

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0618-001
GREEN YORK HOMES - GRANELLI
HOME CORP - LIANA 2 ELEV 1

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

*For conventional framing roof / ceilings
framing shall conform to OBC.9.23*

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

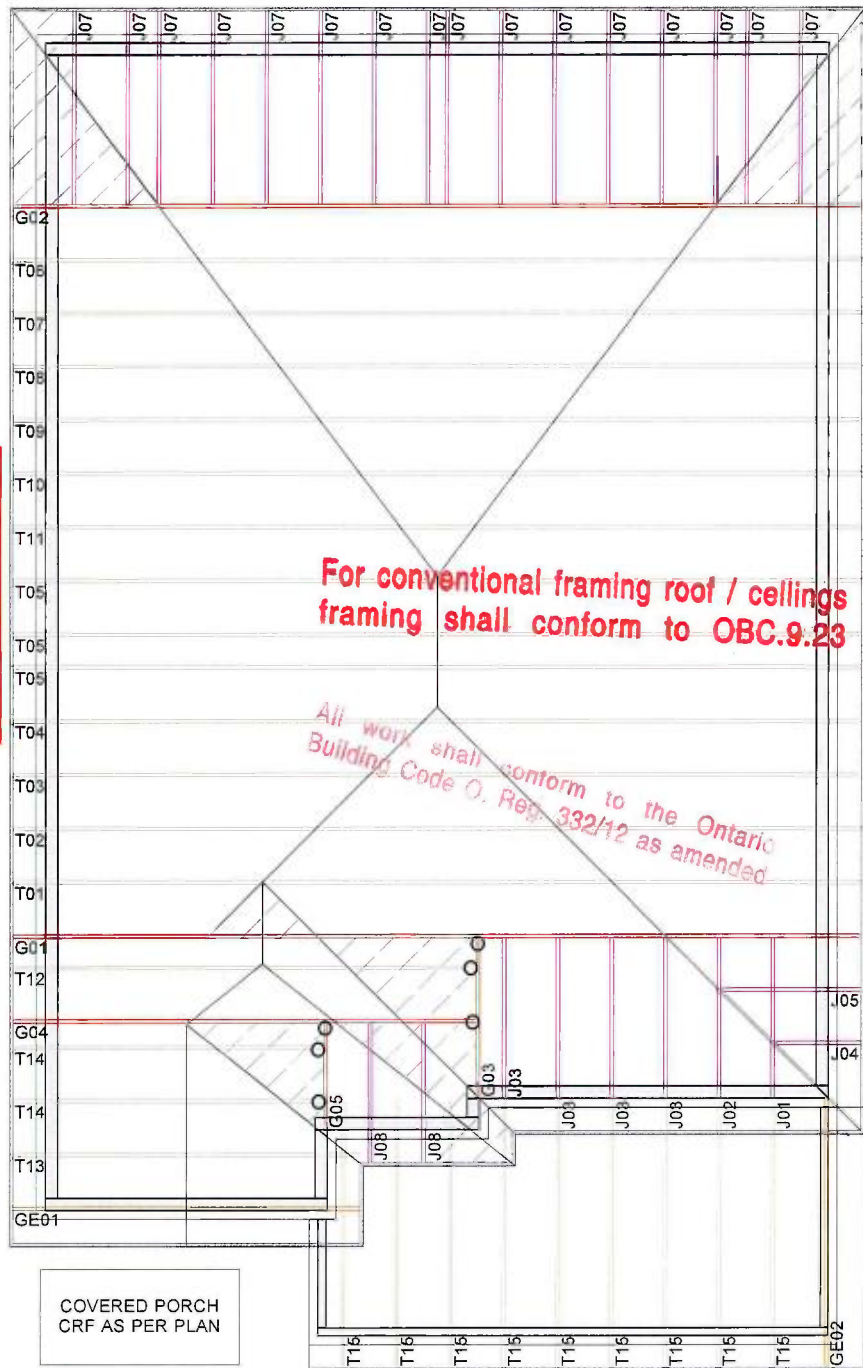
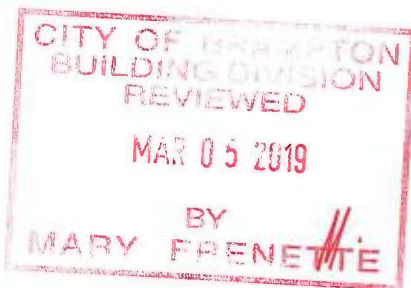
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

Layout based on plans provided by
Jardin Design Group Inc.
Revision #2 dated May 22, 2018

THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.



ENG.JOB:NE0618-001



ROOF TRUSS LAYOUT

19-444490.000.00.RR.



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

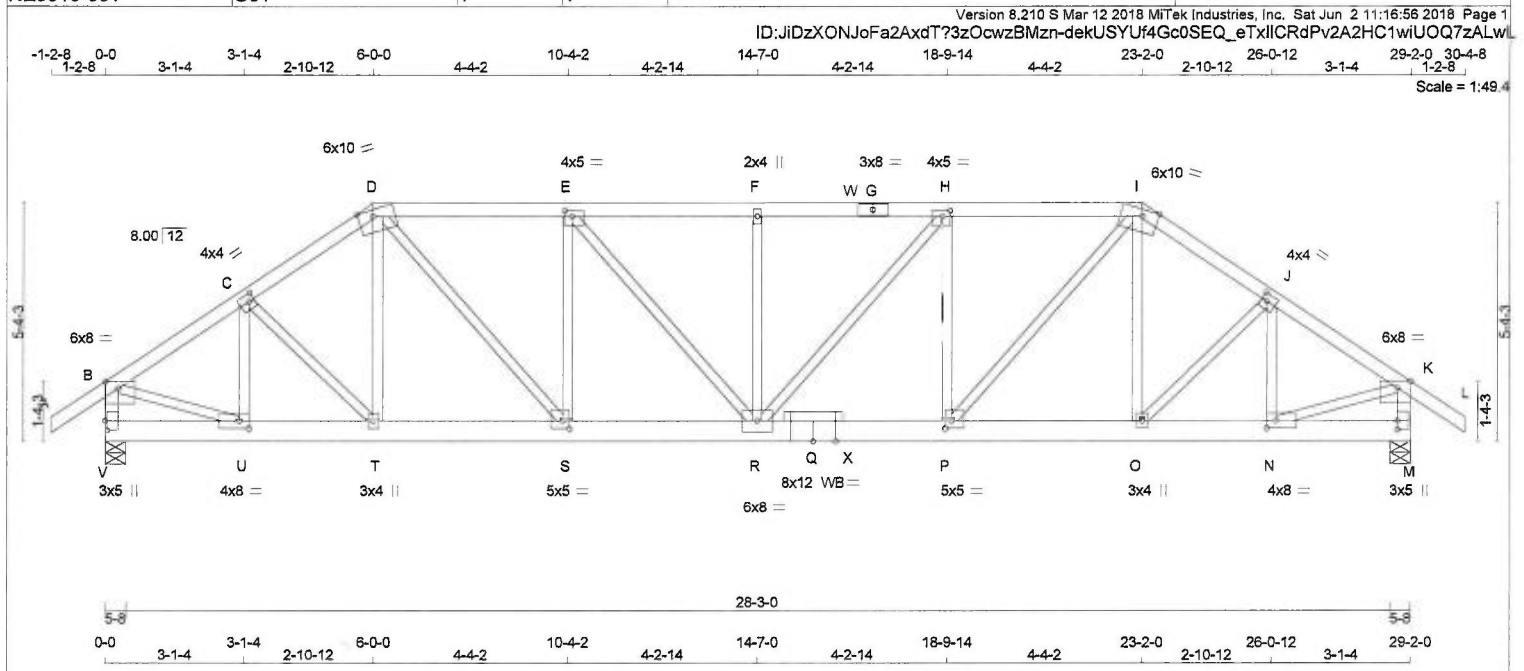
ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE TRUSS BELOW EA CROSS PT. VERT POS. THAN 6" TO HAVE A LATERAL BRACING SO THAT THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6"

DESIGN CRITERIA
SPECIFIED LOADS
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCSC 2012, ABC 2012, CSA 086-09, TPIC 2011
ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	LIANA 2 ELE 1
Customer:	GREEN YORK HOMES
Project:	GRANELLI HOME
Location:	BRAMPTON, ON
Drawn By:	DM
Date:	5/30/2018



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
V - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
V - Q	2x6	DRY	2100F 1.8E	SPF	
Q - M	2x6	DRY	2100F 1.8E	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	8.0	Edge	3.50	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50	
D	TTWW-m	MT20	6.0	10.0	Edge	4.25	
E	TMVW-t	MT20	4.0	5.0	1.50	2.00	
F	TMVW-w	MT20	2.0	4.0			
G	TS-t	MT20	3.0	8.0			
H	TMVW-t	MT20	4.0	5.0	1.50	2.00	
I	TTWW-m	MT20	6.0	10.0	Edge	4.25	
J	TMVW-t	MT20	4.0	4.0	2.00	1.50	
K	TMVW-p	MT20	6.0	8.0	Edge	3.50	
N	BMV1-p	MT20	3.0	5.0	2.50		
M	BMVW-t	MT20	4.0	8.0	2.00	2.50	
O	BMVW-t	MT20	3.0	4.0			
P	BMVW-t	MT20	5.0	5.0	2.00	1.75	
Q	BS-t	MT20	8.0	12.0			
R	BMVW-t	MT20	6.0	8.0			
S	BMVW-t	MT20	5.0	5.0	2.00	1.75	
T	BMVW-t	MT20	3.0	4.0			
U	BMVW-t	MT20	4.0	8.0	2.00	2.50	
V	BMV1-p	MT20	3.0	5.0	2.50	0.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
V	2442	0	2442	0	0	0	5-8	5-8	5-8
M	3055	0	3055	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN
V	1713	1205 / 0	0 / 0	0 / 0	0 / 0	508 / 0	0 / 0	V	2442	0	2442	0	0
M	2146	1489 / 0	0 / 0	0 / 0	0 / 0	657 / 0	0 / 0	M	3055	0	3055	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	UNBRAC
FR-TO		FROM	TO	CSI (LC)	LENGTH	FR-TO		FROM	TO
A-B	0 / 28	-77.4	-77.4	0.10 (1)	10.00	U-C	-741 / 0	0.15 (1)	
B-C	-2682 / 0	-77.4	-77.4	0.20 (1)	4.04	C-T	0 / 384	0.10 (1)	
C-D	-3038 / 0	-77.4	-77.4	0.23 (1)	3.80	T-D	-79 / 26	0.03 (1)	
D-E	-3848 / 0	-77.4	-77.4	0.47 (1)	3.17	D-S	0 / 2031	0.50 (1)	
E-F	-4907 / 0	-77.4	-77.4	0.62 (1)	2.66	S-E	-1610 / 0	0.66 (1)	
F-G	-4907 / 0	-77.4	-77.4	0.83 (1)	2.33	E-R	0 / 1632	0.40 (1)	
G-H	-4907 / 0	-145.9	-145.9	0.83 (1)	2.33	R-F	-305 / 0	0.13 (1)	
H-I	-4621 / 0	-145.9	-145.9	0.79 (1)	2.48	F-H	0 / 441	0.11 (1)	
I-J	-3838 / 0	-77.4	-77.4	0.29 (1)	3.34	P-I	0 / 2199	0.54 (1)	
J-K	-3410 / 0	-77.4	-77.4	0.27 (1)	3.56	O-I	-234 / 52	0.10 (1)	
K-L	0 / 28	-77.4	-77.4	0.10 (1)	10.00	O-J	0 / 468	0.12 (1)	
L-M	-2375 / 0	0.0	0.0	0.26 (1)	5.49	N-J	-839 / 0	0.17 (1)	
M-K	-2965 / 0	0.0	0.0	0.33 (1)	4.94	B-U	0 / 2342	0.58 (1)	
						N-K	0 / 2975	0.74 (1)	
V-U	0 / 0	-18.2	-18.2	0.05 (1)	10.00				
U-T	0 / 2241	-18.2	-18.2	0.17 (1)	10.00				
T-S	0 / 2510	-18.2	-18.2	0.18 (1)	10.00				
S-R	0 / 3849	-18.2	-18.2	0.34 (1)	10.00				
R-Q	0 / 4621	-18.2	-18.2	0.66 (1)	10.00				
Q-X	0 / 4621	-18.2	-18.2	0.66 (1)	10.00				
X-P	0 / 4621	-34.4	-34.4	0.66 (1)	10.00				
P-O	0 / 3172	-34.4	-34.4	0.28 (1)	10.00				
O-N	0 / 2846	-34.4	-34.4	0.20 (1)	10.00				
N-M	0 / 0	-34.4	-34.4	0.05 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)		LOC.		MAX-		MAX+		FACE		DIR.		TYPE	
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	JT	LOC.	LC1	MAX-	MAX+	TYPE
I	23-2-0	-348	-348	---	FRONT	VERT	TOTAL	Q	16-1-4	-1514	-1514	---	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6'-0"
END SETBACK = 6'-0"
END WALL WIDTH = 5'-8"
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL
LOADS APPLIED TO FIRST 12'-6" OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/977 (0.36")

CSI: TC=0.83/1.00 (F-H:1), BC=0.66/1.00 (P-R:1), WB=0.74/1.00 (K-N:1), SS=0.75/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 348.1 lbs FACTORED DOWN AT 23-2-0 ON TOP CHORD, AND 1514.4 lbs FACTORED DOWN AT 16-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

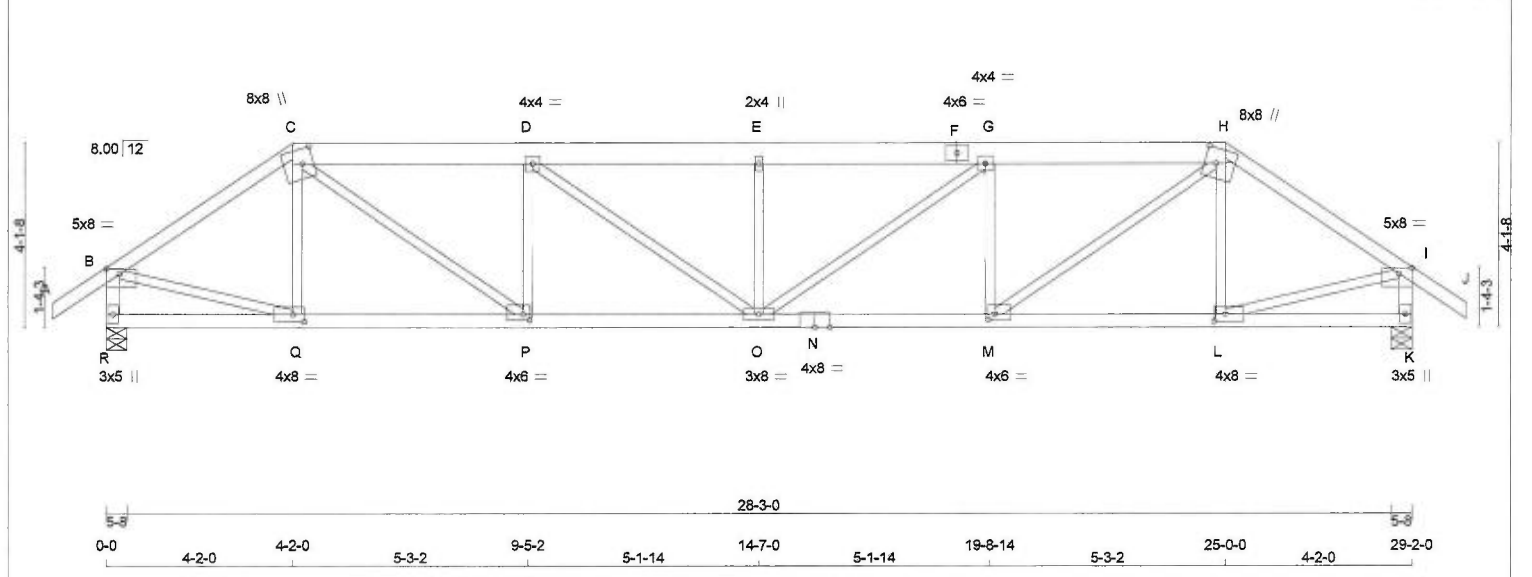
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
 JSI METAL= 0.95 (Q) (INPUT = 1.00)

