GREEN YORK HOMES-GRANELLI HOME CORP-LIANA 2 (ELEV.1&2

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



FAGE I UF 32

Client: Project:

Address:

GREEN YORK HOMES

5/31/2018

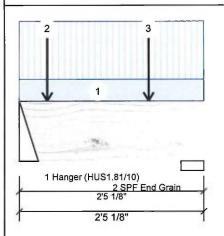
Designer:

Job Name: LIANA 2 (ELEV.1)

Level: Ground Floor

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL



15 PSF

Page 1 of 1

Member Inform	Member Information								
Type:	Girder	Application:	Floor (Residential)						
Plies:	1	Design Method:	LSD						
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012						
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF								

Unfacto	red Reaction	S UNPATTER	NED lb (Uplif	t)	
Brg	Live	Dead	Snow	Wind	
1	299	116	0	0	
2	258	101	0	0	
1					

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	214 ft-lb	1'6 7/8"	11362 ft-lb	0.019 (2%)	1.25D+1.5L	L
Unbraced	214 ft-lb	1'6 7/8"	10729 ft-lb	0.020 (2%)	1.25D+1.5L	L
Shear	438 lb	11 3/4"	4638 lb	0.094 (9%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/58713)	1'4 3/8"	0.067 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/23005)	1'4 9/16"	0.067 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.001 (L/16529)	1'4 7/16"	0.100 (L/240)	0.010 (1%)	D+L	L

Bearings and Factored Reactions

•								
	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
	1 - Hanger	3.000"	15%	145 / 449	594	L	1.25D+1.5L	
	2 - SPF End Grain	3,625"	11%	127 / 387	514	L	1.25D+1.5L	

CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY

NAILING OR BOLTING REQUIREMENTS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

Dead:

A | D | |

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 BC	oπom brace	ed at bearings.	
ID		Load Type	

H						•				
I	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 2-5-2	(Span)3-11-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Point	0-4-6		Far Face	65 lb	175 lb	0 lb	0 lb	J3
l	3	Point	1-8-6		Far Face	72 lb	192 lb	0 lb	0 lb	J3
ŀ		Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA; PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada





Client:

GREEN YORK HOMES

Project: Address:

5/31/2018 Date:

RCO Designer:

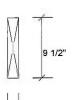
LIANA 2 (ELEV.1) Job Name:

Project #:

F2-A Forex 2.0E-3000Fb LVL

1.750" X 9.500" - PASSED Level: Ground Floor

2 2 SPF End Grain 1 SPF 7'1 1/8"



Page 1 of 1

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Wind

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Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead Snow Live

2	04	40	Ü	Ü

72

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	•
Moment	453 ft-lb	2'11 9/16"	11362 ft-lb	0.040 (4%)	1.25D+1.5L	L	
Unbraced	453 ft-lb	2'11 9/16"	5389 ft-lb	0.084 (8%)	1.25D+1.5L	L	
Shear	228 lb	10 1/2"	4638 lb	0.049 (5%)	1.25D+1.5L	L	
Perm Deff in.	0.004 (L/20045)	3'4 1/8"	0.231 (L/360)	0.020 (2%)	D	Uniform	
LL Defl inch	0.009 (L/9718)	3'3 1/2"	0.231 (L/360)	0.040 (4%)	L	L	
TL Defl inch	0.013 (L/6545)	3'3 11/16"	0.346 (L/240)	0.040 (4%)	D+L	L	

Bearings and Factored Reactions

156

1

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	1.750"	17%	90 / 234	324	L	1.25D+1.5L
2 - SPF End Grain	1.875"	7%	56 / 125	182	L	1.25D+1.5L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 7-1-2	(Span)0-7-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-8-0		Тор	15 PLF	40 PLF	0 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address: 5/31/2018

Designer: RCO

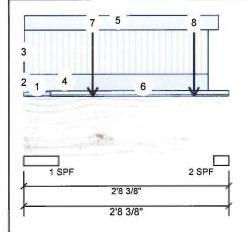
Job Name: LIANA 2 (ELEV.1)

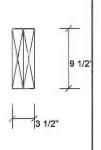
Project #:

F3-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





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Page 1 of 2

Member Illioni	lation
Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Member Information

Application: Design Method:

Building Code:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

Load Sharing: No

Not Checked Vibration: Not Checked

Unfactored	Reactions	UNPATTERNED	lb (Uplift)
15	1.3	D4	C

Brg	Live	Dead	Snow	VVIIIG
1	669	380	0	0
2	523	292	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	475 / 1003	1478	L	1.25D+1.5L
2 - SPF	2.375"	22%	365 / 784	1150	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	607 ft-lb	1'4 1/8"	22724 ft-lb	0.027 (3%)	1.25D+1.5L	L
Unbraced	607 ft-lb	1'4 1/8"	22724 ft-lb	0.027 (3%)	1.25D+1.5L	L
Shear	729 lb	1'2 1/4"	9277 lb	0.079 (8%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/32357)	1'5 1/8"	0.072 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/18072)	1'4 11/16"	0.072 (L/360)	0.020 (2%)	L	L
TL Defl inch	0.002 (L/11598)	1'4 13/16"	0.108 (L/240)	0.020 (2%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID Load Type Location Trib Width Side Dead Live Snow Wind Comments 0-0-0 to 0-4-4 (Span)0-10-7 Top 15 PSF 40 PSF 0 PSF 0 PSF 2 Part. Uniform 0-0-0 to 0-0-2 Top 46 PLF 123 PLF 0 PLF 0 PLF J5 3 Part. Uniform 0-0-0 to 0-0-2 Top 40 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight Part. Uniform 0-0-2 to 2-5-2 Top 92 PLF 246 PLF 0 PLF 0 PLF J5 0 PLF Part. Uniform 0-0-2 to 2-6-12 Top 80 PLF 0 PLF 0 PLF Wall Self Weight

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-pil
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318



Client: Project: Address:

GREEN YORK HOMES

Date: 5/31/2018

Designer: **RCO**

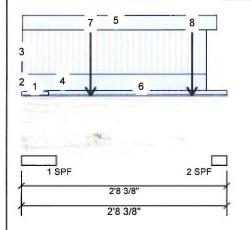
Job Name: LIANA 2 (ELEV.1)

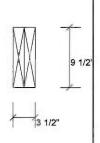
Project #:

F3-A Forex 2.0E-3000Fb LVL 1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor





Page 2 of 2

Continuea	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Tie-In	0-4-4 to 2-8-6	(Span)1-0-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Point	0-10-14		Far Face	120 lb	319 lb	0 lb	0 lb	J5
8	Point	2-2-14		Far Face	83 lb	220 lb	0 lb	0 lb	J5
	Self Weight				8 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvals

Damaged Beams must not be used

Lesign assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA; PR-L318







Member Info	rmation			Unfactor	red Reac	tions U	NPATTERN	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W	Wind
Plies:	2	Design Method:	LSD	1	374		193		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1087		464		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	4.125"	9%	241 / 560	801	L	1.25D+1.5L
				2-SPF	6.875"	15%	580 / 1631	2211	L	1.25D+1.5L

Analysis Results

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	4654 ft-lb	9'8 1/2"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
l	Unbraced	4654 ft-lb	9'8 1/2"	19392 ft-lb	0.240 (24%)	1.25D+1.5L	L
l	Shear	1816 lb	12'5 3/4"	9277 lb	0.196 (20%)	1.25D+1.5L	L
l	Perm Defl in.	0.058 (L/2669)	7'3 7/8"	0.433 (L/360)	0.130 (13%)	D	Uniform
l	LL Defl inch	0.127 (L/1225)	7'5 1/4"	0.433 (L/360)	0.290 (29%)	L	L
l	TL Defl inch	0.186 (L/840)	7'4 13/16"	0.649 (L/240)	0.290 (29%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 9-7-10	(Span)0-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 0-3-0	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Tie-In	0-3-0 to 13-7-15	(Span)0-9-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
4	Part. Uniform	9-8-8 to 13-2-8		Тор	90 PLF	240 PLF	0 PLF	0 PLF		
5	Point	9-8-8		Far Face	116 lb	299 lb	0 lb	0 lb	F1	
	Self Weight				8 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be out or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.

