Layout Name

Design Method

LSD

Description

Created

Builder

16-0-0 Designer

SB

Shipping

Project

Canada

L4A 7X4

Floor

Loads

Live

Dead

905-642-4400

Design Method

Deflection Joist

Ground Floor

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Sales Rep

AMELIA 2 EL- 1 & 2

GREEN YORK HOMES

BRAMPTON,ON

May 28, 2018

GRANELLI HOMES PROJECT

Kott Lumber Company

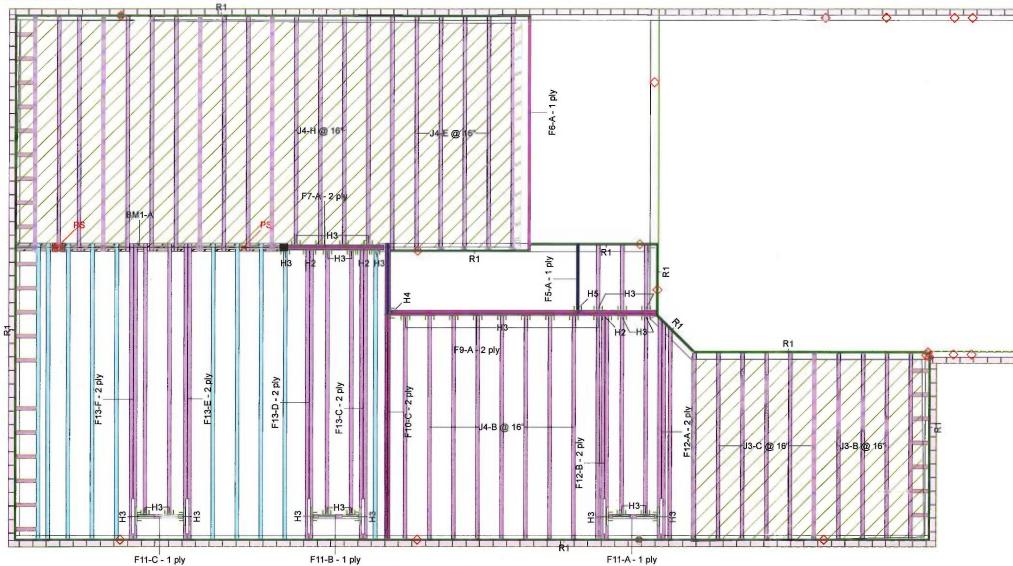
Building Code NBCC 2010 / OBC

LSD

40

15

Ground Floor



THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, **COLUMNS AND FOUNDATION WALLS AND FOOTINGS**

CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS CALCULATION SUMMARY PAGE AS IT

N.A. EL-MASRI

Jun 04, 2018

NE0618-017

USED IN THE DESIGN OF THIS COMPONENT in accordance with the supplier's layout and

ineered floor joists shall be installed gend specifications forming part of the permit d

CITY OF BRAMPTON

Norbord Rimboard Plus 1.125 X 11.875 NJ60H 11.875 NJH 11 875

19-444464.000.00K.

5.25 X 10.25 (Dropped)

Project # 17-55

INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

REFER TO MULTIPLE MEMBER TO MEMBER

PASS THRU FRAMING SQUASH

NOTE PAGE IS AN INTEGRAL PART OF THIS **CONTAINS SPECIFICATIONS AND CRITERIA**

All work shall conform to

BUILDING DIVISION

REVIEWED

MAR 0 5 2019

MARY FRENETTE

Ground Floor LVL/LSL

F10

F9

F6

F7

F5

Joist

Label Description

Forex

Forex

Forex

Label Description

.18 N.J60H

F13 NJH

F12 NJH

F11 NJH

J5 NJH

J4 NJH

J3 NJH

J2 NJH

Label Description

11.875

Label Description

Norbord Rimboar

Pcs Description

3 HU310-2

1 HGUS410

1 HUS1.81/10

34 LF2511

Rim Board

Blockina

Hanger

Label

НЗ

H4

H5

NOTES:

requirements.

BLK1 NJH

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

Width Depth

11.875

11.875

11.875

11.875

11.875

1.75 11.875

Width Depth

2.5

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

Width Depth

Width Depth

2.5 11.875

11.875

Skew Slope

1.125

Framer to verify dimensions on the architectural drawings.

1.75

1.75

1.75

1.75

Qty Plies

2

2

2

Qty

Qty Plies

Plies

Beam/Girder

fasteners

14 16d

12 10d

46 16d

30 16d

Qty

LinFt

Pcs Length

16-0-0

14-0-0

6-0-0

4-0-0

18-0-0

18-0-0

4-0-0

14-0-0

12-0-0

4-0-0

Pcs Length

Pcs Length

Varies 37-0-0

Supported

Member

fasteners

6 10d

1 #8x1 1/4WS

16 16d

10 16d

14-0-0

Pcs Length

2

2

2

10

8

4

3

4

30

13

3

13

Forex 2.0E-3000Fb LVL 1.75 X 11.875

64 JARDIN DR, SUITE 3A VAUGHAN, ON L4K 3P3 Model: AMFLIA 2 Date: MAY 22,2018 REV: 2

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE

5. CCMC -12787-R APA PR-L310(C)

2. Nascor CCMC - 13535-R

3. LVL CCMC -12904-R

4. CAN/CSA-O86-09

1. OBC 2012 O.Reg 332/12 as amended



EWP Studio Version 18.32.085 Powered by iStruct^{TA}

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

480 LL Span L/ Double joist only require filler/backer ply when supporting another member using a face-mounted hanger. TL Span L/ 360 Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. 480 LL Cant 2L/ Install single-ply flush window header along inside face of rimboard/rimjoist TL Cant 2L/ 360 Refer to Nascor specifier guide for installation details. Squash blocks recommended to be installed at end bearing on all first level Deflection Girder joists which support loading from above exceeding two levels floor or roof. LL Span L/ 360 Load transfer blocks to be installed under all point loads TL Span L/ 240 It shall be the framer's responsibility that floor joists and beams LL Cant 2L/ 480 are fastened as per the hanger manufacturer's standards. TL Cant 2L/ 360 Refer to Multiple Member Connection Detail to ply to ply nailing or bolting Decking OSB Deck Thickness 3/4" Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. Fastener Nailed & Glued All other components and structural elements supporting Vibration the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the Building Code O. Reg 332/12 He Ontario allatch area represents ceramic tiled floor with an additional dead load of 5 PSF. God M-2057 The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior Architectural Drawing Info Point Load Support JARDIN DESIGN GROUP Load from Above

Architectural Drawing Info

JARDIN DESIGN GROUP 64 JARDIN DR. SUITE 3A VAUGHAN, ON L4K 3P3 Proiect # 17-55 Model: AMELIA 2 Date: MAY 22,2018

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -12904-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

JOISTS SPACING 16"O/C UNLESS NOTED OTHERWISE

THIS CERTIFICATION IS TO CONFIRM THAT:

1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.

2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS, COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



LVL/LSL (Flush) Label Description Width Depth Qty Plies Pcs Length F10 1.75 Forex 11.875 2 2 2.0E-3000Fb LVL Layout Name 14-0-0 F8 Forex 1.75 11.875 2 2 2.0E-3000Fb LVL LVL/LSL (Dropped) Design Method Label Description Width Qty Plies Pcs Length Depth LSD BM4 Forex 1.75 9.5 3 3 12-0-0 Description 2.0E-3000Fb LVL **GREEN YORK HOMES** вмз Forex 1.75 9.5 6-0-0 2.0E-3000Fb LVL Joist (Flush) Created Label Description Width Depth Qty Plies Pcs Length May 28, 2018 J1 NJ40U 3.5 11.875 17 18-0-0 J8 NJ60H 17 Builder 2.5 11.875 18-0-0 J7 NJ60H 2.5 11.875 16-0-0 Sales Rep J4 NJH 2.5 11.875 14-0-0 44 Designer J3 NJH 2.5 11.875 11 12-0-0 F13 NJH 2.5 11.875 2 4 18-0-0 Rim Board Shipping Width | Depth Plies Label Description Qty Pcs Length Project R1 Norbord Rimboar 1.125 18 11.875 **Builder's Project** Plus 1.125 X 11.875 Blocking Label Description Width Depth Qty Plies Pcs Length Stouffville, Ontario BLK2 NJ60H 2.5 11.875 LinFt Varies 11-0-0 Canada BLK2 NJH 2.5 11.875 LinFt Varies 1-0-0 Hanger 905-642-4400 Beam/Girder Supported Second Floor Member

Label Pcs Description Skew Slope fasteners fasteners H3 11 LF2511 12 10d 1 #8x1 1/4WS 2 HGUS410 46 16d 16 16d

H4 NOTES:

Second Floor

Framer to verify dimensions on the architectural drawings.

Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls. Install single-ply flush window header along inside face of rimboard/rimjoist

Refer to Nascor specifier guide for installation details.

Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof. Load transfer blocks to be installed under all point loads.

