

NE0518-227**GREEN YORK HOMES - GRANELLI
HOME CORP - AMELIA 12 ELE 1****RESPONSIBILITIES**

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

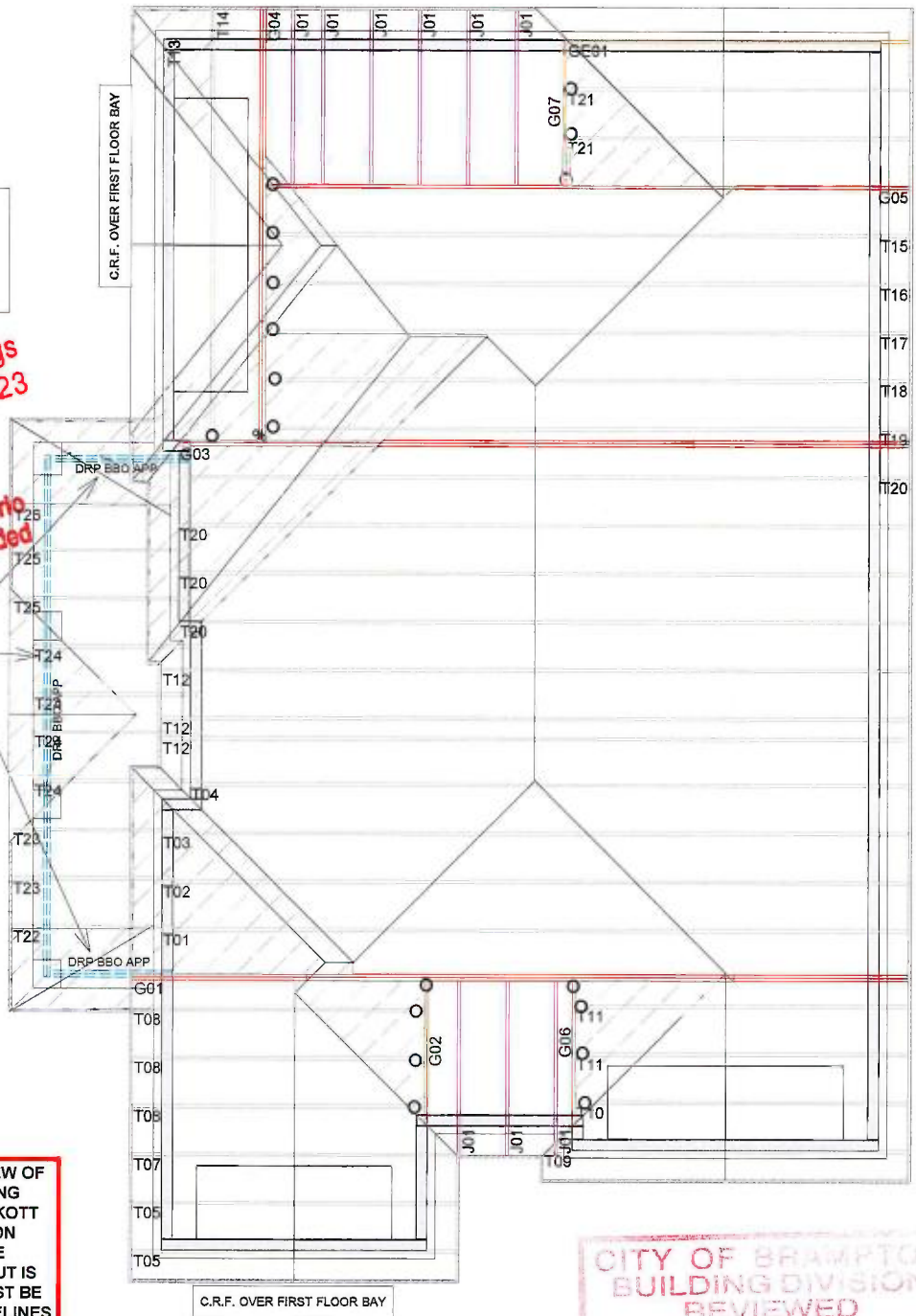
For conventional framing roof / ceilings
framing shall conform to OBC.9.23

Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

For conventional framing roof / ceilings
framing shall conform to OBC.9.23

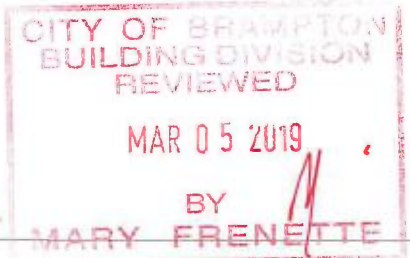
All work shall conform to the Ontario
Building Code O. Reg. 332/12 as amended

PORCH BEAMS ASSUMED TO BE
2x2x10, NO BEAM SCHEDULE
PROVIDED ON PLANS
UNDERSIDE OF BEAMS TO BE 8'
ABOVE FIN. FIRST FLOOR



THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF
STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING
AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT
LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON
THE CALCULATION PAGE OF EACH COMPONENT. THE
PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS
SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE
INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES
AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL
FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED
AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY
AND PERMANENT BRACINGS OF THE ROOF AND BUILDING
ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER
AND TO CONFORM TO BCSI GUIDELINES.

19-444473-000-00RR.



ROOF TRUSS LAYOUT



HANGER LEGEND

X - LUS24
- HGUS26
O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS.
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

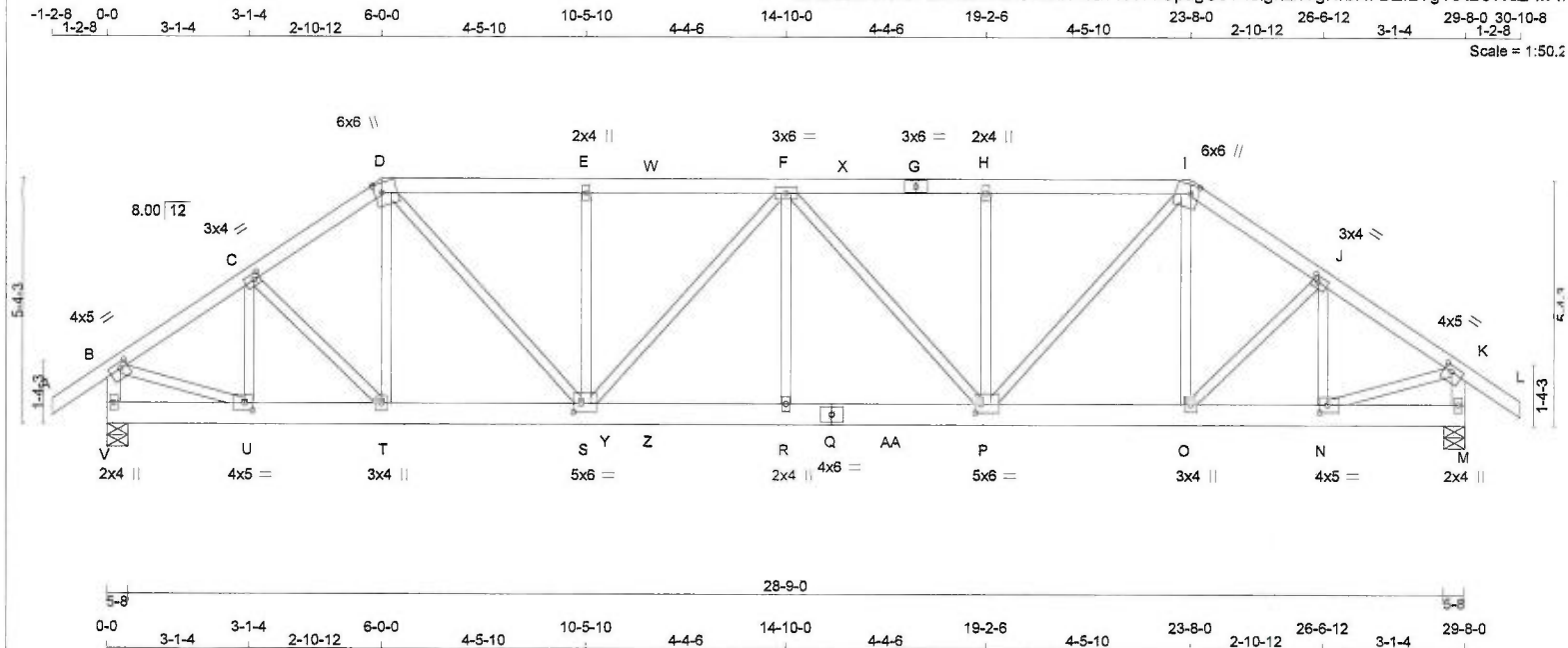
THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	AMILIA 12 ELE 1
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/30/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT
CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE
TRUSS BELOW EA CROSS PT. VERT POS. THAN 6" TO HAVE A LATERAL BRACING SO THAT
THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'



TOTAL WEIGHT = 2 X 149 = 299 lb

LUMBER		N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	SIDE(33.1)
G-I 1	12	TOP
I-L 1	12	TOP
V-B 1	12	TOP
M-K 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-Q 2	12	SIDE(9.1)
Q-M 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2693	0	2693	0	0	5-8	5-8
M	2590	0	2590	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1893	1306 / 0	0 / 0	0 / 0	0 / 0	586 / 0	0 / 0
M	1821	1259 / 0	0 / 0	0 / 0	0 / 0	562 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/28	-77.4	-77.4	0.05 (1)	10.00	U-C	-793/0	0.08 (1)	
B-C	-3000/0	-77.4	-77.4	0.08 (1)	5.27	C-T	0/430	0.05 (1)	
C-D	-3395/0	-77.4	-77.4	0.09 (1)	5.01	T-D	-170/0	0.03 (1)	
D-E	-4442/0	-77.4	-77.4	0.22 (1)	4.37	D-S	0/2445	0.30 (1)	
E-W	-4442/0	-77.4	-77.4	0.22 (1)	4.37	S-E	-431/0	0.09 (1)	
W-F	-4442/0	-143.6	-143.6	0.22 (1)	4.37	S-F	-785/0	0.31 (1)	
F-X	-4271/0	-143.6	-143.6	0.21 (1)	4.45	R-F	0/825	0.10 (1)	
X-G	-4271/0	-77.4	-77.4	0.21 (1)	4.45	F-P	-1044/0	0.41 (1)	
G-H	-4271/0	-77.4	-77.4	0.21 (1)	4.45	P-H	-360/0	0.07 (1)	
H-I	-4271/0	-77.4	-77.4	0.18 (1)	4.48	P-I	0/2399	0.30 (1)	
I-J	-3228/0	-77.4	-77.4	0.08 (1)	5.11	O-I	-237/0	0.05 (1)	
J-K	-2882/0	-77.4	-77.4	0.08 (1)	5.35	O-J	0/372	0.05 (1)	
K-L	0/28	-77.4	-77.4	0.05 (1)	10.00	N-J	-725/0	0.07 (1)	
V-B	-2632/0	0.0	0.0	0.15 (1)	7.01	B-U	0/2618	0.32 (1)	
M-K	-2536/0	0.0	0.0	0.14 (1)	7.11	N-K	0/2516	0.31 (1)	

V- U	0/0	-18.2	-18.2	0.04 (1)	10.00
U- T	0/2505	-18.2	-18.2	0.20 (1)	10.00
T- S	0/2805	-18.2	-18.2	0.21 (1)	10.00
S- Y	0/2805	-18.2	-18.2	0.28 (1)	10.00
Y- Z	0/4980	-18.2	-18.2	0.48 (1)	10.00
Z- R	0/4980	-34.4	-34.4	0.48 (1)	10.00
R- Q	0/4980	-34.4	-34.4	0.63 (1)	10.00
Q- AA	0/4980	-18.2	-18.2	0.63 (1)	10.00
AA- P	0/4980	-18.2	-18.2	0.63 (1)	10.00
P- O	0/2665	-18.2	-18.2	0.27 (1)	10.00
O- N	0/2407	-18.2	-18.2	0.19 (1)	10.00
N- M	0/0	-18.2	-18.2	0.03 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	10-11-4	-891	-891	—	FRONT	VERT	TOTAL
AA	17-0-12	-1011	-1011	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD		= 33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.22/1.00 (D-E:1), BC=0.63/1.00 (P-R:1),
WB=0.41/1.00 (F-P:1), SS=0.25/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

CONTINUED ON PAGE 2



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	6.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWWW-t	MT20	3.0	6.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	6.0	2.25	2.00
J	TMWW-t	MT20	3.0	4.0	1.50	1.50
K	TMVV-t	MT20	4.0	5.0	1.50	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BMWW-t	MT20	4.0	5.0	2.00	2.00
O	BMWW-t	MT20	3.0	4.0		
P	BMWWW-t	MT20	5.0	6.0	2.25	1.50
Q	BS-t	MT20	4.0	6.0		
R	BMW+w	MT20	2.0	4.0		
S	BMWWW-t	MT20	5.0	6.0	2.25	2.00
T	BMWW-t	MT20	3.0	4.0		
U	BMWW-t	MT20	4.0	5.0	2.00	2.00
V	BMV1+p	MT20	2.0	4.0		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)

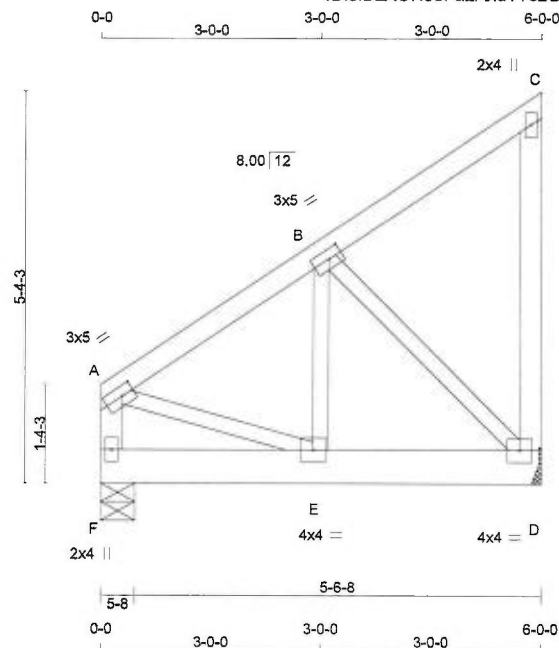
JSI METAL= 0.58 (Q) (INPUT = 1.00)

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 691.0 lbs FACTORED DOWN AT
10-11-4, AND 1010.6 lbs FACTORED DOWN AT
17-0-12 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:31.4

TOTAL WEIGHT = 33 lb
[M]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TMVW-t	MT20	3.0	5.0	1.50 2.00
B	TMVW-t	MT20	3.0	5.0	1.50 2.25
C	TMV+p	MT20	2.0	4.0	
D	BMVW1-t	MT20	4.0	4.0	
E	BMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
D	890	0	890	0	0	MECHANICAL	5-8	
F	890	0	890	0	0	5-8		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	626	434 / 0	0 / 0	0 / 0	192 / 0	0 / 0
F	626	434 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC2 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	-631 / 0	-77.4	-77.4	0.12 (1)	6.25	E-B	0 / 801	0.15 (1)
B-C	-14 / 0	-77.4	-77.4	0.12 (1)	6.25	B-D	-752 / 0	0.22 (1)
D-C	-90 / 0	0.0	0.0	0.04 (1)	7.81	A-E	0 / 562	0.14 (1)
F-A	-617 / 0	0.0	0.0	0.07 (1)	7.81			
F-E	0 / 0	-219.4	-219.4	0.13 (1)	10.00			
E-D	0 / 537	-219.4	-219.4	0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 10-10-8
END DISTANCE = 6-0-0
END SPAN CARRIED = 10-10-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12/1.00 (A-B:1), BC=0.20/1.00 (D-E:1), WB=0.22/1.00 (B-D:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



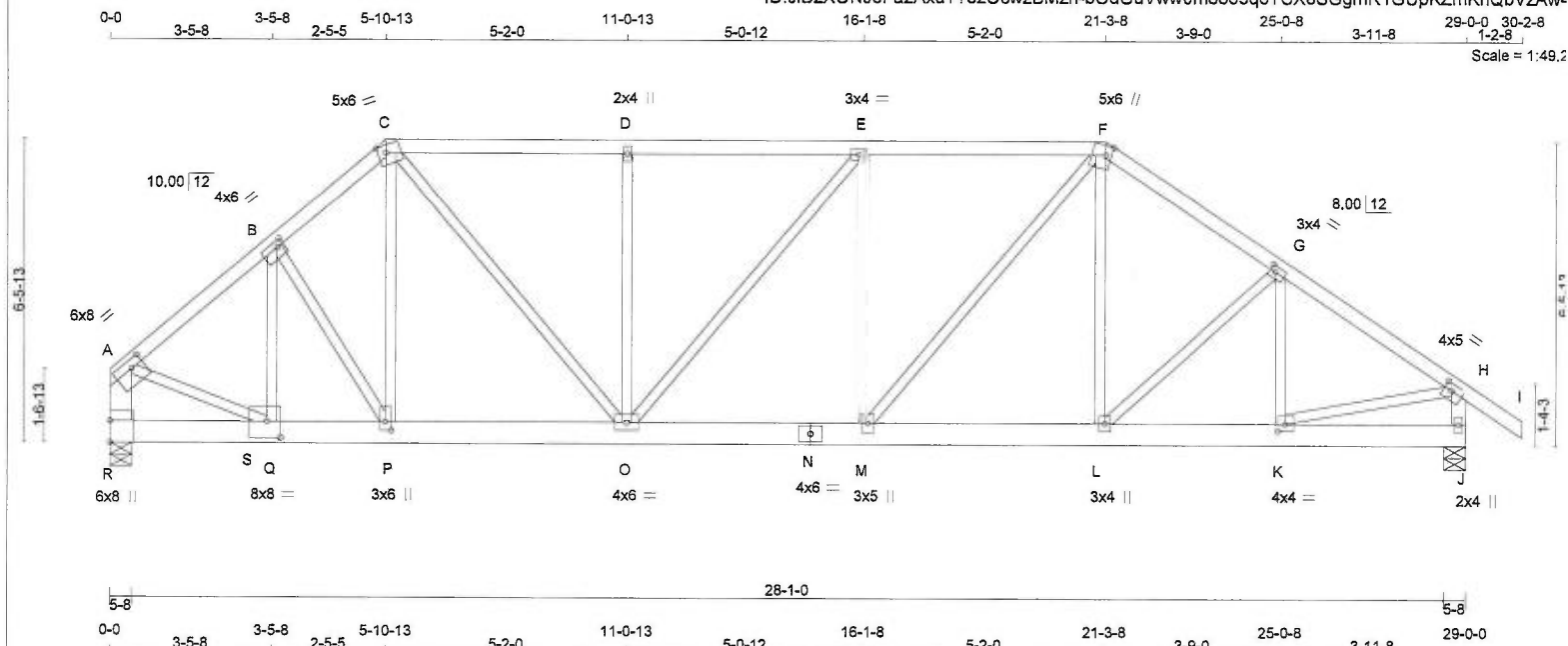
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-227	G02	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 1	PAGE 6 OF 45
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:08:13 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-7C4th9vIFTgxUvFwvU0vw38eLd?d4?yd5Bzs22zAw4r

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.21 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = $2 \times 150 = 299 \text{ lb}$

LUMBER		N. L. G. A. RULES		LUMBER		DESCR.	
CHORDS	SIZE						
A - C	2x4	DRY	No.2			SPF	
C - F	2x4	DRY	No.2			SPF	
F - I	2x4	DRY	No.2			SPF	
R - A	2x6	DRY	No.2			SPF	
J - H	2x4	DRY	No.2			SPF	
R - N	2x6	DRY	No.2			SPF	
N - J	2x6	DRY	No.2			SPF	

ALL WEBS 2x3 DRY
EXCEPT

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- I	1	12	TOP
J- H	1	12	TOP
R- A	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
R- N	2	12	SIDE(168.
N- J	2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	7134	0	7134	0	0	5-8	5-8
J	2187	0	2187	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	5012	3475 / 0	0 / 0	0 / 0	0 / 0	1537 / 0	0 / 0
J	1534	1077 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R. J.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	-7031 / 0	-77.4	-77.4 0.29 (1)	3.52	Q-B	0 / 3860	0.46 (1)		
B-C	-4832 / 0	-77.4	-77.4 0.13 (1)	4.30	B-P	-3189 / 0	0.60 (1)		
C-D	-3638 / 0	-77.4	-77.4 0.21 (1)	4.75	P-C	0 / 2967	0.37 (1)		
D-E	-3638 / 0	-77.4	-77.4 0.21 (1)	4.75	C-O	-170 / 0	0.11 (1)		
E-F	-3133 / 0	-77.4	-77.4 0.20 (1)	5.05	O-D	-415 / 0	0.14 (1)		
F-G	-2846 / 0	-77.4	-77.4 0.12 (1)	5.48	O-E	0 / 791	0.10 (1)		
G-H	-2523 / 0	-77.4	-77.4 0.12 (1)	5.58	M-E	-1034 / 0	0.34 (1)		
H-I	0 / 28	-77.4	-77.4 0.05 (1)	10.00	M-F	0 / 1470	0.18 (1)		
R-A	-6433 / 0	0.0	0.0 0.24 (1)	5.86	L-F	0 / 78	0.01 (4)		
J-H	-2134 / 0	0.0	0.0 0.12 (1)	7.60	L-G	0 / 94	0.01 (1)		
					K-G	-481 / 0	0.06 (1)		
R-S	0 / 0	-354.9	-354.9 0.25 (1)	10.00	A-Q	0 / 5691	0.70 (1)		
S-Q	0 / 0	-18.2	-18.2 0.25 (1)	10.00	K-H	0 / 2173	0.27 (1)		
Q-P	0 / 5400	-18.2	-18.2 0.48 (1)	10.00					
P-O	0 / 3748	-18.2	-18.2 0.28 (1)	10.00					
O-N	0 / 3133	-18.2	-18.2 0.22 (1)	10.00					
N-M	0 / 3133	-18.2	-18.2 0.22 (1)	10.00					
M-L	0 / 2183	-18.2	-18.2 0.16 (1)	10.00					
L-K	0 / 2113	-18.2	-18.2 0.16 (1)	10.00					
K-J	0 / 0	-18.2	-18.2 0.03 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	3-5-8	-5439	-5439	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0
START SPAN CARRIED = 16-6-8
END DISTANCE = 3-0-0
END SPAN CARRIED = 16-6-8
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***

ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIQ 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.29/1.00 (A-B:1), BC=0.48/1.00 (P-Q:1),
WB=0.70/1.00 (A-Q:1), SSI=0.23/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVVW-t	MT20	6.0	8.0	2.00	3.25
B	TMVVW-t	MT20	4.0	6.0	1.50	1.75
C	TTVVW-m	MT20	5.0	6.0	2.00	2.25
D	TMVVW-w	MT20	2.0	4.0		
E	TMVVW-t	MT20	3.0	4.0		
F	TTVVW+m	MT20	5.0	6.0	2.50	1.50
G	TMVVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVVW-t	MT20	4.0	5.0	1.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVVW-t	MT20	4.0	4.0	1.75	1.75
L	BMVVW-t	MT20	3.0	4.0		
M	BMVVW-t	MT20	3.0	5.0		
N	BS-t	MT20	4.0	6.0		
O	BMVVWW-t	MT20	4.0	6.0		
P	BMVVW-t	MT20	3.0	6.0	2.25	1.50
Q	BMVVW-t	MT20	8.0	8.0	4.25	3.50
R	BMV1+t	MT20	6.0	8.0	5.50	

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5438.6 lbs FACTORED DOWN AT 3-5-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

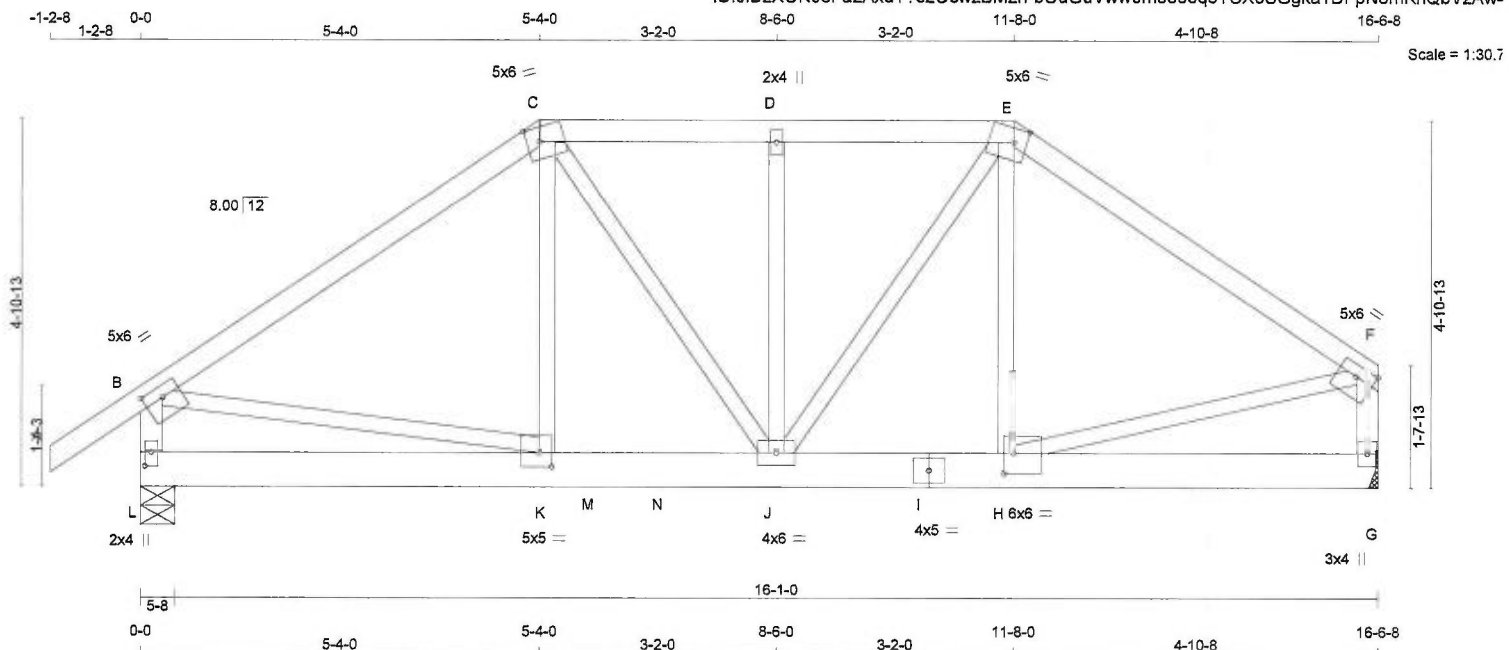
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)

JSI METAL= 0.67 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 81 = 163 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - F	2x4	DRY	No.2 SPF
L - B	2x4	DRY	No.2 SPF
G - F	2x4	DRY	No.2 SPF
L - I	2x6	DRY	No.2 SPF
I - G	2x6	DRY	No.2 SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
E - F	12	TOP
L - B	12	TOP
G - F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - I	9	SIDE (274.4)
I - G	9	SIDE (274.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	3995	0	3995	0	5-8	5-8
G	5439	0	5439	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	LIVE			
L	2804	1957 / 0	0 / 0	0 / 0	0 / 0	847 / 0	0 / 0
G	3821	2850 / 0	0 / 0	0 / 0	0 / 0	1172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX CSI (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A - B	0 / 28	-77.4	-77.4 0.05 (1)	10.00	K - C	0 / 1522	0.19 (1)
B - C	-5270 / 0	-77.4	-77.4 0.41 (1)	3.93	C - J	0 / 1309	0.16 (1)
C - D	-5167 / 0	-77.4	-77.4 0.15 (1)	4.16	J - D	-225 / 0	0.04 (1)
D - E	-5167 / 0	-77.4	-77.4 0.15 (1)	4.16	J - E	0 / 1268	0.16 (1)
E - F	-5298 / 0	-77.4	-77.4 0.34 (1)	3.96	H - E	0 / 1589	0.20 (1)
L - B	-3997 / 0	0.0	0.0 0.22 (1)	5.92	B - K	0 / 4445	0.55 (1)
G - F	-4240 / 0	0.0	0.0 0.24 (1)	5.77	H - F	0 / 4538	0.56 (1)
L - K	0 / 0	-18.2	-18.2 0.10 (1)	10.00			
K - M	0 / 4415	-18.2	-18.2 0.68 (1)	10.00			
M - N	0 / 4415	-18.2	-18.2 0.68 (1)	10.00			
N - J	0 / 4415	-567.0	-567.0 0.68 (1)	10.00			
J - I	0 / 4438	-567.0	-567.0 0.68 (1)	10.00			
I - H	0 / 4438	-567.0	-567.0 0.68 (1)	10.00			
H - G	0 / 0	-567.0	-567.0 0.47 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	6-0-12	-2516	-2516	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 7-0-0
START SPAN CARRIED = 25-5-0
END DISTANCE = 16-6-8
END SPAN CARRIED = 25-5-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.41/1.00 (B-C:1), BC=0.68/1.00 (J-K:1), WB=0.56/1.00 (F-H:1), SSI=0.72/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-t	MT20	5.0	6.0	1.75	Edge
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	6.0	6.0	3.25	1.50
I	BS-t	MT20	4.0	5.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	5.0	2.25	2.00
L	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 2516.5 lbs FACTORED DOWN AT 6-0-12
ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

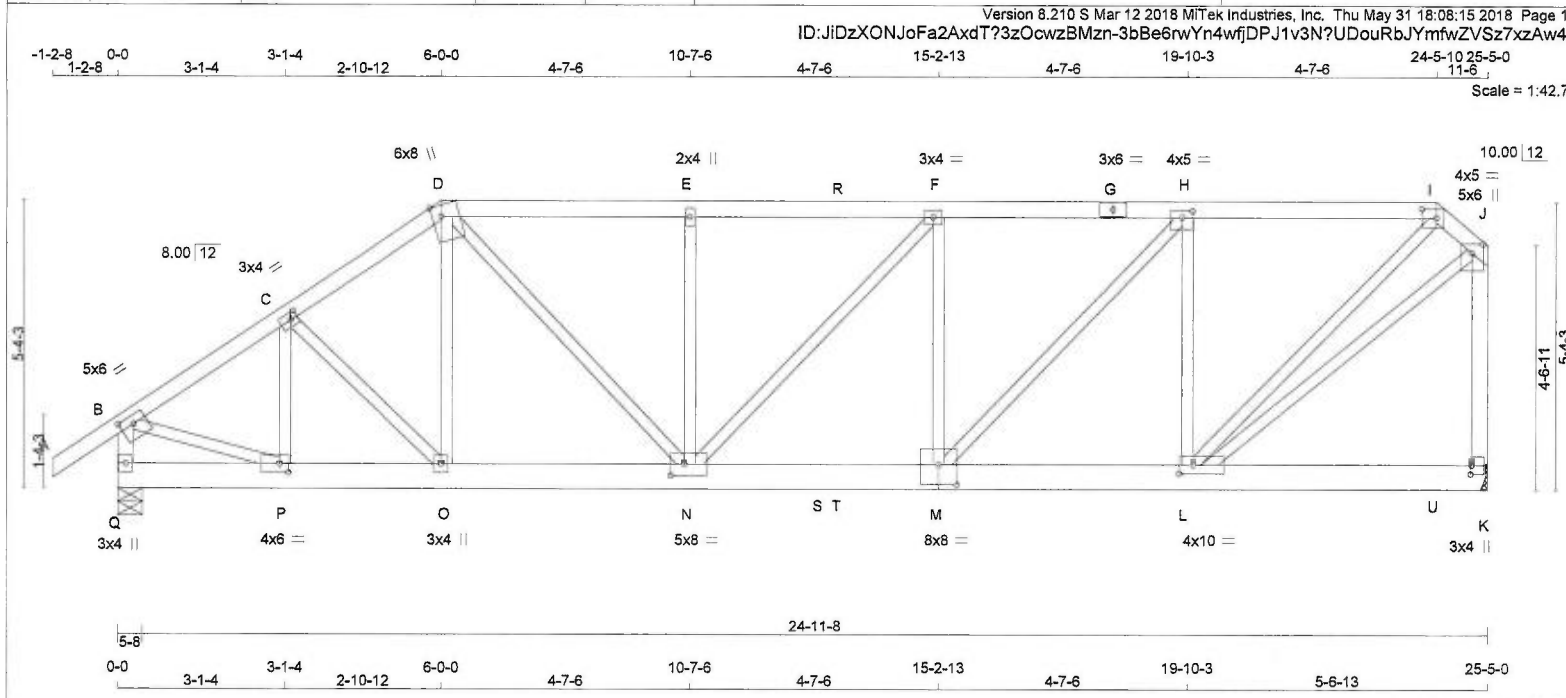
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.68 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:08:15 2018 Page 1
ID: JiDzXONJoFa2AxdT?3zOcwzBMzn-3bBe6nwYn4wfjDPJ1v3N?UDouRbJYmfwZVSz7xzAw4
Scale = 1:42.7

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF	FACTORED	GROSS REACTION	GROSS REACTION
D - G	2x4	DRY	No.2	SPF	JT	VERT	DOWN
G - I	2x4	DRY	No.2	SPF	Q	HORZ	HORZ
I - J	2x4	DRY	No.2	SPF	K	UPLIFT	UPLIFT
Q - B	2x4	DRY	No.2	SPF			
K - J	2x4	DRY	No.2	SPF			
Q - M	2x6	DRY	2100F 1.8E	SPF			
M - K	2x6	DRY	No.2	SPF			

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6'-0-0
RIGHT SETBACK = 11'-6
END SETBACK = 6'-0-0
END WALL WIDTH = 5'-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 12'-0-0 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.82/1.00 (J-K:1), BC=0.57/1.00 (M-N:1), WB=0.78/1.00 (H-L:1), SSI=0.41/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	24-5-10	-55	-55	-	FRONT	VERT	TOTAL
S	13-0-12	-1043	-1043	-	FRONT	VERT	TOTAL

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

FR-TO FROM TO LENGTH FR-TO

A-B 0/28 -77.4 -77.4 0.10 (1) 10.00 P-C -555/0 0.11 (1)

B-C -2231/0 -77.4 -77.4 0.18 (1) 4.39 C-O 0/227 0.06 (1)

C-D -2454/0 -77.4 -77.4 0.19 (1) 4.21 O-D -146/5 0.06 (1)

D-E -3201/0 -77.4 -77.4 0.44 (1) 3.50 D-N 0/1728 0.43 (1)

E-R -3201/0 -77.4 -77.4 0.49 (1) 3.44 N-E -399/0 0.16 (1)

R-F -3201/0 -145.9 -145.9 0.49 (1) 3.44 N-F -283/0 0.24 (1)

F-G -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-F -384/0 0.16 (1)

G-H -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-H 0/1543 0.38 (1)

H-I -2343/0 -145.9 -145.9 0.63 (1) 3.66 L-H -1895/0 0.78 (1)

I-J -1967/0 -77.4 -77.4 0.06 (1) 4.75 L-I 0/1246 0.31 (1)

Q-B -2009/0 0.0 0.0 0.22 (1) 5.91 B-P 0/1950 0.48 (1)

K-J -2426/0 0.0 0.0 0.82 (1) 5.43 L-J 0/1861 0.46 (1)

Q-P 0/0 -18.2 -18.2 0.03 (1) 10.00

P-O 0/1866 -18.2 -18.2 0.13 (1) 10.00

O-N 0/2023 -18.2 -18.2 0.23 (1) 10.00

N-S 0/3393 -18.2 -18.2 0.57 (1) 10.00

S-T 0/3393 -18.2 -18.2 0.57 (1) 10.00

T-M 0/3393 -34.4 -34.4 0.57 (1) 10.00

M-L 0/2343 -34.4 -34.4 0.51 (1) 10.00

L-U 0/0 -34.4 -34.4 0.12 (4) 10.00

U-K 0/0 -34.4 -34.4 0.12 (4) 10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	24-5-10	-55	-55	-	FRONT	VERT	TOTAL
S	13-0-12	-1043	-1043	-	FRONT	VERT	TOTAL

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

FR-TO FROM TO LENGTH FR-TO

A-B 0/28 -77.4 -77.4 0.10 (1) 10.00 P-C -555/0 0.11 (1)

B-C -2231/0 -77.4 -77.4 0.18 (1) 4.39 C-O 0/227 0.06 (1)

C-D -2454/0 -77.4 -77.4 0.19 (1) 4.21 O-D -146/5 0.06 (1)

D-E -3201/0 -77.4 -77.4 0.44 (1) 3.50 D-N 0/1728 0.43 (1)

E-R -3201/0 -77.4 -77.4 0.49 (1) 3.44 N-E -399/0 0.16 (1)

R-F -3201/0 -145.9 -145.9 0.49 (1) 3.44 N-F -283/0 0.24 (1)

F-G -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-F -384/0 0.16 (1)

G-H -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-H 0/1543 0.38 (1)

H-I -2343/0 -145.9 -145.9 0.63 (1) 3.66 L-H -1895/0 0.78 (1)

I-J -1967/0 -77.4 -77.4 0.06 (1) 4.75 L-I 0/1246 0.31 (1)

Q-B -2009/0 0.0 0.0 0.22 (1) 5.91 B-P 0/1950 0.48 (1)

K-J -2426/0 0.0 0.0 0.82 (1) 5.43 L-J 0/1861 0.46 (1)

Q-P 0/0 -18.2 -18.2 0.03 (1) 10.00

P-O 0/1866 -18.2 -18.2 0.13 (1) 10.00

O-N 0/2023 -18.2 -18.2 0.23 (1) 10.00

N-S 0/3393 -18.2 -18.2 0.57 (1) 10.00

S-T 0/3393 -18.2 -18.2 0.57 (1) 10.00

T-M 0/3393 -34.4 -34.4 0.57 (1) 10.00

M-L 0/2343 -34.4 -34.4 0.51 (1) 10.00

L-U 0/0 -34.4 -34.4 0.12 (4) 10.00

U-K 0/0 -34.4 -34.4 0.12 (4) 10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	24-5-10	-55	-55	-	FRONT	VERT	TOTAL
S	13-0-12	-1043	-1043	-	FRONT	VERT	TOTAL

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

FR-TO FROM TO LENGTH FR-TO

A-B 0/28 -77.4 -77.4 0.10 (1) 10.00 P-C -555/0 0.11 (1)

B-C -2231/0 -77.4 -77.4 0.18 (1) 4.39 C-O 0/227 0.06 (1)

C-D -2454/0 -77.4 -77.4 0.19 (1) 4.21 O-D -146/5 0.06 (1)

D-E -3201/0 -77.4 -77.4 0.44 (1) 3.50 D-N 0/1728 0.43 (1)

E-R -3201/0 -77.4 -77.4 0.49 (1) 3.44 N-E -399/0 0.16 (1)

R-F -3201/0 -145.9 -145.9 0.49 (1) 3.44 N-F -283/0 0.24 (1)

F-G -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-F -384/0 0.16 (1)

G-H -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-H 0/1543 0.38 (1)

H-I -2343/0 -145.9 -145.9 0.63 (1) 3.66 L-H -1895/0 0.78 (1)

I-J -1967/0 -77.4 -77.4 0.06 (1) 4.75 L-I 0/1246 0.31 (1)

Q-B -2009/0 0.0 0.0 0.22 (1) 5.91 B-P 0/1950 0.48 (1)

K-J -2426/0 0.0 0.0 0.82 (1) 5.43 L-J 0/1861 0.46 (1)

Q-P 0/0 -18.2 -18.2 0.03 (1) 10.00

P-O 0/1866 -18.2 -18.2 0.13 (1) 10.00

O-N 0/2023 -18.2 -18.2 0.23 (1) 10.00

N-S 0/3393 -18.2 -18.2 0.57 (1) 10.00

S-T 0/3393 -18.2 -18.2 0.57 (1) 10.00

T-M 0/3393 -34.4 -34.4 0.57 (1) 10.00

M-L 0/2343 -34.4 -34.4 0.51 (1) 10.00

L-U 0/0 -34.4 -34.4 0.12 (4) 10.00

U-K 0/0 -34.4 -34.4 0.12 (4) 10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	24-5-10	-55	-55	-	FRONT	VERT	TOTAL
S	13-0-12	-1043	-1043	-	FRONT	VERT	TOTAL

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

FR-TO FROM TO LENGTH FR-TO

A-B 0/28 -77.4 -77.4 0.10 (1) 10.00 P-C -555/0 0.11 (1)

B-C -2231/0 -77.4 -77.4 0.18 (1) 4.39 C-O 0/227 0.06 (1)

C-D -2454/0 -77.4 -77.4 0.19 (1) 4.21 O-D -146/5 0.06 (1)

D-E -3201/0 -77.4 -77.4 0.44 (1) 3.50 D-N 0/1728 0.43 (1)

E-R -3201/0 -77.4 -77.4 0.49 (1) 3.44 N-E -399/0 0.16 (1)

R-F -3201/0 -145.9 -145.9 0.49 (1) 3.44 N-F -283/0 0.24 (1)

F-G -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-F -384/0 0.16 (1)

G-H -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-H 0/1543 0.38 (1)

H-I -2343/0 -145.9 -145.9 0.63 (1) 3.66 L-H -1895/0 0.78 (1)

I-J -1967/0 -77.4 -77.4 0.06 (1) 4.75 L-I 0/1246 0.31 (1)

Q-B -2009/0 0.0 0.0 0.22 (1) 5.91 B-P 0/1950 0.48 (1)

K-J -2426/0 0.0 0.0 0.82 (1) 5.43 L-J 0/1861 0.46 (1)

Q-P 0/0 -18.2 -18.2 0.03 (1) 10.00

P-O 0/1866 -18.2 -18.2 0.13 (1) 10.00

O-N 0/2023 -18.2 -18.2 0.23 (1) 10.00

N-S 0/3393 -18.2 -18.2 0.57 (1) 10.00

S-T 0/3393 -18.2 -18.2 0.57 (1) 10.00

T-M 0/3393 -34.4 -34.4 0.57 (1) 10.00

M-L 0/2343 -34.4 -34.4 0.51 (1) 10.00

L-U 0/0 -34.4 -34.4 0.12 (4) 10.00

U-K 0/0 -34.4 -34.4 0.12 (4) 10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	24-5-10	-55	-55	-	FRONT	VERT	TOTAL
S	13-0-12	-1043	-1043	-	FRONT	VERT	TOTAL

CHORDS
MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS)

FR-TO FROM TO LENGTH FR-TO

A-B 0/28 -77.4 -77.4 0.10 (1) 10.00 P-C -555/0 0.11 (1)

B-C -2231/0 -77.4 -77.4 0.18 (1) 4.39 C-O 0/227 0.06 (1)

C-D -2454/0 -77.4 -77.4 0.19 (1) 4.21 O-D -146/5 0.06 (1)

D-E -3201/0 -77.4 -77.4 0.44 (1) 3.50 D-N 0/1728 0.43 (1)

E-R -3201/0 -77.4 -77.4 0.49 (1) 3.44 N-E -399/0 0.16 (1)

R-F -3201/0 -145.9 -145.9 0.49 (1) 3.44 N-F -283/0 0.24 (1)

F-G -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-F -384/0 0.16 (1)

G-H -3393/0 -145.9 -145.9 0.76 (1) 2.92 M-H 0/1543 0.38 (1)

H-I -2343/0 -145.9 -145.9 0.63 (1) 3.66 L-H -1895/0 0.78 (1)

I-J -1967/0 -77.4 -77.4 0.06 (1) 4.75 L-I 0/1246 0.31 (1)

Q-B -2009/0 0.0 0.0 0.22 (1) 5.91 B-P 0/1950 0.48 (1)

K-J -2426/0 0.0 0.0 0.82 (1) 5.43 L-J 0/1861 0.46 (1)

Q-P 0/0 -18.2 -18.2 0.03 (1) 10.00

P-O 0/1866 -18.2 -18.2 0.1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-227	G05	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 1	PAGE 12 OF 45
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:08:15 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-3bBe6nwYn4wfjDPJ1v3N?UDouRbJYmfwZVSz7xzAw4