3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318

KOT1





Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

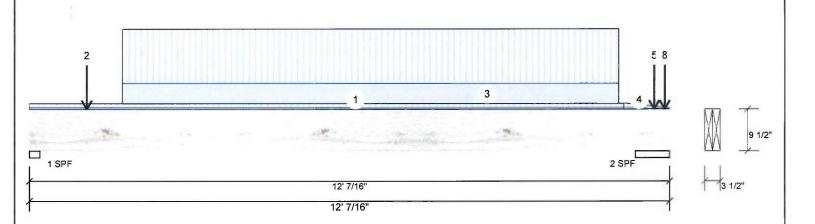
Project #:

Forex 2.0E-3000Fb LVL F4-D

1.750" X 9.500"

2-Ply - PASSED

Level: Ground Floor



Member Inform	nation			Unfacto	red Reac	tions U	NPATTERN	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	2	Design Method:	LSD	1	1545		622		0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	1762		732		0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked	1					
Importance:	Normal	Vibration:	Not Checked	1					
General Load									
Floor Live:	40 PSF			Bearings	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	61%	777 / 2317	3094	L
				2 - SPF	7.754"	21%	915 / 2643	3559	L

Bearings and Factored Reactions										
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.				
1 - SPF	2.375"	61%	777 / 2317	3094	L	1.25D+1.5L				
2 SDE	7 754"	21%	915 / 2643	3559	1	1 25D+1 5I				

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9242 ft-lb	5'9 9/16"	22724 ft-lb	0.407 (41%)	1.25D+1.5L	L
Unbraced	9242 ft-lb	5'9 9/16"	20195 ft-lb	0.458 (46%)	1.25D+1.5L	L
Shear	3271 lb	11 1/8"	9277 lb	0.353 (35%)	1.25D+1.5L	L
Perm Defl in.	0.092 (L/1482)	5'9 9/16"	0.377 (L/360)	0.240 (24%)	D	Uniform
LL Defl inch	0.229 (L/594)	5'9 9/16"	0.377 (L/360)	0.610 (61%)	L	L
TL Defl inch	0.320 (L/424)	5'9 9/16"	0.566 (L/240)	0.570 (57%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind

0

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-2-3	(Span)1-0-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-0-15		Far Face	124 lb	332 lb	0 lb	dl 0	J5
3	Part. Uniform	1-8-15 to 11-0-15		Far Face	101 PLF	270 PLF	0 PLF	0 PLF	
4	Tie-In	11-2-3 to 12-0-7	(Span)1-1-0 to 0-2-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	11-8-15		Far Face	48 lb	128 lb	0 lb	0 lb	J5

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1.

annuling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

4. 5.

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







3 1 SPF 2 SPF 12' 7/16' 12' 7/16"

Dead

16 lb

20 lb

22 lb

8 PLF

Live

34 lb

54 lb

0 lb

Snow

0 lb

0 lb

0 lb

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Wind

0 lb

dl 0 J5

d] 0

Comments

Wall Self Weight

J4

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

.Continued from page 1

Load Type

Self Weight

Point

Point

Point

ID

6

7

8

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply featening details, beam strength values, and code approvals

Location Trib Width

11-11-5

11-11-5

11-11-5

Side

Top

Top

Top

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex

APA: PR-L318



Kott Lumber Company 14 Anderson Blvd, Ontario Canada L4A 7X4 905-642-4400





Page 2 of 2

Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

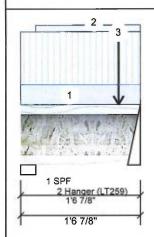
Designer: RCO

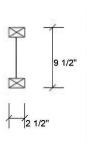
Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F5-A NJH

Level: Ground Floor





Page 1 of 1

Floor (Residential) Application: Design Method: LSD NBCC 2010 / OBC 2012 **Building Code:** Load Sharing: No Deck: Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED lb (Uplift) Live Dead Snow Wind Brg 65 31 0 0 2 135 67 0

Bearings and Factored Reactions Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 9% 39 / 98 136 L 1.25D+1.5L 1 - SPF 2.375" 2.000" 18% 84 / 202 286 L 1.25D+1.5L 2 -Hanger

Analysis Results Location Allowed Analysis Actual Capacity Comb. 52 ft-lb 0.014 (1%) 1.25D+1.5L L 1' 1/8" 3830 ft-lb Moment Unbraced 52 ft-lb 1' 1/8" 3779 ft-lb 0.014 (1%) 1.25D+1.5L L Shear 272 lb 1'5 5/8" 1580 lb 0.172 (17%) 1.25D+1.5L L 11 7/8" 0.044 (L/360) 0.010 (1%) D Perm Defl in. 0.000 Uniform (L/56622) 0.001 11 7/8" 0.044 (L/360) 0.010 (1%) L LL Defl inch L (L/28154) 0.001 11 7/8" 0.067 (L/240) 0.010 (1%) D+L TL Defl inch

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings

(L/18804)

Docton	i nango bracca at bearinge	*							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
3	Point	1-3-7		Near Face	49 lb	99 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- AIRCHING & INSECTION OF THE AIRCHING AND AIR

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

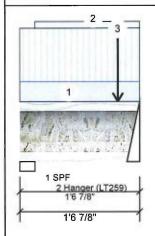
Designer: RCO

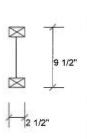
Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F5-B NJH

Level: Ground Floor





Wind

0 0

Page 1 of 1

Member Inform	nation			Unfacto	red React	ions U	INPATTERNI	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N
Plies:	1	Design Method:	LSD	1	63		30		0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	120		59		0
Deflection LL:	360	Load Sharing:	No	100					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing:	s and Fac	tored l	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case
				1 - SPF	2.375"	8%	37 / 94	131	L
nalysis Result	S		2	2 - Hanger	2.000"	16%	74 / 180	254	L

Bearings and Factored Reactions										
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	_			
1 - SPF	2.375"	8%	37 / 94	131	L	1.25D+1.5L				
2 - Hanger	2.000"	16%	74 / 180	254	L	1.25D+1.5L				

Location Allowed Capacity Comb. Case Analysis Actual 0.013 (1%) 1.25D+1.5L L Moment 48 ft-lb 11 11/16" 3830 ft-lb 48 ft-lb 11 11/16" 3779 ft-lb 0.013 (1%) 1.25D+1.5L L Unbraced 0.152 (15%) 1.25D+1.5L L Shear 240 lb 1'5 5/8" 1580 lb Perm Defl in. 0.000 11 7/16" 0.044 (L/360) 0.010 (1%) D Uniform (L/61792) LL Defl inch 0.001 11 1/2" 0.044 (L/360) 0.010 (1%) L (L/30562) TL Defl inch 0.001 11 1/2" 0.067 (L/240) 0.010 (1%) D+L (L/20448)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ſ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Part. Uniform	0-2-6 to 1-6-14		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
l	3	Point	1-3-7		Far Face	40 lb	82 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted atherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Uloist flanges must not be cut or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, siftener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client: Project: **GREEN YORK HOMES**

Address:

Date: 5/31/2018

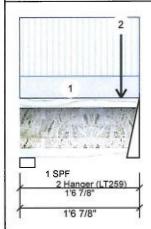
Designer: RCO

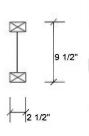
Job Name: LIANA 2 (ELEV.1)

Project #:

NJH 9.500" - PASSED F5-C

Level: Ground Floor





Page 1 of 1

Member Inform	ation
Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code:

Load Sharing:

Deck:

Vibration:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

No Not Checked

Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift) Dead Snow Wind Brg Live 59 22 0 0 0 0 2 115

Bearings and Factored Reactions Cap. React D/L lb Bearing Length

Total Ld. Case Ld. Comb. 1.25D+1.5L 1 - SPF 2.375" 7% 28 / 88 116 L 14% 55 / 173 227 L 1.25D+1.5L 2 -2.000" Hanger

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	39 ft-lb	11"	3830 ft-lb	0.010 (1%)	1.25D+1.5L	L
	Unbraced	39 ft-lb	11"	3779 ft-lb	0.010 (1%)	1.25D+1.5L	L
	Shear	214 lb	1'5 5/8"	1580 lb	0.135 (14%)	1.25D+1.5L	L
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	LL Defl inch	0.000 (L/35261)	10 7/8"	0.044 (L/360)	0.010 (1%)	L	L
	TL Defl inch	0.001 (L/25598)	10 7/8"	0.067 (L/240)	0.010 (1%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	1-4-0		Near Face	28 lb	73 lb	0 lb	0 lb	J2	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Joist flanges must not be cut or drilled Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
- namentaling/erection details
 Damaged Loists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

