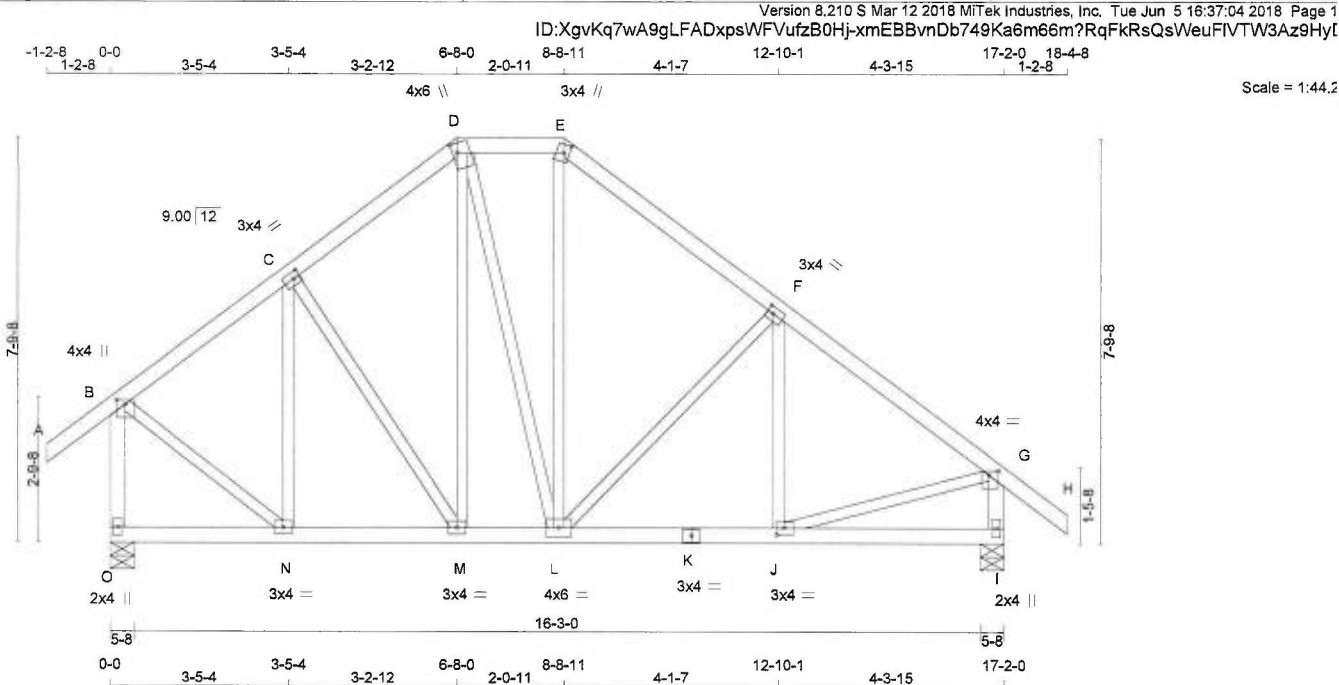
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (E) (INPUT = 0.80)
 JSI METAL= 0.17 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TTW+m	MT20	4.0	6.0	2.25 1.00
E	TTW+m	MT20	3.0	4.0	2.00 1.25
F	TMVW-t	MT20	3.0	4.0	1.50 1.50
G	TMVW-p	MT20	4.0	4.0	1.00 2.25
I	BMV1+p	MT20	2.0	4.0	
J	BMVW-t	MT20	3.0	4.0	1.50 1.75
K	BS-t	MT20	3.0	4.0	
L	BMVW-t	MT20	4.0	6.0	
M	BMVW-t	MT20	3.0	4.0	
N	BMVW-t	MT20	3.0	4.0	
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
O	914	0	0	914	0	0	5-8	5-8	5-8
I	914	0	0	914	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL	DEAD	SOIL
O	640	460 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0	179 / 0	0 / 0	0 / 0	0 / 0
I	640	460 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0	179 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 30	N-C	-267 / 0
B-C	-567 / 0	C-M	-44 / 0
C-D	-575 / 0	M-D	0 / 72
D-E	-468 / 0	D-L	0 / 93
E-F	-608 / 0	L-E	0 / 153
F-G	-785 / 0	L-F	-259 / 0
G-H	0 / 30	J-F	-85 / 47
O-B	-890 / 0	B-N	0 / 576
I-G	-883 / 0	J-G	0 / 668
O-N	0 / 0		
N-M	0 / 468		
M-L	0 / 445		
L-K	0 / 646		
K-J	0 / 646		
J-I	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.18/1.00 (F-G:1) , BC=0.14/1.00 (J-L:1) ,
WB=0.17/1.00 (F-L:1) , SSI=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

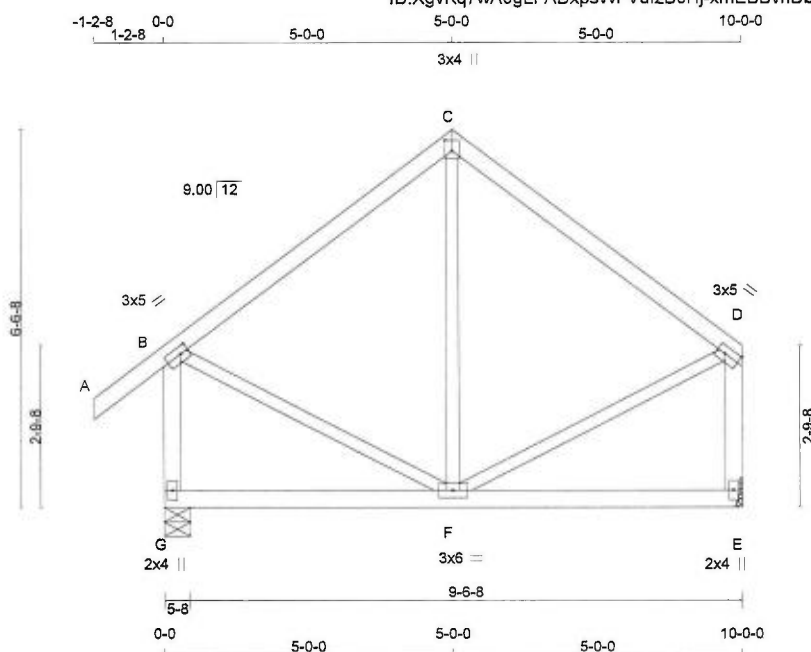
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.24 (G) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 46 lb
 [M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
G	575	575	5-8	5-8
E	474	474	MECHANICAL	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD =		33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.33")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.25/1.00 (B-C:1), BC=0.13/1.00 (F-G:4), WB=0.06/1.00 (C-F:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
 JSI METAL= 0.13 (D) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	Edge
E	BMV1+p	MT20	2.0	4.0		
F	BMVW-t	MT20	3.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	401	293 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
E	333	233 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

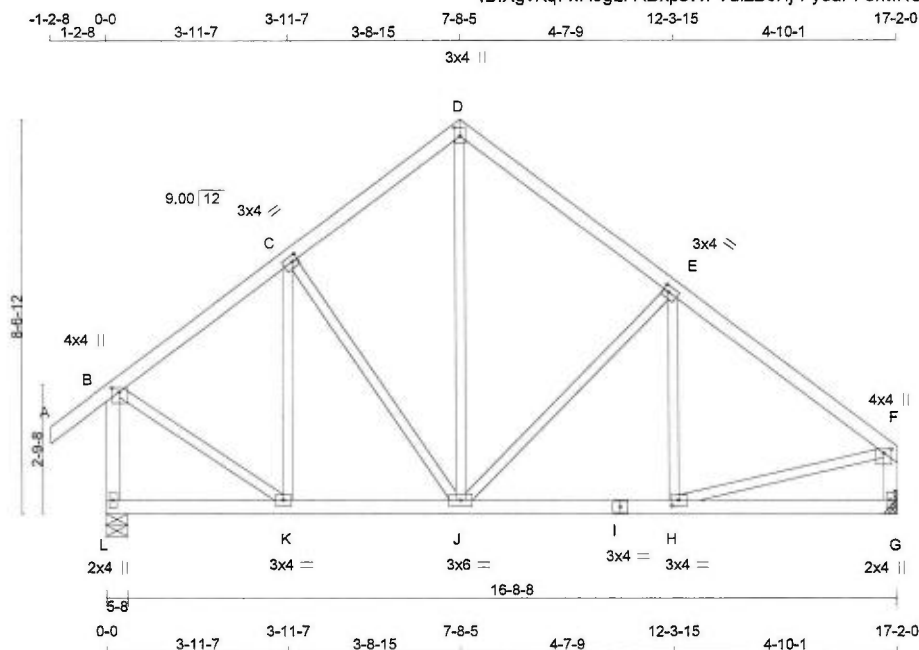
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	F-C	-88 / 57	10.00	0.06 (1)
B-C	-249 / 0	-77.4	-77.4 0.25 (1)	B-F	0 / 222	6.25	0.05 (1)
C-D	-249 / 0	-77.4	-77.4 0.25 (1)	F-D	0 / 222	6.25	0.05 (1)
G-B	-541 / 0	0.0	0.0 0.08 (1)			7.81	
E-D	-441 / 0	0.0	0.0 0.07 (1)			7.81	
G-F	0 / 0	-17.5	-17.5 0.13 (4)			10.00	
F-E	0 / 0	-17.5	-17.5 0.13 (4)			10.00	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMVW-t	MT20	3.0	4.0	1.50	1.75
I	BS-t	MT20	3.0	4.0		
J	BMVW-t	MT20	3.0	6.0		
K	BMVW-t	MT20	3.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
L	VERT	914	0	914	0
G	HORZ	814	0	814	0
	UPLIFT	0	0	5-8	5-8
	IN-SX			MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	COMBINED	640	460 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0
G	COMBINED	571	400 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	K-C	-230 / 0	0.12 (1)
B-C	-586 / 0	-77.4 -77.4	0.15 (1)	6.25	C-J	-111 / 0	0.09 (1)
C-D	-550 / 0	-77.4 -77.4	0.15 (1)	6.25	J-D	0 / 391	0.09 (1)
D-E	-555 / 0	-77.4 -77.4	0.22 (1)	6.25	J-E	-320 / 0	0.26 (1)
E-F	-781 / 0	-77.4 -77.4	0.23 (1)	6.25	H-E	-58 / 63	0.02 (1)
L-B	-885 / 0	0.0 0.0	0.13 (1)	7.81	B-K	0 / 572	0.13 (1)
G-F	-779 / 0	0.0 0.0	0.08 (1)	7.81	H-F	0 / 663	0.15 (1)
L-K	0 / 0	-17.5 -17.5	0.06 (4)	10.00			
K-J	0 / 485	-17.5 -17.5	0.11 (1)	10.00			
J-I	0 / 646	-17.5 -17.5	0.15 (1)	10.00			
I-H	0 / 646	-17.5 -17.5	0.15 (1)	10.00			
H-G	0 / 0	-17.5 -17.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.23/1.00 (E-F:1), BC=0.15/1.00 (H-J:1),
WB=0.26/1.00 (E-J:1), SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