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 55.0 lbs FACTORED DOWN AT 24-5-10 ON TOP CHORD, AND 1043.3 lbs FACTORED DOWN AT 13-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

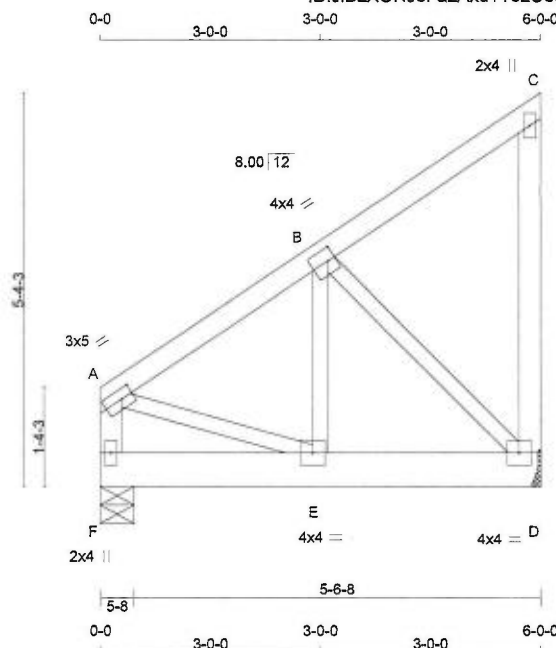
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
 JSI METAL= 0.59 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:31.4

TOTAL WEIGHT = 33 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVVV-t	MT20	3.0	5.0	1.50	2.00
B	TMVVV-t	MT20	4.0	4.0	2.00	1.25
C	TMVV+p	MT20	2.0	4.0		
D	BMVVV-t	MT20	4.0	4.0		
E	BMVVV-t	MT20	4.0	4.0		
F	BMVV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
D	1010	0	1010	0	MECHANICAL
F	1010	0	1010	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
D	710	492 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0
F	710	492 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-717 / 0	-77.4 -77.4	0.12 (1)	6.25	E-B	0 / 720	0.18 (1)
B-C	-14 / 0	-77.4 -77.4	0.11 (1)	6.25	B-D	-852 / 0	0.24 (1)
D-C	-91 / 0	0.0 0.0	0.04 (1)	7.81	A-E	0 / 637	0.16 (1)
F-A	-687 / 0	0.0 0.0	0.08 (1)	7.81			
F-E	0 / 0	-259.3 -259.3	0.15 (1)	10.00			
E-D	0 / 608	-259.3 -259.3	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 12-6-8
END DISTANCE = 6-0-0
END SPAN CARRIED = 12-6-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.23/1.00 (D-E:1), WB=0.24/1.00 (B-D:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



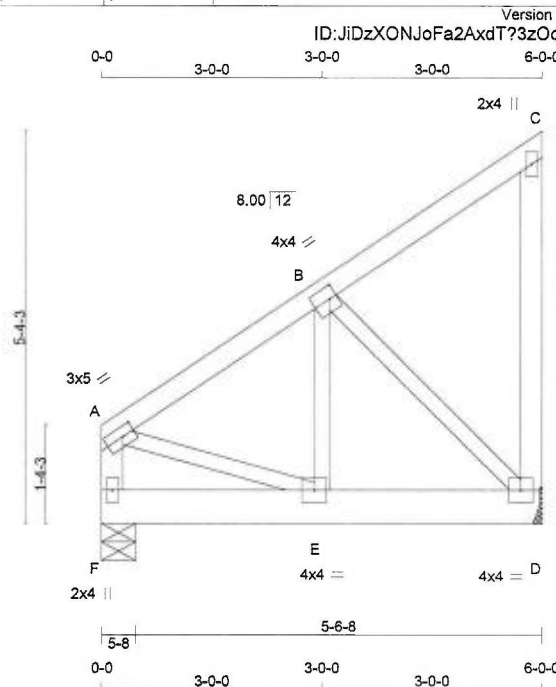
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-227	G06	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 1	PAGE 14 OF 45
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:08:16 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-XnI0JBxAYO2WLNzVacacXhm8bq0rHMF3n9BXfNzAw4

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:31.4

TOTAL WEIGHT = 33 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - A	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	1043	0	1043	0	MECHANICAL	BRG
F	1043	0	1043	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	733	508 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
F	733	508 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	-741 / 0	-77.4 -77.4	0.12 (1)	6.25	E-B	0 / 752	0.19 (1)		
B-C	-14 / 0	-77.4 -77.4	0.11 (1)	6.25	B-D	-880 / 0	0.25 (1)		
D-C	-91 / 0	0.0 0.0	0.04 (1)	7.81	A-E	0 / 658	0.16 (1)		
F-A	-707 / 0	0.0 0.0	0.08 (1)	7.81					
F-E	0 / 0	-270.2 -270.2	0.15 (1)	10.00					
E-D	0 / 628	-270.2 -270.2	0.24 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-0-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 13-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.24/1.00 (D-E:1), WB=0.25/1.00 (B-D:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



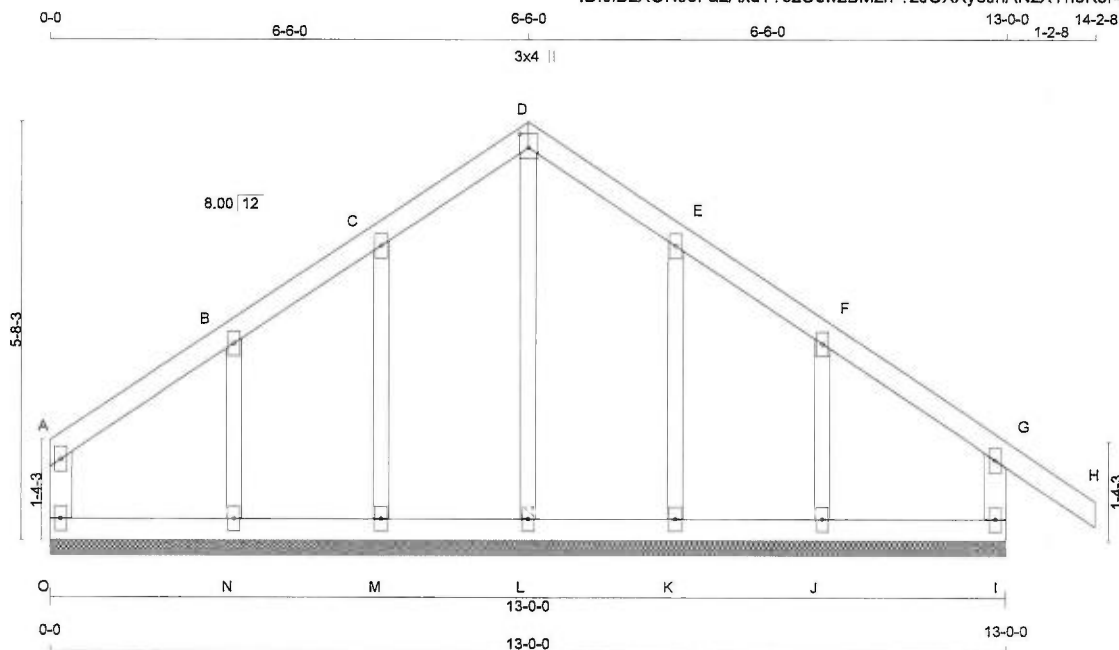
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0518-227	G07	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - AMELIA 12 ELE 1	PAGE 16 OF 45
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:08:17 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-?zJOXXyoJhANzXYh8K5r4viJMEMx0pNC0px4BqzAw4

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.24 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:31.2

TOTAL WEIGHT = 51 lb
[M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	4.0	2.25	1.50
G	TMV+p	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
O-A	-113 / 0	0.0	0.0 0.04 (1)	L-D	-131 / 0	0.06 (1)	0.06 (1)
A-B	-39 / 0	-77.4	-77.4 0.05 (1)	M-C	-148 / 0	0.04 (1)	0.04 (1)
B-C	-37 / 0	-77.4	-77.4 0.05 (1)	N-B	-176 / 0	0.03 (1)	0.03 (1)
C-D	-30 / 0	-77.4	-77.4 0.04 (1)	K-E	-151 / 0	0.04 (1)	0.04 (1)
D-E	-30 / 0	-77.4	-77.4 0.04 (1)	J-F	-166 / 0	0.03 (1)	0.03 (1)
E-F	-36 / 0	-77.4	-77.4 0.04 (1)				
F-G	-43 / 0	-77.4	-77.4 0.08 (1)				
G-H	0 / 28	-77.4	-77.4 0.09 (1)				
I-G	-220 / 0	0.0	0.0 0.03 (1)				
O-N	0 / 34	-18.2	-18.2 0.02 (4)				
N-M	0 / 29	-18.2	-18.2 0.02 (4)				
M-L	0 / 26	-18.2	-18.2 0.02 (4)				
L-K	0 / 26	-18.2	-18.2 0.02 (4)				
K-J	0 / 29	-18.2	-18.2 0.02 (4)				
J-I	0 / 34	-18.2	-18.2 0.03 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (G-H:1), BC=0.03/1.00 (I-J:1), WB=0.06/1.00 (D-L:1), SSI=0.06/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

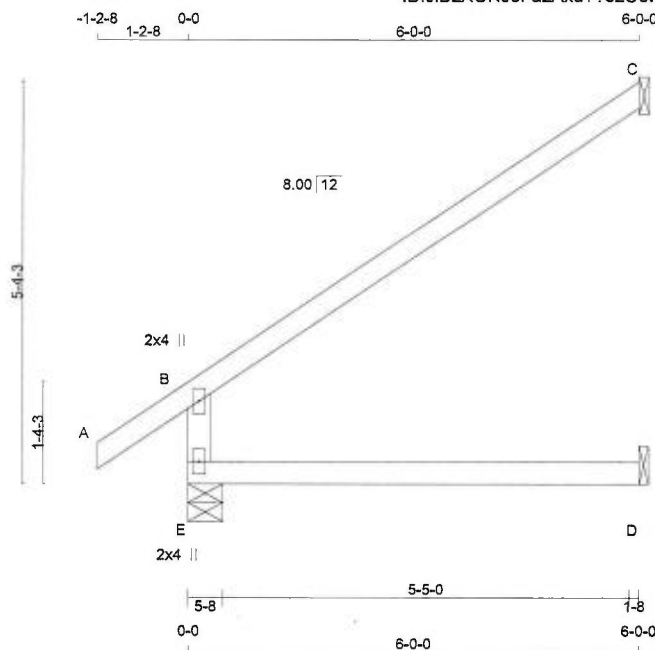
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (A) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 9 X 18 = 162 lb
[M] [F]

LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	454	0	454	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	46	0	51	0	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	316	235 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	(4)	FR-TO		
E-B	-390 / 0	0.0	0.0	0.11 (4)	7.81		
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00		
B-C	-32 / 0	-77.4	-77.4	0.36 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

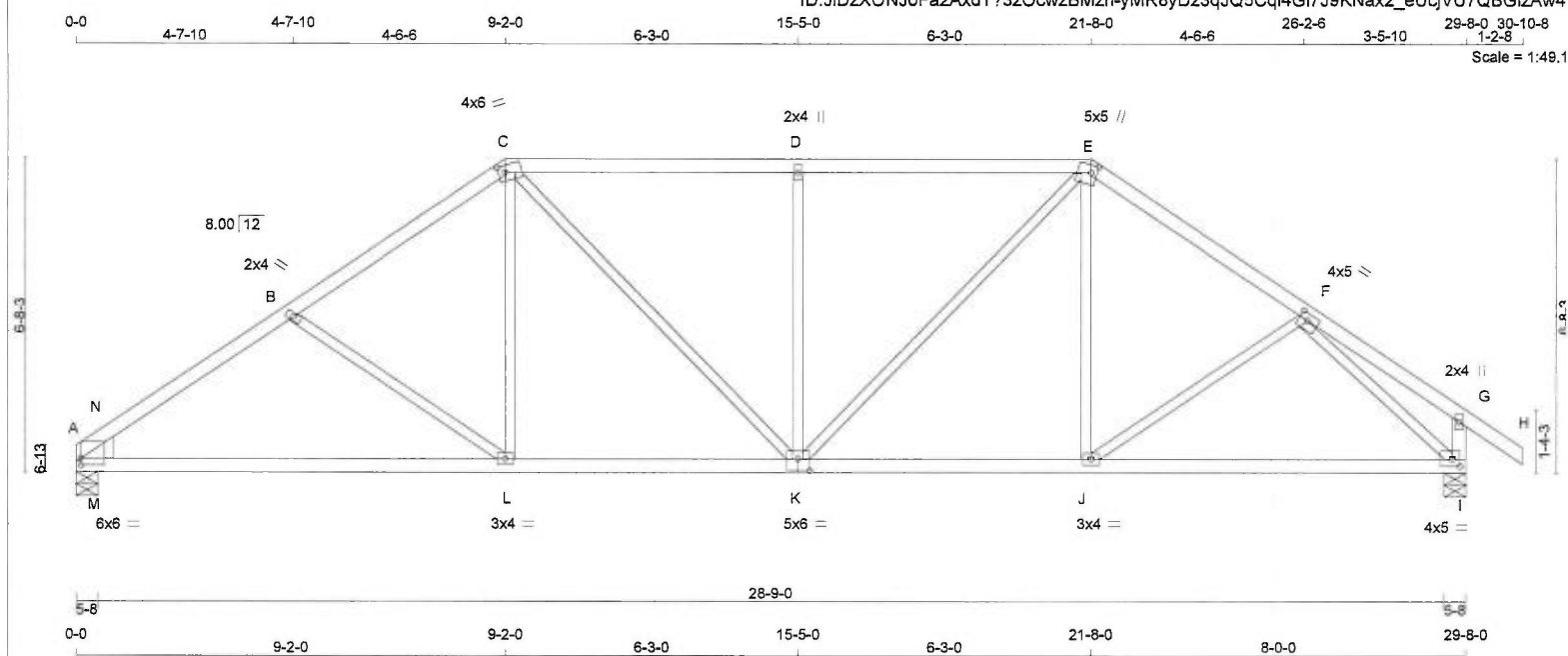
JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Thu May 31 18:08:19 2018 Page 1
 ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-yMR8yDz3qJQ5Cq4GI7J9KNax2_eUcjVU7QBGizAw4



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
A - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	6.0	6.0	1.75	
B	TMW+w	MT20	2.0	4.0		
C	TTWW+m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	5.0	2.00	1.50
F	TMW+t	MT20	4.0	5.0	1.75	2.25
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	5.0	1.75	2.00
J	BMW+t	MT20	3.0	4.0		
K	BSWWW-I	MT20	5.0	6.0	3.00	3.00
L	BMW+t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
JT VERT	DOWN	UPLIFT	IN-SX	
A 1418 0	1418 0	0 0	5-8	5-8
I 1518 0	1518 0	0 0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW
A 996	691 / 0
I 1064	751 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-N	-2429 / 0	-77.4 -77.4	0.22 (1)	4.22	B-L	-324 / 0	0.19 (1)
N-B	-2028 / 0	-77.4 -77.4	0.22 (1)	4.57	L-C	0 / 322	0.07 (1)
B-C	-1721 / 0	-77.4 -77.4	0.24 (1)	4.86	C-K	0 / 404	0.09 (1)
C-D	-1708 / 0	-77.4 -77.4	0.44 (1)	4.62	K-D	-592 / 0	0.44 (1)
D-E	-1708 / 0	-77.4 -77.4	0.44 (1)	4.62	K-E	0 / 554	0.12 (1)
E-F	-1601 / 0	-77.4 -77.4	0.23 (1)	5.01	J-E	0 / 151	0.05 (4)
F-G	0 / 24	-77.4 -77.4	0.20 (1)	10.00	J-F	0 / 62	0.02 (4)
G-H	0 / 28	-77.4 -77.4	0.09 (1)	10.00	F-I	-1823 / 0	0.65 (1)
I-G	-187 / 0	0.0	0.02 (1)	7.81	M-N	0 / 460	0.00 (1)
A-M	0 / 1684	-18.2 -18.2	0.48 (1)	10.00			
M-L	0 / 1684	-18.2 -18.2	0.48 (1)	10.00			
L-K	0 / 1423	-18.2 -18.2	0.35 (4)	10.00			
K-J	0 / 1318	-18.2 -18.2	0.37 (4)	10.00			
J-I	0 / 1308	-18.2 -18.2	0.36 (1)	10.00			

TOTAL WEIGHT = 117 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
 ALLOWABLE DEFL.(TL)= L/360 (0.99")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.48/1.00 (A-M:1) , WB=0.65/1.00 (F-I:1) , SSI=0.33/1.00 (A-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

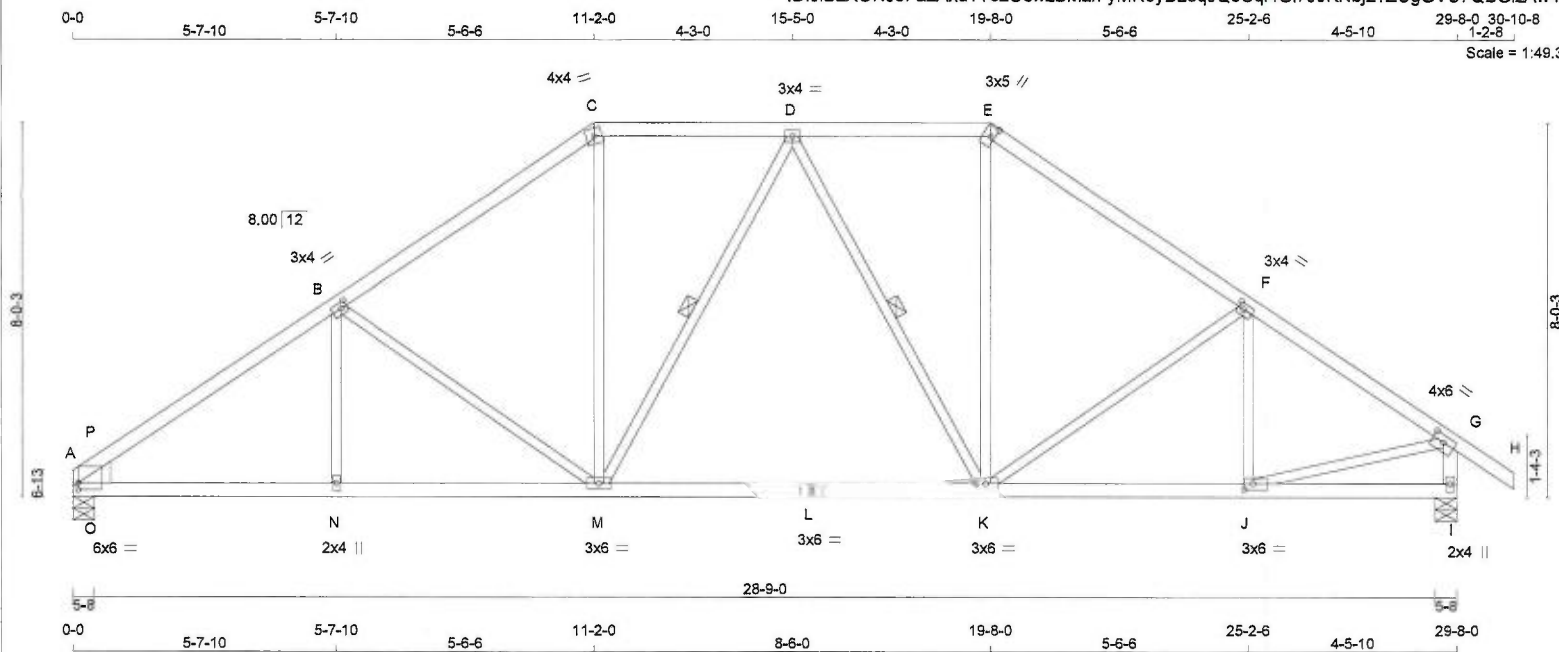
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
 JSI METAL= 0.54 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 123 lb
(M)F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
I - G	2x4	DRY	No.2
A - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	6.0	6.0	1.75	
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	3.0	4.0		
E	TTW-h	MT20	3.0	5.0	2.50	1.00
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMVV-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	3.0	6.0	1.50	2.25
K	BMWW-t	MT20	3.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	3.0	6.0		
N	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	HEEL
	VERT	HORZ	DOWN	HORZ			
A	1418	0	1418	0	5-8	5-8	2x6 L
I	1518	0	1518	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	996	691 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0
I	1064	751 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, D-K. DBS = 20-0-0. CBF = 36 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-P	-2339 / 0	-77.4	-77.4	0.25 (1)	4.25	N-B	0 / 92	0.03 (4)	
P-B	-2002 / 0	-77.4	-77.4	0.30 (1)	4.52	B-M	-465 / 0	0.42 (1)	
B-C	-1573 / 0	-77.4	-77.4	0.32 (1)	4.94	M-C	0 / 556	0.13 (1)	
C-D	-1296 / 0	-77.4	-77.4	0.19 (1)	5.47	M-D	-160 / 0	0.08 (1)	
D-E	-1235 / 0	-77.4	-77.4	0.19 (1)	5.58	D-K	-289 / 0	0.15 (1)	
E-F	-1502 / 0	-77.4	-77.4	0.32 (1)	5.04	K-E	0 / 520	0.12 (1)	
F-G	-1663 / 0	-77.4	-77.4	0.29 (1)	4.86	K-F	-220 / 0	0.20 (1)	
G-H	0 / 28	-77.4	-77.4	0.09 (1)	10.00	J-F	-238 / 4	0.07 (1)	
I-G	-1482 / 0	0.0	0.0	0.15 (1)	6.73	J-G	0 / 1444	0.33 (1)	
						O-P	0 / 339	0.00 (1)	
A-O	0 / 1668	-18.2	-18.2	0.42 (1)	10.00				
O-N	0 / 1668	-18.2	-18.2	0.42 (1)	10.00				
N-M	0 / 1668	-18.2	-18.2	0.40 (1)	10.00				
M-L	0 / 1372	-18.2	-18.2	0.35 (1)	10.00				
L-K	0 / 1372	-18.2	-18.2	0.35 (1)	10.00				
K-J	0 / 1408	-18.2	-18.2	0.34 (1)	10.00				
J-I	0 / 0	-18.2	-18.2	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.32/1.00 (B-C:1), BC=0.42/1.00 (A-O:1), WB=0.42/1.00 (B-M:1), SSI=0.23/1.00 (A-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