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Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:16:57 2018 Page 1
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-6qlsguVHrZkt3O?ABATXHQ_t_JKRnIvA9MExyZzALwK
Scale = 1:49.4



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	1.50	3.50
C	TTWW+m	MT20	8.0	8.0	4.00	3.00
D	TMVW-t	MT20	4.0	4.0		
E	TMVW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	8.0	8.0	4.00	3.00
I	TMVW+p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	3.0	5.0		
L	BMVW-t	MT20	4.0	8.0	2.00	3.00
M	BMVW-t	MT20	4.0	6.0	1.50	1.75
N	BS-t	MT20	4.0	8.0		
O	BMVWWW-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	6.0	1.50	1.75
Q	BMVW-t	MT20	4.0	8.0	2.00	3.00
R	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 241.8 lbs FACTORED DOWN AT 25'-0, AND 241.8 lbs FACTORED DOWN AT 4'-2-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
R	2685	0	2685	0
K	2685	0	2685	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
R	1885 1314 / 0
K	1885 1314 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.10 (1)	10.00	Q-C	-449 / 13	0.11 (1)
B-C	-3149 / 0	-77.4 -77.4	0.46 (1)	3.52	C-P	0 / 2539	0.63 (1)
C-D	-4678 / 0	-145.9 -145.9	0.43 (1)	3.63	P-D	-1309 / 0	0.33 (1)
D-E	-5289 / 0	-145.9 -145.9	0.43 (1)	3.44	D-O	0 / 757	0.19 (1)
E-F	-5289 / 0	-145.9 -145.9	0.43 (1)	3.44	O-E	-711 / 0	0.18 (1)
F-G	-5289 / 0	-145.9 -145.9	0.43 (1)	3.44	O-G	0 / 757	0.19 (1)
G-H	-4678 / 0	-145.9 -145.9	0.43 (1)	3.63	M-G	-1309 / 0	0.33 (1)
H-I	-3149 / 0	-77.4 -77.4	0.46 (1)	3.52	M-H	0 / 2539	0.63 (1)
I-J	0 / 28	-77.4 -77.4	0.10 (1)	10.00	L-H	-449 / 13	0.11 (1)
R-B	-2628 / 0	0.0 0.0	0.30 (1)	5.23	B-Q	0 / 2703	0.67 (1)
K-I	-2628 / 0	0.0 0.0	0.30 (1)	5.23	L-I	0 / 2703	0.67 (1)
R-Q	0 / 0	-34.4 -34.4	0.19 (4)	10.00			
Q-P	0 / 2616	-34.4 -34.4	0.57 (1)	10.00			
P-O	0 / 4678	-34.4 -34.4	0.92 (1)	10.00			
O-N	0 / 4678	-34.4 -34.4	0.92 (1)	10.00			
N-M	0 / 4678	-34.4 -34.4	0.92 (1)	10.00			
M-L	0 / 2616	-34.4 -34.4	0.57 (1)	10.00			
L-K	0 / 0	-34.4 -34.4	0.19 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	4-2-0	-242	-242	—	FRONT	VERT	TOTAL
H	25-0-0	-242	-242	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4'-2-0
END SETBACK = 6'-0-0
END WALL WIDTH = 5'-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADTTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/852 (0.41")

CSI: TC=0.46/1.00 (B-C:1), BC=0.92/1.00 (O-P:1), WB=0.67/1.00 (B-Q:1), SS=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

CONTINUED ON PAGE 2



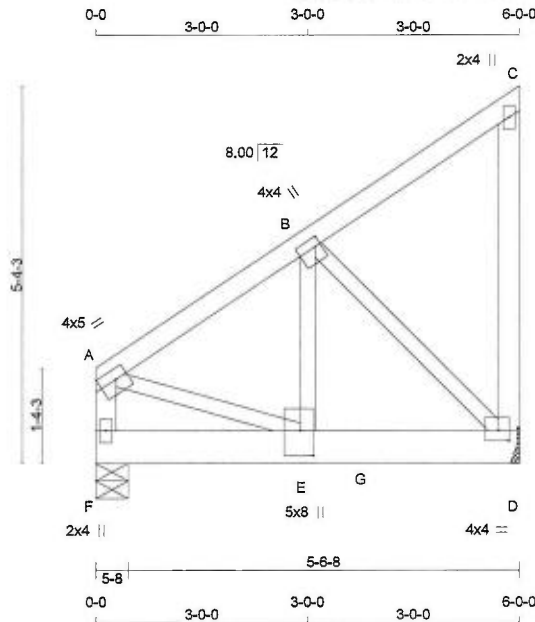
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-001	G02	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 2 ELEV 1	PAGE 6 OF 38
				TRUSS DESC.	

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:16:57 2018 Page 2
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-6qlsguVHrZkt3O?ABATXHQ_t_JKRnIVA9MExyZzALwK

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.90 (N) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:31.4

TOTAL WEIGHT = 33 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+t	MT20	4.0	5.0	1.75 Edge
B	TMVW+t	MT20	4.0	4.0	1.50 1.00
C	TMV+p	MT20	2.0	4.0	
D	BMVW+t	MT20	4.0	4.0	1.75 1.75
E	BMVW+t	MT20	5.0	8.0	4.25 2.25
F	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1300.9 lbs FACTORED DOWN AT 2-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX IN-SX
D	1514	0	1514	0	0	MECHANICAL
F	1064	0	1064	0	0	5-8 5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	DOWN	UP	DOWN	UP	DOWN	UP	DOWN	UP	DOWN	UP	DOWN	UP	DOWN	UP
D	1064	737 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	326 / 0	0 / 0	0 / 0	0 / 0
F	748	518 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED		FACTORED VERT. LOAD		MAX. UNBRACED LENGTH		WEBS MAX. FACTORED	
	FORCE (LBS)	PLF	LC1	MAX	FR-TO	TO	FORCE (LBS)	MAX
A-B	-1147 / 0	-77.4	-77.4	0.12 (1)	5.78	E-B	0 / 1312	0.32 (1)
B-C	-13 / 0	-77.4	-77.4	0.10 (1)	6.25	B-D	-1352 / 0	0.39 (1)
D-C	-93 / 0	0.0	0.0	0.04 (1)	7.81	A-E	0 / 1012	0.25 (1)
F-A	-1040 / 0	0.0	0.0	0.12 (1)	7.88			
F-E	0 / 0	-18.2	-18.2	0.02 (4)	10.00			
E-G	0 / 965	-18.2	-18.2	0.37 (1)	10.00			
G-D	0 / 965	-342.9	-342.9	0.37 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	2-10-12	-1301	-1301	-	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD =	33.6	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 3-10-0
START SPAN CARRIED = 16-0-8
END DISTANCE = 6-0-0
END SPAN CARRIED = 16-0-8
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.12/1.00 (A-B:1), BC=0.37/1.00 (D-E:1),
WB=0.39/1.00 (B-D:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

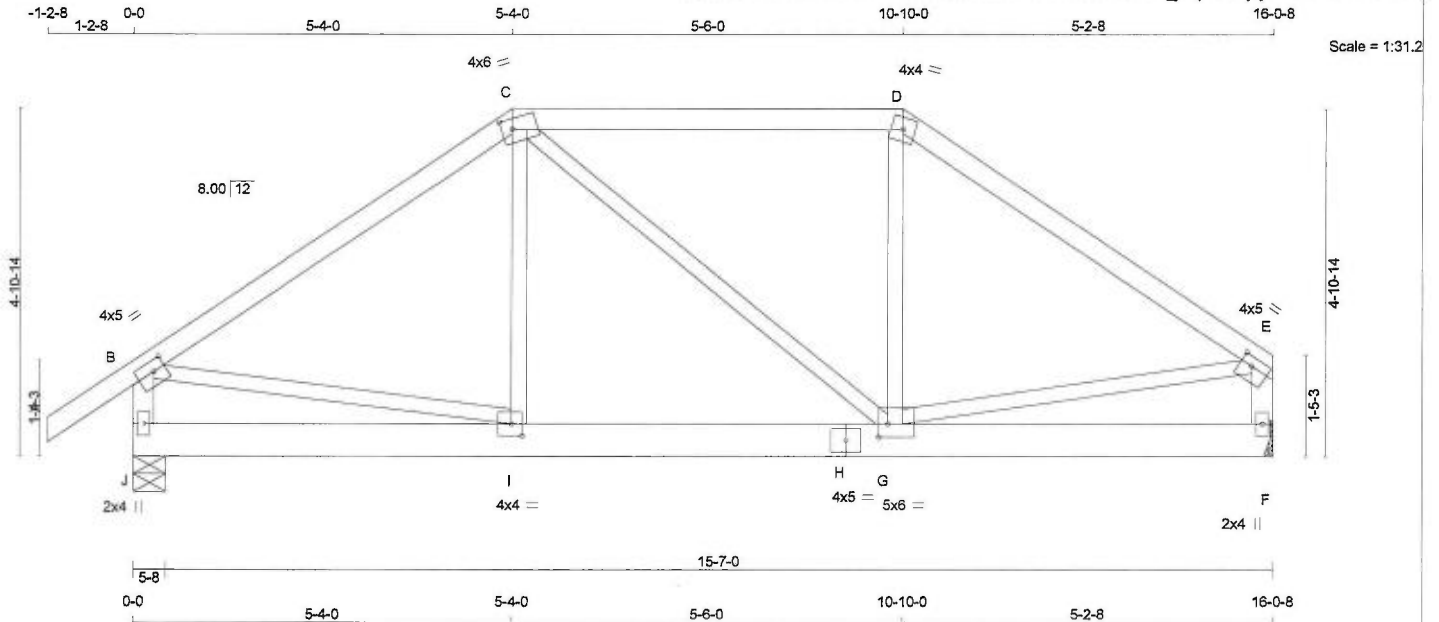
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:31.2

TOTAL WEIGHT = 74 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
J - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - I	2x6	DRY No.2	SPF
I - J	2x6	DRY No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	5.0	1.75	2.00
C TTVW-m	MT20	4.0	6.0	1.75	1.75
D TTVW-m	MT20	4.0	4.0		
E TMVW-t	MT20	4.0	5.0	1.50	2.00
F BMV1+p	MT20	2.0	4.0		
G BMVW-w	MT20	5.0	6.0	2.25	1.50
H BS-t	MT20	4.0	5.0		
I BMVW-t	MT20	4.0	4.0	2.00	1.75
J BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 167.9 lbs FACTORED DOWN AT 10-10-0 ON TOP CHORD, AND 585.0 lbs FACTORED DOWN AT 10-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1124	0	1124	0	0	5-8	5-8
F	1300	0	1300	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	787	562 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
F	914	631 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-77.4 -77.4	0.10 (1)	10.00	I-C	-50 / 68	0.03 (4)
B-C	-1107 / 0	-77.4 -77.4	0.48 (1)	5.34	C-G	0 / 381	0.09 (1)
C-D	-1209 / 0	-77.4 -77.4	0.50 (1)	5.10	G-D	0 / 222	0.07 (4)
D-E	-1444 / 0	-77.4 -77.4	0.48 (1)	4.82	B-I	0 / 934	0.23 (1)
J-B	-1082 / 0	0.0 0.0	0.12 (1)	7.56	G-E	0 / 1222	0.30 (1)
F-E	-1234 / 0	0.0 0.0	0.14 (1)	7.19			
J-I	0 / 0	-18.2 -18.2	0.06 (4)	10.00			
I-H	0 / 919	-18.2 -18.2	0.15 (1)	10.00			
H-G	0 / 919	-18.2 -18.2	0.15 (1)	10.00			
G-F	0 / 0	-25.3 -25.3	0.09 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	10-10-0	-168	-168	---	FRONT	VERT	TOTAL
G	10-5-4	-585	-585	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 33.6	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 5-4-0
RIGHT SETBACK = 5-2-8
END SETBACK = 4-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 5-2-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.50/1.00 (C-D:1), BC=0.15/1.00 (G-I:1), WB=0.30/1.00 (E-G:1), SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

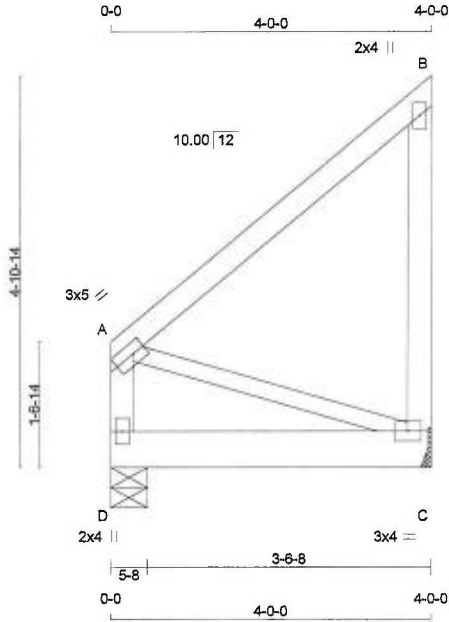
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)

JSI METAL= 0.47 (E) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 22 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
C - B	2x4	DRY	No.2
D - A	2x4	DRY	No.2
D - C	2x6	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-4	MT20	3.0	5.0	1.50	1.75
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-1	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	DESCR.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	MECHANICAL
C	SPF	586	0	586	0	0	5-8	5-8	
D	SPF	586	0	586	0	0			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	COMBINED	285 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
D	COMBINED	285 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
MEMB.	CHORDS		FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	WEBS
	MAX. FACTORED FORCE (LBS)	FACTORED FORCE (LBS)					MAX. FACTORED FORCE (LBS)
A-B	0 / 0	-77.4	-77.4	0.23 (1)	10.00	A-C	0 / 0
C-B	-155 / 0	0.0	0.0	0.06 (1)	7.81		
D-A	-155 / 0	0.0	0.0	0.02 (1)	7.81		
D-C	0 / 0	-215.4	-215.4	0.32 (1)	10.00		

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.3 PSF
TOTAL LOAD	=	33.6	PSF

SPACING = 24.0 IN. G/C

GIRDER TYPE: CStdGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 10-4-8
 END DISTANCE = 4-0-0
 END SPAN CARRIED = 10-4-8
 END WALL WIDTH = 1-8
 APPLIED TO BACK SIDE OF BOTTOM CHORD.
 - ADDTL LOADS BASED ON 55 % OF GSL.
 (DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.23/1.00 (A-B:1), BC=0.32/1.00 (C-D:1),
 WB=0.00/1.00 (A-C:1), SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