It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth) @ 16" o/c. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Hatch area represents ceramic tiled floor with an additional dead load

The framing shown on this layout may be deviate from the architectural drawings. Project Engineer to review and approve the deviation prior

Legend



Point Load Support Load from Above Norbord Rimboard Plus 1.125 X 11.875

NJ40U 11.875 NJ60H 11.875 NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875

5.25 X 10.25 (Dropped)

AMELIA 2 EL- 1

BRAMPTON,ON

GRANELLI HOMES PROJECT

Kott Lumber Company

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360 240

480

360

OSB

5/8"

Nailed & Glued

Gypsum 1/2"

14 Anderson Blvd

14A7X4

Floor

Loads

Live

Dead

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceiling:

Deck

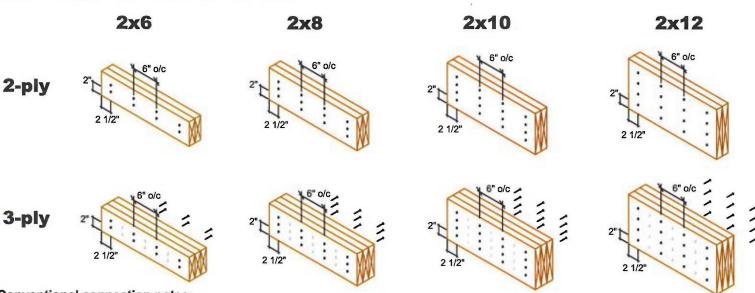
Deflection Girder

EWP Studio Simpson Strong-Tie® Component Solutions™

ULTIPLE MEMBER CONNECTIONS

GREEN YORK HOMES-BRAMPTON-ON-AMELIA 2 ELE-1-2

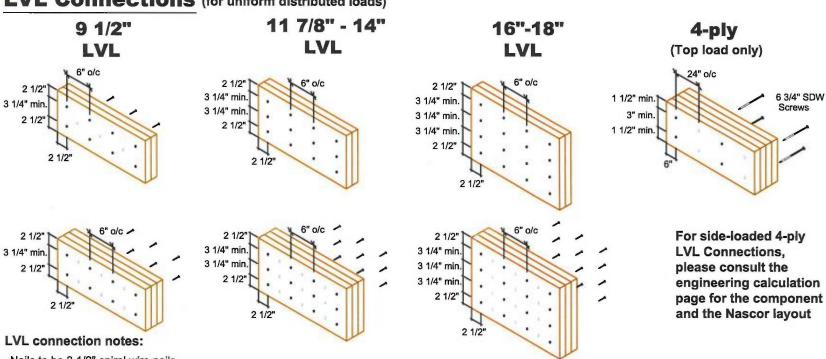
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

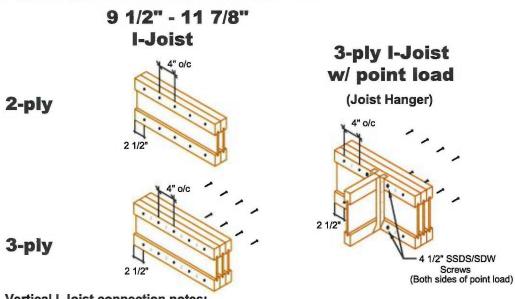
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION **DETAILS**

Date: November 30, 2016

Scale: NTS

3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775 Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

GREEN YORK HOMES-BRAMPTON-ON-AMELIA 2 ELE-1-2

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



Client: Project: Address:

5/30/2018 Date:

SB Designer:

Job Name: AMELIA 2 EL- 1

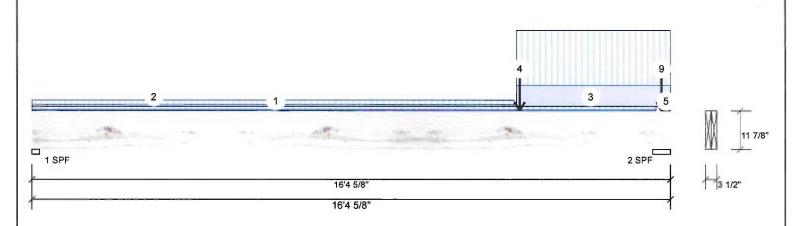
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor



Member Info	ormation			Unfacto	red Reac	tions U	INPATTERN	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	970		457		0	0
Moisture Condit	tion: Dry	Building Code:	NBCC 2010 / OBC 2012	2	3301		1385		0	0
Deflection LL:	360	Load Sharing:	No	1 -						
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	2.375"	40%	571 / 1454	2025	L	1.25D+1.5L
				2 SPE	5.500"	56%	1731 / 4951	6682	1	1 25D+1 5I

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	19001 ft-lb	12'6 3/16"	34261 ft-lb	0.555 (55%)	1.25D+1.5L	L
Unbraced	19001 ft-lb	12'6 3/16"	22688 ft-lb	0.837 (84%)	1.25D+1.5L	L
Shear	5852 lb	15'	11596 lb	0.505 (50%)	1.25D+1.5L	L
Perm Defl in.	0.158 (L/1204)	8'11 7/8"	0.528 (L/360)	0.300 (30%)	D	Uniform
LL Defl inch	0.363 (L/523)	9'1"	0.528 (L/360)	0.690 (69%)	L	L
TL Defl inch	0.522 (L/365)	9' 5/8"	0.793 (L/240)	0.660 (66%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



1.25D+1.5L 1.25D+1.5L

, Laterar c	Mendenness radio bases	on ran occuent main.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-0-2	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-4-7	(Span) 0-11-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	12-5-3 to 16-4-10		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
4	Point	12-6-3		Near Face	1124 lb	2798 lb	0 lb	0 lb	F9
5	Part. Uniform	16-1-14 to 16-4-10		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

and ling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes to pedge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: Date: 5/30/2018 Designer: SB

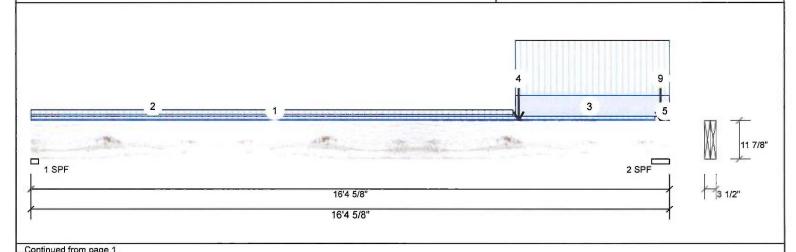
Job Name: AMELIA 2 EL- 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Continued from p	age i							
ID	Load Type	Location Trib	Width Side	Dead	Live	Snow	Wind	Comments
6	Point	16-1-14	Тор	10 lb	26 lb	0 lb	0 lb	J1
7	Point	16-1-14	Тор	2 lb	5 lb	0 lb	0 lb	J1
8	Point	16-1-14	Тор	11 lb	30 lb	0 lb	0 lb	J4
9	Point	16-1-14	Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Self Weight			10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to venify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address:

5/30/2018 Date:

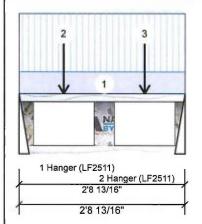
Designer: SB

Job Name: AMELIA 2 EL- 1

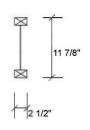
Project #:

11.875" - PASSED F11-A NJH

Level: Ground Floor



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Wind

0

0

Page 1 of 1

Member Info	rmation			Unfacto	red Reac	tions L	INPATTERN	ED Ib	(Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w
Plies:	1	Design Method:	LSD	1	277		104		0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	287		107		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load						_			
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1-	2.000"	34%	130 / 416	546	L

Rearings	and	Factored	Reactions	

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	34%	130 / 416	546	L	1.25D+1.5L	
2 - Hanger	2.000"	35%	134 / 430	565	L	1.25D+1.5L	

Analysis Results Analysis Actual Location Allowed Capacity Comb. Moment 333 ft-lb 1' 5/16" 5390 ft-lb 0.062 (6%) 1.25D+1.5L L 333 ft-lb 1' 5/16" 4941 ft-lb 0.067 (7%) 1.25D+1.5L L Unbraced 559 lb 2'7 9/16" 1810 lb 0.309 (31%) 1.25D+1.5L L Shear Perm Defl in. 0.001 1'3 1/16" 0.084 (L/360) 0.020 (2%) D Uniform (L/22796) LL Defl inch 0.004 (L/8538) 1'3 1/16" 0.084 (L/360) 0.040 (4%) L 1

1'3 1/16" 0.126 (L/240) 0.040 (4%) D+L

Design Notes

1 Fill all hanger nailing holes.

TL Defl inch 0.005 (L/6212)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-8-13	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-8-13		Far Face	94 lb	251 lb	0 lb	0 lb	J3
3	Point	2-0-13		Far Face	91 lb	243 lb	0 lb	0 lb	J3



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Librist flanges must not be cut or drilled.
 Refer to latest copy of the Librist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details.
 Demaged Librists must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







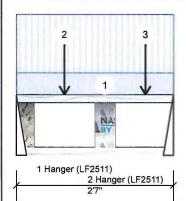
Client: Project: Address: Date: 5/30/2018 Designer:

Job Name: AMELIA 2 EL- 1

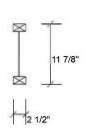
Project #:

11.875" - PASSED F11-B NJH

Level: Ground Floor



2'7"



Wind

0

PAGE / UF 30

Page 1 of 1

Member Information

Γ	Type:	Girder	Application:	Floor (Residential)
l	Plies:	1	Design Method:	LSD
l	Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
l	Deflection LL:	360	Load Sharing:	No
l	Deflection TL:	240	Deck:	Not Checked
l	Importance:	Normal	Vibration:	Not Checked
l	General Load			
l	Floor Live:	40 PSF		
	Dead:	15 PSF		
ı				

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live

320

382

2

Hanger

Dead

120

143

	Bearing:	s and Fac	tored F	Reactions				
ľ	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	2.000"	39%	150 / 480	630	L	1.25D+1.5L	
l	2 -	2.000"	46%	179 / 572	751	L	1.25D+1.5L	

Snow

0

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	417 ft-lb	9 1/2"	5390 ft-lb	0.077 (8%)	1.25D+1.5L	L
l	Unbraced	417 ft-lb	9 1/2"	5011 ft-lb	0.083 (8%)	1.25D+1.5L	L
l	Shear	746 lb	2'5 3/4"	1810 lb	0.412 (41%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/18292)	9 1/2"	0.079 (L/360)	0.020 (2%)	D	Uniform
l	LL Defl inch	0.004 (L/6850)	9 1/2"	0.079 (L/360)	0.050 (5%)	L	L
l	TL Defl inch	0.006 (L/4984)	9 1/2"	0.119 (L/240)	0.050 (5%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

ID	Load Type	Location Trib	Width Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 2-7-0 (Sp	an)1-3-7 Top	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-9-8	Far Face	128 lb	342 lb	0 lb	0 lb	J5	
3	Point	2-1-8	Far Face	110 lb	293 lb	0 lb	0 lb	J5	



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Julst fanges must not be out or drilled

 2. Refer to latest copy of the I Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged Lioists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







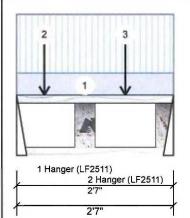
EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address: Date: 5/30/2018

Designer: Job Name: AMELIA 2 EL- 1

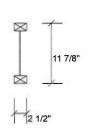
Project #:

F11-C 11.875" - PASSED NJH





15 PSF



Ld. Comb.