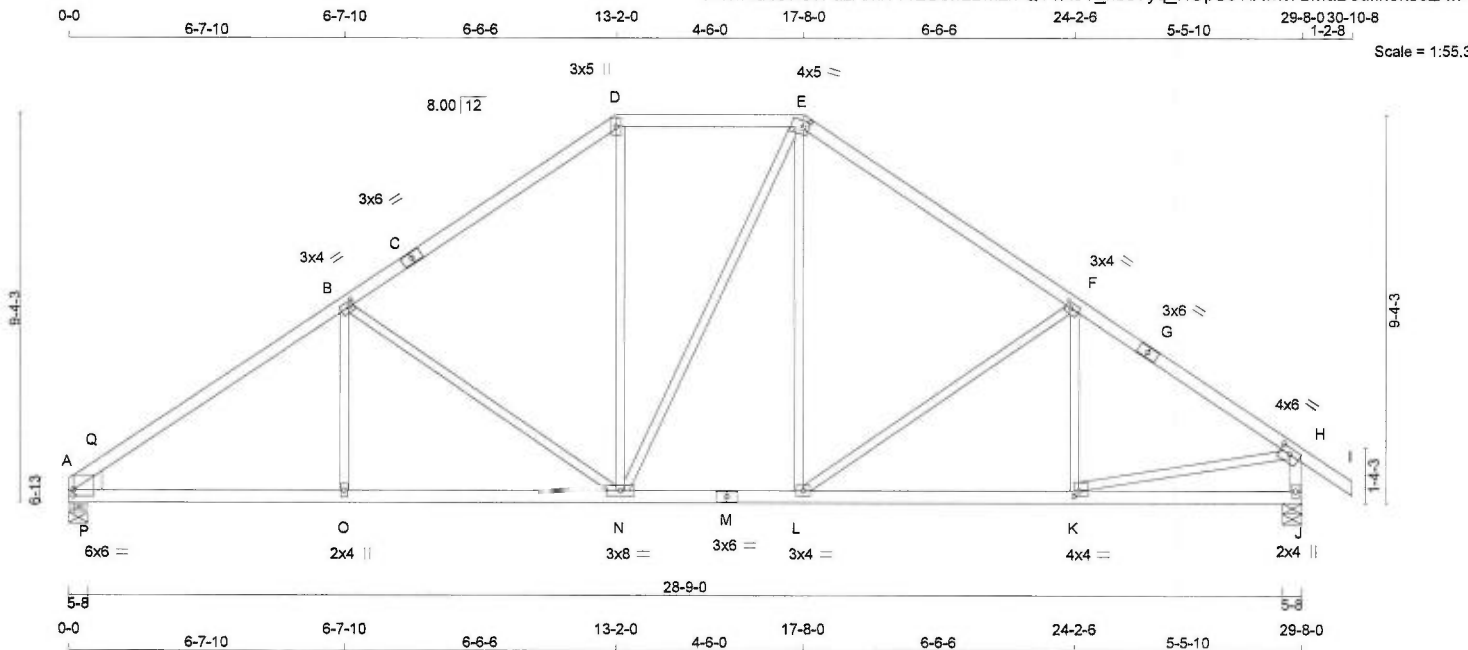
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.49 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 124 lb [M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
A - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	6.0	6.0	1.75	
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TS-t	MT20	3.0	6.0		
D	TTW+p	MT20	3.0	5.0		
E	TTWW-m	MT20	4.0	5.0	1.75	1.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMWW-t	MT20	3.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	3.0	8.0		
O	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
JT VERT	DOWN	HORZ	UPLIFT	IN-SX
A 1418 0	1418 0	0	5-8	5-8
J 1518 0	1518 0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE	
A 996	691 / 0	0 / 0	0 / 0	306 / 0
J 1064	751 / 0	0 / 0	0 / 0	313 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
A-Q	-2260 / 0	-77.4 -77.4	0.27 (1)	4.30	O-B	0 / 136	0.05 (4)
Q-B	-1964 / 0	-77.4 -77.4	0.45 (1)	4.40	B-N	-603 / 0	0.84 (1)
B-C	-1411 / 0	-77.4 -77.4	0.45 (1)	4.98	N-D	0 / 393	0.09 (1)
C-D	-1411 / 0	-77.4 -77.4	0.45 (1)	4.98	D-E	0 / 65	0.01 (1)
D-E	-1155 / 0	-77.4 -77.4	0.22 (1)	5.68	E-F	0 / 319	0.07 (1)
E-F	-1380 / 0	-77.4 -77.4	0.45 (1)	5.02	F-G	-388 / 0	0.54 (1)
F-G	-1696 / 0	-77.4 -77.4	0.42 (1)	4.66	G-H	-147 / 61	0.05 (1)
G-H	-1696 / 0	-77.4 -77.4	0.42 (1)	4.66	H-I	0 / 1465	0.33 (1)
H-I	0 / 28	-77.4 -77.4	0.09 (1)	10.00	I-J	0 / 249	0.00 (1)
I-J	-1479 / 0	0.0 0.0	0.15 (1)	6.74			

A-P	0 / 1643	-18.2 -18.2	0.37 (1)	10.00
P-O	0 / 1643	-18.2 -18.2	0.37 (1)	10.00
O-N	0 / 1643	-18.2 -18.2	0.34 (1)	10.00
N-M	0 / 1126	-18.2 -18.2	0.25 (1)	10.00
M-L	0 / 1126	-18.2 -18.2	0.25 (1)	10.00
L-K	0 / 1440	-18.2 -18.2	0.31 (1)	10.00
K-J	0 / 0	-18.2 -18.2	0.16 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.45/1.00 (E-F:1), BC=0.37/1.00 (A-P:1), WB=0.84/1.00 (B-N:1), SSI=0.25/1.00 (A-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

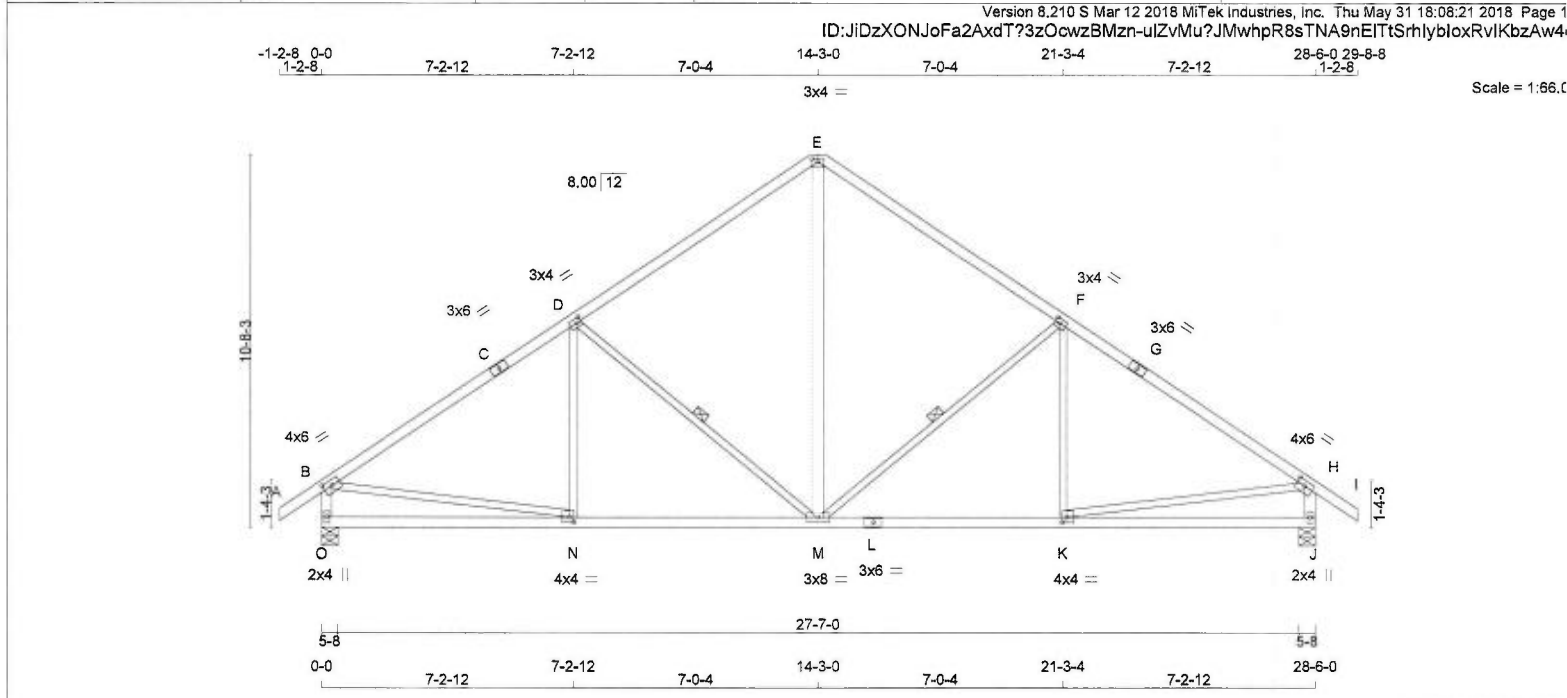
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.54 (K) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

1ST LCASE

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

1025

724 / 0

0 / 0

0 / 0

0 / 0

301 / 0

0 / 0

1ST LCASE

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

1025

724 / 0

0 / 0

0 / 0

0 / 0

301 / 0

0 / 0

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

No.2

SPF

DRY: SEASONED LUMBER.

ALL WEBS

EXCEPT

M - E

2x3

DRY

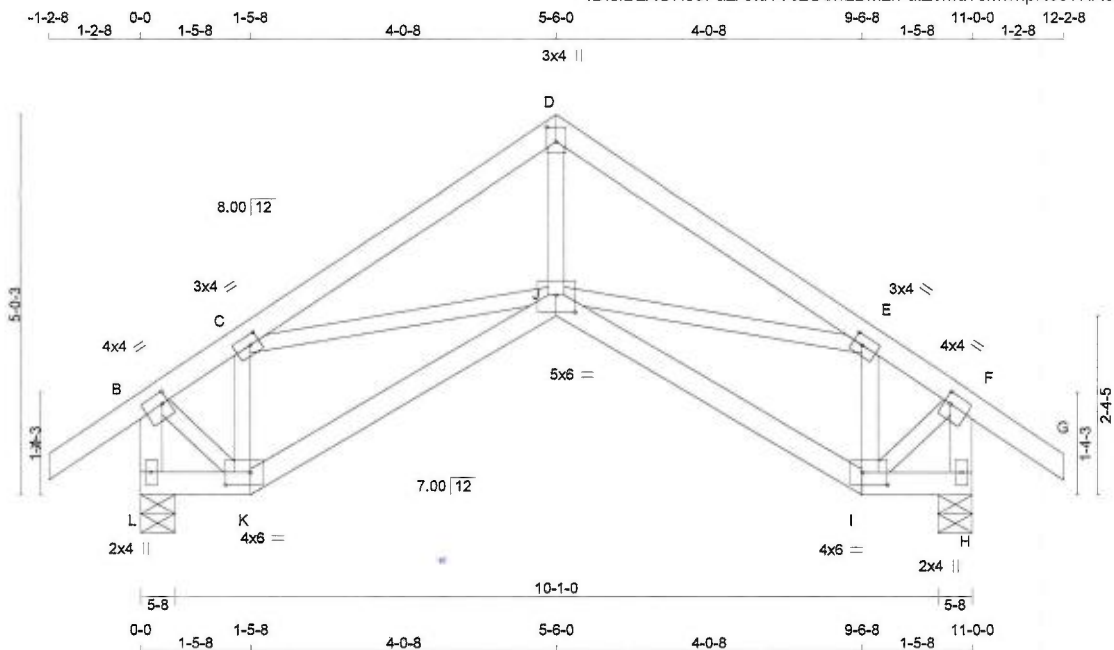
No.2

SPF

DRY: SEASONED LUMBER.

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:30.4

TOTAL WEIGHT = 2 X 48 = 97 lb

LUMBER

N. L. G. A. RULES		LUMBER		DESCR.
CHORDS	SIZE			
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	PLATES	W	L	Y	X
B	TMVW-t	MT20	4.0	4.0	1.75	1.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	4.0	4.0	1.75	1.00
H	BMV1+p	MT20	2.0	4.0		
I	BBVW-p	MT20	4.0	6.0	2.00	4.00
J	BBVWVW-p	MT20	5.0	6.0	2.75	3.00
K	BBVW-p	MT20	4.0	6.0	2.00	4.00
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	626	0	626	0	0	5-8	5-8
H	626	0	626	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	437	316 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
H	437	316 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	J-D	0 / 530	0.12 (1)	
B-C	-418 / 0	-77.4	-77.4	0.12 (1)	6.25	J-E	0 / 257	0.06 (1)	
C-D	-733 / 0	-77.4	-77.4	0.16 (1)	6.25	I-E	-404 / 0	0.06 (1)	
D-E	-733 / 0	-77.4	-77.4	0.16 (1)	6.25	C-J	0 / 257	0.06 (1)	
E-F	-418 / 0	-77.4	-77.4	0.12 (1)	6.25	K-C	-404 / 0	0.06 (1)	
F-G	0 / 28	-77.4	-77.4	0.09 (1)	10.00	B-K	0 / 443	0.10 (1)	
L-B	-612 / 0	0.0	0.0	0.06 (1)	7.81	I-F	0 / 443	0.10 (1)	
H-F	-612 / 0	0.0	0.0	0.06 (1)	7.81				
L-K	0 / 0	-18.2	-18.2	0.01 (4)	10.00				
K-J	0 / 396	-18.2	-18.2	0.12 (4)	10.00				
J-I	0 / 396	-18.2	-18.2	0.12 (4)	10.00				
I-H	0 / 0	-18.2	-18.2	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.12/1.00 (I-J:4) ,
WB=0.12/1.00 (D-J:1) , SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

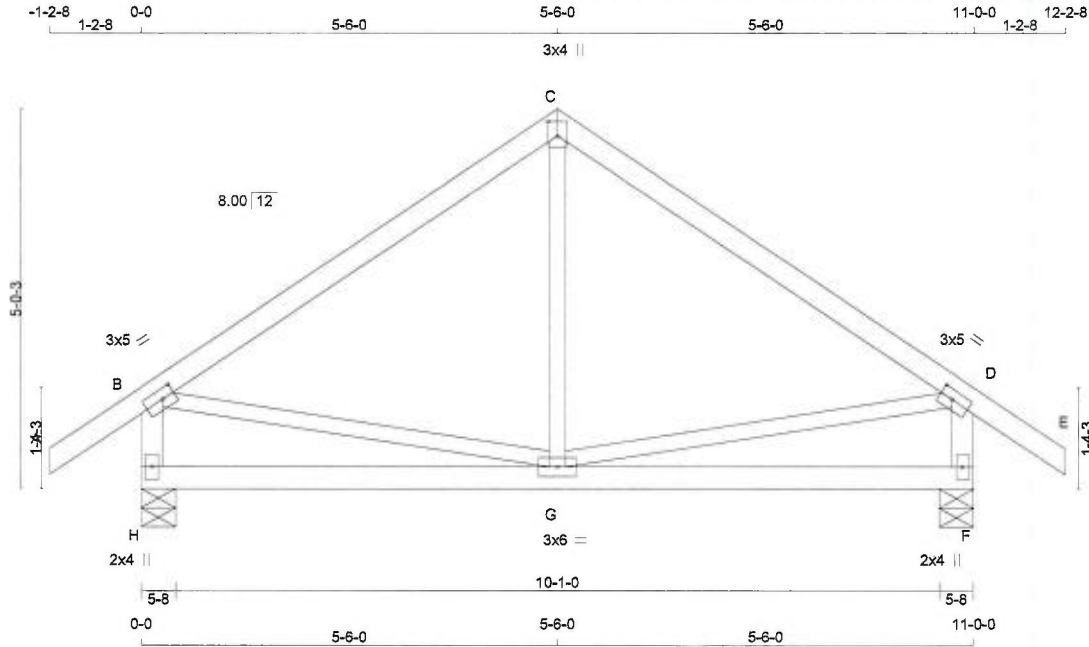
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (D) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





Scale = 1:30.4

TOTAL WEIGHT = 45 lb [M]F

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	
A - C 2x4 DRY No.2	SPF
C - E 2x4 DRY No.2	SPF
H - B 2x4 DRY No.2	SPF
F - D 2x4 DRY No.2	SPF
H - F 2x4 DRY No.2	SPF
ALL WEBS 2x3 DRY No.2	SPF
EXCEPT	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWV-t	MT20	3.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
H	626	0	0	5-8
F	626	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
H	437 316 / 0 0 / 0 0 / 0 121 / 0 0 / 0
F	437 316 / 0 0 / 0 0 / 0 121 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	G-C	0 / 99	0.03 (4)
B-C	-387 / 0	-77.4	-77.4 0.30 (1)	6.25	B-G	0 / 327	0.07 (1)
C-D	-387 / 0	-77.4	-77.4 0.30 (1)	6.25	G-D	0 / 327	0.07 (1)
D-E	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
H-B	-587 / 0	0.0	0.0 0.06 (1)	7.81			
F-D	-587 / 0	0.0	0.0 0.06 (1)	7.81			
H-G	0 / 0	-18.2	-18.2 0.16 (4)	10.00			
G-F	0 / 0	-18.2	-18.2 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.3 PSF	
TOTAL LOAD = 33.6 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.30/1.00 (B-C:1), BC=0.16/1.00 (G-H:4),
WB=0.07/1.00 (B-G:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	618 354 1667 822 2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

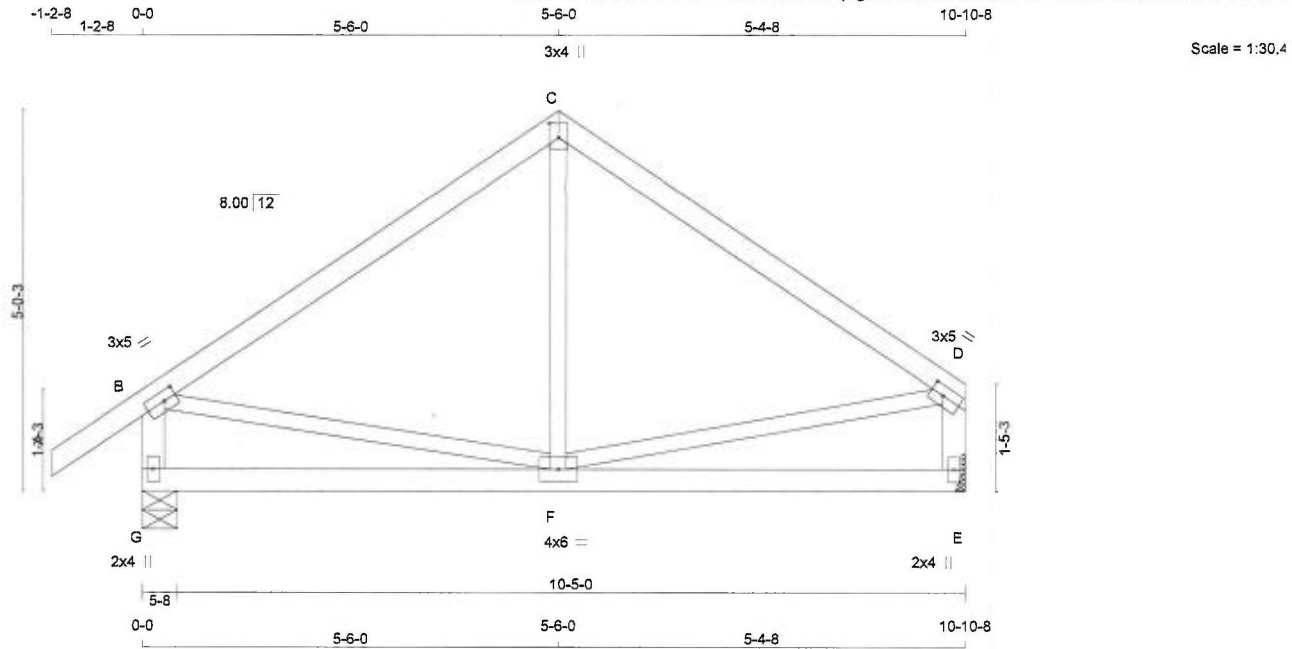
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 43 = 130 lb
[M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVV-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVVV-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
G	620	0	620	0	5-8	5-8		
E	520	0	520	0	0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
G	433	313 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0	
E	365	253 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO				
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	F-C	-1 / 96	0.03 (4)		
B-C	-378 / 0	-77.4	-77.4 0.30 (1)	6.25	B-F	0 / 320	0.07 (1)		
C-D	-378 / 0	-77.4	-77.4 0.29 (1)	6.25	F-D	0 / 321	0.07 (1)		
G-B	-581 / 0	0.0	0.0 0.06 (1)	7.81					
E-D	-482 / 0	0.0	0.0 0.05 (1)	7.81					
G-F	0 / 0	-18.2	-18.2 0.15 (4)	10.00					
F-E	0 / 0	-18.2	-18.2 0.15 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.30/1.00 (B-C:1), BC=0.15/1.00 (F-G:4),
WB=0.07/1.00 (D-F:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		822	2284
			1656