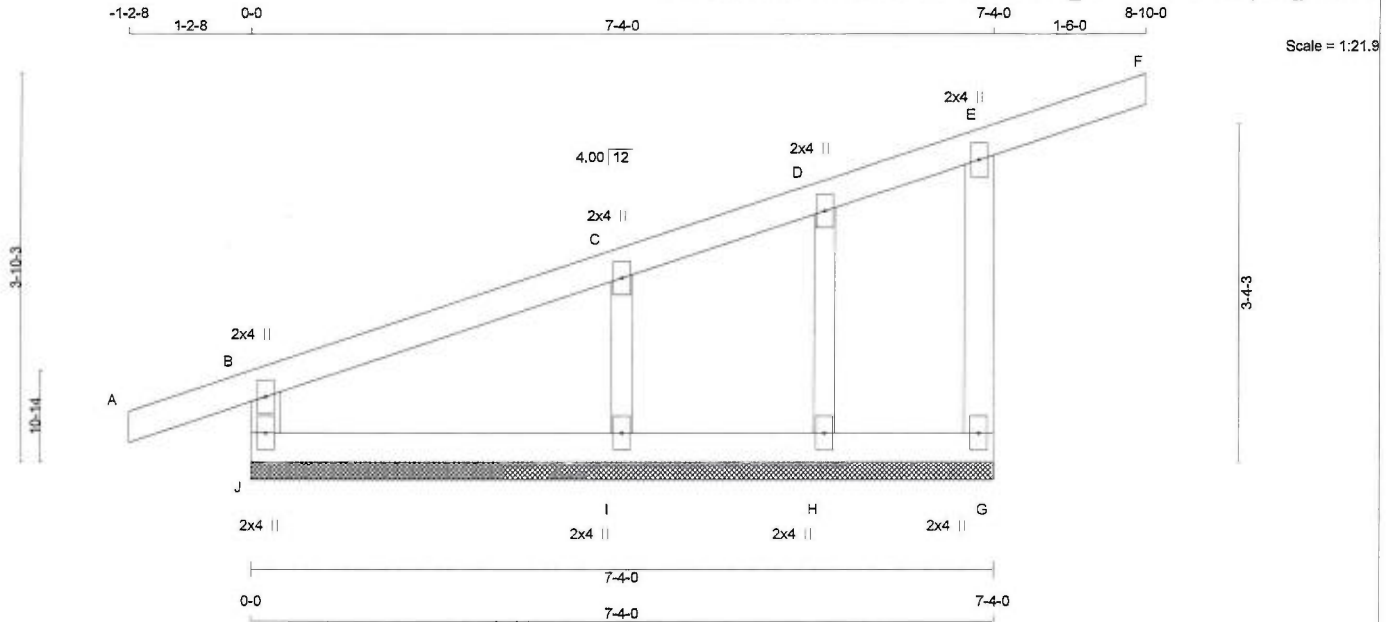
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0618-001	G05	1	1	GREEN YORK HOMES - GRANELLI HOME CORP - LIANA 2 ELEV 1	PAGE 12 OF 38
TRUSS DESC.					

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ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-a0sEtEWvctskhYaMlt_mqdW6Gjq3WMBKO0zVU?zALw.

JSI GRIP= 0.15 (C) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER
J - B	2x4 DRY	No.2
A - F	2x4 DRY	No.2
G - E	2x4 DRY	No.2
J - G	2x4 DRY	No.2

ALL WEBS	2x3 DRY	No.2
ALL GABLE WEBS	2x3 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	2.0	4.0		
G	BMV1+p	MT20	2.0	4.0		
H	BMV1+w	MT20	2.0	4.0		
I	BMV1+w	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
J-B	-245 / 0	0.0	0.0 0.03 (4)	7.81	I-C	-236 / 0	0.04 (1)
A-B	0 / 15	-77.4	-77.4 0.09 (1)	10.00	H-D	-78 / 0	0.01 (1)
B-C	-1 / 0	-77.4	-77.4 0.13 (1)	10.00			
C-D	-1 / 0	-77.4	-77.4 0.09 (1)	10.00			
D-E	0 / 23	-77.4	-77.4 0.09 (1)	10.00			
E-F	-19 / 0	-77.4	-77.4 0.13 (1)	6.25			
G-E	-225 / 0	0.0	0.0 0.04 (1)	7.81			
J-I	-3 / 1	-18.2	-18.2 0.04 (4)	10.00			
I-H	-9 / 0	-18.2	-18.2 0.04 (4)	10.00			
H-G	-11 / 0	-18.2	-18.2 0.01 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-C:1), BC=0.04/1.00 (I-J:4), WB=0.04/1.00 (C-I:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1867

PLATE PLACEMENT TOL. = 0.250 inches

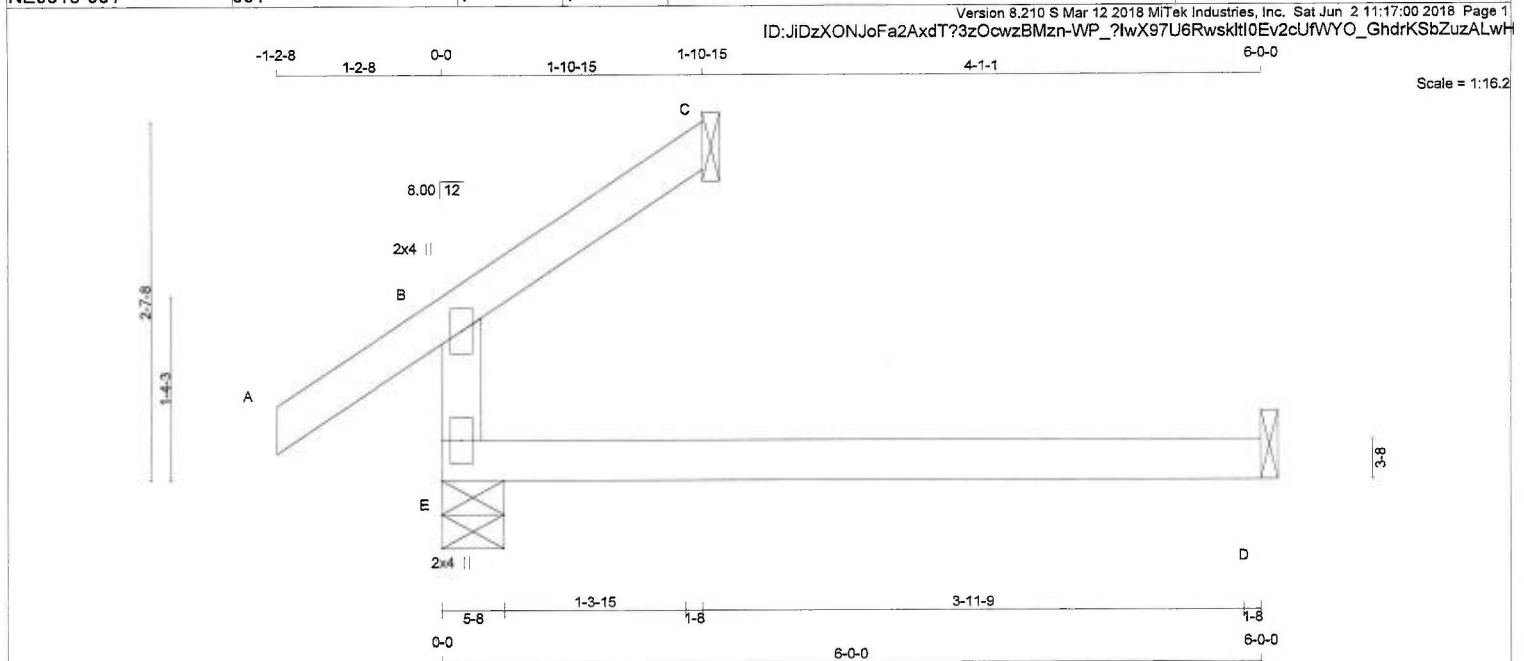
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Sat Jun 2 11:17:00 2018 Page 1
ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-WP_?lwX97U6Rwsktl0Ev2cUfWYO_GhdrKSbZuzALwh

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	SPECIFIED LOADS:	
E - B	2x4	DRY	No.2	SPF	JT	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	23.3 PSF
A - C	2x4	DRY	No.2	SPF	E	VERT	DOWN	UP	IN-SX	DL =	3.0 PSF
E - D	2x4	DRY	No.2	SPF	C	HORZ	UPLIFT	IN-SX	IN-SX	BOT CH. LL =	0.0 PSF
DRY: SEASONED LUMBER.					D					DL =	7.3 PSF
										TOTAL LOAD =	33.6 PSF

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

PLATES (table is in inches)						SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010										
JT	TYPE	PLATES	W	LEN	Y	X	UNFACTORED REACTIONS										THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09									
B	TMV+p	MT20	2.0	4.0			1ST LCASE MAX./MIN. COMPONENT REACTIONS																			
E	BMV1+p	MT20	2.0	4.0			JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL												
							E	181	115 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0												
							C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0												
							D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0												

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD
	FORCE (LBS)	FORCE (LBS)	LC1 MAX (LC)		FORCE (LBS)	FORCE (LBS)	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
E-B	-192 / 0	0.0	0.0 0.11 (4)	E-B	-192 / 0	0.0	0.0 0.11 (4)
A-B	0 / 28	-77.4	-77.4 0.09 (1)	A-B	0 / 28	-77.4	-77.4 0.09 (1)
B-C	-10 / 0	-77.4	-77.4 0.04 (1)	B-C	-10 / 0	-77.4	-77.4 0.04 (1)
E-D	0 / 0	-18.2	-18.2 0.13 (4)	E-D	0 / 0	-18.2	-18.2 0.13 (4)

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.11/1.00 (B-E:4), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (D-E:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

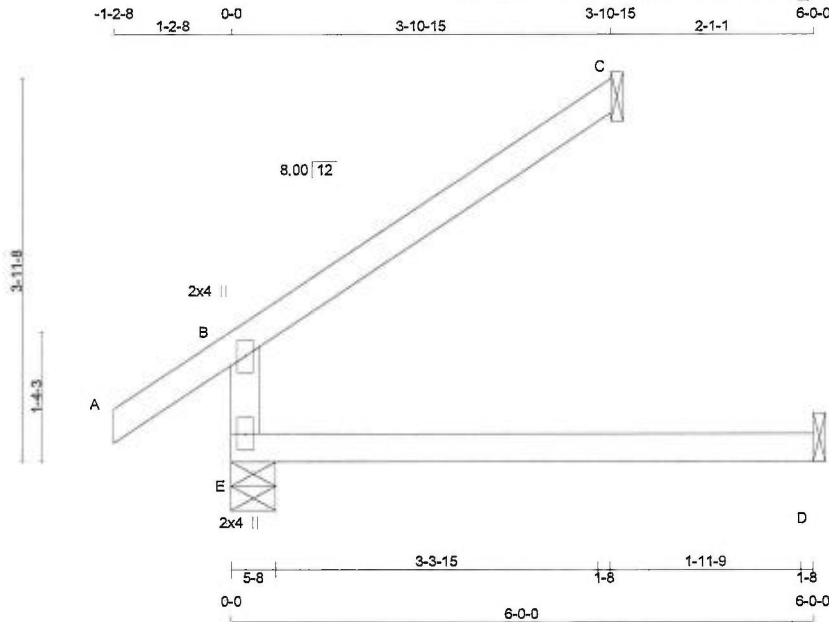
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:22.8

TOTAL WEIGHT = 15 lb
[MIF]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2

DESCR.
SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 2.0 4.0
E BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	353	0	353	0	0	5-8	5-8	
C	114	0	114	0	0	1-8	1-8	
D	46	0	51	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
	COMBINED	SNOW	LIVE									
E	247	174 / 0	0 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0				
C	77	68 / 0	0 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0				
D	37	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	LC1 MAX (LC)	UNBRAC LENGTH
FR-TO		FROM	TO		FR-TO		FROM	TO
E-B	-289 / 0	0.0	0.0 0.11 (4)	7.81				
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00				
B-C	-21 / 0	-77.4	-77.4 0.15 (1)	6.25				
E-D	0 / 0	-18.2	-18.2 0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 986-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.15/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

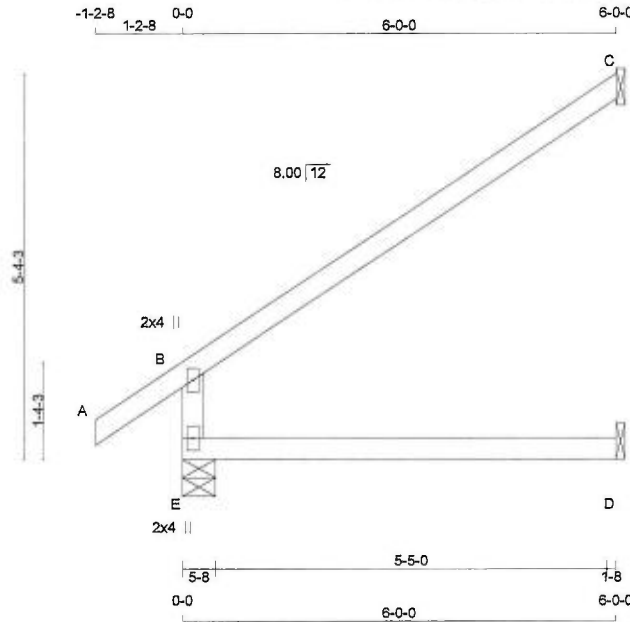
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:30.7

TOTAL WEIGHT = 4 X 18 = 72 lb [M][F]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	454	0	454	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	46	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	DOWN				
E	316	235 / 0	0 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	FORCE (LBS)	PLF		LC1 MAX	LC2 MAX			FORCE (LBS)	MAX. FACTORED
FR-TO		FROM TO							
E-B	-390 / 0	0.0	0.0	0.11 (4)	7.81				
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00				
B-C	-32 / 0	-77.4	-77.4	0.36 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	33.6	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.36/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

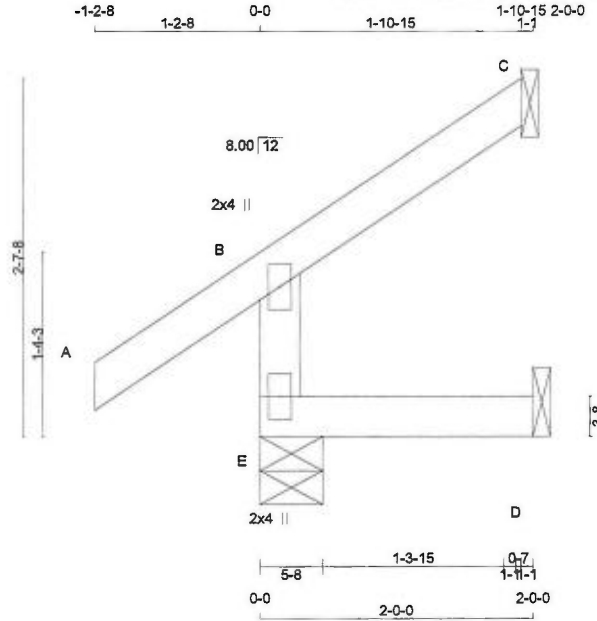
JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



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ID:JiDzXONJoFa2AxdT?3zOcwzBMzn-So6ljcZQf6M9A9t8_j2i_ThqRKfHs9BvJexidmzALwF



Scale = 1:16.2

TOTAL WEIGHT = 8 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX	BRG	IN-SX
E	212	0	212	0	0	5-8	5-8	5-8	5-8	5-8	5-8
C	56	0	56	0	0	1-8	1-8	1-8	1-8	1-8	1-8
D	16	0	16	0	0	1-8	1-8	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	BRG	IN-SX	BRG	IN-SX	BRG	IN-SX
E	146	115 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0	5-8	5-8	5-8	5-8	5-8	5-8
C	38	34 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0	1-8	1-8	1-8	1-8	1-8	1-8
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	1-8	1-8	1-8	1-8	1-8	1-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH (LC)	FR-TO
E-B	-192 / 0	0.0	0.0 0.01 (4)	E-B	7.81	10.00	7.81
A-B	0 / 28	-77.4	-77.4 0.09 (1)	A-B	10.00	10.00	10.00
B-C	-10 / 0	-77.4	-77.4 0.05 (1)	B-C	10.00	10.00	10.00
E-D	0 / 0	-18.2	-18.2 0.02 (4)	E-D	10.00	10.00	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD		33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