1.25D+1.5L

1.25D+1.5L

wember intorn	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)											
Live	Dead	Snow	Wind								
384	144	0	0								
322	121	0	0								
	Live 384	Live Dead 384 144	Live Dead Snow 384 144 0	Live Dead Snow Wind 384 144 0 0							

180 / 576

151 / 482

Total Ld. Case

756 L

633 L

Cap. React D/L lb

Analysis Results

Dead:

ŀ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	420 ft-lb	1'9 1/2"	5390 ft-lb	0.078 (8%)	1.25D+1.5L	L
	Unbraced	420 ft-lb	1'9 1/2"	5011 ft-lb	0.084 (8%)	1.25D+1.5L	L
	Shear	751 lb	1 1/4"	1810 lb	0.415 (41%)	1.25D+1.5L	L
	Perm Defl in.	0.002 (L/18157)	1'9 1/2"	0.079 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.004 (L/6812)	1'9 1/2"	0.079 (L/360)	0.050 (5%)	L	L
L	TL Defl inch	0.006 (L/4954)	1'9 1/2"	0.119 (L/240)	0.050 (5%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

47%

39%

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL CINT I CARE OVER BEARING

Bearings and Factored Reactions

Bearing Length

Hanger

Hanger

2 -

2.000"

2.000"

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.

4 Bottom hange braced at bearings.						POINT LOADS OVER BEARINGS.				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 2-7-0	(Span)1-3-7 to 1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-5-8		Far Face	111 lb	295 lb	0 lb	0 lb	J5	
3	Point	1-9-8		Far Face	129 lb	344 lb	0 lb	0 lb	J5	



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Libist flanges must not be cut or drilled Refer to latest copy of the IJoist product Information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/er
- anadungerection details
 Demaged Jloists must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering order.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Project: Address:

5/30/2018 Date:

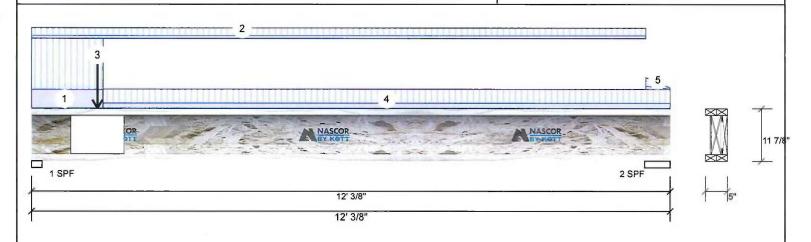
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #

11.875" F12-A 2-Ply - PASSED NJH

Level: Ground Floor



Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Girder Type: Application: Floor (Residential) Live Dead Snow Wind Plies: 2 Design Method: 474 177 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 2 193 72 0 0 Deflection LL: Load Sharing: 360 Deflection TL: Deck: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" 28% 222 / 711 933 1.25D+1.5L 2 - SPF 5.754" 10% 90 / 290 1.25D+1.5L 380 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1243 ft-lb	4'8 13/16"	10780 ft-lb	0.115 (12%)	1.25D+1.5L	L
Unbraced	1243 ft-lb	4'8 13/16"	1248 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	914 lb	1 5/8"	3620 lb	0.252 (25%)	1.25D+1.5L	L
Perm Defl in.	0.011 (L/12159)	5'6 3/16"	0.383 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.030 (L/4552)	5'6 3/16"	0.383 (L/360)	0.080 (8%)	L	L
TL Defl inch	0.042 (L/3312)	5'6 3/16"	0.574 (L/240)	0.070 (7%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2, THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 11'2" o.c.

5 Bottom flange braced at bearings.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARING

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)3-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 11-6-14	(Span)0-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	107 lb	287 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 12-0-6	(Span)0-10-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	11-6-14 to 12-0-6	(Span)0-6-5 to 0-0-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	



TAGE & OF 30

Page 1 of 1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

chemicals

- Handling & Installation
- Light flanges must not be cut or drilled.

 Light flanges must not be cut or drilled. Refer to latest copy of the IJoist product information details for framing dotalls, stiffener tables, web hole chart, bridging details, multiply fastening details and handling/erection details.

 Damaged IJoist must not be used.

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid
- lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load peaning length>= 3.5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







Wind 0 0

Ld. Comb. 1.25D+1.5L

1.25D+1.5L

Page 1 of 1

Client: Project: Address:

5/30/2018 Date:

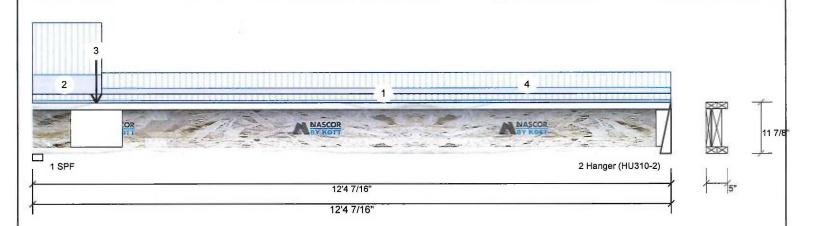
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F12-B NJH

Level: Ground Floor



2 -

Hanger

2.500"

Member Inform	nation			Unfacto	red Reac	tions U	INPATTERN	ED lb ((Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	w
Plies:	2	Design Method:	LSD	1	473		178		0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	193		72		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	28%	222 / 710	932	L

Analysis Res	ults
--------------	------

Α	nalysis	Actual	Location	Allowed	Capacity	Comb.	Case
M	loment	1321 ft-lb	5'1 9/16"	10780 ft-lb	0.123 (12%)	1.25D+1.5L	L
U	nbraced	1321 ft-lb	5'1 9/16"	1322 ft-lb	0.999 (100%)	1.25D+1.5L	L
s	hear	913 lb	1 5/8"	3620 lb	0.252 (25%)	1.25D+1.5L	L
Р	erm Defl in.	0.013 (L/11026)	5'10 1/8"	0.403 (L/360)	0.030 (3%)	D	Uniform
LI	L Defl inch	0.035 (L/4136)	5'10 1/8"	0.403 (L/360)	0.090 (9%)	L	L
Т	L Defl inch	0.048 (L/3008)	5'10 1/8"	0.604 (L/240)	0.080 (8%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 10'10" o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 2.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT

10%

91 / 290

380 L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

/ WOD Sunctions	required at bearing L.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-4-7	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-1-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Near Face	104 lb	277 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 12-4-7	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Uses not to be treated with fire retardant or corrosive

Handling & Installation

- 1. Judist flanges must not be cut or drilled
 2. Refer to laster copy of the Judist product information details for framing details, subject tables, web hole chart, bridging details, multi-py frastening details and handling/erection details
 2. Design assumes too flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4



Jun 04, 2018

EL-MASRI E



INEUD 10-U I FAGE II OF 30

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address:

5/30/2018 Date:

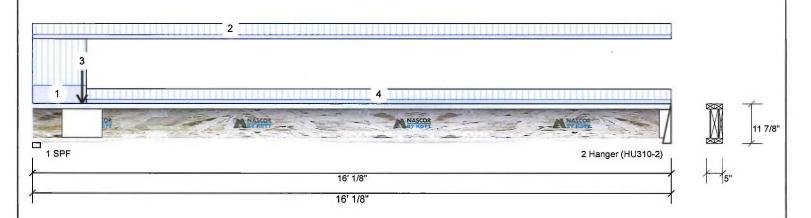
SB Designer:

Job Name: AMELIA 2 EL- 1

Project #:

2-Ply - PASSED 11.875" F13-C NJH

Level: Ground Floor



Member Info	rmation			Unfactor	ed React	ions UNPATTERN	ED lb (Uplif
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow
Plies:	2	Design Method:	LSD	1	629	236	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	243	91	0
Deflection LL:	360	Load Sharing:	No	1 -			
Deflection TL:	240	Deck:	Not Checked				
Importance:	Normal	Vibration:	Not Checked				
General Load							
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions	
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Ca

Analysis Results

ı	,						
Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	2103 ft-lb	6'10 15/16"	10780 ft-lb	0.195 (20%)	1.25D+1.5L	L
	Unbraced	2103 ft-lb	6'10 15/16"	2107 ft-lb	0.998 (100%)	1.25D+1.5L	L
l	Shear	1219 lb	1 5/8"	3620 lb	0.337 (34%)	1.25D+1.5L	L
١	Perm Defl in.	0.033 (L/5696)	7'8"	0.524 (L/360)	0.060 (6%)	D	Uniform
١	LL Defl inch	0.088 (L/2135)	7'8"	0.524 (L/360)	0.170 (17%)	L	L
ı	TL Defl inch	0.122 (L/1553)	7'8"	0.786 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 9' o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 2.