PLATE PLACEMENT TOL. = 0.250 inches

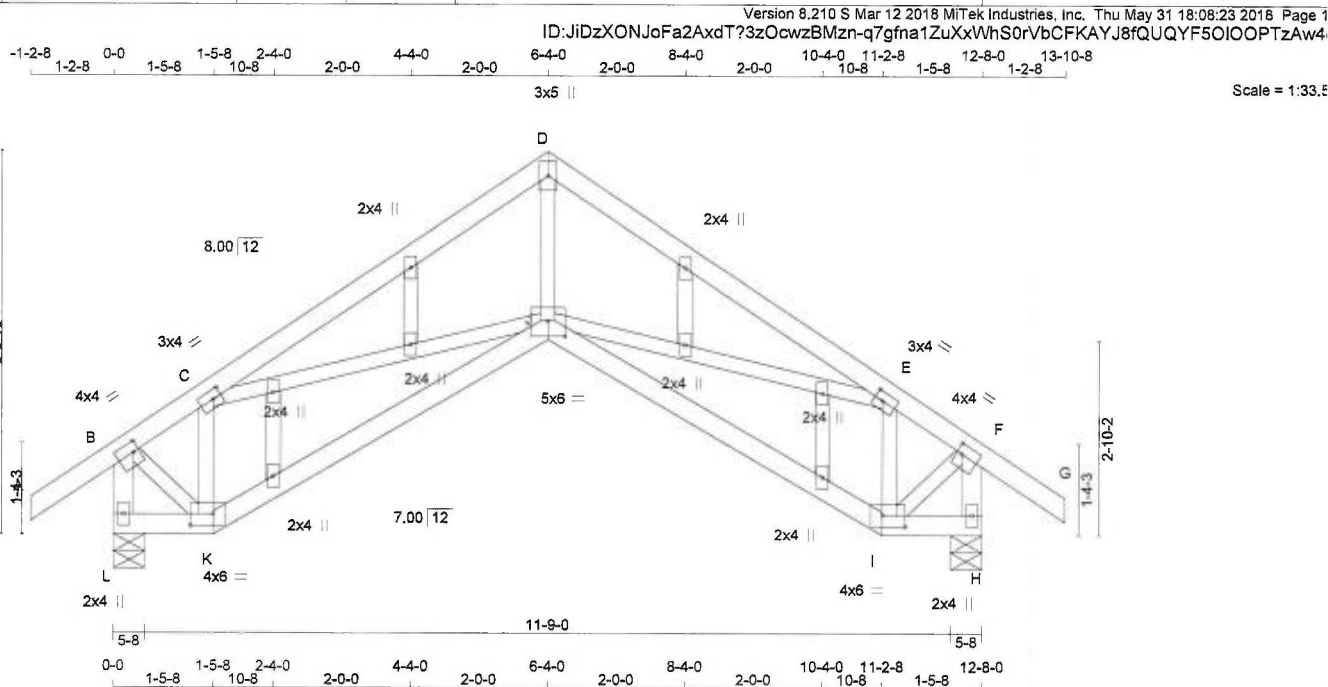
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 58 lb [M]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	4.0	4.0	1.75	1.00
C	TMVV-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	5.0		
E	TMVV-t	MT20	3.0	4.0	1.50	1.50
F	TMVV-t	MT20	4.0	4.0	1.75	1.00
H	BMV1+p	MT20	2.0	4.0		
I	BBVV-p	MT20	4.0	6.0	2.00	4.00
J	BBVV-p	MT20	5.0	6.0	2.75	3.00
K	BBVV-p	MT20	4.0	6.0	2.00	4.00
L	BMV1+p	MT20	2.0	4.0		
M, N, O, P, Q, R, S, T						
M	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	705	0	705	0	5-8	5-8
H	705	0	705	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
L	493	355 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	493	355 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	J-D	0 / 723	0.16 (1)	
B-C	-503 / 0	-77.4	-77.4	0.19 (1)	6.25	J-E	0 / 355	0.08 (1)	
C-D	-955 / 0	-77.4	-77.4	0.23 (1)	6.09	I-E	-512 / 0	0.08 (1)	
D-E	-955 / 0	-77.4	-77.4	0.23 (1)	6.09	C-J	0 / 355	0.08 (1)	
E-F	-503 / 0	-77.4	-77.4	0.19 (1)	6.25	K-C	-512 / 0	0.08 (1)	
F-G	0 / 28	-77.4	-77.4	0.09 (1)	10.00	B-K	0 / 556	0.13 (1)	
L-B	-692 / 0	0.0	0.0	0.07 (1)	7.81	I-F	0 / 556	0.13 (1)	
H-F	-692 / 0	0.0	0.0	0.07 (1)	7.81				
L-K	0 / 0	-18.2	-18.2	0.01 (4)	10.00				
K-J	0 / 497	-18.3	-18.3	0.16 (4)	10.00				
J-I	0 / 497	-18.3	-18.3	0.16 (4)	10.00				
I-H	0 / 0	-18.2	-18.2	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.23/1.00 (C-D:1), BC=0.16/1.00 (I-J:4), WB=0.16/1.00 (D-J:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

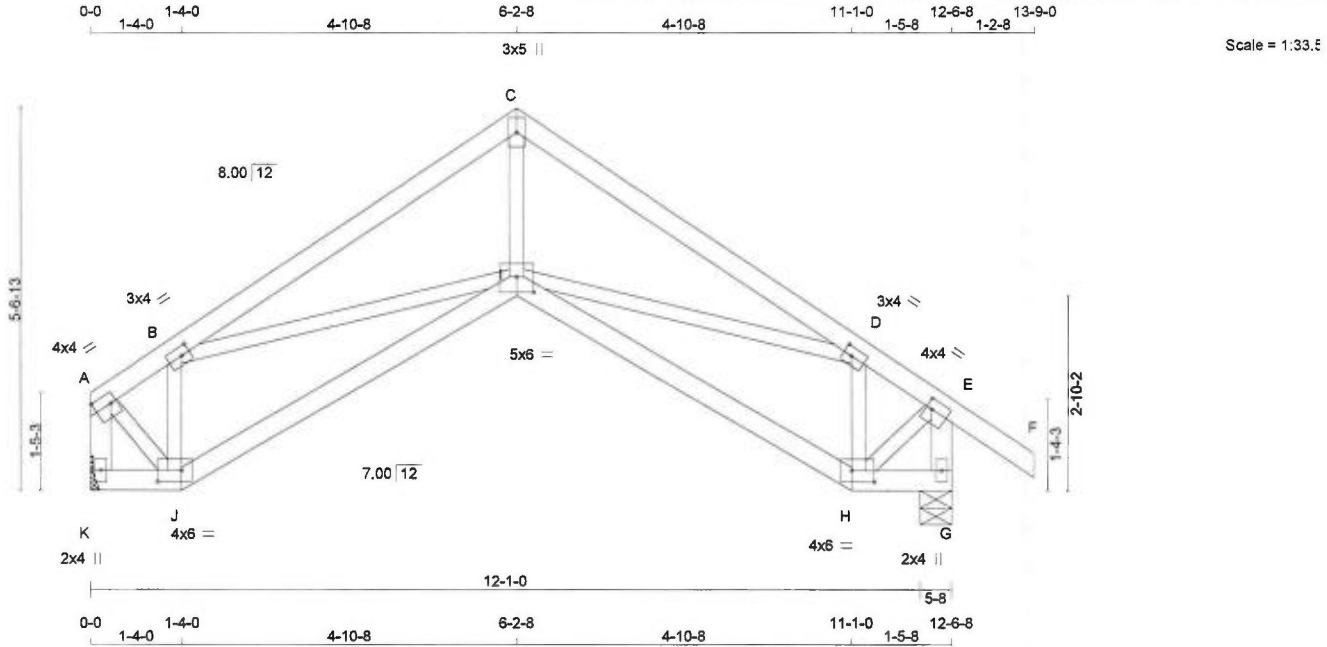
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 52 lb [M]F

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - C	2x4	DRY	No.2		
C - F	2x4	DRY	No.2		
K - A	2x4	DRY	No.2		
G - E	2x4	DRY	No.2		
K - J	2x4	DRY	No.2		
J - I	2x4	DRY	No.2		
I - H	2x4	DRY	No.2		
H - G	2x4	DRY	No.2		
ALL WEBS	2x3	DRY	No.2		
EXCEPT			SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-t	MT20	4.0	4.0	1.75	Edge
B	TMVV-t	MT20	3.0	4.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0		
D	TMVV-t	MT20	3.0	4.0	1.50	1.50
E	TMVV-t	MT20	4.0	4.0	1.75	1.00
G	BMV1+p	MT20	2.0	4.0		
H	BBVV-p	MT20	4.0	6.0	2.00	4.00
I	BBVV-p	MT20	5.0	6.0	2.75	3.00
J	BBVV-p	MT20	4.0	6.0	2.00	4.25
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
K	594	0	594	0	0
G	704	0	704	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	418	289 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0
G	493	355 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	MAX FACTORED CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	-469 / 0	-77.4	-77.4 0.20 (1)	6.25	J-B	-573 / 0	0.09 (1)
B-C	-953 / 0	-77.4	-77.4 0.23 (1)	6.09	B-I	0 / 354	0.08 (1)
C-D	-952 / 0	-77.4	-77.4 0.23 (1)	6.09	I-C	0 / 722	0.16 (1)
D-E	-502 / 0	-77.4	-77.4 0.19 (1)	6.25	I-D	0 / 354	0.08 (1)
E-F	0 / 28	-77.4	-77.4 0.09 (1)	10.00	H-D	-511 / 0	0.08 (1)
K-A	-582 / 0	0.0	0.0 0.06 (1)	7.81	A-J	0 / 595	0.13 (1)
G-E	-691 / 0	0.0	0.0 0.07 (1)	7.81	H-E	0 / 555	0.12 (1)
K-J	0 / 0	-18.2	-18.2 0.01 (4)	10.00			
J-I	0 / 496	-18.3	-18.3 0.16 (4)	10.00			
I-H	0 / 496	-18.3	-18.3 0.16 (4)	10.00			
H-G	0 / 0	-18.2	-18.2 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.23/1.00 (C-D:1), BC=0.16/1.00 (H-I:4), WB=0.16/1.00 (C-I:1), SI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

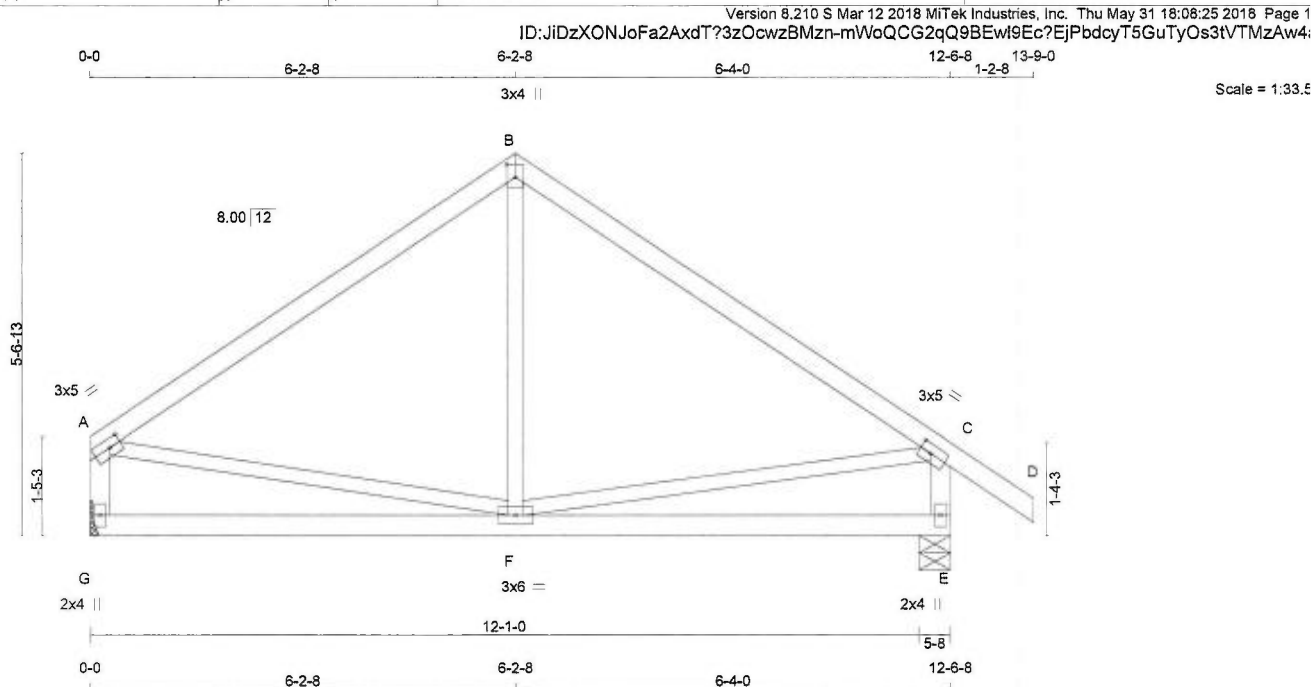
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 49 = 99 lb [M]F

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
G - A	2x4	DRY	No.2	SPF	
E - C	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00	
B	TTW+p	MT20	3.0	4.0	2.25	1.50	
C	TMVW-t	MT20	3.0	5.0	1.50	2.00	
E	BMV1+p	MT20	2.0	4.0			
F	BMVWV-t	MT20	3.0	6.0			
G	BMV1+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	600	0	600	0	0	0	MECHANICAL		
E	699	0	699	0	0	0	5-8	5-8	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	421	292 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0
E	489	352 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-450 / 0	-77.4 -77.4	0.39 (1)	6.25	F-B	0 / 116	0.04 (4)
B-C	-450 / 0	-77.4 -77.4	0.40 (1)	6.25	A-F	0 / 380	0.09 (1)
C-D	0 / 28	-77.4 -77.4	0.09 (1)	10.00	F-C	0 / 379	0.09 (1)
G-A	-557 / 0	0.0 0.0	0.06 (1)	7.81			
E-C	-655 / 0	0.0 0.0	0.07 (1)	7.81			
G-F	0 / 0	-18.2 -18.2	0.21 (4)	10.00			
F-E	0 / 0	-18.2 -18.2	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.40/1.00 (B-C:1), BC=0.21/1.00 (F-G:4), WB=0.09/1.00 (A-F:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

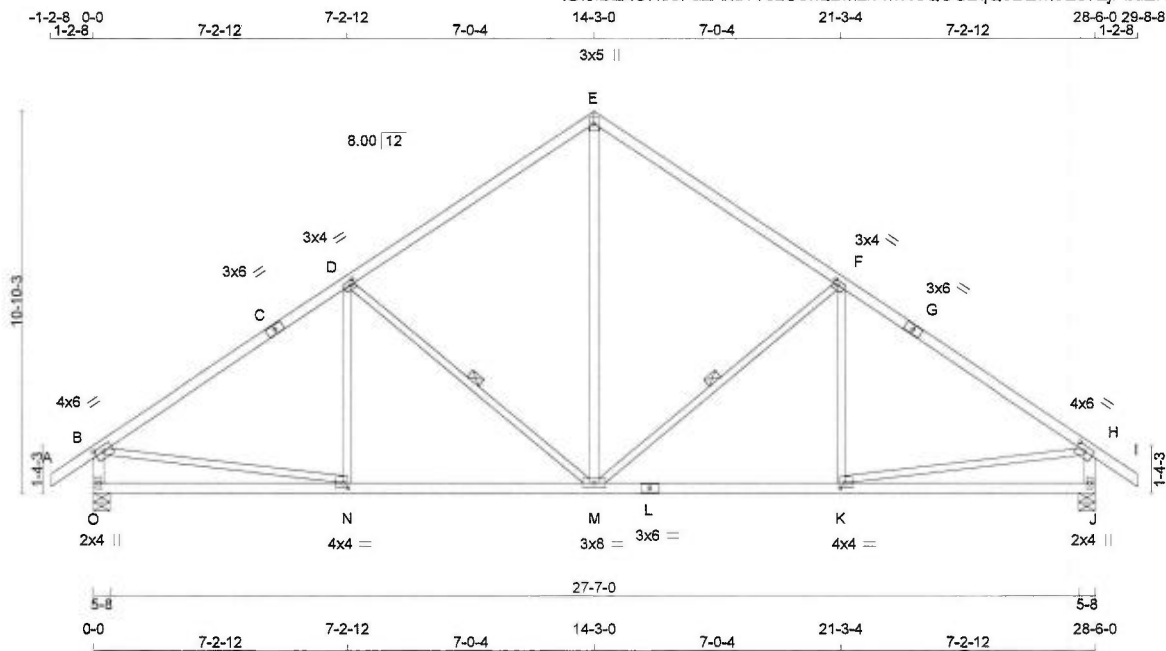
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:65.4

TOTAL WEIGHT = $3 \times 124 = 371 \text{ lb}$
[M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)		PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TS-4	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TS-1	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	1.75	1.50
L	BS-1	MT20	3.0	6.0		
M	BMVWV-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	4.0	1.75	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1462	0	1462	0	0	5-8	5-8
J	1462	0	1462	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1025	724 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0
J	1025	724 / 0	0 / 0	0 / 0	0 / 0	301 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0 . CBF = 69 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	M-E	0 / 832	0.13 (1)
B-C	-1591 / 0	-77.4	-77.4 0.63 (1)	4.50	M-F	-554 / 0	0.32 (1)
C-D	-1591 / 0	-77.4	-77.4 0.63 (1)	4.50	K-F	-47 / 108	0.04 (4)
D-E	-1154 / 0	-77.4	-77.4 0.56 (1)	5.16	D-M	-554 / 0	0.32 (1)
E-F	-1154 / 0	-77.4	-77.4 0.56 (1)	5.16	N-D	-47 / 108	0.04 (4)
F-G	-1591 / 0	-77.4	-77.4 0.63 (1)	4.50	B-N	0 / 1368	0.31 (1)
G-H	-1591 / 0	-77.4	-77.4 0.63 (1)	4.50	K-H	0 / 1368	0.31 (1)
H-I	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
O-B	-1408 / 0	0.0	0.0 0.14 (1)	6.87			
J-H	-1408 / 0	0.0	0.0 0.14 (1)	6.87			
O-N	0 / 0	-18.2	-18.2 0.23 (4)	10.00			
N-M	0 / 1354	-18.2	-18.2 0.34 (1)	10.00			
M-L	0 / 1354	-18.2	-18.2 0.34 (1)	10.00			
L-K	0 / 1354	-18.2	-18.2 0.34 (1)	10.00			
K-J	0 / 0	-18.2	-18.2 0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.95")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.95")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.63/1.00 (F-H:1), BC=0.34/1.00 (K-M:1),
WB=0.32/1.00 (F-M:1), SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

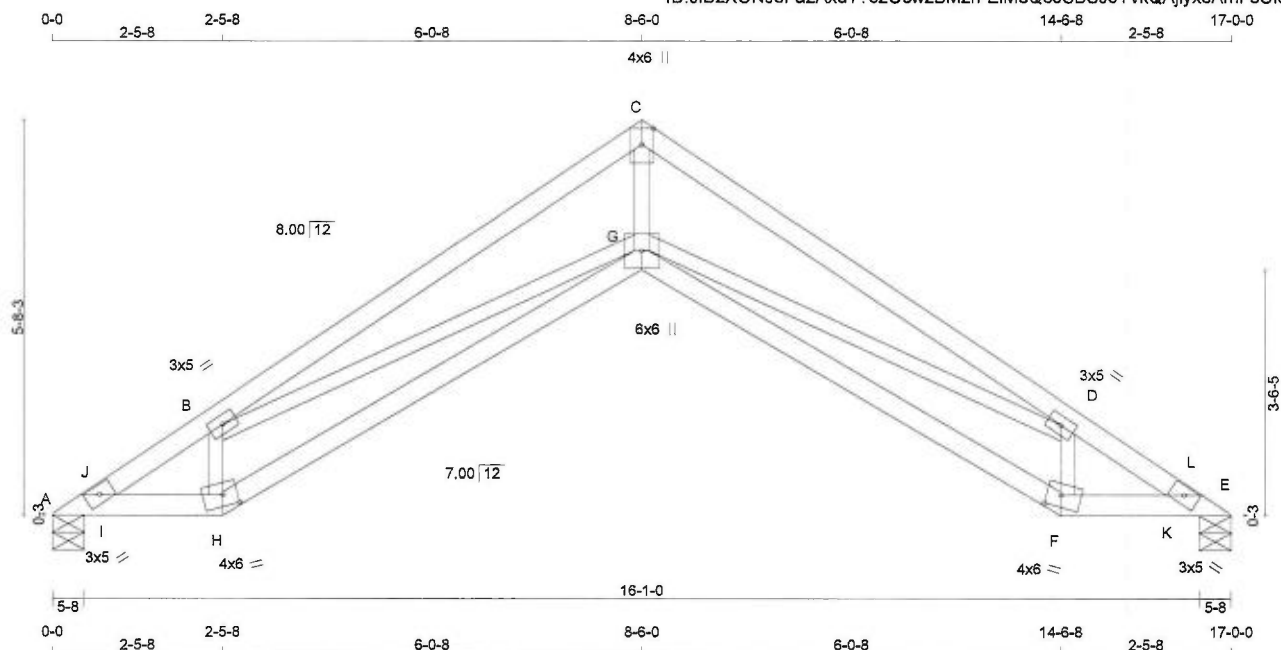
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.51 (N) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 58 lb [M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	5.0	
B	TMWW-t	MT20	3.0	5.0	
C	TTW+p	MT20	4.0	6.0	2.75 2.00
D	TMWW-t	MT20	3.0	5.0	
E	TBM1-h	MT20	3.0	5.0	
F	BBW-m	MT20	4.0	6.0	2.00 2.50
G	BBWWV+p	MT20	6.0	6.0	
H	BBW-m	MT20	4.0	6.0	2.00 2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
A	813	0	813	0	0	5-8	5-8	5-8	5-8
E	813	0	813	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	571	396 / 0	0 / 0	0 / 0	0 / 0	0 / 0	175 / 0
E	571	396 / 0	0 / 0	0 / 0	0 / 0	0 / 0	175 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS		WEBS		CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-J	-1825 / 0	-77.4	-77.4 0.20 (1)	G-C	0 / 2152	0.48 (1)	10.00
J-B	-1718 / 0	-77.4	-77.4 0.34 (1)	G-D	0 / 485	0.11 (1)	10.00
B-C	-2317 / 0	-77.4	-77.4 0.49 (1)	F-D	-721 / 0	0.10 (1)	10.00
C-D	-2317 / 0	-77.4	-77.4 0.49 (1)	B-G	0 / 485	0.11 (1)	10.00
D-L	-1718 / 0	-77.4	-77.4 0.34 (1)	H-B	-721 / 0	0.10 (1)	10.00
L-E	-1825 / 0	-77.4	-77.4 0.20 (1)	I-J	0 / 118	0.00 (1)	10.00
				K-L	0 / 118	0.00 (1)	10.00
A-I	0 / 1580	-18.2	-18.2 0.38 (1)				
I-H	0 / 1576	-18.3	-18.3 0.38 (1)				
H-G	0 / 1682	-18.2	-18.2 0.38 (1)				
G-F	0 / 1682	-18.2	-18.2 0.38 (1)				
F-K	0 / 1576	-18.3	-18.3 0.38 (1)				
K-E	0 / 1580	-18.3	-18.3 0.38 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/767 (0.27")