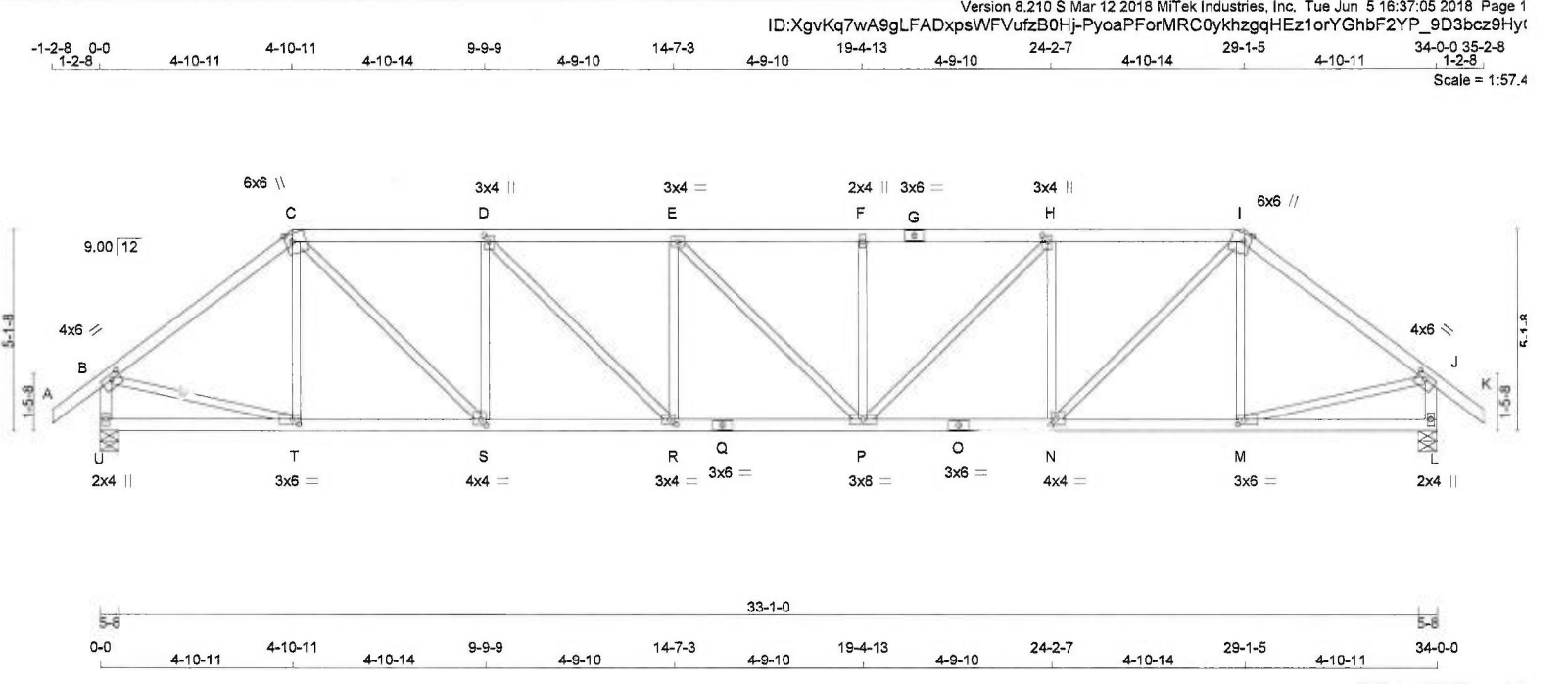
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.24 (H) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 143 lb									
N. L. G. A. RULES										BEARINGS										DESIGN CRITERIA									
CHORDS SIZE										FACTORED										SPECIFIED LOADS:									
LUMBER										MAXIMUM FACTORED										TOP CH. LL = 23.3 PSF									
DESCR.										INPUT										DL = 3.0 PSF									
SPF										BRG										BOT CH. LL = 0.0 PSF									
A - C 2x4 DRY No.2										BRG										DL = 7.0 PSF									
C - G 2x4 DRY No.2										IN-SX										TOTAL LOAD = 33.3 PSF									
G - I 2x4 DRY No.2										IN-SX																			
I - K 2x4 DRY No.2																													
U - B 2x4 DRY No.2																													
L - J 2x4 DRY No.2																													
U - Q 2x4 DRY No.2																													
Q - O 2x4 DRY No.2																													
O - L 2x4 DRY No.2																													
ALL WEBS EXCEPT 2x3 DRY No.2																				SPACING = 24.0 IN. C/C									
										UNFACTORED REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
										1ST LCASE MAX./MIN. COMPONENT REACTIONS																			
										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL																			
										U 1200 852 / 0 0 / 0 0 / 0 0 / 0 348 / 0 0 / 0																			
										L 1200 852 / 0 0 / 0 0 / 0 0 / 0 348 / 0 0 / 0																			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TTWW+m	MT20	6.0	6.0	Edge	2.00
D	TMVW+t	MT20	3.0	4.0	1.75	1.50
E	TMVW-t	MT20	3.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW+t	MT20	3.0	4.0	1.75	1.50
I	TTWW+m	MT20	6.0	6.0	Edge	2.00
J	TMVW-t	MT20	4.0	6.0	1.50	2.75
L	BMV1+p	MT20	2.0	4.0		
M	BMVW-t	MT20	3.0	6.0	1.50	2.00
N	BMVW-t	MT20	4.0	4.0	1.75	1.50
O	BS-t	MT20	3.0	6.0		
P	BMVWWW-t	MT20	3.0	8.0		
Q	BS-t	MT20	3.0	6.0		
R	BMVW-t	MT20	3.0	4.0	1.50	1.75
S	BMVW-t	MT20	4.0	4.0	1.75	1.50
T	BMVW-t	MT20	3.0	6.0	1.50	2.00
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	T-C	-222 / 13
B-C	-1774 / 0	-77.4	-77.4	0.42 (1)	4.58	C-S	0 / 1340
C-D	-2368 / 0	-77.4	-77.4	0.36 (1)	4.13	S-D	-859 / 0
D-E	-2811 / 0	-77.4	-77.4	0.40 (1)	3.81	D-R	0 / 628
E-F	-2810 / 0	-77.4	-77.4	0.32 (1)	3.90	R-E	-361 / 0
F-G	-2810 / 0	-77.4	-77.4	0.40 (1)	3.81	E-P	-2 / 0
G-H	-2810 / 0	-77.4	-77.4	0.40 (1)	3.81	P-F	-360 / 0
H-I	-2368 / 0	-77.4	-77.4	0.36 (1)	4.13	P-H	0 / 626
I-J	-1774 / 0	-77.4	-77.4	0.41 (1)	4.58	N-H	-859 / 0
J-K	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N-I	0 / 1341
U-B	-1678 / 0	0.0	0.0	0.17 (1)	6.41	M-I	-222 / 13
L-J	-1678 / 0	0.0	0.0	0.17 (1)	6.41	B-T	0 / 1452
						M-J	0 / 1452

U-T

T-S

S-R

0 / 0

0 / 1412

0 / 2368

-17.5

-17.5

-17.5

0.10 (4)

0.28 (1)

0.42 (1)

10.00

10.00

10.00

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")

ALLOWABLE DEFL.(TL)= L/360 (1.13")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.49/1.00 (P-R:1) , WB=0.33/1.00 (D-S:1) , SSI=0.18/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

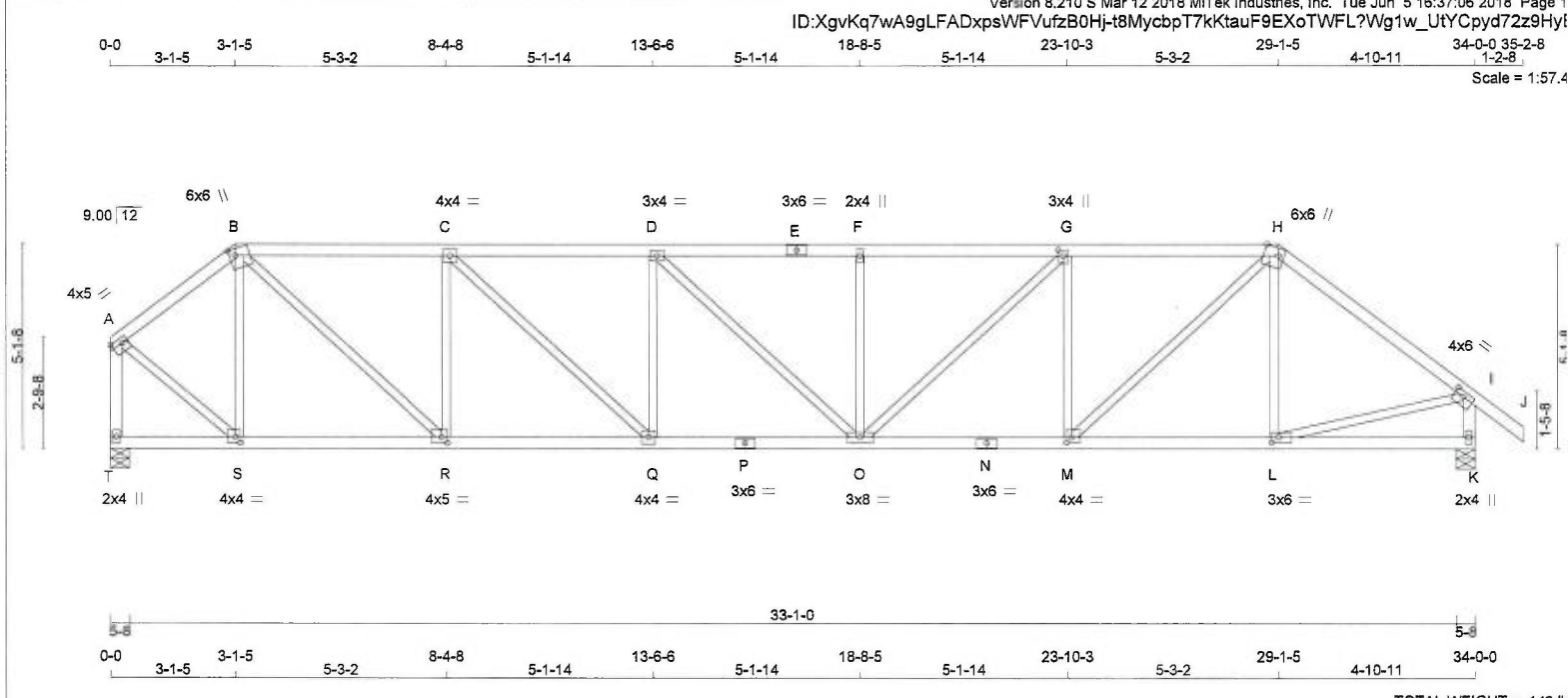
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - B 2x4 DRY No.2 SPF B - E 2x4 DRY No.2 SPF E - H 2x4 DRY No.2 SPF H - J 2x4 DRY No.2 SPF T - A 2x4 DRY No.2 SPF K - I 2x4 DRY No.2 SPF T - P 2x4 DRY No.2 SPF P - N 2x4 DRY No.2 SPF N - K 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX T 1613 0 1613 0 0 5-8 5-8 K 1713 0 1713 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL T 1132 792 / 0 0 / 0 0 / 0 340 / 0 0 / 0 K 1200 852 / 0 0 / 0 0 / 0 348 / 0 0 / 0				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.0 PSF TOTAL LOAD = 33.3 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.13") CALCULATED VERT. DEFL.(LL) = L/999 (0.17") ALLOWABLE DEFL.(TL)= L/360 (1.13") CALCULATED VERT. DEFL.(TL) = L/999 (0.30") CSI: TC=0.47/1.00 (F-G:1), BC=0.49/1.00 (O-Q:1), WB=0.39/1.00 (C-R:1), SSI=0.19/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.88 (K) (INPUT = 0.90) JSI METAL= 0.75 (P) (INPUT = 1.00)			
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DRY: SEASONED LUMBER.