Unfactored Reactions	UNPATTERNED	lb (Uplift)
----------------------	-------------	-------------

Brg	Live	Dead	Snow	Wind	
1	629	236	0	0	
2	243	91	0	0	
1					

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	37%	295 / 944	1238	L	1.25D+1.5L
2 - Hanger	2.500"	13%	114 / 364	478	L	1.25D+1.5L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

		3							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 16-0-2	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	143 lb	382 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 16-0-2	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Jun 04 2018

Page 1 of 1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the deelign criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Librist flanges must not be cut or drilled Refer to latest copy of the IJoist product information details for framing details, stiftener tables, web hole chart, bridging details, multi-ply fastening details and handinglerection details Damaged IJoists must not be used Deging assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=2.5 inches
 For flat roofs provide proper drainage to prevent

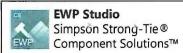
Manufacturer Info

Nascor by Kott









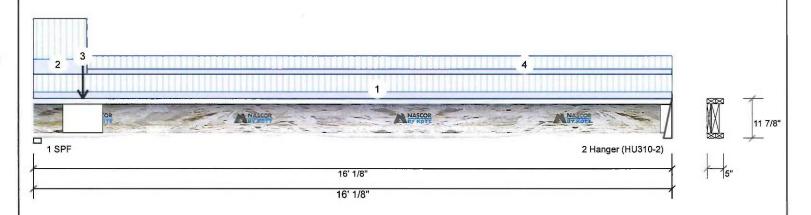
Client: Project: Address:

5/30/2018 Date: Designer: SB

Job Name: AMELIA 2 EL- 1

11.875" 2-Ply - PASSED F13-D NJH

Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions	UNPATTERNED	lb	(Uplift)	
----------------------	-------------	----	----------	--

Brg	Live	Dead	Snow	VVind	
1	723	271	0	0	
2	398	149	0	0	
I					

Bearings and Factored Reactions

I	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
I	1 - SPF	2.375"	43%	339 / 1084	1423	L	1.25D+1.5L
1	2 - Hanger	2.500"	22%	187 / 597	784	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3230 ft-lb	7'5 3/4"	10780 ft-lb	0.300 (30%)	1.25D+1.5L	L
Unbraced	3230 ft-lb	7'5 3/4"	3243 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1400 lb	1 5/8"	3620 lb	0.387 (39%)	1.25D+1.5L	L
Perm Defl in.	0.051 (L/3734)	7'9 13/16"	0.524 (L/360)	0.100 (10%)	D	Uniform
LL Defl inch	0.135 (L/1400)	7'9 13/16"	0.524 (L/360)	0.260 (26%)	L	L
TL Defl inch	0.185 (L/1018)	7'9 13/16"	0.786 (L/240)	0.240 (24%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange must be laterally braced at a maximum of 7'5" o.c.
- 6 Bottom flange braced at bearings.
- 7 Web stiffeners required at Bearing 2.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 16-0-2	(Span)1-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Near Face	120 lb	320 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 16-0-2	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Jun 04, 2018

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Joint flanges must not be out or drilled Refer to latest copy of the I Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged I Joists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 7. For flat roofs provide proper drainage to prevent populing.

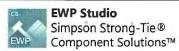
Manufacturer Info

Nascor by Kott









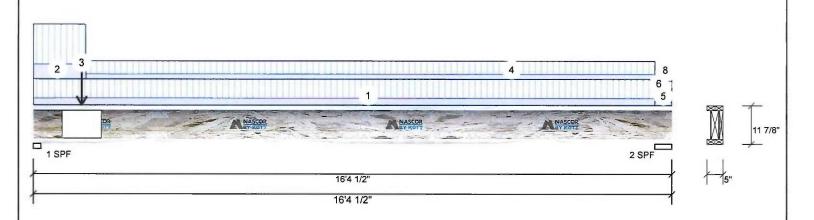
Client: Project: Address: Date: 5/30/2018 Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

2-Ply - PASSED F13-E NJH 11.875"

Level: Ground Floor



Member Infor	mation			Unfactor	ed React	tions Ul	NPATTERN	ED
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	
Plies:	2	Design Method:	LSD	1	741		278	
Moisture Condition	i: Dry	Building Code:	NBCC 2010 / OBC 2012	2	416		157	
Deflection LL:	360	Load Sharing:	No	_				
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fac	tored R	eactions	
Dead:	15 PSF			Bearing I	_ength	Cap.	React D/L lb	7
				1 - SPF 2	2.375"	44%	348 / 1112	1

Unfact	ored	Reactions	UNPAT	TERNED	lb	(Uplift)	

Bearings and Factored Reactions										
	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
	1 - SPF	2.375"	44%	348 / 1112	1460	L	1.25D+1.5L			
_	2 - SPF	5.250"	23%	196 / 624	821	L	1.25D+1.5L			

Snow

0

Wind

0

Analysis Results

Ţ,	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	3385 ft-lb	7'6 13/16"	10780 ft-lb	0.314 (31%)	1.25D+1.5L	L
'	Unbraced	3385 ft-lb	7'6 13/16"	3390 ft-lb	0.999 (100%)	1.25D+1.5L	L
:	Shear	1436 lb	1 5/8"	3620 lb	0.397 (40%)	1.25D+1.5L	L
	Perm Defl in.	0.054 (L/3540)	7'10 3/4"	0.529 (L/360)	0.100 (10%)	D	Uniform
	L Defl inch	0.143 (L/1328)	7'10 3/4"	0.529 (L/360)	0.270 (27%)	L	L
-	TL Defl inch	0.197 (L/966)	7'10 3/4"	0.793 (L/240)	0.250 (25%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 7'3" o.c.

5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-11-4	(Span)1-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-2-14		Far Face	121 lb	322 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 15-11-4	(Span)1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	15-11-4 to 16-4-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	15-11-4 to 16-4-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Continued on	page 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Libint flanges must not be cut or drilled Rofer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged IJoists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent populing.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Manufacturer Info

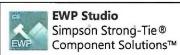
Nascor by Kott







Page 2 of 2



Client: Project: Address:

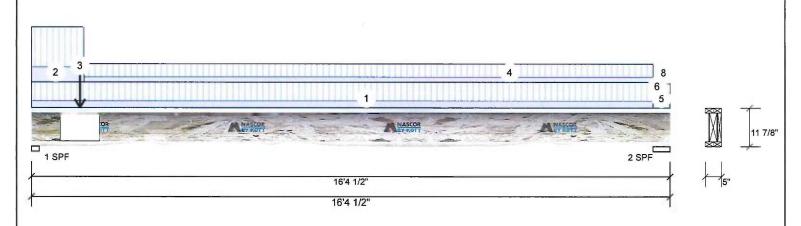
5/30/2018 Date: Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-E NJH

Level: Ground Floor



.Continued from page 1

l	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	7	Part. Uniform	16-1-14 to 16-4-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
ı	8	Part. Uniform	16-1-14 to 16-4-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- Installation
 I. Joist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details multi-ply fastening details and handling/erection details
 I. Damaged IJoist must not be used
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffenors for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client: Project: Address: Date: 5/30/2018

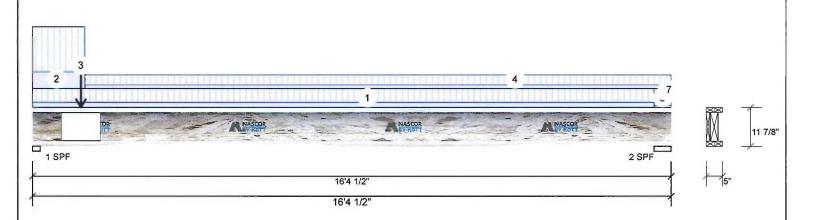
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-F NJH

Level: Ground Floor



Member Inforn	nation			Unfacto	red Reac	tions U	INPATTERNI	ED lb ((Upli
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W
Plies:	2	Design Method:	LSD	1	673		252		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	287		108		0
Deflection LL:	360	Load Sharing:	No	1 7					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored I	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. C
				1 - SPF	2.375"	40%	316 / 1010	1326	L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2438 ft-lb	7'1 7/8"	10780 ft-lb	0.226 (23%)	1.25D+1.5L	L
Unbraced	2438 ft-lb	7'1 7/8"	2454 ft-lb	0.994 (99%)	1.25D+1.5L	L
Shear	1305 lb	1 5/8"	3620 lb	0.360 (36%)	1.25D+1.5L	L
Perm Defl in.	0.039 (L/4884)	7'9 3/8"	0.529 (L/360)	0.070 (7%)	D	Uniform
LL Defl inch	0.104 (L/1832)	7'9 3/8"	0.529 (L/360)	0.200 (20%)	L	L
TL Defl inch	0.143 (L/1332)	7'9 3/8"	0.793 (L/240)	0.180 (18%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'5" o.c.