Designer: **RCO**

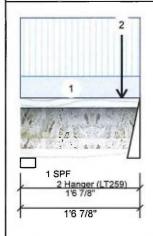
Job Name: LIANA 2 (ELEV.1)

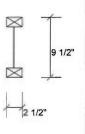
Level: Ground Floor

Project #:

F5-D NJH 9.500" - PASSED







Page 1 of 1

Member Inform	nation	
Туре:	Girder	
Plies:	1	
Moisture Condition:	Dry	
Deflection LL:	360	
Deflection TL:	240	
Importance:	Normal	
General Load		
Floor Live:	40 PSF	

15 PSF

Application: Floor (Residential) Design Method: LSD **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No

Not Checked Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)							
Brg	Live	Dead	Snow	Wind			
1	59	22	0	0			
2	118	44	0	0			

Cap. React D/L lb

28 / 89

55 / 177

Analysis Results Analysis Allowed Actual Location Capacity Comb. 39 ft-lb 11 1/16" 3830 ft-lb 0.010 (1%) 1.25D+1.5L L Moment Unbraced 39 ft-lb 11 1/16" 3779 ft-lb 0.010 (1%) 1.25D+1.5L L 218 lb 0.138 (14%) 1.25D+1.5L L Shear 1'5 5/8" 1580 lb Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 10 15/16" 0.044 (L/360) 0.010 (1%) L (L/34841) 0.001 10 15/16" 0.067 (L/240) 0.010 (1%) D+L L TL Defl inch

Deck:

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

7% 15%

Bearings and Factored Reactions

Bearing Length

1 - SPF 2.375"

Hanger

2.000"

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS



Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Total Ld. Case

116 L

231 L

Design Notes

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.
- 4 Bottom flange braced at bearings.

(L/25376)

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-6-14	(Span)3-2-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-4-0		Far Face	28 lb	76 lb	0 lb	0 lb	J2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Librist fanges must not be cut or drilled Refer to latest copy of the i.Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details Damaged Librist must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes. 1.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

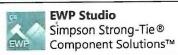
Manufacturer Info

Nascor by Kott









GREEN YORK HOMES Client: Project:

Address:

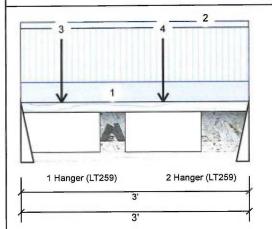
Date: 5/31/2018

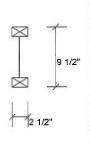
Designer: **RCO** Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED NJH F6-A

Level: Ground Floor





Page 1 of 1

Туре:	Girder
Plies:	1
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF
Dead:	15 PSF

Application: Design Method:

Building Code:

Vibration:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

Load Sharing: Deck:

Not Checked Not Checked Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	274	135	0	0
2	215	105	0	0

Analysis Results

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	420 ft-lb	1'10 7/16"	3830 ft-lb	0.110 (11%)	1.25D+1.5L	L
ı	Unbraced	420 ft-lb	1'10 7/16"	3411 ft-lb	0.123 (12%)	1.25D+1.5L	L
l	Shear	573 lb	1 1/4"	1580 lb	0.363 (36%)	1.25D+1.5L	L
	Perm Defl in.	0.003 (L/12262)	1'10 7/16"	0.093 (L/360)	0.030 (3%)	D	Uniform
ı	LL Defl inch	0.006 (L/6011)	1'10 7/16"	0.093 (L/360)	0.060 (6%)	L	L
l	TL Defl inch	0.008 (L/4033)	1'10 7/16"	0.140 (L/240)	0.060 (6%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	37%	169 / 411	581	L	1.25D+1.5L	
2 - Hanger	2.000"	29%	132 / 323	455	L	1.25D+1.5L	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top flange unbraced.

4 Bottom riange braced at bearings.										
ľ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Part. Uniform	0-0-0 to 3-0-0		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
l	3	Point	0-6-7		Far Face	84 lb	169 lb	0 lb	0 lb	J3
l	4	Point	1-10-7		Far Face	106 lb	217 lb	0 lb	0 lb	JЗ

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 Lioist langes must not be cut or drilled
 Refer to latest copy of the Lioist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multiply fastening details and
 handling/erection details
 Damaged Lioist must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.
- 5 Provide lateral support at bearing points to avoid lateral displacement and rotation 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 Inches 7. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

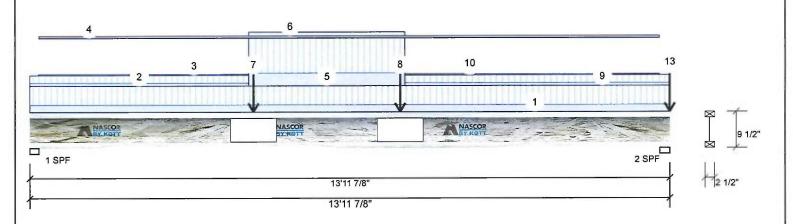
Designer: **RCO**

Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED NJH F7-A

Level: Ground Floor



Member Infor	mation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Conditio	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

		LINIDATTEDNIED	m.	ALL PECA
Unfactored	Keactions	UNPATTERNED	ID	(Uplift)

Brg	Live	Dead	Snow	Wind
1	370	181	0	0
2	533	273	0	0

Bearings and Factored Reactions											
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.					
1 - SPF	2.375"	49%	226 / 555	781	L	1.25D+1.5L					
2 - SPF	2.625"	72%	342 / 800	1141	L	1.25D+1.5L					

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	3313 ft-lb	7'1"	3830 ft-lb	0.865 (87%)	1.25D+1.5L	L
	Unbraced	3313 ft-lb	7'1"	3323 ft-lb	0.997 (100%)	1.25D+1.5L	L
l	Shear	774 lb	1 5/8"	1580 lb	0.490 (49%)	1.25D+1.5L	L
l	Perm Defl in.	0.155 (L/1064)	6'11 3/8"	0.457 (L/360)	0.340 (34%)	D	Uniform
ı	LL Defl inch	0.315 (L/522)	6'11 3/8"	0.457 (L/360)	0.690 (69%)	L	L
l	TL Defl inch	0.470 (L/350)	6'11 3/8"	0.685 (L/240)	0.690 (69%)	D+L	L

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 3' o.c.
- 3 Bottom flange braced at bearings.

[Design Notes										
	TL Defl inch	0.470 (L/350)	6'11 3/8"	0.685 (L/240)	0.690 (69%)	D+L	L				
	LL Defl inch	0.315 (L/522)	6'11 3/8"	0.457 (L/360)	0.690 (69%)	L	L				
	Perm Defl in.	0.155 (L/1064)	6'11 3/8"	0.457 (L/360)	0.340 (34%)	D	Uniform				
	Shear	774 lb	1 5/8"	1580 lb	0.490 (49%)	1.25D+1.5L	L				
	Unbraced	3313 ft-lb	7'1"	3323 ft-lb	0.997 (100%)	1.25D+1.5L	L				

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



o Dottoili	nange braced at bearing	J								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 13-11-14	(Span)0-11-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 4-9-6	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Part. Uniform	0-2-6 to 4-9-6		Тор	1 PLF	0 PLF	0 PLF	0 PLF		
4	Part. Uniform	0-2-6 to 13-9-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF		
5	Tie-In	4-9-6 to 8-2-6	(Span)1-8-11 to 1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
6	Part. Uniform	4-9-6 to 8-2-6		Тор	4 PLF	0 PLF	0 PLF	0 PLF		
7	Point	4-10-10		Near Face	59 lb	120 lb	0 lb	0 lb	F5	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- I. Julist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/arection details.
- Design assumes top flampe to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stifferens for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: Address: **GREEN YORK HOMES**