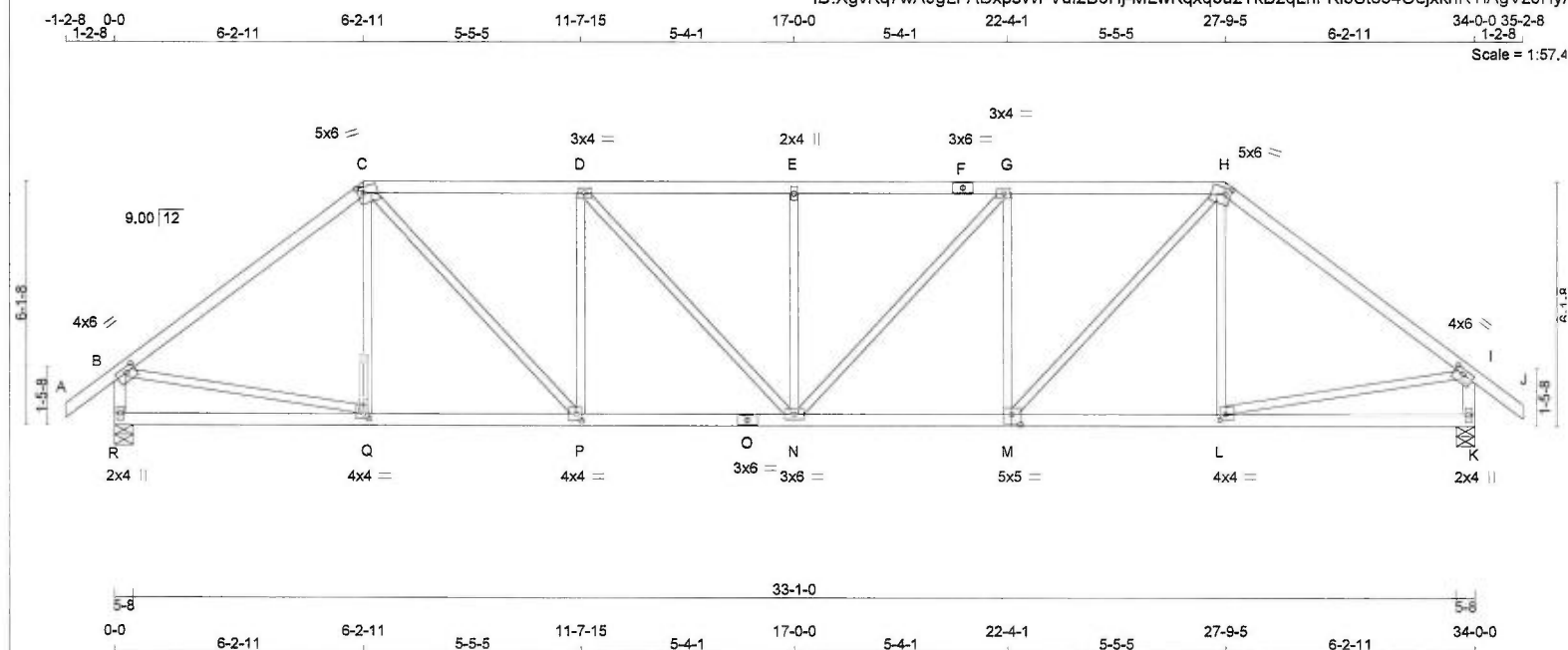
PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMVW-t MT20 4.0 5.0 1.50 Edge
B TTWW+m MT20 6.0 6.0 2.00 1.75
C TMVW-t MT20 4.0 4.0
D TMVW-t MT20 3.0 4.0
E TS-t MT20 3.0 6.0
F TMVW+w MT20 2.0 4.0
G TMVW-t MT20 3.0 4.0 1.75 1.50
H TTWW+m MT20 6.0 6.0 Edge 4.25
I TMVW-t MT20 4.0 6.0 1.50 2.75
K BMV1+p MT20 2.0 4.0
L BMVW-t MT20 3.0 6.0 1.50 2.00
M BMVW-t MT20 4.0 4.0 1.50 1.50
N BS-t MT20 3.0 6.0
O BMVW-t MT20 3.0 8.0
P BS-t MT20 3.0 6.0
Q BMVW-t MT20 4.0 4.0
R BMVW-t MT20 4.0 5.0 1.75 1.75
S BMVW-t MT20 4.0 4.0 1.75 1.50
T BMV1+p MT20 2.0 4.0
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

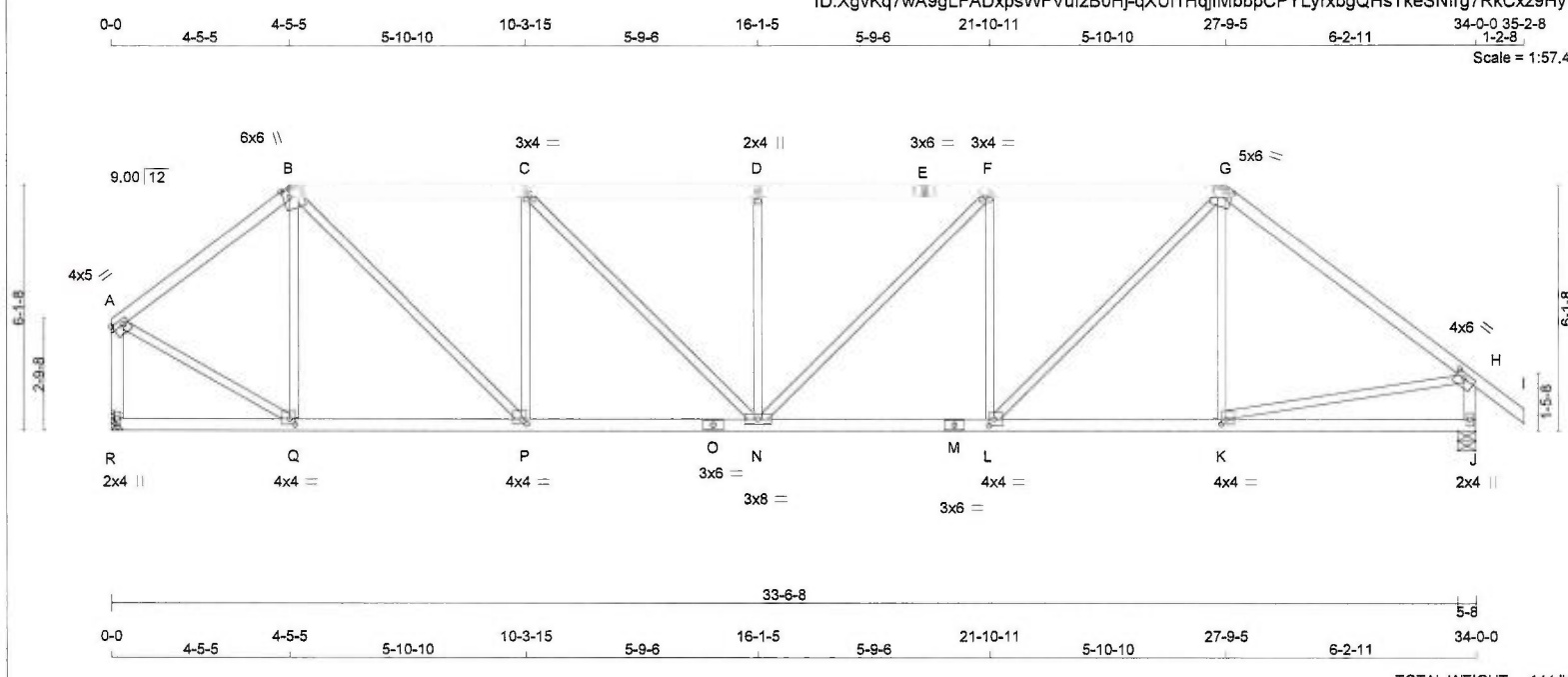
LOADING TOTAL LOAD CASES: (4)				CHORDS MAX. FACTORED FORCE VERT. LOAD LC1 MAX (LBS) (PLF) CSI (LC)				WEBS MAX. FACTORED FORCE VERT. LOAD LC1 MAX (LBS) (PLF) CSI (LC)			
FR-TO	MEMB.	FROM	TO	FR-TO	MEMB.	FROM	TO	FR-TO	MEMB.	FROM	TO
A-B	-1216 / 0	-77.4	-77.4 0.15 (1)	5.65	S-B	-656 / 0	0.25 (1)	A-B	-1216 / 0	-77.4	-77.4 0.15 (1)
B-C	-2155 / 0	-77.4	-77.4 0.40 (1)	4.25	B-R	0 / 1626	0.37 (1)	B-C	-2155 / 0	-77.4	-77.4 0.40 (1)
C-D	-2755 / 0	-77.4	-77.4 0.46 (1)	3.78	R-C	-1009 / 0	0.39 (1)	C-D	-2755 / 0	-77.4	-77.4 0.46 (1)
D-E	-2844 / 0	-77.4	-77.4 0.37 (1)	3.84	C-Q	0 / 822	0.19 (1)	D-E	-2844 / 0	-77.4	-77.4 0.37 (1)
E-F	-2844 / 0	-77.4	-77.4 0.37 (1)	3.84	Q-D	-472 / 0	0.18 (1)	E-F	-2844 / 0	-77.4	-77.4 0.37 (1)
F-G	-2844 / 0	-77.4	-77.4 0.47 (1)	3.73	D-O	0 / 122	0.03 (1)	F-G	-2844 / 0	-77.4	-77.4 0.47 (1)
G-H	-2425 / 0	-77.4	-77.4 0.42 (1)	4.03	O-F	-387 / 0	0.15 (1)	G-H	-2425 / 0	-77.4	-77.4 0.42 (1)
H-I	-1774 / 0	-77.4	-77.4 0.42 (1)	4.58	O-G	0 / 574	0.13 (1)	H-I	-1774 / 0	-77.4	-77.4 0.42 (1)
I-J	0 / 30	-77.4	-77.4 0.09 (1)	10.00	M-G	-839 / 0	0.33 (1)	I-J	0 / 30	-77.4	-77.4 0.09 (1)
T-A	-1596 / 0	0.0	0.0 0.24 (1)	6.54	M-H	0 / 1372	0.31 (1)	T-A	-1596 / 0	0.0	0.0 0.24 (1)
K-I	-1678 / 0	0.0	0.0 0.17 (1)	6.41	L-H	-220 / 16	0.09 (1)	K-I	-1678 / 0	0.0	0.0 0.17 (1)
T-S	0 / 0	-17.5	-17.5 0.08 (4)	10.00	A-S	0 / 1222	0.27 (1)	T-S	0 / 0	-17.5	-17.5 0.08 (4)
S-R	0 / 956	-17.5	-17.5 0.21 (1)	10.00	L-I	0 / 1453	0.33 (1)	S-R	0 / 956	-17.5	-17.5 0.21 (1)
R-Q	0 / 2155	-17.5	-17.5 0.39 (1)	10.00				R-Q	0 / 2155	-17.5	-17.5 0.39 (1)
Q-P	0 / 2755	-17.5	-17.5 0.49 (1)	10.00				Q-P	0 / 2755	-17.5	-17.5 0.49 (1)
P-O	0 / 2755	-17.5	-17.5 0.49 (1)	10.00				P-O	0 / 2755	-17.5	-17.5 0.49 (1)
O-N	0 / 2425	-17.5	-17.5 0.43 (1)	10.00				O-N	0 / 2425	-17.5	-17.5 0.43 (1)
N-M	0 / 2425	-17.5	-17.5 0.43 (1)	10.00				N-M	0 / 2425	-17.5	-17.5 0.43 (1)
M-L	0 / 1412	-17.5	-17.5 0.29 (1)	10.00				M-L	0 / 1412	-17.5	-17.5 0.29 (1)
L-K	0 / 0	-17.5	-17.5 0.11 (4)	10.00				L-K	0 / 0	-17.5	-17.5 0.11 (4)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Tue Jun 5 16:37:08 2018 Page 1
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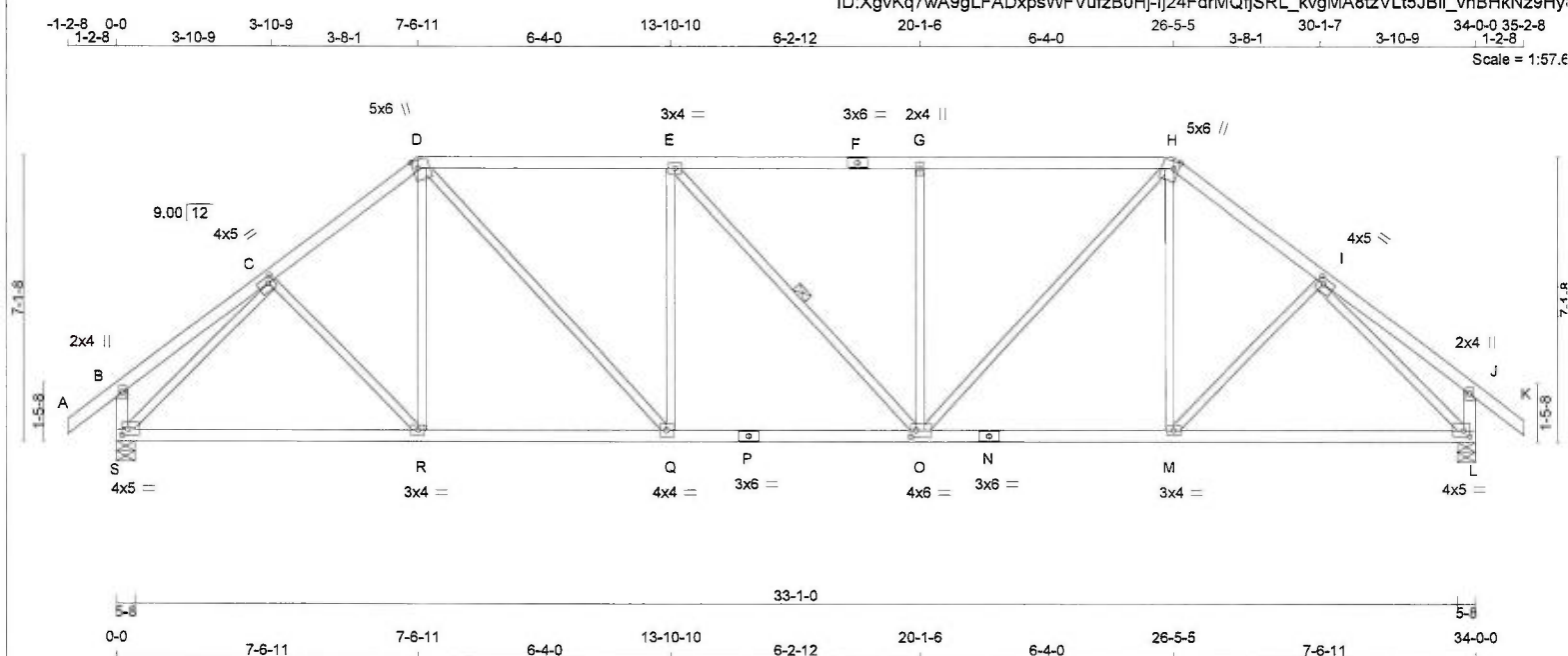


LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY				TOTAL WEIGHT = 144 lb			
N. L. G. A. RULES				BUILDING DESIGNER				[M]			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				DESIGN CRITERIA			
A - B	2x4	DRY	No.2	FACTORED				SPECIFIED LOADS:			
B - E	2x4	DRY	No.2	GROSS REACTION				TOP CH. LL = 23.3 PSF			
E - G	2x4	DRY	No.2	DOWN				DL = 3.0 PSF			
G - I	2x4	DRY	No.2	UP				BOT CH. LL = 0.0 PSF			
R - A	2x4	DRY	No.2	MECHANICAL				DL = 7.0 PSF			
J - H	2x4	DRY	No.2	5-8				TOTAL LOAD = 33.3 PSF			
R - O	2x4	DRY	No.2	5-8				SPACING = 24.0 IN. C/C			
O - M	2x4	DRY	No.2	5-8				LOADING IN FLAT SECTION BASED ON A			
M - J	2x4	DRY	No.2	5-8				SLOPE OF 2.00/12 MINIMUM			
ALL WEBS	2x3	DRY	No.2	UNFACTORED REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
EXCEPT				1ST LCASE				OR SMALL BUILDING REQUIREMENTS OF			
				MAX./MIN. COMPONENT REACTIONS				PART 9, NBC 2010			
				JT COMBINED				THIS DESIGN COMPLIES WITH:			
				R 1132				- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
				J 1200				- CSA 086-09			
								- TPIC 2011			
								(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.			
								RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED			
								ROOF LIVE LOAD			
								ALLOWABLE DEFL.(LL)= L/360 (1.13")			
								CALCULATED VERT. DEFL.(LL)= L/ 999 (0.13")			
								ALLOWABLE DEFL.(TL)= L/360 (1.13")			
								CALCULATED VERT. DEFL.(TL)= L/ 999 (0.23")			
								CSI: TC=0.73/1.00 (G-H:1), BC=0.41/1.00 (L-N:1),			
								WB=0.50/1.00 (C-P:1), SSI=0.21/1.00 (B-C:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
								COMP=1.10 SHEAR=1.10 TENS= 1.10			
								COMPANION LIVE LOAD FACTOR = 0.50			
								TRUSS PLATE MANUFACTURER IS NOT			
								RESPONSIBLE FOR QUALITY CONTROL IN			
								THE TRUSS MANUFACTURING PLANT .			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 618 354 1667 822 2284 1656			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.90 (B) (INPUT = 0.90)			
								JSI METAL= 0.55 (M) (INPUT = 1.00)			

DRY: SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J			
PLATES (table is in inches)				BRACING			
JT TYPE	PLATES	W	LEN Y X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.			
A TMVW-t	MT20	4.0	5.0 1.50 Edge	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY			
B TTWW+m	MT20	6.0	6.0 Edge 2.00	APPLIED.			
C TMWW-t	MT20	3.0	4.0	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.			
D TMW+w	MT20	2.0	4.0	LOADING			
E TS-t	MT20	3.0	6.0	TOTAL LOAD CASES: (4)			
F TMWW-t	MT20	3.0	4.0	CHORDS			
G TTWW-m	MT20	5.0	6.0 1.75 1.75	MAX. FACTORED			
H TMVW-t	MT20	4.0	6.0 1.50 2.75	MEMB. FORCE			
J BMV1+p	MT20	2.0	4.0	VERT. LOAD LC1 MAX			
K BMWW-t	MT20	4.0	4.0 1.50 1.50	CS1 (LC)			
L BMWW-t	MT20	4.0	4.0 2.00 1.50	MAX. MEMB. FORCE			
M BS-t	MT20	3.0	6.0	UNBRAC			
N BMWW-t	MT20	3.0	8.0	LENGTH FR-TO			
O BS-t	MT20	3.0	6.0	MEMB. FORCE			
P BMWW-t	MT20	4.0	4.0 1.50 1.50	MAX CS1 (LC)			
Q BMWW-t	MT20	4.0	4.0 1.75 1.50	FR-TO			
R BMV1+p	MT20	2.0	4.0	A-B -1365 / 0			
				B-C -2042 / 0			
				C-D -2379 / 0			
				D-E -2379 / 0			
				E-F -2379 / 0			
				F-G -2208 / 0			
				G-H -1783 / 0			
				H-I 0 / 30			
				R-A -1584 / 0			
				J-H -1668 / 0			
				R-Q 0 / 0			
				Q-P 0 / 1081			
				P-O 0 / 2042			
				O-N 0 / 2042			
				N-M 0 / 2208			
				M-L 0 / 2208			
				L-K 0 / 1422			
				K-J 0 / 0			

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
F - H 2x4 DRY No.2 SPF
H - K 2x4 DRY No.2 SPF
S - B 2x4 DRY No.2 SPF
L - J 2x4 DRY No.2 SPF
S - P 2x4 DRY No.2 SPF
P - N 2x4 DRY No.2 SPF
N - L 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
S	1713	0	1713	0
L	1713	0	1713	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1200	852 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0
L	1200	852 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O. DBS = 20-0-0 . CBF = 0 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 30	-77.4 -77.4	0.09 (1)	10.00	C-R	0 / 62	0.02 (4)
B-C	0 / 19	-77.4 -77.4	0.16 (1)	10.00	R-D	0 / 115	0.04 (4)
C-D	-1777 / 0	-77.4 -77.4	0.21 (1)	4.82	D-Q	0 / 858	0.19 (1)
D-E	-1989 / 0	-77.4 -77.4	0.56 (1)	4.21	Q-E	-526 / 0	0.46 (1)
E-F	-1988 / 0	-77.4 -77.4	0.55 (1)	4.21	E-O	-2 / 0	0.00 (1)
F-G	-1988 / 0	-77.4 -77.4	0.55 (1)	4.21	O-G	-526 / 0	0.46 (1)
G-H	-1987 / 0	-77.4 -77.4	0.55 (1)	4.22	H-I	0 / 856	0.19 (1)
H-I	-1777 / 0	-77.4 -77.4	0.21 (1)	4.82	I-L	0 / 115	0.04 (4)
I-J	0 / 19	-77.4 -77.4	0.16 (1)	10.00	M-I	0 / 62	0.02 (4)
J-K	0 / 30	-77.4 -77.4	0.09 (1)	10.00	S-C	-1992 / 0	0.96 (1)
S-B	-215 / 0	0.0 0.0	0.02 (1)	7.81	I-L	-1992 / 0	0.96 (1)
L-J	-215 / 0	0.0 0.0	0.02 (1)	7.81			
S-R	0 / 1374	-17.5 -17.5	0.35 (1)	10.00			
R-Q	0 / 1405	-17.5 -17.5	0.35 (1)	10.00			
Q-P	0 / 1989	-17.5 -17.5	0.38 (1)	10.00			
P-O	0 / 1989	-17.5 -17.5	0.38 (1)	10.00			
O-N	0 / 1405	-17.5 -17.5	0.35 (1)	10.00			
N-M	0 / 1405	-17.5 -17.5	0.35 (1)	10.00			
M-L	0 / 1374	-17.5 -17.5	0.35 (1)	10.00			

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.56/1.00 (D-E:1), BC=0.38/1.00 (C-Q:1),
WB=0.96/1.00 (I-L:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