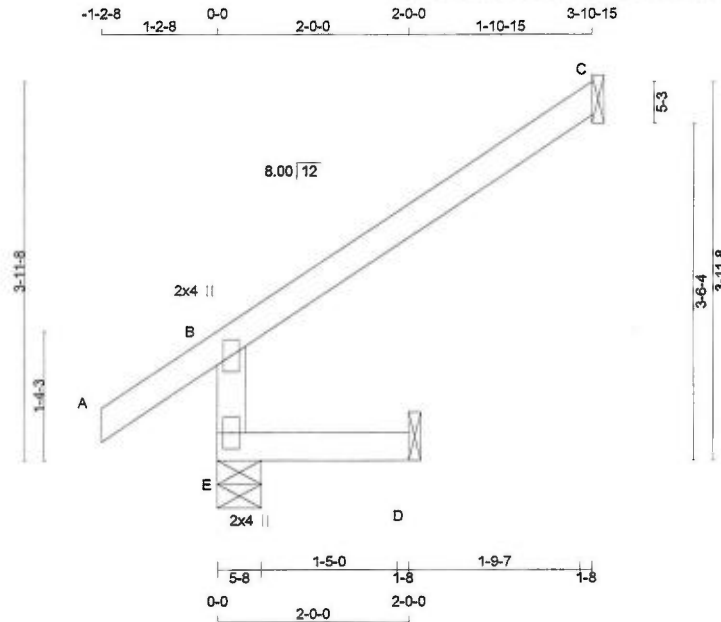
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.1

TOTAL WEIGHT = 11 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	REQRD		
GROSS REACTION		GROSS REACTION		BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	309	0	309	0	0	5-8	5-8
C	114	0	114	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	212	174 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	77	68 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
	FR-TO	FROM TO	LC1	LC2				
E-B	-289 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00			
B-C	-21 / 0	-77.4	-77.4	0.20 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

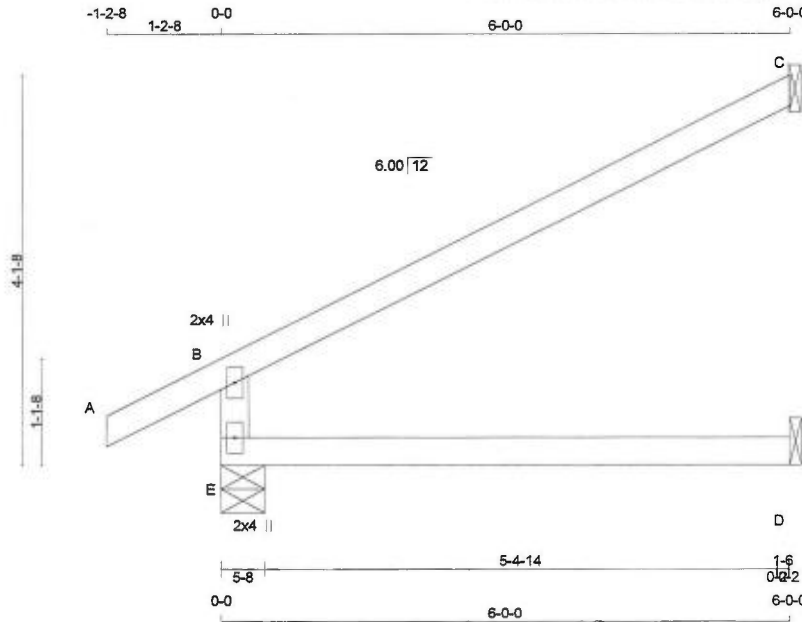
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:23.4

TOTAL WEIGHT = 16 X 17 = 271 lb
(M)(F)

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
E	453	0	453	0	0	5-8	5-8	5-8
C	174	0	174	0	0	1-8	1-8	1-8
D	45	0	50	0	0	1-8	1-8	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	316	234 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORZ. LOAD (LC1)	MAX. FORCE (LBS)	MAX. HORZ. LOAD (LC1)	MAX. VERT. LOAD (LC1)	MAX. HORZ. LOAD (LC1)
FR-TO								
E-B	-389 / 0	0.0	0.0	0.11 (4)	7.81			
A-B	0 / 22	-77.4	-77.4	0.09 (1)	10.00			
B-C	-26 / 0	-77.4	-77.4	0.36 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.38/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822

PLATE PLACEMENT TOL. = 0.250 inches

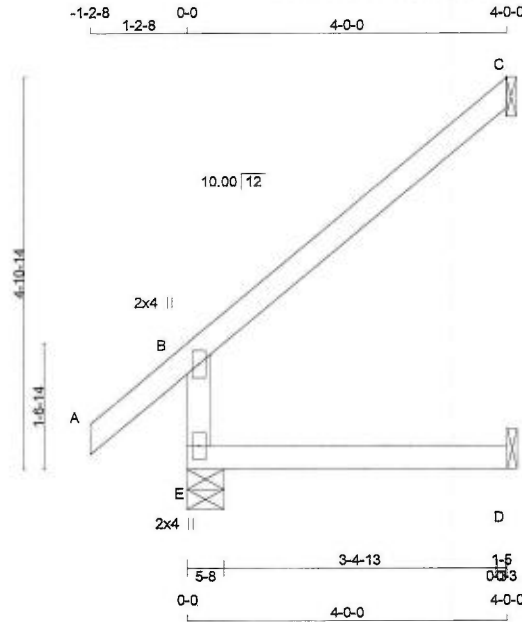
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:27.7

TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	335	0	335	0	0	5-8	5-8	
C	116	0	116	0	0	1-8	1-8	
D	32	0	36	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	233	177 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
C	79	70 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-294 / 0	0.0	0.0	0.04 (4)	7.81		
A-B	0 / 32	-77.4	-77.4	0.09 (1)	10.00		
B-C	-25 / 0	-77.4	-77.4	0.21 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (B-C:1), BC=0.06/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

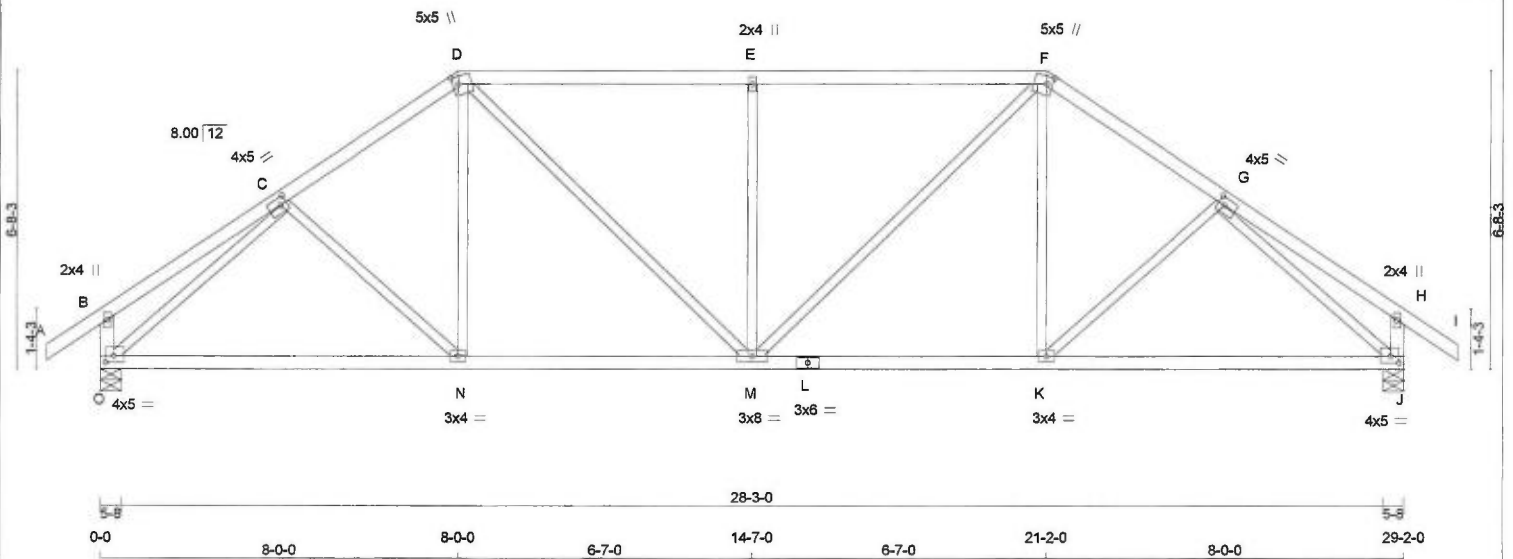
JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:48.5



TOTAL WEIGHT = 122 lb [M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
O 1494	0	1494	0	0
J 1494	0	1494	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX	CSI (LC)
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	C-N	-37 / 47	0.02 (1)		
B-C	0 / 19	-77.4	-77.4	0.19 (1)	10.00	N-D	0 / 184	0.05 (4)		
C-D	-1566 / 0	-77.4	-77.4	0.17 (1)	5.11	D-M	0 / 517	0.12 (1)		
D-E	-1659 / 0	-77.4	-77.4	0.48 (1)	4.60	M-E	-625 / 0	0.46 (1)		
E-F	-1659 / 0	-77.4	-77.4	0.48 (1)	4.60	K-F	0 / 517	0.12 (1)		
F-G	-1566 / 0	-77.4	-77.4	0.17 (1)	5.11	K-F	0 / 184	0.05 (4)		
G-H	0 / 19	-77.4	-77.4	0.19 (1)	10.00	K-G	-37 / 47	0.02 (1)		
H-I	0 / 28	-77.4	-77.4	0.09 (1)	10.00	O-C	-1788 / 0	0.86 (1)		
O-B	-221 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-1788 / 0	0.86 (1)		
J-H	-221 / 0	0.0	0.0	0.02 (1)	7.81					
O-N	0 / 1313	-18.2	-18.2	0.36 (1)	10.00					
N-M	0 / 1288	-18.2	-18.2	0.37 (4)	10.00					
M-L	0 / 1288	-18.2	-18.2	0.37 (4)	10.00					
L-K	0 / 1288	-18.2	-18.2	0.37 (4)	10.00					
K-J	0 / 1313	-18.2	-18.2	0.36 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.48/1.00 (D-E:1), BC=0.37/1.00 (M-N:4), WB=0.86/1.00 (C-O:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

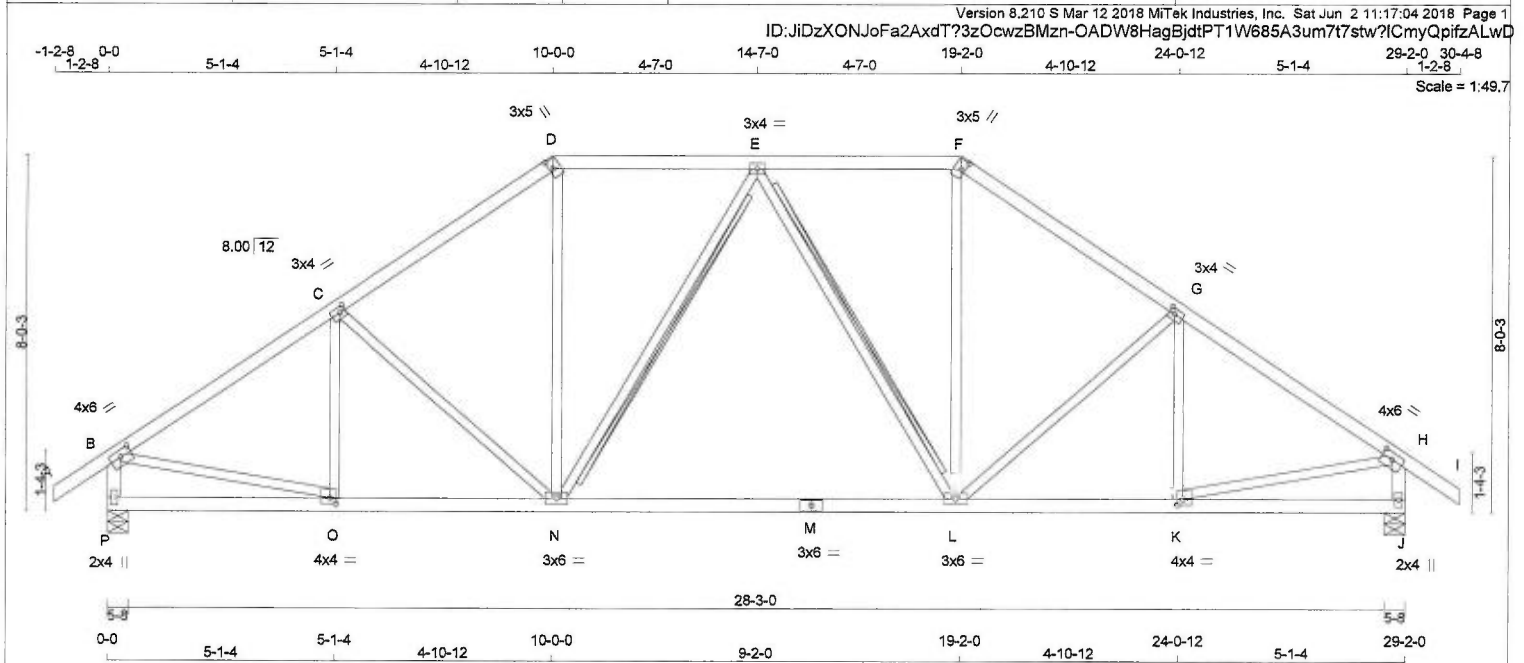
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 128 lb [M/F]