Date: 5/31/2018

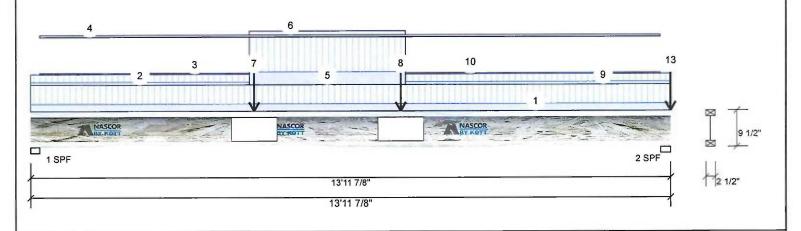
Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

9.500" - PASSED F7-A NJH

Level: Ground Floor



ı	Continued from p	age 1									
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
	8	Point	8-1-2		Near Face	67 lb	135 lb	0 lb	0 lb	F5	
	9	Tie-In	8-2-6 to 13-11-14	(Span)0-4-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
	10	Part. Uniform	8-2-6 to 13-9-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF		
	11	Point	13-11-14		Тор	35 lb	91 lb	0 lb	0 lb	J5	
	12	Point	13-11-14		Тор	41 lb	93 lb	0 lb	0 lb	J5	
	13	Point	13-11-14		Тор	27 lb	0 lb	0 lb	0 lb	Wall Self Weight	

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. Uoist fanges must not be out or drilled

2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, muiti-ply fastening details and handling/erection details

3. Damaged IJoists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address: Date: 5/31/2018

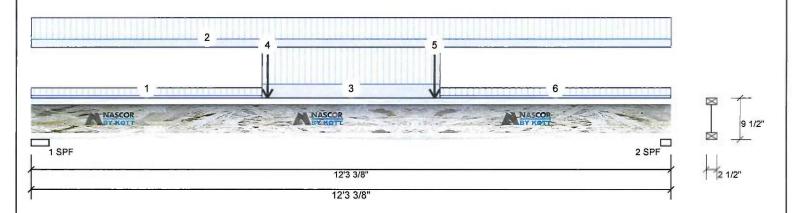
Designer: RCO

Job Name: LIANA 2 (ELEV.1)

Project #:

NJH 9.500" - PASSED F7-B

Level: Ground Floor



ilciniber innonn	idition			Ollia
Type:	Girder	Application:	Floor (Residential)	Brg
Plies:	1	Design Method:	LSD	1
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2
Deflection LL:	360	Load Sharing:	No	
Deflection TL:	240	Deck:	Not Checked	
Importance:	Normal	Vibration:	Not Checked	
General Load				
Floor Live:	40 PSF			Beari
Dead:	15 PSF			Bear
				1 - S

Unfactored Reactions	UNPATTERNED	lb (Uplift)
----------------------	-------------	-------------

Brg	Live	Dead	Snow	Wind
1	331	125	0	0
2	324	122	0	0
1				

Bear	ings	and	Facto	red	Reac	tions	
							,

Bearing	Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.125"	41%	156 / 496	652	L	1.25D+1.5L
2 - SPF	2.375"	40%	152 / 486	638	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2390 ft-lb	6'2 3/4"	3830 ft-lb	0.624 (62%)	1.25D+1.5L	L
Unbraced	2390 ft-lb	6'2 3/4"	2404 ft-lb	0.994 (99%)	1.25D+1.5L	L
Shear	637 lb	3 3/8"	1580 lb	0.403 (40%)	1.25D+1.5L	L
Perm Defl in	. 0.072 (L/1978)	6'2 1/2"	0.395 (L/360)	0.180 (18%)	D	Uniform
LL Defl inch	0.191 (L/744)	6'2 9/16"	0.395 (L/360)	0.480 (48%)	L	L
TL Defl inch	0.263 (L/541)	6'2 9/16"	0.593 (L/240)	0.440 (44%)	D+L	L

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** OINT LOADS OVER BEARINGS



Design Notes

- Girders are designed to be supported on the bottom edge only.
- 2 Top flange must be laterally braced at a maximum of 4'9" o.c.

L	3 Bottom nange	braced at bearings.					I OINT LOAL	2 OAFIC DEV	TAINOS.	
ſ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 4-5-3	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 12-3-6	(Span) 0-11-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Tie-In	4-5-3 to 7-10-3	(Span)1-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	4	Point	4-6-7		Far Face	44 lb	115 lb	0 lb	0 lb	F5
	5	Point	7-8-15		Far Face	44 lb	118 lb	0 lb	0 lb	F5
	6	Tie-In	7-10-3 to 12-3-6	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and lo

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LOUNTY & INStallation

Loist flanges must not be cut or drilled
Refer to latest copy of the Lioist product information
details for framing details, stiffener tables, web hole
chart, bridging details, multi-by fastening details and
handling/erection details
Damaged Loists must not be used
Design assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length= 3,5 linches
 For flat roofs provide proper drainage to prevent populars.

ponding

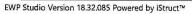
Manufacturer Info

Nascor by Kott









Member Information

Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Project:

GREEN YORK HOMES

Address:

Date: 5/31/2018

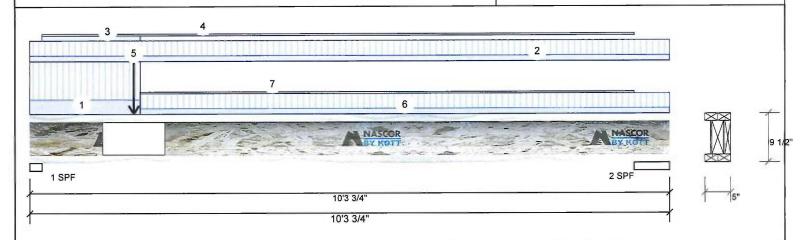
Designer: RCO

LIANA 2 (ELEV.1) Job Name:

Project #:

9.500" 2-Ply - PASSED F8-A NJH

Level: Ground Floor



ACTION THEORY	iution			omac.				(-	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	514		250	0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	325		155	0	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	s and Fac	tored F	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	2.375"	34%	313 / 771	1084 L	1.25D+1.5L
				2-SPF	6.875"	22%	194 / 488	682 L	1.25D+1.5L
Analysis Result	s								

Actual Location Allowed Capacity Comb. Analysis 1756 ft-lb 4'2 9/16" 7660 ft-lb 0.229 (23%) 1.25D+1.5L L Moment 1.25D+1.5L L Unbraced 1756 ft-lb 4'2 9/16" 1759 ft-lb 0.999 (100%)1059 lb 1 5/8" 3160 lb 0.335 (34%) 1.25D+1.5L L Perm Defl in. 0.024 (L/4916) 4'8 11/16" 0.322 (L/360) 0.070 (7%) D

Uniform LL Defl inch 0.048 (L/2404) 4'8 11/16" 0.322 (L/360) 0.150 (15%) L L TL Defl inch 0.072 (L/1614) 4'8 11/16" 0.483 (L/240) 0.150 (15%) D+L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 8'5" o.c.

5 Bottom flange braced at bearings.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED Ib (Uplift)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



3 BULUIII IIa	inge braced at bearing:	S.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-3-12	(Span)1-3-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-6 to 9-8-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-2		Far Face	105 lb	215 lb	0 lb	0 lb	F6
6	Tie-In	1-9-6 to 10-3-12	(Span)1-4-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-6 to 9-8-15		Тор	3 PLF	0 PLF	0 PLF	0 PLF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

- Andling & Installation

 Loist flanges must not be cut or drilled

 Refer to latest copy of the Libist product information details for framing details, steffener tables, web hote chart, bridging details, multi-ply fastening details and handling/erection details

 Damaged Loists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario 14A7X4





Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project:

Address:

GREEN YORK HOMES

5/31/2018 Date:

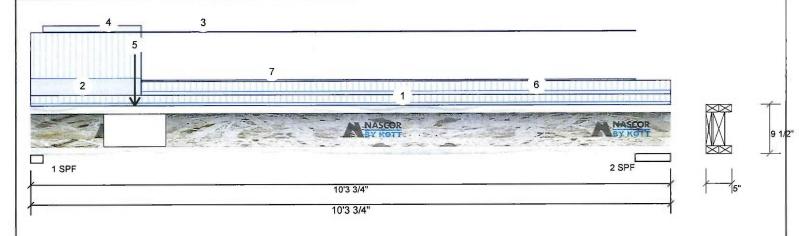
RCO Designer:

Job Name: LIANA 2 (ELEV.1)