5 Pottom flance braced at bearings

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)

1	6/3	252	U	U
2	287	108	0	0

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.375"	40%	316 / 1010	1326	L	1.25D+1.5L	
2-SPF	5.250"	16%	135 / 430	566	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

5 Bottom fl.	ange braced at bearing	s.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-11-4	(Span)0-11-0 to 0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-2	(Span)3-0-0 to 3-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	SOFESSIONAL
3	Point	1-2-14		Near Face	144 lb	384 lb	0 lb	0 lb	F11
4	Tie-In	1-4-2 to 16-4-8	(Span)0-8-0 to 0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	N.A. EL-MASRI
5	Tie-In	15-11-4 to 16-4-8	(Span)0-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	Char Engage
6	Part. Uniform	16-1-14 to 16-4-8		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Jun 049 2018
7	Part. Uniform	16-1-14 to 16-4-8		Тор	1 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LIGHTHING ON INSTAILATION

 Libratis frames must not be cut or drilled

 Refer to latest copy of the Libratis product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged Lipratis must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott

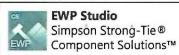


Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400



Wind





Client: Project: Address: Date: 5/30/2018

Designer: SB

Job Name: AMELIA 2 EL- 1

Level: Ground Floor

Project #:

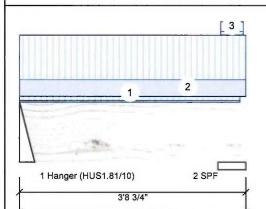
F5-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" - PASSED

Floor (Residential)

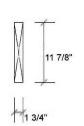
No Not Checked

Not Checked

NBCC 2010 / OBC 2012



3'8 3/4"



Wind

Type:	Girder
Plies:	1
Moisture Conditi	on: Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Norma
General Load	
Floor Live:	40 PSF
Dead:	15 PSF
Analysis Resu	ılts
Analysis /	Actual
	10 0 11

Unfactored Reactions UNPATTERNED Ib (Uplift)

1	462	182	0	0
2	534	210	0	0

Snow

Dead

Bearings and Factored Reactions

Live

	,						
Bearing	Length	Cap. R	leact D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	3.000"	24%	227 / 693	920	L	1.25D+1.5L	
•	5 500"	18%	262 / 801	1063	T.	1 25D±1 5I	

Location Allowed Capacity Comb. Moment 646 ft-lb 1'9 1/8" 17130 ft-lb 0.038 (4%) 1.25D+1.5L L Unbraced 646 ft-lb 1'9 1/8" 13452 ft-lb 0.048 (5%) 1.25D+1.5L L 304 lb 1'2 1/8" 5798 lb 0.052 (5%) 1.25D+1.5L L Shear Perm Defl in. 0.001 1'9 1/8" 0.105 (L/360) 0.010 (1%) D Uniform (L/32215) 0.003 1'9 1/8" 0.105 (L/360) 0.030 (3%) L LL Defl inch L

(L/12665) TL Defl inch 0.004 (L/9091) 1'9 1/8" 0.157 (L/240) 0.030 (3%) D+L

Application:

Design Method: **Building Code:**

Load Sharing:

Deck:

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-7-9	(Span)1-1-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Tie-In	3-3-12 to 3-8-3	(Span)2-7-10 to 2-7-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

L



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address: Date: 5/30/2018

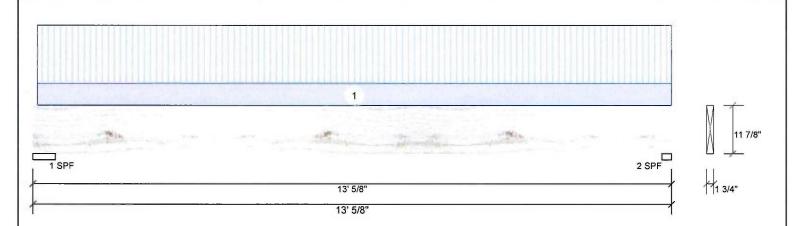
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

1.750" X 11.875" - PASSED F6-A Forex 2.0E-3000Fb LVL

Level: Ground Floor



Member Inform	nation			Unfactor	ed Reacti	ons U	NPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	1	Wind
Plies:	1	Design Method:	LSD	1	120		77	C)	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	117		74	()	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fact	ored R	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	5.500"	5%	96 / 180	276	L	1.25D+1.5L
				2 - SPF :	2.375"	11%	93 / 176	269	L	1.25D+1.5L
Analysis Result	S		02					_		

Analysis Actual 824 ft-lb Moment 824 ft-lb Unbraced 222 lb Shear

Location Allowed Capacity Comb. 6'7 7/8" 17130 ft-lb 0.048 (5%) 1.25D+1.5L L 0.229 (23%) 1.25D+1.5L L 6'7 7/8" 3591 ft-lb 0.038 (4%) 1.25D+1.5L L 1'4 5/8" 5798 lb Perm Defl in. 0.014 6'7 7/8" 0.417 (L/360) 0.030 (3%) D Uniform (L/10405) LL Defl inch 0.023 (L/6605) 6'7 7/8" 0.417 (L/360) 0.050 (5%) L L

6'7 7/8" 0.626 (L/240) 0.060 (6%) D+L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

TL Defl inch 0.037 (L/4040)

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-2 to 13-0-10	(Ѕрап)0-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				



FAGE II OF 30

Page 1 of 1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

READ ALL NOTES ON THIS PAGE AND ON

NOTE PAGE IS AN INTEGRAL PART OF THIS

CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER

ENGINEERING NOTE PAGE ENP-2. THIS

CALCULATION SUMMARY PAGE AS IT

CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS. PASS THRU FRAMING SQUASH

BLOCK IS REQUIRED AT ALL

POINT LOADS OVER BEARINGS.







EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: Date: 5/30/2018

Designer: SB

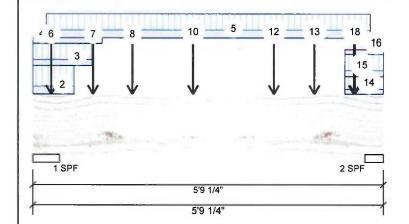
Job Name: AMELIA 2 EL- 1

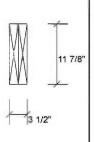
Project #:

F7-A Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Level: Ground Floor





Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal

Member Information

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No Deck:

Vibration:

Not Checked Not Checked Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	3319	1453	0	0
2	3053	1319	0	0

Bearings and Factored Reactions 40 PSF 15 PSF Bearing Length

Cap. React D/L lb Total Ld. Case Ld. Comb. 6795 L 1 - SPF 5.250" 60% 1817 / 4978 1.25D+1.5L 2 - SPF 3.625" 80% 1648 / 4579 6227 L 1.25D+1.5L

Analysis Results

General Load

Floor Live:

Dead:

L	,						
ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	5096 ft-lb	2'7 5/8"	34261 ft-lb	0.149 (15%)	1.25D+1.5L	L
l	Unbraced	5096 ft-lb	2'7 5/8"	33024 ft-lb	0.154 (15%)	1.25D+1.5L	L
l	Shear	4068 lb	1'4 3/8"	11596 lb	0.351 (35%)	1.25D+1.5L	L
l	Perm Defl in.	0.008 (L/7346)	2'9 3/4"	0.172 (L/360)	0.050 (5%)	D	Uniform
l	LL Defl inch	0.019 (L/3189)	2'9 1/2"	0.172 (L/360)	0.110 (11%)	L	L
	TL Defl inch	0.028 (L/2224)	2'9 9/16"	0.258 (L/240)	0.110 (11%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Late	ral slenderness ratio based o	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
2	Part. Uniform	0-0-0 to 0-8-2		Тор	122 PLF	326 PLF	0 PLF	0 PLF	J1
3	Part. Uniform	0-0-0 to 0-11-10		Тор	96 PLF	255 PLF	0 PLF	0 PLF	J4
4	Part. Uniform	0-0-0 to 1-1-10		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Part. Uniform	0-2-10 to 5-6-10		Far Face	123 PLF	253 PLF	0 PLF	0 PLF	
6	Point	0-3-10		Near Face	111 lb	296 lb	0 lb	0 lb	J8

For flat roofs provide proper drainage to prevent ponding

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply regarding installation requirements, multi-ply fastening details, beam strength values, and code

- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid

 lateral displacement and rotation

Manufacturer Info

APA: PR-L318



Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: Date:

5/30/2018 Designer: SB

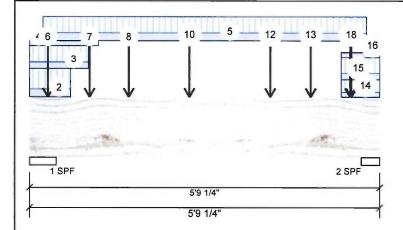
Job Name: AMELIA 2 EL- 1

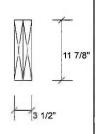
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED

Level: Ground Floor





ł	Continued from p	age 1								
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	7	Point	0-11-14		Тор	520 lb	1340 lb	0 lb	0 lb	ВМЗ ВМЗ
I	8	Point	1-7-10		Near Face	149 lb	398 lb	0 lb	0 lb	F13
I	10	Point	2-7-10		Near Face	128 lb	342 lb	0 lb	0 lb	J5
١	12	Point	3-11-10		Near Face	110 lb	293 lb	0 lb	0 lb	J5
l	13	Point	4-7-10		Near Face	91 lb	243 lb	0 lb	0 lb	F13
l	14	Part. Uniform	5-1-10 to 5-9-4		Тор	75 PLF	199 PLF	0 PLF	0 PLF	J1
l	15	Part. Uniform	5-1-10 to 5-9-4		Тор	96 PLF	255 PLF	0 PLF	0 PLF	J4
l	16	Part. Uniform	5-1-10 to 5-9-4		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
l	17	Point	5-3-7		Тор	445 lb	1139 lb	0 lb	0 lb	ВМЗ ВМЗ
l	18	Point	5-3-10		Near Face	81 lb	215 lb	0 lb	0 lb	J8
١		Self Weight				10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corresive

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







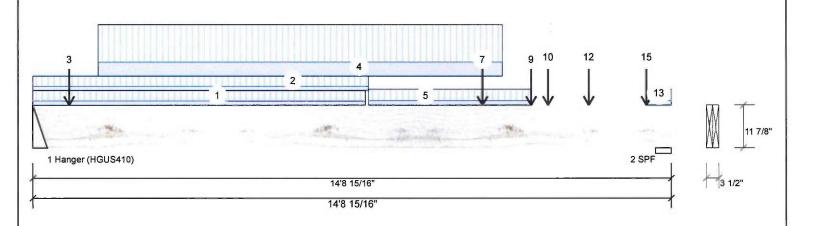
Project: Address: 5/30/2018

Designer: Job Name: AMELIA 2 EL- 1

Project #:

1.750" X 11.875" 2-Ply - PASSED F9-A Forex 2.0E-3000Fb LVL

Level: Ground Floor



Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	2798	1124	0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	2830	1139	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 -	4.000"	54% 1405 / 4197	5602 L	1.25D+1.5L
				Hanger				
Analysis Resul	alysis Results			2 - SPF	4.376"	60% 1424 / 4245	5668 L	1.25D+1.5L

Analysis Actual Location Allowed Capacity Comb. Case Moment 20001 ft-lb 7'5 13/16" 34261 ft-lb 0.584 (58%) 1.25D+1.5L L Unbraced 20001 ft-lb 7'5 13/16" 24916 ft-lb 0.803 (80%) 1.25D+1.5L L 5567 lb 13'5 7/16" 11596 lb 0.480 (48%) 1.25D+1.5L L Shear Perm Defl in. 0.160 (L/1060) 7'4 7/8" 0.472 (L/360) 0.340 (34%) D Uniform 7'4 7/8" 0.472 (L/360) 0.850 (85%) L LL Defl inch 0.400 (L/426) L TL Defl inch 0.560 (L/304) 7'4 7/8" 0.709 (L/240) 0.790 (79%) D+L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED lb (Uplift)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Design Notes

1 Fill all hanger nailing holes.

Member Information

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7. Lateral slenderness ratio based on full section width

15			T 1 145 141	0.1			_	145 1		Т
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 7-8-1	(Span)3-6-1 to 3-4-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 7-8-15	(Span)3-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	0-10-1		Near Face	107 lb	286 lb	0 lb	0 lb	J4	
4	Part. Uniform	1-6-1 to 10-10-1		Near Face	93 PLF	247 PLF	0 PLF	0 PLF		
5	Tie-In	7-9-0 to 11-6-1	(Span)3-9-15 to 3-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	(

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318



Kott Lumber Company 14 Anderson Blvd. Ontario Canada L4A 7X4 905-642-4400



Jun 04 2018



Page 1 of 2

Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: 5/30/2018

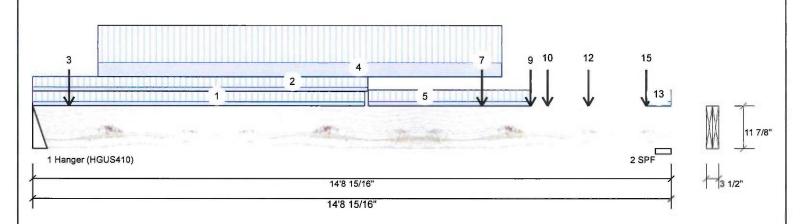
Designer:

Job Name: AMELIA 2 EL- 1

Project #:

F9-A Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Ground Floor



Continued t	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	10-4-9		Тор	13 lb	35 lb	0 lb	0 lb	
7	Point	10-4-9		Far Face	182 lb	462 lb	0 lb	0 lb	F5
8	Point	11-6-1		Far Face	32 lb	84 lb	0 lb	0 lb	J2
9	Point	11-6-1		Near Face	80 lb	213 lb	0 lb	0 lb	J4
10	Point	11-10-12		Near Face	72 lb	193 lb	0 lb	0 lb	F12
11	Point	12-10-1		Far Face	34 lb	92 lb	0 lb	0 lb	J2
12	Point	12-10-1		Near Face	94 lb	251 lb	0 lb	0 lb	J3
13	Tie-In	14-2-1 to 14-8-15	(Span)3-9-14 to 3-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
14	Point	14-2-1		Far Face	25 lb	67 lb	0 lb	0 lb	J2
15	Point	14-2-1		Near Face	91 lb	242 lb	0 lb	0 lb	J3
	Self Weight				10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-pty
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318









Project: Address:

Designer:

Job Name: AMELIA 2 EL- 1

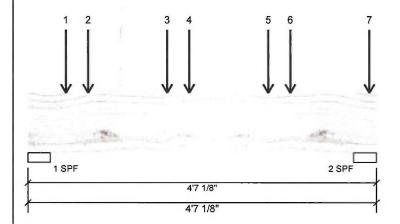
Project #:

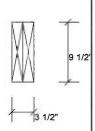
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED Level: Second Floor

5/30/2018





Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance;	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application:

Design Method: **Building Code:** NBCC 2010 / OBC 2012

Load Sharing: Deck:

Vibration:

Not Checked Not Checked

Floor (Residential)

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1340	520	0	0
2	1139	445	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.516"	35%	650 / 2010	2660	L	1.25D+1.5L	
2 - SPF	3.625"	29%	556 / 1709	2265	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2504 ft-lb	2'1 1/2"	22724 ft-lb	0.110 (11%)	1.25D+1.5L	L
Unbraced	2504 ft-lb	2'1 1/2"	22724 ft-lb	0.110 (11%)	1.25D+1.5L	L
Shear	1923 lb	3'6 3/4"	9277 lb	0.207 (21%)	1.25D+1.5L	L
Perm Defl in.	0.005 (L/10742)	2'1 9/16"	0.137 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.012 (L/4171)	2'1 9/16"	0.137 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.016 (L/3005)	2'1 9/16"	0.206 (L/240)	0.080 (8%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



ID	Load Type	Location Trib V	Vidth Side	Dead	Live	Snow	Wind	Comments
1	Point	0-6-0	Тор	162 lb	432 lb	0 lb	0 lb	J1
2	Point	0-9-8	Тор	127 lb	338 lb	0 lb	0 lb	J4
3	Point	1-10-0	Тор	162 lb	432 lb	0 lb	0 lb	J1
4	Point	2-1-8	Тор	127 lb	338 lb	0 lb	0 lb	J4
5	Point	3-2-0	Тор	162 lb	432 lb	0 lb	0 lb	J1

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LIVL beams must not be cut or drilled
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

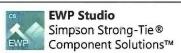
APA: PR-L318







Page 2 of 2



Client: Project: Address:

5/30/2018 Date: SB Designer:

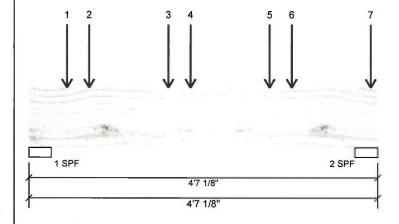
Job Name: AMELIA 2 EL- 1

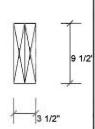
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED Level: Second Floor





..Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Point	3-5-8		Тор	127 lb	338 lb	0 lb	0 lb	J4
7	Point	4-6-0		Тор	63 lb	169 lb	0 lb	0 lb	J1
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the design orthera and loadings shown. It is the design orthera and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 testening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex









Client: Project: Address: Date: 5/30/2018

Designer: S B

Job Name: AMELIA 2 EL- 1

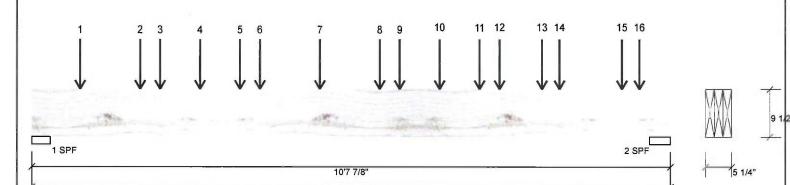
Project #:

BM4-A Forex 2.0E-3000Fb LVL

1.750" X 9.500"

3-Ply - PASSED

Level: Second Floor



10'7 7/8'

Туре:	Girder	Application:	Floor (Residential)
Plies:	3	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF	1	

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)
------------	-----------	-------------	----	----------

Brg	Live	Dead	Snow	Wind
1	2952	1269	0	0
2	3096	1297	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.688"	50%	1586 / 4428	6014	L	1.25D+1.5L	
2 - SPF	4.188"	46%	1621 / 4644	6265	L	1.25D+1.5L	

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	15176 ft-lb	4'9 11/16"	35449 ft-lb	0.428 (43%)	1.25D+1.5L	L
	Unbraced	15176 ft-lb	4'9 11/16"	35449 ft-lb	0.428 (43%)	1.25D+1.5L	L
	Shear	5599 lb	1' 7/16"	13915 lb	0.402 (40%)	1.25D+1.5L	L
	Perm Defl in.	0.086 (L/1413)	5'3 9/16"	0.337 (L/360)	0.250 (25%)	D	Uniform
	LL Defl inch	0.201 (L/605)	5'3 5/8"	0.337 (L/360)	0.600 (60%)	L	L
I	TL Defl inch	0.287 (L/424)	5'3 5/8"	0.506 (L/240)	0.570 (57%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral slenderness ratio based on full section width.