LUMBER

N. L. G. A. RULES

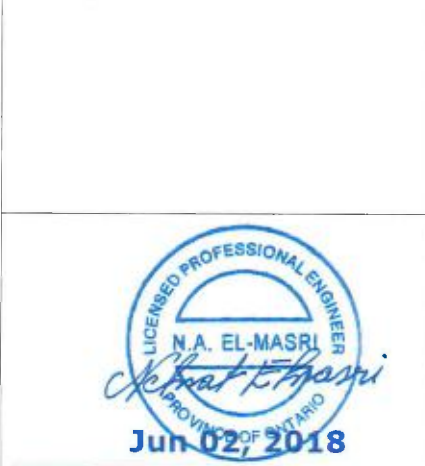
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	6.0	1.75	3.00
C	TMVW-I	MT20	3.0	4.0	1.50	1.50
D	TTW+H	MT20	3.0	5.0	2.50	1.00
E	TMVW-I	MT20	3.0	4.0		
F	TTW+H	MT20	3.0	5.0	2.50	1.00
G	TMVW-I	MT20	3.0	4.0	1.50	1.50
H	TMVW-I	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-I	MT20	4.0	4.0	1.50	1.50
L	BMVW-I	MT20	3.0	6.0		
M	BS-I	MT20	3.0	6.0		
N	BMVW-I	MT20	3.0	6.0		
O	BMVW-I	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
P	1494	0	1494	0	0	5-8	5-8	5-8
J	1494	0	1494	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
P	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH FR-TO	MAX. FACTORED LC1 MAX CSI (LC)
FR-TO				FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	O-C	-198 / 11	10.00	0.07 (1)
B-C	-1635 / 0	-77.4	-77.4 0.29 (1)	C-N	-244 / 0	4.91	0.20 (1)
C-D	-1468 / 0	-77.4	-77.4 0.27 (1)	N-D	0 / 520	5.13	0.12 (1)
D-E	-1205 / 0	-77.4	-77.4 0.22 (1)	E-N	-248 / 0	5.59	0.16 (1)
E-F	-1205 / 0	-77.4	-77.4 0.22 (1)	N-E	-248 / 0	5.59	0.16 (1)
F-G	-1468 / 0	-77.4	-77.4 0.27 (1)	E-L	-248 / 0	5.59	0.16 (1)
G-H	-1635 / 0	-77.4	-77.4 0.29 (1)	L-F	0 / 520	5.13	0.12 (1)
H-I	0 / 28	-77.4	-77.4 0.09 (1)	L-G	-244 / 0	4.91	0.20 (1)
P-B	-1451 / 0	0.0	0.0 0.15 (1)	K-G	-198 / 11	10.00	0.07 (1)
J-H	-1451 / 0	0.0	0.0 0.15 (1)	B-O	0 / 1408	6.79	0.32 (1)
				K-H	0 / 1408	6.79	0.32 (1)
P-O	0 / 0	-18.2	-18.2 0.11 (4)				
O-N	0 / 1381	-18.2	-18.2 0.36 (1)				
N-M	0 / 1330	-18.2	-18.2 0.35 (1)				
M-L	0 / 1330	-18.2	-18.2 0.35 (1)				
L-K	0 / 1381	-18.2	-18.2 0.36 (1)				
K-J	0 / 0	-18.2	-18.2 0.11 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.29/1.00 (B-C:1), BC=0.36/1.00 (N-O:1), WB=0.32/1.00 (B-O:1), SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

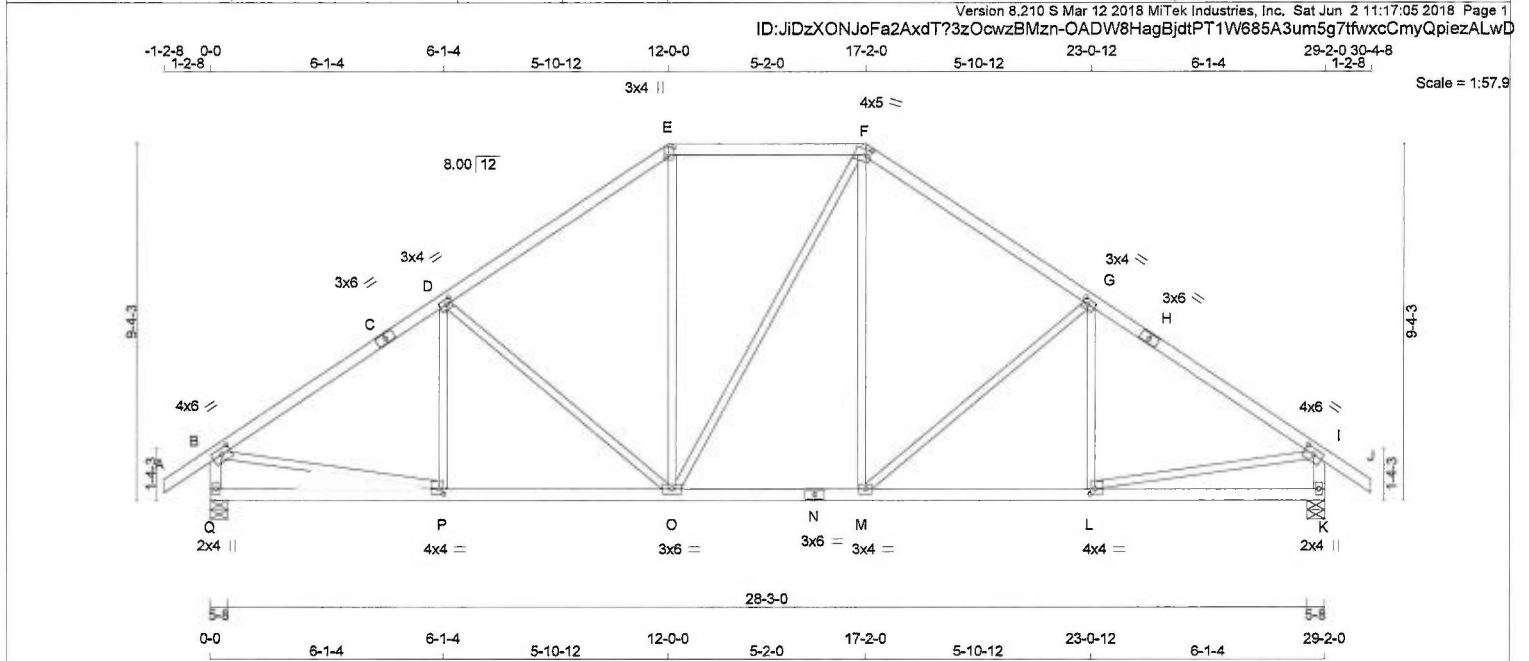
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)





TOTAL WEIGHT = 130 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	4.0	2.50	1.50
F	TTW+p	MT20	4.0	5.0	1.75	1.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
Q	1494	0	1494	0	5-8	5-8
K	1494	0	1494	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	P-D	-113 / 71	0.05 (1)	
B-C	-1649 / 0	-77.4	-77.4 0.43 (1)	D-O	-403 / 0	0.52 (1)	
C-D	-1649 / 0	-77.4	-77.4 0.43 (1)	O-E	0 / 359	0.08 (1)	
D-E	-1344 / 0	-77.4	-77.4 0.39 (1)	E-F	0 / 1	0.00 (1)	
E-F	-1094 / 0	-77.4	-77.4 0.28 (1)	F-G	0 / 357	0.08 (1)	
F-G	-1343 / 0	-77.4	-77.4 0.39 (1)	G-H	-404 / 0	0.52 (1)	
G-H	-1650 / 0	-77.4	-77.4 0.43 (1)	H-I	-112 / 71	0.05 (1)	
H-I	-1650 / 0	-77.4	-77.4 0.43 (1)	I-J	0 / 1417	0.32 (1)	
I-J	0 / 28	-77.4	-77.4 0.09 (1)	J-K	0 / 1418	0.32 (1)	
J-K	-1448 / 0	0.0	0.0 0.15 (1)				
K-L	-1448 / 0	0.0	0.0 0.15 (1)				
Q-P	0 / 0	-18.2	-18.2 0.16 (4)				
P-O	0 / 1398	-18.2	-18.2 0.30 (1)				
O-N	0 / 1094	-18.2	-18.2 0.24 (1)				
N-M	0 / 1094	-18.2	-18.2 0.24 (1)				
M-L	0 / 1398	-18.2	-18.2 0.31 (1)				
L-K	0 / 0	-18.2	-18.2 0.16 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.43/1.00 (G-I:1), BC=0.31/1.00 (L-M:1),
WB=0.52/1.00 (G-M:1), SSI=0.19/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

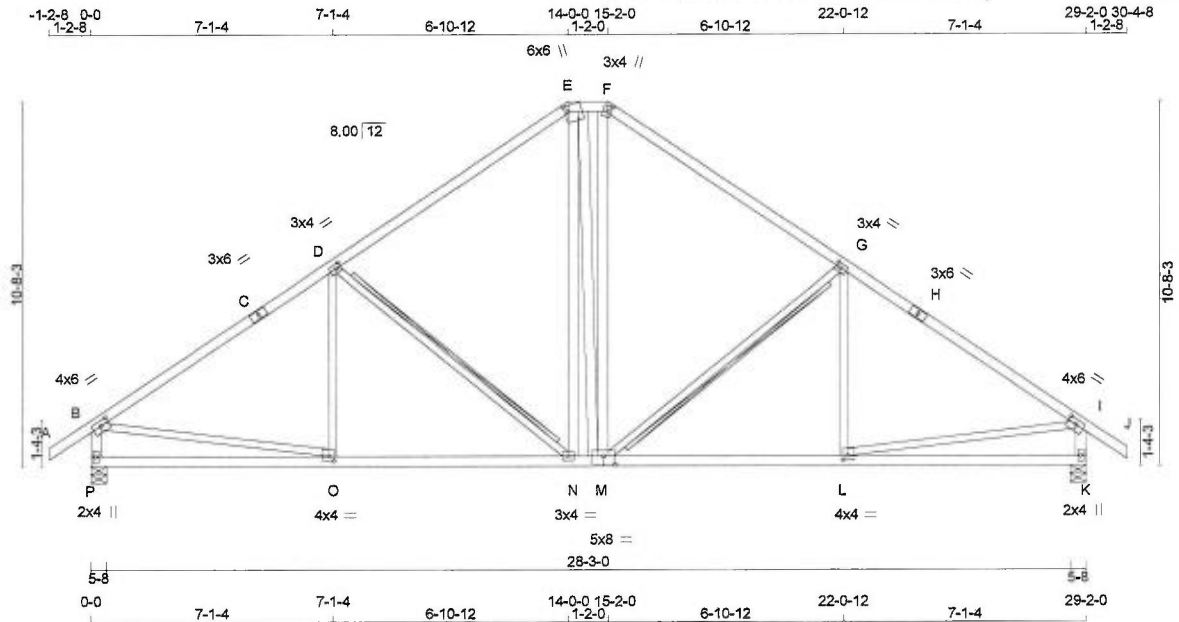
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.52 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 148 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
P - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
M - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - E	2x4	DRY	No.2
E - M	2x4	DRY	No.2
M - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW+m	MT20	6.0	6.0	2.00	1.25
F	TTW+m	MT20	3.0	4.0	2.00	1.25
G	TMWW-t	MT20	3.0	4.0	1.50	1.50
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BSWWW-t	MT20	5.0	8.0	3.00	4.00
N	BMWW-t	MT20	3.0	4.0		
O	BMWW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
P	1494	0	1494	0	0	5-8	5-8	
K	1494	0	1494	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
	COMBINED	SNOW	LIVE									
P	1047	739 / 0	0 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	308 / 0	0 / 0	0 / 0	0 / 0
K	1047	739 / 0	0 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	308 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-N, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	O-D	-54 / 107	0.04 (4)	
B-C	-1641 / 0	-77.4 -77.4 0.61 (1)	4.48	D-N	-541 / 0	0.35 (1)	
C-D	-1641 / 0	-77.4 -77.4 0.61 (1)	4.48	N-E	0 / 372	0.08 (1)	
D-E	-1213 / 0	-77.4 -77.4 0.55 (1)	5.10	E-M	0 / 37	0.01 (1)	
E-F	-986 / 0	-77.4 -77.4 0.02 (1)	6.25	M-F	0 / 409	0.07 (1)	
F-G	-1213 / 0	-77.4 -77.4 0.55 (1)	5.10	M-G	-538 / 0	0.35 (1)	
G-H	-1640 / 0	-77.4 -77.4 0.61 (1)	4.48	L-G	-59 / 105	0.04 (4)	
H-I	-1640 / 0	-77.4 -77.4 0.61 (1)	4.48	B-O	0 / 1410	0.32 (1)	
I-J	0 / 28	-77.4 -77.4 0.09 (1)	10.00	L-I	0 / 1409	0.32 (1)	
P-B	-1442 / 0	0.0 0.0 0.15 (1)	6.81				
K-I	-1441 / 0	0.0 0.0 0.15 (1)	6.81				
P-O	0 / 0	-18.2 -18.2 0.23 (4)	10.00				
O-N	0 / 1396	-18.2 -18.2 0.35 (1)	10.00				
N-M	0 / 982	-18.2 -18.2 0.26 (1)	10.00				
M-L	0 / 1395	-18.2 -18.2 0.34 (1)	10.00				
L-K	0 / 0	-18.2 -18.2 0.22 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 33.6	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.61/1.00 (B-D:1), BC=0.35/1.00 (N-O:1), WB=0.35/1.00 (D-N:1), SSI=0.22/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

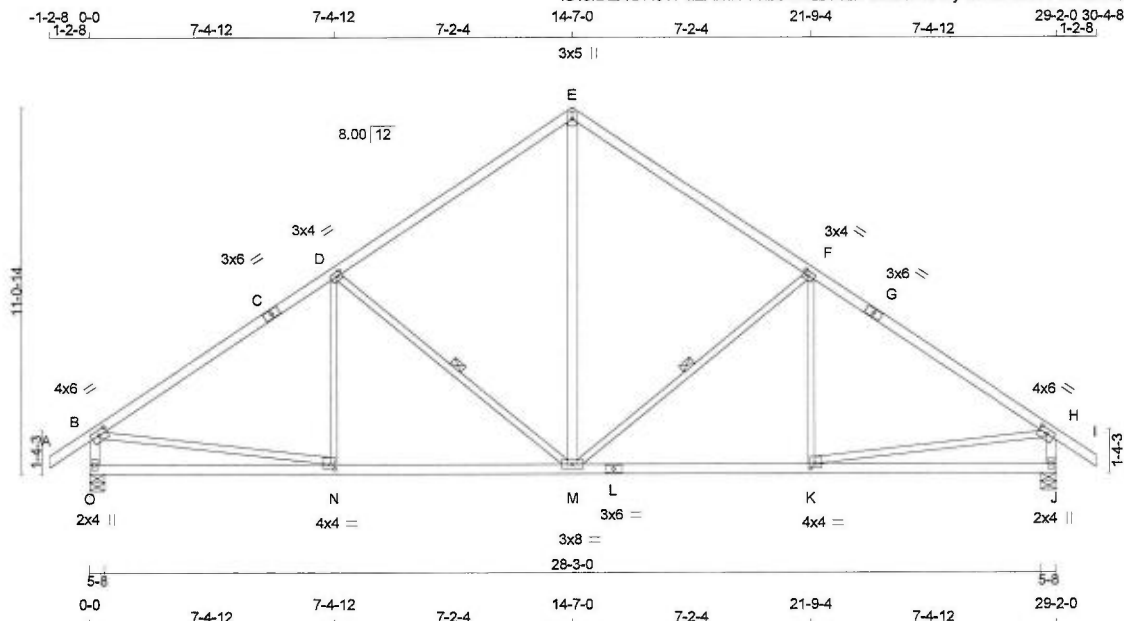
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:66.8

TOTAL WEIGHT = 3 X 126 = 379 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
M - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTW+p	MT20	3.0	5.0		
F	TMVW-t	MT20	3.0	4.0	1.50	1.50
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	1.50	1.50
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	4.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
O	1494	0	1494	0	0	5-8	5-8	5-8
J	1494	0	1494	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M. DBS = 20-0-0. CBF = 71 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	MAX. FACTORED LC1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	M-E	0 / 853	0.14 (1)
B-C	-1635 / 0	-77.4	-77.4 0.67 (1)	4.39	M-F	-571 / 0	0.35 (1)
C-D	-1635 / 0	-77.4	-77.4 0.67 (1)	4.39	K-F	-45 / 112	0.04 (4)
D-E	-1183 / 0	-77.4	-77.4 0.60 (1)	5.05	D-M	-571 / 0	0.35 (1)
E-F	-1183 / 0	-77.4	-77.4 0.60 (1)	5.05	N-D	-45 / 112	0.04 (4)
F-G	-1635 / 0	-77.4	-77.4 0.67 (1)	4.39	B-N	0 / 1405	0.32 (1)
G-H	-1635 / 0	-77.4	-77.4 0.67 (1)	4.39	K-H	0 / 1405	0.32 (1)
H-I	0 / 28	-77.4	-77.4 0.09 (1)	10.00			
O-B	-1439 / 0	0.0	0.0 0.15 (1)	6.81			
J-H	-1439 / 0	0.0	0.0 0.15 (1)	6.81			
O-N	0 / 0	-18.2	-18.2 0.24 (4)	10.00			
N-M	0 / 1392	-18.2	-18.2 0.35 (1)	10.00			
M-L	0 / 1392	-18.2	-18.2 0.35 (1)	10.00			
L-K	0 / 1392	-18.2	-18.2 0.35 (1)	10.00			
K-J	0 / 0	-18.2	-18.2 0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	33.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CSI: TC=0.67/1.00 (B-D:1), BC=0.35/1.00 (M-N:1), WB=0.35/1.00 (D-M:1), SSI=0.23/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