Project #

2-Ply - PASSED 9.500" F8-B NJH

Level: Ground Floor



Member Information Type: Girder Plies: 2									
Туре:	Girder								
Plies:	2								
Moisture Condition:	Dry								
Deflection LL:	360								
Deflection TL:	240								
Importance:	Normal								
General Load									
Floor Live:	40 PSF								
Dead:	15 PSF								

Application: Floor (Residential) Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: Deck: Not Checked

Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift) Snow Wind Live Dead

1	451	220	0	0
2	194	93	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	30%	276 / 676	951	L	1.25D+1.5L
2-SPF	6.875"	13%	116 / 291	407	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	•
Moment	1286 ft-lb	3' 1/2"	7660 ft-lb	0.168 (17%)	1.25D+1.5L	L	
Unbraced	1286 ft-lb	3' 1/2"	1290 ft-lb	0.997 (100%)	1.25D+1.5L	L	
Shear	930 lb	1 5/8"	3160 lb	0.294 (29%)	1.25D+1.5L	L	
Perm Defl in.	0.017 (L/6925)	4'6"	0.322 (L/360)	0.050 (5%)	D	Uniform	
LL Defl inch	0.034 (L/3400)	4'6"	0.322 (L/360)	0.110 (11%)	L	L	
TL Defl inch	0.051 (L/2280)	4'6"	0.483 (L/240)	0.110 (11%)	D+L	L	

Vibration:

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 9'7" o.c.

L	5 Bottom flange	braced at bearings.				SECRETARY OF THE SECRETARY				
Г	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
١	1	Tie-In	0-0-0 to 10-3-12	(Span)0-7-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Tie-In	0-0-0 to 1-9-6	(Span)3-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Part. Uniform	0-2-6 to 9-9-0		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	4	Part. Uniform	0-2-6 to 1-9-6		Тор	9 PLF	0 PLF	0 PLF	0 PLF	
l	5	Point	1-8-2		Near Face	135 lb	274 lb	0 lb	0 lb	F6
	6	Tie-In	1-9-6 to 10-3-12	(Span)0-8-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	7	Part. Uniform	1-9-6 to 9-9-0		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Handling & Installation

 1. IJoist flanges must not be cut or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 1. Damaged IJoists must not be used

 2. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott







Page 1 of 1

EWP Studio Simpson Strong-Tie® Component Solutions™

Client: Project: **GREEN YORK HOMES**

Address:

Date: 5/31/2018

Designer: RCO

Job Name: LIANA 2 (ELEV.1)

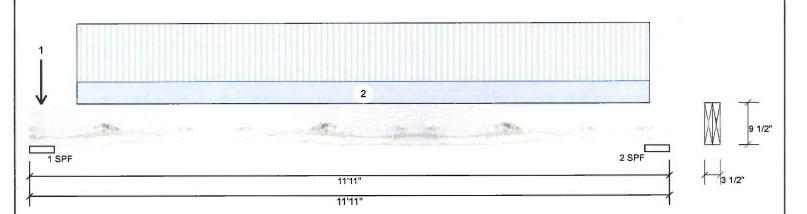
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member	Information
Type:	Girder

Plies: 2 Moisture Condition: Dry Deflection LL: Deflection TL: 240 Importance: Normal General Load

40 PSF 15 PSF

Application: Design Method:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

Building Code: Load Sharing: Deck:

Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1378	565	0	0
2	1362	559	0	0
1				

Bearings and Factored Reactions

Bearing L	_ength	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 5	5.500"	23%	706 / 2068	2774	L	1.25D+1.5L
2-SPF 5	5.500"	23%	699 / 2043	2742	L	1.25D+1.5L

Analysis Results

Floor Live: Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7560 ft-lb	5'11 5/8"	22724 ft-lb	0.333 (33%)	1.25D+1.5L	L
Unbraced	7560 ft-lb	5'11 5/8"	20280 ft-lb	0.373 (37%)	1.25D+1.5L	L
Shear	2345 lb	1'2 1/4"	9277 lb	0.253 (25%)	1.25D+1.5L	L
Perm Defl in.	0.074 (L/1812)	5'11 9/16"	0.371 (L/360)	0.200 (20%)	D	Uniform
LL Defl inch	0.180 (᠘740)	5'11 9/16"	0.371 (L/360)	0.490 (49%)	L	L
TL Defl inch	0.254 (L/525)	5'11 9/16"	0.556 (L/240)	0.460 (46%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



/ Late	Lateral significantess fatio based on full section width.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Point	0-2-10		Тор	52 lb	138 lb	0 lb	0 lb	J5	
2	Part. Uniform	0-10-10 to 11-6-10		Тор	92 PLF	244 PLF	0 PLF	0 PLF		
	Self Weight				8 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
 - approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client: Project:

Address:

GREEN YORK HOMES

5/31/2018

RCO

Designer:

Job Name: LIANA 2 (ELEV.1)

Project #:

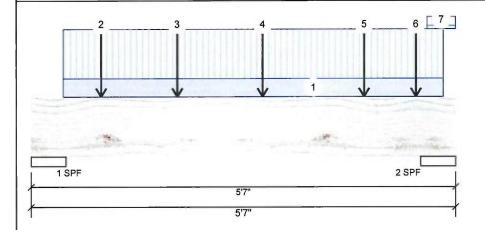
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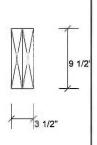
Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor





Page 1 of 2

Member Information

Type: Girder Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance:

Normal 40 PSF

15 PSF

Application: Design Method:

Floor (Residential)

NBCC 2010 / OBC 2012 **Building Code:** No

Load Sharing: Deck:

Not Checked Not Checked Vibration:

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	vvina
1	1315	519	0	0
2	1964	809	0	0
1				

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	5.500"	22%	649 / 1973	2622	L	1.25D+1.5L	
2 - SPF	5.500"	33%	1012 / 2947	3959	L	1.25D+1.5L	

Analysis Results

General Load

Floor Live: Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	3183 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L	
Unbraced	3183 ft-lb	3' 9/16"	22724 ft-lb	0.140 (14%)	1.25D+1.5L	L	
Shear	3768 lb	4'4 3/4"	9277 lb	0.406 (41%)	1.25D+1.5L	L	
Perm Defl in.	0.007 (L/7855)	2'10 1/8"	0.160 (L/360)	0.050 (5%)	D	Uniform	
LL Defl inch	0.019 (L/3082)	2'10 1/8"	0.160 (L/360)	0.120 (12%)	L	L	
TL Defl inch	0.026 (L/2214)	2'10 1/8"	0.240 (L/240)	0.110 (11%)	D+L	L	

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



o Lateral Sieria	cilicos fatio basca off i	an occuon wiati.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-5-1 to 5-5-1		Far Face	104 PLF	276 PLF	0 PLF	0 PLF	
2	Point	0-11-1		Near Face	112 lb	292 lb	0 lb	0 lb	J5
3	Point	1-11-1		Near Face	105 lb	281 lb	0 lb	0 lb	J5
4	Point	3-0-9		Near Face	123 lb	327 lb	0 lb	0 lb	J5
5	Point	4-4-9		Near Face	100 lb	268 lb	0 lb	0 lb	J5
6	Point	5-0-12		Near Face	319 lb	713 lb	0 lb	0 lb	F4

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVI beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318





Client: Project:

Address:

GREEN YORK HOMES

Date:

5/31/2018

RCO Designer:

Job Name: LIANA 2 (ELEV.1)

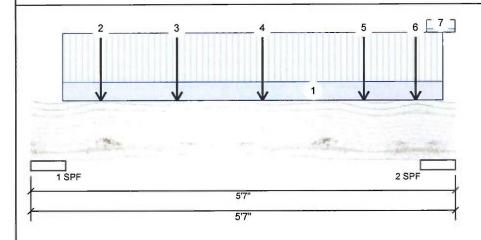
Project #:

Forex 2.0E-3000Fb LVL F10-A

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Page 2 of 2

.Continued from page 1

ID Load Type 7 Tie-In

Location Trib Width Side 5-2-8 to 5-7-0 (Span)2-6-3

Dead 15 PSF

8 PLF

Live 40 PSF Snow 0 PSF

Wind Comments 0 PSF

Self Weight

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals
- Handling & Installation
- and ling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirementes, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Page 1 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

Client:

GREEN YORK HOMES

Project: Address:

5/31/2018 Date:

RCO Designer:

LIANA 2 (ELEV.1) Job Name:

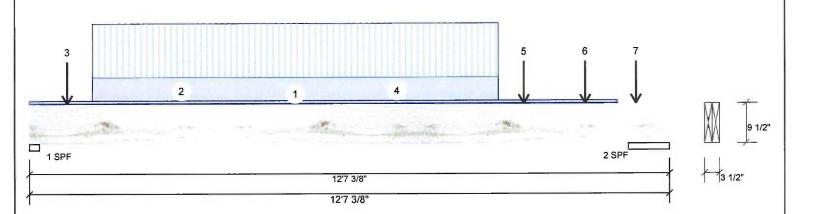
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Member Inform	nation			Unfacto	red React	tions U	INPATTERNI	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	
Plies:	2	Design Method:	LSD	1	1567		722		0	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	1790		804		0	
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load										_
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ī
				1 - SPF	2.375"	64%	902 / 2351	3253	L	
				2-SPF	9.714"	18%	1005 / 2685	3690	L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9822 ft-lb	6'	22724 ft-lb	0.432 (43%)	1.25D+1.5L	L
Unbraced	9822 ft-lb	6'	20006 ft-lb	0.491 (49%)	1.25D+1.5L	L
Shear	3223 lb	11 1/8"	9277 lb	0.347 (35%)	1.25D+1.5L	L
Perm Defl in.	0.115 (L/1224)	6' 1/16"	0.391 (L/360)	0.290 (29%)	D	Uniform
LL Defl inch	0.251 (L/561)	6'	0.391 (L/360)	0.640 (64%)	L	L
TL Defl inch	0.366 (L/385)	6' 1/16"	0.587 (L/240)	0.620 (62%)	D+L	L

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Wind 0 0

Ld. Comb. 1.25D+1.5L 1.25D+1.5L

o Lateral	sienderness ratio based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 11-7-2	(Span)0-6-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-2-7 to 11-1-5		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-8-15		Far Face	104 lb	238 lb	0 ال	0 lb	J5
4	Part. Uniform	1-2-15 to 9-2-15		Far Face	114 PLF	266 PLF	0 PLF	0 PLF	
5	Point	9-8-15		Far Face	128 lb	294 lb	0 lb	0 lb	J5
6	Point	10-11-7		Far Face	123 lb	292 lb	0 lb	0 lb	J5
ontinued or	n page 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & installation:
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-pily
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA; PR-L318





Page 2 of 2

EWP Studio Simpson Strong-Tie® Component Solutions™

GREEN YORK HOMES Client: Project:

Location Trib Width

11-11-7

Side

Far Face

Address:

5/31/2018

RCO

Designer: Job Name: LIANA 2 (ELEV.1)

Project #:

Date:

Forex 2.0E-3000Fb LVL

Load Type

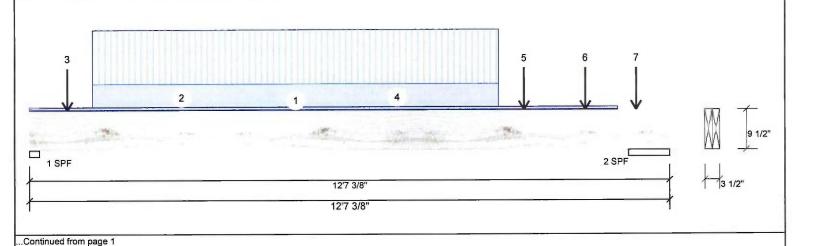
Self Weight

Point

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



Dead

105 lb

8 PLF

Live

281 lb

Snow

0 lb

Wind

0 lb J5

Comments

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

ID

7

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-pil
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Project: Address: **GREEN YORK HOMES**

Client:

Date: 5/31/2018

Designer:

Job Name: LIANA 2 (ELEV.1)

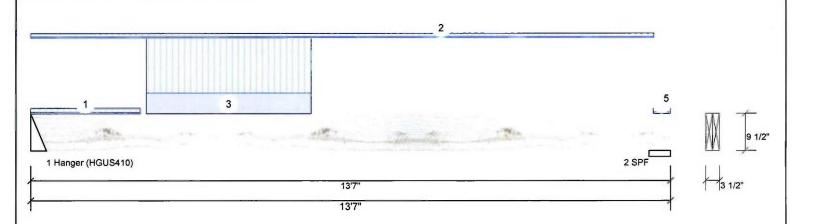
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 9.500"

2-Ply - PASSED

Level: Second Floor



MEHIDEL HITOTH	lation				Ulliacto	reu neac	uons o	MENTICKIN	ן מו ע	Opinity		
Туре:	Girder		Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind	
Plies:	2		Design Method:	LSD	1	713		319		0	0	
Moisture Condition:	Dry		Building Code:	NBCC 2010 / OBC 2012	2	357		186		0	0	
Deflection LL:	360		Load Sharing:	No								
Deflection TL:	240		Deck:	Not Checked								
Importance:	Normal		Vibration:	Not Checked								
General Load												_
Floor Live:	40 PSF				Bearings	and Fac	tored F	Reactions				
Dead:	15 PSF				Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
					1 - Hanger	4.000"	14%	398 / 1070	1468	L	1.25D+1.5L	
Analysis Result	s				2 - SPF	5.500"	6%	233 / 535	768	L	1.25D+1.5L	
Analysis Act	ual	Location A	llowed Capac	ity Comb. Case				-				

Actual	Location	Allowed	Capacity	Comb.	Case
4658 ft-lb	5' 1/8"	22724 ft-lb	0.205 (20%)	1.25D+1.5L	L
4658 ft-lb	5' 1/8"	19429 ft-lb	0.240 (24%)	1.25D+1.5L	L
1393 lb	1' 3/4"	9277 lb	0.150 (15%)	1.25D+1.5L	L
0.058 (L/2674)	6'2 5/8"	0.431 (L/360)	0.130 (13%)	D	Uniform
0.128 (L/1212)	6'1 5/16"	0.431 (L/360)	0.300 (30%)	L	L
0.186 (L/834)	6'1 3/4"	0.646 (L/240)	0.290 (29%)	D+L	L
	4658 ft-lb 4658 ft-lb 1393 lb 0.058 (L/2674) 0.128 (L/1212)	4658 ft-lb 5' 1/8" 4658 ft-lb 5' 1/8" 1393 lb 1' 3/4" 0.058 (L/2674) 6'2 5/8" 0.128 (L/1212) 6'1 5/16"	4658 ft-lb 5' 1/8" 22724 ft-lb 4658 ft-lb 5' 1/8" 19429 ft-lb 1393 lb 1' 3/4" 9277 lb 0.058 (L/2674) 6'2 5/8" 0.431 (L/360) 0.128 (L/1212) 6'1 5/16" 0.431 (L/360)	4658 ft-lb 5' 1/8" 22724 ft-lb 0.205 (20%) 4658 ft-lb 5' 1/8" 19429 ft-lb 0.240 (24%) 1393 lb 1' 3/4" 9277 lb 0.150 (15%) 0.058 (L/2674) 6'2 5/8" 0.431 (L/360) 0.130 (13%) 0.128 (L/1212) 6'1 5/16" 0.431 (L/360) 0.300 (30%)	4658 ft-lb 5' 1/8" 22724 ft-lb 0.205 (20%) 1.25D+1.5L 4658 ft-lb 5' 1/8" 19429 ft-lb 0.240 (24%) 1.25D+1.5L 1393 lb 1' 3/4" 9277 lb 0.150 (15%) 1.25D+1.5L 0.058 (L/2674) 6'2 5/8" 0.431 (L/360) 0.130 (13%) D 0.128 (L/1212) 6'1 5/16" 0.431 (L/360) 0.300 (30%) L

Design Notes

1 Fill all hanger nailing holes.

Member Information

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Unfactored Reactions UNPATTERNED Ib (Unlift)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Page 1 of 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 2-3-14	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 13-2-12	(Span)0-8-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Part. Uniform	2-5-7 to 5-11-7		Тор	90 PLF	240 PLF	0 PLF	0 PLF		
4	Tie-In	13-2-10 to 13-7-0	(Span)0-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
5	Tie-In	13-2-12 to 13-7-0	(Span)0-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
	Self Weight				8 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be out or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA; PR-L318