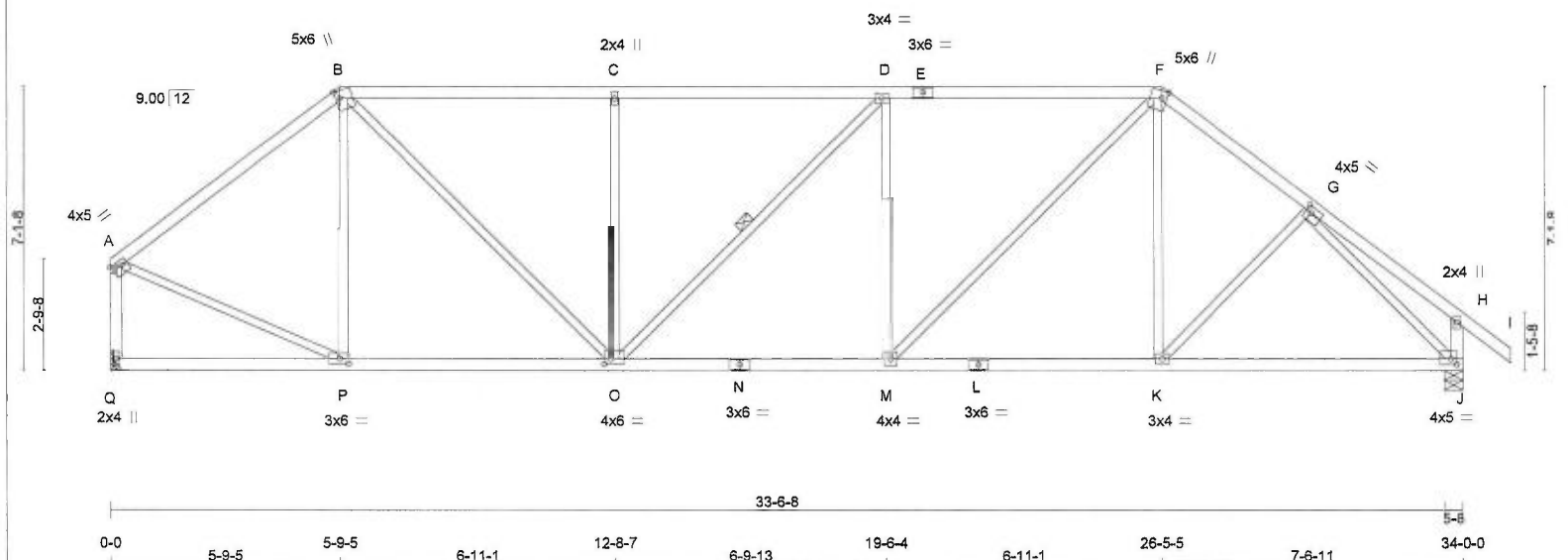
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.59 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 145 lb
 [M]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
Q	1613	0	1613	0	MECHANICAL	5-8
J	1713	0	1713	0	MECHANICAL	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT Q TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
Q	1132	792 / 0	0 / 0	0 / 0	340 / 0	0 / 0
J	1200	852 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O. DBS = 20-0-0. CBF = 15 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-1442 / 0	-77.4	-77.4	0.56 (1)	4.77	P-B	-357 / 0	0.31 (1)	
B-C	-1938 / 0	-77.4	-77.4	0.67 (1)	4.10	B-O	0 / 1111	0.25 (1)	
C-D	-1938 / 0	-77.4	-77.4	0.67 (1)	4.10	O-C	-578 / 0	0.51 (1)	
D-E	-2022 / 0	-77.4	-77.4	0.69 (1)	4.02	O-D	-118 / 0	0.08 (1)	
E-F	-2022 / 0	-77.4	-77.4	0.69 (1)	4.02	M-D	-494 / 0	0.44 (1)	
F-G	-1777 / 0	-77.4	-77.4	0.21 (1)	4.81	M-F	0 / 865	0.19 (1)	
G-H	0 / 19	-77.4	-77.4	0.16 (1)	10.00	K-F	0 / 119	0.04 (4)	
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	K-G	0 / 62	0.02 (4)	
Q-A	-1573 / 0	0.0	0.0	0.23 (1)	6.58	A-P	0 / 1249	0.28 (1)	
J-H	-215 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-1993 / 0	0.96 (1)	
Q-P	0 / 0	-17.5	-17.5	0.17 (4)	10.00				
P-O	0 / 1146	-17.5	-17.5	0.28 (1)	10.00				
O-N	0 / 2022	-17.5	-17.5	0.40 (1)	10.00				
N-M	0 / 2022	-17.5	-17.5	0.40 (1)	10.00				
M-L	0 / 1406	-17.5	-17.5	0.36 (1)	10.00				
L-K	0 / 1406	-17.5	-17.5	0.36 (1)	10.00				
K-J	0 / 1374	-17.5	-17.5	0.35 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL)= L/360 (1.13")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.69/1.00 (D-F:1), BC=0.40/1.00 (M-O:1),
 WB=0.96/1.00 (G-J:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

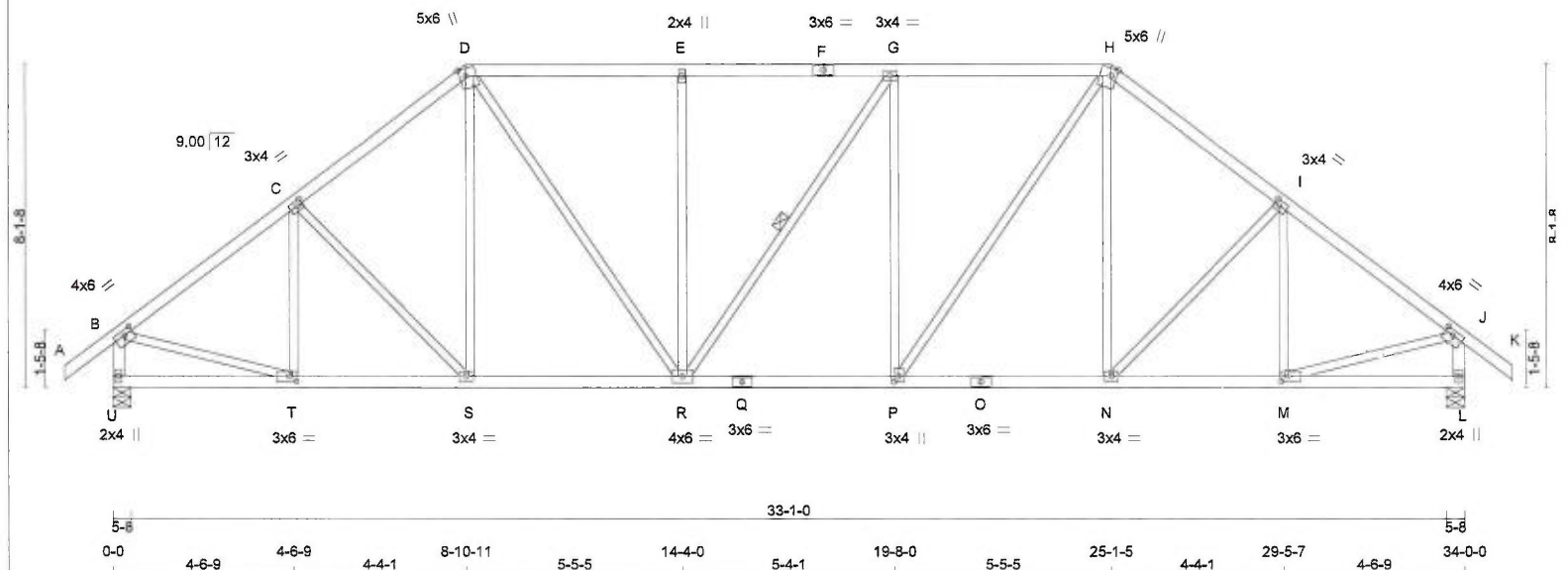
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
 JSI METAL= 0.60 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 160 lb [M/JF]

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT		
U	1713	0	1713	0	5-8	5-8
L	1713	0	1713	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE		
U	1200	852 / 0	0 / 0	0 / 0	348 / 0	0 / 0
L	1200	852 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R. DBS = 20-0-0. CBF = 0 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A- B	0 / 30	-77.4	-77.4	0.09 (1)	10.00	T- C	-279 / 0	0.10 (1)	
B- C	-1780 / 0	-77.4	-77.4	0.24 (1)	4.80	C- S	-121 / 0	0.09 (1)	
C- D	-1725 / 0	-77.4	-77.4	0.23 (1)	4.86	S- D	0 / 177	0.04 (4)	
D- E	-1738 / 0	-77.4	-77.4	0.30 (1)	4.77	D- R	0 / 662	0.15 (1)	
E- F	-1738 / 0	-77.4	-77.4	0.28 (1)	4.80	R- E	-450 / 0	0.57 (1)	
F- G	-1738 / 0	-77.4	-77.4	0.28 (1)	4.80	R- G	-2 / 0	0.00 (1)	
G- H	-1740 / 0	-77.4	-77.4	0.30 (1)	4.77	P- G	-450 / 0	0.57 (1)	
H- I	-1725 / 0	-77.4	-77.4	0.23 (1)	4.86	P- H	0 / 665	0.15 (1)	
I- J	-1780 / 0	-77.4	-77.4	0.24 (1)	4.80	N- H	0 / 175	0.04 (4)	
J- K	0 / 30	-77.4	-77.4	0.09 (1)	10.00	N- I	-121 / 0	0.09 (1)	
U- B	-1677 / 0	0.0	0.0	0.17 (1)	6.41	M- I	-279 / 0	0.10 (1)	
L- J	-1677 / 0	0.0	0.0	0.17 (1)	6.41	B- T	0 / 1486	0.33 (1)	
						M- J	0 / 1486	0.33 (1)	
U- T	0 / 0	-17.5	-17.5	0.07 (4)	10.00				
T- S	0 / 1442	-17.5	-17.5	0.28 (1)	10.00				
S- R	0 / 1360	-17.5	-17.5	0.27 (1)	10.00				
R- Q	0 / 1740	-17.5	-17.5	0.33 (1)	10.00				
Q- P	0 / 1740	-17.5	-17.5	0.33 (1)	10.00				
P- O	0 / 1360	-17.5	-17.5	0.27 (1)	10.00				
O- N	0 / 1360	-17.5	-17.5	0.27 (1)	10.00				
N- M	0 / 1443	-17.5	-17.5	0.28 (1)	10.00				
M- L	0 / 0	-17.5	-17.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (1.13")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.30/1.00 (G-H:1), BC=0.33/1.00 (P-R:1),
 WB=0.57/1.00 (G-P:1), SSI=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