CSI: TC=0.49/1.00 (C-D:1), BC=0.38/1.00 (A-I:1), WB=0.48/1.00 (C-G:1), SSI=0.24/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

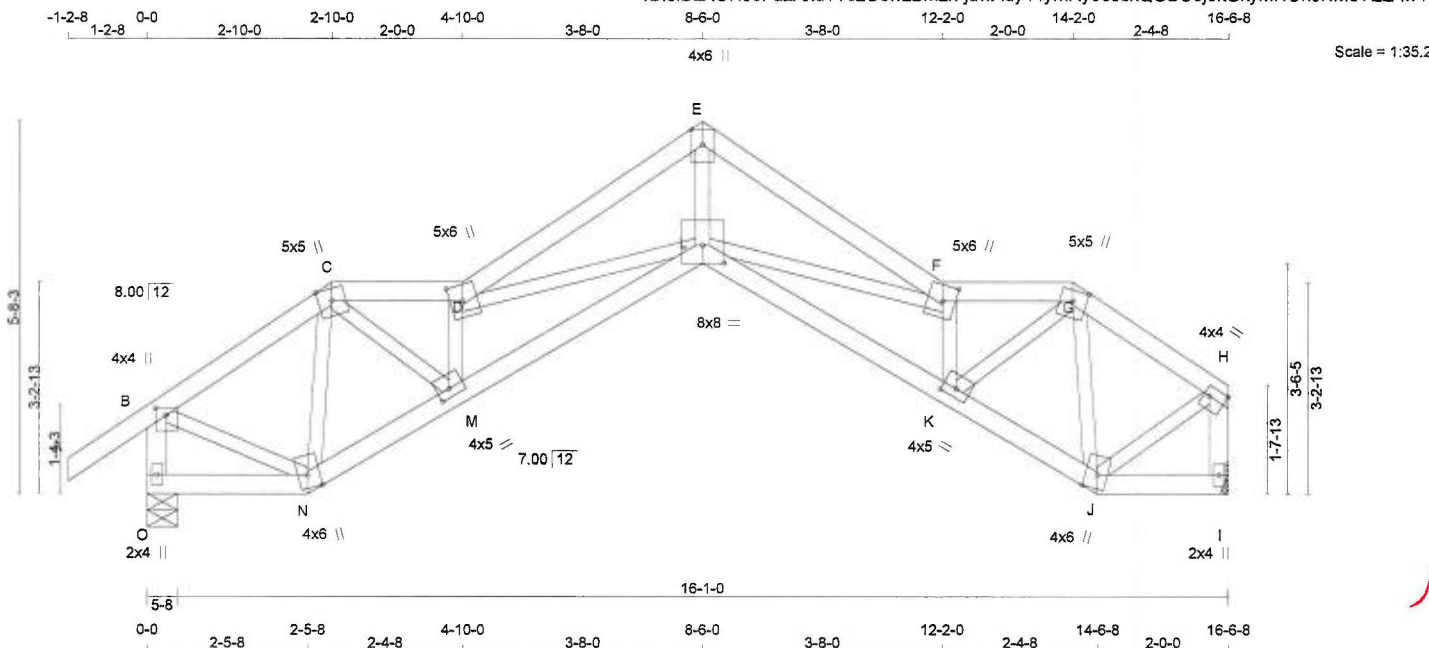
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.65 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:35.2

TOTAL WEIGHT = 69 lb [M]F

LUMBER		N. L. G. A. RULES		LUMBER		DESCR.	
CHORDS	SIZE						
A - C	2x4	DRY	No.2	SPF			
C - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
E - F	2x4	DRY	No.2	SPF			
F - G	2x4	DRY	No.2	SPF			
G - H	2x4	DRY	No.2	SPF			
O - B	2x4	DRY	No.2	SPF			
I - H	2x4	DRY	No.2	SPF			
O - N	2x4	DRY	No.2	SPF			
N - L	2x4	DRY	No.2	SPF			
L - J	2x4	DRY	No.2	SPF			
J - I	2x4	DRY	No.2	SPF			

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	5.0	Edge	
D	TTWW+m	MT20	5.0	6.0	3.00	2.25
E	TTWW+p	MT20	4.0	6.0	2.75	2.00
F	TTWW+m	MT20	5.0	6.0	3.00	2.25
G	TTWW+m	MT20	5.0	5.0	2.00	2.50
H	TMVW-t	MT20	4.0	4.0	1.75	Edge
I	BMV1+p	MT20	2.0	4.0		
J	BBWW+m	MT20	4.0	6.0	Edge	2.25
K	BMWW-t	MT20	4.0	5.0	1.50	2.50
L	BBWWV-p	MT20	8.0	8.0	3.25	4.00
M	BMWW-t	MT20	4.0	5.0	1.50	2.25
N	BBWW+m	MT20	4.0	6.0	Edge	2.25
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
O	891	0	891	0	0	5-8	5-8	
I	791	0	791	0	0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	623	445 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0
I	556	385 / 0	0 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	MAX LC1 (LC)
FR-TO				FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	N-C	-609 / 0	10.00	0.11 (1)
B-C	-732 / 0	-77.4	-77.4 0.09 (1)	C-M	0 / 1288	6.25	0.29 (1)
C-D	-1674 / 0	-77.4	-77.4 0.07 (1)	M-D	-1411 / 0	5.09	0.21 (1)
D-E	-2163 / 0	-77.4	-77.4 0.19 (1)	D-L	0 / 37	4.48	0.01 (1)
E-F	-2163 / 0	-77.4	-77.4 0.19 (1)	L-E	0 / 2106	4.48	0.47 (1)
F-G	-1571 / 0	-77.4	-77.4 0.06 (1)	L-F	0 / 142	5.22	0.03 (1)
G-H	-615 / 0	-77.4	-77.4 0.06 (1)	K-F	-1393 / 0	6.25	0.20 (1)
O-B	-868 / 0	0.0	0.0 0.09 (1)	K-G	0 / 1274	7.81	0.29 (1)
I-H	-773 / 0	0.0	0.0 0.08 (1)	G-J	-647 / 0	7.81	0.12 (1)
				B-N	0 / 657		0.15 (1)
				J-H	0 / 612		0.14 (1)
O-N	0 / 0	-18.2	-18.2 0.03 (4)				
N-M	0 / 792	-18.2	-18.2 0.15 (1)				
M-L	0 / 2047	-18.2	-18.2 0.38 (1)				
L-K	0 / 1927	-18.3	-18.3 0.36 (1)				
K-J	0 / 686	-18.2	-18.2 0.14 (1)				
J-I	0 / 0	-18.3	-18.3 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.55")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.55")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.19/1.00 (E-F:1), BC=0.38/1.00 (L-M:1), WB=0.47/1.00 (E-L:1), SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

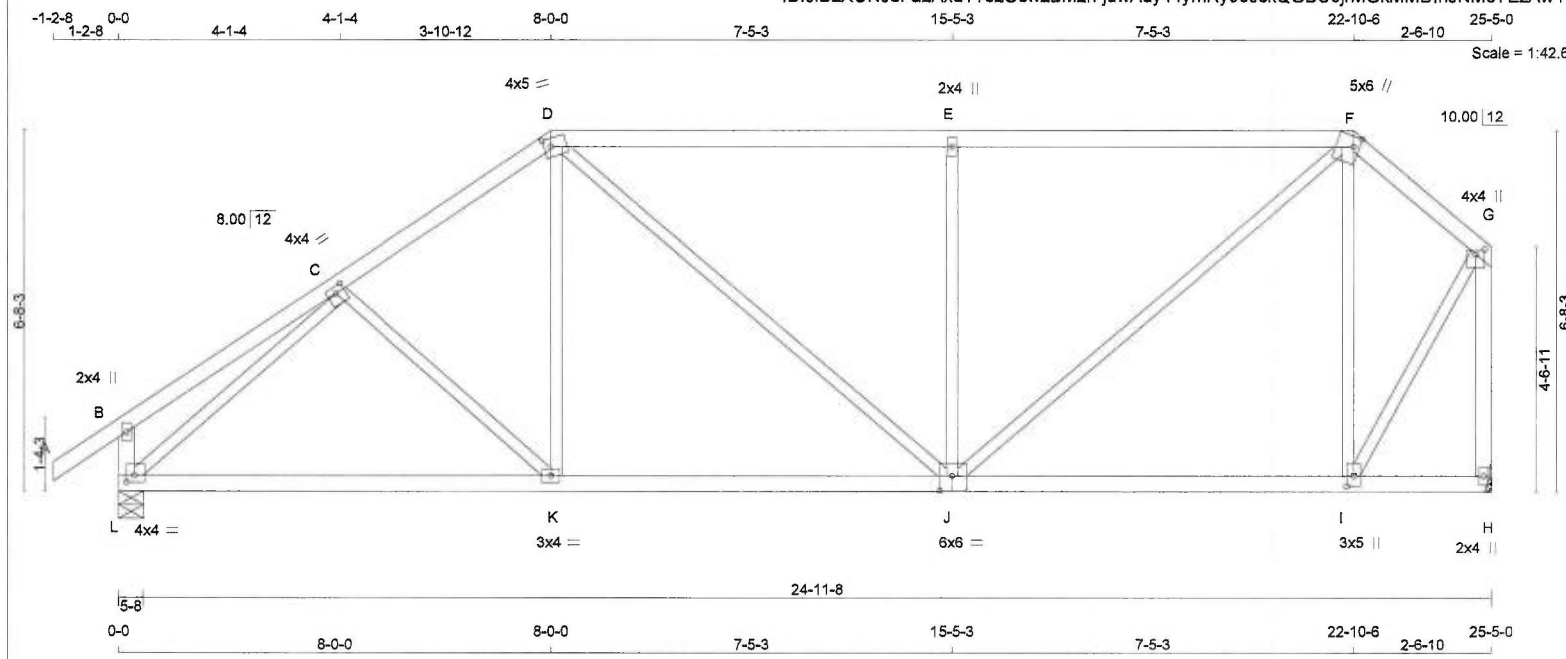
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.37 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 111 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0			
C	TMWW-t	MT20	4.0	4.0	1.50	2.00	
D	TTWW-m	MT20	4.0	5.0	1.75	1.50	
E	TMW+w	MT20	2.0	4.0			
F	TTWW+m	MT20	5.0	6.0	2.25	1.25	
G	TMVW+p	MT20	4.0	4.0	1.00	2.00	
H	BMV1+p	MT20	2.0	4.0			
I	BMVW+t	MT20	3.0	5.0	2.25	1.50	
J	BSWWW-t	MT20	6.0	6.0	3.25	2.75	
K	BMWW-t	MT20	3.0	4.0			
L	BMVW-t	MT20	4.0	4.0	1.50	1.75	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1315	0	5-8	5-8
H	1215	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	921	652 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
H	854	592 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	C-K	-80 / 33	0.04 (1)
B-C	0 / 19	-77.4	-77.4 0.19 (1)	10.00	K-D	0 / 215	0.05 (4)
C-D	-1296 / 0	-77.4	-77.4 0.22 (1)	5.43	D-J	0 / 239	0.05 (1)
D-E	-1245 / 0	-77.4	-77.4 0.86 (1)	4.32	J-E	-709 / 0	0.53 (1)
E-F	-1245 / 0	-77.4	-77.4 0.86 (1)	4.32	J-F	0 / 1047	0.24 (1)
F-G	-601 / 0	-77.4	-77.4 0.09 (1)	6.25	I-F	-614 / 0	0.45 (1)
L-B	-220 / 0	0.0	0.0 0.02 (1)	7.81	L-C	-1525 / 0	0.74 (1)
H-G	-1217 / 0	0.0	0.0 0.44 (1)	7.26	I-G	0 / 864	0.19 (1)
L-K	0 / 1120	-18.2	-18.2 0.35 (4)	10.00			
K-J	0 / 1064	-18.2	-18.2 0.36 (4)	10.00			
J-I	0 / 450	-18.2	-18.2 0.21 (4)	10.00			
I-H	0 / 0	-18.2	-18.2 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.86/1.00 (D-E:1), BC=0.36/1.00 (J-K:4), WB=0.74/1.00 (C-L:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

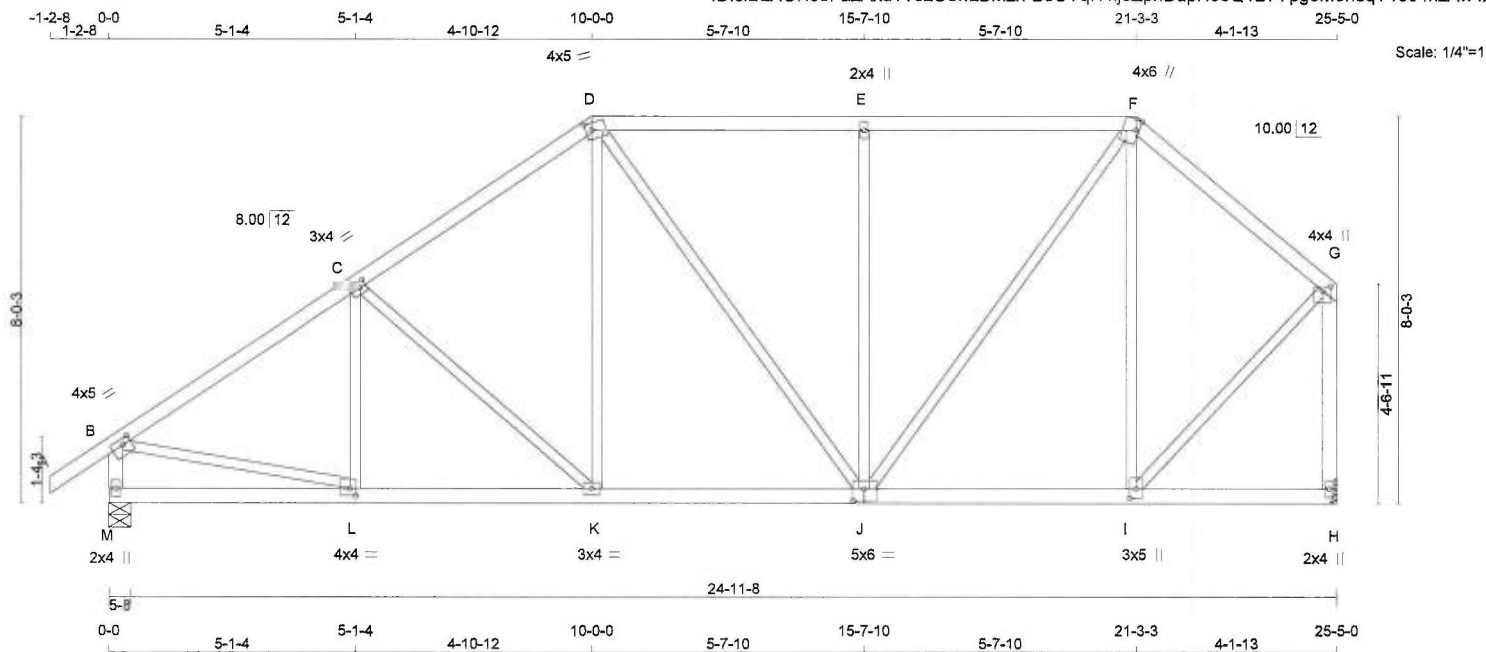
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 120 lb [M]F

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTVW-m	MT20	4.0	5.0	1.75	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTVW+m	MT20	4.0	6.0	2.50	0.75
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW+t	MT20	3.0	5.0	2.25	1.50
J	BSVW-w	MT20	5.0	6.0	3.00	2.75
K	BMVW-t	MT20	3.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.75	1.50
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1315	0	1315	0	0	5-8	5-8		
H	1215	0	1215	0	0	MECHANICAL			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
M	921	652 / 0	0 / 0	0 / 0	270 / 0	0 / 0
H	854	592 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	L-C	-139 / 43	0.05 (1)
B-C	-1392 / 0	-77.4 -77.4	0.28 (1)	5.23	C-K	-295 / 0	0.24 (1)
C-D	-1179 / 0	-77.4 -77.4	0.26 (1)	5.58	K-D	0 / 293	0.07 (1)
D-E	-990 / 0	-77.4 -77.4	0.33 (1)	5.86	D-J	0 / 48	0.01 (1)
E-F	-990 / 0	-77.4 -77.4	0.33 (1)	5.86	J-E	-534 / 0	0.86 (1)
F-G	-732 / 0	-77.4 -77.4	0.18 (1)	6.25	J-F	0 / 738	0.17 (1)
M-B	-1275 / 0	0.0 0.0	0.13 (1)	7.13	I-F	-455 / 0	0.56 (1)
H-G	-1187 / 0	0.0 0.0	0.43 (1)	7.33	B-L	0 / 1202	0.27 (1)
					I-G	0 / 790	0.18 (1)
M-L	0 / 0	-18.2 -18.2	0.10 (4)	10.00			
L-K	0 / 1179	-18.2 -18.2	0.24 (1)	10.00			
K-J	0 / 962	-18.2 -18.2	0.21 (1)	10.00			
J-I	0 / 555	-18.2 -18.2	0.15 (4)	10.00			
I-H	0 / 0	-18.2 -18.2	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.43/1.00 (G-H:1), BC=0.24/1.00 (K-L:1), WB=0.66/1.00 (E-J:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

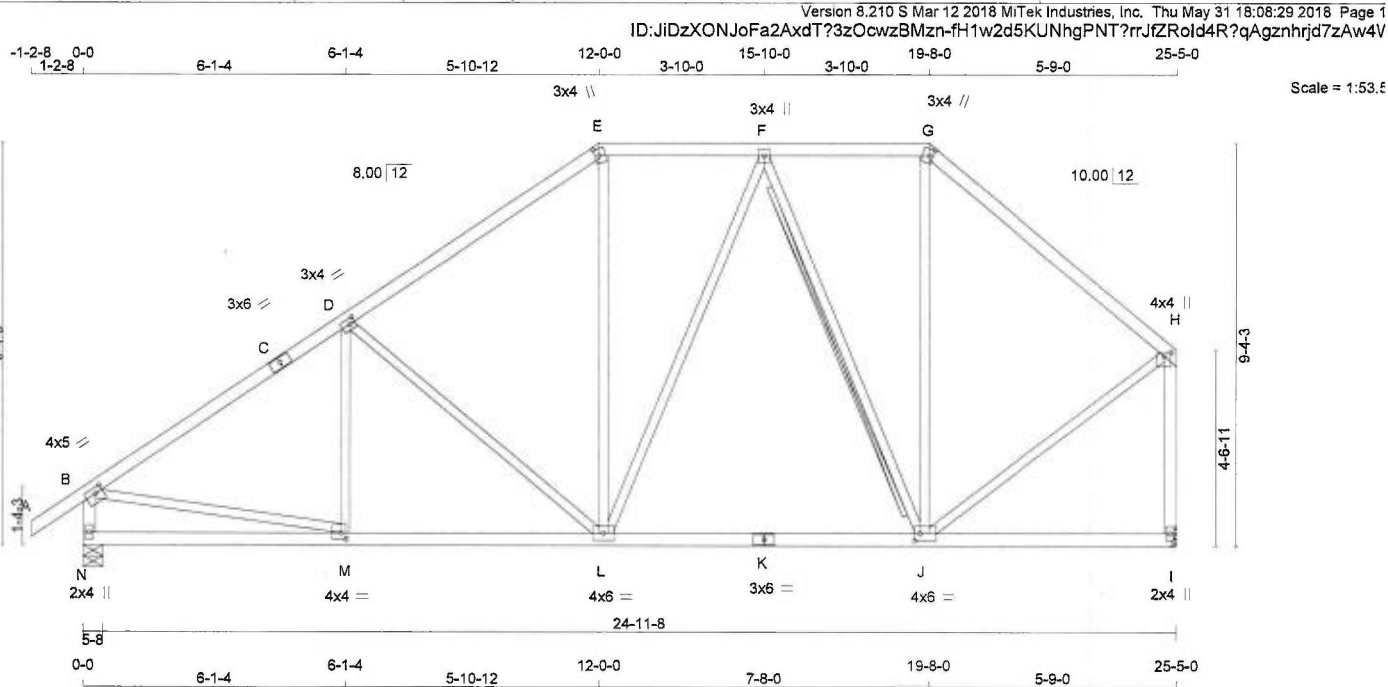
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+m	MT20	3.0	4.0	2.00	1.25
F	TMWW-t	MT20	3.0	4.0		
G	TTW+m	MT20	3.0	4.0	2.00	1.00
H	TMVV+p	MT20	4.0	4.0	1.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
N	1315	0	1315	0	5-8	5-8	
I	1215	0	1215	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	921	652 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
I	854	592 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	M-D	-82 / 75	0.04 (1)
B-C	-1391 / 0	-77.4 -77.4	0.41 (1)	5.06	D-L	-430 / 0	0.55 (1)
C-D	-1391 / 0	-77.4 -77.4	0.41 (1)	5.06	L-E	0 / 284	0.06 (1)
D-E	-1061 / 0	-77.4 -77.4	0.38 (1)	5.64	J-G	0 / 172	0.04 (4)
E-F	-858 / 0	-77.4 -77.4	0.15 (1)	6.25	B-M	0 / 1199	0.27 (1)
F-G	-614 / 0	-77.4 -77.4	0.14 (1)	6.25	J-H	0 / 754	0.17 (1)
G-H	-798 / 0	-77.4 -77.4	0.34 (1)	6.25	F-J	-515 / 0	0.41 (1)
N-B	-1268 / 0	0.0 0.0	0.13 (1)	7.15	L-F	0 / 127	0.03 (1)
I-H	-1176 / 0	0.0 0.0	0.42 (1)	7.35			
N-M	0 / 0	-18.2 -18.2	0.15 (4)	10.00			
M-L	0 / 1183	-18.2 -18.2	0.28 (1)	10.00			
L-K	0 / 810	-18.2 -18.2	0.26 (4)	10.00			
K-J	0 / 810	-18.2 -18.2	0.26 (4)	10.00			
J-I	0 / 0	-18.2 -18.2	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.42/1.00 (H-I:1), BC=0.28/1.00 (L-M:1), WB=0.55/1.00 (D-L:1), SS=0.19/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