ID Trib Width Load Type Location Side Dead Live Snow Wind Comments Point 0-9-11 278 lb 686 lb 0 lb J1 J4 1 Top 0 lb 2 **Point** 1-9-11 117 lb 257 lb 0 lb 0 lb J4 Top 3 Point 2-1-11 162 lb 431 lb 0 lb 0 lb J1 Top 4 Point 2-9-11 Top 117 lb 257 lb 0 lb J4 5 Point 3-5-11 162 lb 431 lb 0 lb 0 lb J1 Top

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Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

1 Dry son

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

5. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding Manufacturer Info

Forex



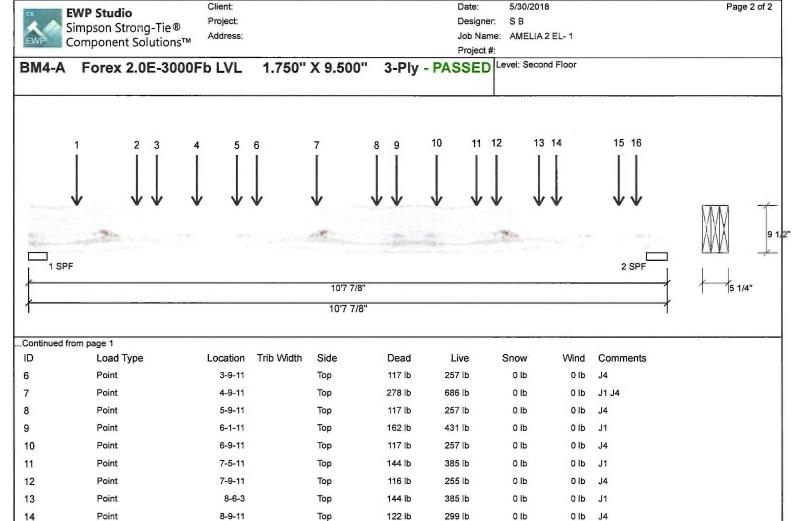
Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400

PROFESSIONAL

-MASRL







163 lb

128 lb

11 PLF

Top

Top

434 lb

340 lb

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

0 lb

0 lb

J1

0 lb

0 lb J4

15

16

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Point

Point

Self Weight

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

9-10-3

10-1-11

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding Installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info







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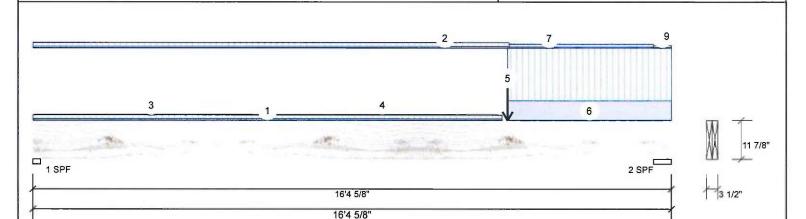


Client: Project: Address: Date: 5/30/2018 Designer:

Job Name: AMELIA 2 EL- 1

Project #:

2-Ply - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL 1.750" X 11.875"



Member Inform	ember Information				Unfactored Reactions		
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		
Plies:	2	Design Method:	LSD	1	818		
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	2494		
Deflection LL:	360	Load Sharing:	No				
Deflection TL:	240	Deck:	Not Checked	1			
Importance:	Normal	Vibration:	Not Checked				
General Load							
Floor Live:	40 PSF			Bearing	s and Fac	tored	
Dead:	15 PSF			Bearing	Length	Cap	

Jnfactored	Reactions	UNPAT	TERNED	lb	(Uplift)	

Brg	Live	Dead	Snow	vvina
1	818	446	0	0
2	2494	1160	0	0

Canu

Total Ld Cone

105-4

Dand

d Reactions

Dearing	Lengui	Cap.	React DIL ID	IUlai	Lu. Case	Lu. Comb.	
1 - SPF	2.375"	35%	557 / 1228	1785	L	1.25D+1.5L	
2-SPF	5 500"	44%	1450 / 3741	5191	1	1.25D+1.5I	

Ponet D/L lb

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15265 ft-lb	12'2 1/4"	34261 ft-lb	0.446 (45%)	1.25D+1.5L	L
Unbraced	15265 ft-lb	12'2 1/4"	22688 ft-lb	0.673 (67%)	1.25D+1.5L	L
Shear	4477 lb	15'	11596 lb	0.386 (39%)	1.25D+1.5L	L
Perm Defl in.	0.145 (L/1312)	8'10 3/4"	0.528 (L/360)	0.270 (27%)	D	Uniform
LL Defl inch	0.290 (L/656)	9'	0.528 (L/360)	0.550 (55%)	L	L
TL Defl inch	0.435 (L/437)	8'11 9/16"	0.793 (L/240)	0.550 (55%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral stenderness ratio based on full section width

0 Lateral	Siendenness rado pased	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comment
1	Tie-In	0-0-0 to 12-0-8	(Span) 0-10-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-2-10	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-9 to 11-11-1		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Tapered Start	0-0-10		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	End	11-11-1			0 PLF	0 PLF	0 PLF	0 PLF	
5	Point	12-2-4		Far Face	862 lb	1824 lb	0 lb	0 lb	F8

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply regarding installation requirements, multi-ply fastening details, beam strength values, and code

applovals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 1 of 2

INEUDIO-UI/ FAGE 29 OF 30 Client: 5/30/2018 Date: Page 2 of 2 **EWP Studio** Project: Designer: S B Simpson Strong-Tie® Address: Job Name: AMELIA 2 EL- 1 Component Solutions™ Project #: Forex 2.0E-3000Fb LVL 1.750" X 11.875" 2-Ply - PASSED Level: Second Floor F10-A 2 5 6 2 SPF 1 SPF

Continued	I from page 1					-			
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Part. Uniform	12-2-4 to 16-4-10		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
7	Tie-In	12-2-10 to 15-11-2	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Tie-In	15-11-2 to 16-4-10	(Span)0-4-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
9	Tie-In	16-0-4 to 16-4-10	(Span)0-11-1	1 Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				

16'4 5/8" 16'4 5/8"

> REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the design criteria and loadings shown. It is the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code

- papervals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client: Project: Address:

5/30/2018 Date:

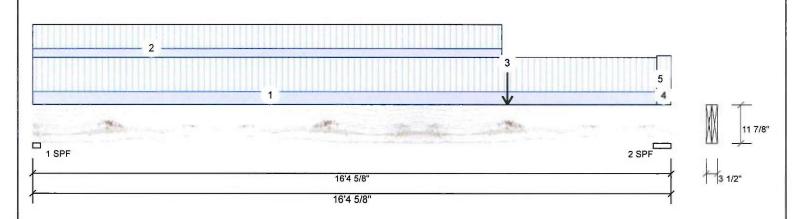
Designer:

Job Name: AMELIA 2 EL- 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED Level: Second Floor



Туре:	Girder	Application:	Floor (Residential)	Br
Plies:	2	Design Method:	LSD	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	Ш
Deflection LL:	360	Load Sharing:	No	
Deflection TL:	240	Deck:	Not Checked	
Importance:	Normal	Vibration:	Not Checked	
General Load				 -
Floor Live:	40 PSF			В
Dead:	15 PSF			
				1

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	700	368	0	0
2	1629	782	0	0

Bearings and Factored Reactions

bearing	Lengin	Cap. N	eact D/L ib	IUlai	Lu. Case	Lu. Comb.	
1 - SPF	2.375"	30%	460 / 1050	1511	L	1.25D+1.5L	
2 - SPF	5.500"	29%	977 / 2443	3420	L	1.25D+1.5L	

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	12574 ft-lb	12'2 1/4"	34261 ft-lb	0.367 (37%)	1.25D+1.5L	L	
Unbraced	12574 ft-lb	12'2 1/4"	22688 ft-lb	0.554 (55%)	1.25D+1.5L	L	
Shear	3351 lb	15'	11596 lb	0.289 (29%)	1.25D+1.5L	L	
Perm Defl in.	0.118 (L/1617)	8'10 3/8"	0.528 (L/360)	0.220 (22%)	D	Uniform	
LL Defl inch	0.241 (L/788)	8'11 7/16"	0.528 (L/360)	0.460 (46%)	L	L	
TL Defl inch	0.359 (L/530)	8'11 1/16"	0.793 (L/240)	0.450 (45%)	D+L	L	

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

L										
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comment
	1	Tie-In	0-0-0 to 16-0-4	(Span)0-11-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 12-0-8	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Point	12-2-4		Near Face	816 lb	1854 lb	0 lb	0 lb	F8
l	4	Tie-In	16-0-4 to 16-4-10	(Span)0-4-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	5	Tie-In	16-0-4 to 16-4-10	(Span)0-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
		Self Weight				10 PLF				



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

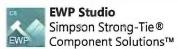
- fastening details, beam strength values, and social approvals
 Damaged Beams must not be used
 Denign assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info For flat roofs provide proper drainage to prevent ponding









Client: Project: Address: Date: 5/30/2018

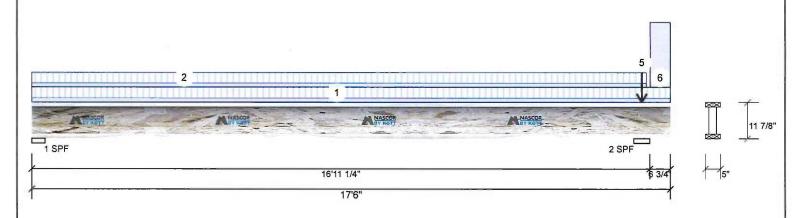
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-A NJH

Level: Second Floor



Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored	Reactions	UNPATTERNED	lb	(Uplift)	

Live	Dead	Snow	Wind	
224	82	0 (-2)	0	
261	300	217	0	
	224	224 82	224 82 0 (-2)	224 82 0 (-2) 0

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L ID	iotai	La. Case	La. Comb.	
1 - SPF	4.375"	12%	102 / 337	440	L_	1.25D+1.5L	
2 - SPF	5.250"	24%	375 / 500	875	LL	1.25D+1.5L	