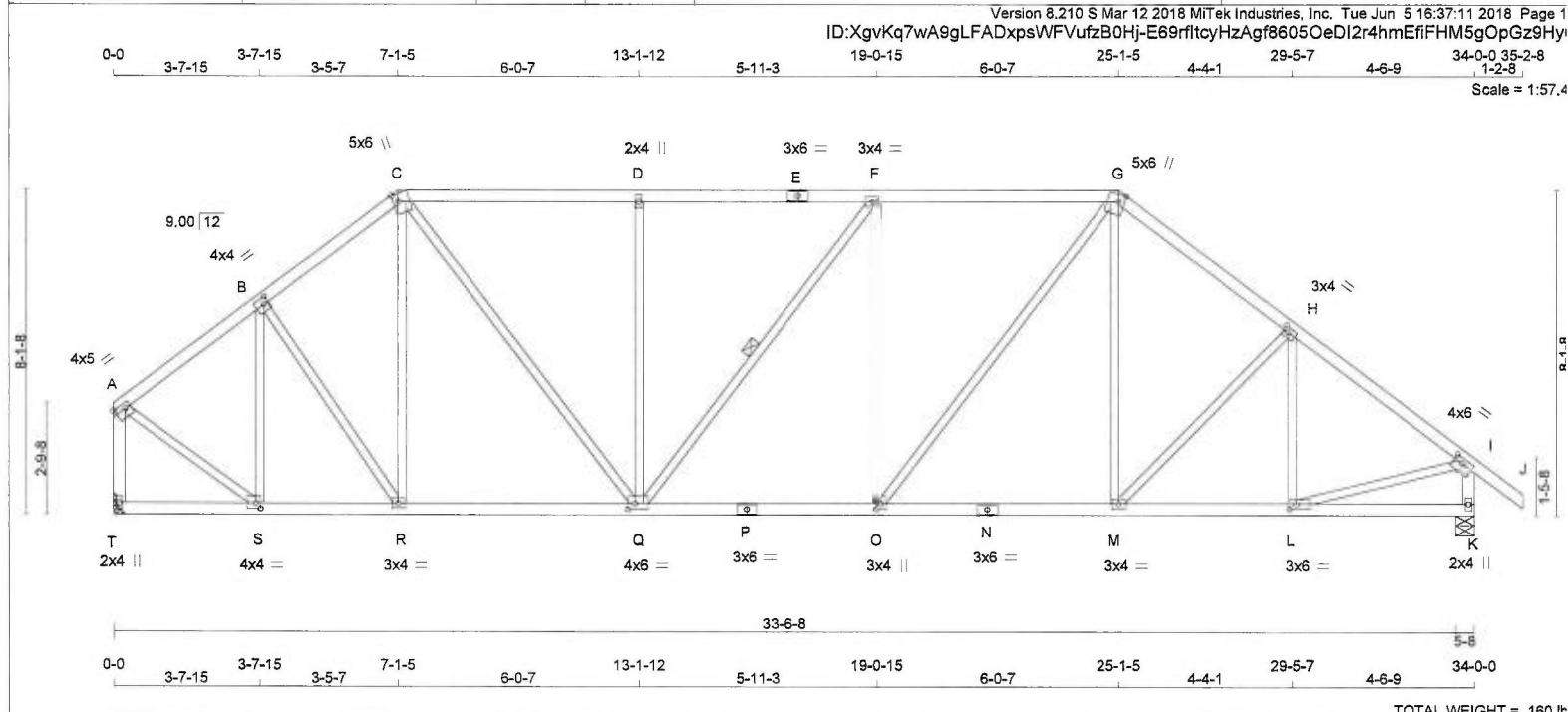
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
 JSI METAL= 0.49 (J) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - J	2x4	DRY	No.2 SPF
T - A	2x4	DRY	No.2 SPF
K - I	2x4	DRY	No.2 SPF
T - P	2x4	DRY	No.2 SPF
P - N	2x4	DRY	No.2 SPF
N - K	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	5.0	1.50	Edge
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	3.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.75
H	TMVW-t	MT20	3.0	4.0	1.50	1.50
I	TMVW-t	MT20	4.0	6.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	3.0	6.0	1.50	2.00
M	BMVW-t	MT20	3.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW+t	MT20	3.0	4.0	1.75	1.50
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	6.0	1.75	2.00
R	BMVW-t	MT20	3.0	4.0		
S	BMVW-t	MT20	4.0	4.0	1.75	1.50
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
BEARINGS						
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
T	1613	0	1613	0	0	MECHANICAL
K	1713	0	1713	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT T TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	WIND	DEAD	SOIL
T	1132	792 / 0	0 / 0	0 / 0	0 / 0	340 / 0	0 / 0
K	1200	852 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q. DBS = 20-0-0. CBF = 13 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1275 / 0	-77.4 -77.4	0.18 (1)	5.50	S-B	-639 / 0	0.30 (1)
B-C	-1483 / 0	-77.4 -77.4	0.19 (1)	5.18	B-R	0 / 235	0.05 (1)
C-D	-1700 / 0	-77.4 -77.4	0.46 (1)	4.59	R-C	-103 / 32	0.13 (1)
D-E	-1700 / 0	-77.4 -77.4	0.47 (1)	4.58	C-Q	0 / 873	0.20 (1)
E-F	-1700 / 0	-77.4 -77.4	0.47 (1)	4.58	Q-D	-501 / 0	0.64 (1)
F-G	-1765 / 0	-77.4 -77.4	0.48 (1)	4.50	Q-F	-108 / 0	0.07 (1)
G-H	-1726 / 0	-77.4 -77.4	0.31 (1)	4.75	O-F	-417 / 0	0.53 (1)
H-I	-1779 / 0	-77.4 -77.4	0.32 (1)	4.69	O-G	0 / 661	0.15 (1)
I-J	0 / 30	-77.4 -77.4	0.09 (1)	10.00	M-G	0 / 181	0.04 (4)
T-A	-1584 / 0	0.0 0.0	0.23 (1)	6.56	M-H	-120 / 0	0.09 (1)
K-I	-1677 / 0	0.0 0.0	0.17 (1)	6.41	L-H	-281 / 0	0.10 (1)
					A-S	0 / 1248	0.28 (1)
					L-I	0 / 1486	0.33 (1)
T-S	0 / 0	-17.5 -17.5	0.05 (4)	10.00			
S-R	0 / 1035	-17.5 -17.5	0.22 (1)	10.00			
R-Q	0 / 1166	-17.5 -17.5	0.25 (1)	10.00			
Q-P	0 / 1765	-17.5 -17.5	0.35 (1)	10.00			
P-O	0 / 1765	-17.5 -17.5	0.35 (1)	10.00			
O-N	0 / 1361	-17.5 -17.5	0.28 (1)	10.00			
N-M	0 / 1361	-17.5 -17.5	0.28 (1)	10.00			
M-L	0 / 1442	-17.5 -17.5	0.29 (1)	10.00			
L-K	0 / 0	-17.5 -17.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.48/1.00 (F-G:1), BC=0.35/1.00 (O-Q:1), WB=0.64/1.00 (D-Q:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

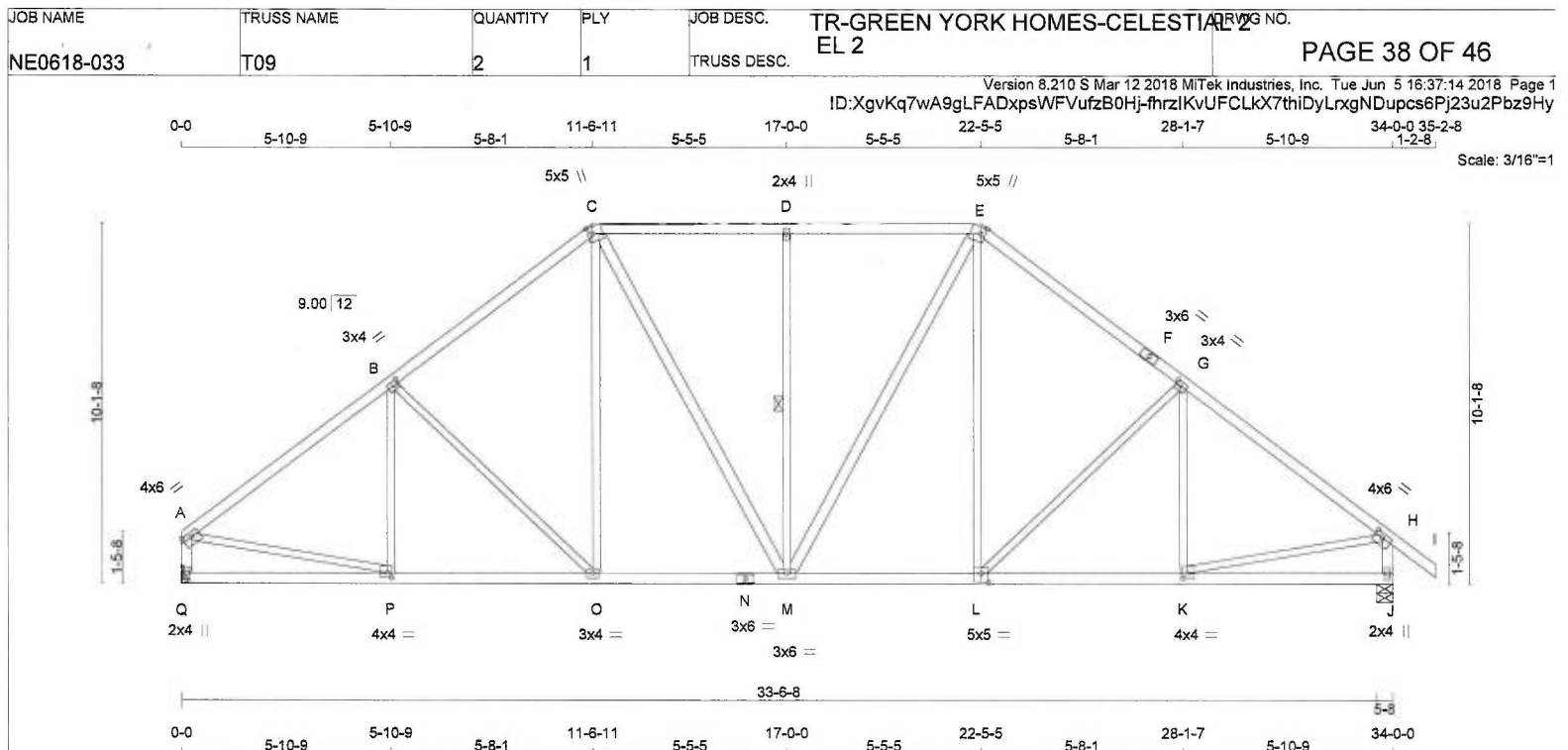
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.50 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 167 = 334 lb
 [M]F

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
Q - A	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
Q - N	2x4	DRY	No.2 SPF
N - L	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
C - M	2x4	DRY	No.2 SPF
M - E	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG MECHANICAL	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
Q	1613	0	1613	0	0	5-8
J	1713	0	1713	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT Q TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
Q	1132	792 / 0	0 / 0	0 / 0	0 / 0	340 / 0	0 / 0
J	1200	852 / 0	0 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M. DBS = 20-0-0. CBF = 64 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO					
FR-TO								
A-B	-1819 / 0	-77.4	-77.4	0.42 (1)	4.57	P-B	-172 / 46	0.09 (1)
B-C	-1604 / 0	-77.4	-77.4	0.39 (1)	4.82	B-O	-313 / 0	0.42 (1)
C-D	-1426 / 0	-77.4	-77.4	0.32 (1)	5.11	O-C	0 / 313	0.07 (1)
D-E	-1426 / 0	-77.4	-77.4	0.32 (1)	5.11	C-M	0 / 346	0.06 (1)
E-F	-1604 / 0	-77.4	-77.4	0.39 (1)	4.82	M-D	-513 / 0	0.36 (1)
F-G	-1604 / 0	-77.4	-77.4	0.39 (1)	4.82	M-E	0 / 346	0.06 (1)
G-H	-1819 / 0	-77.4	-77.4	0.42 (1)	4.57	L-E	0 / 313	0.07 (1)
H-I	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-G	-314 / 0	0.42 (1)
Q-A	-1570 / 0	0.0	0.0	0.16 (1)	6.58	K-G	-173 / 46	0.09 (1)
J-H	-1670 / 0	0.0	0.0	0.17 (1)	6.42	A-P	0 / 1508	0.34 (1)
						K-H	0 / 1508	0.34 (1)
Q-P	0 / 0	-17.5	-17.5	0.14 (4)	10.00			
P-O	0 / 1481	-17.5	-17.5	0.31 (1)	10.00			
O-N	0 / 1258	-17.5	-17.5	0.26 (1)	10.00			
N-M	0 / 1258	-17.5	-17.5	0.26 (1)	10.00			
M-L	0 / 1258	-17.5	-17.5	0.26 (1)	10.00			
L-K	0 / 1481	-17.5	-17.5	0.31 (1)	10.00			
K-J	0 / 0	-17.5	-17.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
 ALLOWABLE DEFL.(TL)= L/360 (1.13")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.42/1.00 (G-H:1), BC=0.31/1.00 (O-P:1),
 WB=0.42/1.00 (G-L:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