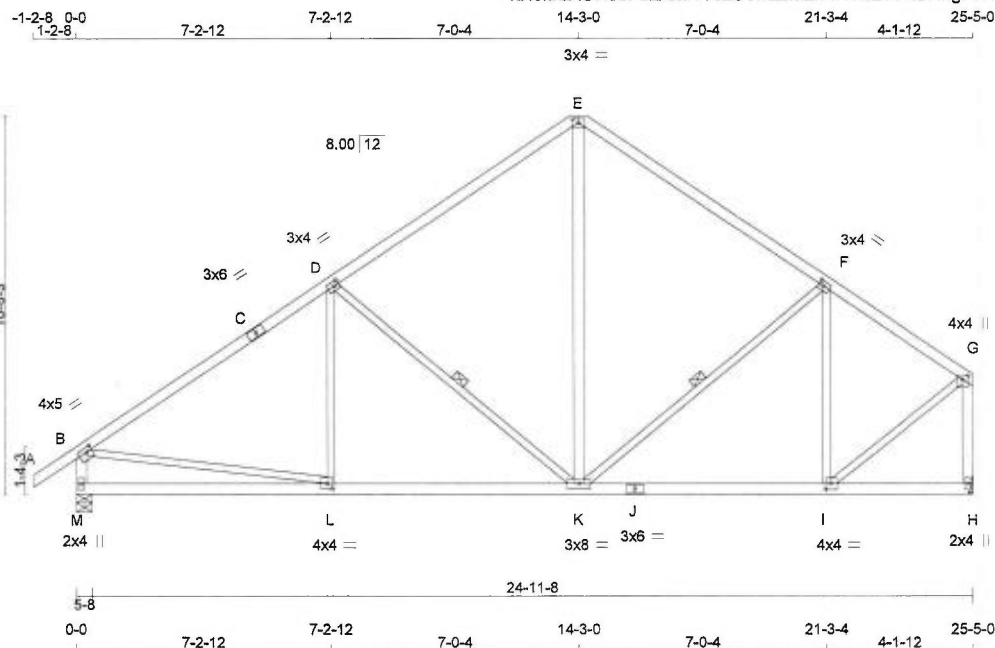
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:65.2

TOTAL WEIGHT = 115 lb [M]F

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - E	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVV-t	MT20	4.0	5.0	1.50 2.00
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	3.0	4.0	1.50 1.50
E	TTW-p	MT20	3.0	4.0	1.25 2.00
F	TMWW-t	MT20	3.0	4.0	1.50 1.50
G	TMVV+p	MT20	4.0	4.0	1.25 2.00
H	BMV1+p	MT20	2.0	4.0	
I	BMWW-t	MT20	4.0	4.0	2.00 1.75
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	3.0	8.0	
L	BMWW-t	MT20	4.0	4.0	1.75 1.75
M	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1315	0	1315	0	0
H	1215	0	1215	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
M	921	652 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0	
H	854	592 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-K. DBS = 20-0-0. CBF = 72 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	L-D	-23 / 114	0.04 (4)
B-C	-1372 / 0	-77.4 -77.4	0.60 (1)	4.79	D-K	-576 / 0	0.33 (1)
C-D	-1372 / 0	-77.4 -77.4	0.60 (1)	4.79	K-E	0 / 549	0.09 (1)
D-E	-915 / 0	-77.4 -77.4	0.55 (1)	5.65	K-F	-80 / 0	0.05 (1)
E-F	-905 / 0	-77.4 -77.4	0.48 (1)	5.81	I-F	-475 / 0	0.29 (1)
F-G	-904 / 0	-77.4 -77.4	0.38 (1)	5.96	B-L	0 / 1184	0.27 (1)
M-B	-1261 / 0	0.0 0.0	0.13 (1)	7.16	I-G	0 / 986	0.22 (1)
H-G	-1191 / 0	0.0 0.0	0.23 (1)	7.32			
M-L	0 / 0	-18.2 -18.2	0.22 (4)	10.00			
L-K	0 / 1172	-18.2 -18.2	0.31 (4)	10.00			
K-J	0 / 791	-18.2 -18.2	0.23 (4)	10.00			
J-I	0 / 791	-18.2 -18.2	0.23 (4)	10.00			
I-H	0 / 0	-18.2 -18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.60/1.00 (B-D:1), BC=0.31/1.00 (K-L:4), WB=0.33/1.00 (D-K:1), SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

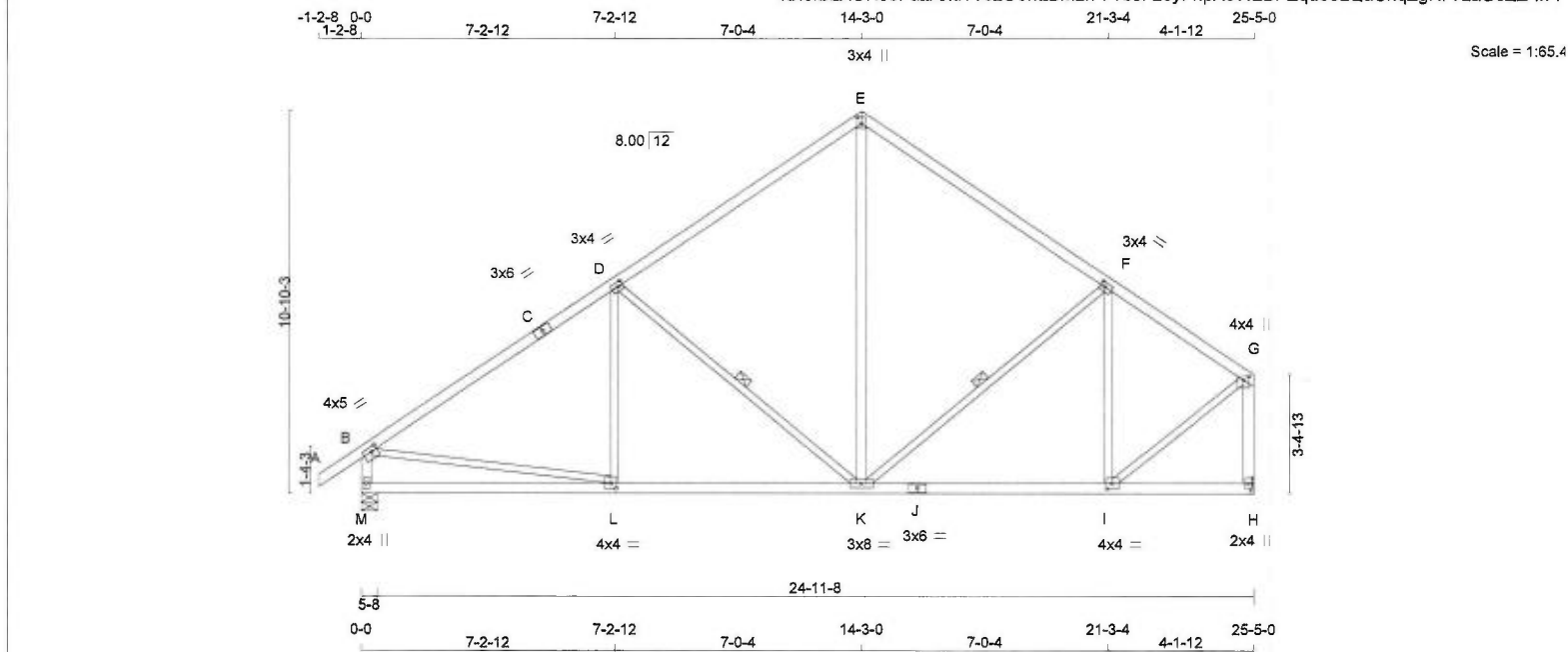
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 115 lb [M]/F

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-t	MT20	4.0	5.0	1.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	4.0	2.25	1.50
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMVV+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.75
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	FACTORED	MAXIMUM FACTORED	BRG	BRG	BRG	BRG	BRG	BRG
	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX
M	1315	0	1315	0	0	5-8	5-8		
H	1215	0	1215	0	0	MECHANICAL			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
M	921	652 / 0	0 / 0	0 / 0	270 / 0
H	854	592 / 0	0 / 0	0 / 0	262 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-K, F-K. DBS = 20-0-0. CBF = 72 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	L-D	-23 / 114	0.04 (4)	
B-C	-1372 / 0	-77.4 -77.4	0.60 (1)	D-K	-576 / 0	0.33 (1)	
C-D	-1372 / 0	-77.4 -77.4	0.60 (1)	K-E	0 / 549	0.09 (1)	
D-E	-915 / 0	-77.4 -77.4	0.55 (1)	E-F	-80 / 0	0.05 (1)	
E-F	-905 / 0	-77.4 -77.4	0.48 (1)	F-G	-475 / 0	0.29 (1)	
F-G	-904 / 0	-77.4 -77.4	0.38 (1)	G-H	0 / 1184	0.27 (1)	
M-B	-1261 / 0	0.0 0.0	0.13 (1)	H-G	0 / 986	0.22 (1)	
H-G	-1191 / 0	0.0 0.0	0.23 (1)				
M-L	0 / 0	-18.2 -18.2	0.22 (4)				
L-K	0 / 1172	-18.2 -18.2	0.31 (4)				
K-J	0 / 791	-18.2 -18.2	0.23 (4)				
J-I	0 / 791	-18.2 -18.2	0.23 (4)				
I-H	0 / 0	-18.2 -18.2	0.14 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.60/1.00 (B-D:1), BC=0.31/1.00 (K-L:4), WB=0.33/1.00 (D-K:1), SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

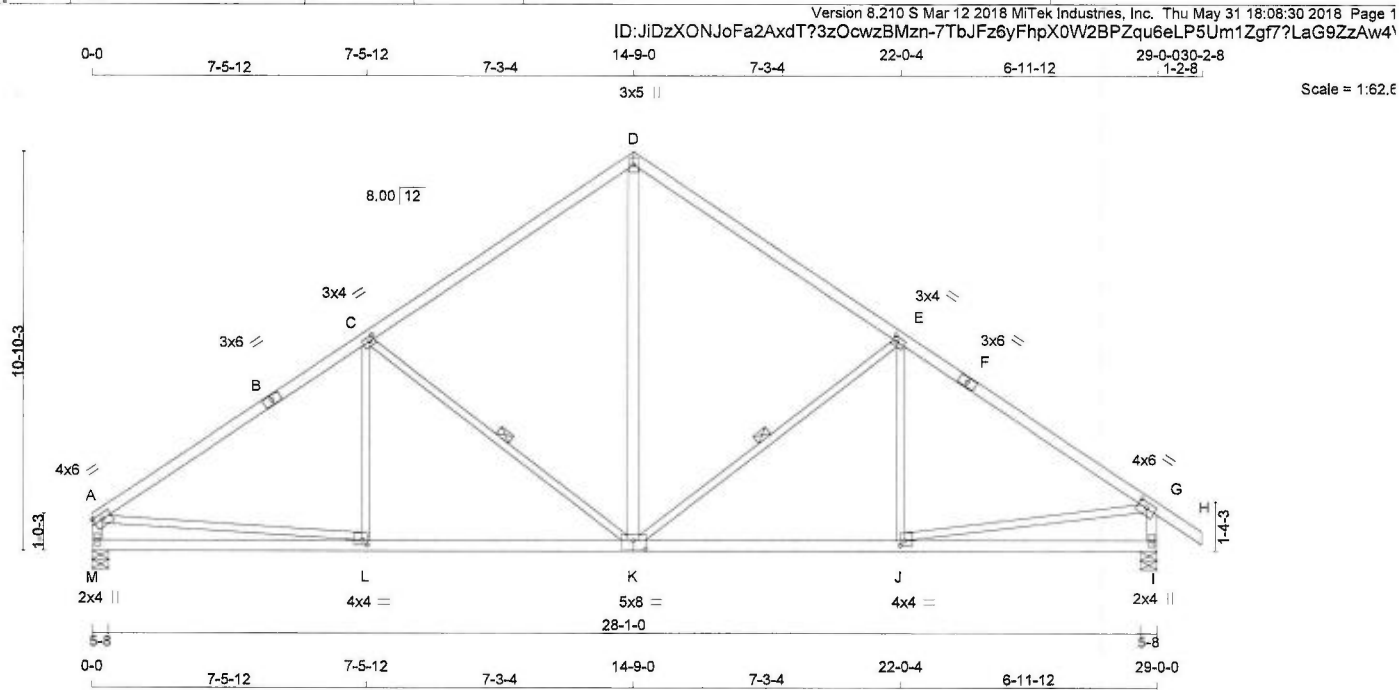
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 4 X 123 = 491 lb [M]F

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
K - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-t	MT20	4.0	6.0	1.75	Edge
B	TS-t	MT20	3.0	6.0		
C	TMVV-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	5.0		
E	TMVV-t	MT20	3.0	4.0	1.50	1.50
F	TS-t	MT20	3.0	6.0		
G	TMVV-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVV-t	MT20	4.0	4.0	1.50	1.50
K	BSWWW-t	MT20	5.0	8.0	3.00	4.00
L	BMVV-t	MT20	4.0	4.0	1.50	1.50
M	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
M	1386	0	5-8	5-8
I	1486	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	974	675 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
I	1042	735 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-K. DBS = 20-0-0. CBF = 79 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1718 / 0	-77.4 -77.4	0.70 (1)	L-C	0 / 130	0.05 (4)	
B-C	-1718 / 0	-77.4 -77.4	0.70 (1)	C-K	-634 / 0	0.38 (1)	
C-D	-1195 / 0	-77.4 -77.4	0.61 (1)	K-D	0 / 856	0.14 (1)	
D-E	-1192 / 0	-77.4 -77.4	0.59 (1)	K-E	-547 / 0	0.32 (1)	
E-F	-1636 / 0	-77.4 -77.4	0.61 (1)	J-E	-61 / 103	0.04 (4)	
F-G	-1636 / 0	-77.4 -77.4	0.61 (1)	A-L	0 / 1468	0.33 (1)	
G-H	0 / 28	-77.4 -77.4	0.09 (1)	J-G	0 / 1408	0.32 (1)	
M-A	-1330 / 0	0.0	0.0				
I-G	-1434 / 0	0.0	0.0				
M-L	0 / 0	-18.2 -18.2	0.24 (4)				
L-K	0 / 1462	-18.2 -18.2	0.36 (1)				
K-J	0 / 1393	-18.2 -18.2	0.34 (1)				
J-I	0 / 0	-18.2 -18.2	0.22 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.70/1.00 (A-C:1) . BC=0.36/1.00 (K-L:1) . WB=0.38/1.00 (C-K:1) . SSI=0.23/1.00 (A-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

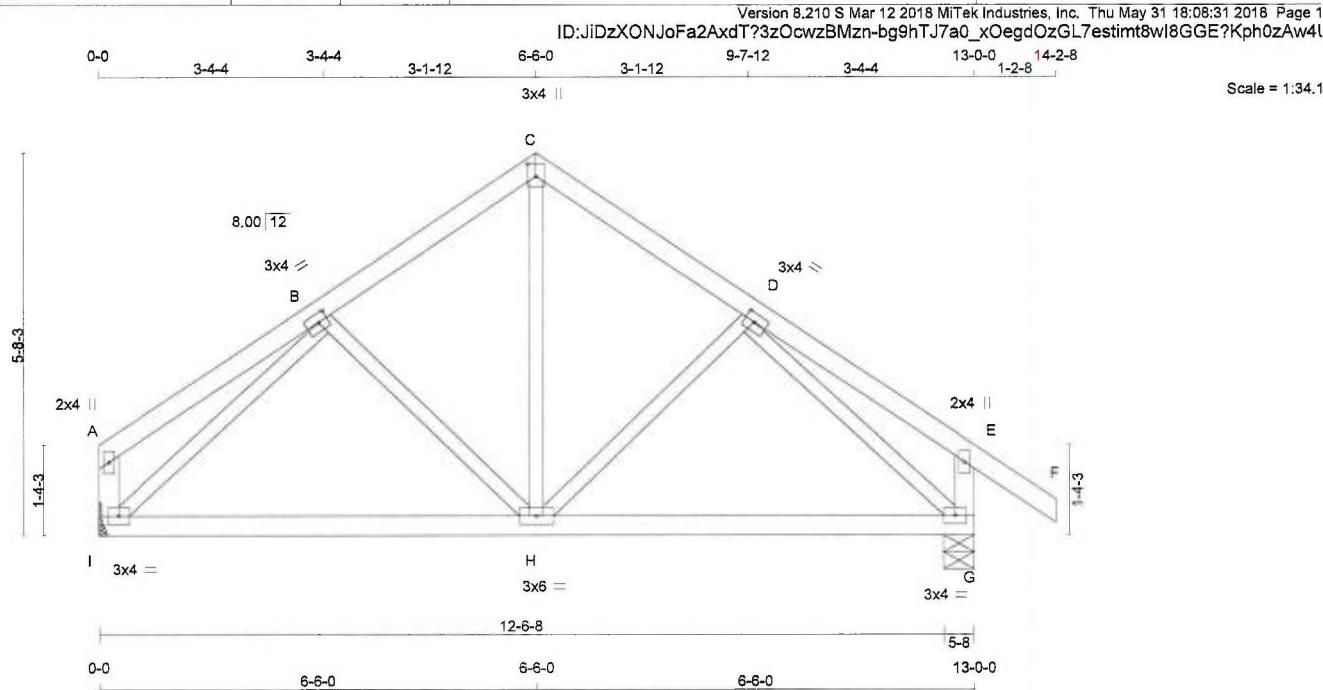
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.55 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 55 = 109 lb [M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
I - A	2x4	DRY	No.2
G - E	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMVW-t	MT20	3.0	4.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	4.0	1.50	1.75
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	4.0		
H	BMVWV-t	MT20	3.0	6.0		
I	BMVW1-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	621	0	621	0	0
G	721	0	721	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
I	437	303 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
G	504	363 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 15	-77.4	-77.4 0.13 (1)	H-C	0 / 328	0.07 (1)	
B-C	-488 / 0	-77.4	-77.4 0.10 (1)	H-D	-130 / 7	0.04 (1)	
C-D	-488 / 0	-77.4	-77.4 0.10 (1)	B-H	-130 / 7	0.04 (1)	
D-E	0 / 15	-77.4	-77.4 0.13 (1)	I-B	-682 / 0	0.22 (1)	
E-F	0 / 28	-77.4	-77.4 0.09 (1)	D-G	-682 / 0	0.22 (1)	
I-A	-97 / 0	0.0	0.0 0.01 (1)				
G-E	-197 / 0	0.0	0.0 0.02 (1)				
I-H	0 / 487	-18.2	-18.2 0.25 (4)				
H-G	0 / 487	-18.2	-18.2 0.25 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.13/1.00 (D-E-1), BC=0.25/1.00 (G-H-4), WB=0.22/1.00 (D-G-1), SSI=0.10/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

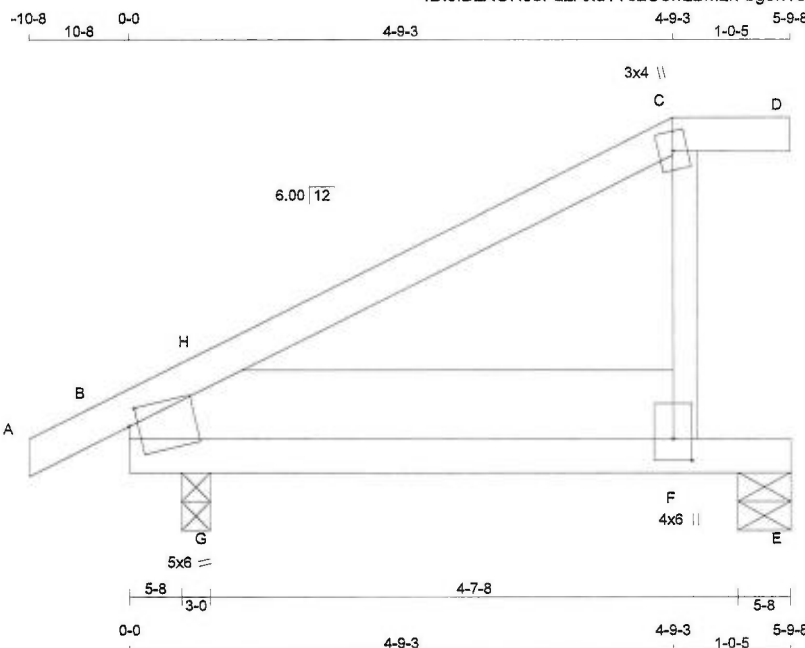
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:20.2