Analysis Results

Member Information

Analysis Res	uits					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-103 ft-lb	16'11 1/4"	9702 ft-lb	0.011 (1%)	1.25D+1.5S +0.5L	_L
Unbraced	-103 ft-lb	16'11 1/4"	9651 ft-lb	0.011 (1%)	1.25D+1.5S +0.5L	_L
Pos Moment	1711 ft-lb	8'4 1/2"	10780 ft-lb	0.159 (16%)	1.25D+1.5L	L_
Unbraced	1711 ft-lb	8'4 1/2"	1717 ft-lb	0.996 (100%)	1.25D+1.5L	L_
Shear	589 lb	16'11 1/4"	3186 lb	0.185 (18%)	1.25D+1.5S	_L
Perm Defl in.	0.027 (L/7357)	8'4 1/4"	0.542 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.076 (L/2581)	8'5 1/4"	0.542 (L/360)	0.140 (14%)	L	L_
TL Defl inch	0.102 (L/1911)	8'4 15/16"	0.813 (L/240)	0.130 (13%)	D+L	L_
LL Cant	-0.008 (2L/1791)	Rt Cant	0.200 (2L/480)	0.038 (4%)	L	L_
TL Cant	-0.010 (21/1395)	Rt Cant	0.300	0.032 (3%)	D+L	L_

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'9" o.c.
- 5 Bottom flange must be laterally braced at a maximum of 10'5" o.c.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Liust flanges must not be out or dilled Refer to latest copy of the Libist product information details for framing details, stiffener tables, web hole chart, bridging details, multiply fastering details and handling/erection details Damaged Libists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- 5. Provide lateral support at bearing points to avoid
- interior details support at treating points to avoid lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

 7. For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott







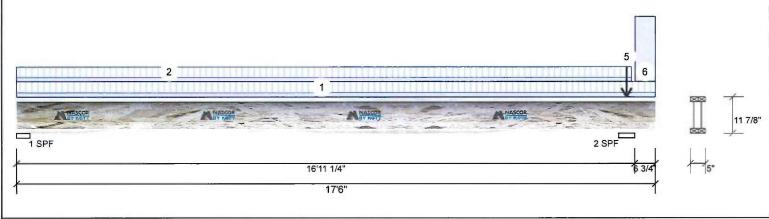
Client: Project: Address: Date: 5/30/2018 Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

2-Ply - PASSED 11.875" F13-A NJH

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 17-6-0	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 16-10-2	(Span)0-7-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	16-8-10		Тор	35 lb	0 lb	0 lb	0 lb	Wall Self Weight
4	Point	16-8-10		Тор	126 lb	27 lb	215 lb	0 lb	F1 F1
5	Point	16-8-10		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight
6	Part. Uniform	16-11-6 to 17-6-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Uloist flanges must not be out or dilled

 2. Refer to latest copy of the I Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged I Joists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2



Client: Project: Address:

5/30/2018 Date:

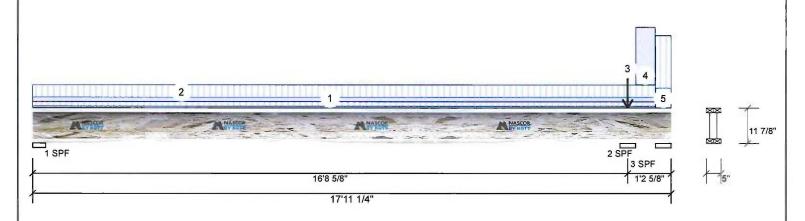
Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-B NJH

Level: Second Floor



Туре:	Girder	Application:	Floor (Residential)	Brg
Plies:	2	Design Method:	LSD	1
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2
Deflection LL:	360	Load Sharing:	No	3
Deflection TL:	240	Deck:	Not Checked	1
Importance:	Normal	Vibration:	Not Checked	1
General Load				-
Floor Live:	40 PSF			Bear
Dead:	15 PSF			Bea
				1 1

Jnfactored	Reactions	UNPATTERNED	lb	(Uplift)
-------------------	-----------	-------------	----	----------

Brg	Live	Dead	Snow	Wind	_
1	171	64	0	0	
2	764	335	0	0	
3	0 (-494)	(-160)	0	0	

rings and Factored Reactions

_								-
	3 - SPF	5.250"	27%	-224 / -737	-961 (-961)	L_	1.25D+1.5L	
	2 - SPF	5.250"	23%	419 / 1146	1565	LL	1.25D+1.5L	
	1 - SPF	4.375"	9%	80 / 257	337	L_	1.25D+1.5L	
	Bearing	Length	Сар.	React D/L Ib	lotal	Ld. Case	Ld. Comb.	

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-889 ft-lb	16'8 5/8"	10780 ft-lb	0.082 (8%)	1.25D+1.5L	LL
Unbraced	-889 ft-lb	16'8 5/8"	10039 ft-lb	0.089 (9%)	1.25D+1.5L	LL
Pos Moment	1135 ft-lb	7'4"	10780 ft-lb	0.105 (11%)	1.25D+1.5L	L_
Unbraced	1135 ft-lb	7'4"	1139 ft-lb	0.997 (100%)	1.25D+1.5L	L_
Shear	1095 lb	16'8 5/8"	3620 lb	0.302 (30%)	1.25D+1.5L	LL
Perm Defl in.	0.018 (L/11068)	8' 5/16"	0.547 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.047 (L/4149)	8' 5/16"	0.547 (L/360)	0.090 (9%)	L	L_
TL Defl inch	0.065 (L/3018)	8' 5/16"	0.821 (L/240)	0.080 (8%)	D+L	L_

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Tie-down connection required at bearing 3 for uplift 961 lb (Combination 1.25D+1.5L, Load Case L_).
- 5 Top flange must be laterally braced at a maximum of 11'6" o.c.
- 6 Bottom flange must be laterally braced at a maximum of 10'5" o.c.

ſ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 17-6-0	(Span)0-3-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 17-6-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
١	3	Point	16-8-10		Тор	31 lb	0 lb	0 lb	0 lb	Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive
- Handling & Installation
- Librist flanges must not be out or drilled Refer to latest copy of the I, Joist product information details for framing details, stiffener tables, web hote chart, bridging details, multi-ply fastering details and handling/erection details Damaged Joists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths=3.5 inches
 For flat roofs provide proper drainage to prevent

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario L4A 7X4 905-642-4400



ROFESSIONA



Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™ Client: Project: Address:

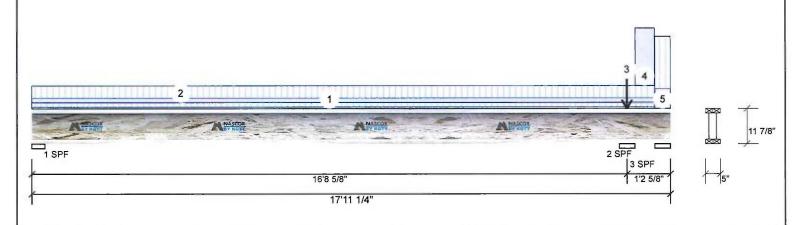
5/30/2018 Date: Designer: SB

Job Name: AMELIA 2 EL- 1

Project #:

11.875" 2-Ply - PASSED F13-B NJH

Level: Second Floor



.. Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Part. Uniform	16-11-6 to 17-5-14		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Tie-In	17-6-0 to 17-11-4	(Span)3-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Dist not to be treated with fire retardant or corrosive

- Handling & Installation

 1. Julist flanges must not be cut or drilled

 2. Refer to latest copy of the Julist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-by fastening details and handling/erection details

 3. Damaged Julists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Manufacturer Info

Nascor by Kott







Client: Project: Address: Date: 5/30/2018 Designer: SB

Job Name: AMELIA 2 EL- 1

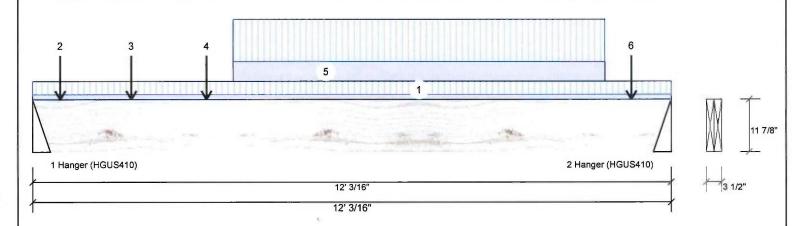
Project #:

1.750" X 11.875" F8-A Forex 2.0E-3000Fb LVL 2-Ply - PASSED

Level: Second Floor

FAGE 30 OF 30

Page 1 of 1



Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored	Reactions	UNPATTERNED	Ib (Uplift)
D		D1	0

Brg	Live	Dead	Snow	vvina
1	1854	816	0	0
2	1824	862	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10792 ft-lb	6' 7/16"	34261 ft-lb	0.315 (31%)	1.25D+1.5L	L
Unbraced	10792 ft-lb	6' 7/16"	28134 ft-lb	0.384 (38%)	1.25D+1.5L	L
Shear	3604 lb	1'3 1/8"	11596 lb	0.311 (31%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/2121)	6' 7/16"	0.383 (L/360)	0.170 (17%)	D	Uniform
LL Defl inch	0.140 (L/983)	6' 1/8"	0.383 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.205 (L/671)	6' 1/4"	0.574 (L/240)	0.360 (36%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	4.000"	37%	1021 / 2780	3801	L	1.25D+1.5L
2 - Hanger	4.000"	37%	1078 / 2736	3814	L	1.25D+1.5L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBI CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

7 Lateral s	slenderness ratio based c	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-0-3	(Span)3-9-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-6-4		Near Face	90 lb	240 lb	0 lb	0 lb	J4
3	Point	1-10-4		Near Face	124 lb	330 lb	0 lb	0 lb	J4
4	Point	3-3-4		Near Face	109 lb	290 lb	0 lb	0 lb	J4
5	Part. Uniform	3-9-4 to 10-9-4		Near Face	113 PLF	240 PLF	0 PLF	0 PLF	
6	Point	11-3-4		Near Face	109 lb	227 lb	0 lb	0 lb	J4
	Self Weight				10 PLF				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product regarding installation requirement
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding





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PROFESSIONA