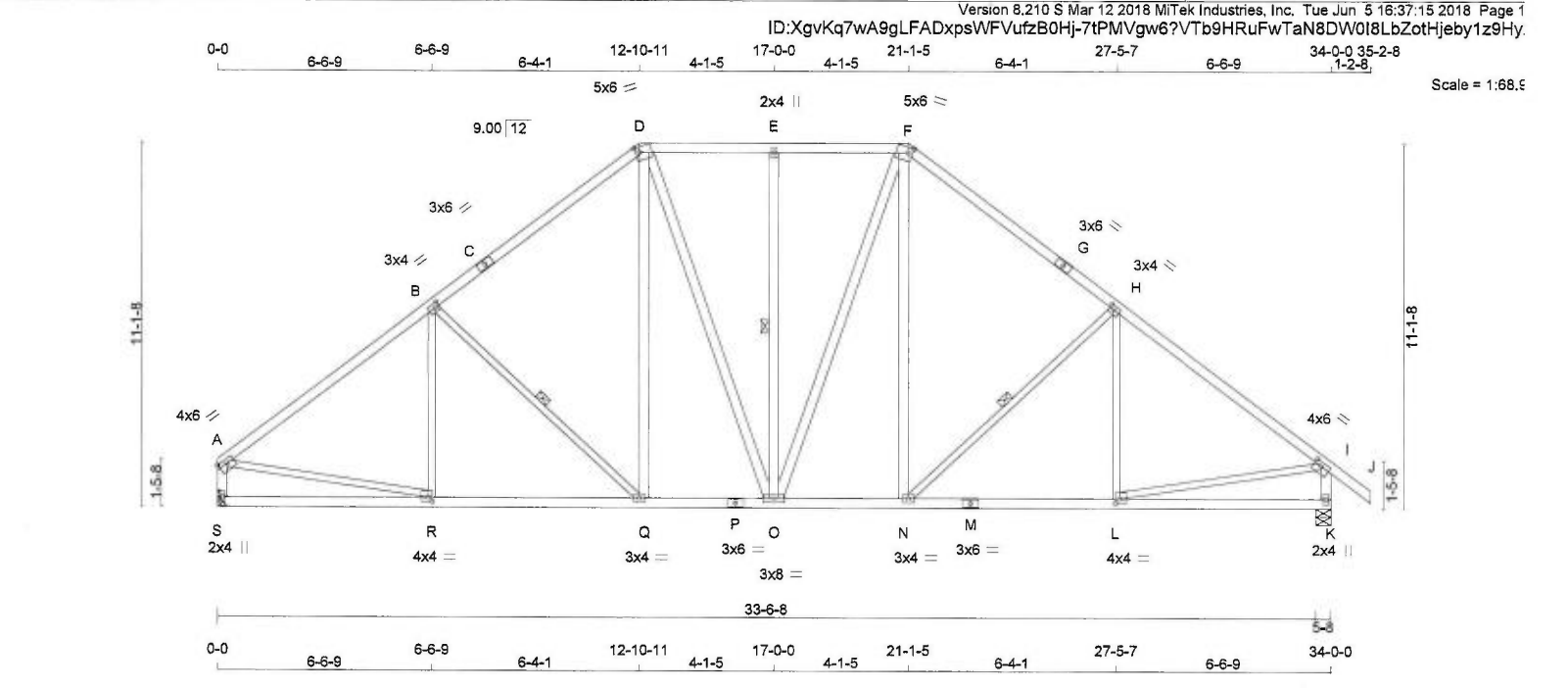
JSI GRIP= 0.89 (K) (INPUT = 0.90)
 JSI METAL= 0.55 (K) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



CONTINUED ON PAGE 2





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
S - A	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
R - B	2x3	DRY	No.2
B - Q	2x3	DRY	No.2
N - H	2x3	DRY	No.2
L - H	2x3	DRY	No.2
A - R	2x3	DRY	No.2
L - I	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	Edge
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TS-t	MT20	3.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	3.0	4.0	1.50	1.50
I	TMVW-t	MT20	4.0	6.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	3.0	8.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	3.0	4.0		
R	BMVW-t	MT20	4.0	4.0	1.50	1.50
S	BMV1+p	MT20	2.0	4.0		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
S	1643	0	1643	0	0	0	0	MECHANICAL	
K	1744	0	1744	0	0	5-8	5-8		
A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT S TO RESIST THE MAX FACTORED REACTIONS.									
UNFACTORED REACTIONS									
1ST LCASE	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
S	1156	792 / 0	0 / 0	0 / 0	0 / 0	365 / 0	0 / 0		
K	1225	852 / 0	0 / 0	0 / 0	0 / 0	372 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K									
BRACING									
FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.									
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF B-Q, E-O, H-N. DBS = 20-0-0. CBF = 52 LBS.									
DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)		
FR-TO		FROM	TO	CSI (LC)	FR-TO				
A-B	-1865 / 0	-77.4	-77.4	0.54 (1)	4.37	R-B	-134 / 65	0.09 (1)	
B-C	-1578 / 0	-77.4	-77.4	0.49 (1)	4.72	B-Q	-398 / 0	0.21 (1)	
C-D	-1578 / 0	-77.4	-77.4	0.49 (1)	4.72	Q-D	0 / 367	0.06 (1)	
D-E	-1324 / 0	-84.9	-84.9	0.20 (1)	2.00	D-O	0 / 253	0.04 (1)	
E-F	-1324 / 0	-84.9	-84.9	0.20 (1)	2.00	O-E	-418 / 0	0.27 (1)	
F-G	-1578 / 0	-77.4	-77.4	0.49 (1)	4.72	Q-F	0 / 253	0.04 (1)	
G-H	-1578 / 0	-77.4	-77.4	0.49 (1)	4.72	N-F	0 / 367	0.06 (1)	
H-I	-1865 / 0	-77.4	-77.4	0.54 (1)	4.37	N-H	-398 / 0	0.21 (1)	
I-J	0 / 30	-77.4	-77.4	0.09 (1)	10.00	L-H	-134 / 65	0.09 (1)	
S-A	-1597 / 0	0.0	0.0	0.17 (1)	6.54	A-R	0 / 1544	0.35 (1)	
K-I	-1697 / 0	0.0	0.0	0.18 (1)	6.38	L-I	0 / 1544	0.35 (1)	
S-R	0 / 0	-17.5	-17.5	0.18 (4)	10.00				
R-Q	0 / 1521	-17.5	-17.5	0.34 (1)	10.00				
Q-P	0 / 1235	-17.5	-17.5	0.26 (1)	10.00				
P-O	0 / 1235	-17.5	-17.5	0.26 (1)	10.00				
O-N	0 / 1235	-17.5	-17.5	0.26 (1)	10.00				
N-M	0 / 1521	-17.5	-17.5	0.34 (1)	10.00				
M-L	0 / 1521	-17.5	-17.5	0.34 (1)	10.00				
L-K	0 / 0	-17.5	-17.5	0.18 (4)	10.00				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH. LL	=	23.3	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	33.3	PSF
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 9.00/12 AND -9.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
- CSA 086-09			
- TPIC 2011			
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (1.13")			
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.07")			
ALLOWABLE DEFL.(TL)= L/360 (1.13")			
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.13")			
CSI: TC=0.54/1.00 (A-B:1), BC=0.34/1.00 (Q-R:1), WB=0.35/1.00 (A-R:1), SS=0.20/1.00 (A-B:1)			
DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
COMPANION LIVE LOAD FACTOR = 0.50			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR SECTION (PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.90 (L) (INPUT = 0.90)			
JSI METAL= 0.57 (R) (INPUT = 1.00)			
CONTINUED ON PAGE 2			
KOTT			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

LICENSED PROFESSIONAL ENGINEER
N.A. EL-MASRI
Jun 05, 2018

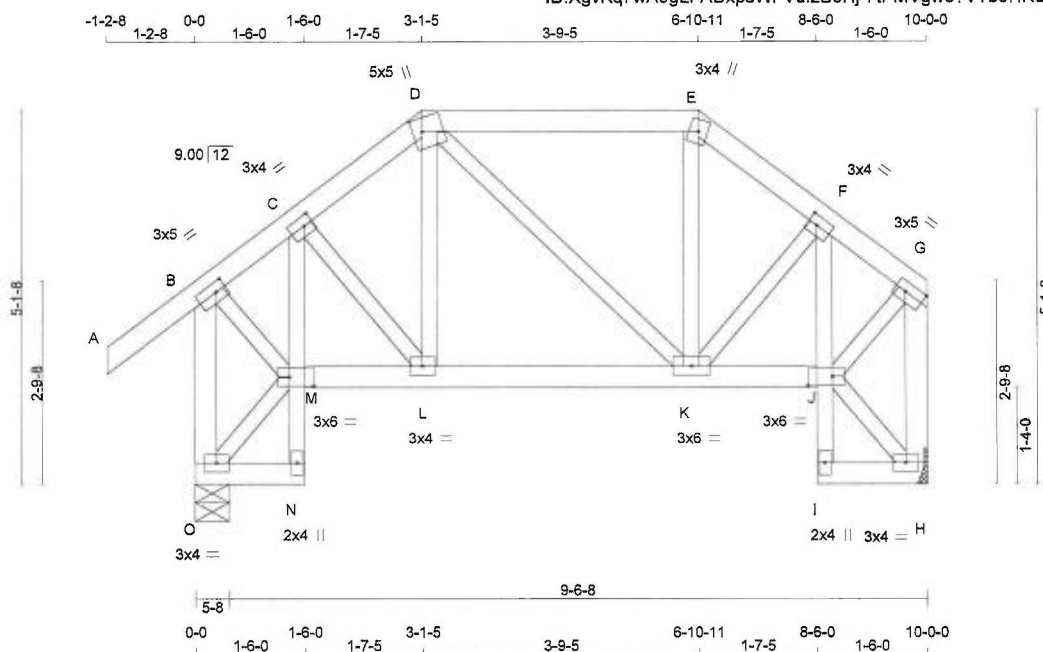
KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TR- GREEN YORK HOMES-CELESTIAL 2	DWG NO.
NE0618-033	T10	4	1	TRUSS DESC.	EL 2	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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Scale = 1:31.5

TOTAL WEIGHT = 56 lb [M/JF]