Client: Project: Address: **GREEN YORK HOMES**

5/31/2018

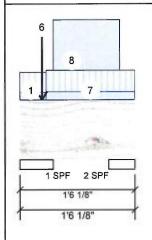
Designer:

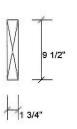
LIANA 2 (ELEV.1) Job Name:

Project #:

Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED F9-A

Level: Second Floor





Page 1 of 2

Member Inform	nation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	1	Design Method:	LSD	1	283		457	56	6	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	20		43		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	5.250"	37%	571 / 991	1561	L	1.25D+1.5S +0.5L
Analysis Result	ts			2 - SPF	4.125"	2%	53 / 29	83	L	1.25D+1.5L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	10 ft-lb	9 5/8"	7385 ft-lb	0.001 (0%)	1.4D	Uniform	
Unbraced	10 ft-lb	9 5/8"	7385 ft-lb	0.001 (0%)	1.4D	Uniform	
Shear	40 lb	5 1/4"	3015 lb	0.013 (1%)	1.4D	Uniform	
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

+ DOLLOIII	bracea at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 0-4-2	(Span)1-3-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Point	0-3-8		Тор	10 lb	0 lb	23 lb	0 lb		
3	Point	0-3-8		Тор	18 lb	0 lb	0 lb	0 lb	Wall Self Weight	
4	Point	0-3-8		Тор	386 lb	262 lb	537 lb	0 lb	F12 F12	
5	Point	0-3-8		Тор	2 lb	0 lb	6 lb	0 lb		
6	Point	0-3-8		Тор	6 lb	0 lb	0 lb	0 lb	Wall Self Weight	
7	Tie-In	0-4-2 to 1-6-2	(Span)1-4-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
ontinued or	n page 2									

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
festening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318







Client:

Address:

GREEN YORK HOMES

Project:

Date: 5/31/2018

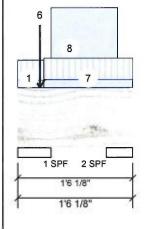
RCO Designer:

Job Name: LIANA 2 (ELEV.1)

Project #:

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F9-A

Level: Second Floor



Page 2 of 2

.Continued from page 1

ID Load Type 8 Part. Uniform

Location Trib Width 0-5-4 to 1-3-12

Side Тор

Dead 64 PLF

4 PLF

Live 0 PLF Snow 0 PLF Comments

0 PLF Wall Self Weight

Self Weight

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the clintended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals Handling & Installation
- andling & installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally reetrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Client:

GREEN YORK HOMES

Project: Address:

5/31/2018 Date:

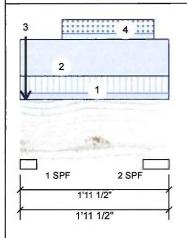
Designer: RCO

Job Name: LIANA 2 (ELEV.2)

Project #

1.750" X 9.500" - PASSED Forex 2.0E-3000Fb LVL F9-A

_evel: Second Floor



Wind

Page 1 of 1

Member Inform	ation	
Туре:	Girder	
Plies:	1	
Moisture Condition:	Dry	
Deflection LL:	360	
Deflection TL:	240	
importance:	Normal	
General Load		
Floor Live:	40 PSF	

15 PSF

Application: Design Method:

Building Code:

Load Sharing:

Floor (Residential) LSD

NBCC 2010 / OBC 2012

No

Not Checked Deck: Not Checked Vibration:

Unfactored	Reactions	UNPATTERNED	lb (Uplift)
Brg	Live	Dead	Snow

1	184	324	365	0
2	31	90	19	0

Bearings and Factored Reactions

_							
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	2.625"	41%	406 / 639	1045	L	1.25D+1.5S +0.5L	
2 - SPF	4.125"	4%	126 / 0	126	Uniform	1.4D	

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	35 ft-lb	11 1/16"	7385 ft-lb	0.005 (0%)	1.4D	Uniform
Unbraced	35 ft-lb	11 1/16"	7285 ft-lb	0.005 (0%)	1.4D	Uniform
Shear	8 lb	10 5/8"	4638 lb	0.002 (0%)	1.25D+1.5S +0.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 Bottom B	raced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-11-8	(Span)1-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 1-11-8		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-0-14		Тор	248 lb	157 lb	355 lb	0 lb	F11 F11
4	Part. Uniform	0-6-10 to 1-9-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF	
	Self Weight				4 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code.

papprovals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex







Client:

GREEN YORK HOMES

Project: Address:

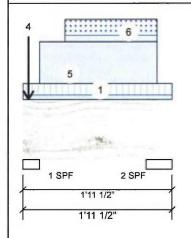
5/31/2018 Date:

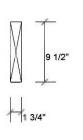
RCO Designer:

LIANA 2 (ELEV.2) Job Name:

Project #:

1.750" X 9.500" - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL F9-B





Wind

Page 1 of 1

Member Inform	rintormation						
Type:	Girder						
Plies:	1						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						

40 PSF

15 PSF

Application: Design Method:

Building Code: Load Sharing:

Floor (Residential)

NBCC 2010 / OBC 2012

Deck: Not Checked Vibration: Not Checked

Unfactored	Reactions	UNPATTERNED	lb (Uplift)
Brg	Live	Dead	Snow

1	173	318	370	0
1 2	18	73	19	0
100				
ļ				

Analysis Posults

General Load

Floor Live: Dead:

Allalysis in	larysis itesures							
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case		
Moment	34 ft-lb	11 1/16"	7385 ft-lb	0.005 (0%)	1.4D	Uniform		
Unbraced	34 ft-lb	11 1/16"	7285 ft-lb	0.005 (0%)	1.4D	Uniform		
Shear	7 lb	10 5/8"	4638 lb	0.002 (0%)	1.25D+1.5S +0.5L	L		

Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) **Bearings and Factored Reactions**

caring.	and rac	.torea i	teactions				
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	П
1 - SPF	2.625"	41%	398 / 642	1040	L	1.25D+1.5S +0.5L	
2-SPF	4.125"	4%	102 / 0	102	Uniform	1.4D	

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL **POINT LOADS OVER BEARINGS**



Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

ı	4 Bottom braced at bearings.										
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
١	1	Tie-In	0-0-0 to 1-11-8	(Span)0-10-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
I	2	Point	0-0-14		Тор	250 lb	157 lb	360 lb	0 lb	F11 F11	
I	3	Point	0-0-14		Тор	5 lb	0 lb	0 lb	0 lb	Wall Self Weight	
I	4	Point	0-0-14		Тор	5 lb	0 lb	0 lb	di 0	Wall Self Weight	
I	5	Part. Uniform	0-2-10 to 1-9-2		Тор	64 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
I	6	Part. Uniform	0-6-10 to 1-9-2		Тор	10 PLF	0 PLF	24 PLF	0 PLF		
I		Self Weight				4 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the cimended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening odetalls, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318







Layout Name

Design Method

Description

Created

Builder

Sales Rep

Designer

Shipping

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

D:\Users\rochavillo\WORK FROM

\GRANELLI HOME CORP\MODELS

LSD

2012

40

15

480

360

480

360

360

240

480

360

OSB

3/4"

Nailed & Glued

LIANA 2\LIANA 2 ELEV 1\FLOOR

Building Code NBCC 2010 / OBC

HOME\GREEN YORK HOMES

Project

Canada

L4A 7X4

Job Path

905-642-4400

Ground Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor

Loads

Live

Dead

RCO

May 29, 2018

LSD

LIANA 2 (ELEV.1&2)

GRANELLI HOMES CORP. BRAMPTON, ONT.

GREEN YORK HOMES

Ground Floor (2x 8 Fr F2-A -_J3-C= .16-C J6-B J8-D 11111 ► F7-A - 1 ply

THIS CERTIFICATION IS TO CONFIRM THAT:

- 1. THE LOADS USED IN THE CALCULATION OF THE ATTACHED APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT.
- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO THE NASCOR SPECIFIER GUIDE. MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE MEMBER CONNECTION DETAIL.

ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS. COLUMNS AND FOUNDATION WALLS AND FOOTINGS INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS







Load from Above Wall Wall Opening Norbord Rimboard Plus 1,125 X 9.5 NJH 9.5 Forex 2.0E-3000Fb LVL 1.75 X 9.5

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Engineered floor joists shall be installed in accordance with the supplier's layout and

specifications forming part of the permit drawings.