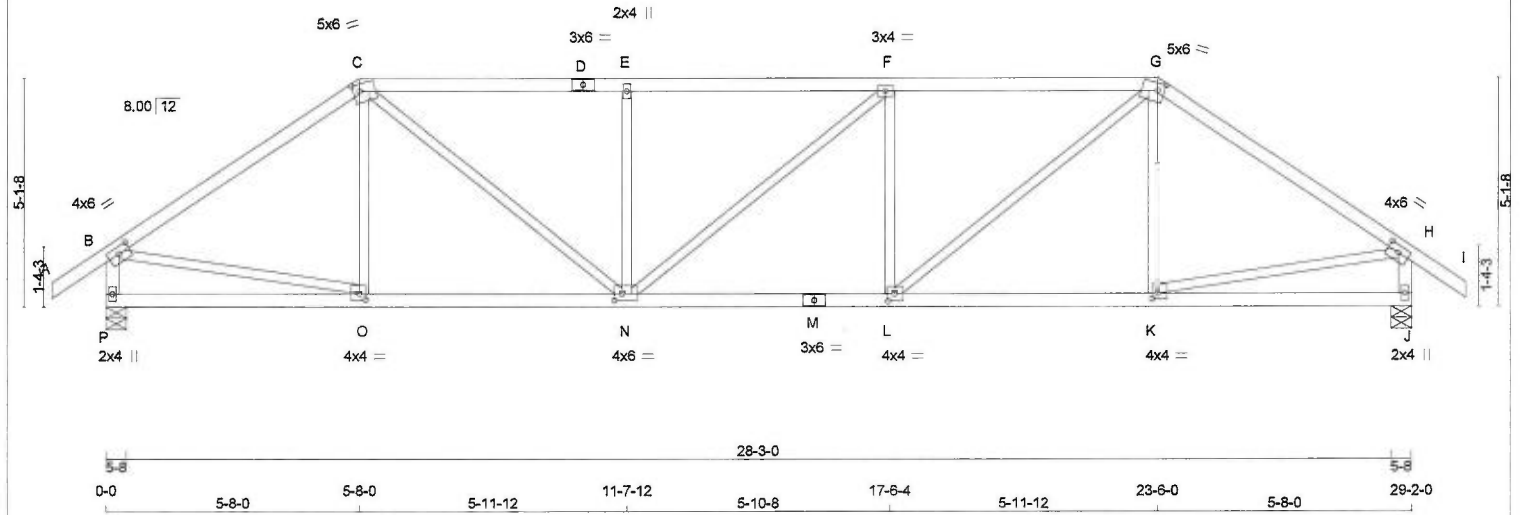
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.52 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMW-w	MT20	2.0	4.0		
F	TMW-w	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.00	1.75
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1-p	MT20	2.0	4.0		
K	BMW-w	MT20	4.0	4.0	1.50	1.50
L	BMW-w	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	6.0		
N	BMW-w	MT20	4.0	6.0	1.75	2.00
O	BMW-w	MT20	4.0	4.0	1.50	1.50
P	BMV1-p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	
P	1494	0	1494	0	0	5-8	
J	1494	0	1494	0	0	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	C-C	-118 / 61	0.05 (1)	
B-C	-1609 / 0	-77.4 -77.4 0.55 (1)	4.58	C-N	0 / 955	0.22 (1)	
C-D	-2079 / 0	-77.4 -77.4 0.48 (1)	4.22	N-E	-495 / 0	0.19 (1)	
D-E	-2079 / 0	-77.4 -77.4 0.48 (1)	4.22	N-F	-2 / 0	0.00 (1)	
E-F	-2078 / 0	-77.4 -77.4 0.49 (1)	4.21	L-F	-497 / 0	0.19 (1)	
F-G	-2080 / 0	-77.4 -77.4 0.49 (1)	4.21	L-G	0 / 958	0.22 (1)	
G-H	-1609 / 0	-77.4 -77.4 0.55 (1)	4.58	K-G	-118 / 61	0.05 (1)	
H-I	0 / 28	-77.4 -77.4 0.09 (1)	10.00	B-O	0 / 1358	0.31 (1)	
P-B	-1452 / 0	0.0 0.0 0.15 (1)	6.79	K-H	0 / 1358	0.31 (1)	
J-H	-1452 / 0	0.0 0.0 0.15 (1)	6.79				
P-O	0 / 0	-18.2 -18.2 0.15 (4)	10.00				
O-N	0 / 1335	-18.2 -18.2 0.30 (1)	10.00				
N-M	0 / 2080	-18.2 -18.2 0.39 (1)	10.00				
M-L	0 / 2080	-18.2 -18.2 0.39 (1)	10.00				
L-K	0 / 1334	-18.2 -18.2 0.30 (1)	10.00				
K-J	0 / 0	-18.2 -18.2 0.15 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.55/1.00 (B-C:1), BC=0.39/1.00 (L-N:1), WB=0.31/1.00 (B-O:1), SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

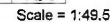
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.57 (M) (INPUT = 1.00)

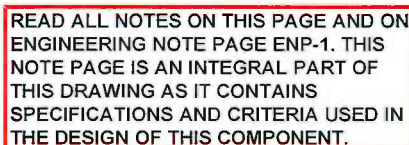


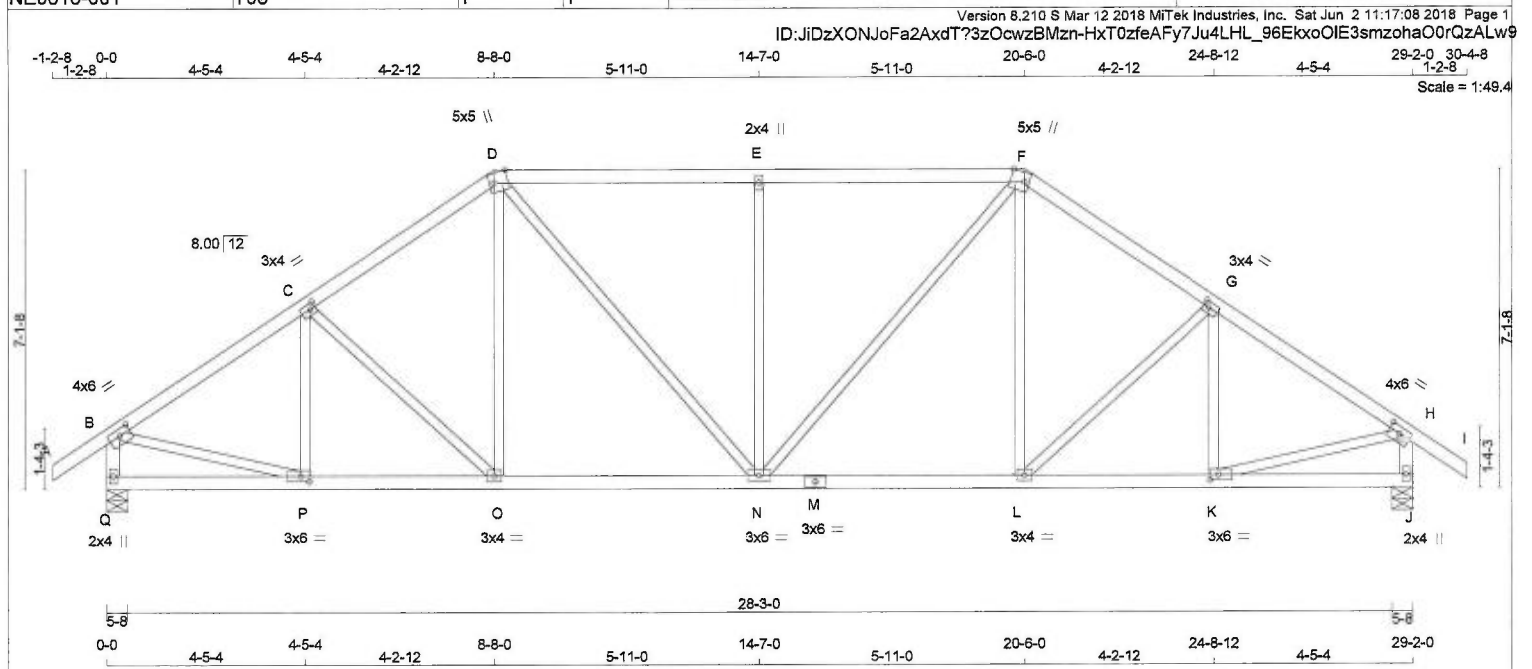
READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.53 (G) (INPUT = 1.00)





TOTAL WEIGHT = 128 lb [M]F

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	VERT	DOWN
D - F	2x4	DRY	No.2	SPF	GROSS REACTION	HORZ	UPLIFT
F - I	2x4	DRY	No.2	SPF	FACTORED	BRG	IN-SX
Q - B	2x4	DRY	No.2	SPF	FACTORED	BRG	IN-SX
J - H	2x4	DRY	No.2	SPF	FACTORED	BRG	IN-SX
Q - M	2x4	DRY	No.2	SPF	FACTORED	BRG	IN-SX
M - J	2x4	DRY	No.2	SPF	FACTORED	BRG	IN-SX

UNFACTORED REACTIONS				UNFACTORED REACTIONS			
1ST LCASE	MAX	MIN	COMPONENT REACTIONS	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
J	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

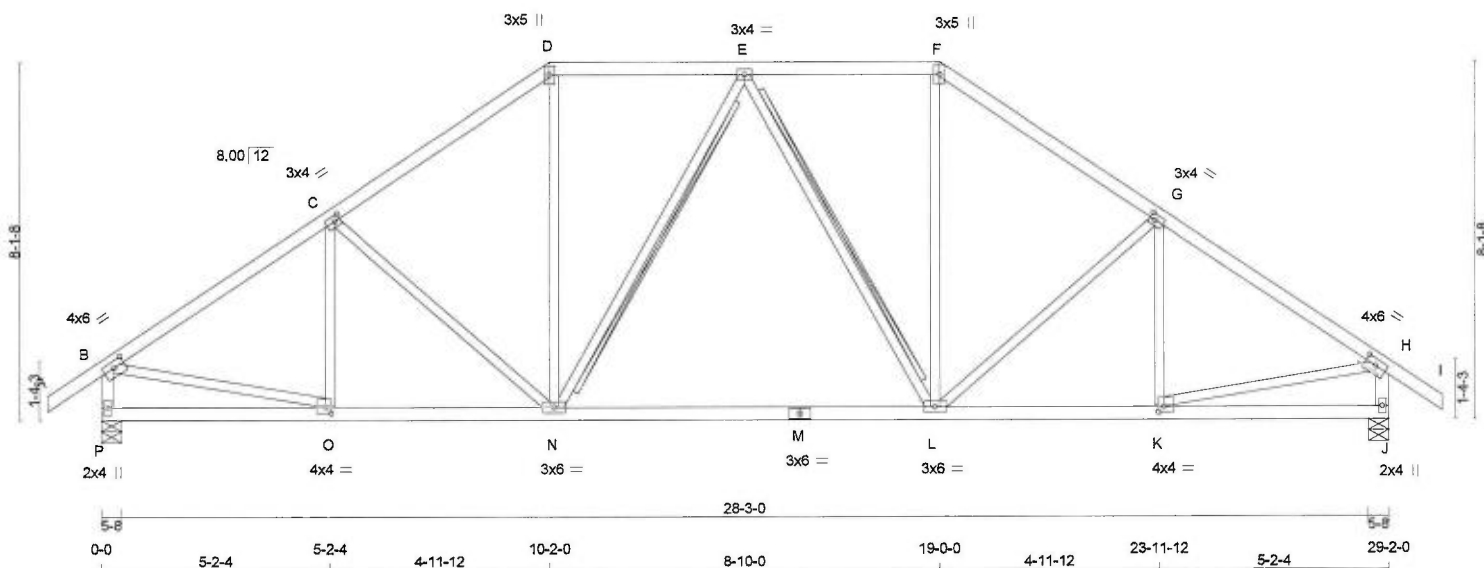
LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	P-C	-237 / 0	0.07 (1)	
B-C	-1616 / 0	-77.4 -77.4 0.21 (1)	5.01	C-D	-154 / 0	0.09 (1)	
C-D	-1526 / 0	-77.4 -77.4 0.21 (1)	5.13	D-E	0 / 201	0.05 (1)	
D-E	-1542 / 0	-77.4 -77.4 0.38 (1)	4.88	E-F	0 / 443	0.10 (1)	
E-F	-1542 / 0	-77.4 -77.4 0.38 (1)	4.88	F-G	-560 / 0	0.49 (1)	
F-G	-1526 / 0	-77.4 -77.4 0.21 (1)	5.13	G-H	0 / 443	0.10 (1)	
G-H	-1616 / 0	-77.4 -77.4 0.21 (1)	5.01	H-I	0 / 201	0.05 (1)	
H-I	0 / 28	-77.4 -77.4 0.09 (1)	10.00	I-J	-154 / 0	0.09 (1)	
Q-B	-1458 / 0	0.0 0.0 0.15 (1)	6.77	J-K	-237 / 0	0.07 (1)	
J-H	-1458 / 0	0.0 0.0 0.15 (1)	6.77	K-L	0 / 1398	0.31 (1)	
				L-K	0 / 1398	0.31 (1)	
Q-P	0 / 0	-18.2 -18.2 0.07 (4)	10.00				
P-O	0 / 1362	-18.2 -18.2 0.28 (1)	10.00				
O-N	0 / 1252	-18.2 -18.2 0.27 (1)	10.00				
N-M	0 / 1252	-18.2 -18.2 0.27 (1)	10.00				
M-L	0 / 1252	-18.2 -18.2 0.27 (1)	10.00				
L-K	0 / 1362	-18.2 -18.2 0.28 (1)	10.00				
K-J	0 / 0	-18.2 -18.2 0.07 (4)	10.00				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF
SPACING = 24.0 IN./C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")
CSI: TC=0.38/1.00 (D-E:1), BC=0.28/1.00 (O-P:1), WB=0.49/1.00 (E-N:1), SSI=0.22/1.00 (D-E:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 0.50
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

-1-2-8 0-0 5-2-4 5-2-4 4-11-12 10-2-0 4-5-0 14-7-0 4-5-0 19-0-0 4-11-12 23-11-12 5-2-4 29-2-0 30-4-8 1-2-8 Scale = 1:50.1



TOTAL WEIGHT = 129 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
P - B	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
P - M	2x4 DRY	No.2	SPF		
M - J	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	4.0	6.0	1.75	3.00	
C TMVW-t	MT20	3.0	4.0	1.50	1.50	
D TTVW-p	MT20	3.0	5.0			
E TMVW-t	MT20	3.0	4.0			
F TTVW-p	MT20	3.0	5.0			
G TMVW-t	MT20	3.0	4.0	1.50	1.50	
H TMVW-t	MT20	4.0	6.0	1.75	3.00	
J BMV1-p	MT20	2.0	4.0			
K BMVW-t	MT20	4.0	4.0	1.50	1.50	
L BMVW-t	MT20	3.0	6.0			
M BS-t	MT20	3.0	6.0			
N BMVW-t	MT20	3.0	6.0			
O BMVW-t	MT20	4.0	4.0	1.50	1.50	
P BMV1-p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
P	1494	0	1494	0
J	1494	0	1494	0