LUMBER				DESCR.			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER				
A - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
E - G	2x4	DRY	No.2	SPF			
O - B	2x4	DRY	No.2	SPF			
H - G	2x4	DRY	No.2	SPF			
O - N	2x4	DRY	No.2	SPF			
N - C	2x3	DRY	No.2	SPF			
M - J	2x4	DRY	No.2	SPF			
I - F	2x3	DRY	No.2	SPF			
I - H	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	5.0	5.0	2.25	1.50
E	TTW+m	MT20	3.0	4.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TMVW-t	MT20	3.0	5.0	1.50	Edge
H	BMVW1-t	MT20	3.0	4.0		
I	BMV+p	MT20	2.0	4.0		
J	BMVWW-I	MT20	3.0	6.0	1.50	4.00
K	BMVWW-t	MT20	3.0	6.0		
L	BMVWW-t	MT20	3.0	4.0		
M	BMVWW-I	MT20	3.0	6.0	1.50	4.00
N	BMV+p	MT20	2.0	4.0		
O	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
O	581	0	0	581	0	0	5-8	5-8	
H	468	0	0	468	0	0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	MAX/MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	405	297 / 0	0 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
H	328	229 / 0	0 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	C-L	0 / 71	0.02 (1)
B-C	-322 / 0	-77.4	-77.4 0.07 (1)	6.25	L-D	0 / 43	0.01 (4)
C-D	-358 / 0	-77.4	-77.4 0.03 (1)	6.25	D-K	0 / 9	0.00 (1)
D-E	-291 / 0	-77.4	-77.4 0.14 (1)	6.25	K-E	0 / 46	0.02 (4)
E-F	-370 / 0	-77.4	-77.4 0.02 (1)	6.25	K-F	0 / 38	0.01 (1)
F-G	-323 / 0	-77.4	-77.4 0.02 (1)	6.25	B-M	0 / 304	0.07 (1)
O-B	-566 / 0	0.0	0.0 0.08 (1)	7.81	J-G	0 / 340	0.08 (1)
H-G	-453 / 0	0.0	0.0 0.07 (1)	7.81	O-M	-4 / 0	0.00 (1)
					J-H	-3 / 0	0.00 (1)
O-N	0 / 3	-17.5	-17.5 0.01 (4)	10.00			
N-M	0 / 14	0.0	0.0 0.01 (1)	10.00			
M-C	-173 / 0	0.0	0.0 0.01 (1)	7.81			
M-L	0 / 238	-17.5	-17.5 0.06 (1)	10.00			
L-K	0 / 284	-17.5	-17.5 0.07 (1)	10.00			
K-J	0 / 285	-17.5	-17.5 0.06 (1)	10.00			
I-J	0 / 14	0.0	0.0 0.01 (1)	10.00			
J-F	-195 / 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 / 2	-17.5	-17.5 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.14/1.00 (D-E:1), BC=0.07/1.00 (K-L:1), WB=0.08/1.00 (G-J:1), SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

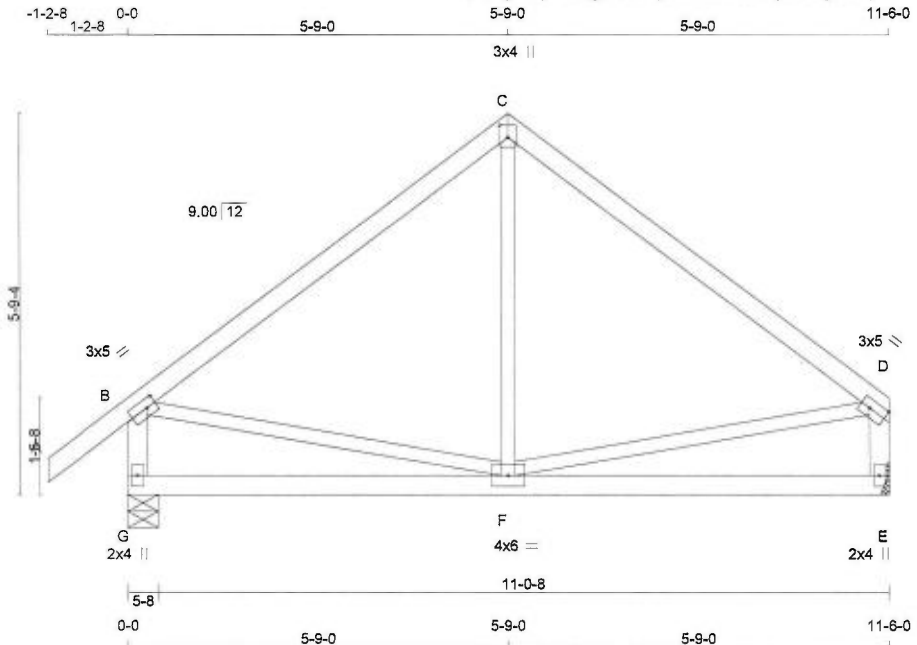
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JSI GRIP= 0.77 (G) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
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TOTAL WEIGHT = 2 X 47 = 94 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	Edge
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	646	0	646	0	0
E	545	0	545	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	451	328 / 0	0 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
E	383	268 / 0	0 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 30	-77.4	-77.4 0.09 (1)	10.00	F-C	0 / 101	0.03 (4)
B-C	-376 / 0	-77.4	-77.4 0.33 (1)	6.25	B-F	0 / 306	0.07 (1)
C-D	-376 / 0	-77.4	-77.4 0.33 (1)	6.25	F-D	0 / 306	0.07 (1)
G-B	-607 / 0	0.0	0.0 0.06 (1)	7.81			
E-D	-507 / 0	0.0	0.0 0.05 (1)	7.81			
G-F	0 / 0	-17.5	-17.5 0.17 (4)	10.00			
F-E	0 / 0	-17.5	-17.5 0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.17/1.00 (E-F:4) ,
WB=0.07/1.00 (B-F:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

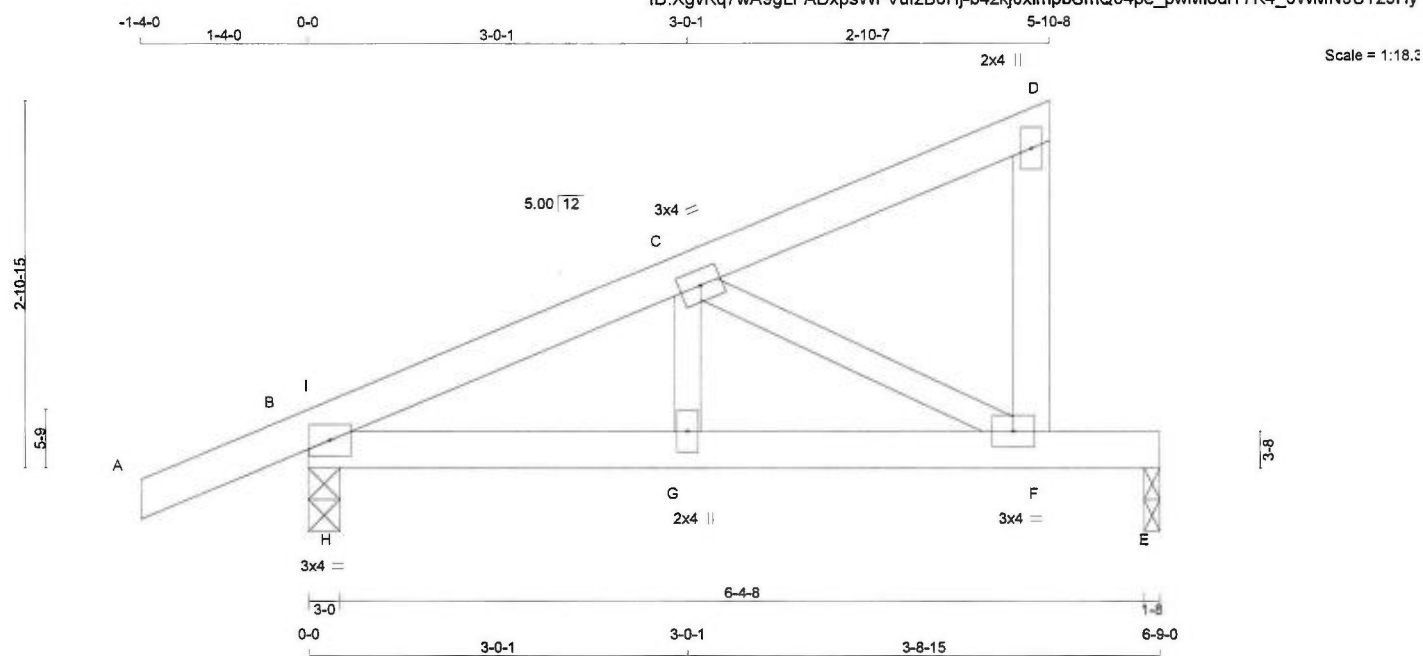
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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TOTAL WEIGHT = 4 X 23 = 90 lb [M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	423	0	423	0	3-0	3-0
E	257	0	257	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				SOIL
	SNOW	LIVE	PERM.	LIVE	WIND	DEAD	
B	295	219 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
E	182	119 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED	FACTORED	MAX	MAX.	MEMB.	MAX. FACTORED	MAX	
	FORCE					VERT. LOAD		UNBRAC
	(LBS)	(PLF)	LC1	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 16	-77.4	-77.4	0.10 (1)	10.00	G-C	0 / 167	0.04 (1)
B-I	-505 / 0	-77.4	-77.4	0.05 (1)	6.25	C-F	-493 / 0	0.10 (1)
I-C	-479 / 0	-77.4	-77.4	0.05 (1)	6.25	H-I	-24 / 37	0.00 (1)
C-D	-7 / 0	-77.4	-77.4	0.07 (1)	10.00			
F-D	-94 / 0	0.0	0.0	0.01 (1)	7.81			
B-H	0 / 442	-17.5	-17.5	0.08 (1)	10.00			
H-G	0 / 442	-17.5	-17.5	0.15 (1)	10.00			
G-F	0 / 442	-17.5	-17.5	0.37 (1)	10.00			
F-E	0 / 0	-17.5	-17.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.10/1.00 (A-B:1), BC=0.37/1.00 (F-G:1), WB=0.10/1.00 (C-F:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (F) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



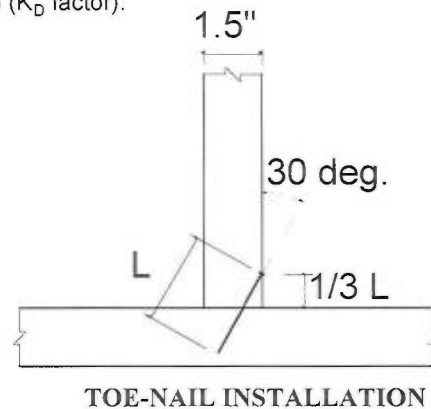
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



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PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

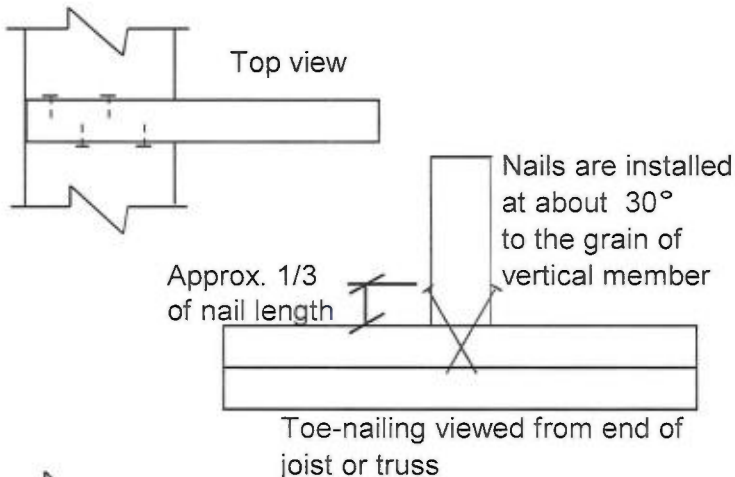
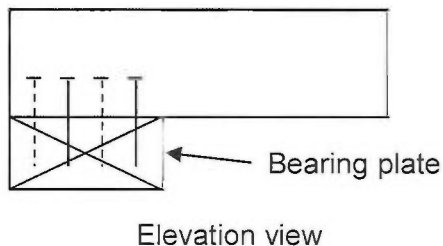
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

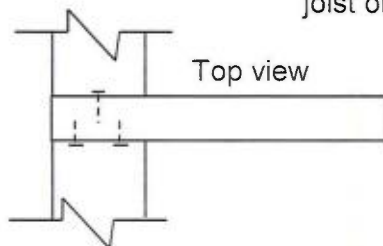
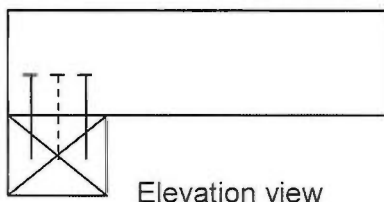
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to wind or earthquake load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017