TOTAL WEIGHT = 28 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
REINFORCING MEMBERS			
HW1	2x8	DRY	No.2
ALL WEBS			
2x3	DRY	No.2	
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBMW1-m	MT20	5.0	6.0	1.75 1.00
C	TTW+m	MT20	3.0	4.0	
F	BMWW+t	MT20	4.0	6.0	2.25 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	SPF	GROSS REACTION	VERT	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	SPF	270	0	270	0	5-8	5-8		
B	SPF	357	0	357	0	3-0	3-0		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
E	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
E	190	131 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0		
B	249	183 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LOAD LC1 (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM TO		FR-TO			
A-B	0 / 2	-77.4 -77.4	0.05 (1)	F-C	-205 / 0	0.04 (1)	10.00
B-H	-636 / 0	-77.4 -77.4	0.15 (1)	F-H	-510 / 0	0.06 (1)	6.25
H-C	-21 / 0	-77.4 -77.4	0.15 (1)	G-H	0 / 277	0.00 (1)	6.25
C-D	0 / 0	-77.4 -77.4	0.06 (1)				10.00
B-G	0 / 508	-18.2 -18.2	0.23 (1)				10.00
G-F	0 / 508	-18.2 -18.2	0.41 (1)				10.00
F-E	0 / 0	-18.2 -18.2	0.33 (1)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/765 (0.09")

CSI: TC=0.15/1.00 (B-H:1), BC=0.41/1.00 (F-G:1), WB=0.06/1.00 (F-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

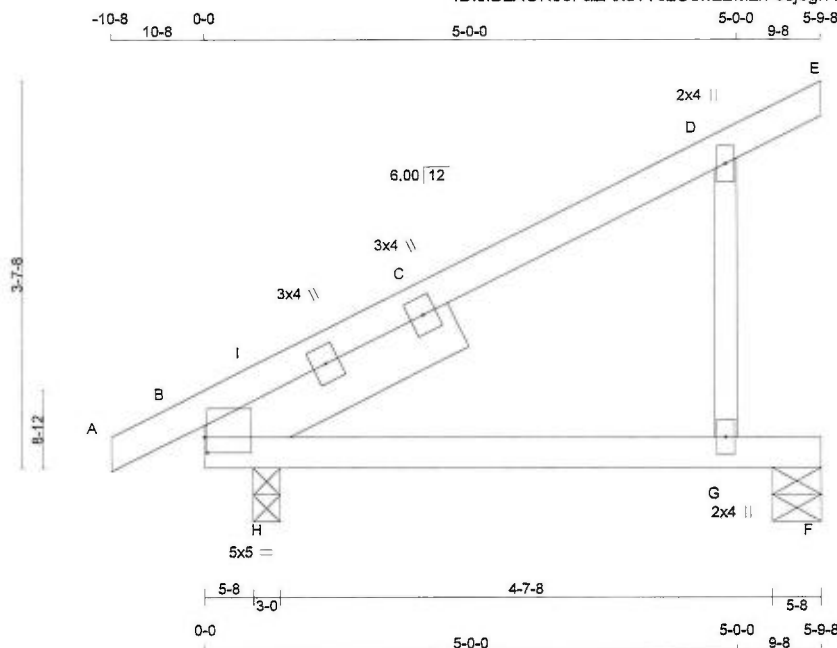
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (B) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 22 = 44 lb
(M)F

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS		No.2		SPF
A - E	2x4	DRY		
B - F	2x4	DRY		
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY				
DRY: SEASONED LUMBER.				SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT								
B	357	0	357	0	0	3-0	3-0	
F	275	0	275	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
B	249	183 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0	
F	193	134 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS		MEMB.	FACTORED	
	MAX. FACTORED	FORCE	FACTORED	FORCE		MAX. FACTORED	FORCE
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	UNBRAC LENGTH FR-TO	FR-TO	FORCE (LBS)	CS1 (LC)
A-B	0 / 2	-77.4	-77.4 0.05 (1)	10.00	G-D	-266 / 0	0.05 (1)
B-I	-304 / 0	-77.4	-77.4 0.07 (1)	6.25	H-I	-215 / 0	0.00 (1)
I-C	-157 / 0	-77.4	-77.4 0.34 (1)	6.25	H-C	0 / 340	0.04 (1)
C-D	-157 / 0	-77.4	-77.4 0.34 (1)	6.25			
D-E	-18 / 0	-77.4	-77.4 0.04 (1)	6.25			
B-H	0 / 249	-18.2	-18.2 0.09 (1)	10.00			
H-G	0 / 6	-18.2	-18.2 0.33 (1)	10.00			
G-F	0 / 0	-18.2	-18.2 0.33 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/869 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/473 (0.15")

CS1: TC=0.34/1.00 (D-I:1), BC=0.33/1.00 (G-H:1), WB=0.05/1.00 (D-G:1), SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

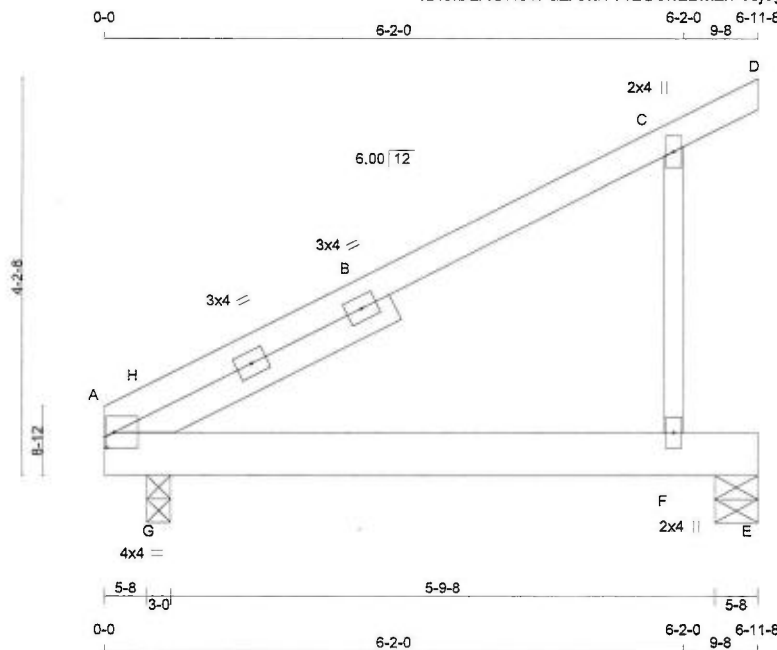
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 4 X 28 = 111 lb
[M]F

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - D	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				DESCR.
HW1	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMBR1-I	MT20	4.0	4.0	2.00 1.00
A	RT-I	MT20	3.0	4.0	
A	RT-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
F	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A	338	0	338	0	0	3-0	3-0	
E	332	0	332	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	238	166 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
E	233	162 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH FR-TO	WEBS		MAX. UNBRAC LENGTH FR-TO
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. (LC)		MAX. FACTORED FORCE (LBS)	MAX. (LC)	
FR-TO		FROM TO						
A-H	-82 / 243	-77.4	-77.4	0.19 (1)	6.25	F-C	-258 / 0	0.06 (1)
H-B	-41 / 27	-77.4	-77.4	0.29 (1)	6.25	G-H	-475 / 0	0.00 (1)
B-C	-41 / 27	-77.4	-77.4	0.29 (1)	6.25	G-B	-92 / 86	0.02 (4)
C-D	-18 / 0	-77.4	-77.4	0.04 (1)	6.25			
A-G	-68 / 69	-18.2	-18.2	0.17 (1)	6.25			
G-F	0 / 5	-18.2	-18.2	0.26 (1)	10.00			
F-E	0 / 0	-18.2	-18.2	0.19 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/744 (0.11")

CSI: TC=0.29/1.00 (C-H:1), BC=0.26/1.00 (F-G:1), WB=0.06/1.00 (C-F:1), SSI=0.35/1.00 (A-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

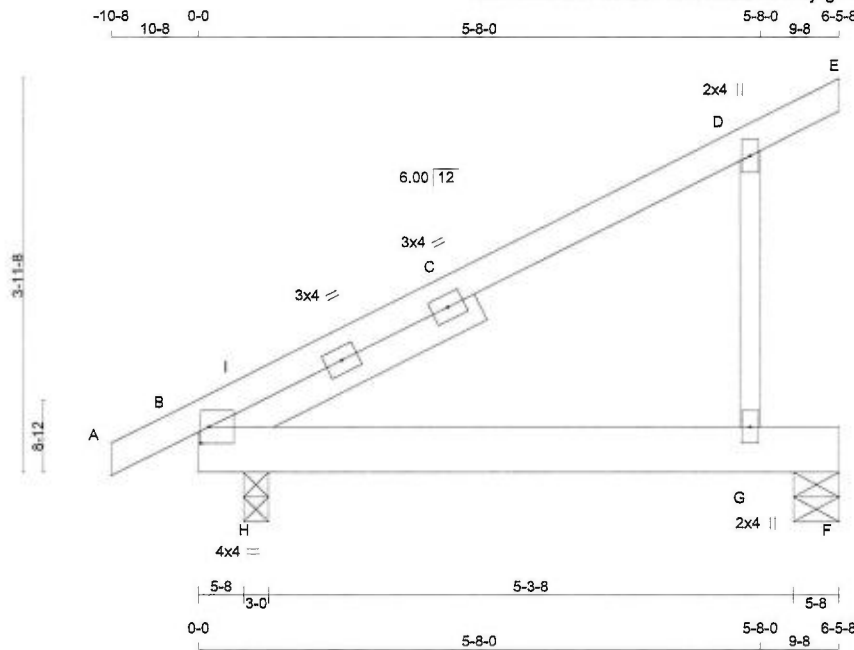
JSI GRIP= 0.46 (A) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:23.2



TOTAL WEIGHT = $2 \times 27 = 54 \text{ lb}$
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - E	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED			MAXIMUM FACTORED			INPUT	REQRD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	388	0	388	0	0	3-0	3-0
F	308	0	308	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	271	198 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	216	150 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A- B	0 / 5	-77.4	-77.4	0.05 (1)	10.00	G- D	-243 / 0		0.05 (1)
B- I	-74 / 178	-77.4	-77.4	0.15 (1)	6.25	H- I	-401 / 0		0.00 (1)
I- C	-36 / 14	-77.4	-77.4	0.25 (1)	6.25	H- C	-63 / 76		0.02 (4)
C- D	-36 / 14	-77.4	-77.4	0.25 (1)	6.25				
D- E	-18 / 0	-77.4	-77.4	0.04 (1)	6.25				
B- H	-44 / 80	-18.2	-18.2	0.14 (1)	6.25				
H- G	0 / 5	-18.2	-18.2	0.22 (1)	10.00				
G- F	0 / 0	-18.2	-18.2	0.18 (1)	10.00				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBR1-I	MT20	4.0	4.0	2.00	1.00
B	RT-t	MT20	3.0	4.0		
B	RT-t	MT20	3.0	4.0		
D	TMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 920 (0.08")

CSI: TC=0.25/1.00 (D-I:1), BC=0.22/1.00 (G-H:1),
WB=0.05/1.00 (D-G:1), SSI=0.30/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

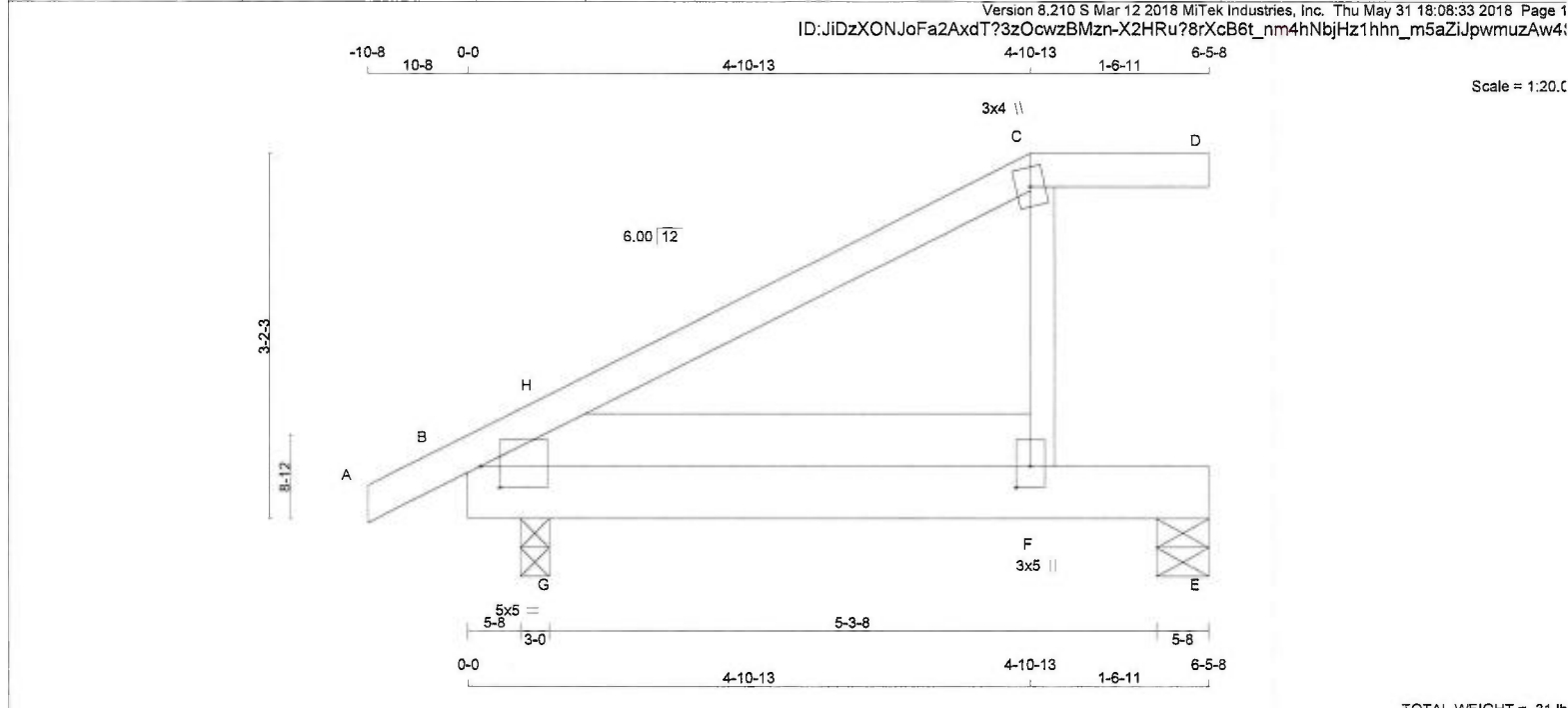
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.**





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - D 2x4 DRY No.2 SPF B - E 2x6 DRY No.2 SPF REINFORCING MEMBERS HW1 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.							DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th>JT</th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr><tr><th>E</th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th></tr><tr><td>E</td><td>294</td><td>0</td><td>294</td><td>0</td></tr><tr><td>B</td><td>396</td><td>0</td><td>396</td><td>0</td></tr></table> UNFACTORED REACTIONS <table><tr><th>JT</th><th>1ST LCASE COMBINED</th><th>MAX./MIN. SNOW</th><th>LIVE</th><th>PERM. LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>207</td><td>142 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>65 / 0</td><td>0 / 0</td></tr><tr><td>B</td><td>276</td><td>203 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>73 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>LC1 MAX CSI (LC)</th><th>MAX. UNBRAC LENGTH</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRAC LENGTH</th></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 5</td><td>-77.4 -77.4</td><td>0.05 (1)</td><td>10.00</td><td>F-C</td><td>-248 / 0</td><td>0.04 (1)</td></tr><tr><td>B-H</td><td>-719 / 0</td><td>-77.4 -77.4</td><td>0.16 (1)</td><td>6.25</td><td>F-H</td><td>-598 / 0</td><td>0.10 (1)</td></tr><tr><td>H-C</td><td>-23 / 0</td><td>-77.4 -77.4</td><td>0.16 (1)</td><td>6.25</td><td>G-H</td><td>0 / 291</td><td>0.00 (1)</td></tr><tr><td>C-D</td><td>0 / 0</td><td>-77.4 -77.4</td><td>0.13 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>B-G</td><td>0 / 599</td><td>-18.2 -18.2</td><td>0.14 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>G-F</td><td>0 / 599</td><td>-18.2 -18.2</td><td>0.34 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>F-E</td><td>0 / 0</td><td>-18.2 -18.2</td><td>0.27 (1)</td><td>10.00</td><td></td><td></td><td></td></tr></table>										JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	E	VERT	HORZ	DOWN	HORZ	E	294	0	294	0	B	396	0	396	0	JT	1ST LCASE COMBINED	MAX./MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	E	207	142 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0	B	276	203 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	FR-TO		FROM TO			FR-TO			A-B	0 / 5	-77.4 -77.4	0.05 (1)	10.00	F-C	-248 / 0	0.04 (1)	B-H	-719 / 0	-77.4 -77.4	0.16 (1)	6.25	F-H	-598 / 0	0.10 (1)	H-C	-23 / 0	-77.4 -77.4	0.16 (1)	6.25	G-H	0 / 291	0.00 (1)	C-D	0 / 0	-77.4 -77.4	0.13 (1)	10.00				B-G	0 / 599	-18.2 -18.2	0.14 (1)	10.00				G-F	0 / 599	-18.2 -18.2	0.34 (1)	10.00				F-E	0 / 0	-18.2 -18.2	0.27 (1)	10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 23.3 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 33.6 PSF SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.22") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.22") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06") CSI: TC=0.16/1.00 (B-H:1) , BC=0.34/1.00 (F-G:1) , WB=0.10/1.00 (F-H:1) , SSI=0.18/1.00 (E-F:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES				
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG																																																																																																																																													
E	VERT	HORZ	DOWN	HORZ																																																																																																																																													
E	294	0	294	0																																																																																																																																													
B	396	0	396	0																																																																																																																																													
JT	1ST LCASE COMBINED	MAX./MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL																																																																																																																																										
E	207	142 / 0	0 / 0	0 / 0	0 / 0	65 / 0	0 / 0																																																																																																																																										
B	276	203 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0																																																																																																																																										
CHORDS				WEBS																																																																																																																																													
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH																																																																																																																																										
FR-TO		FROM TO			FR-TO																																																																																																																																												
A-B	0 / 5	-77.4 -77.4	0.05 (1)	10.00	F-C	-248 / 0	0.04 (1)																																																																																																																																										
B-H	-719 / 0	-77.4 -77.4	0.16 (1)	6.25	F-H	-598 / 0	0.10 (1)																																																																																																																																										
H-C	-23 / 0	-77.4 -77.4	0.16 (1)	6.25	G-H	0 / 291	0.00 (1)																																																																																																																																										
C-D	0 / 0	-77.4 -77.4	0.13 (1)	10.00																																																																																																																																													
B-G	0 / 599	-18.2 -18.2	0.14 (1)	10.00																																																																																																																																													
G-F	0 / 599	-18.2 -18.2	0.34 (1)	10.00																																																																																																																																													
F-E	0 / 0	-18.2 -18.2	0.27 (1)	10.00																																																																																																																																													

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



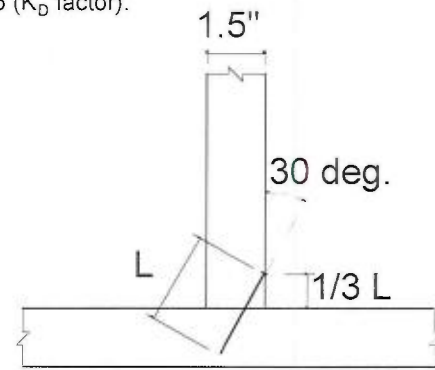
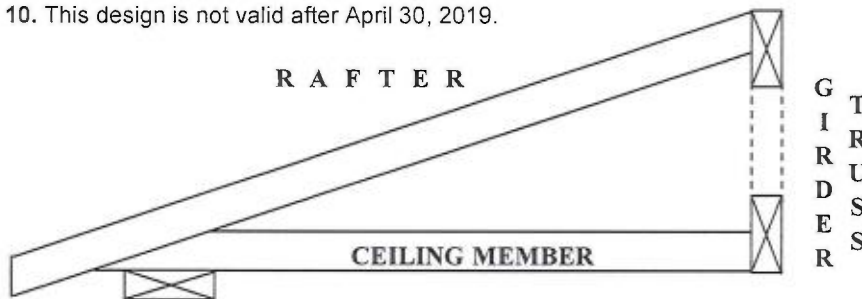
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

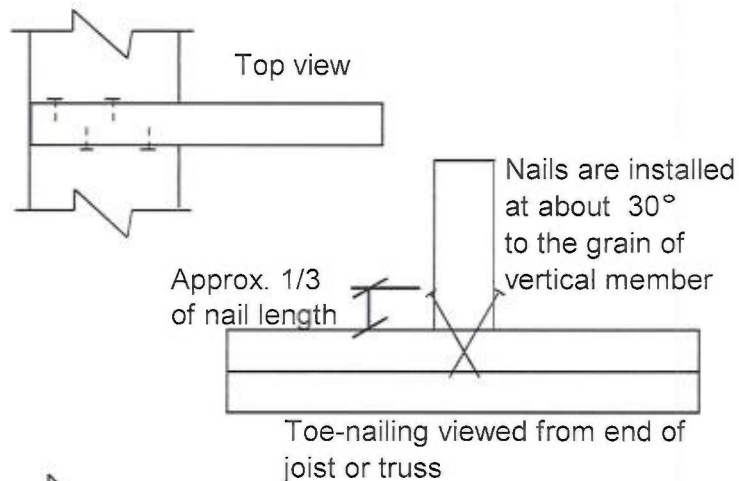
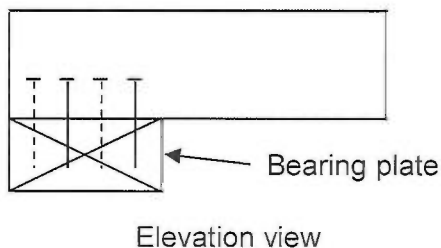
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

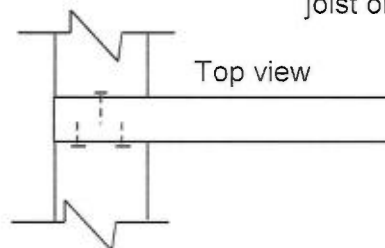
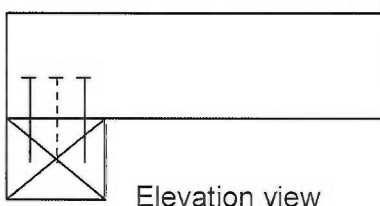
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek Canada Inc
100 Industrial Rd
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017