All work shall conform to the Model Higher 2

F7 NJH 2.5 9.5 2 14-0-0 F8 NJH 2.5 9.5 4 12-0-0 F6 NJH 2.5 9.5 1 4-0-0 F5 NJH 2.5 9.5 4 2-0-0 J6 NJH 2.5 9.5 11 16-0-0 J5 NJH 2.5 9.5 20 14-0-0 J4 NJH 2.5 9.5 8 12-0-0 J3 NJH 2.5 9.5 4 10-0-0 J2 NJH 2.5 9.5 4 6-0-0 J1 NJH 2.5 9.5 4-0-0 1 Rim Board Label Description Width Depth Plies Qty Pcs Length Norbord Rimboard 1.125 13 Plus 1.125 X 9.5 Blocking Label Description Width Depth Qty Plies Pcs Length BLK1 NJH 2.5 9.5 LinFt Varies 29-0-0 Hanger Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners Unknown Hanger H2 25 LT259 4 10dx1 1/2 2 10dx1 1/2 1 HUS1.81/10 30 16d 10 16d

Width Depth

9.5

9.5

9.5

1.75

1.75

1.75

1.75

Width | Depth

Plies

2

Plies

Qty

Qty

Pcs Length

Pcs Length

2

8-0-0

4-0-0

4-0-0

NOTES:

Ground Floor LVL/LSL (Flush)

F2

F3

F1

Label Description

Forex

Forex

Forex

Forex

Label Description

Joist (Flush)

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

- . Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- I. Install single-ply flush window header along inside face of rimboard/rimjoist
- . Refer to Nascor specifier guide for installation works.
- Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or mof
- Load transfer blocks to be installed under all point loads.
- It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or boltina requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and pracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load

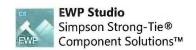
The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.1; May 22,2018 Project No: 17-55

Building Code O. Reg. 332/12 as amended

M-2057



19-444468.000.00RR.





Pcs Length

14-0-0

2

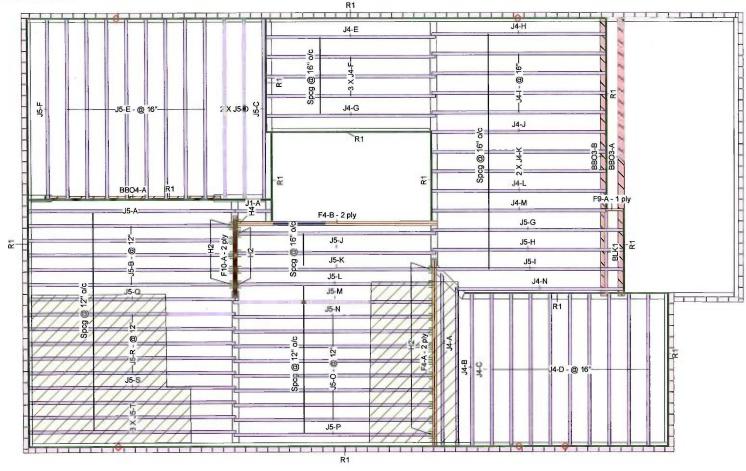
4 10dx1 1/2

46 16d

2 10dx1 1/2

16 16d

Second Floor



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- 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE FOR THE LOADS AND SPACING SHOWN ON THIS LAYOUT.

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.



Load from Above Wall Norbord Rimboard Plus 1.125 X 9.5 NJH 9.5 Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped)

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- LVL CCMC -14056-R



Legend



Forex 2.0E-3000Fb LVL 1.75 X 9.5

- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

F10	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	6-0-0] -
F9	Forex 2.0E-3000Fb LVL	1.75	9.5			1	2-0-0	
LVL/LS	L (Dropped)							ŀ
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	
BBO4	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	12-0-0	
I Joist ((Flush)							10
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	1
J5	NJH	2.5	9.5			45	14-0-0	E
J4	NJH	2.5	9.5			28	12-0-0	1
J1	NJH	2.5	9.5			1	4-0-0	15
Rim Bo	oard] 3
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	L
R1	Norbord Rimboard Plus 1.125 X 9.5	1.125	9.5			17	12	
Blockin	ng				V			S
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	F
BLK1	NJH	2.5	9.5	LinFt		Varies	5-0-0	E
Hanger	•							먇
				Bea	am/Girde		oported ember	
Label	Pcs Description	tion Skew Slope fasteners faste						1

Width Depth Qty Plies

2

9.5

1.75

H4 NOTES:

H2

Second Floor LVL/LSL (Flush) Label Description

Forex

2.0E-3000Fb LVL

F4

- Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger
- Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
- 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- 5. Refer to Nascor specifier guide for installation works.

21 LT259

1 HGUS410

- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads.
- 8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an addtional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr., Suite 3A, Vaughan, ON Date: Rev.1; May 22,2018 Project No: 17-55 Model: Liana 2

Layout Name LIANA 2 (ELEV.1) Design Method

LSD

Description GRANELLI HOMES CORP. BRAMPTON, ONT.

Created May 29, 2018

Builder **GREEN YORK HOMES**

Sales Rep Designer

RCO Shipping

Project Builder's Project

Kott Lumber Company 14 Anderson Blvd Stouffville, Ontario

L4A 7X4 905-642-4400

Job Path

Canada

D:\Llsers\rochavillo\WORK FROM HOME\GREEN YORK HOMES **\GRANELLI HOME CORP\MODELS** \LIANA 2\LIANA 2 ELEV 1\FLOOR

Second Floor Design Method

LSD Building Code NBCC 2010 / OBC 2012

40

15

480

360

480

360

360

240

480

360

OSB

Loads Live Dead

Deflection Joist LL Span L/ TL Span L/ LL Cant 2L/ TL Cant 2L/

Deflection Girder LL Span L/ TL Span L/

> LL Cant 2L/ TL Cant 2L/

> Decking Deck

Thickness 5/8" Fastener Nailed & Glued Vibration

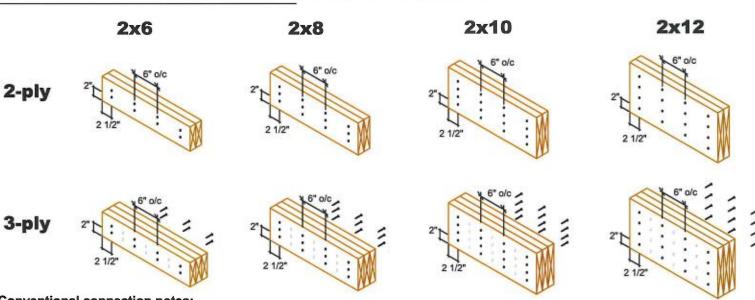
Ceiling: Gypsum 1/2"



TIPLE MEMBER CONNECTIONS

HOME CORP-LIANA 2 (ELEV.1&2

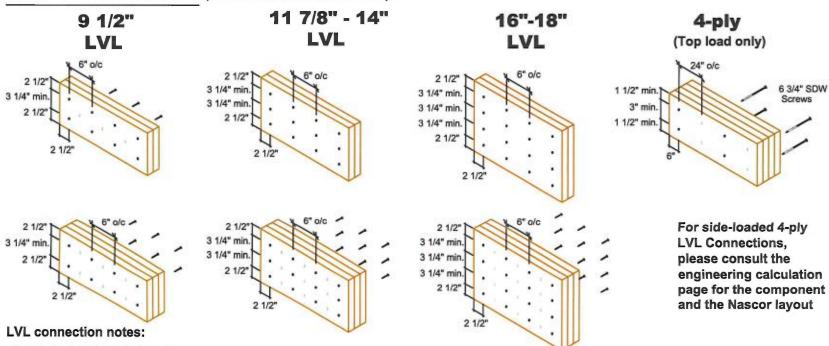
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

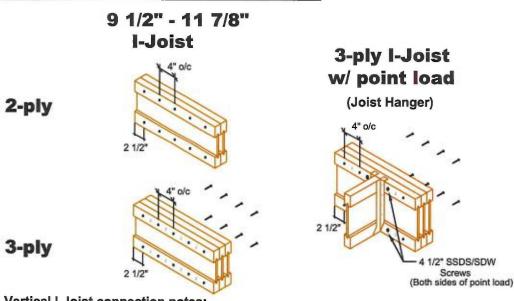
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.



MULTI-PLY CONNECTION DETAILS

Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751