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
P	1047	739 / 0	0 / 0	0 / 0	308 / 0
J	1047	739 / 0	0 / 0	0 / 0	308 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. CS1 (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. CS1 (LC)
FR-TO			FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4	0.09 (1)	10.00	O-C	-189 / 17	0.05 (1)
B-C	-1637 / 0	-77.4	-77.4	0.30 (1)	4.89	C-N	-258 / 0	0.22 (1)
C-D	-1458 / 0	-77.4	-77.4	0.28 (1)	5.13	N-E	0 / 516	0.12 (1)
D-E	-1196 / 0	-77.4	-77.4	0.20 (1)	5.63	E-L	-236 / 0	0.15 (1)
E-F	-1196 / 0	-77.4	-77.4	0.20 (1)	5.63	L-F	0 / 516	0.12 (1)
F-G	-1458 / 0	-77.4	-77.4	0.28 (1)	5.13	G-H	-258 / 0	0.22 (1)
G-H	-1637 / 0	-77.4	-77.4	0.30 (1)	4.89	H-I	-189 / 17	0.05 (1)
H-I	0 / 28	-77.4	-77.4	0.09 (1)	10.00	I-K	-258 / 0	0.22 (1)
P-B	-1451 / 0	0.0	0.0	0.15 (1)	6.79	B-O	0 / 1410	0.32 (1)
J-H	-1451 / 0	0.0	0.0	0.15 (1)	6.79	K-H	0 / 1410	0.32 (1)
P-O	0 / 0	-18.2	-18.2	0.11 (4)	10.00			
O-N	0 / 1383	-18.2	-18.2	0.35 (1)	10.00			
N-M	0 / 1310	-18.2	-18.2	0.33 (1)	10.00			
M-L	0 / 1310	-18.2	-18.2	0.33 (1)	10.00			
L-K	0 / 1383	-18.2	-18.2	0.35 (1)	10.00			
K-J	0 / 0	-18.2	-18.2	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.30/1.00 (B-C:1), BC=0.35/1.00 (N-O:1), WB=0.32/1.00 (B-O:1), SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

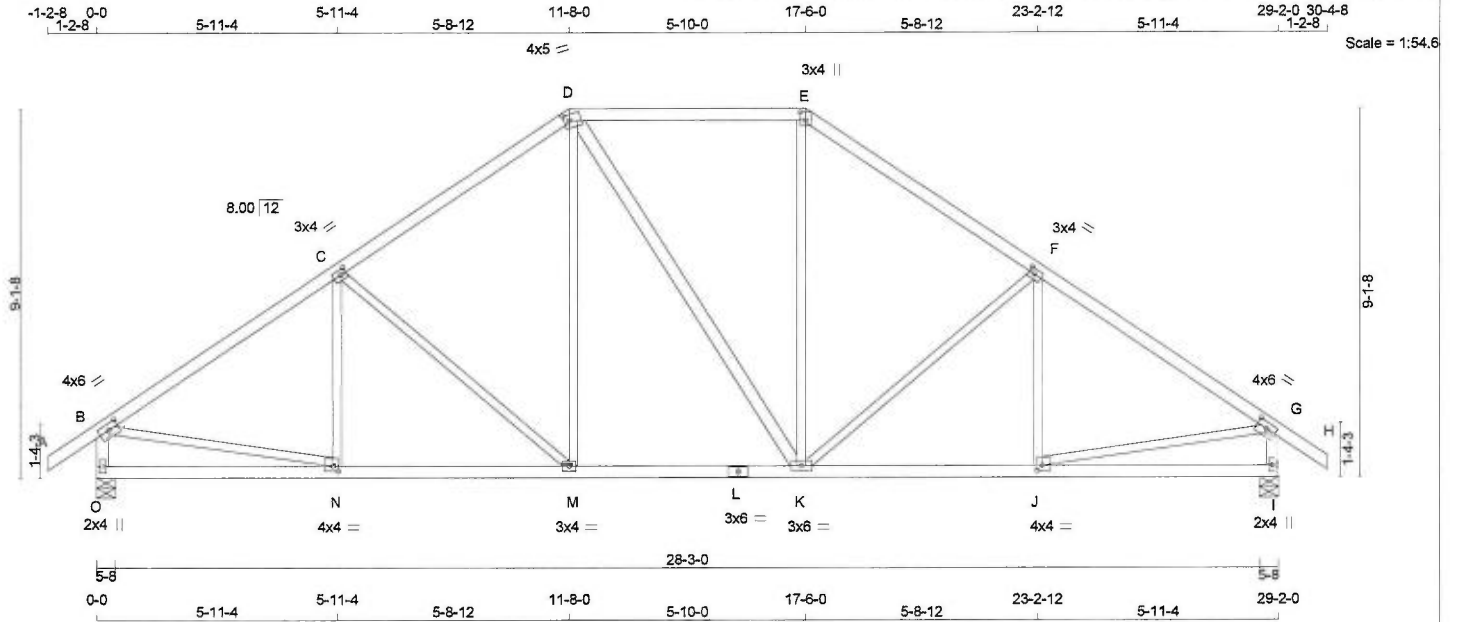
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT D - K	2x3	DRY	No.2
	2x4	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.75 3.00
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TTWV-m	MT20	4.0	5.0	1.75 1.50
E	TTWV-p	MT20	3.0	4.0	2.50 1.50
F	TMVW-t	MT20	4.0	6.0	1.75 3.00
G	TMVW-t	MT20	4.0	6.0	1.75 3.00
I	BMV1+p	MT20	2.0	4.0	
J	BMVW-t	MT20	4.0	4.0	1.50 1.50
K	BMVWV-t	MT20	3.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMVW-t	MT20	3.0	4.0	
N	BMVW-t	MT20	4.0	4.0	1.50 1.50
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
O	1494	0	1494	0	0	5-8	5-8	5-8	5-8
I	1494	0	1494	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0
I	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. LC1 CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	N-C	-124 / 63	10.00	0.05 (1)
B-C	-1649 / 0	-77.4	-77.4 0.40 (1)	C-M	-379 / 0	4.75	0.45 (1)
C-D	-1364 / 0	-77.4	-77.4 0.37 (1)	M-D	0 / 349	5.15	0.08 (1)
D-E	-1113 / 0	-77.4	-77.4 0.36 (1)	D-K	0 / 0	5.57	0.00 (1)
E-F	-1365 / 0	-77.4	-77.4 0.37 (1)	K-E	0 / 350	5.15	0.08 (1)
F-G	-1649 / 0	-77.4	-77.4 0.40 (1)	K-F	-379 / 0	4.75	0.45 (1)
G-H	0 / 28	-77.4	-77.4 0.09 (1)	J-F	-125 / 63	10.00	0.05 (1)
O-B	-1449 / 0	0.0	0.0 0.15 (1)	B-N	0 / 1417	6.79	0.32 (1)
I-G	-1449 / 0	0.0	0.0 0.15 (1)	J-G	0 / 1417	6.79	0.32 (1)
O-N	0 / 0	-18.2	-18.2 0.15 (4)				
N-M	0 / 1397	-18.2	-18.2 0.30 (1)				
M-L	0 / 1112	-18.2	-18.2 0.24 (1)				
L-K	0 / 1112	-18.2	-18.2 0.24 (1)				
K-J	0 / 1396	-18.2	-18.2 0.30 (1)				
J-I	0 / 0	-18.2	-18.2 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 33.6	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.40/1.00 (B-C:1), BC=0.30/1.00 (M-N:1),
WB=0.45/1.00 (C-M:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

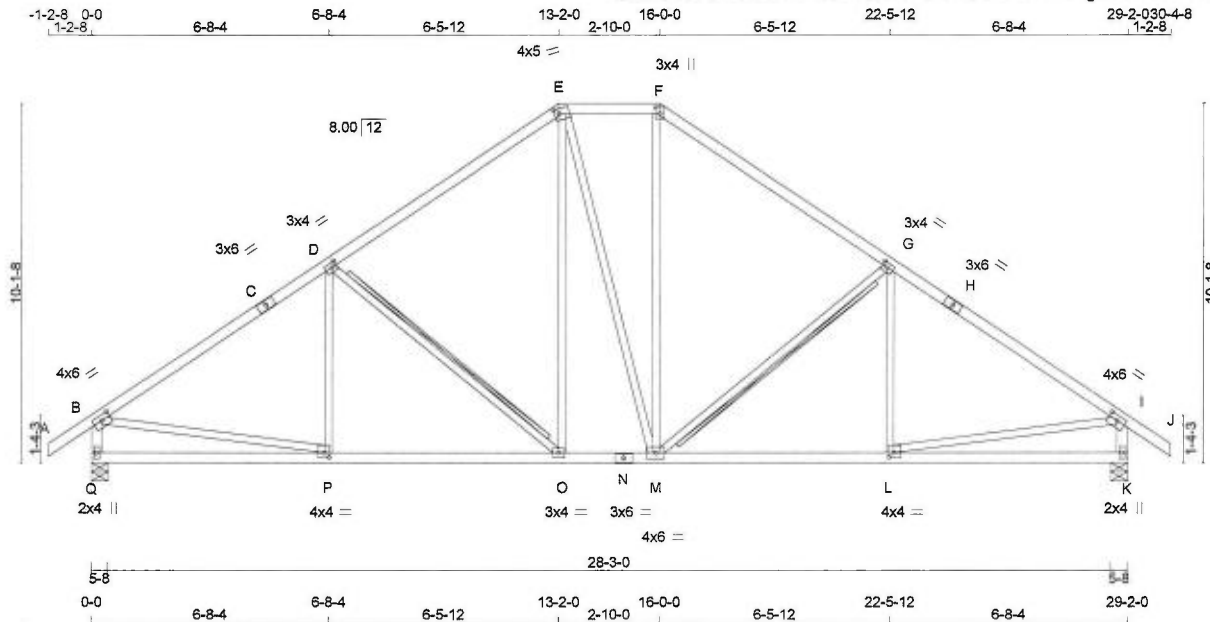
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.52 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:62.3

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TS-1	MT20	3.0	6.0		
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTWW-m	MT20	4.0	5.0	1.75	1.50
F	TTWW-p	MT20	3.0	4.0	2.25	1.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TS-1	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.50	1.50
M	BMVW-t	MT20	4.0	6.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
Q	1494	0	1494	0	0	5-8	5-8	5-8	5-8
K	1494	0	1494	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
Q	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	1047	739 / 0	0 / 0	0 / 0
K	1047	739 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0	1047	739 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-O, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4 0.09 (1)	10.00	P-D	-75 / 94	0.04 (1)	
B-C	-1647 / 0	-77.4 -77.4 0.53 (1)	4.59	D-O	-486 / 0	0.28 (1)	
C-D	-1647 / 0	-77.4 -77.4 0.53 (1)	4.59	O-E	0 / 384	0.09 (1)	
D-E	-1268 / 0	-77.4 -77.4 0.48 (1)	5.13	E-M	0 / 5	0.00 (1)	
E-F	-1030 / 0	-77.4 -77.4 0.09 (1)	6.09	M-F	0 / 389	0.09 (1)	
F-G	-1270 / 0	-77.4 -77.4 0.48 (1)	5.13	M-G	-484 / 0	0.27 (1)	
G-H	-1646 / 0	-77.4 -77.4 0.53 (1)	4.59	L-G	-78 / 93	0.04 (1)	
H-I	-1646 / 0	-77.4 -77.4 0.53 (1)	4.59	B-P	0 / 1415	0.32 (1)	
I-J	0 / 28	-77.4 -77.4 0.09 (1)	10.00	L-I	0 / 1414	0.32 (1)	
Q-B	-1445 / 0	0.0 0.0 0.15 (1)	6.80				
K-I	-1444 / 0	0.0 0.0 0.15 (1)	6.80				
Q-P	0 / 0	-18.2 -18.2 0.21 (4)	10.00				
P-O	0 / 1399	-18.2 -18.2 0.33 (1)	10.00				
O-N	0 / 1029	-18.2 -18.2 0.23 (1)	10.00				
N-M	0 / 1029	-18.2 -18.2 0.23 (1)	10.00				
M-L	0 / 1398	-18.2 -18.2 0.33 (1)	10.00				
L-K	0 / 0	-18.2 -18.2 0.20 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 33.6	PSF	

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.53/1.00 (B-D:1), BC=0.33/1.00 (O-P:1), WB=0.32/1.00 (B-P:1), SSI=0.21/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

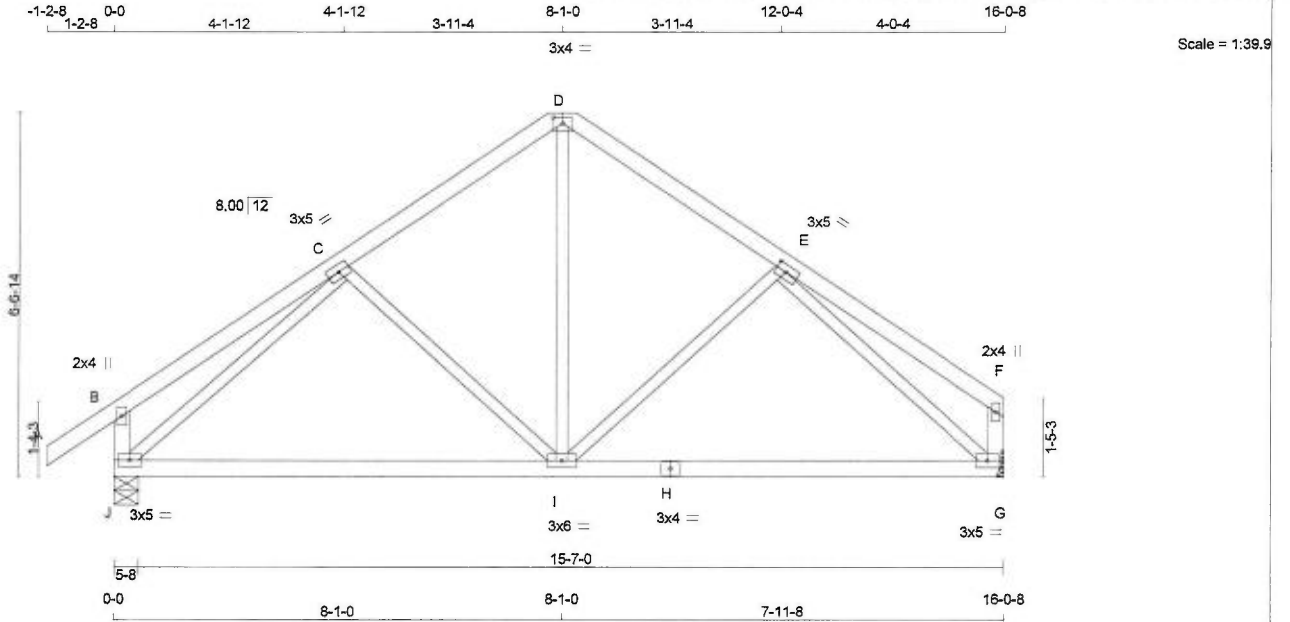
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.52 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 66 lb
[M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMVW1-t	MT20	3.0	5.0	
D	TTW-p	MT20	3.0	4.0	1.25 2.00
E	TMVW1-t	MT20	3.0	5.0	1.50 2.25
F	TMV+p	MT20	2.0	4.0	
G	BMVW1-t	MT20	3.0	5.0	
H	BS-t	MT20	3.0	4.0	
I	BMVW1-t	MT20	3.0	6.0	
J	BMVW1-t	MT20	3.0	5.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
J	867	0	867	0	0	5-8	5-8		
G	767	0	767	0	0	MECHANICAL			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	607	434 / 0	0 / 0	0 / 0	0 / 0	173 / 0	0 / 0
G	539	374 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	C-I	-190 / 2	0.10 (1)
B-C	0 / 20	-77.4 -77.4	0.20 (1)	10.00	I-D	0 / 425	0.10 (1)
C-D	-622 / 0	-77.4 -77.4	0.16 (1)	6.25	I-E	-172 / 6	0.09 (1)
D-E	-621 / 0	-77.4 -77.4	0.15 (1)	6.25	J-C	-871 / 0	0.43 (1)
E-F	0 / 20	-77.4 -77.4	0.19 (1)	10.00	E-G	-866 / 0	0.41 (1)
J-B	-220 / 0	0.0	0.02 (1)	7.81			
G-F	-115 / 0	0.0	0.01 (1)	7.81			
J-I	0 / 641	-18.2 -18.2	0.38 (4)	10.00			
I-H	0 / 627	-18.2 -18.2	0.38 (4)	10.00			
H-G	0 / 627	-18.2 -18.2	0.38 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.20/1.00 (B-C:1), BC=0.38/1.00 (I-J:4),
WB=0.43/1.00 (C-J:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

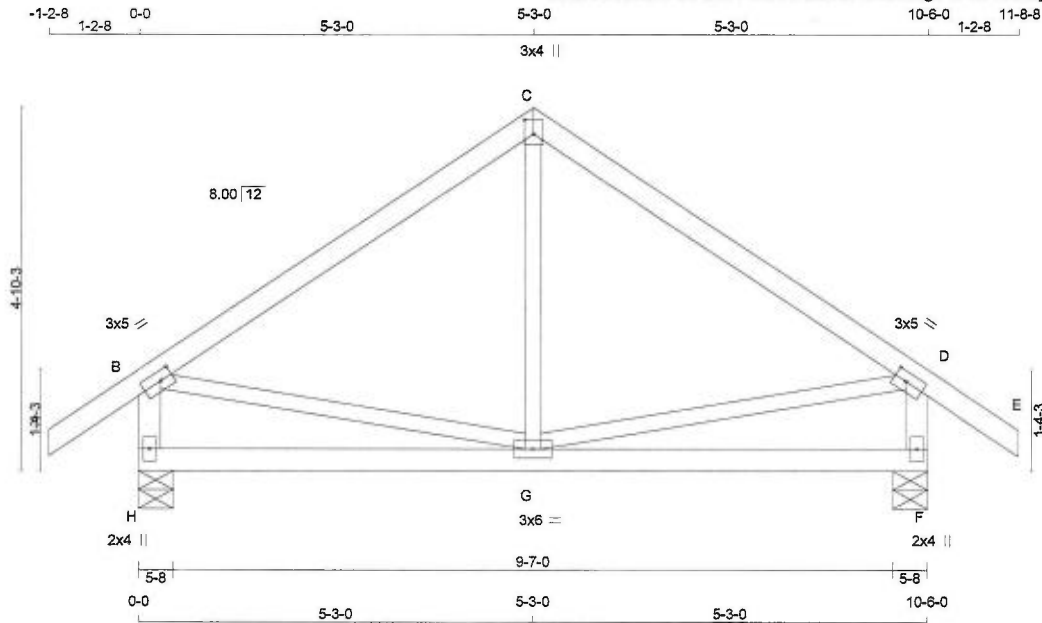
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:29.5

TOTAL WEIGHT = 44 lb
[M]F

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	3.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
H	602	0	602	0	0	5-8	5-8	5-8	5-8
F	602	0	602	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	420	305 / 0	0 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
F	420	305 / 0	0 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-77.4 -77.4	0.09 (1)	10.00	G-C	-1 / 93	0.03 (4)
B-C	-365 / 0	-77.4 -77.4	0.27 (1)	6.25	B-G	0 / 310	0.07 (1)
C-D	-365 / 0	-77.4 -77.4	0.27 (1)	6.25	G-D	0 / 310	0.07 (1)
D-E	0 / 28	-77.4 -77.4	0.09 (1)	10.00			
H-B	-565 / 0	0.0 0.0	0.06 (1)	7.81			
F-D	-565 / 0	0.0 0.0	0.06 (1)	7.81			
H-G	0 / 0	-18.2 -18.2	0.14 (4)	10.00			
G-F	0 / 0	-18.2 -18.2	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.3	PSF	
TOTAL LOAD = 33.6	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.27/1.00 (B-C:1), BC=0.14/1.00 (F-G:4),
WB=0.07/1.00 (D-G:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

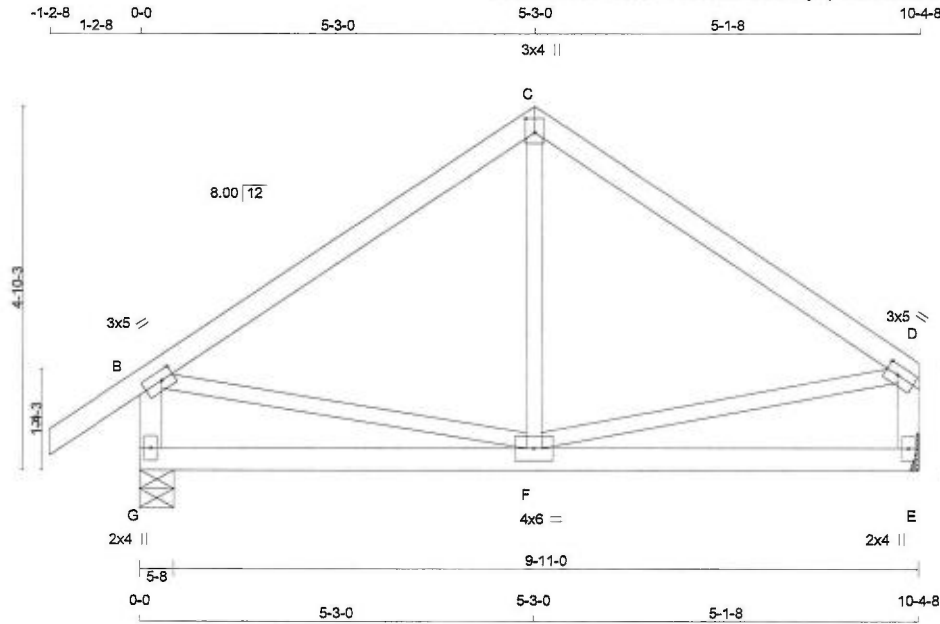
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:29.5

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
G	596	0	596	0	5-8	5-8		
E	496	0	496	0	0	MECHANICAL		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
G	416	302 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
E	348	242 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-77.4	-77.4 0.09 (1)	10.00	F-C	-6 / 90	0.03 (4)
B-C	-356 / 0	-77.4	-77.4 0.27 (1)	6.25	B-F	0 / 302	0.07 (1)
C-D	-356 / 0	-77.4	-77.4 0.26 (1)	6.25	F-D	0 / 303	0.07 (1)
G-B	-559 / 0	0.0	0.0 0.06 (1)	7.81			
E-D	-460 / 0	0.0	0.0 0.05 (1)	7.81			
G-F	0 / 0	-18.2	-18.2 0.14 (4)	10.00			
F-E	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.27/1.00 (B-C:1), BC=0.14/1.00 (E-F:4), WB=0.07/1.00 (D-F:1), SS=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

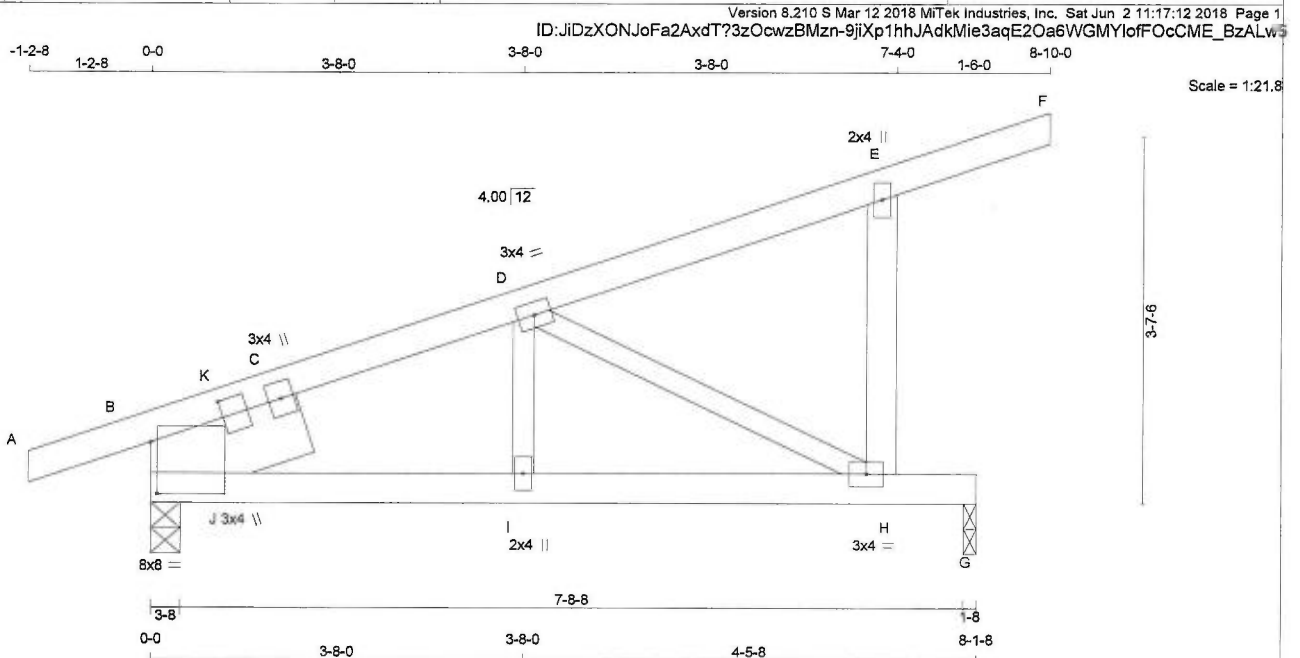
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF

REINFORCING MEMBERS
HW1 2x8 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBMR1-I	MT20	8.0	8.0	6.25	0.75
B RT+I	MT20	3.0	4.0	2.00	9.00
B RT+I	MT20	3.0	4.0		
D TMWW-I	MT20	3.0	4.0		
E TMV+p	MT20	2.0	4.0		
H BMVW+I	MT20	3.0	4.0		
I BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	494	0	494	0	3-8	3-8
G	438	0	438	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
B	345	253 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
G	307	219 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	-77.4	-77.4	0.06 (1)	I-D	0 / 199	10.00	0.04 (1)
B-K	-516 / 0	-77.4	-77.4 0.01 (1)	D-H	-581 / 0	6.25	0.15 (1)
K-C	-413 / 0	-77.4	-77.4 0.12 (1)	J-K	0 / 418	6.25	0.00 (1)
C-D	-413 / 0	-77.4	-77.4 0.12 (1)	J-C	-420 / 0	6.25	0.02 (1)
D-E	-4 / 0	-77.4	-77.4 0.13 (1)			10.00	
E-F	-19 / 0	-77.4	-77.4 0.10 (1)			6.25	
H-E	-248 / 0	0.0	0.0 0.04 (1)			7.81	
B-J	0 / 337	-18.2	-18.2 0.08 (1)			10.00	
J-I	0 / 520	-18.2	-18.2 0.20 (1)			10.00	
I-H	0 / 520	-18.2	-18.2 0.55 (1)			10.00	
H-G	0 / 0	-18.2	-18.2 0.46 (1)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 33.6 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(TL) = $L/795$ (0.12")

CSI: TC=0.13/1.00 (D-E:1), BC=0.55/1.00 (H-I:1), WB=0.15/1.00 (D-H:1), SSI=0.34/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.19 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



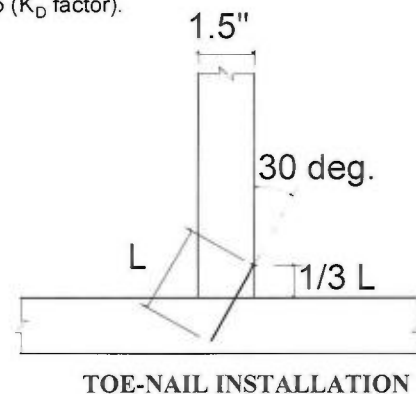
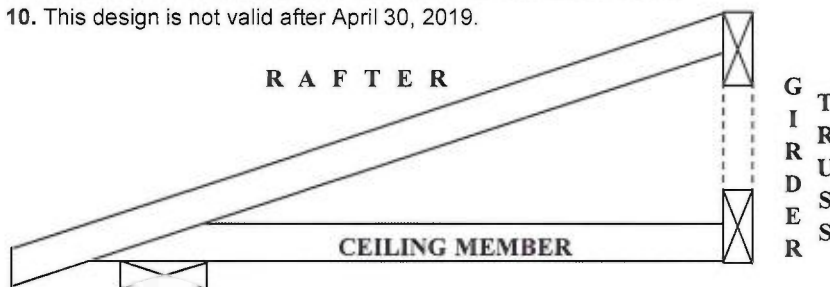
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

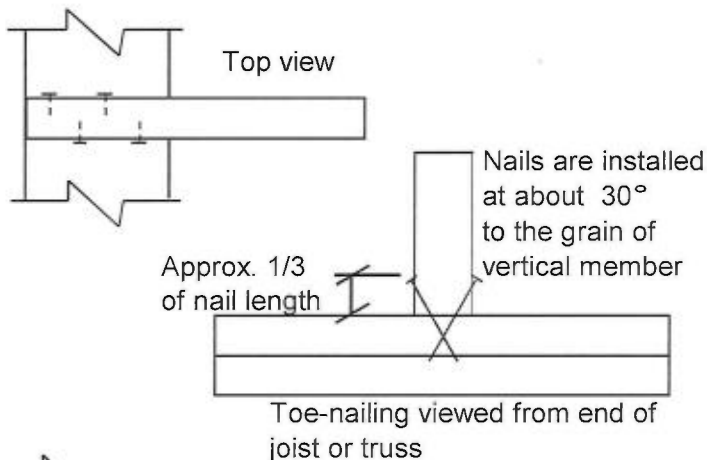
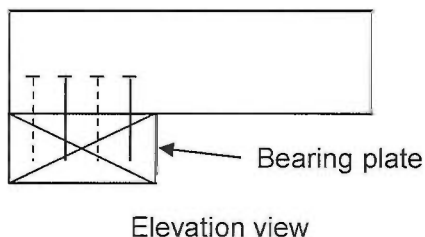
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

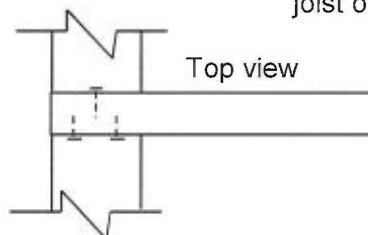
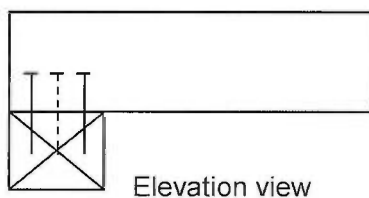
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek®
MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

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Certificate No. 10889485



April 26, 2017